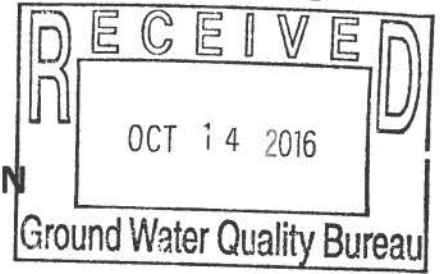


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NEW MEXICO COPPER CORPORATION

NEW MINE PERMIT No. SI027RN

UPDATED MINING OPERATION AND RECLAMATION PLAN

FOR ITS

COPPER FLAT MINE

SUBMITTED TO

NEW MEXICO MINING & MINERALS DIVISION PURSUANT TO

19.10.6.602.D.(15) and 19.10.6.603 NMAC

OCTOBER 2016

PREPARED BY



Velasquez Environmental Management Services Inc.



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- Appendix B: Impoundment Design Report, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corp. November, 2015
- Appendix C: Process Facility Containment Report, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corp., November, 2015
- Appendix D: Site Diversion Analysis, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corporation, December 2015 as Revised, June, 2016
- Appendix E: Mine Reclamation and Closure Plan, Copper Flat Mine, Golder Associates Inc., October, 2016



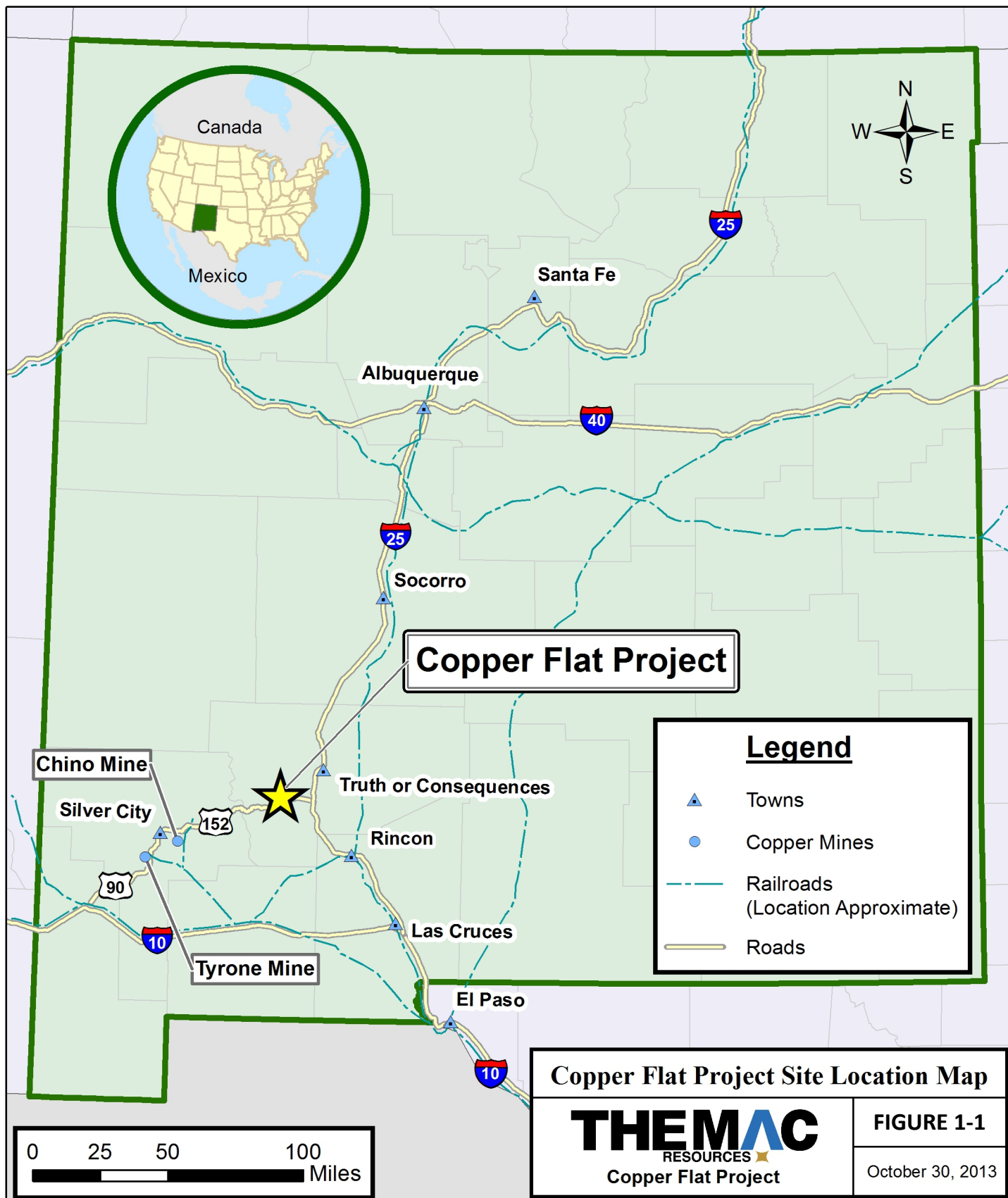
1.0 INTRODUCTION

New Mexico Copper Corporation (NMCC) is developing the Copper Flat Mine located approximately 150 miles south of Albuquerque, New Mexico and 20 miles southwest of Truth or Consequences, NM, north of NM State Highway 152 between the communities of Caballo to the east and Hillsboro to the west in Sierra County, as shown on Figure 1-1. NMCC has proposed to mine approximately 125 million tons of copper ore. Over the life of the mine, it will produce approximately 113 million tons of ore and 33 million tons of waste rock. The mine life for the current reserve is estimated at 11 to 12 years.

On July 18, 2012 NMCC submitted its Permit Application Package (PAP) to the New Mexico Mining and Minerals Division (MMD), including the Mining Operation and Reclamation Plan (MORP) for its Copper Flat Mine Project in Sierra County, NM. On February 18, 2013 the MMD provided comments on NMCC's PAP, including NMCC's Baseline Data Report (BDR) and the MORP. In the intervening time NMCC and MMD have resolved all of MMD's comments related to the BDR with the exception of a determination of the hydrologic consequences of the operation and reclamation on the permit and affected areas as required by 19.10.6.602.D.(13)(g)(v) of the Mining Act regulations. NMCC will submit a revised hydrologic consequences analysis in response to MMD's request under separate cover as an addendum to the BDR when it becomes available.

This document updates NMCC's MORP submittal of July 18, 2012, in particular with respect to Sections 3.15 through 4.8 of the 2012 MORP document. This update addresses proposed mine operations and reclamation and closure of the Copper Flat Project and provides the most recent information available consistent with the information contained in NMCC's Discharge Permit (DP) application submitted to the New Mexico Environment Department (NMED) in December 2015 and revised in June 2016, currently undergoing technical review. The information herein is also consistent with the information contained in the Bureau of Land Management's (BLM) draft Environmental Impact Statement (DEIS) published for public comment in November 2015, in particular, with regard to Alternative 2 as described in the DEIS and designated by the BLM as the preferred alternative. This document also takes into account MMD's February 18, 2013 comments and provides more detailed information that may resolve or render a specific comment moot.

This MORP update is organized in a manner that allows the reviewer to compare subsection of 19.10.6.602.D.(15), Description of the Proposed Mine Operation and Reclamation Plan, and 19.10.6.603, Performance and Reclamation Standards and Requirements, of the Mining Act regulations to see how NMCC proposed operations and reclamation of Copper Flat will comply with MMD's Mining Act regulations. Section 2.0 presents NMCC's Mine Operation Plan for Copper Flat and Section 3.0 presents its Reclamation and Closure Plan. The contents of this



Copper Flat Project Site Location Map

THEMAC
 RESOURCES
 Copper Flat Project

FIGURE 1-1

October 30, 2013



MORP update are also supported by the appended documents included herewith as well as the other documents included as part of the MORP update by reference herein.

As indicated above, NMCC submitted its DP application to the NMED for review and approval. The DP application was determined administratively complete in December 2015 and is currently undergoing technical review. NMED provided a set comments and a request for additional information in March 2016. NMCC submitted the requested information in June 2016, including a commitment to submit the information contained in this document in response to NMED's request for a more detailed Closure Plan. This MORP update is consistent with and includes much of the same information as was provided to NMED in its DP application. NMCC considers that the Reclamation Plan as contained herein is also the Closure Plan required by the NMED Copper Rules. The DP application as revised in June 2016 is, therefore, included in this MORP by reference.

NMED's requirements for ground water protection during operation of Copper Flat and subsequent reclamation at the end of the project are tied to each other through the New Mexico Mining Act regulations and the New Mexico Water Quality Control Act Copper Rules (NMED Copper Rules) regulatory approval process for the mine. The objective of the Reclamation and Closure Plan is to reclaim and close the facility in a manner protective of ground water in conformance with the NM Copper Rules, meet the reclamation requirements of the New Mining Act and return the mine area to conditions similar to those present before reestablishment of the mine. Reclamation of the site will re-establish the post-mining land uses consistent with the land uses of the site and the surrounding area, i.e., wildlife habitat, grazing, mining and recreation as identified by the Bureau of Land Management in its approved Land Use Management Plan (BLM 1986).



2.0 MINE OPERATIONS PLAN - 19.10.6.602.D.(15)

This Section provides a detailed description of the NMCC's proposed construction and operation of the Copper Flat Project. It is organized to provide the information requested in Sections 19.10.602.D.(15)(a) through (f) of the New Mexico Mining Act regulations and how the operation will meet the performance standards and requirements of 19.10.6.603 NMAC.

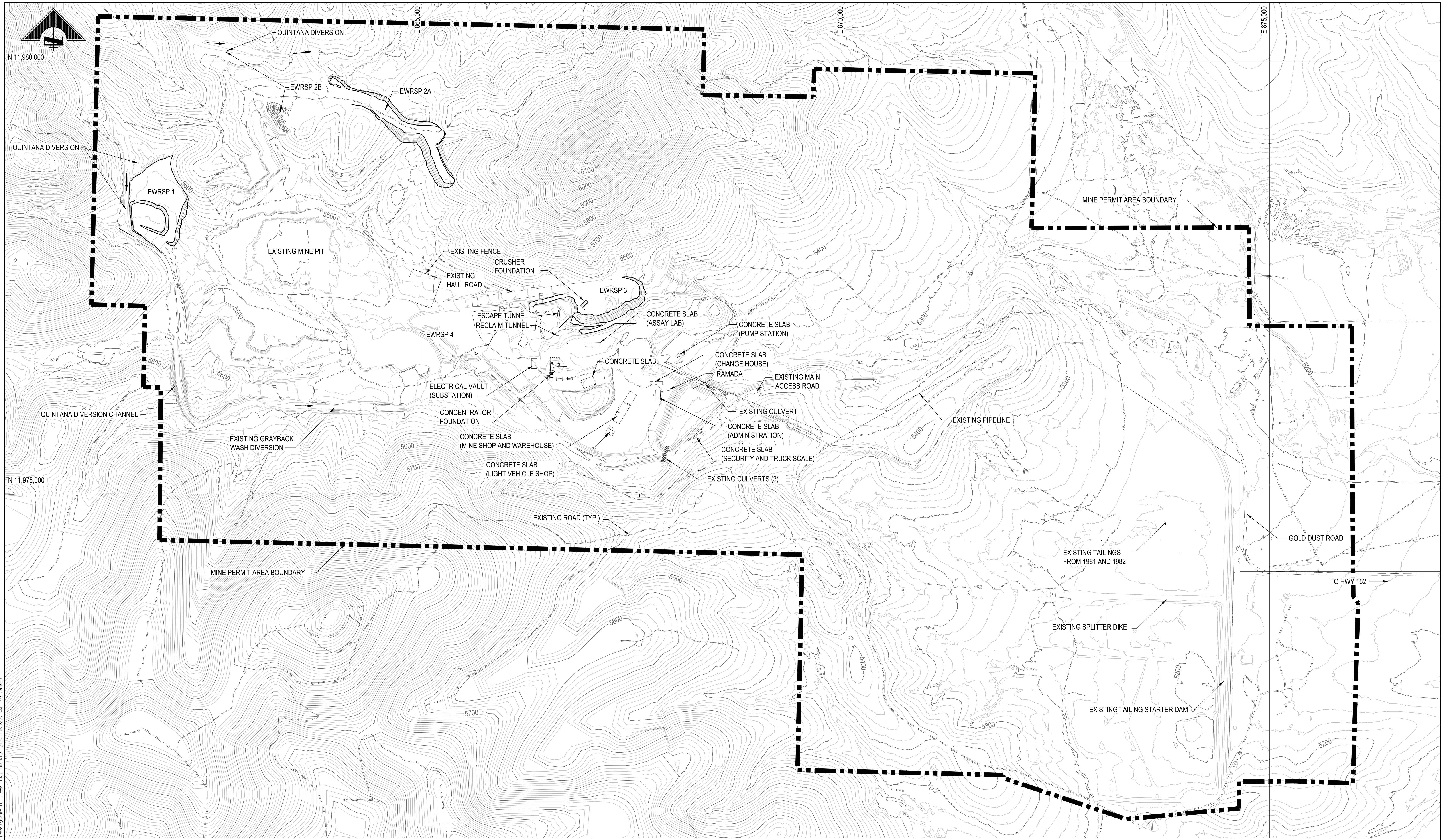
2.1 Type & Methods of Mining - 19.10.6.602.D.(15)(a) & (b)

This Section describes the type and method of mining and the engineering techniques proposed at the Copper Flat mine. It contains the required maps and describes the approximate time table and general sequence to be followed in constructing and operating the mine, including the number of acres of land anticipated to be disturbed.

NMCC proposes to construct an open pit mine at its Copper Flat Project. This new facility will entail the expansion of an existing open pit previously developed and operated for a short time in 1982 by Quintana Minerals Corporation (Quintana). A portion of the ore body at Copper Flat is exposed at and near the surface and will be mined by conventional truck and shovel open pit methods. Figure 2-1 is a map of the site as it currently exists, showing the various existing facilities as constructed by Quintana when it operated the mine. Figure 2-2 provides a map of the proposed site facility showing the location of the various features of the project described in more detail later herein. These maps provide the reviewer the ability to differentiate between what currently exists as a disturbance on-site as compared to NMCC's proposed activities.

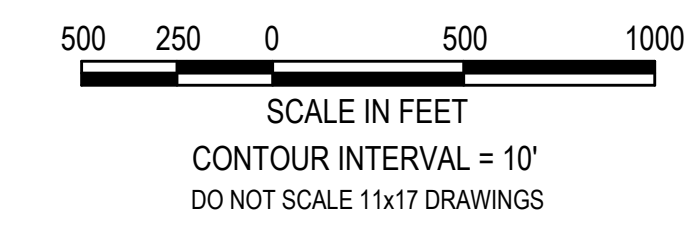
NMCC proposes to re-establish and expand the previous Quintana mining activities conducted at Copper Flat in 1982. The facilities will be similar to those of the previous operator, including an open pit mine, concentrate production facilities, waste rock stockpiles (WRSPS) and a tailings storage facility (TSF). Upon receiving the required permit approvals the project will begin site preparation and construction for approximately 2 years. The operating life of the project ("life of mine") is anticipated to be 11 to 12 years. Thereafter, the site will be closed and reclaimed per an approved reclamation and closure plan. Table 2-1 provides an approximate timetable indicating development, construction, operation and reclamation of the Copper Flat Project beginning from the time NMCC obtains all of the required permissions and approvals.

NMCC will mine approximately 113 million tons of ore and 45 million tons of waste rock during the operating life of the mine (158 million tons). Annually, the mining operation will supply 8.9 million 10.8 million tons of copper ore to the mill for processing (an average rate of approximately 25.5 to 29.6 thousand tons per day) depending on operational conditions in the concentrator. Table 2-2 shows the estimated annual mine and process production schedule for Copper Flat. Waste rock production will be highest in the early years of



NOTES:
 CONCRETE FOUNDATIONS SHOWN REFLECT PLANS BY QUINTANA MINERALS CORPORATION FROM 1981 AND MOST RECENT SURVEY CONDUCTED.

SITE PLAN
 SCALE: 1" = 500'



PRELIMINARY
 FOR AGENCY REVIEW



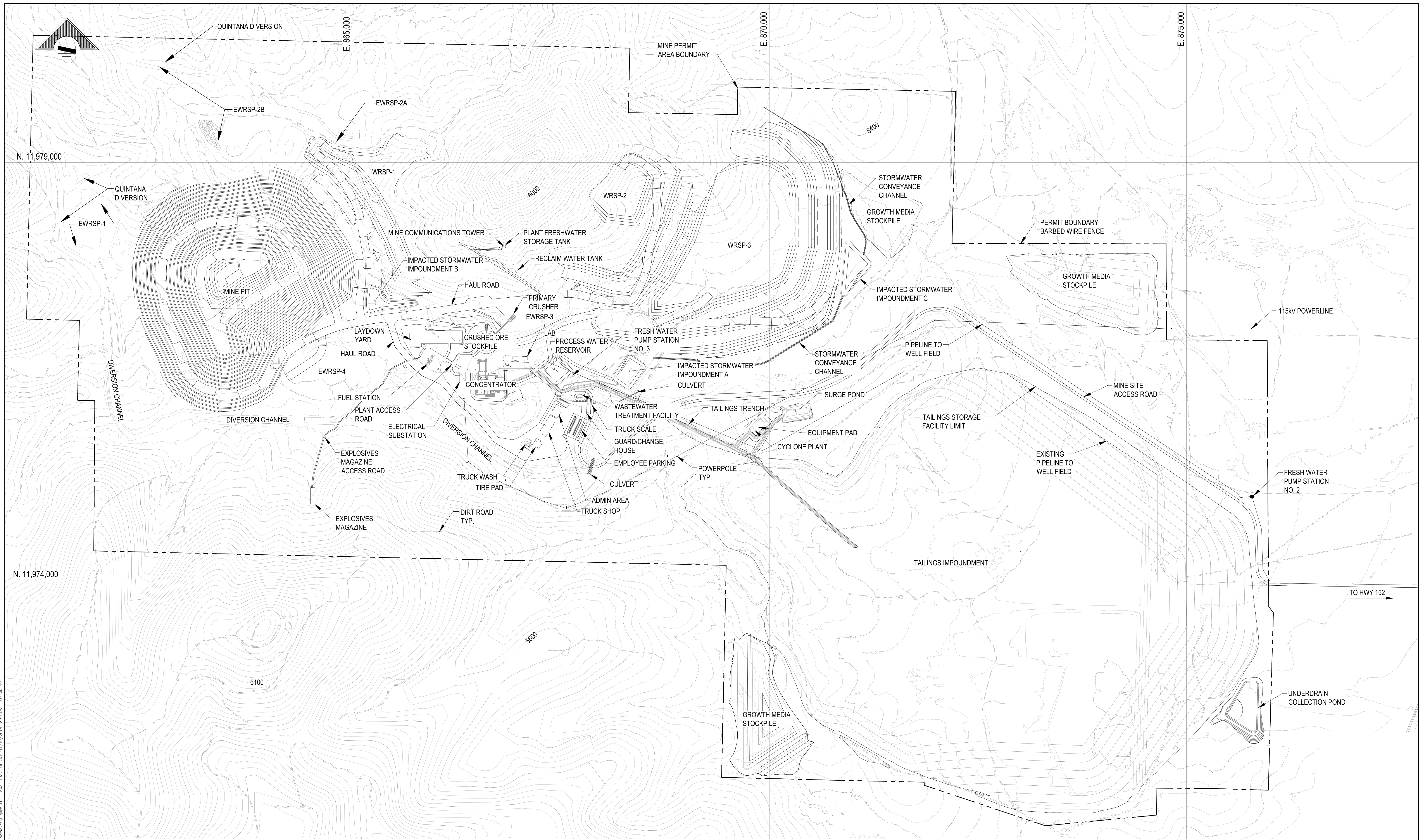
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COPPER FLAT PROJECT

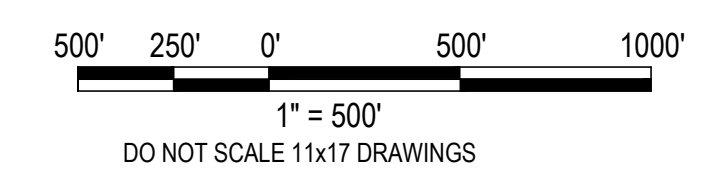
SITE GENERAL CIVIL MINE SITE EXISTING CONDITIONS

JOB NO. M3 PN-12085
 DWG NO. **FIGURE 2-1**
 REV NO. P2 DATE 20 NOV 15



EWRSP = EXISTING WASTE ROCK STOCKPILE
 WRSP = WASTE ROCK STOCKPILE

SITE PLAN
 SCALE: 1:500



PRELIMINARY
 FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT

SCALE:	1" = 500'	DATE:	
DESIGNED BY:	SAM	DATE:	DEC12
DRAWN BY:	SAM	DATE:	DEC12
CHECKED BY:	TDL	DATE:	JAN13
PROJECT MGR:	RKZ		
CLIENT APPR:			

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COPPER FLAT PROJECT

SITE GENERAL CIVIL PROJECT AREA PROPOSED SITE PLAN

JOB NO. M3 PN-120085
 DWG. NO. **FIGURE 2-2**
 REV. NO. P18 DATE 16 NOV 15

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**TABLE 2-1
COPPER FLAT DEVELOPMENT SEQUENCE AND SCHEDULE**

Project Build Out Sequence		Disturbed Acres		19.10.1602.D(15)(c) Reference	Project Reclamation Sequence	
Year	Project Activity	Facility	Cumulative		Year	Reclamation Activity
1	Mobilize Construction	0.00	0.00	Other Facility or Structures (c)xiii	1	
	Plant Site Grading	84.41	84.41	Other Facility or Structures (c)xiii		
	TSF Phase 1	451.50	535.91	Tailings Storage Facility (c)vii		
	Top Dressing Stockpile 1	29.33	565.24	Topsoil & Topdressing Stockpiles (c)xi		
	Construct Mill	8.51	573.75	Mills (c)viii		
	Construct Ancillary Facilities	8.89	582.64	Other Facility or Structures (c)xiii		
	Storage Areas	3.22	585.86	Storage Areas (c)x		
	EWRSP 1	15.34	601.20	Waste Rock Stockpiles (c)xii		
	EWRSP 2A	8.33	609.53	Waste Rock Stockpiles (c)xii		
	EWRSP 2B	12.73	622.26	Waste Rock Stockpiles (c)xii		
	EWRSP 3	19.54	641.80	Waste Rock Stockpiles (c)xii		
	EWRSP 4	18.10	659.90	Waste Rock Stockpiles (c)xii		
	Mine Haul Roads	5.97	665.87	Waste Rock Stockpiles (c)xii		
Impoundments : TSF; Proc; SW A	12.92	678.79	Impoundments (c)ii			
Collection Ditches: SW A	1.38	680.17	Impoundments (c)ii			
2	Top Dressing Stockpile 2	31.55	711.72	Topsoil & Topdressing Stockpiles (c)xi	2	Reclaim EWRSP 1
	Top Dressing Stockpile 3	3.53	715.25	Topsoil & Topdressing Stockpiles (c)xi		Reclaim EWRSP 2A
	Construct Ancillary Facilities	21.10	736.35	Other Facility or Structures (c)xiii		Reclaim EWRSP 2B
	Open Pit	82.66	819.01	Open Pit (c)vi		
	WRSP 1	3.97	822.98	Waste Rock Stockpiles (c)xii		
	WRSP 2	2.44	825.42	Waste Rock Stockpiles (c)xii		
	WRSP 3	6.07	831.49	Waste Rock Stockpiles (c)xii		
	Mine Haul Roads	11.03	842.52	Waste Rock Stockpiles (c)xii		
	EWRSP 4	4.52	847.04	Waste Rock Stockpiles (c)xii		
	Ore Stockpile	2.07	849.11	Ore Stockpiles (c)i		
Impoundments : Surge; SW B; SW C	8.99	858.10	Impoundments (c)ii			
Collection Ditches: SW B; SW C	4.42	862.52	Impoundments (c)ii			
3	Top Dressing Stockpile 3	10.58	873.10	Topsoil & Topdressing Stockpiles (c)xi	3	
	Open Pit	66.13	939.23	Open Pit (c)vi		
	WRSP 1	27.80	967.03	Waste Rock Stockpiles (c)xii		
	WRSP 2	4.88	971.91	Waste Rock Stockpiles (c)xii		
	WRSP 3	18.20	990.11	Waste Rock Stockpiles (c)xii		
TSF Phase 2	28.22	1,018.33	Tailings Storage Facility (c)vii			
4	WRSP 1	7.94	1,026.27	Waste Rock Stockpiles (c)xii	4	
	WRSP 2	19.51	1,045.78	Waste Rock Stockpiles (c)xii		
	WRSP 3	18.20	1,063.98	Waste Rock Stockpiles (c)xii		
	TSF Phase 3	28.22	1,092.20	Tailings Storage Facility (c)vii		
5	Open Pit	8.27	1,100.47	Open Pit (c)vi	5	
	WRSP 2	14.63	1,115.10	Waste Rock Stockpiles (c)xii		
	WRSP 3	18.20	1,133.30	Waste Rock Stockpiles (c)xii		
	TSF Phase 4	28.22	1,161.52	Tailings Storage Facility (c)vii		
6	Open Pit (buildout complete)	8.27	1,169.79	Open Pit (c)vi	6	
	WRSP 1	0.00	1,169.79	Waste Rock Stockpiles (c)xii		
	WRSP 2	4.88	1,174.67	Waste Rock Stockpiles (c)xii		
7	WRSP 3	18.20	1,192.87	Waste Rock Stockpiles (c)xii	7	
	WRSP 2, 3	2.44	1,195.31	Waste Rock Stockpiles (c)xii		
	WRSP 3	18.20	1,213.51	Waste Rock Stockpiles (c)xii		
8	TSF Phase 5 (buildout complete)	28.22	1,241.73	Tailings Storage Facility (c)vii	8	
9 - 11	WRSP 3 (buildout complete)	6.07	1,259.93	Waste Rock Stockpiles (c)xii	10 - 11	WRSP 3 Contour
12					12	WRSP 3 Contour, TSF Draindown - Active Evaporation
13					13	Pit Rapid Fill, WRSP 2-Upper Lift Contour, WRSP 1- Contour, TSF Draindown - Active Evaporation
14	Mining and Processing Ends				14	Rapid Fill, WRSP-2 Upper Lift Contour, WRSP 1 - Contour, Fill & Contour, WRSP 3, 2, 1, EWRSP 4 Cover & Seed, TSF Draindown - Active Evaporation
15					15	Process Area Demo, Fill & Contour, WRSP 3, 2, 1, EWRSP 3 & 4 Contour, Cover & Seed, Pit Area Contour, TSF Contour, Draindown - Active Evaporation
16					16	Process Area Fill & Contour, WRSP 3, 2, 1, EWRSP 3 & 4 Contour, Cover, Seed, TSF Contour, Draindown - Active Evaporation
17					17	TSF Contour, Draindown - Active Evaporation
18	Evaporation Pond Construction (Project Buildout Complete)	24.05	1,290.05	Impoundments (c)ii	18	TSF Contour & Cover, Draindown - Active Evaporation, Passive Evaporation
19					19	TSF Contour, Cover, Draindown - Passive Evaporation
20 - 21					20 - 21	TSF Contour, Cover, Seed, Draindown - Passive Evaporation
22 - 38					22 - 38	TSF Draindown - Passive Evaporation
39					39	TSF Evaporation Pond Fill, Cover & Seed

TABLE 2-2
Copper Flat Project
Estimated Mine & Process Scheduler

Period	Ore Annual Kton	WRSP 1 Annual Kton	WRSP 2 Annual Kton	WRSP 3 Annual Kton	Total WRSP Annual Kton	Total Mined Annual Kton	Strip Ratio	Mine Avg TPD	Process Annual Kton	Process Avg TPD
Preproduction	360,000	32,000	30,000	48,000	110,000	470,000	0.31	7,380	0	0
Year 1	8,940,000	2,073,000	1,346,000	5,141,000	8,560,000	17,500,000	0.96	47,950	9,300,000	25,480
Year 2	10,800,000	1,055,000	2,544,000	3,312,000	6,700,000	17,500,000	0.62	47,950	10,800,000	29,590
Year 3	10,800,000	0	1,756,000	4,156,000	6,700,000	17,500,000	0.62	47,950	10,800,000	29,590
Year 4	10,800,000	0	628,000	4,944,000	6,700,000	17,500,000	0.62	47,950	10,800,000	29,590
Year 5	10,800,000	0	0	6,072,000	6,700,000	17,500,000	0.62	47,950	10,800,000	29,590
Year 6	10,250,000	0	0	5,924,000	5,924,000	15,949,000	0.59	43,700	10,025,000	27,470
Year 7	9,900,000	0	0	2,491,000	2,491,000	12,391,000	0.25	33,950	9,900,000	27,120
Year 8	9,900,000	0	0	718,000	718,000	10,618,000	0.07	29,090	9,900,000	27,120
Year 9	9,900,000	0	0	71,000	71,000	9,971,000	0.01	27,320	9,900,000	27,120
Year 10	9,900,000	0	0	3,000	3,000	9,903,000	0.00	27,130	9,900,000	27,120
Year 11	9,900,000	0	0	1,000	1,000	9,901,000	0.00	27,130	9,900,000	27,120
Year 12	1,059,000	0	0	4,000	4,000	1,063,000	0.00	26,580	1,059,000	26,480
Total	113,084,000	3,160,000	8,637,000	32,885,000	44,682,000	157,766,000	0.40	38,340	113,084,000	27,890



production while the mine is developed, i.e., 8.5 million tons in the first year to 2.5 million tons in the seventh year. Thereafter, waste rock production will decrease significantly, i.e., 718,000 tons in year eight to as little as 4,000 tons in year twelve.

The area inside the proposed permit area boundary is 2,190 acres. NMCC’s proposed Copper Flat Project will disturb approximately 1,290 acres within the permit area, 910 acres of which were originally disturbed by previous mining operations. There may also be some additional acreage disturbance on lands outside of the permit area boundary related to ancillary facilities such as the well field, the substation and power line, and the water pipeline.

Table 2-3 summarizes the approximate number of acres disturbed by the Copper Flat Project at the end of mine life. The total amount of acreage to be reclaimed is discussed in Section 3.0.

TABLE 2-3 Copper Flat Project Disturbed Acreage		
Project Area	Associated 19.10.6.602.D.(15)(c) Requirement	Acres Disturbed
Crushed Ore Stockpile	Ore Dumps & Stockpiles - (c)i	2
Ponds & Surface Impoundments	Impoundments - (c)ii & iii	52
Mine Pit	Pits - (c)vi	165
Tailings Storage Facility	Tailings Disposal Facilities - (c)vii	565
Concentrator	Mills - (c)viii	9
Laydown Yard & Fuel Station	Storage Areas - (c)x	3
Growth Media Stockpiles	Topsoil and Top Dressing Stockpiles - (c)xi	75
Waste Rock Stockpiles (existing & proposed)	Waste Rock Stockpiles - (c)xii	305
Administration/Warehouse/Other Facilities	Other Facilities or Structures - (c)xiii	114
Total		1290

2.1.1 Existing Open Pit

Quintana created the existing open pit shown in Figure 2-1 in 1982 when they brought the property into production as an open pit mine and mineral processing plant. The initial mine stripping required to expose the ore body occurred during the four- to six-month period immediately preceding startup of the mineral processing plant. Following startup the open pit and processing plant were in commercial production for three and a half months. At that time, all operations were halted due to a significant decline in copper prices. Approximately 3 million tons of overburden material and 1.2 million tons of ore were mined from the open pit by Quintana. No commercial mining has occurred at this open pit since 1982.

The elevation of the bottom level of the existing Quintana pit is 5,400 feet above mean sea level (amsl), approximately 100 feet beneath the original pre-mining ground surface. The existing open pit encompasses approximately 80 acres of existing disturbance. The bottom benches of the existing pit are flooded, forming a small 5 acre water body. The water level



fluctuates with the season in response to precipitation year over year. The depth of the water varies with the underlying open pit and ranges from 10 feet deep to 35 feet deep. The pit currently contains approximately 80 acre-feet (AF) of water.

2.1.2 Proposed Open Pit

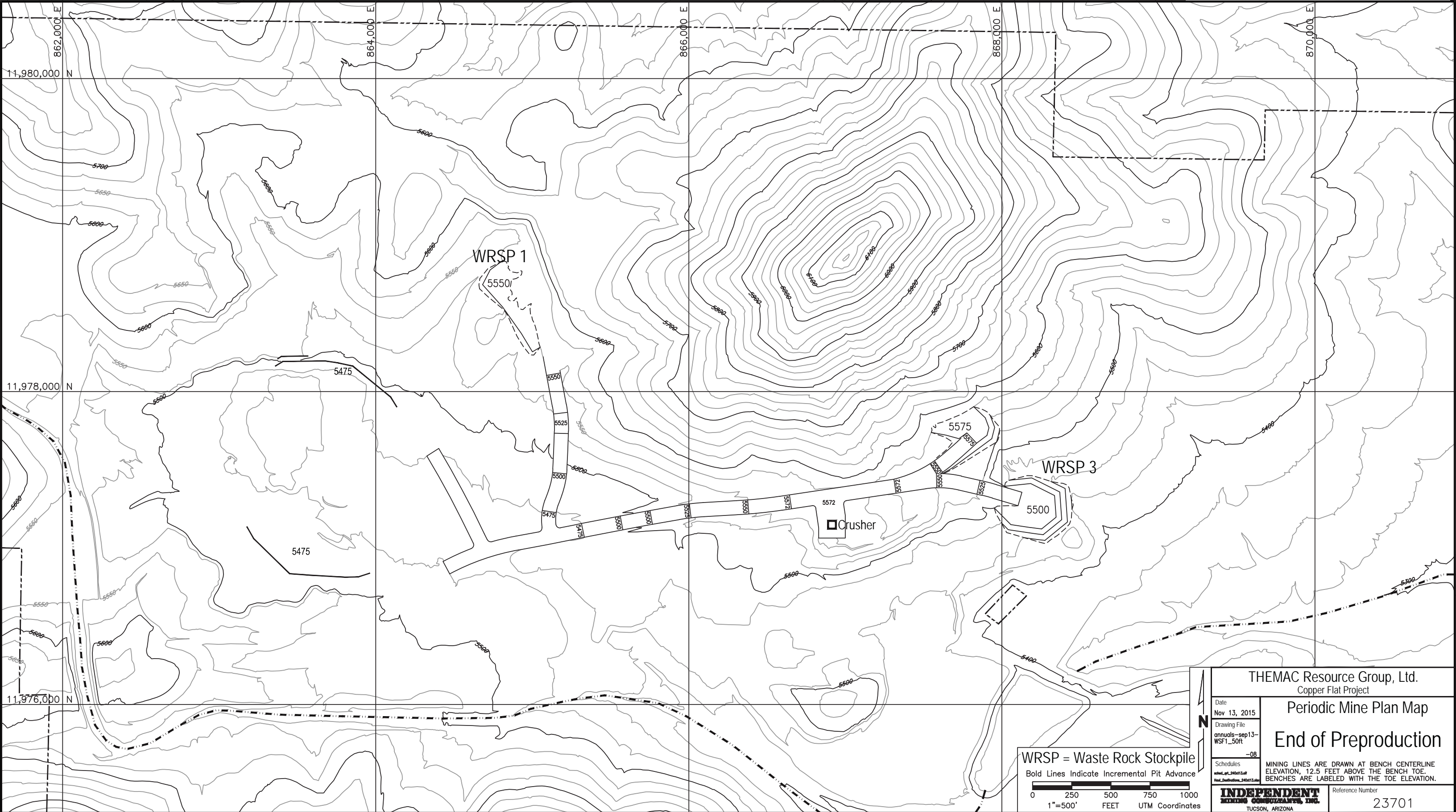
As shown in Figure 2-2, NMCC's proposed open pit will be created through the expansion of the existing open pit to a total of approximately 166 acres, including the pit, explosives magazine and magazine access road. A multiple bench, open pit mining method will be used to mine the Copper Flat ore body. The pit material will be drilled, blasted, and excavated, creating benches approximately 25 ft. high. The blasted material will be loaded by wheel loader to haul trucks where it will be hauled either to the waste rock piles for storage or as ore to the primary crusher where it will be fed, crushed and conveyed to the mill for processing. A description of the process is provided in more detail later herein. Figures 2-3 through 2-11 depict expansion of the pit over time.

Over the 12-year life of the mine, approximately 113 million tons of copper ore and 45 million tons of waste rock will be mined and removed from the open pit. The proposed mining activities will enlarge the open pit over time to a diameter of approximately 2,800 feet. The open pit will reach a depth of approximately 4,650 feet above sea level, which will be approximately 850 to 900 feet beneath the original pre-mining ground surface. The area of the pit will be expanded to approximately 165 acres. The existing diversions of Grayback Arroyo, shown on Figure 2-1, constructed by Quintana during its operation of the mine will provide diversion of water around the pit and will not be affected by the proposed pit expansion (see Figure 2-2).

2.1.3 Waste Rock Stockpiles (WRSPs)

Waste rock will be hauled from the mine pit and placed in designated stockpile areas shown on Figure 2-2 and described in more detail later herein. These new WRSPs will be constructed in an area of the site that is completely underlain by andesite bedrock, a geologic formation that has a transmissivity of less than 10^{-6} centimeters per second (cm/sec) (SRK, May 2013), thus providing a natural liner. These WRSPs will be constructed as units segregated by the grade of copper contained in the waste rock material so as to maximize its potential to be processed as ore in the future. WRSP-1 is located inside the post-mining open pit surface drainage area and will contain the highest non-ore grade material. WRSP-2 will contain the next highest non-grade material. WRSP-3 will contain all the remaining material.

These new WRSPs will cover approximately 221 acres, including haul roads. They will be built generally to a configuration of 3 horizontal to 1 vertical slope angles (18.4 degrees) to help facilitate reclamation at the end of the mine life. Each lift within the stockpile will be

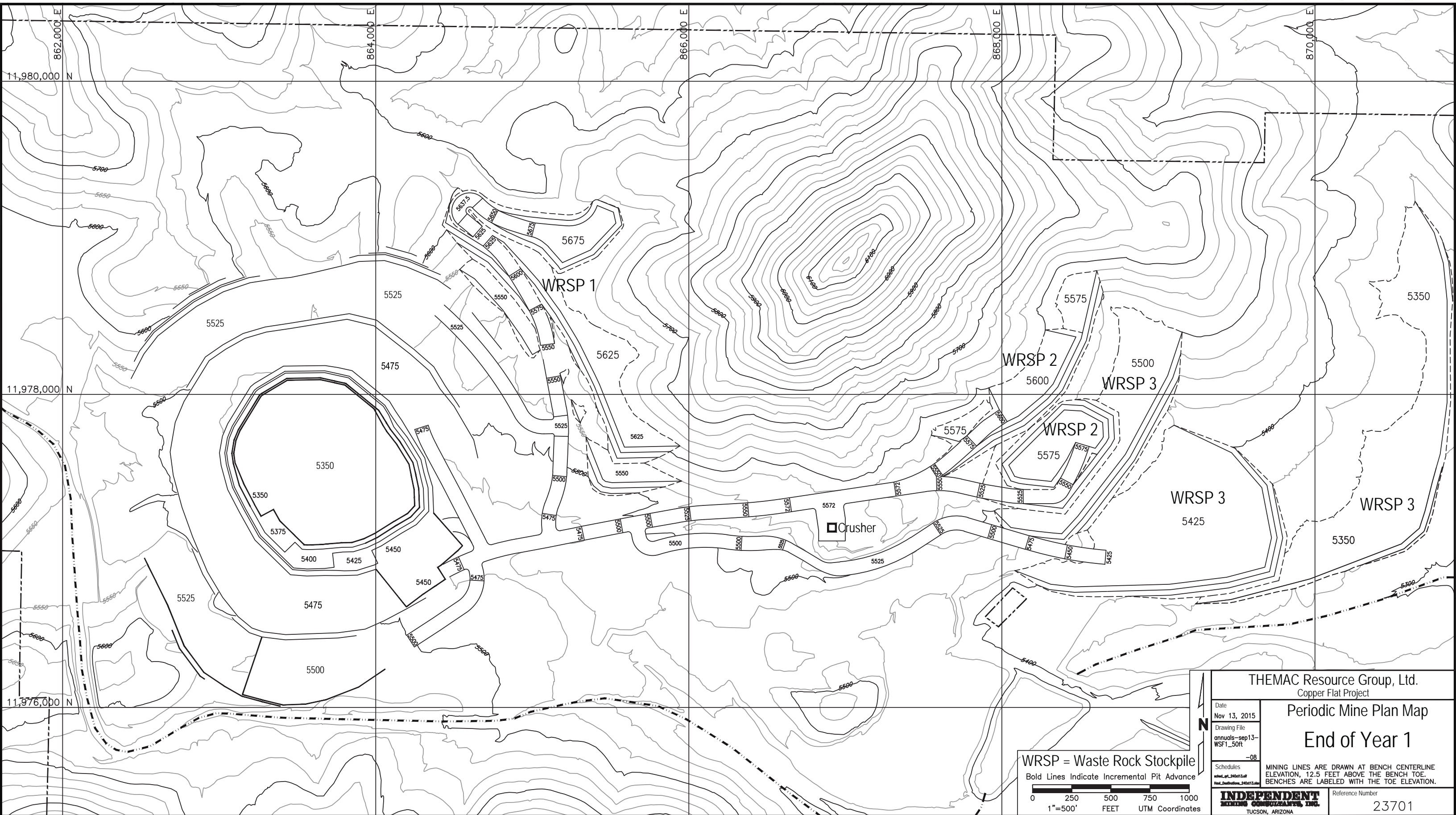


WRSP = Waste Rock Stockpile
Bold Lines Indicate Incremental Pit Advance

0 250 500 750 1000
1"=500' FEET UTM Coordinates

THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map End of Preproduction
Drawing File annuals-sep13- WSF1_50ft	
Schedules mining_commitments_2013	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number 23701	

FIGURE 2-3

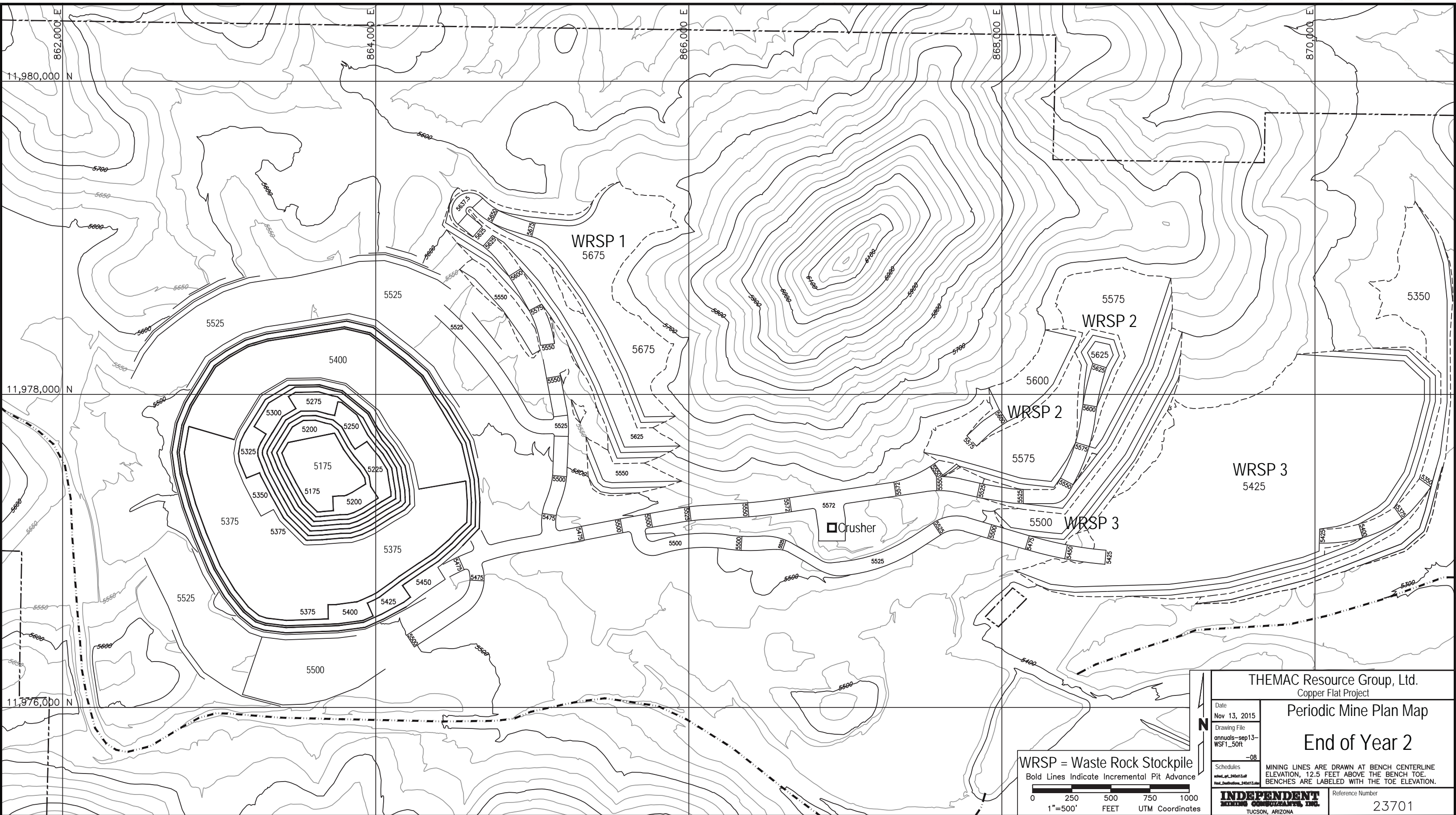


WRSP = Waste Rock Stockpile
 Bold Lines Indicate Incremental Pit Advance

0 250 500 750 1000
 1"=500' FEET UTM Coordinates

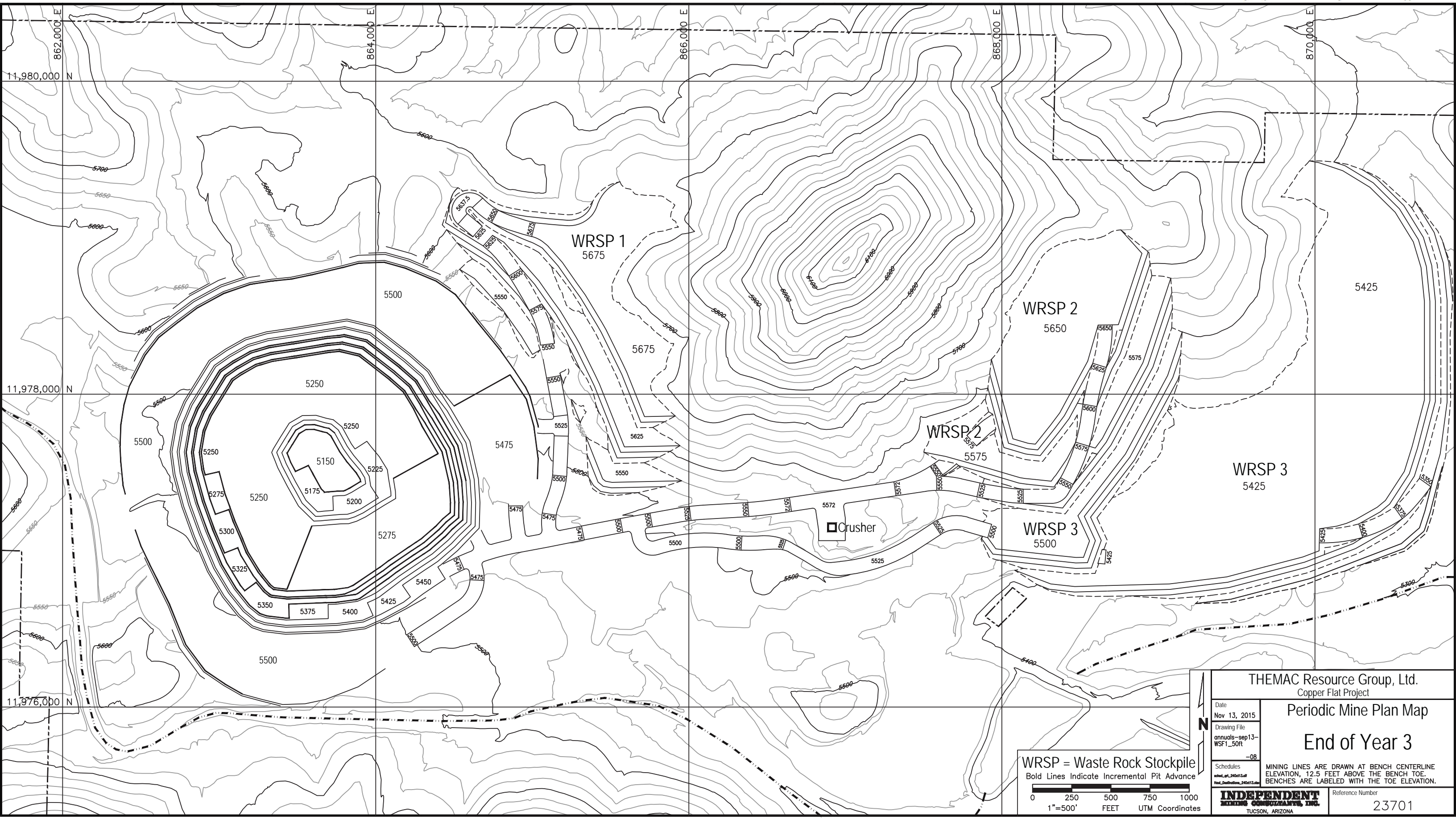
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map End of Year 1
Drawing File annuals-sep13-WSF1_50ft	
Schedules mining-compliance_2015	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number 23701	

FIGURE 2-4
 Page 2-9
 13713



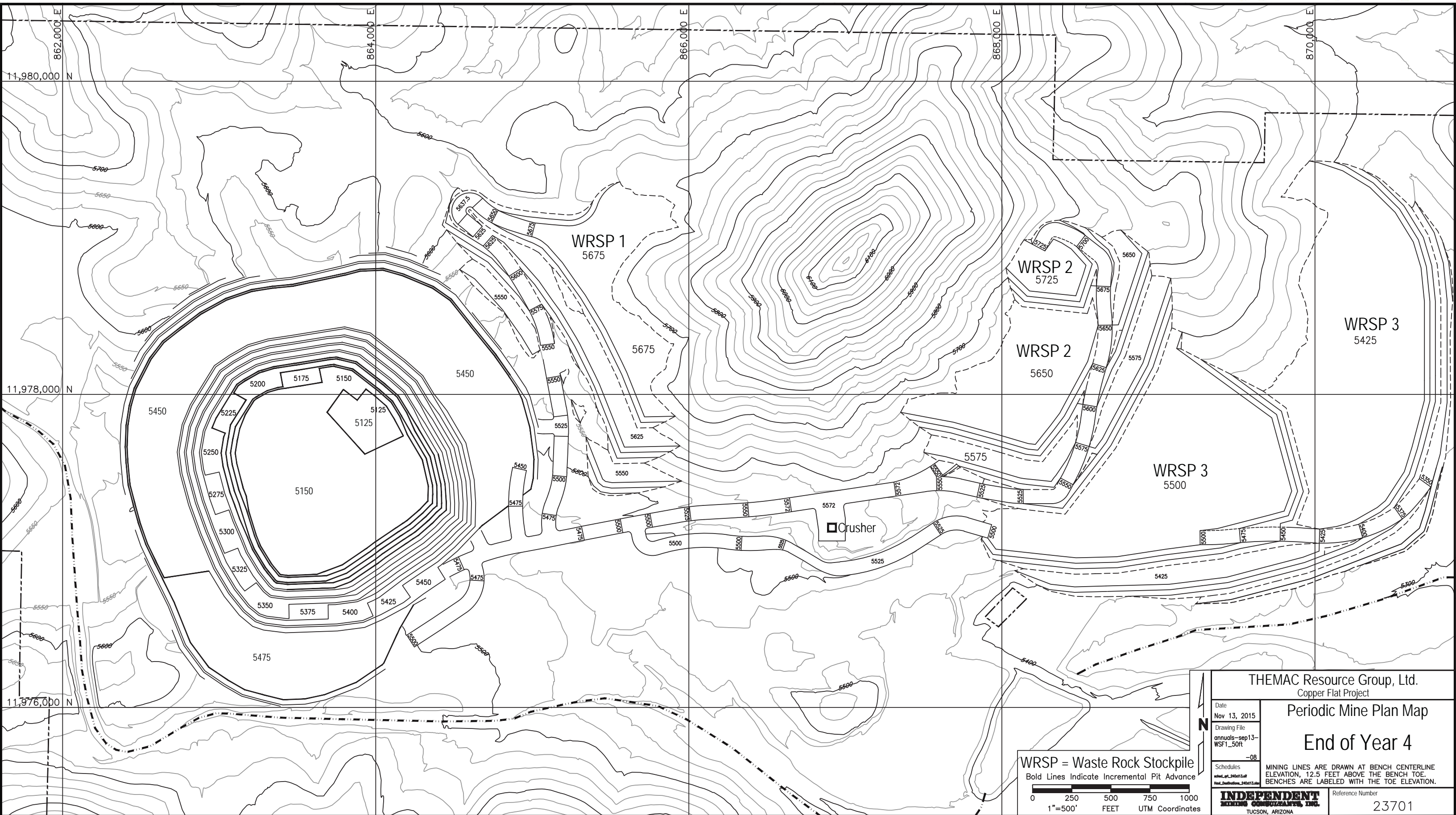
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13-WSF1_50ft	End of Year 2
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

FIGURE 2-5
 Page 2-10



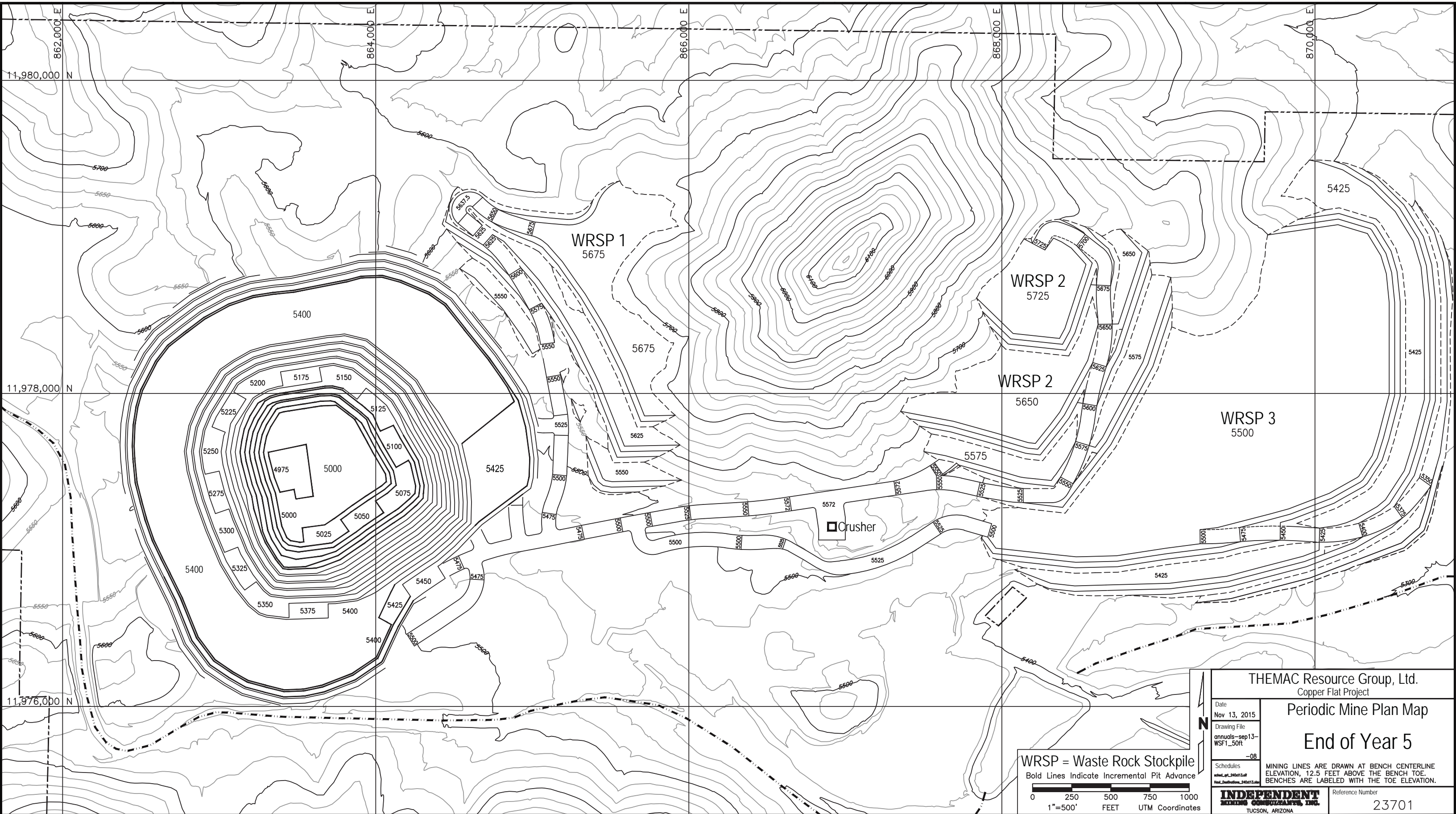
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 3
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

FIGURE 2-6



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13-WSF1_50ft	End of Year 4
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number 23701	

FIGURE 2-7
 Page 2-12
 13

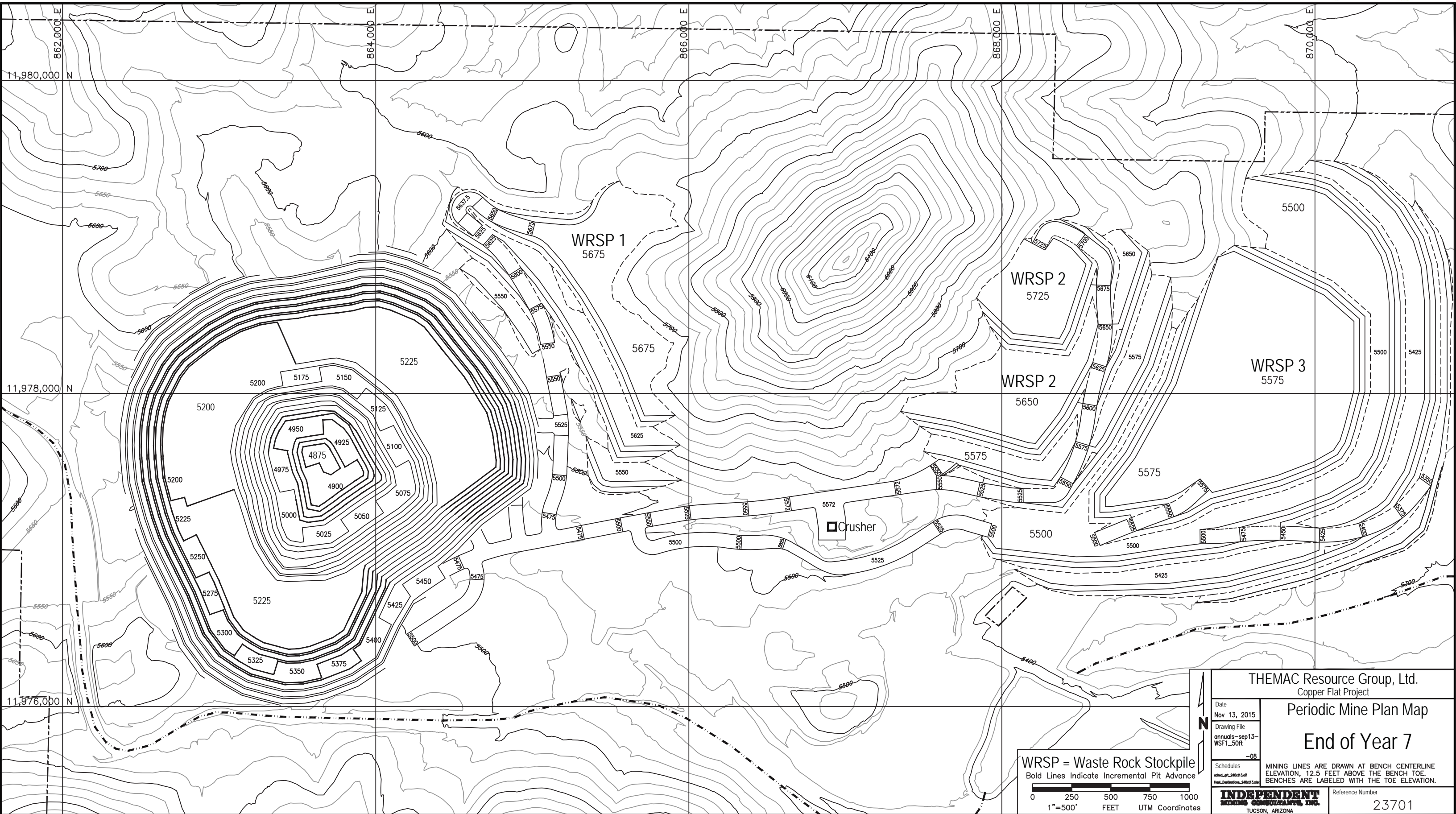


THEMAC Resource Group, Ltd. Copper Flat Project		
Date Nov 13, 2015	Periodic Mine Plan Map	
Drawing File annuals-sep13-WSF1_50ft	End of Year 5	
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.	
<table border="1"> <tr> <td style="text-align: center;">INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA</td> <td style="text-align: center;">Reference Number 23701</td> </tr> </table>		INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701	

WRSP = Waste Rock Stockpile
 Bold Lines Indicate Incremental Pit Advance

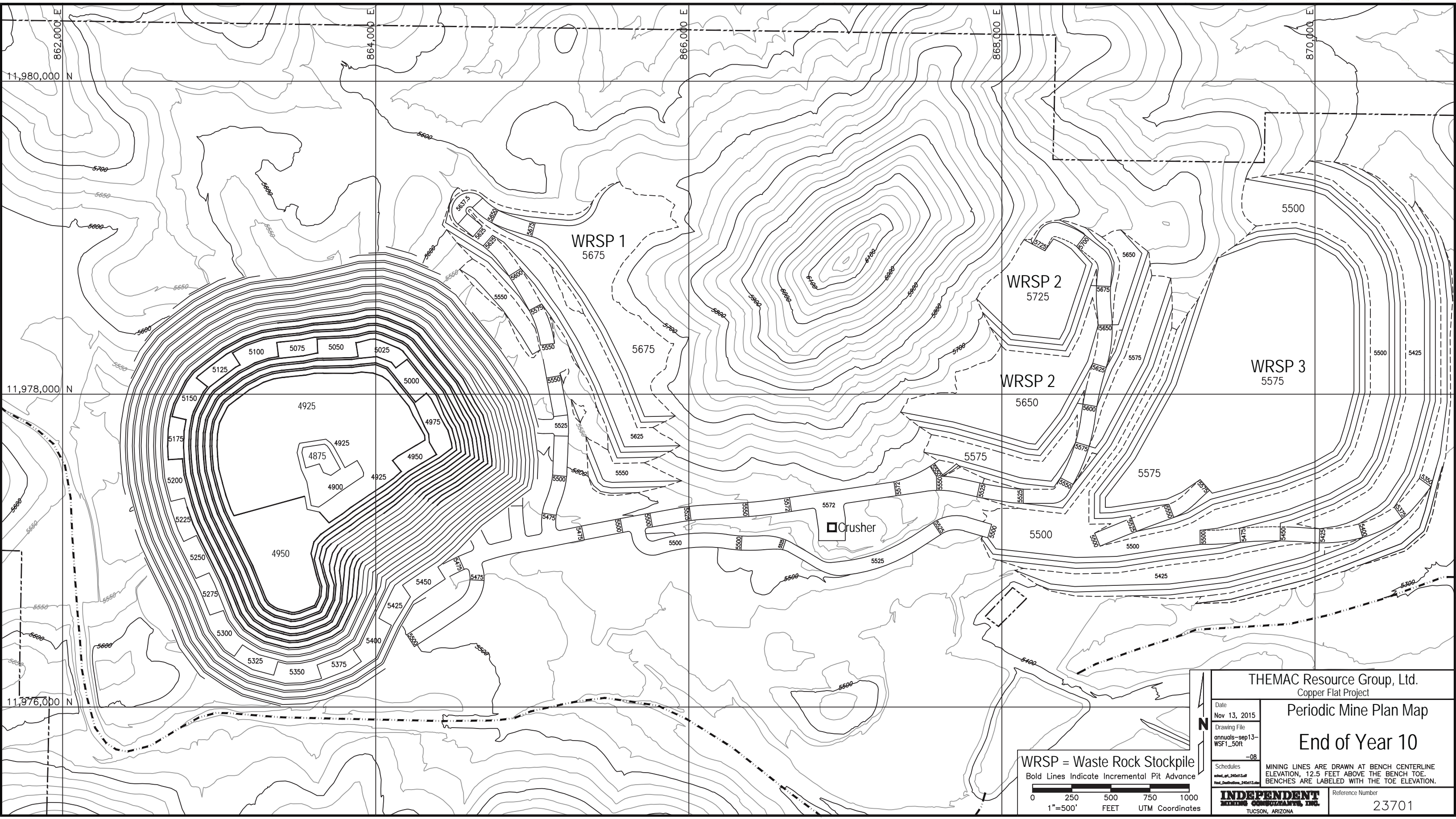
0 250 500 750 1000
 1"=500' FEET UTM Coordinates

FIGURE 2-8
 Page 2-13



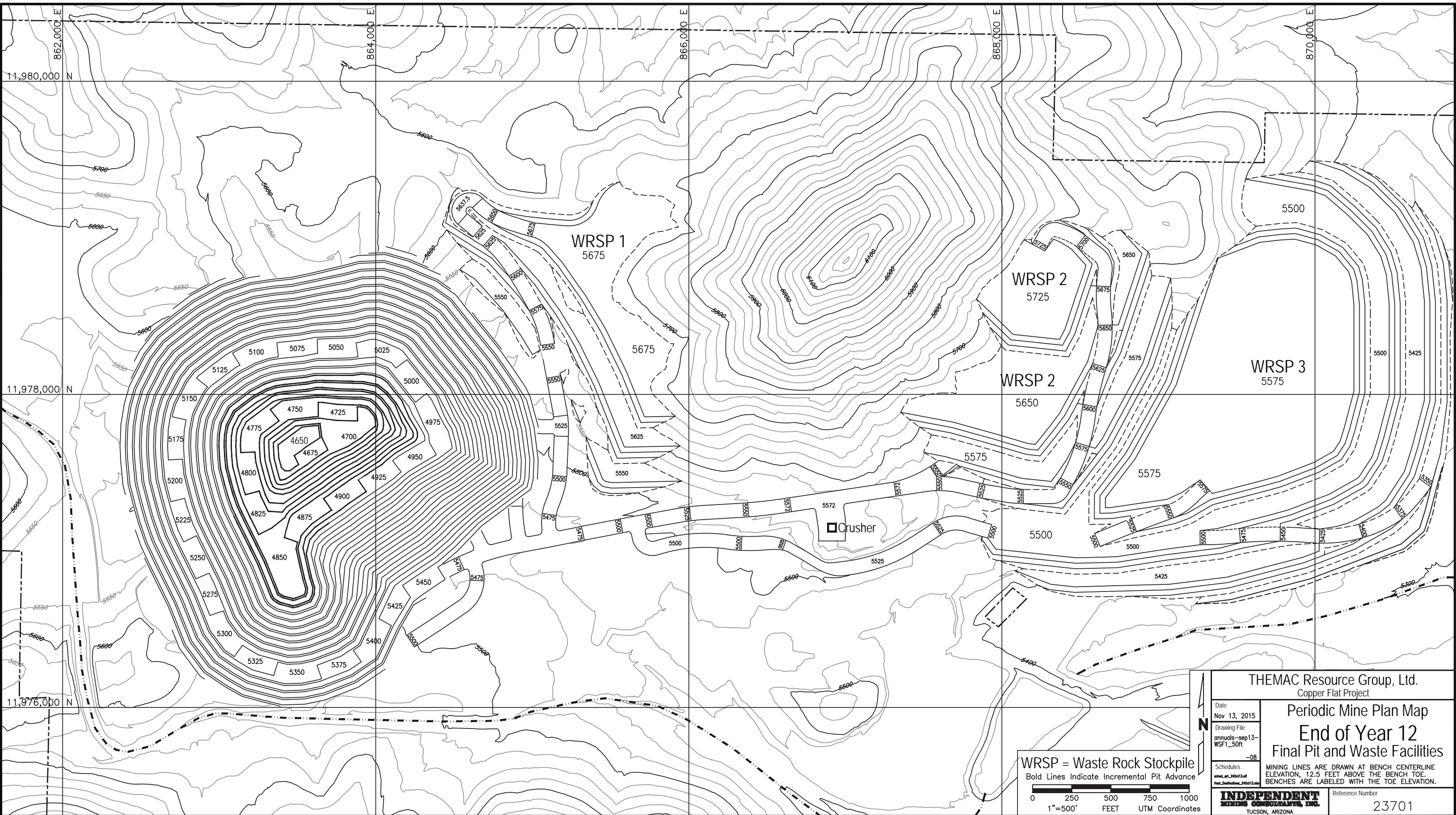
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13-WSF1_50ft	End of Year 7
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

FIGURE 2-9



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 10
Schedules mining-comp13- TUCSON, ARIZONA	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
Reference Number 23701	

FIGURE 2-10



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map End of Year 12 Final Pit and Waste Facilities
Drawing File annuals-sep13-WSF1_50ft	
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number 23701	

FIGURE 2-11



approximately 75 ft. high and be placed at angle of repose (35.54 degrees) with benches sufficiently wide enough between lifts to maintain the 3 to 1 overall angle for the stockpile. Interceptor trenches will be constructed above each waste rock stockpile to limit storm water run-on. Surface water runoff collection trenches will be constructed to collect and route runoff from the proposed stockpiles to storm water impoundments. The collection trenches will be constructed into the andesite bedrock to prevent water from entering the alluvial surface material down-gradient of the WRSP and in a manner to maximize positive flow while minimizing the potential for ponding and erosion.

In addition to the proposed WRSPs, there are also four existing waste rock stockpiles (EWRSP) on-site generated by the previous Quintana mining activities. These EWRSPs, identified in Figure 2-1, represent an additional 84 acres of disturbance. EWRSP-1 and EWRSP-2B are located in the open pit surface drainage area. EWRSP-2A is located largely coincident with the location of proposed WRSP-1. EWRSP-3 is located in the plant process area next to the primary crusher. EWSRP-4 is also located in the plant process area and will be utilized during operations as a storage area.

2.1.4 Ore Processing Facility

The ore will be processed through a conventional sulfide flotation concentrator, using standard crushing, grinding and flotation technologies. It will be trucked from the open pit to the plant area, crushed and temporarily stored at a stockpile before being processed through a copper sulfide flotation mill, using a flowsheet very similar the Quintana operation (see Figure 2-2). The mill will process ore at an average throughput rate of 27,890 tons per day over the life of the operation. Milling will also include a molybdenum processing circuit and a gravity gold recovery circuit. The processing facility will be located on approximately 128 acres, including the crushed ore stockpile, concentrator, laydown yard and fuel station, process water reservoir and the administration, warehouse, and other facilities. A detailed discussion of the concentration plant process is provided later herein.

The copper concentrate will be shipped in bulk form by truck to an off-site smelter or rail loadout facility. Molybdenum concentrate will be filtered, dried, packaged and shipped by truck to purchasers for further refining. Coarse gold concentrate recovered from the gravity gold circuit will be shipped to a refinery for further processing.

2.1.5 Tailings Storage Facility (TSF)

NMCC will construct a new tailings storage facility (TSF) which will include a lined tailings impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam. The tailings impoundment and dam will encompass approximately 604 acres of the site, including the footprint of the tailings dam and impoundment, the cyclone plant and surge tank and the surge pond, associated pipeline



trenches and a future evaporation pond constructed after the end of life of the project to provide evaporation of drain-down water from the TSF long-term.

Whole tailings will be transported from the processing facility to the TSF cyclone plant via pipeline where the coarse tailings fraction, i.e., the sand, will be separated from the fines fraction. The sand fraction will be utilized to construct the tailings embankment dam in phases during operations. The fines fraction will be disposed of in the TSF impoundment behind the embankment dam. The TSF impoundment will be equipped with a water reclaim or recycle system to maximize water reuse. A water reclaim barge will be installed within the impoundment to recycle water from the impoundment back to the process facility.

The TSF includes an underdrain collection system constructed at the bottom of the TSF impoundment above the liner as well as a dam embankment underdrain blanket constructed under the dam. These features will collect free water that drains from the impoundment and the dam into an underdrain collection pond and route it back to the process facility for re-use. The dam blanket drain will provide the mechanism that will drain water within the dam out of the structure and allow the dam to become consolidated and stable as it is continually constructed during operations as the sand fraction of tailings is emplaced on the dam. The tailings impoundment underdrain system will allow free water in the impoundment to be drained out of the impoundment from underneath the impoundment to the underdrain collection pond and also be recycled to the processing facility. The TSF underdrain collection pond will also serve to capture surface water runoff routed from the downstream face of the tailings dam via runoff control ditches to the pond.

The tailings delivery from the process plant will have an associated surge pond that will be part of the cyclone plant located at the TSF. The purpose of the surge pond will be to capture and temporarily retain tailings materials in the event of a temporary upset in the cyclone plant or the process facility.

Appendix A, Feasibility Level Design, 30,000 TPD Tailings Storage Facility and Tailings Distribution and Water Reclaim Systems, Copper Flat Project, Sierra County, New Mexico, November, 2015, prepared by Golder Associates Inc. (Golder), provides the technical design detail for the TSF. This document is also an appendix to NMCC's Discharge Permit application.

2.1.6 Cover Material Stockpiles

In addition to the facilities described above, three cover material stockpiles, identified as growth media stockpiles (GMSP) in various documents that will be developed from soils material that will be salvaged from the WRSP and TSF construction footprints. The location of these GMSPs is shown in Figure 2-2. These stockpiles will total approximately 75 acres in size and will be utilized as cover material for the various disturbed areas of the mine site at closure



and reclamation. Sections 2.2.10 and 4.5 provide a discussion on the use of the terms cover materials, topsoil, topdressing and growth media materials. Section 3.0 provides the details of the use of cover material in NMCC's Reclamation and Closure Plan.

2.1.7 Off-Site Ancillary Facilities

The Copper Flat Project also includes several off-site facilities that are integral to the project, including an electrical substation located on land owned by the State of New Mexico, nine separate 5-acre mill-site claim sites, and an approximate 8-mile long fresh water pipeline. The mill sites are associated with the well field and utilized as support facilities for the pipeline. Figure 2-12 identifies the location of these ancillary facilities relative to the mine.

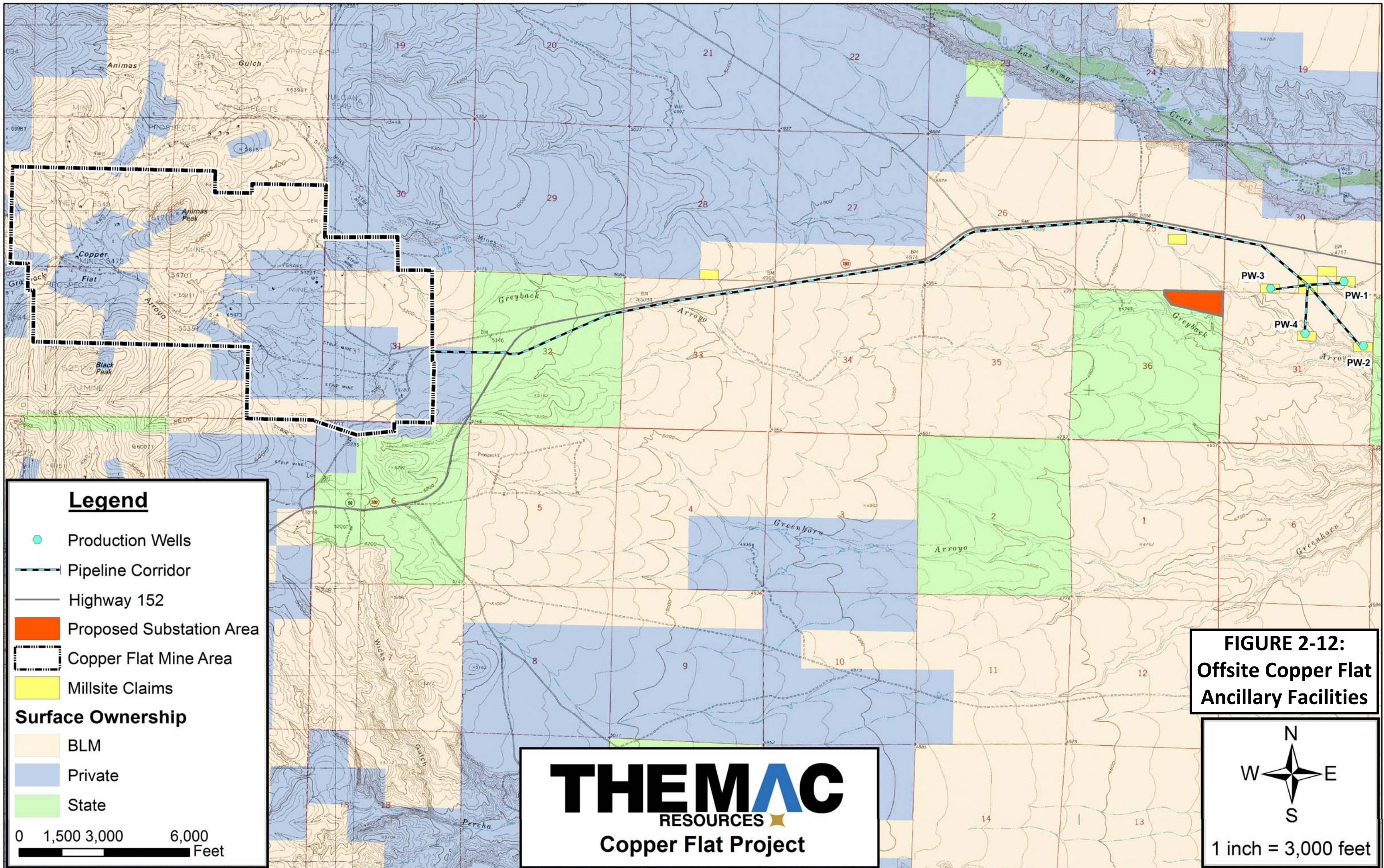
2.1.8 Closure and Reclamation

NMCC has prepared a Reclamation and Closure Plan as described in Section 3.0 and Appendix E. Section 3.0 provides a description of NMCC's plans for reclamation, including a detailed description of how the disturbed area will be reclaimed to meet the requirements of Section 69-36-7H of the Mining Act (see 19.10.6.602.D.(15)(g) NMAC of the Mining Act regulations). The objective of the Reclamation and Closure Plan is to reclaim and close the facility in a manner protective of ground water in conformance with the NM Copper Rules, meet the reclamation requirements of the New Mexico Mining Act, and return the mine area to conditions similar to those present before NMCC's reestablishment of the mine (BLM DEIS 2015, p. 2-34). The Copper Flat facility will be reclaimed to restore the land to its current use. The Mining Operation Plan and the Reclamation Plan has been designed to use the most appropriate technology (MAT) and best management practices (BMPs) to assure protection of human health and safety, the and the environment.

Section 4.0 provides the description of how the Reclamation and Closure Plan will meet the performance standards required in 19.10.6.603 NMAC. Appendix E presents the detailed design of the Reclamation and Closure Plan. Sections 3.0, 4.0 and Appendix E, together, also provide the information required by Section 20.6.7.11.T of the NMED Copper Rules.

2.2 Maps and Plans for the Mine Facility – 19.10.6.602.D.(15)(c)

This Section provides the maps and plans and other details, including the location, size, and capacities for each unit of the Copper Flat Project facilities described below. Additional details are provided in the various appended documents that contain design documentation and analyses that support the design.



**FIGURE 2-12:
Offsite Copper Flat
Ancillary Facilities**

THEMAC
RESOURCES
Copper Flat Project

N
W E
S
1 inch = 3,000 feet

Legend

- Production Wells
- Pipeline Corridor
- Highway 152
- Proposed Substation Area
- Copper Flat Mine Area
- Millsite Claims

Surface Ownership

- BLM
- Private
- State

0 1,500 3,000 6,000
Feet



2.2.1 Leach pads, heaps, ore dumps and stockpiles- 19.10.6.602.D.(15)(c)(i)

The Copper Flat Project does not propose to construct and operate any leach pads or heaps. This section discusses the coarse ore stockpile located near the primary crusher in the plant process area as shown on Figure 2-2.

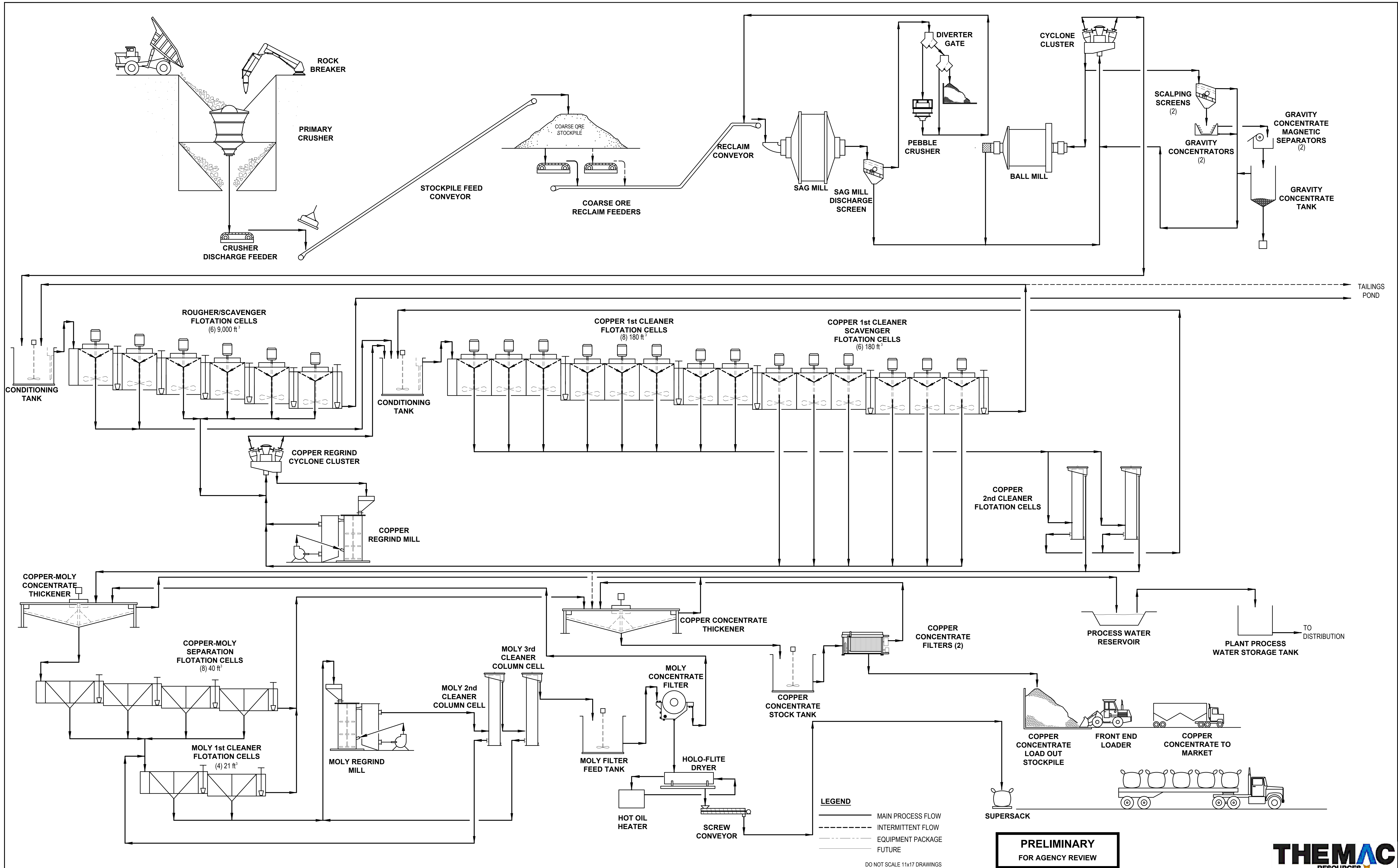
Figure 2-13 is a schematic diagram of the processing plant that shows that ore will be hauled from open pit in trucks and fed directly to the primary crusher for the first stage of crushing to produce a coarse ore stockpile (called the crushed ore stockpile in Figure 2-2). The crusher will size the run-of-mine rock to a nominal 8 inches in diameter or less. A small, temporary run-of-mine ore stockpile may be located adjacent to the crusher only when ore trucked from the open pit cannot be fed directly into the primary crusher; for example, when the crusher is temporarily out of service. Typical operating procedure will be to feed ore directly into the crusher from the open pit. Material contained in the temporary run of mine stockpile by the crusher will be fed into the crusher when direct feed from the open pit is not available.

The crushed rock will be fed by an apron feeder onto the stockpile feed belt conveyor for transport to the coarse ore stockpile where it will be temporarily stored prior to being fed into to the grinding mill. The belt conveyor will include a stacker for placing the coarse ore into the stockpile as seen on Figure 2-13. The coarse ore stockpile will have a design capacity of approximately 75,000 tons.

Ore will be drawn from the coarse ore stockpile and transported by belt conveyor passing through a reclaim tunnel located beneath the stockpile to feed the SAG mill in the grinding circuit. The conveyor system will be equipped with two variable speed apron feeders that will feed the reclaim conveyor. Ore handled through this part of the process will be relatively dry. Water associated with this part of the process will include:

- Moisture associated with the ore;
- Water spray used to control dust within the primary crusher pocket and at the stockpile feed stacker; and
- Water used for housekeeping purposes.

The primary crusher and the coarse ore reclaim equipment will be located below ground level in reinforced concrete structures. These concrete structures are existing structures from the Quintana operation and have concrete sumps built into the structures to contain excess water. Water collected in these sumps will be reused within the ore processing circuit via a pumping and recycling system that will be installed during the construction phase of the project. Water used for housekeeping purposes will be confined to use within the concrete structures where it will be contained and recycled via the sump collection and recycle system.



DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW

THEMAC
RESOURCES

REFERENCES		REFERENCES		REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT

SCALE:	NONE	DATE:	JUL 11
DESIGNED BY:	EA	DRAWN BY:	FC
CHECKED BY:		PROJECT MGR:	
CLIENT APPR:			

COPPER FLAT PROJECT

PROCESSING PLANT OVERALL FLOW SHEET 30,000 TPD CONVENTIONAL TAILINGS

JOB NO. M3 PN-120065

DWG NO. **FIGURE 2-13**

REV NO. P6 DATE 18 MAR 13



Water spray used for dust control at the stockpile feed stacker will be exterior to the concrete structures. The sprays will only be used when necessary to control dust and will be controlled to minimize excess moisture. During normal operations, water from these sprays will evaporate once the ore reaches the coarse ore stockpile, although some residual moisture will remain below the surface of the pile. In the event of an upset condition, any excess water from these sprays will be contained in the storm water impoundment that will control runoff from the plant facility area. The entire plant facility area will be contoured to control and capture precipitation falling onto the plant area in a lined impoundment. The captured water will be recycled for use in the process.

2.2.2 Impoundments and ponds – 19.10.6.602.D.(15)(c)(ii) & (iii)

NMCC will construct several impoundments and ponds at Copper Flat, including:

- Three impacted storm water impoundments to manage runoff;
- A process water reservoir to store and condition recycle water and process makeup water;
- A surge pond to manage upset conditions;
- A tailings impoundment to store tailings and produce water for recycle;
- An underdrain collection pond to capture free tailings liquids from the impoundment and the dam; and
- An evaporation pond (coincident with the underdrain collection pond) to capture and evaporate residual water that the tailings impoundment may continue to produce long-term after site closure.

Details of the design of the impacted storm water impoundments and the process water reservoir are provided in Appendix B, Impoundment Design Report, Copper Flat Project, November, 2015, prepared by M3 Engineering & Technology Corporation (M3). Details of the design of the underdrain collection pond and surge pond are provided in Appendix A. This Subsection discusses the impacted storm water impoundments and the process water reservoir. The tailings impoundment and associated underdrain collection pond and the surge pond are discussed in detail in Subsection 2.2.6, below. The evaporation pond is discussed in Section 3.0 and Appendix E.

IMPACTED STORM WATER IMPOUNDMENTS AND PROCESS WATER RESERVOIR

NMCC proposes to construct a process water reservoir and three (3) impacted storm water impoundments shown on Figure 2-2. The nomenclature of “impacted storm water impoundment” is derived from the NMED Copper Rules. However, the purpose of these impoundments can and will provide water management capabilities beyond just storm water, as discussed herein. The process water reservoir, located in the center of the plant process



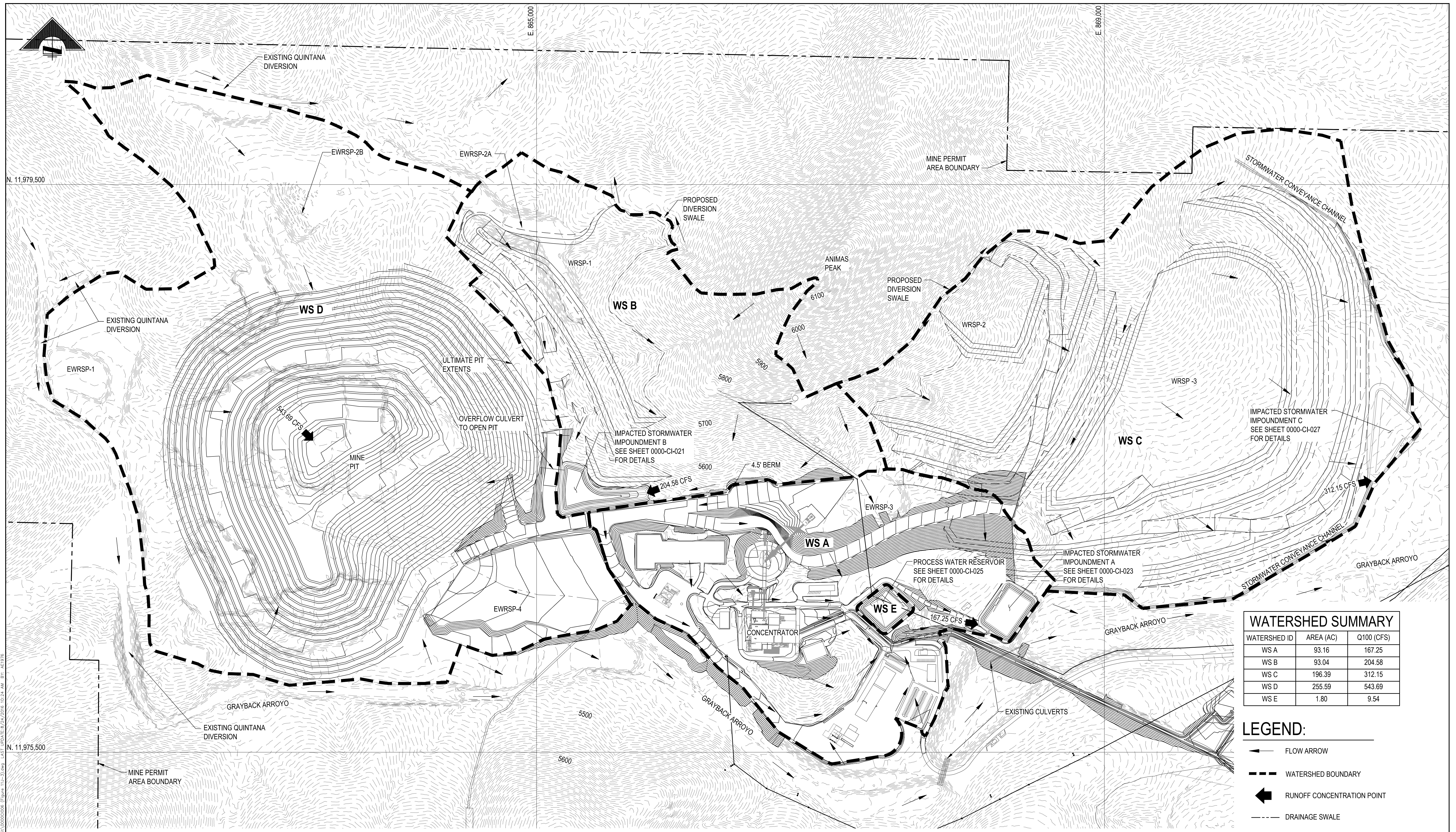
area, will hold water from several sources including reclaimed water from the TSF, captured storm water, and fresh make-up water from the off-site well field.

Two of the storm water impoundments will capture surface water runoff from the waste rock stockpiles and drainage from under the waste rock stockpile areas. The third will capture surface water runoff from the process plant area. Appendix B provides the technical design details for the impoundments and the process water reservoir. Appendix B is also an Appendix to NMCC's Discharge Permit application.

Figure 2-14 shows the various watershed areas that NMCC will develop on-site by grading and contouring the areas to manage and capture surface water runoff. The developed watershed areas shown are as follows:

- Watershed area A (WS A) wherein the process facilities and ancillary plant areas, including the ore stockpile, will be located. EWRSP-3, as shown in Figure 2-14, also called the low-grade ore stockpile in earlier documents, is also located within WS A;
- Watershed area B (WS B) is a portion of the open pit surface drainage area (OPSDA) wherein the proposed new Waste Rock Stockpile no. 1 (WRSP-1) will be located. EWRSP-2A, as shown in Figure 2-14, also called the north waste rock disposal facility in earlier documents is located at the northern edge of WS B;
- Watershed area C (WS C), wherein the proposed new Waste Rock Stockpiles no. 2 and 3 (WRSP-2 and WRSP-3) will be located;
- Watershed area D (WS D) is a portion of the open pit surface drainage area wherein the open pit is located. Existing waste rock stockpiles EWRSP-1, also called the west waste rock disposal facility in earlier documents, EWRSP-2B, also called the north waste rock stockpile in earlier documents, and EWRSP-4, also called the south waste rock stockpile in earlier documents, shown in Figure 2-14, are located within WS D; and
- Watershed E (WS E) represents the footprint of the process water reservoir, the impoundment that holds process water prior to it being introduced into the process circuit. WS E is depicted as a watershed to indicate that the reservoir will not collect any storm water runoff from the plant site. Only precipitation that falls directly on the reservoir will be collected therein.

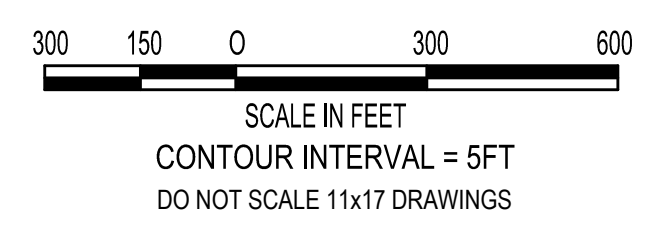
Storm water impoundments will be constructed within watershed areas A, B, and C at locations shown in Figure 2-14 to manage and capture storm water runoff from each area and water that may flow from the interface between the bottom of the WRSPs and the andesite bedrock. Impacted storm water impoundment A is designed to capture and manage surface water runoff from WS A, i.e., the plant area. Impacted storm water impoundment B is designed to capture



WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
WS A	93.16	167.25
WS B	93.04	204.58
WS C	196.39	312.15
WS D	255.59	543.69
WS E	1.80	9.54

- LEGEND:**
- FLOW ARROW
 - - - WATERSHED BOUNDARY
 - ◀ RUNOFF CONCENTRATION POINT
 - - - DRAINAGE SWALE

PLAN VIEW
SCALE: 1" = 300'



PRELIMINARY
FOR AGENCY REVIEW



EWRSP = EXISTING WASTE ROCK STOCKPILE
WRSP = WASTE ROCK STOCKPILE

REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT
0000-CI-021	IMPACTED STORMWATER IMPOUNDMENT A PLAN VIEW														
0000-CI-023	IMPACTED STORMWATER IMPOUNDMENT B PLAN VIEW														
0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT D PLAN VIEW														

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GENERAL SITE CIVIL
MINED MINE AREA
DEVELOPED WATERSHED AREAS

JOB NO. M3 PN-120085
DWG NO. **FIGURE 2-14**
REV. NO. P12 DATE 24 AUG 16



and manage water from WS B which contains WRSP 1. Impacted storm water impoundment C is designed to capture and manage water from WS C which contains WRSP 2 and WRSP-3.

As noted above, developed WS B and D are sub-watersheds of the larger open pit surface drainage area. The entire area naturally drains to the mine pit, with the exception of the EWRSP-4 area, which will be re-contoured during operations to drain to the pit. NMCC has opted to develop these two sub-watersheds separately in order to provide control of the amount of water that will report to the mine pit under normal operating conditions. Surface water runoff from WS D will flow directly to the bottom of the pit as shown in Figure 2-14. As such, no additional impoundment is needed at that location. Water contributed from WS B will be diverted to flow into Impacted Storm water Impoundment B as shown on Figure 2-14.

As described in more detail in Appendix B, Impacted Storm water Impoundment B will be constructed at the lower southwestern corner of developed WS B to capture water from the proposed new WRSP-1 under normal operating conditions. However, should overflow from this impoundment occur as a result of an extraordinary precipitation event, it will flow over the spillway and into the open pit via a culvert, as shown in Figure 2-14 and discussed in Appendix B. This will allow NMCC to control the flow of water into the pit while maximizing the harvesting of storm water for use as process water.

The process water reservoir, i.e., WS E, is designed to hold all of the water recycled from the tailings storage facility, process makeup water and water transferred from the impacted storm water impoundments for introduction into the process circuit. It is designed so that only precipitation that falls directly on the footprint of the reservoir will be captured by the reservoir. Water captured in impoundments A, B and C will be transported to the process water reservoir within 30 days for use as an additional source of make-up process water. Water recycled from the TSF will be continually pumped from the TSF to the process water reservoir.

Liner system design

Appendix B provides the details of the liner design for the impacted storm water impoundments and process water reservoir. The impacted storm water impoundment liner design is consistent with the requirements of the NMED Copper Rules for impoundments that will store impacted water for less than 30 days. It will consist of a compacted liner bedding fill layer, overlain with a 60 mil high density polyethylene (HDPE) geomembrane or equivalent. The liner bedding will be a minimum of six inches of sand or fine soil.

The process water reservoir will be double-lined with a lower 60 mil, or equivalent, HDPE geomembrane and a 60 mil, or equivalent, upper HDPE geomembrane liner. An HDPE geonet will be placed between the liners to serve as the pond leak collection and recovery system



(LCRS) and to minimize pressure on the lower pond liner. This design is in accordance with the requirements of the NMED Copper Rules.

Size and storage capacity

The design storage capacity for each of the impacted storm water impoundments is driven by the size of the watershed, required storm intensity and duration, and freeboard. Table 2-4 provides the storage capacity for each storm water impoundment and the process water reservoir.

TABLE 2-4 Impoundment Storage Capacity		
Impoundment	Size (Acres)	Capacity (Gal)
Impacted storm water impoundment A	2.90	7,307,000
Impacted storm water impoundment B	2.69	5,598,000
Impacted storm water impoundment C	4.44	10,514,000
Process Water Reservoir	2.12	5,434,000

Design capacity of the impoundments is based on anticipated normal operating conditions at the site plus prevention of overflow resulting from a 100-year, 24-hour return interval storm event while maintaining two feet of freeboard. The process water reservoir is sized to contain the water that will be pumped from the water reclaim system and the underdrain collection pond at the TSF plus capacity for necessary process makeup water pumped from the freshwater off-site well field or the impacted storm water impoundments.

2.2.3 Diversions - 19.10.6.602.D.(15)(c)(iv)

This Subsection requires that the applicant provide maps and plans indicating location, size and capacities for the mine facility diversions. A diversion is defined in 19.10.1.7.D.(3) as a channel, embankment, ore other manmade structure constructed to divert water from one area to another. Diversions at the Copper Flat facility include the following:

- The Grayback Arroyo diversion and other diversion structures constructed to divert surface water around and away from the site;
- Diversions at the TSF to impoundments and/or ponds; and
- Diversion channels, ditches swales, curbs, contours and other manmade surface water control features constructed to manage and divert surface water off of waste rock stockpiles and the plant site.

Grayback Arroyo Diversion

With regard to the Grayback Arroyo and other diversion structures, NMCC will utilize some existing water diversion structures, as discussed in more detail below, at the Copper Flat

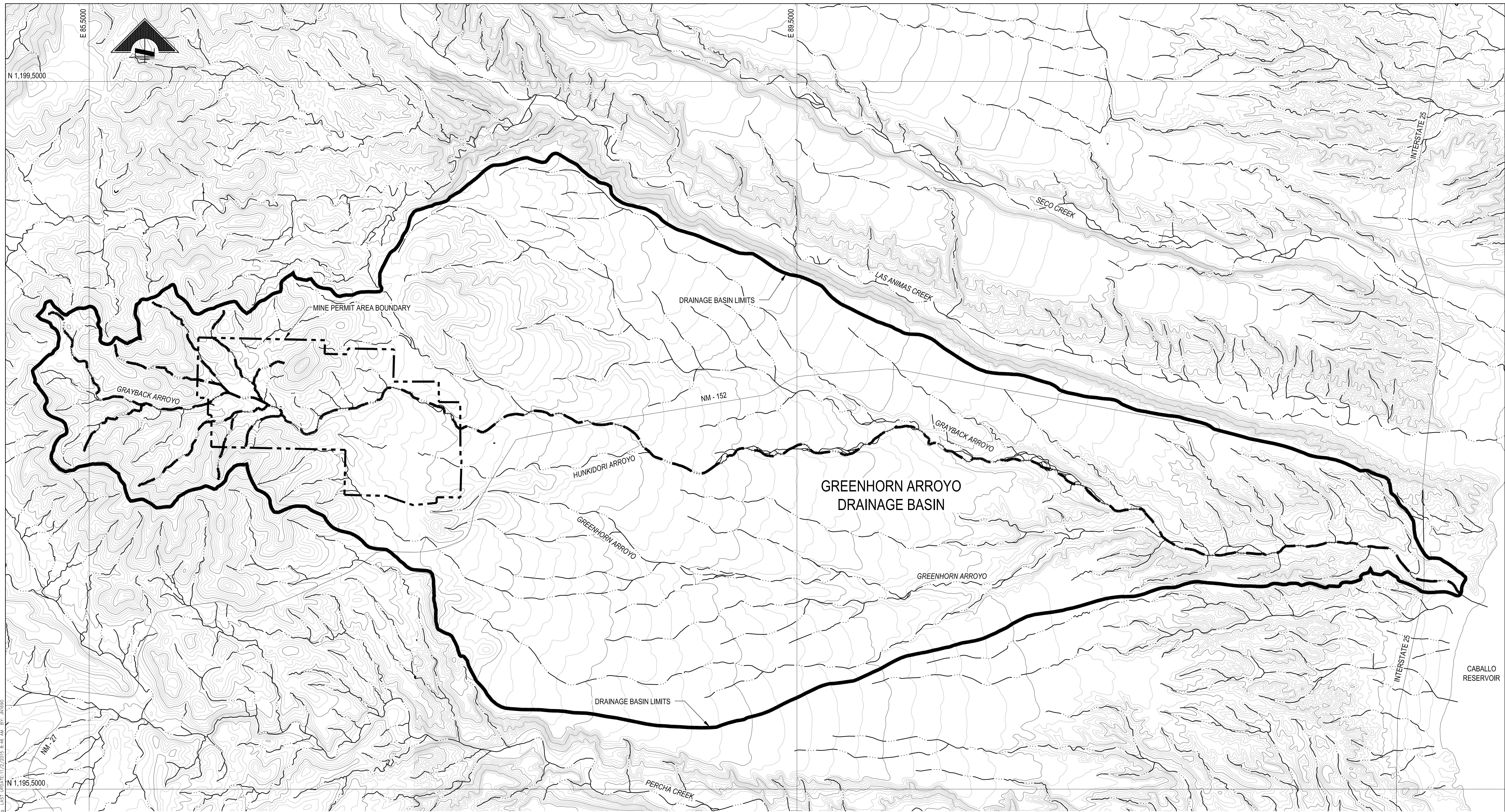


Project. NMCC's consulting engineers, M3, performed a peak discharge and volume analysis of the drainage areas contributing to Grayback Arroyo at the site. The report is included as Appendix D. The purpose of the analysis was to evaluate the existing diversions and water conveyance features existing at the Copper Flat site as to their adequacy in conveying flows from storm events and protecting the site from flooding. The return periods analyzed were the 100-year, 200-year, and 500-year 24 hour storms. The capability of certain existing culverts within Grayback Arroyo to safely pass those storms without overtopping the proposed facility roadway or pipeline corridor was also analyzed. Appendix D provides the information required by the Mining Act regulations and the results of the analysis. Following is a synopsis of the results. This document is also an appendix to NMCC's Discharge Permit application.

Figure 2-15 shows the location of the Copper Flat Project permit area boundary within the Greenhorn Arroyo drainage basin watershed. It shows the Greenhorn Arroyo watershed as it existed prior to any mining. As shown in Figure 2-15, the site is located in the head of the basin. Figure 2-16 provides a closer view of the headwater drainage of the watershed in relation to the mine permit area boundary. Grayback Arroyo and its tributaries naturally begin as the headwaters of the drainage basin and converge at the western side of the site, transecting the mine permit area draining from west to east. The main-stem of Grayback Arroyo enters the western boundary of the site; several small unnamed arroyos, tributary to Grayback Arroyo, enter the site at the northwest and southwest corners of the site. As seen on Figure 2-15, Hunkidori Gulch begins at the eastern edge of the site down-gradient of the operations and drains to the east, away from the site. Greenhorn Arroyo begins off of the southeast corner of the site down-gradient from the operations and also drains east. Hunkidori and Greenhorn Arroyos have no impact upon the Copper Flat facility as they are located down-gradient of the mine. Only Grayback Arroyo and its unnamed arroyos, which enter the site from the west, need be the subject of consideration with respect to surface water run-on and runoff management.

Preproduction site preparation activities conducted by Quintana in the early 1980's included the construction of diversion structures to Grayback Arroyo and unnamed arroyos to divert drainage around the site. These structures are shown in Figure 2-17. Diversion structures were constructed to divert the headwaters of Grayback arroyo and its western tributaries as they entered the western site boundary to the south around the open pit. Another diversion structure, similar in purpose but smaller in size, was constructed at the northwest corner of the site to divert drainage from small tributaries to Grayback Arroyo, diverting them to the north east around Animas Peak away from the site into a sub-watershed that joins Grayback Arroyo east of the of the site boundary.

In addition to the diversion structures, Quintana installed large diameter culverts, as shown on Figure 2-18, where the tailings transport pipeline and the access road cross over Grayback Arroyo. These structures are still in place and will be used to control storm water passing



LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN
SCALE: 1" = 2500'



SCALE IN FEET
CONTOUR INTERVAL = 50'
DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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SCALE: 1" = 2500'	DATE
DESIGNED BY JPN	OCT 15
DRAWN BY JPN	OCT 15
CHECKED BY	
PROJECT MGR	
CLIENT APPR.	

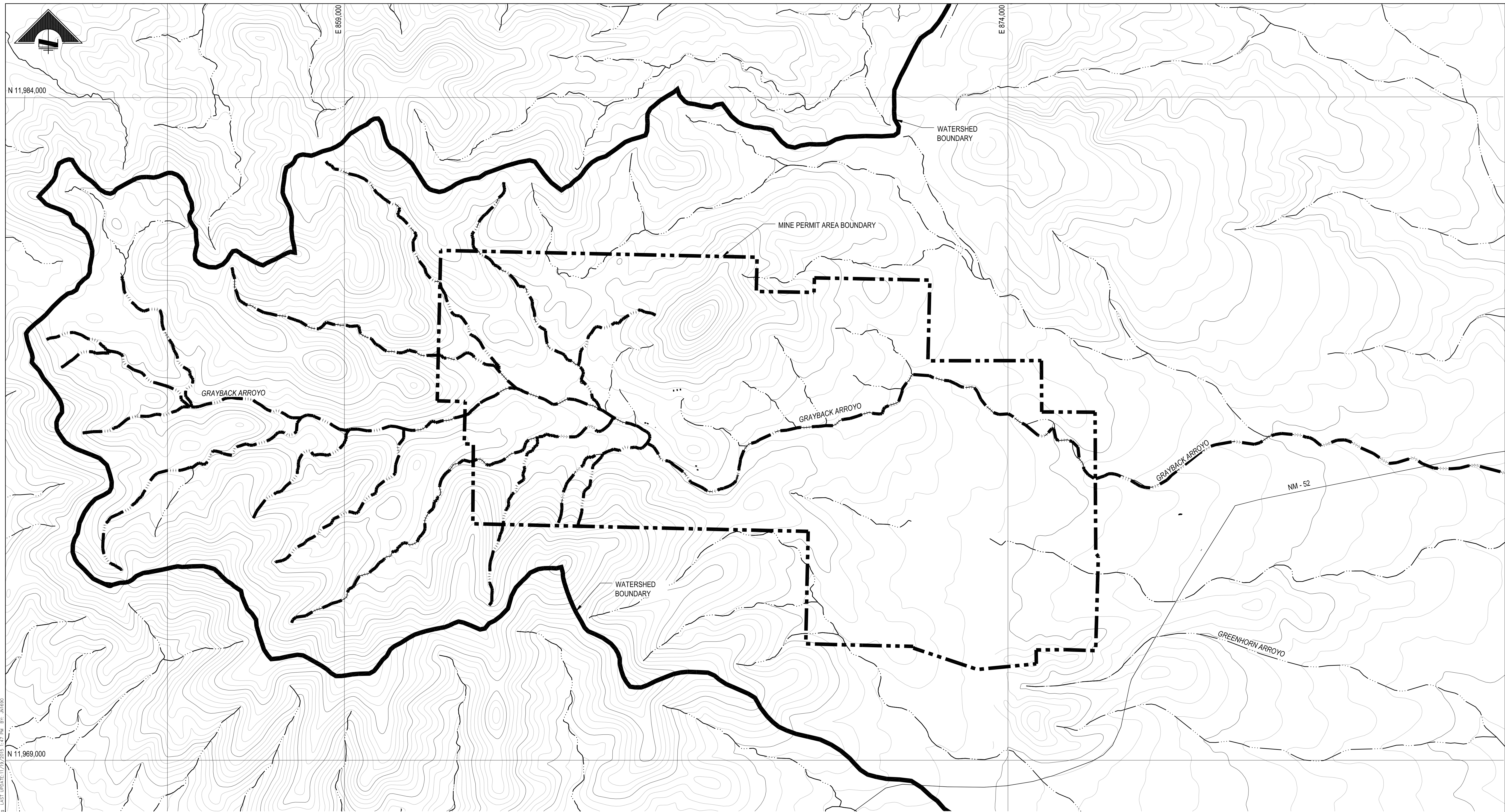
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**GENERAL SITE
CIVIL
GREENHORN ARROYO
PRE-MINING WATERSHED BASIN**

JOB NO. M3 PN-12085
 DWG. NO. **FIGURE 2-15**
 REV. NO. P2 DATE 20 NOV 15

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LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN

SCALE: 1" = 1000'



SCALE IN FEET
CONTOUR INTERVAL = 20 FT
DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS				SCALE: 1" = 1000'		DATE	
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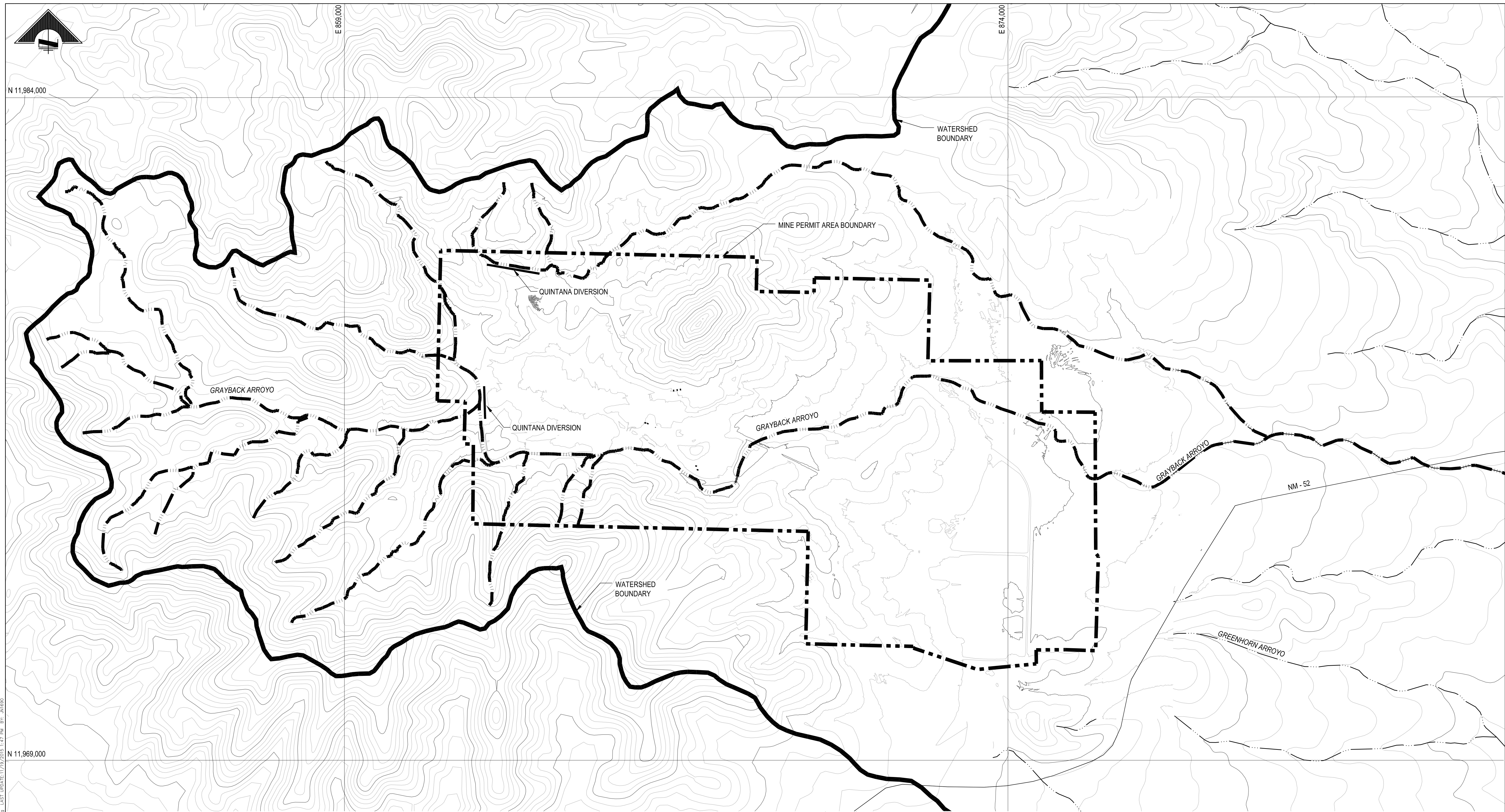
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COPPER FLAT PROJECT

**GENERAL SITE
COPPER FLAT SITE
EXISTING HYDROLOGY
PRE-QUINTANA MINING**

JOB NO. M3 PN-120085
 DWG. NO. **FIGURE 2-16**
 REV. NO. P1 DATE 20 NOV 15

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LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN

SCALE: 1" = 1000'



SCALE IN FEET
CONTOUR INTERVAL = 20 FT
DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS				SCALE: 1" = 1000'		DATE	
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT	DESIGNED BY	DATE		
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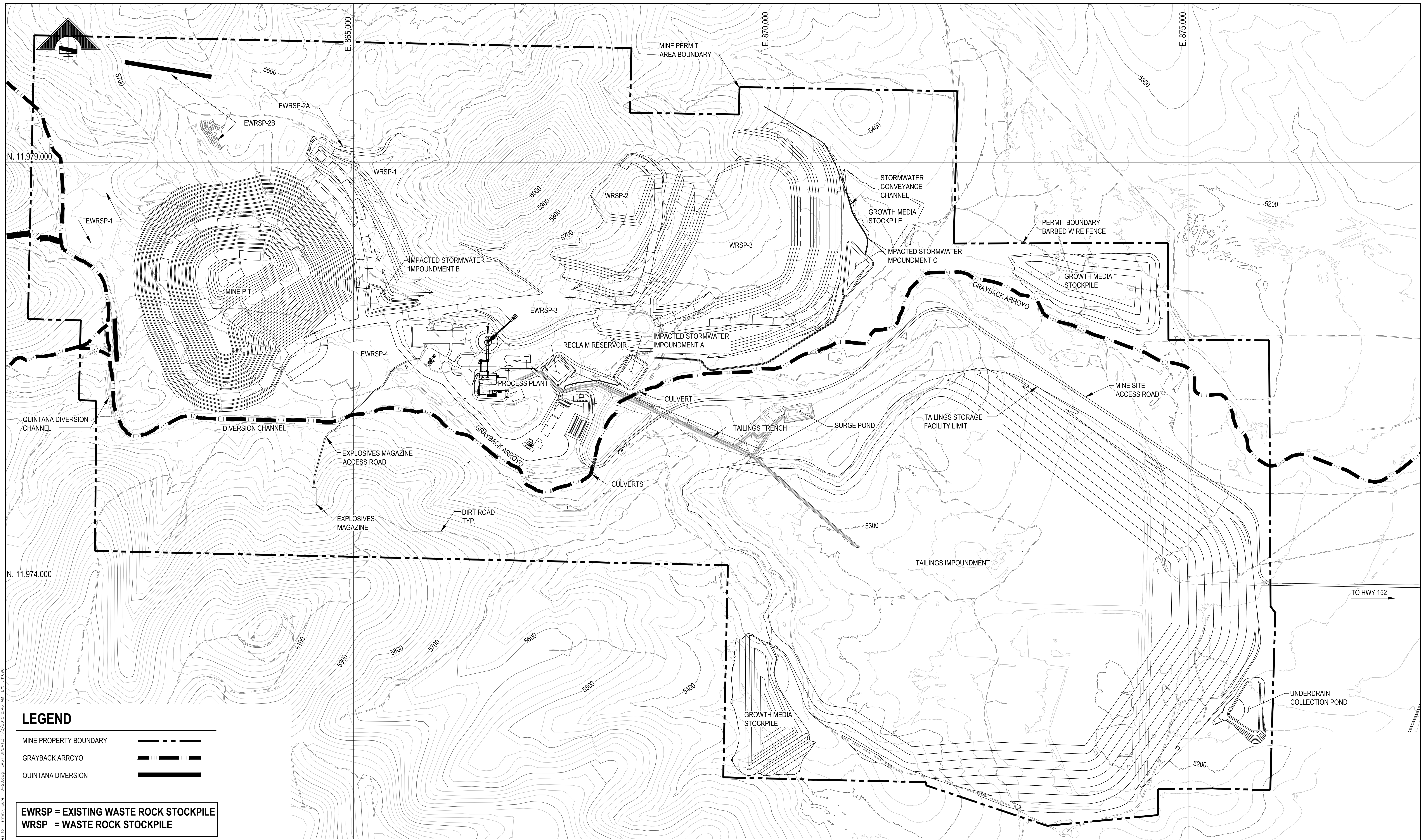
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COPPER FLAT PROJECT

**GENERAL SITE
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POST QUINTANA MINING**

JOB NO. M3 PN-120085
DWG. NO. **FIGURE 2-17**
REV. NO. P1 DATE 20 NOV 15

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LEGEND

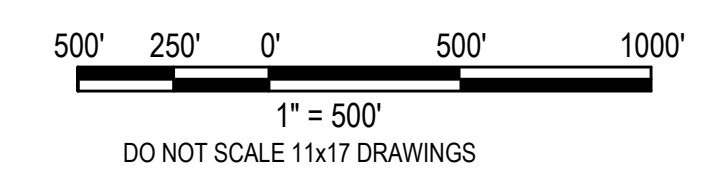
MINE PROPERTY BOUNDARY

GRAYBACK ARROYO

QUINTANA DIVERSION

EWRSP = EXISTING WASTE ROCK STOCKPILE
WRSP = WASTE ROCK STOCKPILE

SITE PLAN
SCALE: 1:500



PRELIMINARY
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT

SCALE: 1" = 500'	DATE
DESIGNED BY JPN	OCT 15
DRAWN BY JPN	OCT 15
CHECKED BY	
PROJECT MGR RKZ	
CLIENT APPR.	

COPPER FLAT PROJECT

SITE GENERAL CIVIL
GRAYBACK ARROYO DIVERSION THROUGH NMCC PROJECT SITE

JOB NO. M3 PN-120085
 DWG. NO. **FIGURE 2-18**
 REV. NO. P1 DATE 20 NOV 15

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through the site. Figure 2-18 also shows these structures in relation NMCC's proposed Copper Flat Project facilities. The pre-mine development natural topography and drainage pattern of the site is shown in Figure 2-19. Sixteen (16) sub-basin watersheds naturally contributed to Grayback Arroyo. The upstream drainages merged in the central portion of the current Copper Flat Project area and passed through to the eastern boundary via Grayback Arroyo.

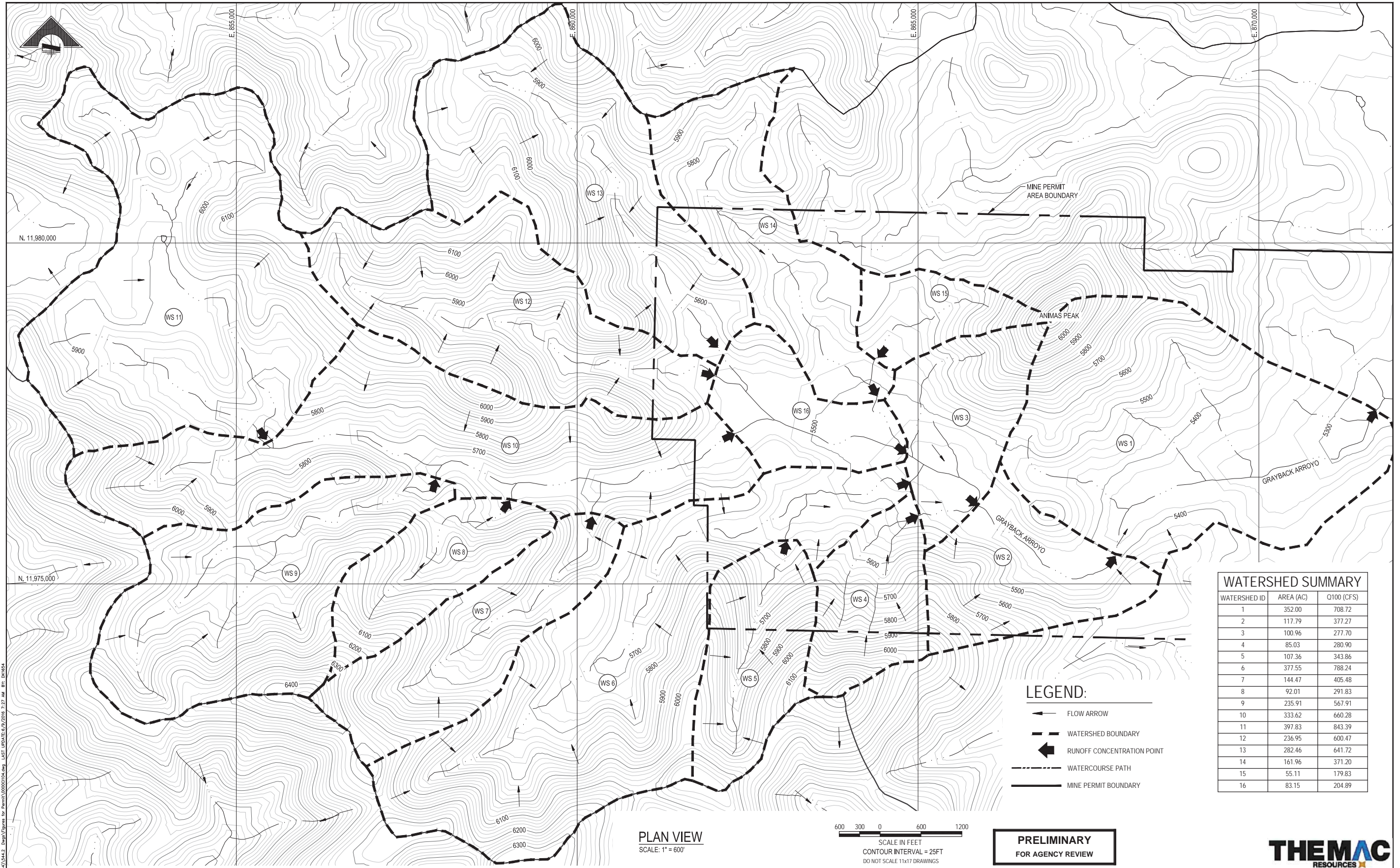
M3's analysis evaluated the diversions and culverts to determine their adequacy in conveying flows from storm events and protecting the site from flooding. Peak discharge and volume analyses for a 100-year, 200-year and 500 year 24-hour storm event was performed for drainage areas contributing to Grayback Arroyo located within the Copper Flat site area. Culvert and channel capacity analysis for the two culvert crossings was conducted for Grayback to determine water surface elevations during the design storm events. Peak flows were analyzed for each sub-basin contributing flow upstream of the site. Figure 2-20 shows the upstream sub-basins in relation to the site.

Diversion of surface drainages away from the mining area was accomplished by Quintana when they developed the site by constructing the diversions described above. Future development of the site by NMCC will result in changes to watersheds shown on Figure 2-19 in the area of the site. Watershed areas no. 15 and 16 will be completely within open pit surface drainage area and will be eliminated as tributaries to Grayback Arroyo. Portions of watersheds 1, 2, 3 and 14 will be incorporated into the site storm water control area of the site and will no longer contribute directly to Grayback Arroyo. These differences can be seen by comparing Figure 2-19 to 2-20.

M3 evaluated the storm flows in the Grayback Arroyo drainage for the 100-year, 200-year, and 500-year 24-hour storm events for the pre-Quintana, i.e., natural conditions in comparison to Post-Quintana NMCC proposed site conditions. The results demonstrate that the existing diversion structures and culverts provide appropriate surface water management of the drainage that is protective of the site.

TSF Diversions

With regard to diversions and other control features at the TSF, Section 6.6, Surface Water Management, of Appendix A, the TSF design report, contains a discussion of the design features of the diversion structures planned to manage surface water runoff at this location. Diversion ditches will be constructed as shown on Figure 2-21, to divert run-on away from the impoundment. The TSF will be built in phases, as shown in Figure 2-21, requiring a series of ditches to be constructed through phase 3 of dam construction to divert run-on to the TSF. When subsequent phases are constructed in the later years of the project, the footprint of the TSF will be such that there will be no run-on of surface water to the TSF as seen on Figure 2-22. The design details for these ditches are contained in Appendix A.



WATERSHED SUMMARY

WATERSHED ID	AREA (AC)	Q100 (CFS)
1	352.00	708.72
2	117.79	377.27
3	100.96	277.70
4	85.03	280.90
5	107.36	343.86
6	377.55	788.24
7	144.47	405.48
8	92.01	291.83
9	235.91	567.91
10	333.62	660.28
11	397.83	843.39
12	236.95	600.47
13	282.46	641.72
14	161.96	371.20
15	55.11	179.83
16	83.15	204.89

LEGEND:

- ← FLOW ARROW
- - - WATERSHED BOUNDARY
- ◀ RUNOFF CONCENTRATION POINT
- · - · - WATERCOURSE PATH
- MINE PERMIT BOUNDARY

PLAN VIEW
SCALE: 1" = 600'

SCALE IN FEET
CONTOUR INTERVAL = 25FT
DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



COPPER FLAT PROJECT
GENERAL SITE
CIVIL
PRE QUINTANA
EXISTING WATERSHED AREAS

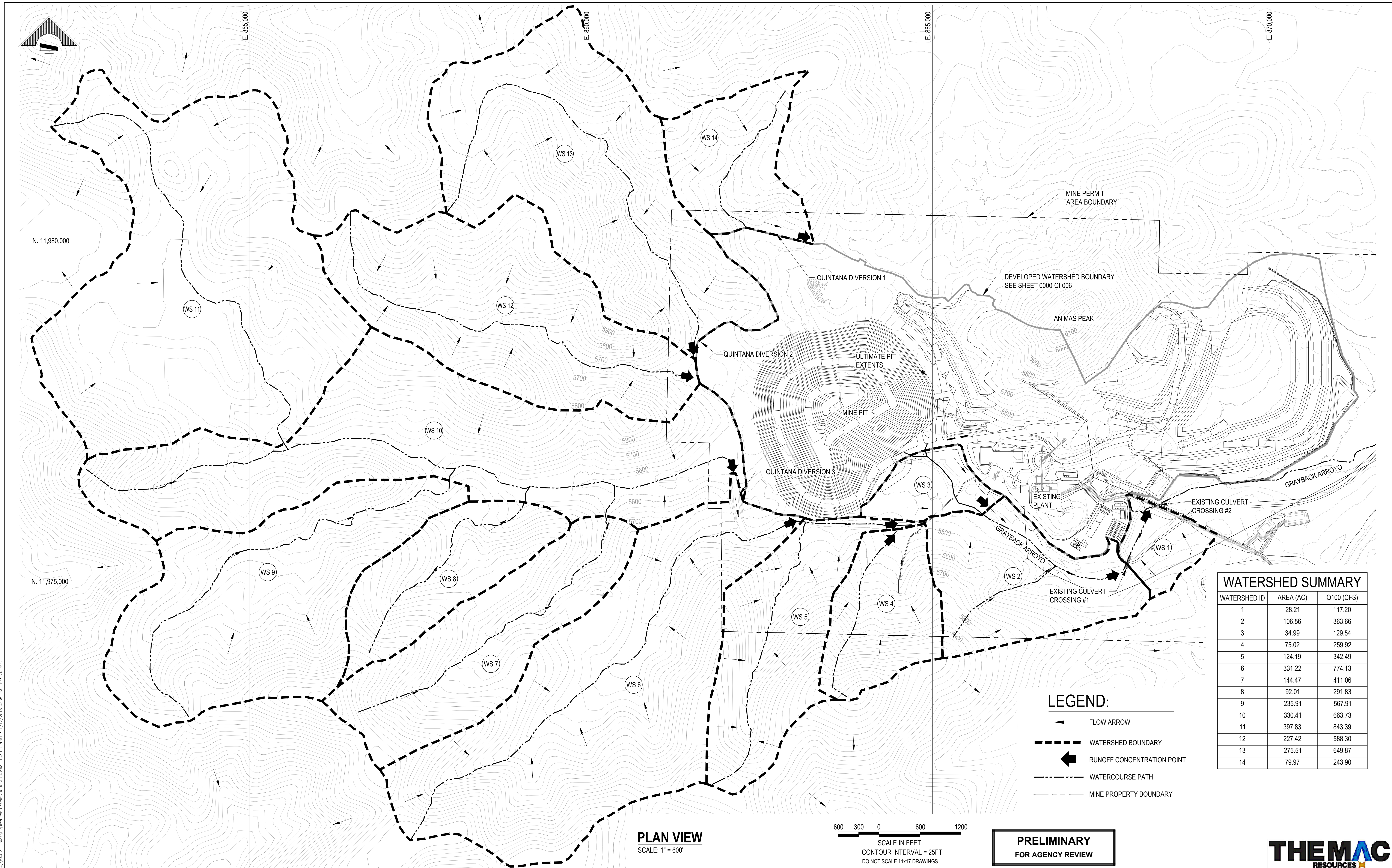
JOB NO. M3 PN-120885
DWG NO.
FIGURE 2-19
REVNO. DATE
P2 09 JUN 16

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SCALE: 1" = 600'
DESIGNED BY: AJE OCT 15
DRAWN BY: AJE OCT 15
CHECKED BY:
PROJECT MGR:
CLIENT APPR:

REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT

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WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
1	28.21	117.20
2	106.56	363.66
3	34.99	129.54
4	75.02	259.92
5	124.19	342.49
6	331.22	774.13
7	144.47	411.06
8	92.01	291.83
9	235.91	567.91
10	330.41	663.73
11	397.83	843.39
12	227.42	588.30
13	275.51	649.87
14	79.97	243.90

LEGEND:

- FLOW ARROW
- WATERSHED BOUNDARY
- ⬇️ RUNOFF CONCENTRATION POINT
- - - WATERCOURSE PATH
- - - MINE PROPERTY BOUNDARY

PLAN VIEW
SCALE: 1" = 600'

SCALE IN FEET
600 300 0 600 1200
CONTOUR INTERVAL = 25FT
DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



REFERENCES		REFERENCES		REVISIONS				REVISIONS								
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	
0000-CI-006	DEVELOPED WATERSHED AREAS															

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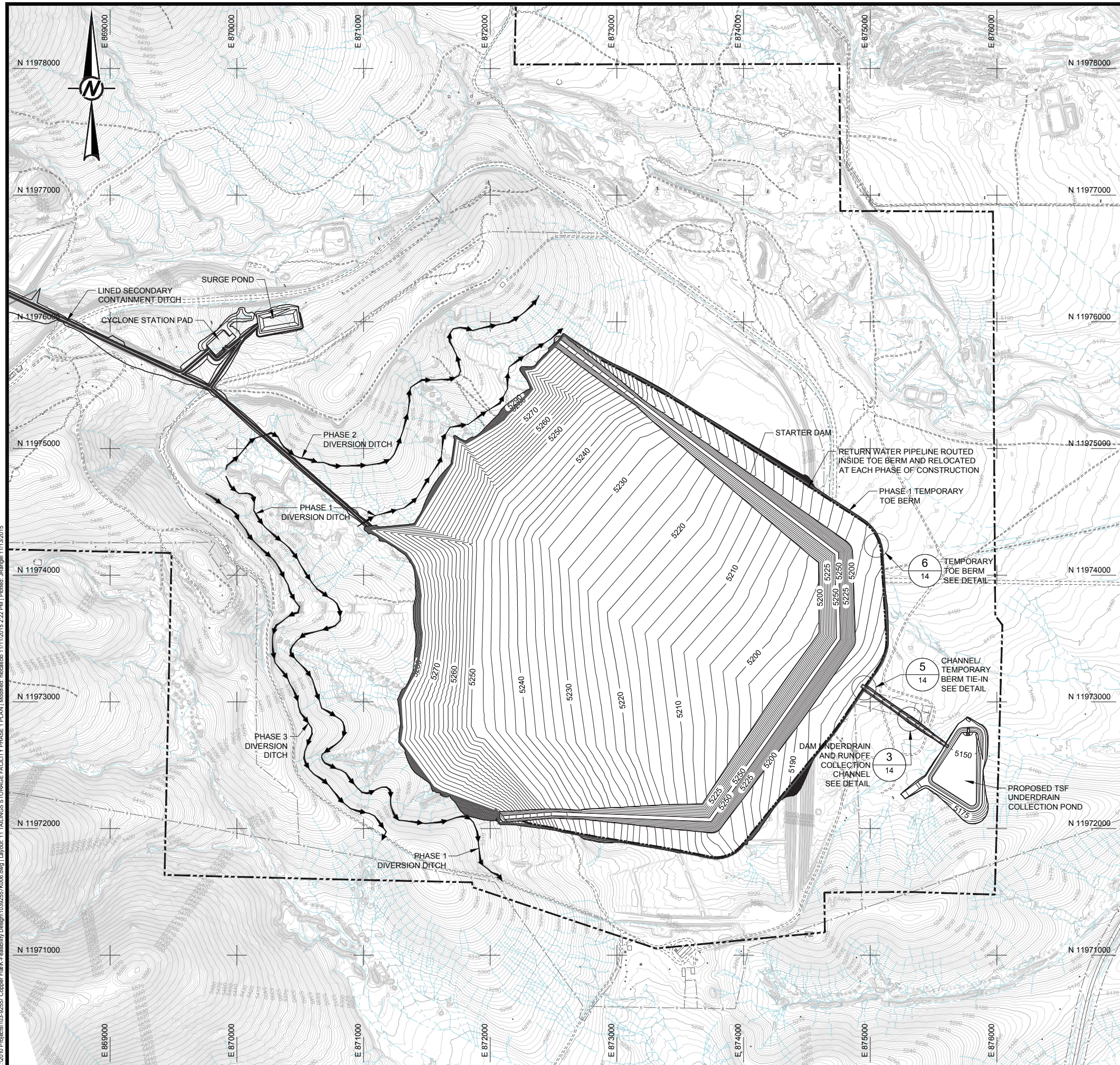
COPPER FLAT PROJECT

GENERAL SITE CIVIL
POST QUINTANA
EXISTING WATERSHED AREAS

JOB NO. M3 PN-120085
DWG. NO. **FIGURE 2-20**
REV. NO. P1 DATE 20 NOV 15

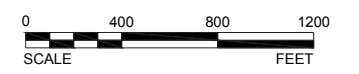
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K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K006.dwg | Layout: 11 TAILINGS STORAGE FACILITY PHASE 1 PLAN | Modified: inccasco 11/11/2015 2:22 PM | Plotted: Range 11/13/2015



LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

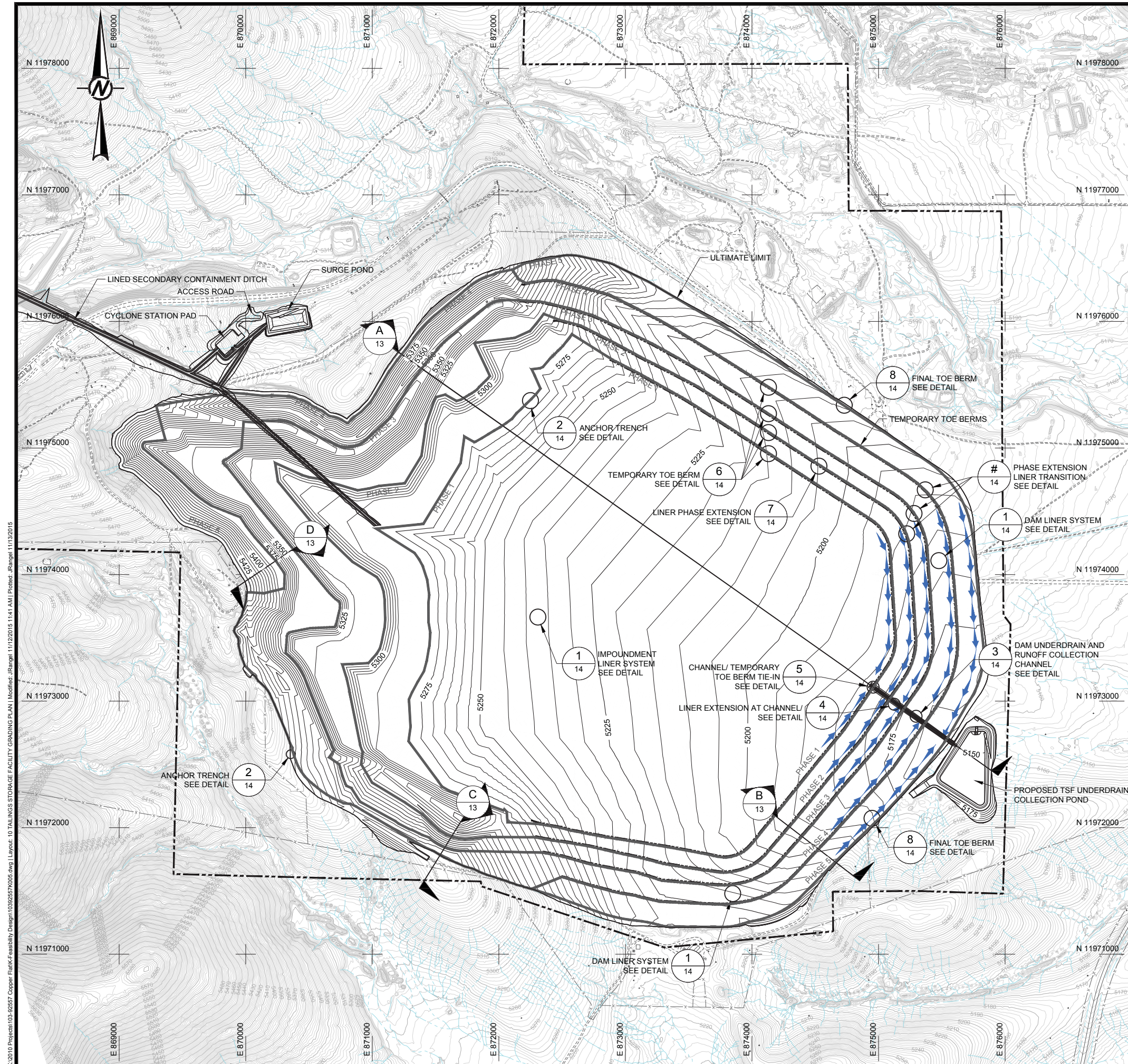
PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TAILINGS STORAGE FACILITY DIVERSION DITCH LOCATIONS

PROJECT No.	103-92557	FILE No.	10392557K006	
DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

11



LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- DRAINAGE
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION

K:\2010 Projects\103-92557\Copper Flat\KFeasibility Design\10392557\K005.dwg | Layout: 10 TAILINGS STORAGE FACILITY GRADING PLAN | Modified: 11/12/2015 11:41 AM | Plotted: 11/13/2015



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
**TAILINGS STORAGE FACILITY
DAM LIFT PHASES**

PROJECT No.	103-92557	FILE No.	10392557K005	
DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

10



Other Diversion Structures

With respect to diversion channels, ditches swales, curbs, contours and other manmade surface water control features constructed to manage and divert surface water off of waste rock stockpiles and the plant site, Appendix B contains a discussion of the design features incorporated into the design of the stockpiles and plant site to manage surface water runoff from these areas.

2.2.4 Disposal Systems – 19.10.6.602.D.(15)(c)(v)

Disposal systems contemplated at the Copper Flat facility include:

- Waste Rock Stockpiles,
- Tailings Disposal Facilities,
- Sewage disposal,
- Solid waste disposal, and
- Hazardous and chemical waste disposal.

Information regarding the waste rock stockpile disposal system is contained in Subsection 2.2.9 below. Information regarding the TSF disposal system is contained in Subsection 2.2.6 below.

Sewage Disposal

With respect to sewage disposal, NMCC will not use a septic tank and leach field treatment system for disposal of domestic wastes at the Copper Flat Mine. Disposal of domestic wastes generated will be accomplished by installing a single packaged wastewater treatment plant to serve the majority of employees and visitors at the mine. Individual portable toilet facilities for outlying areas of the operation will be utilized, as needed.

The packaged wastewater treatment plant will receive and treat domestic wastes from buildings located in the administration, concentrator, and mine shop areas. The packaged system will be sized for a load based on the number of mine employees and visitors expected at the mine during a 24-hour period and applying an average water use of 50 gallons per day per person. Breaking down the employee headcount for the mine by the planned rotation schedule indicates 160 employees per day will be using facilities connected to the package plant. An additional 40 persons per day are assumed to account for visitors and contractors. Based on these figures, a 10,000 gallon per day plant has been selected for Copper Flat Mine. The plant will be located on a pre-existing concrete slab near the main gate as shown in Figure 2-2. The plant will generate effluent treated to secondary treatment levels. Treated effluent from the plant will be piped to the tailings storage facility for disposal in the impoundment. System specifications and installation will conform to State and local regulations. Individual portable toilet facilities will be provided for employees working in outlying areas of the operation such as the pit, mine stockpile areas, the primary crusher, and the TSF. The portable toilets will be maintained by a licensed contractor on a regular basis.



Solid Waste Disposal

Solid non-hazardous waste generated at the site will include paper, wood, scrap metal and domestic trash. These materials will be disposed of in a permitted on-site Class III sanitary landfill on private land permitted by the State of New Mexico, or by other methods approved by the State and Sierra County. When recycling services are available, scrap paper, wood, and scrap metal will be sold for recycling to a dealer and transported off-site. Electronics will be held onsite and recycled appropriately as sufficient quantities are generated.

Hazardous and Non-Hazardous Waste Disposal

The Copper Flat facility will be a small generator of hazardous waste as defined in 40 CFR 260.10. Small quantity generators generate more than 100 kilograms, but less than 1,000 kilograms of hazardous waste per month. Management of hazardous waste materials at Copper Flat will comply with all applicable Federal, State and local requirements. All hazardous waste generated at Copper Flat will be managed and transported off-site by a licensed contractor for disposal in accordance with state and federal regulations.

2.2.5 Pits – 19.10.6.602.D.(15)(c)(vi)

NMCC proposes to construct an open pit mine at its Copper Flat project as shown in Figure 2-2 and discussed in detail in Section 2.1.2. This new facility will entail the expansion of an existing open pit previously developed and operated for a short time in 1982 by Quintana. A portion of the ore body at Copper Flat is exposed at the surface and the ore body is proposed to be mined solely by conventional truck and shovel open pit methods. The proposed mining activities will enlarge the open pit over time to a diameter of approximately 2,800 feet. The area of the pit will be expanded to approximately 165 acres. All material mined will be drilled and blasted and loaded into mine haul trucks for removal from the open pit. Ore will be hauled to the primary crusher and then conveyed to the mill as described below. Waste rock will be placed in designated stockpile areas as described in Section 2.2.11.

2.2.6 Tailings Disposal Facilities – 19.10.6.602.D.(15)(c)(vii)

NMCC will construct a tailings storage facility (TSF) which will include, a lined tailings impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam, and a water reclaim or recycle system to maximize water reuse. The TSF will also include a cyclone plant to separate the tailings coarse and fine fractions and a surge pond to handle potential upset conditions at its Copper Flat Project. Appendix A provides the technical design detail for the TSF. As indicated previously, this document is also part of NMCC's Discharge Permit application.

The footprint of the proposed new TSF and related facilities shown in Figure 2-2 will be approximately 604 acres in size at full capacity. As described in Appendix A, a centerline construction method using tailings sand produced from the underflow of the cyclone plant for



construction of the dam will be utilized. A starter dam will be constructed using borrow material to provide initial storage capacity and to provide a location for initial discharge of tailings. The centerline construction method allows construction of a stable, drained tailings dam using coarse tailings sands while reducing the quantity of fill material required for dam construction. The design for the new TSF will comply with the design and dam safety guidelines of the New Mexico Office of the State Engineer (OSE) Dam Safety Bureau.

The tailings impoundment is designed to store the tailings produced through processing 113 million tons of ore over approximately 11 years. Tailings deposition will occur continuously from ore processing at an annual average rate of approximately 27,890 tons per day. The tailings impoundment will be lined with an 80-mil HDPE, or equivalent liner, placed on a minimum 12-inch thick liner bedding fill material. In the initial phases of construction the bedding material will consist of recovered sand from the old Quintana starter dam. Later phases of construction will require the use of selected crushed and screened native materials or selected local soil be utilized. Bedding material will comply with agency approved specifications.

The TSF will have two separate underdrain systems; a dam underdrain underlying the dam to collect draining water from the coarse sands used to construct the dam, and an impoundment underdrain system overlying the impoundment liner to collect draining water that is collected behind the dam. The liner will extend from the impoundment under the dam and through the drainage collection ditch which will form a lined conveyance to the underdrain collection pond. Both underdrain systems will overlie the liner. The detail of their design is contained in Appendix A.

The dam underdrain will provide the mechanism that will drain water within the dam, out of the structure to the underdrain collection pond. This will allow the dam to become consolidated and stable as it is continually constructed during operations as the sand fraction of tailings is emplaced on the dam. The tailings impoundment underdrain system will allow free water in the impoundment to be drained out from underneath the impoundment to the underdrain collection pond. Water collected in the underdrain collection pond will be recycled to the processing facility. The underdrain, together with the impoundment synthetic liner, will provide significant mitigation against the potential for seepage from the impoundment. It will also contribute to the ability to recycle water from the system to the process facility while contributing to the stability of the TSF.

The drained water will be collected in the collection trench and be routed to the underdrain collection pond. Collected water will be pumped back to the process facility for reuse. The TSF collection trench and underdrain collection pond will also serve to capture surface water runoff routed from the downstream face of the tailings dam via runoff control ditches to the pond.



The TSF underdrain collection pond will be double-lined with a 60 mil, or equivalent, HDPE geomembrane liner. An HDPE geonet will be placed between the liners to serve as the LCRS and to minimize pressure on the lower pond liner. The pond is sized to contain 24 hours of underdrain flow at maximum estimated drainage rates from the dam and impoundment underdrains, as well as runoff from the 100-year, 24-hour storm event of 3.73 inches incident on the downstream dam face. The pond capacity is approximately 12.24 million gallons with 2 feet of freeboard. Design details are contained in Appendix A.

The TSF will also be equipped with a water reclaim system to collect water forming on the surface of the tailings impoundment for return to the processing facility. The water reclaim system will consist of a floating barge located within the tailings impoundment containing pumps to remove water from the impoundment, a pipeline to transport the reclaimed water to the process water reservoir.

The TSF will also have an associated surge pond that will be part of the cyclone plant located at the TSF. The purpose of the surge pond is to capture and temporarily retain tailings materials in the event of a temporary upset at the cyclone plant. It will also provide temporary storage in the event that an upset occurs in the tailings circuit. The surge pond liner will consist of a compacted liner bedding fill layer, overlain with a 60 mil HDPE geomembrane or equivalent. The liner bedding will be a minimum of six inches of sand or fine soil. It is designed to retain tailings and other process water that is not diverted directly to the tailings impoundment in the event of temporary upset conditions.

Under normal operating conditions the surge pond will be empty. It is designed to receive tailings materials and process water on a temporary basis. Feasibility level design capacity of the surge pond is 1.6 million gallons, sufficient to handle the volume of upset conditions plus direct precipitation from a 100-year 24 hour precipitation event with at least 2 feet of freeboard. This design capacity conservatively assumes that an upset would occur during a maximum precipitation event and that the cyclone plant is running at maximum design rates, allowing time for operators to react to the situation. In addition, the pond will be equipped with dedicated pumps and water level actuators to automatically begin pumping materials to the TSF. The pumps will be tied into the site emergency power grid. The process control room will be equipped with emergency alarms that notify the operator of an upset condition immediately to allow the operator to make necessary adjustments in the process, as needed.

2.2.7 Mills (Process Facilities) – 19.10.6.602.D.(15)(c)(viii)

The Copper Flat ore processing facilities will be constructed at the site of the original Quintana processing plant site which is located southeast of the existing open pit as shown in Figure 2-2. The plant facilities will be approximately 128 acres in size. Ore processing will consist of a conventional sulfide flotation plant to extract copper, a molybdenum processing



circuit, and a gravity gold recovery circuit. No smelting, refining or SX/EW operations will be conducted at the Copper Flat site. The plant will produce copper and molybdenum concentrates as well as a small amount of coarse gold concentrate. Figure 2-13 is a conceptual flow diagram of the process. The ore will be fed to the primary crusher and crushed and ground to a fine particle size and then processed through mineral flotation circuits. Ore processing activities will continue 24 hours per day, seven days per week, 365 days per year. The plant will process approximately 11 million tons per year at an average rate of 27,890 tons per day over the life of the project. An overview of the process area is provided herein, including preliminary isometric drawings, to aid the reader in visualizing how the ore is processed.

The process selected for recovering the copper and molybdenum minerals is considered “conventional.” The sulfide ore is crushed and ground to a fine size and processed through mineral flotation circuits. The following items summarize the process operations required to extract copper and molybdenum from the Copper Flat sulfide ore. The major equipment for the mineral processing plant is discussed below.

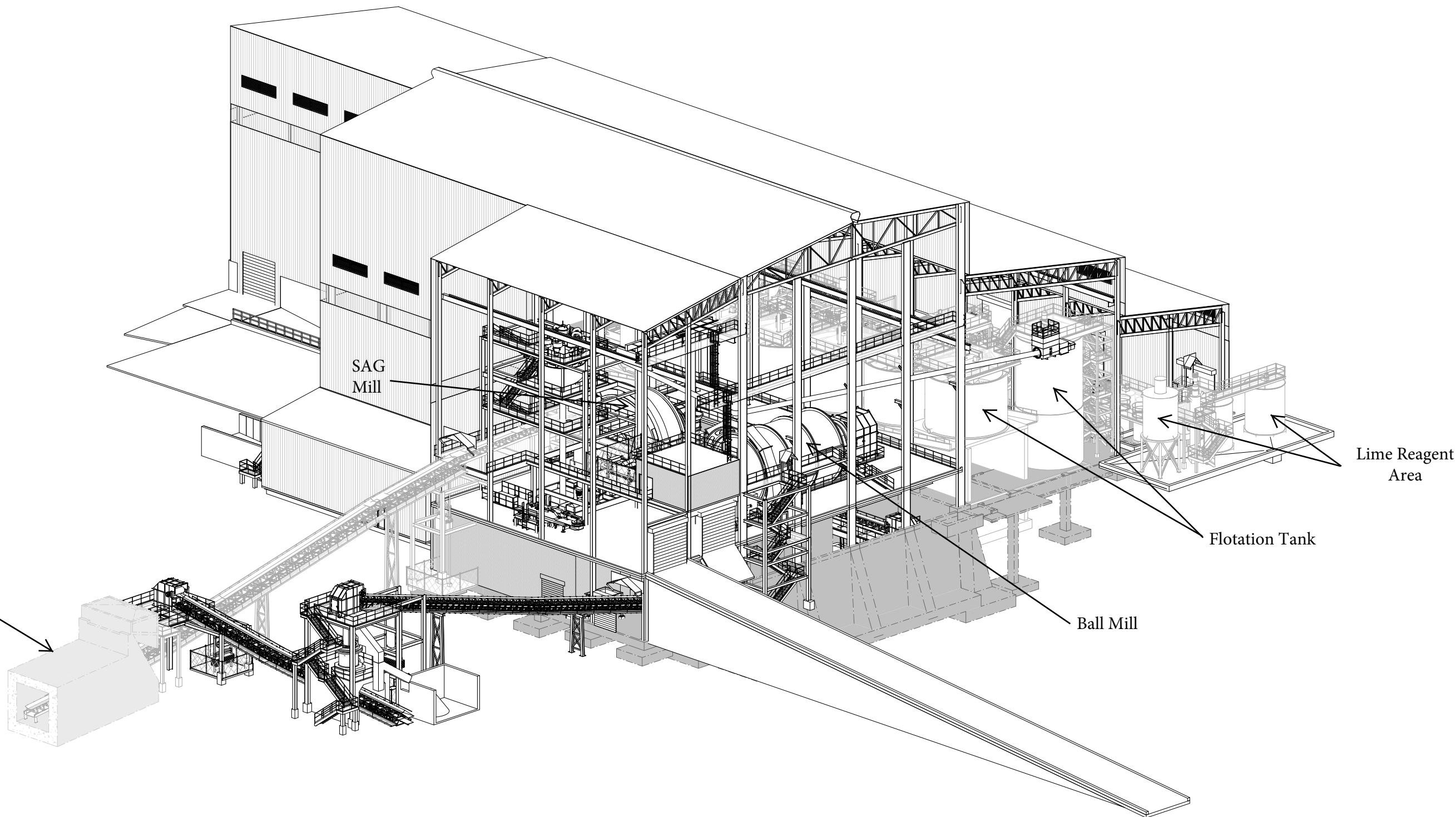
Primary Crushing and Coarse Ore Stockpile

As shown on Figure 2-13, run-of-mine ore will be trucked from the mine to the primary crusher where it will be dumped directly into the crusher dump pocket that feeds a gyratory crusher. A rock breaker will be installed at the dump pocket for use on oversized material. Primary crushed ore will be withdrawn from the crusher discharge pocket by a variable speed, crusher discharge apron feeder. The crusher discharge feeder will feed the coarse ore conveyor that will discharge to coarse ore stockpile. The crushing production rate will be monitored by a belt scale mounted on the conveyor. Tramp iron will be removed using a self-cleaning magnet that will be located at the transfer point between the crusher discharge feeder and the stockpile feed conveyor.

As seen on Figure 2-23, an existing reclaim tunnel is beneath the stockpile location. Ore will be withdrawn from coarse ore reclaim stockpile by variable speed apron feeders. The feeders will discharge to a conveyor belt that feeds the SAG mill in the grinding circuit. Fugitive dust will be controlled with water sprays at the discharge of the stockpile feed conveyor. Dust control in the coarse ore stockpile area will be by dry dust collector systems installed as part of the crushing area.

Crushing and Grinding

Ore from the coarse ore stockpile will be fed through the reclaim tunnel as shown on Figure 2-23 to the SAG mill. Ore will be ground to final product size in a SAG mill and ball mill grinding circuit. The SAG mill will operate in closed circuit with SAG mill discharge screen and pebble



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crusher. The SAG mill discharge screen undersize will flow by gravity to the primary cyclone feed sump and the screen oversize will be transported by conveyors to the pebble crusher. Pebble crushing will be conducted in a short-head cone crusher. The SAG mill discharge screen oversize can bypass the pebble crusher via diverter gate ahead of the pebble crusher. The bypassed screen oversize will feed a second diverter gate which will either feed the pebble crusher conveyor that transports crushed pebble to the SAG mill or dump pebbles to the pebble stockpile. Tramp iron, and broken media will be removed using a self-cleaning belt magnet that will be installed over the SAG mill oversize conveyor ahead of the pebble crusher.

Secondary grinding will be performed in a ball mill which will operate in closed circuit with a cluster of hydrocyclones. The ball mill will discharge into a cyclone feed sump. The contents of the sump will be transferred using a slurry pump to a hydrocyclone cluster. Most of the hydrocyclone underflow slurry will report to the ball mill, but a portion of the underflow will be taken through a Knelson-type gravity concentrator circuit to collect gravity recoverable gold. The gravity separation circuit will consist of two Knelson-type concentrators, each of which will have an upstream scalping screen to remove oversize material. The gravity concentrates will pass through magnetic separators for removal of tramp iron and broken grinding media. The tailings from the gravity concentrators will be pumped back to the cyclone feed sump.

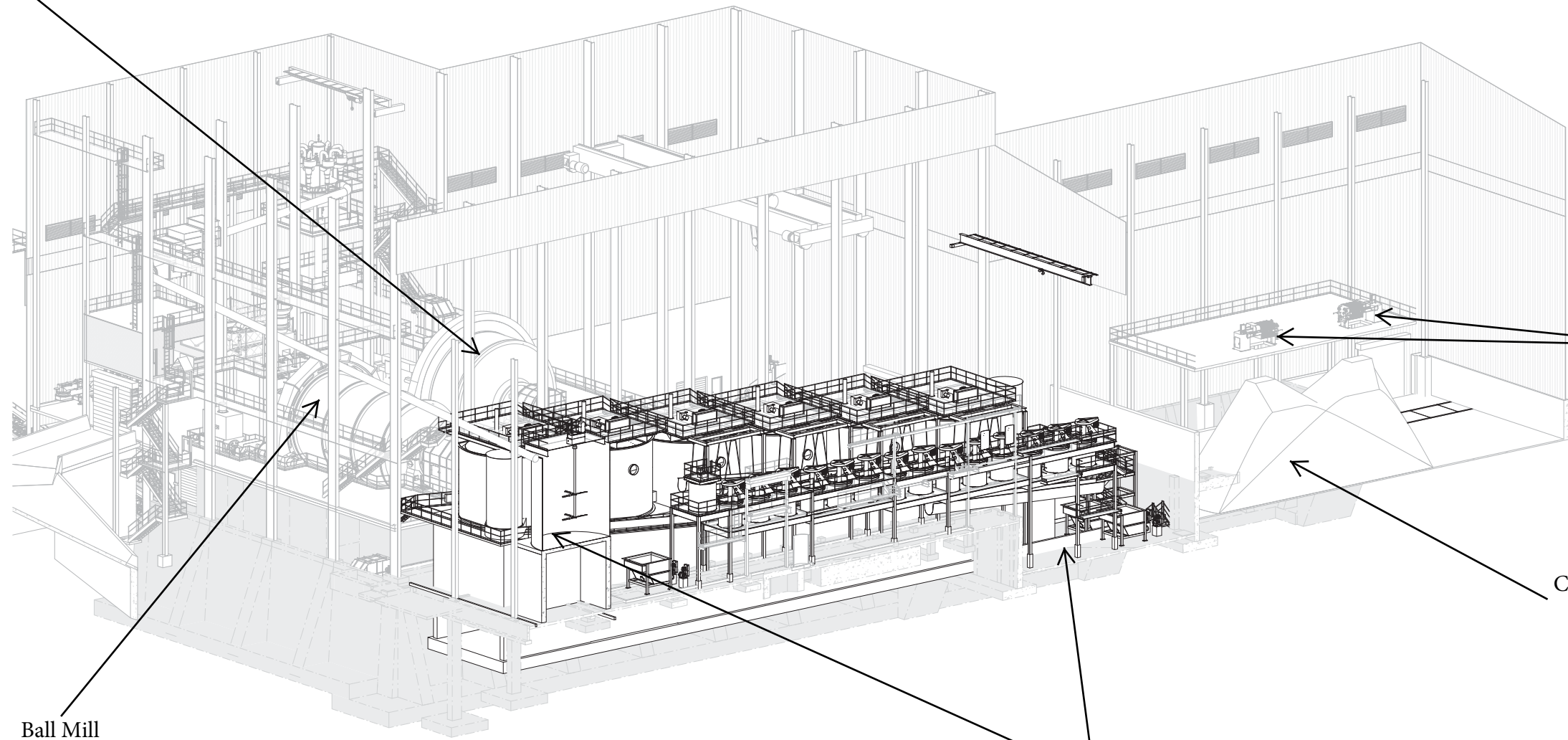
Hydrocyclone overflow (final grinding circuit product) will flow by gravity to the rougher flotation conditioning tank ahead of the rougher flotation cells. The overflow slurry will be sampled and analyzed for metallurgical control prior to flotation.

Grinding balls will be added to SAG mill and ball mill using ball loading systems. Lime slurry will be added to the SAG mill and ball mill feed to adjust the pH of the slurry. If needed, lime slurry may also be added to the primary grinding sumps. In addition, fuel oil will be added to the SAG mill feed to aid in molybdenite collection.

Flotation

Primary grinding hydrocyclone overflow will flow by gravity to the bulk flotation circuit (see Figures 2-23 and 2-24). The bulk flotation circuit will consist of a conditioning tank, one row of rougher flotation cells, a rougher concentrate vertical regrind mill, one row of first cleaner/cleaner-scavenger flotation cells and two second cleaner column flotation cells. The rougher flotation row will consist of six tank type rougher flotation cells with a drop between each cell. Flotation reagents will be added to the hydrocyclone overflow in the rougher flotation conditioning tank where the slurry will be agitated to allow the reagents to react with the ore particles before feeding to the rougher flotation cells. The flotation concentrate from the last four rougher flotation cells will be transported by gravity to the rougher concentrate regrind sump. Tailing from the rougher flotation cell will be sampled and transported to the tailings treatment facility. Rougher flotation tailings will be sampled for metallurgical control.

SAG Mill



Ball Mill

Copper Concentrate Pressure Filters

Copper Concentrate Product

Copper Flotation Area

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FIGURE 2-24

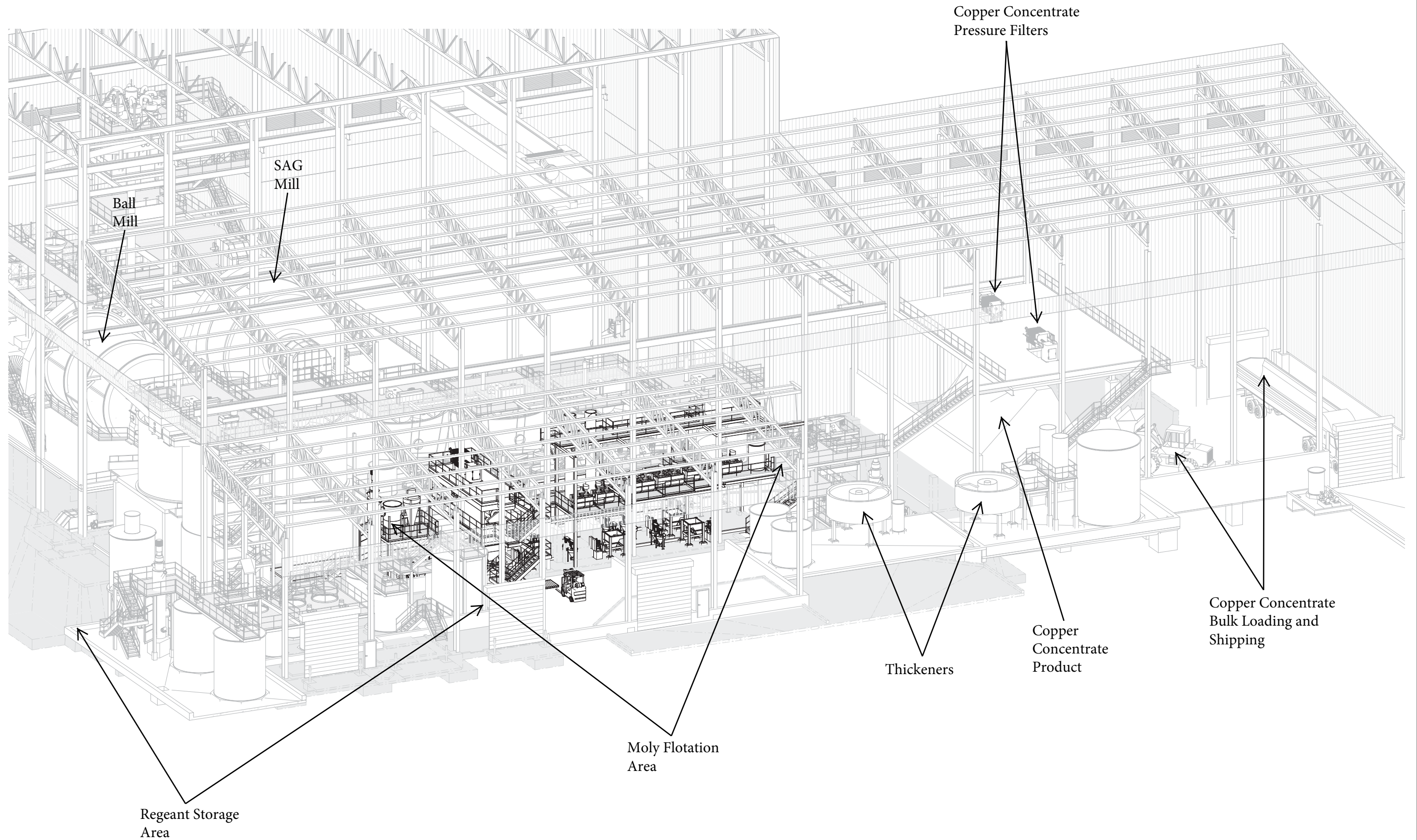


Concentrate from the last four rougher flotation cells, combined with first cleaner-scavenger concentrate and regrind cyclone underflow, will be pumped from the copper regrind cyclone feed pump box to copper regrind cyclone cluster. Copper regrind cyclone underflow will flow by gravity to the copper regrind mill. The copper regrind mill will operate in closed circuit with hydrocyclone.

Molybdenite Flotation

Figure 2-25 identifies the location of the molybdenum circuit. Regrind cyclone overflow, final regrind circuit product, will flow by gravity to an agitated conditioning tank. Second cleaner tailing and flotation reagents will also be added into this tank. Conditioning tank discharge will flow by gravity to the first cleaner/cleaner-scavenger flotation cells. The first cleaner flotation will consist of eight tank type flotation cells. Concentrate from the first cleaner flotation cells will be pumped to the concentrate distribution box. Tailing from the first cleaner flotation cells will flow by gravity to the first cleaner-scavenger cells. The first cleaner-scavenger flotation circuit will consist of six tank type rougher flotation cells. Concentrate from the cleaner-scavenger cells will be returned to the bulk concentrate regrind circuit sump using a froth pump. Tailing from the cleaner-scavenger cells will be pumped back to the rougher flotation circuit. Cleaner-scavenger tailing may be sent to the final tailing sump. Two discharge ports in the concentrate distribution box will direct the slurry to the feed inlets for the second cleaner column cells operated in parallel. Second cleaner tailing slurry will be pumped from the two columns to the first cleaner conditioning tank from where it will be pumped to the first cleaner flotation cells. The second column cleaner concentrate slurry will be pumped to the copper-moly concentrate thickener. A blower will supply air to bulk second cleaner column cells to the bulk mechanical rougher, first cleaner/cleaner-scavenger and second cleaner bulk flotation tank cells. Flotation reagents will be added at several points in the bulk flotation circuit. Flotation reagents will be added at several points in the bulk flotation circuit.

Bulk second cleaner concentrate will be transported to the copper-moly concentrate thickener. Thickener overflow will be pumped by a horizontal centrifugal pump from an overflow sump to the plant reclaim water storage tank. Copper-moly thickener underflow will be pumped by a slurry pump to the molybdenite flotation circuit. The molybdenum flotation circuit will consist of one row of copper-moly separation (rougher) flotation cells, one row of molybdenite first cleaner flotation cells, a moly regrind circuit, one molybdenite second cleaner flotation cell, and one molybdenite third cleaner flotation cell. The copper-moly separation (rougher) flotation row will consist of eight mechanical rougher flotation cells. Concentrate from the copper-moly separation (rougher) cells will be pumped by froth pump to the molybdenite first cleaner flotation cells. Tailing from the copper-moly separation cells will flow by gravity to the copper concentrate thickener. The molybdenite first cleaner flotation row will consist of four mechanical cells. Concentrate from the molybdenite first cleaner cells will be pumped by froth pump to the feed sump of the molybdenite concentrate regrind circuit. Tailing from the



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molybdenite first cleaner flotation cells will flow by gravity to the feed launder of the copper concentrate thickener. Concentrate from the moly first cleaner cells will be sampled. Molybdenite concentrate regrinding will be performed in a vertical mill. Molybdenite first cleaner flotation concentrate will feed the vertical mill which will discharge into the moly regrind discharge pump box and pumped to the moly second cleaner column cell.

Slurry will be pumped by the second moly cleaner feed pump to the molybdenite second cleaner flotation column cell. Tailing from the molybdenite second cleaner column cell will be pumped to the moly first cleaner flotation cells. Molybdenite second cleaner concentrate will be pumped to the moly third cleaner column cell. Concentrate from the molybdenite third cleaner column cell will be pumped to the agitated moly filter feed tank that feeds the moly filtering and drying circuit. Tailing from the molybdenite third cleaner column cell will be pumped to the molybdenite regrind cyclone feed sump. A blower will supply air to the second and third moly cleaner column cells.

Flotation reagents will be added at several points in the molybdenite flotation circuit. Molybdenite circuit process streams will be sampled for metallurgical control. Sample points include: concentrate from the copper-moly separation (rougher) flotation row, concentrate from molybdenite first cleaner flotation row, and concentrate from molybdenite third cleaner column cell.

Copper Concentrate Dewatering

Final copper concentrate will be a combination of tailings from copper-moly separation flotation and moly first cleaner flotation cells. Each tailing stream will be sampled before being transported to the copper concentrate thickener feed box from where the combined tailings will be fed to the copper concentrate thickener. Thickener overflow will be pumped from the overflow pump box by a horizontal centrifugal pump to the copper-moly concentrate thickener feed box. Thickener underflow will be pumped by variable speed horizontal centrifugal slurry pump to the copper concentrate stock tank from which it will be pumped to the copper concentrate filters. Horizontal centrifugal pumps will transport copper concentrate slurry from agitated concentrate stock tank to two automatic plate-and-frame pressure filters. The filters will discharge batches of filter cake to a copper concentrate stockpile at the east end of the mill building. Filtrate and filter wash water will be returned to the feed box of the copper-moly concentrate thickener. A front-end loader will fill highway haulage trucks with copper concentrate on a built-in truck scale. A wheel wash system for the concentrate haulage trucks will ensure that concentrate will not be carried out of the load out area.

Molybdenite Concentration

Molybdenite concentrate from the molybdenite third cleaner column cell will flow by gravity to the moly filter feed tank. Concentrate from the agitated tank will be pumped to a disc filter for



dewatering. Filter cake will discharge to a conveyor that feeds a Holoflite-type hot oil dryer. The dryer will discharge via a screw conveyor to the molybdenite concentrate storage bins. Filtrate will be pumped to the copper-moly thickener.

Tailings Dewatering

Tailings from the bulk rougher flotation row will flow by gravity to a tailings separation facility where hydrocyclones will be used to separate the coarser sands to build the dam. Underflow sands will be pumped to the crest of the tailings storage facility (TSF). Cyclone overflow fines will be pumped to the TSF and spigotted to the interior of the impoundment. Further settling of the fines produces a supernatant water pond at the back (upstream) of the impoundment that will be reclaimed and pumped to the Reclaim Reservoir. Drainage from the tailings materials will be captured by a synthetic liner and conveyed via a drainage system to a underdrain collection pond. Collected seepage water will be pumped to the Process Water Reservoir.

Reagent Storage and Mixing

Reagents requiring handling, mixing, and distribution system include:

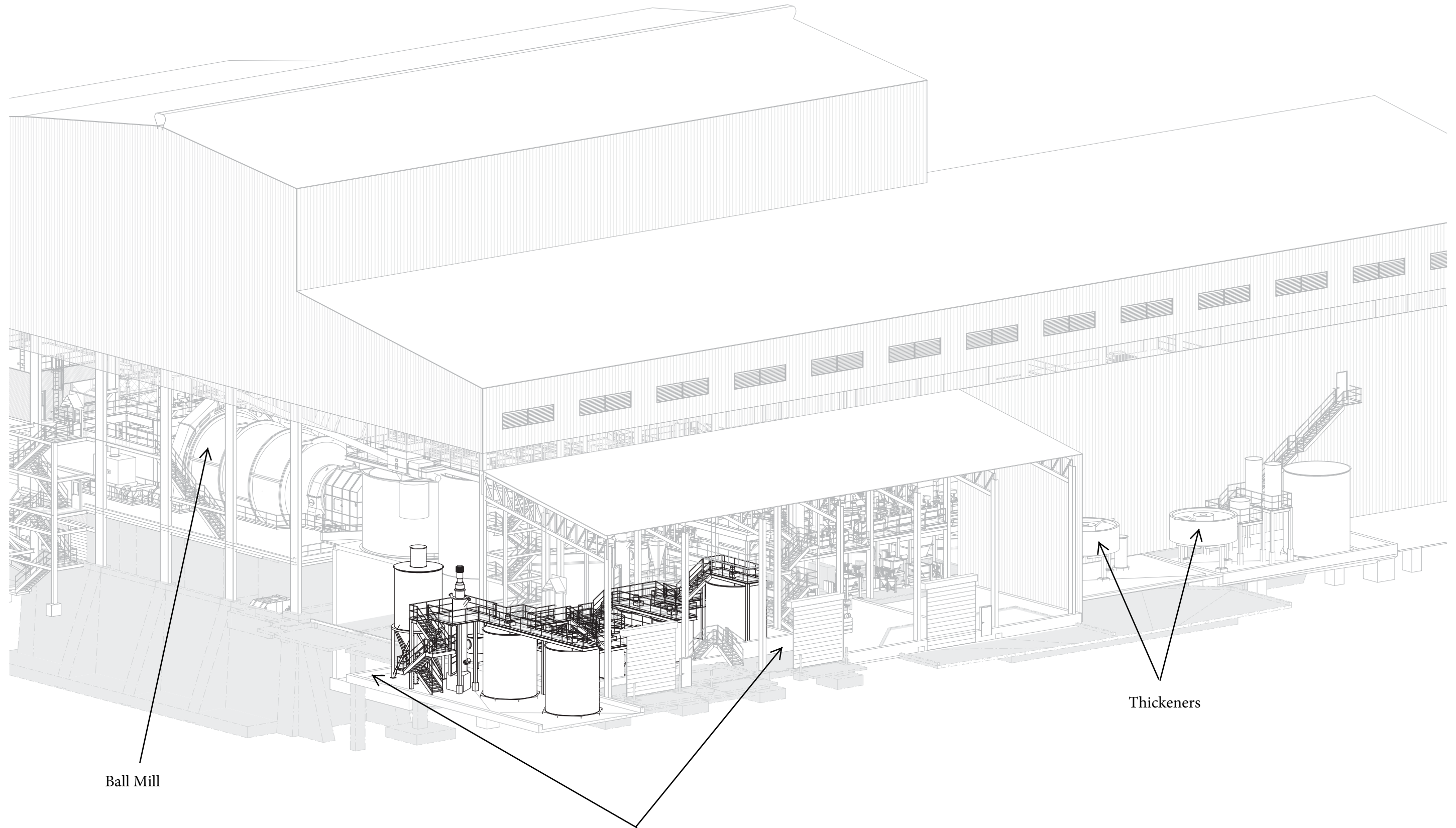
- Potassium Amyl Xanthate (PAX, collector)
- Methyl Isobutyl Carbinol (MIBC, frother)
- Sodium Hydrosulfide (NaHS), copper mineral depressant)
- Flocculant
- Pebble Lime (CaO, pH modifier)
- Fuel oil (molybdenite collector)
- Butyl dithiophosphate
- Antiscalant

Figure 2-26 identifies the general reagent area.

Process Water Handling and Disposal

All water used in the processing of ore will be either be contained within the ore processing circuit, discharged to the TSF, or be in the copper and molybdenum concentrates as moisture content. The plant water system will consist of a lined process water reservoir and a plant process water storage tank. Both will be located near the plant site. Water will be delivered to the reservoir via pipelines. Water reporting to the reservoir includes the following:

- Recycled process water from the TSF;
- Makeup water from the fresh water tank (water from the well field);
- Copper/molybdenum concentrate thickener and copper concentrate thickener; overflows; and
- Storm water from the impacted storm water impoundments.



Ball Mill

Reagent Storage Area

Thickeners

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FIGURE 2-26



Water from the process water reservoir will be pumped to the plant process water tank. The tank will deliver water the processing areas for use as needed a via gravity-flow pipeline. Approximately 73 percent of the water required for processing ore will be provided by recycling water back from the TSF and storm water harvesting. Approximately 23 percent of the water used for processing ore will remain entrained within the tailings. The remaining 4 percent will be lost to evaporation or as moisture in the concentrates. The amount of water in the concentrates will be less than 1 percent of the total water used for processing ore.

2.2.8 Water Treatment Facilities - 19.10.602.D.(15)(c)(ix)

The Copper Flat Project is designed to prevent water discharge into the environment. Therefore, no water treatment facilities other than the packaged waste water treatment facility discussed in Section 2.2.4, above, are planned.

2.2.9 Storage Areas - 19.10.6.602.D.(15)(c)(x)

Planned storage areas for equipment, vehicles, chemicals and solutions at Copper Flat will all be located within the area described as the plant facilities as shown in Figure 2-2. All equipment and vehicles will be utilized and maintained within the confines of the mine permit area. There will be areas specified during operations for parking of mine equipment and vehicles when not in use such that any leakage and/or potential spillage from them will be captured within contained and curbed areas of the facility and not released off-site.

As shown in Figure 2-14 and discussed in Section 2.2.2, all surface runoff is managed so that it reports either to the open pit or an impacted storm water impoundment. Appendix C provides a discussion of the various containment areas designed within the process area to contain the chemicals and solutions utilized at the site to ensure that all potential releases are managed and contained. Appendix C was produced, in part, to address the NMED's Copper Rule with respect to describing proposed sumps, tanks, pipelines and truck and equipment wash units, including information for each unit regarding its location purpose construction material, dimensions and capacity. Appendix C also provides the information requirements of 19.10.6.602.D.(15)(c)(x) of the Mining Act Rules. For example, Drawing No. 0000-CI-008 in Appendix C is a scaled map of the location of the various process facility containment areas. Drawing No. 0000-GA-050 is a scaled map of the concentrator area identifying the containment arrangement for all of the process tanks, including the locations of the sumps and tanks. Drawing No. 1010-AR-012 is a scaled map of the truck shop tank farm showing the location of the tanks and sump. Drawing No. 1010-GA-010 is a scaled map of the fuel station showing the location of the tanks and sumps. Drawing No. 1010-GA-001 is a scaled drawing showing the location of the Truck Wash and its sumps or settling tanks.



2.2.10 Topsoil & Topdressing Stockpiles – 19.10.6.602.D.(15)(c)(xi)

Figure 2-2 identifies the location of three topsoil and topdressing stockpiles (also called Growth Media Stockpiles) that will be developed as part of the site development and construction phases of operation. The general term growth media is used rather than a more specific term such as topsoil, because various natural materials would be stockpiled during construction of the mine for use as growth media during reclamation. Primary considerations for selection of growth media are the quantity required to support reclamation and the available water holding capacity of the materials (BLM DEIS November 2015, p. 3-140). The GMSPs will be made up of soils and underlying suitable unconsolidated alluvial and colluvial materials salvaged from areas where the TSF and waste rock stockpiles will be constructed as discussed in the Golder Supplemental Soils Investigation of July 8, 2013, submitted to the MMD and NMED. The report was submitted as part of NMCC's Copper Flat Mine Baseline Data Report Addendum dated July 17, 2013 (NMCC 2013) as Appendix C, Supplemental Soils Investigation.

Golder collected soil samples during a geotechnical investigation conducted in December 2012 and January 2013 as part of their studies conducted in designing the TSF (see Appendix A). The soils investigation provided sufficient information to develop additional insight about the presence of potential cover materials on-site for reclamation and quantify soil resources available. As shown in Figure 2-2, GMSP No. 1 will be located at the southwest corner of the TSF. GMSP No. 2 will be located north of the TSF north of Grayback Arroyo. GMSP No. 3 will be located east of WRSP-3.

Golder's supplemental soils investigation provides the soils data gathered, including sample and field descriptions of soils in and around the footprint of the proposed TSF and WRSP-3. The information has been utilized to develop salvage strategies for the GMSPs in conjunction with construction of the TSF and WRSP-3 and the Reclamation and Closure Plan (see Appendix E). Section 3.0 of Appendix C of Golder's soils investigation discusses the soil resource characterization conducted, including physical and chemical properties, and reclamation suitability. Section 4.0 of the investigation provides estimates of cover material available.

2.2.11 Waste Rock Stockpiles 19.10.6.602.D.(15)(c)(xii)

NMCC will construct three new waste rock stockpiles (WRSP) in conjunction with operation of its Copper Flat Project as discussed previously in Section 2.1.3. NMCC considers that all material excavated from a mine facility that is not ore or clean topsoil is waste rock. Figure 2-2 shows the location of each of the WRSPs relative to the mine and the process area. Figures 2-3 through 2-11 show the design and construction sequencing for the mine pit and the new proposed WRSPs beginning at the preproduction stage through the life of the mine. As indicated in Section 2.1.3, the proposed WRSPs will be built generally to a configuration of 3 horizontal to 1 vertical slope. This design consideration is an example of NMCC's commitment to "operating for closure", or "design for closure", as it facilitates reclamation at the end of the



mine life. Each lift within the stockpile will be approximately 75 ft. high and be placed at angle of repose (1.5 horizontal to 1 vertical) with bench setbacks left between lifts sufficiently wide enough to maintain the 3 horizontal to 1 vertical 1 overall angle for the stockpile.

Surface water runoff collection trenches will be constructed, as needed, to collect and route runoff from the proposed stockpiles to the storm water impoundments describe above. These trenches will be constructed into the andesite bedrock to prevent water from entering the alluvial surface material down-gradient of the WRSP and in a manner to maximize positive flow while minimizing the potential for ponding and erosion.

The planned storage or disposal capacity of the proposed new WRSPs over the life of the mine is as follows:

- WRSP-1 – 3.16 million tons
- WRSP-2 – 8.64 million tons
- WRSP-3 – 32.89 million tons

In addition to the proposed new stockpiles, there exist four small waste rock stockpiles at the site (i.e., EWRSP-1, EWRSP-2A and 2B, EWRSP-3, and EWRSP-4) and generated by the previous Quintana operation. Their location is shown on Figure 2-1. EWRSP-1 is located at the western edge of the open pit surface drainage area (OPSDA) and contains approximately 486,000 tons of material. EWRSP-2A and 2B are located at the northwest side of the site within the OPSDA and contain approximately 760,050 tons of material in total. EWRSP-3 is located next to the primary crusher within the plant facility area and contains approximately 333,300 tons of material. Approximately 123,000 tons of this material consists of unprocessed ore remaining on-site at the end of Quintana’s operations, 24,000 tons of which is a small amount of unprocessed run-of-mine ore, 44,000 tons of which is crushed ore contained in the coarse ore stockpile area, and 55,000 tons of which was removed from the coarse ore stockpile and utilized to backfill the process building foundations. EWRSP-4 is located southeast of the mine pit and contains approximately 1.0 million tons of material. As discussed later herein in the Reclamation and Closure Plan, the EWRSPs will be reclaimed either during operations as part of NMCC’s contemporaneous reclamation or at the end of the life of mine at closure.

2.2.12 Other Facilities and Structures – 19.10.6.602.D.(15)(c)(xiii)

In addition to the Copper Flat facilities described above within the permit area boundary, there are several other ancillary facilities and structures, located off-site, existing and proposed, that will contribute to the project. These facilities are located within nine mill site claims held by NMCC east of the Copper Flat site as seen on Figure 2-12. These mill site claims are on federal land managed by the Bureau of Land Management (BLM). Seven mill site claims are clustered together in the southern half of Section 30, T5S, R5W and the northern half of Section 31, T5S,



R5W, south of State Highway 152. An eighth mill site claim is located in the southeast quarter of Section 28, T15S, R6W, north of Highway 152, and the ninth mill site claim is located in the southeast quarter of Section 25, T15S, R6W, just south of the highway. Each mill site is five acres in size. Portions of these sites have been previously developed and disturbed during installation of the water wells, pipeline and access roads installed by Quintana in the late 1970's or early 1980's to provide water for their operations at Copper Flat. NMCC will operate these wells to provide water for the process facilities. The mill sites will also be utilized for other water-related infrastructure uses such as staging and storage areas for booster tanks, pumps and electrical equipment, maintenance, and monitoring. Access to the mill sites will be along existing unpaved roads. The land is also grazed by cattle.

In addition to the mill site claims controlled by NMCC, Figure 2-12 also shows the location of the proposed substation site in the northeast corner of Section 36, T15S, R6W, land owned by the State of New Mexico and managed by the New Mexico State Land Office (SLO). This 30 acre area is the proposed location of a proposed 10 acre power substation that will be installed and tied into an existing high voltage power line in order to provide the power needed to operate the mine. Access to the substation will be along an existing unpaved road.

2.3 Wildlife Impacts Contingency Plan – 19.10.6.602.D.(15)(d)

Impacts to wildlife from operation of the Copper Flat Project are not expected to be significant (BLM DEIS Nov. 2015, p. 2-95). At the completion of mining activities, the site will be restored to conditions and standards that meet approved post-mining land uses. These uses will include native plant communities similar to surrounding undisturbed areas for wildlife habitat, and grazing land potentially suitable for livestock. Once reclamation is successfully completed, wildlife populations would be expected to return to existing (i.e., pre-mining operation) levels (BLM DEIS Nov. 2015, p. 3-137 and 138).

The Mining Act regulations require that a contingency plan be developed for mitigating impacts to wildlife when there has been an emergency or accidental discharge of a toxic substance that may impact wildlife. It is highly unlikely that there will be any emergency or accidental discharge of toxic substances from the Copper Flat facilities. All process chemicals, diesel fuel, gasoline, hydraulic oils, lubricants, antifreeze and other such liquids will all be stored in such a manner so as to protect them from accidental discharge. All tanks, reagent storage areas and process areas have been designed to provide secondary containment per regulatory requirements. In addition, the processing facility and its related storage areas where toxic substances could be housed is designed such that accidental spills or other upset conditions that may occur will be routed to sumps and/or lined secondary containment ditches that will transport those materials to a surge pond for collection and then pumped either back to the process facility or directly to the TSF. The design of these sumps and ditches is discussed in the various design documents of the appendices.



Borrow areas will be kept free of steep walls and will be sloped and stabilized to allow for safe wildlife entry and exit and prevent erosion. NMCC will construct BLM-approved fencing to prevent livestock from entering the pit, WRSPs, and TSF. Fences of appropriate height will be constructed around water and solution ponds to keep out larger wildlife such as deer and antelope. In areas where a higher level of security or safety is needed, such as the mine substation, chain-link fences suitable for wildlife exclusion will be erected. Gates or cattle guards will be installed along roadways within the proposed mine area as appropriate. NMCC will monitor the fences on a regular basis and repairs will be made, as needed. In the event that livestock manage to enter the proposed mine area via a gate or opening in a fence, the grazing permittee will be contacted immediately. NMCC will assist, as requested, in moving these animals out of the proposed mine area.

The use of avian exclusion devices will be employed, as needed, to prevent deleterious exposure of birds to toxic chemicals or conditions used or created by mining and mineral processing operations.

NMCC's operations Spill Prevention, Control and Countermeasure Plan (SPCC) will provide contingencies to mitigate potential impacts from emergency or accidental releases of petroleum substances, including safeguards and quick clean-up measures to prevent detrimental impacts to humans and wildlife. All other potentially toxic materials will be stored in secured facilities that will exclude wildlife entry.

2.4 Sediment Control – 19.10.6.602.D.(15)(e)

The Copper Flat facility is designed to be a zero discharge facility. As such, sediment control is an important design feature at the site. As described in Appendix D, Grayback Arroyo and its tributaries up-gradient of the site have been diverted entirely around the site. Therefore, the only sedimentation that will be potentially produced will be from surface water runoff from several on-site sources including the mine pit area, the waste rock stockpiles, the process plant facilities and the TSF.

A Storm Water Pollution Prevention Plan (SWPPP) will be developed for construction and maintained during operation. Sediment control will be achieved by the use of BMPs including regrading, seeding and mulching, silt fences, straw bale dams, diversion ditches with energy dissipaters, and rock check dams at appropriate locations during construction and operation. Diversion structures, including existing structures, will divert run-on away from disturbed areas. All sediment control structures will be monitored and maintained on a regular basis. During operations, all runoff from the plant site will be directed into impacted storm water impoundments and other ponds, as discussed below. During reclamation, all ponds will be backfilled, re-contoured and graded, surfaces covered with top dressing, and vegetated.



2.4.1 Open Pit

The open pit surface drainage area is a closed basin that will capture all surface runoff at the bottom of the pit. As such, sediments from this area will be managed entirely within the OPSDA and will not contribute sediments to surface water drainages during operations or upon reclamation. The water collected at the bottom of the pit during operations will all be utilized to control dust within the OPSDA and, to the extent allowed, elsewhere in the operation.

2.4.2 Waste Rock Stockpiles

Sediments produced from the waste rock stockpiles will be managed by constructing surface water trenches as may be necessary during operations to capture, route, and divert runoff into impacted storm water impoundments. Appendix B provides the details of the design of these impoundments and associated trenches, including the criteria for their size, dimension, and capacity.

Runoff from proposed WRSP-1, 2, and 3, as seen on Figures 2-2 and 2-14, will be managed by routing it to Impacted Storm Water Impoundments B and C. Runoff from existing waste rock stockpiles EWRSP-1, EWRSP-2B and EWRSP-4 will be captured in the OPSDA as shown in Figure 2-14. In addition, as discussed in the Reclamation and Closure Plan, EWRSP-1 and EWRSP-2B will be reclaimed during operations, further reducing sediment production. Runoff from the area of EWRSP-2A will be captured in Impacted Storm Water Impoundment B as EWRSP-2A will be incorporated into proposed WRSP-1 during operations. EWRSP-4 will be re-contoured to route runoff into the OPSDA as part of the site preparation process. The area will be utilized as an equipment laydown area during operations. The southern out-slopes of this stockpile will be reclaimed as discussed in the Reclamation and Closure Plan to protect against potential sediments being introduced into Grayback Arroyo from that location. Runoff from EWRSP-3 will be captured in Impacted Storm Water Impoundment A, which will manage runoff from the plant facilities.

2.4.3 Process Plant Facilities

As shown on Figures 2-2 and 2-14, runoff from the process plant facilities will be managed by contouring the plant site footprint to route runoff to Impacted Storm Water Impoundment A. Runoff from all of the facilities located in this area including the primary crusher, crushed ore stockpile, concentrator, fuel station, reagent storage area, administration building, parking lot, EWRSP-3, and all of the associated ancillary areas will be captured in Impacted Storm Water Impoundment A. Runoff from the laydown area containing EWRSP-4 will be routed to the open pit during operations.

An additional impoundment, i.e., the process water reservoir, can also be seen on Figures 2-2 and 2-14. It should be noted that no runoff from the process plant facilities area will be directly captured by this impoundment. However, the process water reservoir is designed such that all



runoff water captured in the Impacted Storm Water Impoundments will be transferred to the process water reservoir for use in the process.

2.4.4 Tailings Storage Facility

The location of the TSF is shown on Figure 2-2. Appendix A contains detailed discussion of the design of the TSF. Potential sediment contribution from runoff at the TSF will be from the outer slopes of the dam. Table 6 (page 20) of Appendix A contains the storm water diversion design criteria utilized. The TSF dam will be constructed in phases using the coarse tailings sand from the process as shown in Figure 2-22. Storm water will be diverted around and away from the footprint of the dam as shown in Figure 2-21 via diversion ditches. As such, all precipitation that falls within the footprint of the tailings impoundment from the crest of the dam inward will remain within the impoundment. Precipitation that may fall up-gradient of the impoundment will be diverted around and away from the impoundment. Therefore, the only opportunity for sediment production at the TSF will be from precipitation runoff from the outer slopes of the embankment. As shown on Figure 2-22, a lined runoff collection trench will be constructed at the toe of the dam at each phase of construction to capture surface water runoff from the outer slopes of the dam and route it to the underdrain collection pond located at the southeastern corner of the TSF as seen in the Figure cited above. The purpose of the underdrain collection pond, in addition to capturing and managing storm water runoff from the tailings dam, is to capture water from the dam underdrain and impoundment underdrain collection systems of the TSF. The collected water will be pumped to the process water reservoir discussed above for use in the process.

2.4.5 Sediment Management at Construction & Reclamation

BMPs will be used to limit erosion and reduce sediment in runoff from the Project facilities and disturbed areas during construction, operations, and reclamation. Sections 3.0 of the updated MORP and Section 5.0 of the Reclamation and Closure Plan discuss structural and operational BMPs that will be used to minimize erosion and control sediment. Disturbance will be limited to preserve existing vegetation to the maximum extent possible. Following construction activities, areas such as cut and fill embankments and GMSPs will be seeded as soon as practicable and safe. Revegetation of disturbed areas will reduce the potential for wind and water erosion. Concurrent reclamation will be utilized to the extent practicable to accelerate revegetation of disturbed areas. All sediment and erosion control measures will be inspected periodically and repairs performed as needed. Additional details regarding BMPs will be included in the SWPPP permit required for mine construction and operation.

2.4.6 Non-Point Source Sediment Monitoring

As indicated above, Copper Flat is designed to be a zero discharge facility. As such, there are no non-point discharges that NMCC anticipates will require monitoring. However, NMCC will manage non-point sources to the extent they may occur during construction or reclamation



with the use of BMPs including such things as seeding and mulching of disturbed areas, silt fences, straw bale check dams, diversion ditches with energy dissipaters, and rock check dams, as necessary. NMCC has submitted a proposed monitoring plan to the NMED (see Appendix E of the DP Application), pursuant to the requirements of the NMED Copper Rules and its proposed Discharge Plan. This plan has a surface water monitoring component proposed that NMCC will incorporate into this MORP update. Figure 2-27 identifies five (5) surface water quality sampling locations that will be monitored per the plan. NMCC believes this component of the DP monitoring plan will provide the information required by the Mining Act regulations.

The following mitigations address potential non-point source charge management as identified in the BLM DEIS for Copper Flat (BLM DEIS November 2015, p. 3-46):

- Prior to initiation of mine construction or other surface disturbing activities, NMCC will obtain a Multi-Sector General Permit for Storm water Discharges Associated with Industrial Activity and comply with all requirements of that permit.
- Prior to initiation of mine construction or other surface disturbing activities, NMCC will provide final designs for storm water diversion structures and other associated BMPs for review.
- The SWPPP and all associated inspection and maintenance records will be available for inspection upon request.

Because non-point source pollution is regulated by existing laws and regulations and NMCC must comply with those laws, potential effects to water quality from non-point source discharge of sediments are not considered to be significant.

2.5 Post-mining Land Use – 19.10.6.602.D.(15)(f)

The New Mexico Mining Act rules define Post-Mining Land Use (PMLU) as;

“a beneficial use or multiple uses which will be established on a permit area after completion of a mining project. The PMLU may involve active management of the land. The use shall be selected by the owner of the land and approved by the Director [of MMD]. The uses, which may be approved as PMLUs, may include agriculture, commercial or ecological uses that would ensure compliance with Federal, State or local laws, regulations and standards and which are feasible.” 19.10.1.7. P. (5) NMAC.

The Copper Flat Project will be developed and operated on a combination of federal land administered by the BLM and private land owned by NMCC. The current land uses of federal lands administered by the BLM in the area of the Copper Flat facility have been identified

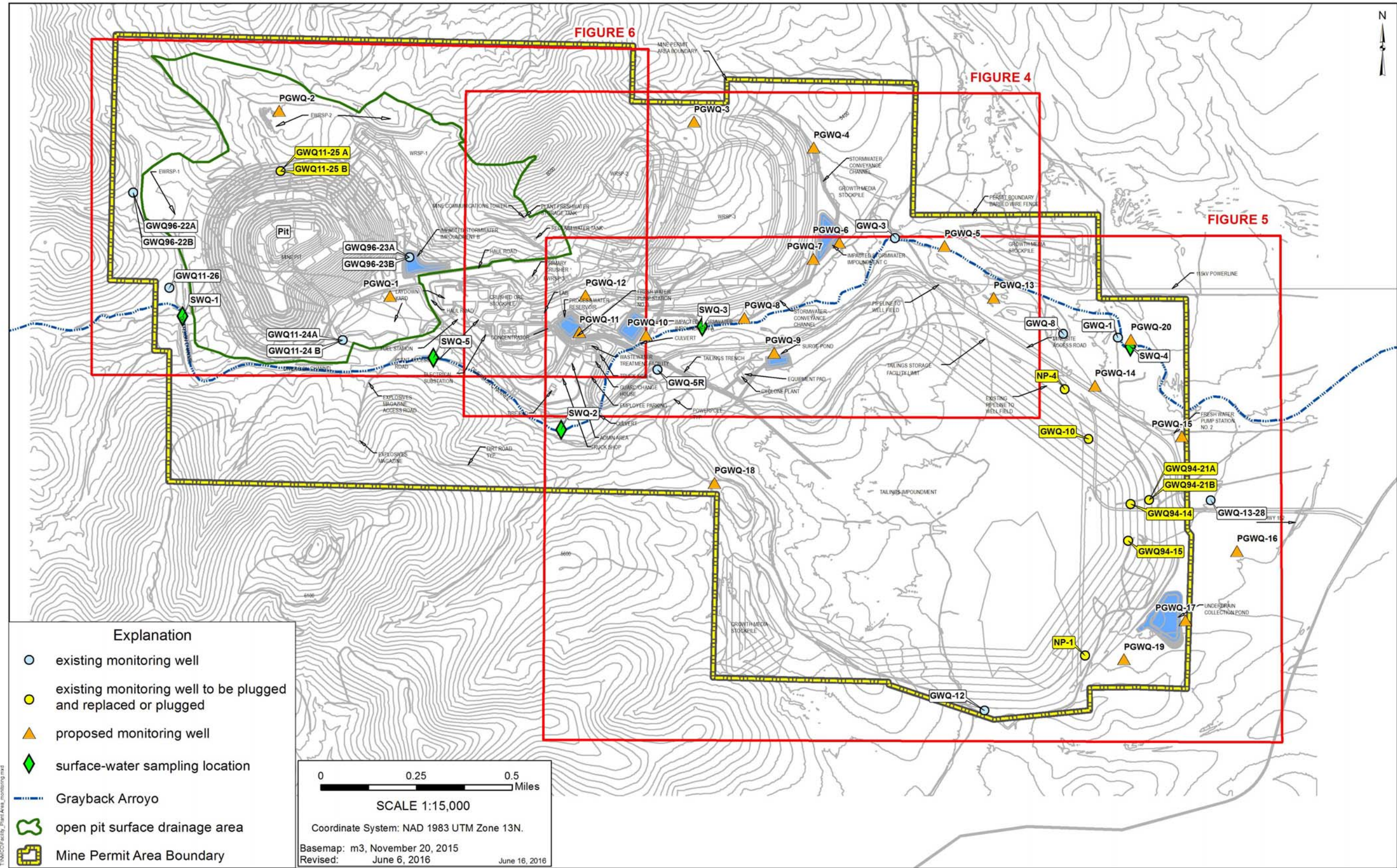


FIGURE 2-27: Proposed surface water quality monitoring locations.



previously in BLM's 1986 White Sands Resource Management Plan (BLM 1986), e.g., grazing, wildlife habitat, recreation and mining.

Land use in the project area will not change from pre-mining approved purposes and the project area will continue to support these approved uses. As described in Appendix E, reclamation and closure of the disturbed area will result in post-mining land uses at Copper Flat that will be sustainable and in keeping with uses currently approved and in use. Mining, grazing, recreation and wildlife habitat are the designations consistent with the surrounding land uses of the Copper Flat site and are appropriate for the site upon reclamation. The Reclamation and Closure Plan is designed to re-establish grazing in the area and allow for long-term use of the reclaimed areas by wildlife known to historically use the area without affecting the potential for other uses such as mining and recreation.

At completion of mining activities, the site will be reclaimed to establish a native plant community similar to the surrounding area. NMCC's reclamation of the site will establish an enhanced native plant community in the area as much of the surrounding area not disturbed by the Copper Flat Project has been significantly disturbed by other historic activities. NMCC's reclamation will result in the development of an early-stage grass/shrub community that will provide a locally-important increase in plant community diversity. Establishment of native vegetation on reclaimed areas at Copper Flat will result in increased erosion protection and direct habitat improvement relative to current conditions.

While the aerial extent of the mine pit will be increased from its current size and, therefore, physically result in permanent loss of some grazing area and wildlife habitat, the vegetation enhancement resulting from reclamation of currently disturbed areas will greatly increase grazing potential and wildlife habitat. As such, there is expected to be a net gain for the land use of the area once reclaimed. In addition, the pit walls created by mining and the pit lake that will form over time upon mine closure will provide enhanced avian wildlife habitat and a water source for transient wildlife. The pit water is known to be devoid of aquatic life and would not be expected to be a future source for aquatic habitat and in any case remain closed to the public as it will remain private land after mine closure.

With respect to the need to obtain approval from the landowner(s) of a post-mining land use, NMCC believes that no such approval is required in this case. The only two landowners of concern at Copper Flat are the federal government and the company. The company is committed to a reclamation and closure plan that re-establishes grazing and wildlife habitat land use of the site at closure. The BLM has approved land uses in its Resource Management Plan for the area. Approval of the Mine Operations Plan required by the BLM for NMCC's mining activities will constitute the approval by the BLM of NMCC's Reclamation and Closure Plan and its goal to return the land to its pre-mining use.



3.0 PROPOSED RECLAMATION PLAN – 19.10.6.602.D.(15)(g)

Appendix E, Mine Reclamation and Closure Plan, Copper Flat Mine, October 2016, prepared by Golder, provides the detailed description of how the disturbed area will be reclaimed to meet the requirements of 69-36-7(H)4 and the performance and reclamation standards and requirements of the Mining Act regulations. This Section of the MORP update is organized to provide the information requested in Sections 19.10.602.D.(15)(h) through(k) of the New Mexico Mining Act regulations and how the Reclamation and Closure Plan will meet the reclamation standards and requirements of 19.10.6.603 NMAC. The Reclamation and Closure Plan and associated design criteria conform to the reclamation requirements described 19.10.6.602.D.(15) NMAC and 19.10.6.603 NMAC, the closure requirements in the Copper Mine Rules (*Subsection A of 20.6.7.18 NMAC, 20.6.7.33 NMAC, 20.6.7.34 NMAC and 20.6.7.35 NMAC*) and applicable mine reclamation regulations set forth by the Bureau of Land Management (BLM) (3809.401(b)(3) and 3809.420(b)(3)).

The objective of the Reclamation and Closure Plan is to reclaim and close the facility in a manner protective of ground water in conformance with the NM Copper Rules, meet the reclamation requirements of the New Mining Act and return the mine area to conditions similar to those present before reestablishment of the mine (BLM DEIS 2015, p. 2-34). Reclamation of the site will re-establish the post-mining land uses consistent with the land uses of the site and the surrounding area, i.e., wildlife habitat, grazing, mining and recreation as identified by the BLM in its approved Land Use Management Plan (BLM 1986).

The Reclamation and Closure Plan has been prepared to address the actions that will be undertaken to reclaim the Copper Flat site at the end of the life of the mine as a result of disturbance created by the previous mine operation conducted by Quintana and those caused by NMCC mining operations. Golder has identified the general setting of the Copper Flat Mine area as they currently exist (see Figure E2 of Appendix E) and the configuration of the site at the end of mine life (see Figure E3 of Appendix E). These figures provide the basis for the reclamation design presented in Appendix E. The reclamation designs are depicted in the drawing set provided in Attachment E1 of Appendix E. This Reclamation and Closure Plan describes contemporaneous reclamation that will be conducted, to the extent practicable, during mine operations, facilities to be reclaimed and closed following cessation of mining operations, and the components of the site that will remain post-closure, following completion of reclamation.

The plans and methods developed and presented in the Reclamation and Closure Plan represent detailed designs for reclamation of the facilities sufficient for agency review and approval. Construction design documents and construction quality assurance/construction quality control (CQA/CQC) plans will be prepared by NMCC for submittal to and approval by the



State of New Mexico within 180 days of submission of a notice of intent to implement the closure plan per the NMED Copper Rules (20.6.7.34.B, NMAC). The CQA/CQC plan will provide a detailed description of the work proposed to be performed to close the site and the final reclamation designs for the facilities to be closed. Post-closure monitoring activities will be conducted in accordance with Section 20.6.7.35 NMAC, and post-closure monitoring and maintenance requirements that may be contained in the Copper Flat Mine Permit.

3.1 Reclamation Schedule & Sequence – 19.10.6.602.D.(15)(h)

Section 4.0 of Appendix E provides the anticipated reclamation schedule and sequence for the Copper Flat Mine. In addition, Table 2-1 of this MORP update includes a summary of the sequence and schedule presented in Appendix E. The schedule is based on consideration of practical phasing of the reclamation projects to account for the anticipated labor, equipment and other resources that would be necessary to complete these projects based on current conditions, sequential closure of facilities in a phased cost efficient manner, and total annual acreages that would be reclaimed over this period. The anticipated durations for reclamation presented include earthwork and reseeding. The reclamation schedule is based on the number of years and months from the time NMCC obtains permit approvals to begin operations. Contemporaneous reclamation of EWRSP-1, EWRSP-2, and portions of EWRSP-4 will begin during the initial mine preproduction period.

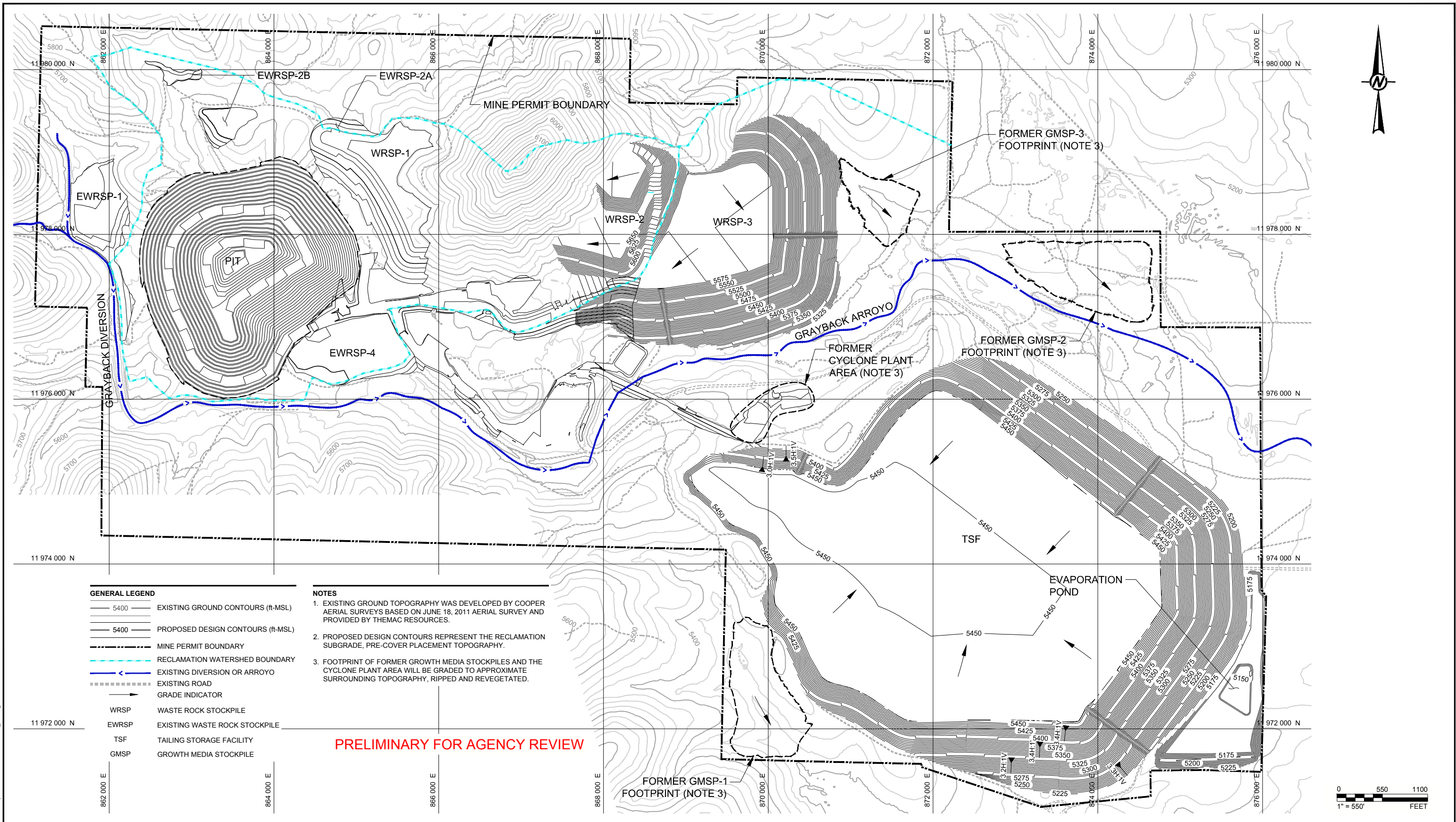
3.2 Reclamation Topographic Map(s) – 19.10.6.602.D.(15)(i)

Figure 3-1 presents the anticipated surface configuration of the permit area upon the completion of reclamation and closure operations. Appendix E contains a drawing package that provides additional detailed depictions of the reclaimed surface various facility units.

3.3 PAG after Reclamation – 19.10.6.602.D.(15)(j)

Generation of acid or other toxic drainage from overburden and waste materials following reclamation that could cause federal or state standards to be exceeded is very unlikely because of the manner in which the overburden and waste will be characterized and disposed of during operations, combined with the reclamation and closure measure that will be implemented as described in more detail in Appendix E. A description of NMCC's waste characterization and handling plan that will be utilized during operations is provided below. These operations practices have been taken into account in the Reclamation and Closure Plan design to ensure that acid generation or other toxic drainage from the site does not occur after reclamation.

NMMC submitted a proposed Mine Plan of Operations (MPO) for the Copper Flat Project in December, 2011 to the Las Cruces, NM office of the Bureau of Land Management (NMCC 2010).



GENERAL LEGEND

	5400	EXISTING GROUND CONTOURS (ft-MSL)
	5400	PROPOSED DESIGN CONTOURS (ft-MSL)
		MINE PERMIT BOUNDARY
		RECLAMATION WATERSHED BOUNDARY
		EXISTING DIVERSION OR ARROYO
		EXISTING ROAD
		GRADE INDICATOR
	WRSP	WASTE ROCK STOCKPILE
	EWRSP	EXISTING WASTE ROCK STOCKPILE
	TSF	TAILING STORAGE FACILITY
	GMSP	GROWTH MEDIA STOCKPILE

- NOTES**
- EXISTING GROUND TOPOGRAPHY WAS DEVELOPED BY COOPER AERIAL SURVEYS BASED ON JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
 - PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.
 - FOOTPRINT OF FORMER GROWTH MEDIA STOCKPILES AND THE CYCLONE PLANT AREA WILL BE GRADED TO APPROXIMATE SURROUNDING TOPOGRAPHY, RIPPED AND REVEGETATED.

PRELIMINARY FOR AGENCY REVIEW

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-09-30	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	TS
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

SEAL	CLIENT	 NEW MEXICO COPPER CORPORATION	PROJECT	COPPER FLAT PROJECT
	CONSULTANT	 TUCSON OFFICE 4730 N. ORACLE ROAD, SUITE 200 TUCSON, ARIZONA UNITED STATES OF AMERICA [+1] (520) 888 8818 www.golder.com	TITLE	GENERAL ARRANGEMENT FINAL RECLAMATION TOPOGRAPHY
			PROJECT NO.	1531453
			CONTROL	0500
			REV.	C
			4 of 25	FIGURE 3-1

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A4S-D



This document, revised in June, 2011, was also provided to the NM MMD and the NMED for review and comment as part of the Environmental Impact Statement (EIS) that is currently being prepared by the BLM for the Copper Flat Project. Appendix C of the MPO contains a Mine Waste Management Plan for the waste rock which includes a plan for waste characterization and handling. MPO Appendix C also contains results of waste characterization work performed by SRK Consulting U.S. Inc. (SRK) in 1997 in support of an EIS that was being prepared at the time for mining activities proposed by Alta Gold Corporation.

In the intervening time since the MPO was first proposed by NMCC in 2010, the waste characterization portion of the plan was implemented. The results of this work have been submitted and discussed with the NM MMD and the NMED and is utilized in the design of the WRSPs described in the MORP update as well as NMCC's DP application.

NMCC will initiate the materials handling plan contained in Appendix C of the MPO, as ultimately approved by the BLM, NMED and other constituent agencies. Implementation of this plan will result in preventing the release of acid generating materials and other toxic drainage that cause federal or state standards to be exceeded following reclamation.

3.3.1 Waste Characterization

SRK performed extensive geochemical characterization studies in support of NMCC's proposed Copper Flat Project (SRK 2013). The resulting report and additional documentation requested by the NMED upon review of SRK's report represent NMCC's material characterization efforts to date and are incorporated into this application by reference.

SRK conducted a mine waste characterization program for the Copper Flat Project. The geochemical testing of mine waste materials provided the characterization required to determine the potential for Acid Rock Drainage and Metal Leaching (ARDML) from mining facilities. This provided the basis for a quantitative risk assessment and evaluation of the options for design, construction, operation, reclamation and closure of the tailings and waste rock disposal facilities.

The Copper Flat mine waste characterization program was designed to investigate the potential for ARDML due to exposure and oxidation of sulfide minerals, such as pyrite, that are unstable under atmospheric conditions. Upon exposure to oxygen and water, sulfide minerals will oxidize, releasing metals, acidity and sulfate. SRK's geochemical characterization investigated the potential for rock that will be exposed in the Copper Flat waste rock disposal facility and pit walls to generate acid and leach when exposed to the atmosphere. The results of the characterization program were used in quantitative numerical predictions to assess the potential future leachate chemistry associated with the mine facilities, specifically the waste rock stockpiles and the TSF.



SRK's investigation concluded that with respect to waste rock, acid generation is not anticipated to occur for most of the un-weathered waste rock materials during operations. SRK concluded that the acid generating potential of the Copper Flat materials, i.e., acid rock drainage (ARD), is largely dependent on the sulfide mineral content and that the sulfide concentrations in the material varied from less than analytical detection limits to a maximum of 2.52 weight percent (wt%) that was highest in the transitional waste material. Transitional material is the oxidized and partially oxidized surface rock material that overlies the fresh rock material below the surface. Where oxidation of this overlying material is complete, waste rock produced from it will be inert with respect to acid generation. Partially oxidized material occurs in a transition zone beneath the oxidized cover and the underlying un-oxidized material. This transitional material has been exposed to oxidizing conditions over geologic time. Such material will typically exhibit a low paste pH and high paste conductivity, and will be generally acid generating. Examples of this condition can be found in the exposed pit walls and where transitional waste material was deposited on the existing waste rock stockpiles.

SRK determined that 96% of the waste rock that will be produced at Copper Flat will consist of sulfide, non-oxidized Quartz Monzonite/Breccia waste, which typically exhibits either non-acid forming characteristics or a low potential for acid generation. Samples collected by SRK in their investigation from the surface of the existing waste rock stockpiles and pit walls indicated that there is some potential for acid generation from material mined by previous mining operations and exposed to natural weathering conditions. However, as indicated previously, most of the existing waste rock stockpiles will be reclaimed and the existing pit walls will be mined by NMCC's proposed operation.

3.3.2 Material Handling Plan

NMCC's proposed material handling plan is based, in part, on SRK's recommendations to minimize the potential for acid leaching from the waste rock stockpiles and tailings. Based on SRK's findings the materials that will be generated will have only a low potential for acid leaching, NMCC anticipates that it will, generally, not be necessary segregate waste rock. As described in Appendix C of the MPO, most of the waste rock produced by the operation will be low and high sulfide material with only small amounts of oxide and transitional material produced near the surface of the proposed pit expansion. Additionally, it should be noted that the terms "high sulfide" and "low sulfide" are terms relative to specific conditions at Copper Flat and, therefore, to each other. That is, at Copper Flat low sulfide material is defined as material having less than 0.5% sulfide and high sulfide material will have 0.5% sulfide or more. It is further noted that the Copper Flat ore body is a very low sulfide ore body in relation to other ore bodies in the region. The waste produced at Copper Flat will be primarily quartz monzonite and andesite and the ore will be primarily breccia. NMCC will implement a waste material classification program as described in Section 2.5 of the MPO. It is anticipated that the waste rock generated will oxidize very slowly and may only, potentially, produce acid over some



period of time. The acid rock ARD potential associated with un-oxidized waste rock is relative in the long-term. SRK determined that the waste rock can be considered as being inert with respect to ARD for a timeframe in the order approximately 20 years. Therefore, the vast majority of the waste material, i.e., about 96%, that will be generated at Copper Flat will generally have a low ARD potential. As indicated above, the transitional materials that have been exposed to oxidizing conditions over time have the most potential for acid generation.

Waste Material Classification

Section 2.6.2 of the MPO generally describes the general material handling approach the will be implemented at Copper Flat. The subsequent work conducted by SRK and presented in its May, 2013 Geochemical Characterization Report confirms the approach to be utilized. However, while the approach is generally the same as described in Appendix C of the MPO, there will be some aspects of material handling that may differ from the information provided therein as supported by the later findings of SRK's material characterization studies.

The overarching approach to waste rock material handling to control ARD will be to control the movement of water through the waste rock stockpiles, in combination with continual diligent monitoring and characterization of the waste materials produced to confirm SRK's conclusion that the majority of the material has a low ARD potential. As discussed in more detail below, NMCC believes that depositing the small amounts of material with high ARD potential; i.e., the transitional waste, when encountered, along with the large amounts of non-ARD materials will further reduce the potential for the ARD materials to create acid. The buffering capacity of the large volume non-transitional waste will neutralize the small volume transitional waste. Should field characterization reveal that more ARD materials than anticipated are being generated the materials handling plan will be adjusted to consider isolation, encapsulation and other means of treatment to mitigate the potential for acid generation. The non-transitional waste will be used as base material in any areas where it has been determined in the field that transitional material should be segregated. However, as a practical matter, under normal circumstances large volumes of non-transitional waste will typically be placed below, above, and all around the transitional waste produced.

SRK recommended in its May 2013 report that, during proposed operations, specific controls would be needed to collect storm water runoff from the waste rock stockpiles and that storm-water diversions would be required to prevent run-on. SRK also recommended that covering the waste rock stockpiles with a re-vegetated 36-inch cover at the end of mine life would reduce infiltration of water and flux of oxygen into the facility, and thus, limit oxidation of sulfide minerals. SRK also noted that migration of seepage from the waste rock stockpiles into the underlying bedrock would be anticipated to be very small, or nil, because of the low permeability of the andesite underlying the area. These recommendations have been incorporated into NMCC's design of its waste rock stockpiles.



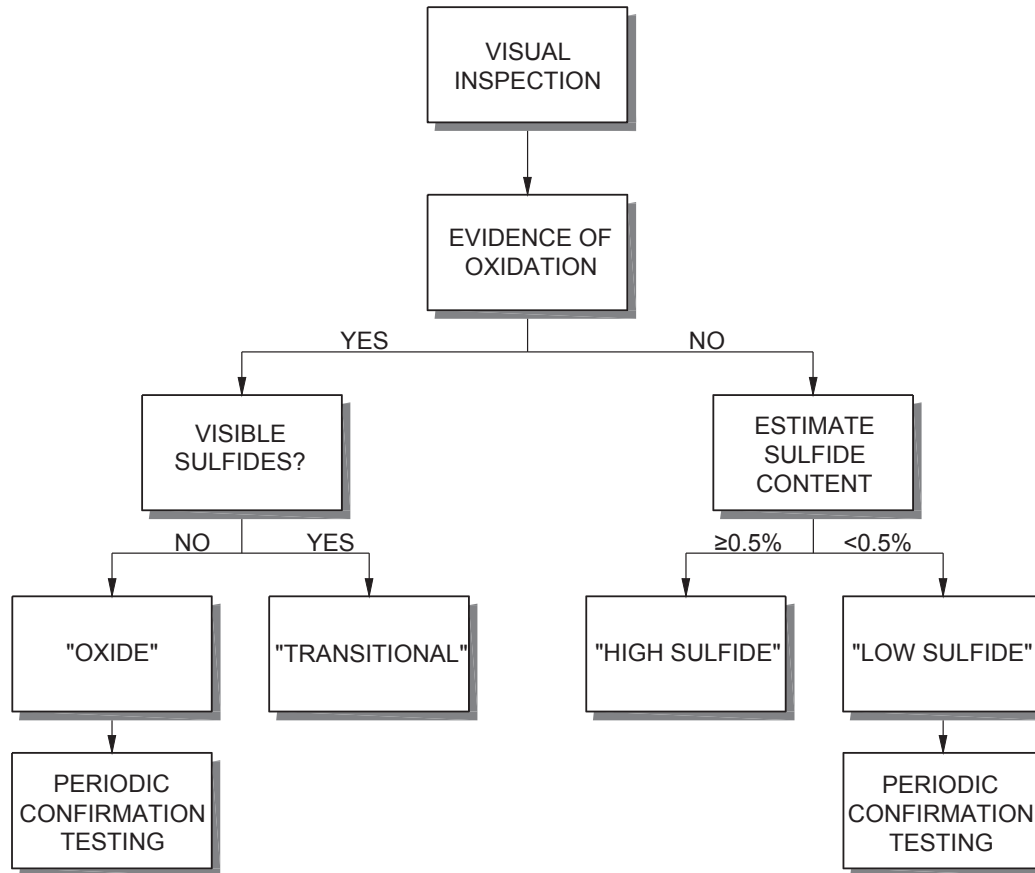
Figure 3-2 describes process that will be utilized in the NMCC's proposed waste classification program during operations. Following the evaluation path shown in Figure 3-2, a determination will be made as to classification, and periodic confirmation testing will be conducted. Confirmation testing will include a field testing program for representative samples of the cuttings, as determined by the qualified geologist or technician. The tests conducted will include paste pH, saturated paste conductivity, and acid base accounting testing (total sulfur, sulfate sulfur, and NP testing).

As described in more detail in Section 2.6.2.5 of Appendix C of the MPO, prior to blasting active benches in the open pit, the drill cuttings from each drill blasthole will be inspected. Blasthole drill cuttings will be visually inspected by a qualified geologist or trained technician prior to blasting and removal of the material from the pit. The rock type, color, degree of oxidation, sulfide content and other pertinent features will be noted and transferred to the bench plan maps. All material characterized as oxide or low sulfide waste will be sampled for confirmation testing. Material classified as low sulfide rock will be subject to periodic confirmation testing at a frequency initially on one confirmation test for each five blastholes designated as oxide waste rock. This frequency will be adjusted as ongoing testing and field observation continues to demonstrate consistent reproducible results supporting visual waste classification. NMCC anticipates a frequency of confirmation testing in the longer term to be one test for every 20 holes. Confirmation testing will be performed on-site using a Leco Furnace to evaluate the classification determinations made.

Waste Rock Flagging and Routing

Waste rock from the open pit will be examined as benches are mined to identify sulfide bearing transition material that may represent ARD potential. Specific procedures will be established by the mine operations team when preparing for startup. However it is anticipated that the procedures employed by the operation will be similar to the following description, which follows standard mine geology practices and methods. The operations team will develop full details after the team is assembled to begin startup of the mine. As requested by the state regulatory agencies, NMCC will meet with them as plans develop to discuss the plan and receive input prior to implementation.

The mine waste rock identification, flagging, and routing process will be similar to mine ore control procedures and the two processes will be completed simultaneously. Identification, digging plan design, field identification for operations, determination of destination and placement, and routing of ore and waste materials will be the responsibility of the mine technical services team, which typically includes geology engineering, and surveying disciplines.



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PROJECT
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 COPPER FLAT PROJECT
 SIERRA COUNTY, NEW MEXICO

TITLE
WASTE CLASSIFICATION FLOWCHART

PROJECT No. 153-1453	PHASE 0006	Rev. 1	FIGURE 3-2
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The mining and ore/waste classification cycle begins with blasthole drilling. At Copper Flat, all benches mined will require blasting; therefore all areas mined will be drilled on a regular grid pattern across the full width and length of each bench. Each blasthole will be assigned a unique identification number (ID) for data tracking, and each will be surveyed and plotted onto a blasthole location map that is spatially tied to a three dimensional (3D) model of the mine geology. As holes are drilled and surveyed, the ID and survey coordinates of each blasthole will be logged into a blasthole data base. After drilling, and before blasting, the geology of the bench surfaces and exposed mine faces will be examined for key geologic parameters and the geology mapped to the same scale as the blasthole maps. The blasthole cuttings will be visually examined to determine rock type, sulfide content, and oxidation level and samples taken for laboratory analysis. Data from field examinations will be logged into the blasthole data base and plotted onto the blasthole map, with the analytical results added to the maps and the database upon receipt from the laboratory. When the geologic and analytical data for a specific area is complete, the technical services team will develop ore and waste zone boundaries and identify material types that are subject to a specific routing plan, such as for ore transport or transport of potential ARD material to the WRSP. The boundaries and material designations will be transferred back to the blasthole maps and survey coordinates produced for identifying material boundary lines in the field.

After blasting, and before excavation begins, the material boundary lines will be established on the top of the broken rock by survey, and the broken rock visually examined again to determine if any field adjustment in classification is needed. Field adjustments will be transferred to the blasthole maps for record keeping. Even after having been blasted, the area will be closed to excavation and material removal until all data is received and the “dig plan” is finalized by the technical services team. When an area is opened for excavation of material, the specific ore and waste boundary lines will be visually identified on the top of the broken rock and specific material types designated with color coded flagging. Copies of the blasthole maps showing the corresponding material types and boundaries will be provided to the mine equipment operators for reference during material removal.

As excavation proceeds, the loader operators will selectively excavate specific ore or waste material types following the dig plan established by the technical services team. Each haul truck will be loaded with only the one type of material designated. After loading, the loader operator will communicate the material type to the truck operator through an established signal system. The material type loaded into the truck will designate a pre-determined destination for the load. The truck operator will track loads by material type and destination and the load information will be compiled and maintained by mine staff for reporting.



Transitional Waste Material Disposal

Section 2.6.2.2 of Appendix C of the MPO provides that material classified as “transition waste” will be isolated in a waste rock stockpile area and covered with a minimum of six feet of “non-transition” material. SRK’s investigation determined that the transitional material to be excavated will be less than 4% of the total volume of waste produced from the operation. SRK’s analysis was performed prior to development of the mine plan proposed in this MORP update for Copper Flat, NMCC’s DP application and Alternative 2 of the DEIS. Review of the most recent geologic model and the mine plan indicates that transitional material will still be produced at the same ratio relative to non-transition waste as determined during the SRK investigation.

NMCC anticipates that the transitional waste will be produced in the first 8 years of operation (approximately 5.4 million tons), with about half of it produced in the first 2 years. Some of this material will be disposed of in WRSP-1, which located in the OSPDA. The remainder will be disposed of in WRSP-2 and 3. During the same two years as much as 5.2 million tons of non-transitional acid neutralizing waste material, will also be produced. Some of this acid neutralizing material will be used as neutralizing material for those areas where transitional material may be deposited. NMCC will lay a minimum 10 ft. of base of non-transitional waste underlying the area where transitional material will be deposited in the WRSPs and ensure that at least 10 feet of non-transitional waste surrounds the transitional waste in such a manner that the transitional waste is not exposed to oxidation.

The remaining approximate 2.6 million tons of transitional material will be produced over years 3 through 8 at an average rate of approximately 433 thousand tons per year while at the same time about 27.6 million tons, an average of 4.6 million tons of acid neutralizing non-transitional waste will be produced. As such, the greatest volume of waste material generated, by far, will be classified as un-oxidized high sulfide and/or un-oxidized low sulfide waste. As confirmed by SRK’s waste rock characterization investigations, this material poses a low level, short-term risk for ARD. While NMCC considers it unnecessary, and perhaps to some extent, contrary to the desire to minimize potential acid generation, to isolate and concentrate that material in one area, NMCC will continue to identify potential ARD generating waste during operations and take steps to establish disposal areas within the WRSPs for this material ensuring that a minimum of 10 ft. of non-transitional acid-neutralizing waste surrounds the transitional waste within the WRSPs where the potential ARD generating material will be deposited. As a practical matter, NMCC will ensure that non-transitional material is placed below, above and all around the transitional material wherever possible, providing a thick neutralizing “blanket” around the transitional material.

Quality assurance testing will also be performed in addition to the daily field sampling. Up to 10 archived blasthole samples will be randomly selected and subjected to paste pH, saturated



paste conductivity, and acid base accounting testing (total sulfur, sulfate sulfur, and NP testing). The testing will be performed by a third-party independent state approved laboratory. The samples will be classified with respect to ARD potential on the basis of NP/AP ratio. The samples will be located on the appropriate bench plan maps and the quality assurance test classifications will be compared to the operational waste classification designations.

SRK has performed significant kinetic testing of waste materials since the time of submittal of the MPO. Humidity cell testing results were first reported to NMED in SRK's May, 2013 report. Continued and more extensive humidity cell results were reported to NMED in February, 2014 in an SRK report titled "Humidity Cell Termination Report for the Copper Flat Project, New Mexico." These documents are included in this MORP update by reference. As indicated earlier in this document, these studies provide the basis for NMCC's mine waste characterization and handling plans.

At the end of the mine life the waste rock stockpiles will be reclaimed in accordance with the approved Reclamation and Closure Plan. As determined by SRK the majority of the waste rock materials can be categorized as being inert from the standpoint of acid generation for about twenty years before beginning to develop ARD potential if left uncovered. The anticipated life of the mine is 11 years. The waste rock stockpiles will be covered with a minimum of 3 ft. of soil. Therefore, covering the waste rock stockpiles with a minimum of 3 ft. of soil will mitigate the potential for acid generation and impacts to groundwater.

3.4 Contemporaneous Reclamation – 19.10.6.602.D.(15)(k)

The concept of operating for closure and contemporaneous reclamation are means by which NMCC will strive to reduce erosion, provide early impact mitigation, limit costs and reduce final reclamation work. NMCC is committed to maximizing these concepts at the Copper Flat Project where feasible. It has designed mine facilities to employ contemporaneous reclamation to the extent appropriate and practicable. Re-contouring, placement of cover materials and revegetation will be implemented, where and when operational conditions permit in areas where mine operation activities are discontinued.

Contemporaneous reclamation is integrated into the design and construction of the Copper Flat facilities, in particular, the WRSPs. The WRSPs will be constructed in their final configurations with the first lift built to occupy the projected footprint of each stockpile. All sequential lifts will be set back to facilitate final out-slope grading and accommodate inter-bench slopes and cross bench drainages as discussed in more detail in the Reclamation and Closure Plan. As each lift is completed, any portion not needed for access to other lifts will be regraded, covered and revegetated as soon as practicable. The top surface of each lift of the WRSPs will also be



constructed to a minimum final grade of 1 percent to minimize the final grading operations and achieve positive drainage.

As discussed earlier, at Copper Flat there are several existing waste rock stockpiles (EWRSP-1, 2A and 2B, 3 and 4, see Figure 2-14, that were generated by Quintana during previous operations. NMCC has incorporated these EWRSPs into its contemporaneous reclamation efforts. EWRSP-1 and 2B will be reclaimed during operations as discussed previously and described in the Reclamation and Closure Plan.

EWRSP-2A will be incorporated into new WRSP-1 during operations as it is constructed. WRSP-1 will be reclaimed per the Reclamation and Closure Plan. EWRSP-3 will be incorporated into the process plant facility during operations. As such, it will be reclaimed along with WRSP-1 at the end-of-life of the project. EWRSP-4 will be re-contoured and utilized as a laydown area during operations and will be reclaimed at the end of mining. However, the side-slopes of EWRSP-4 will be reclaimed during operations as described in the Reclamation and Closure Plan to mitigate against potential surface water impacts to Grayback Arroyo.

Opportunities for contemporaneous reclamation of the TSF are limited as they could interfere with operations and could jeopardize dam safety. However, during operation of the TSF, management of tails deposition while constructing the embankment will assist in achieving the desired outslope grade to accommodate final reclamation grading. Also, in the later operational period of tailings deposition, discharge of tailings from selected locations will be used to relocate the supernatant pool to a location adjacent to the location of what will be the post-closure spillway. This will reduce grading requirements and limit earthmoving operations in areas where working conditions can be more challenging due to the presence of soft and saturated tailings. Tailings discharge may also be used to create nominal surface topography on the final top surface that will assist with developing a final drainage pattern.

NMCC may also decommission some access roads and other ancillary facilities prior to final mine closure when determined to be no longer needed for mine operations.



4.0 PERFORMANCE & RECLAMATION STANDARDS & REQUIREMENTS – 19.10.6.603

The Reclamation and Closure Plan described above and in Appendix E has been developed to meet the site-specific characteristics of the mining operation and the site. As indicated herein previously, the current land uses in the area of the Copper Flat facility have been identified by the BLM to include activities such as grazing, wildlife habitat and mining. Reclamation of the disturbed area will result in post-mining land uses at Copper Flat that will be sustainable and in keeping with previous historic uses. Aside from mining, grazing, wildlife habitat, and recreation are the designations consistent with the surrounding land uses of the Copper Flat site and are appropriate for the site upon reclamation. The Reclamation and Closure Plan is designed to re-establish grazing in the area and allow for long-term use of the reclaimed areas by wildlife known to historically use the area without affecting the potential for other uses such as mining and recreation. This section describes how the Copper Flat operation will meet the performance and reclamation standards and requirements of the Mining Act rules and the NMED Copper Rules closure requirements.

4.1 Most Appropriate Technology and Best Management Practices-19.10.603.A

NMCC has designed its operations and reclamation plans to protect human health and safety, the environment, wildlife and domestic animals using Most Appropriate Technology (MAT) and Best Management Practices (BMPs). MAT in mine operations is understood as the selection and application of the most suitable mining technology to achieve the intended purpose while reducing impacts to the environment. The selection of a MAT is typically accomplished in mine feasibility studies that evaluate mining technologies, processes and operating methods. The Copper Flat Project has been designed and will be operated using both MAT and BMPs based on site-specific technical and economic feasibility. Mining technologies, processes and operating methods proposed by NMCC are provided in Section 2.0.

BMPs are defined as any program, technology, process, siting criteria, operating method, measure or device, which controls, prevents, removes or reduces impacts to the environment. BMPs are accepted, effective and practical methods including structural or engineered control devices, systems and materials as well as operational or procedural practices used to prevent or reduce environmental impacts of ground disturbing activities. NMCC will meet or exceed applicable state and federal reclamation requirements through application of MAT and BMPs. NMCC has designed its operations and reclamation plans to use the most appropriate technology for an open pit mine operation. Structural BMPs will be used to limit erosion and



reduce sediment in precipitation runoff from proposed Project facilities and disturbed areas during construction, operations and reclamation. These structural BMPs will include:

- Surface stabilization measures such as dust control, regrading, mulching, riprap, temporary and permanent revegetation/reclamation and placing growth media;
- Run-on and runoff control and conveyance measures such as hardened channels, runoff diversions; and
- Sediment traps and barriers such as check dams, grade stabilization structures, sediment detention, sediment/silt fence and straw bale barriers and sediment traps.
- Apply water to control dust on haul roads and other disturbance areas;
- Interim seeding of stockpiles and surface disturbance areas;
- Use of certified weed-free seed and mulch;
- Cleaning heavy equipment before entering the mine area; and
- Noxious weed monitoring and treatment.

BMPs will be employed at appropriate locations during mine construction, operation and reclamation phases of the project and structures will be inspected periodically, with repairs performed as needed. NMCC will limit disturbance and preserve existing vegetation to the maximum extent possible. Additional details regarding structural and operational BMPs will be included in the SPCC plan and the SWPPP permit required for mine operation.

4.1.1 Hydrologic Investigations at Copper Flat

Utilization of MAT and BMP to protect ground water and surface water is the dominant theme in design, operation and reclamation at Copper Flat as it is the medium of greatest concern with respect to potential for environmental impact. As such, all facets of the operation have been designed to ensure water resource protection during operations and following reclamation.

NMCC and its water resource consultant, John Shoemaker & Associates (JSAI), have conducted extensive hydrologic investigation of the Copper Flat site and the surrounding area in support of NMCC's permitting activities for the project. These activities include supplementing the NMCC Abatement Plan previously submitted to NMED and the BDR submitted to NMED and NM MMD, providing detailed analysis for the BLM EIS and supporting various requests for information by the NM OSE.

NMCC has previously submitted to the various agencies, including NM MMD and NMED, JSAI's document titled "Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico, August 2014" (JSAI 2014c). This document, the subsequent review documentation provided by NMCC and JSAI are, therefore, incorporated into this MORP update by reference. The following information is a synopsis of the volumes of information provided in the groundwater model documentation in an effort to provide specifics



as required by the NMED Copper Rules and NMCC's Discharge Plan application and has been utilized in determining MAT for the site.

General Hydrogeologic Setting

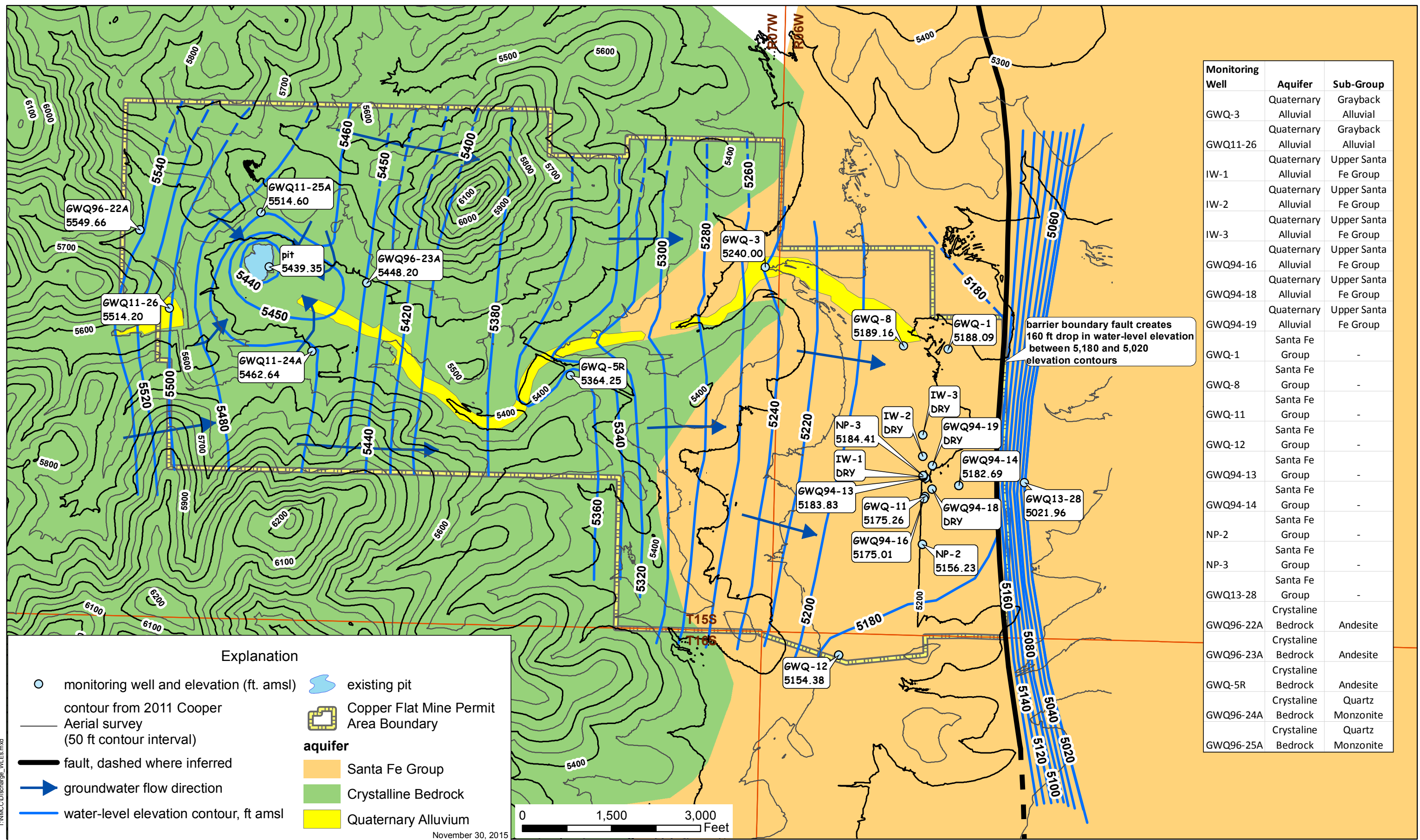
The Copper Flat facility will be located in area that includes the following water-bearing formations within the Animas Uplift: crystalline bedrock (andesite and quartz monzonite) in the western part of the mine permit area, Santa Fe Group sedimentary deposits in the eastern part of the mine permit area, and alluvium of upper Grayback Arroyo overlying the crystalline bedrock and Santa Fe Group in the mine permit area. Figure 4-1 presents the water-bearing formations at the facility, potentiometric surface contours and direction of groundwater flow, selected monitoring wells, and topography. Figure 4-2 presents a geologic map of the facility and surrounding area, and transects of hydrogeologic cross-sections.

Groundwater flow is mainly from west to east in the mine permit area, with groundwater discharging from the crystalline bedrock as subsurface flow across the contact with the Santa Fe Group, and as evaporation from the open pit. Monitoring wells designated as "dry" in Figure 4-1 are shallow wells installed to depths of less than 60 ft. in the Upper Santa Fe Group.

The potentiometric surface contours in the vicinity of the existing open pit shown in Figure 4-1 demonstrate the open pit to be hydraulic sink. Cross-sections PA-PA' and PX-PX' presented in Figures 4-2, 4-3 and 4-4 show groundwater flow in the crystalline bedrock in the vicinity of the open pit. Hydrogeologic cross-sections presented in Figure 4-2 (i.e., TA-TA' and TB-TB') and Figures 4-5 and 4-6 depict the Upper Santa Fe Group in the mine permit area. The crystalline bedrock at the facility is relatively impermeable and groundwater recharge from local precipitation to the crystalline bedrock is limited by low hydraulic conductivity. The groundwater system at the facility conducts little water, and the eastern mine permit boundary generally coincides with the East Animas Fault, shown in Figures 4-1 and 4-2, which acts as a barrier to groundwater flow.

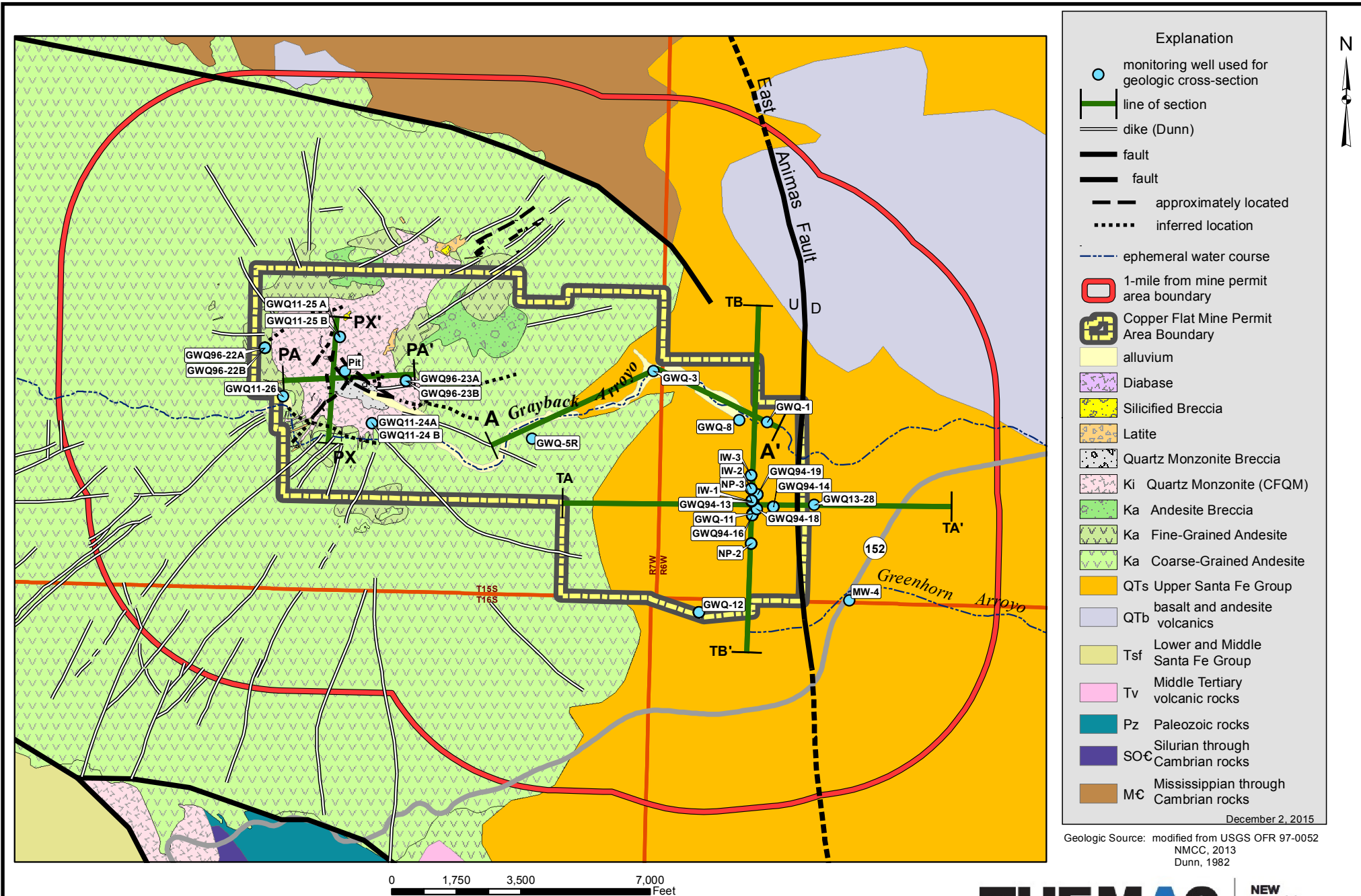
A portion of the original Grayback Arroyo watershed within the mine permit area now drains to and includes the open pit. Grayback Arroyo is an ephemeral drainage in the mine permit area. However, groundwater levels are close to the surface, and there can be base flow discharge to Grayback Arroyo following wet periods. The cross-section A-A', presented in Figures 4-2 and 4-7, show the alluvium of upper Grayback Arroyo overlying the crystalline bedrock and Santa Fe Group in the mine permit area.

The Copper Flat porphyry copper-molybdenum deposit is hosted by the Cretaceous quartz monzonite in the western part of the mine permit area (Figure 4-2). Faults to the north and south of the mine permit boundary juxtapose the andesite with older, Paleozoic sedimentary rocks. The eastern mine permit boundary generally coincides with the East Animas Fault, which



Monitoring Well	Aquifer	Sub-Group
GWQ-3	Quaternary Alluvial	Grayback Alluvial
GWQ11-26	Quaternary Alluvial	Grayback Alluvial
IW-1	Quaternary Alluvial	Upper Santa Fe Group
IW-2	Quaternary Alluvial	Upper Santa Fe Group
IW-3	Quaternary Alluvial	Upper Santa Fe Group
GWQ94-16	Quaternary Alluvial	Upper Santa Fe Group
GWQ94-18	Quaternary Alluvial	Upper Santa Fe Group
GWQ94-19	Quaternary Alluvial	Upper Santa Fe Group
GWQ-1	Santa Fe Group	-
GWQ-8	Santa Fe Group	-
GWQ-11	Santa Fe Group	-
GWQ-12	Santa Fe Group	-
GWQ94-13	Santa Fe Group	-
GWQ94-14	Santa Fe Group	-
GWQ94-14	Santa Fe Group	-
GWQ94-13	Santa Fe Group	-
GWQ94-14	Santa Fe Group	-
NP-2	Santa Fe Group	-
NP-3	Santa Fe Group	-
GWQ13-28	Santa Fe Group	-
GWQ96-22A	Crystalline Bedrock	Andesite
GWQ96-23A	Crystalline Bedrock	Andesite
GWQ-5R	Crystalline Bedrock	Andesite
GWQ96-24A	Crystalline Bedrock	Quartz Monzonite
GWQ96-25A	Crystalline Bedrock	Quartz Monzonite

FIGURE 4-1: Water bearing formations, Copper Flat Mine, Sierra County New Mexico.



Explanation

- monitoring well used for geologic cross-section
- line of section
- dike (Dunn)
- fault
- fault
- approximately located
- inferred location
- - - - - ephemeral water course
- 1-mile from mine permit area boundary
- Copper Flat Mine Permit Area Boundary
- alluvium
- Diabase
- Silicified Breccia
- Latite
- Quartz Monzonite Breccia
- Ki Quartz Monzonite (CFQM)
- Ka Andesite Breccia
- Ka Fine-Grained Andesite
- Ka Coarse-Grained Andesite
- QTs Upper Santa Fe Group
- basalt and andesite volcanics
- Tsf Lower and Middle Santa Fe Group
- Tv Middle Tertiary volcanic rocks
- Pz Paleozoic rocks
- SOc Silurian through Cambrian rocks
- Mc Mississippian through Cambrian rocks

December 2, 2015

Geologic Source: modified from USGS OFR 97-0052
NMCC, 2013
Dunn, 1982

FIGURE 4-2: Mine site geologic map, Sierra County, New Mexico.

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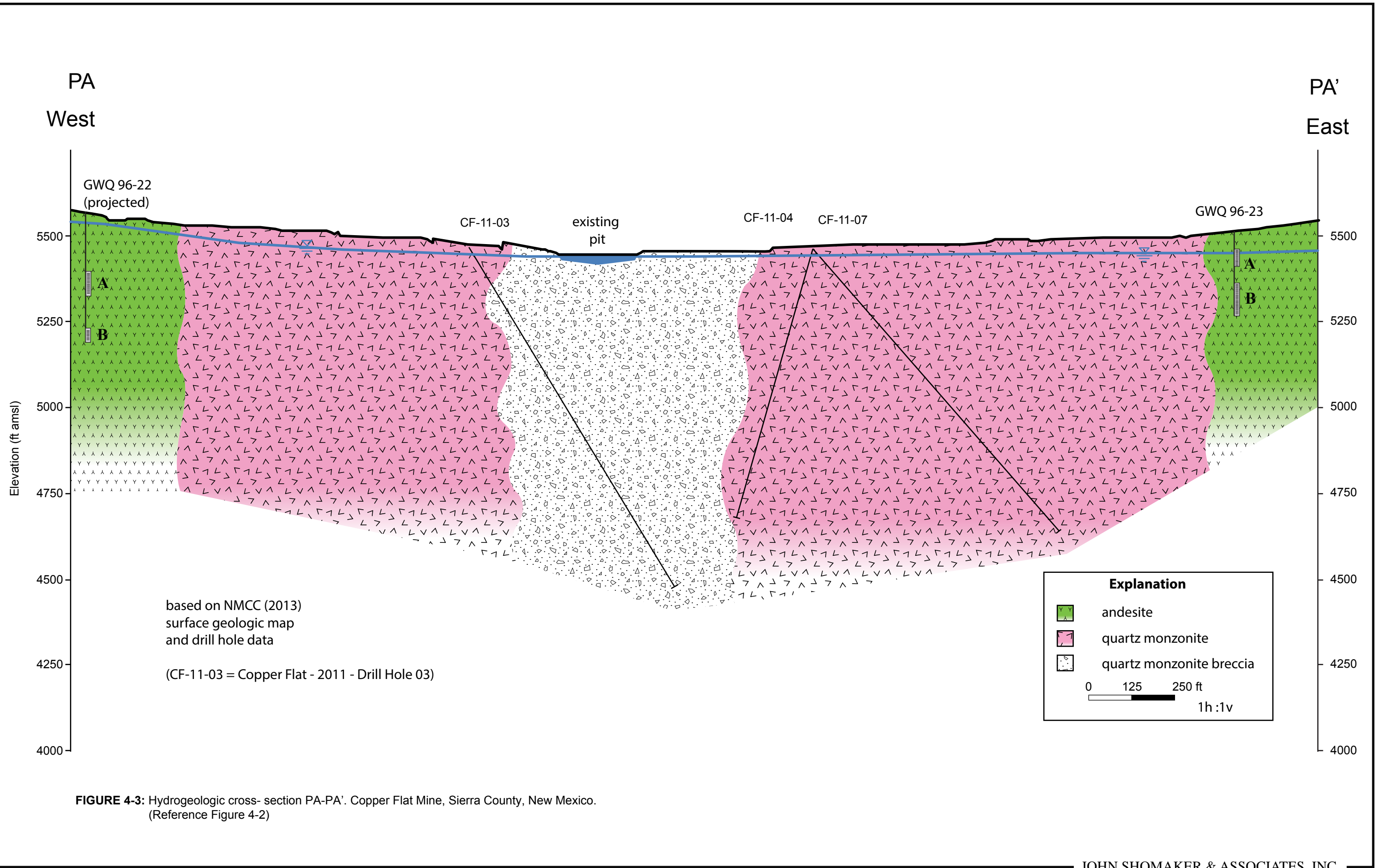


FIGURE 4-3: Hydrogeologic cross- section PA-PA'. Copper Flat Mine, Sierra County, New Mexico.
(Reference Figure 4-2)

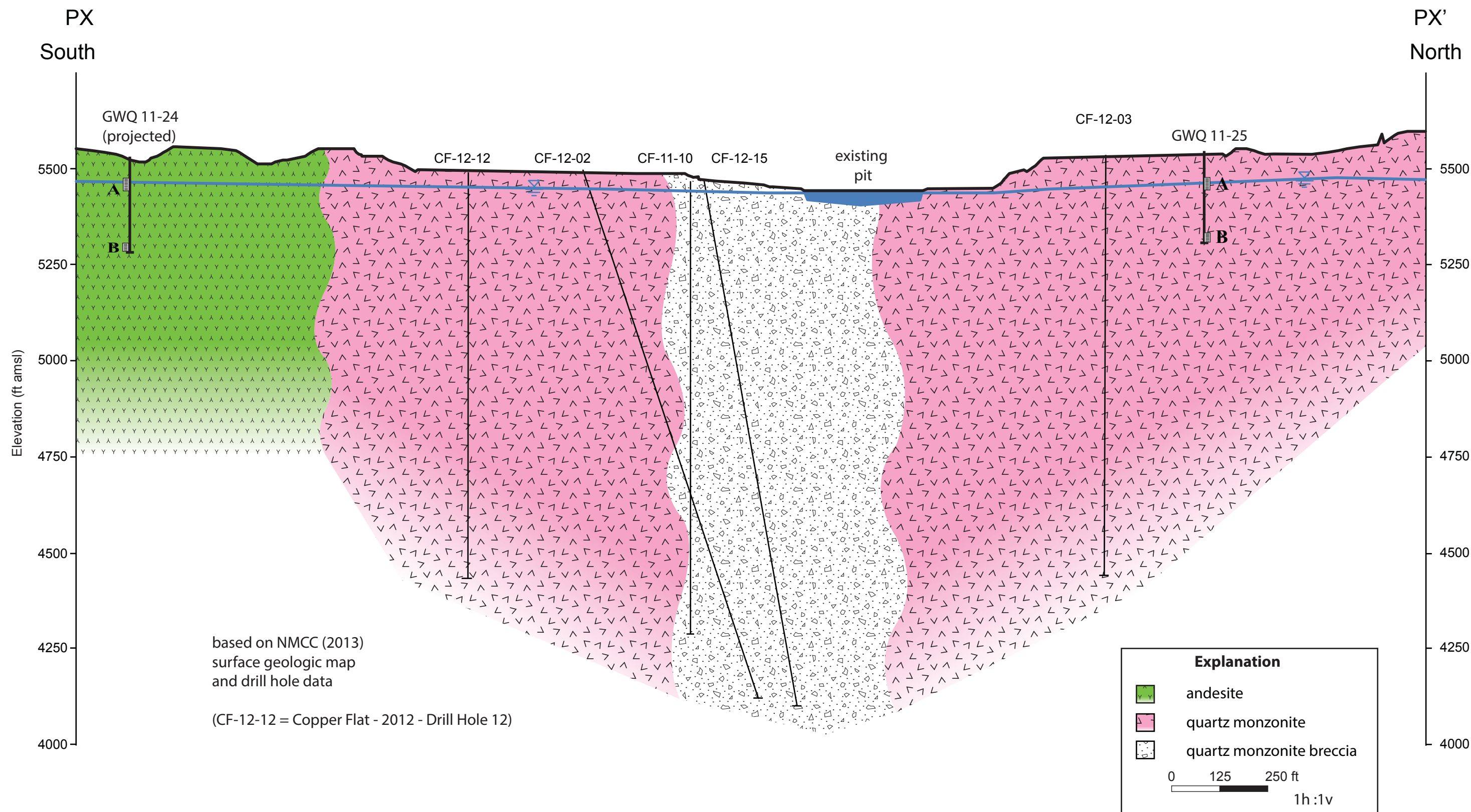


FIGURE 4-4: Hydrogeologic cross-section PX-PX'. Copper Flat Mine, Sierra County, New Mexico.
(Reference Figure 4-2)

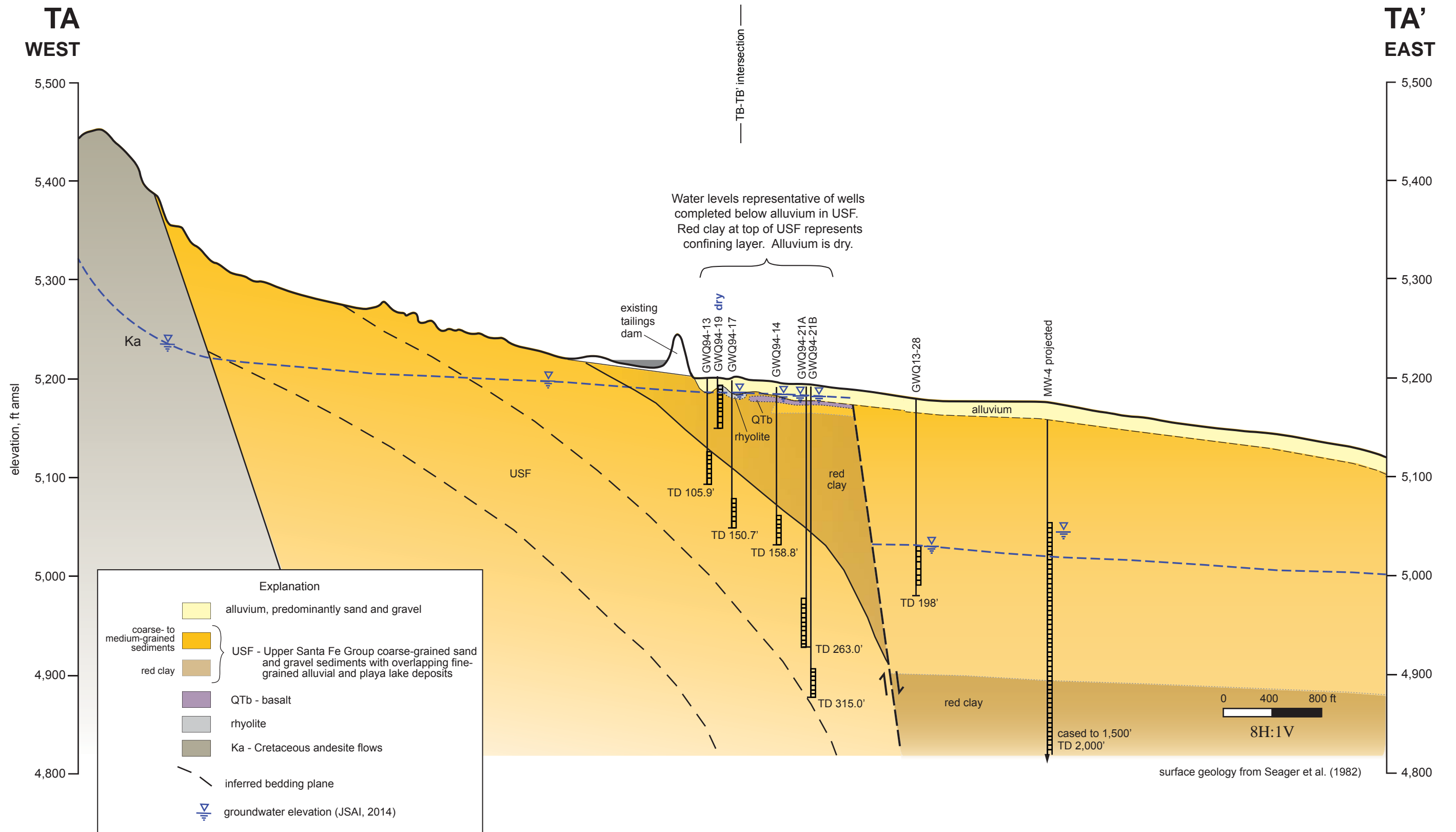


FIGURE 4-5: West to east geologic cross-section TA-TA' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico. (Reference Figure 4-2)

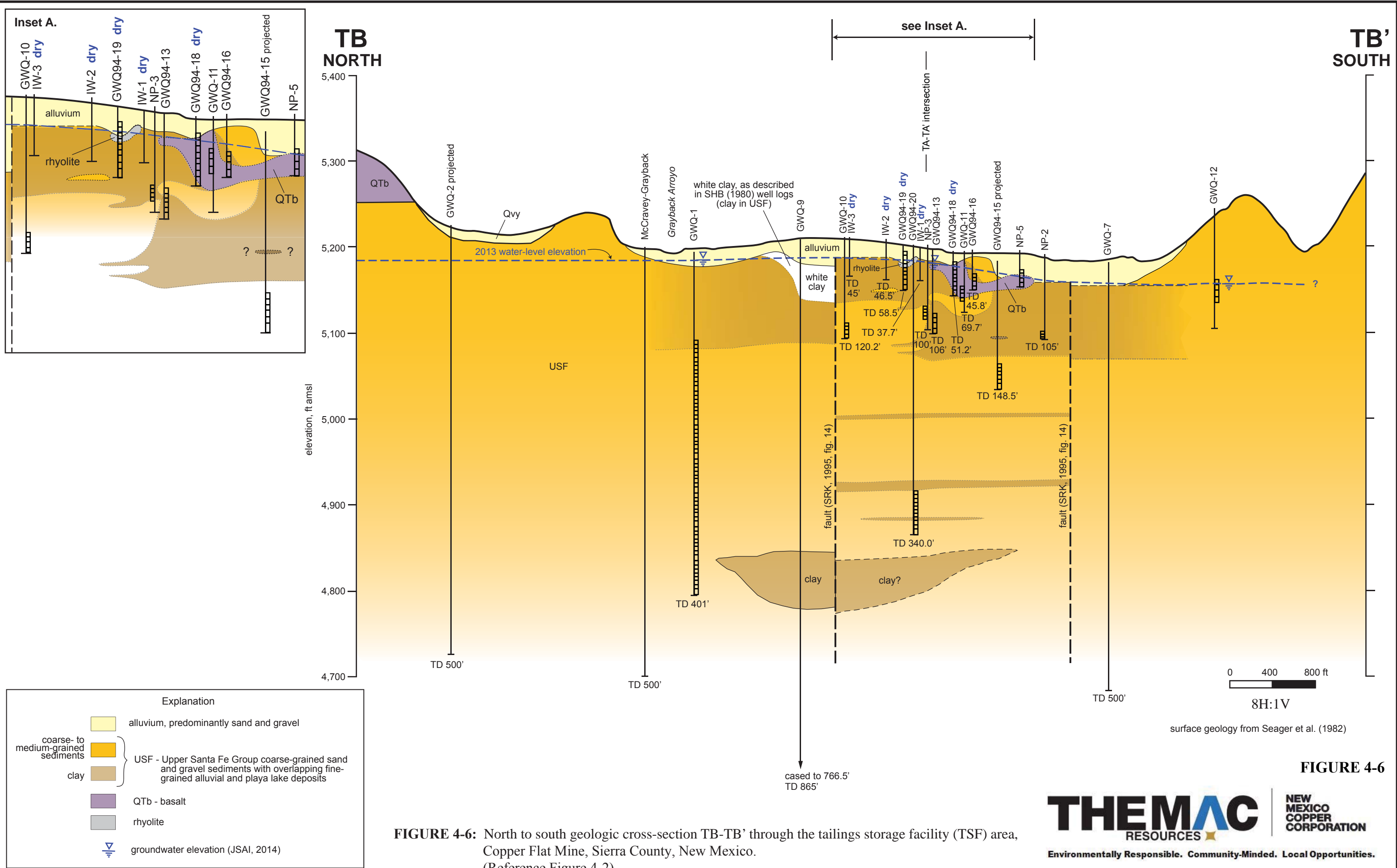


FIGURE 4-6: North to south geologic cross-section TB-TB' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico. (Reference Figure 4-2)

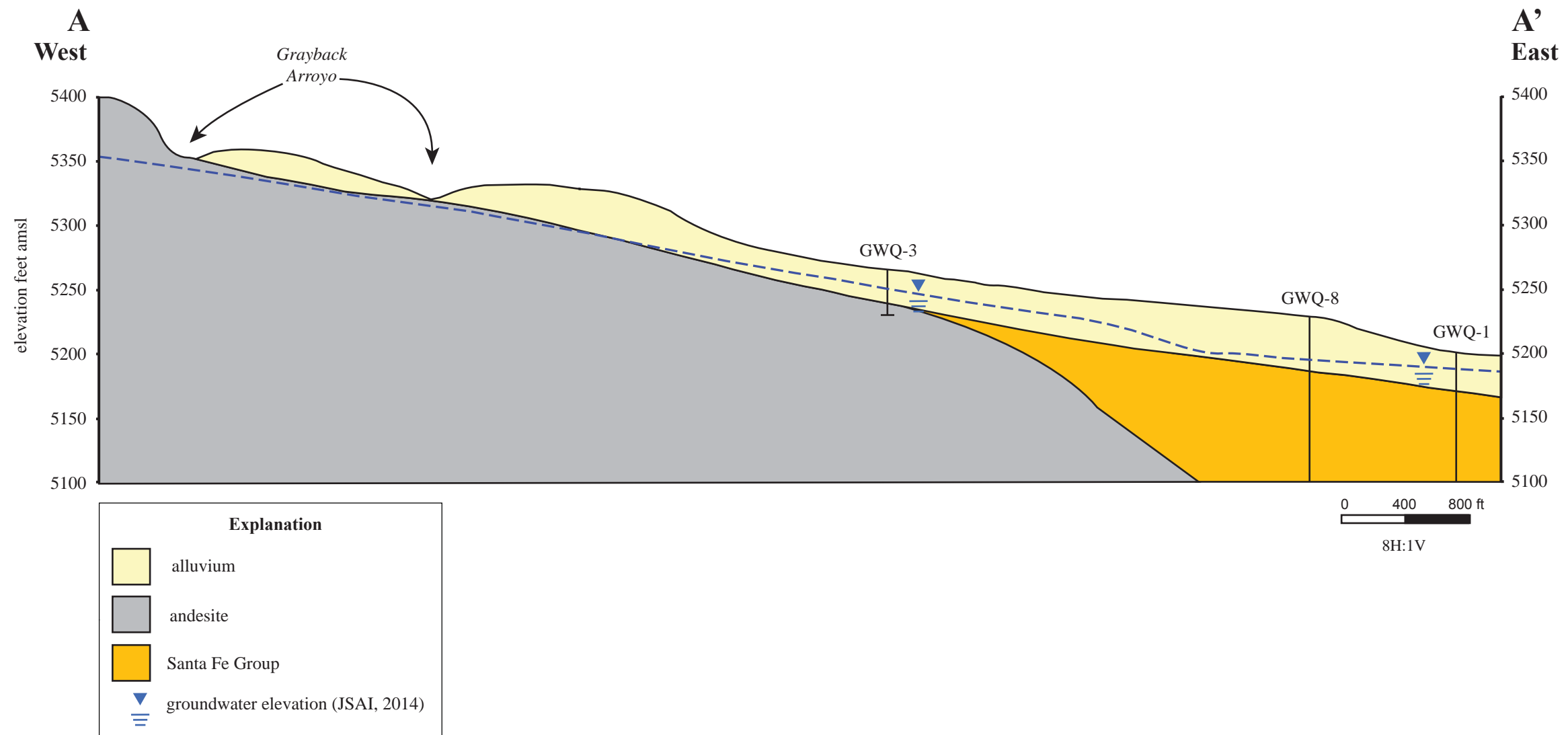


FIGURE 4-7: West to east geologic cross-section along Grayback Arroyo, Copper Flat Mine, Sierra County, New Mexico. (Reference Figure 4-2)



defines the eastern edge of the Animas Uplift. The Santa Fe Group deposits in the mine permit area are located west of the East Animas Fault. As indicated above, Figures 4-5 through 4-7 present hydrogeologic cross-sections showing geologic formations, water-bearing formations, and groundwater depths.

Most of the precipitation that recharges the groundwater system at the facility originates in the upper part of the watersheds to the west of the mine permit area. The main groundwater systems in the region are found in Santa Fe Group sedimentary deposits downstream of the mine permit area, with groundwater conveyed through more permeable Paleozoic sedimentary rocks located to the north and south of the facility. Runoff from Grayback Arroyo infiltrates the Santa Fe Group sedimentary deposits downstream of the mine permit area.

4.1.2 Implementation of MAT and BMPs at Copper Flat

Table 4-1 presents a summary of the potential sources of water constituents, discharge types, and locations at Copper Flat. Figures 2-2 and 2-14 show the locations of the potential sources during operations. The potential pathways for migration of constituents to ground water identified in Table 4-1 could potentially source from direct infiltration into the water bearing formations, release of fluids to the surface, or run-on and runoff of precipitation through and off of the site. NMCC has designed the Copper Flat facilities to maximally incorporate MAT and BMPs to protect human health and the environment based on site-specific technical and economic feasibility as discussed below.

Tailings Storage Facility (TSF)

The TSF will be a lined impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam. It will also include a water reclaim or recycle system to increase water reuse. The TSF also includes a cyclone plant to separate the tailings coarse and fine fractions and a surge pond to handle potential upset conditions at its Copper Flat Project. The location of the proposed new TSF is shown in Figure 2-2. Appendix A provides the technical design detail for the TSF. The TSF will be constructed using the coarse tailings sands materials produced from processing the ore for its mineral content. Whole tailings material will be transported via a pipeline from the processing facility to the TSF and delivered to the cyclone plant where sands and slimes fractions will be separated. The coarse or sand fraction of the tailings, or cyclone underflow, will then be pumped to the TSF for use in construction of the dam. The slimes or fines fraction of the tailings, or cyclone overflow, will be pumped to the TSF and deposited in the impoundment that will form behind the dam. Storm water run-on will be diverted around the tailings impoundment as shown in Figure 2-21. The diversion ditches are designed to be able to safely pass the peak flow generated by the 100-year storm event. The tailings impoundment will be lined with an 80-mil HDPE, or equivalent liner, placed on a minimum 12-inch thick liner



TABLE 4-1 Potential Sources of Water Constituents, Discharge Types, and Locations		
Potential Source	Discharge Type	Source Location
Tailings Storage Facility (TSF)	Tailings, process water and impacted storm water	Southeast area of site
TSF cyclone plant surge pond	Tailings	Southeast area north of TSF
Tailings slurry pipeline conveyance	Tailings	Process Area to TSF Permit Area Boundary
TSF water recycle system-under-drain collection pond	process water and impacted storm water	Eastern edge of site
TSF water recycle system pipeline conveyances	Process water and impacted storm water	TSF area to process water reservoir
Process water reservoir	Process water and impacted storm water	East-central area of plant site
Impacted storm water impoundment A	Impacted storm water and process water	Plant site Area
Impacted storm water impoundment B	Impacted storm water	Southwest corner of WS B
Impacted storm water impoundment C	Impacted storm water	Southeast corner of WS C
Waste Rock Stockpile (WRSP)-1	Impacted storm water	Western side of Watershed (WS) A
WRSP-2	Impacted storm water	Western portion of WS B
WRSP-3	Impacted storm water	Eastern portion of WS B
Open Pit	Impacted storm water, mine water	Western side of site
Material handling and processing-primary crushing	Process water	Central portion of site within WS A
Material handling and processing-crushing and grinding	Process water	Central portion of site within WS A
Material handling and processing-flotation and concentration	Process water	Central portion of site within WS A
Plant site sumps, tanks, pipelines and truck and equipment wash units	Process water	Central portion of site within WS A
Packaged water treatment plant	Influent sanitary waste, treated effluent water to the TSF	Central portion of site within WS A
Mobile Equipment Fuel Station	Petroleum Products-Diesel, Gasoline, Oil	Central portion of site within WS A

bedding fill material. An underdrain collection system will be installed on top of the liner as described in Appendix A to collect free water that drains from the contents of the impoundment. The collected water will drain via a collection gallery into an underdrain collection pond constructed at the foot of the impoundment dam outside of the dam structure as shown in Figure 2-2 and the design document.



A blanket underdrain collection system will also be installed under the dam to collect and water that drains from the tailings sands used to construct the dam. As the dam drains its free water, it will consolidate the sand material and add stability to the dam. The water captured by the blanket underdrain will also be collected in the undrain collection pond. The underdrain collection pond itself will have a double liner of 60-mil HDPE, or equivalent, and will be equipped with a leakage collection system to detect and collect any leakage through the primary liner layer.

The underdrain collection pond will also provide the collection point for surface water runoff from the down-slope face of the dam. As shown in Figure 2-22, the dam will be constructed in several phases. The dam will be constructed using an engineered material placement technique known as “centerline” construction. During reclamation, inter-benches will be cut into the face of the dam that will provide the location for the placement of runoff collection ditches to capture runoff and route it to the runoff collection trench at the toe of the dam. The runoff collection trench will be lined with 60-mil HDPE as described in Appendix A.

Water from the runoff collection trench will be routed to the underdrain collection pond. The water from the pond will continually be pumped back to the process water reservoir for reuse in processing. The pipeline that transports this water back to the process facility from the underdrain pond will be installed within the lined runoff collection trench, around the TSF as shown in Figure 4-8 and described in Appendix A. Should there be any leakage or spillage from the pipeline, it will simply run back via gravity to the underdrain pond. The underdrain collection pond will be sized to contain 24 hours of underdrain flow at maximum estimated drainage rates from the dam and impoundment underdrains, as well as runoff from the 100-year, 24-hour storm event of 3.73 inches incident on the downstream dam face. The pond capacity will be approximately 12.22 million gallons with 2 feet of freeboard.

The TSF will be equipped with a floating water reclaim barge located in the pond within the impoundment. The purpose of the barge is to pump as much free water as possible gathered within the impoundment back to the process water reservoir for reuse in the process. The water reclaimed from the tailings impoundment will be transported back to the process facility through a pipeline located within a trench lined with 60-mil HDPE that will provide secondary containment in the event of a spill or leak in the line. This lined trench will also contain the tailings pipeline that will transport whole tailings from the processing facility to the cyclone plant describe above. The pipeline from the underdrain collection pond to the process facility will combine with the barge reclaim water pipeline up-gradient of the TSF as shown in Figure 4-8.

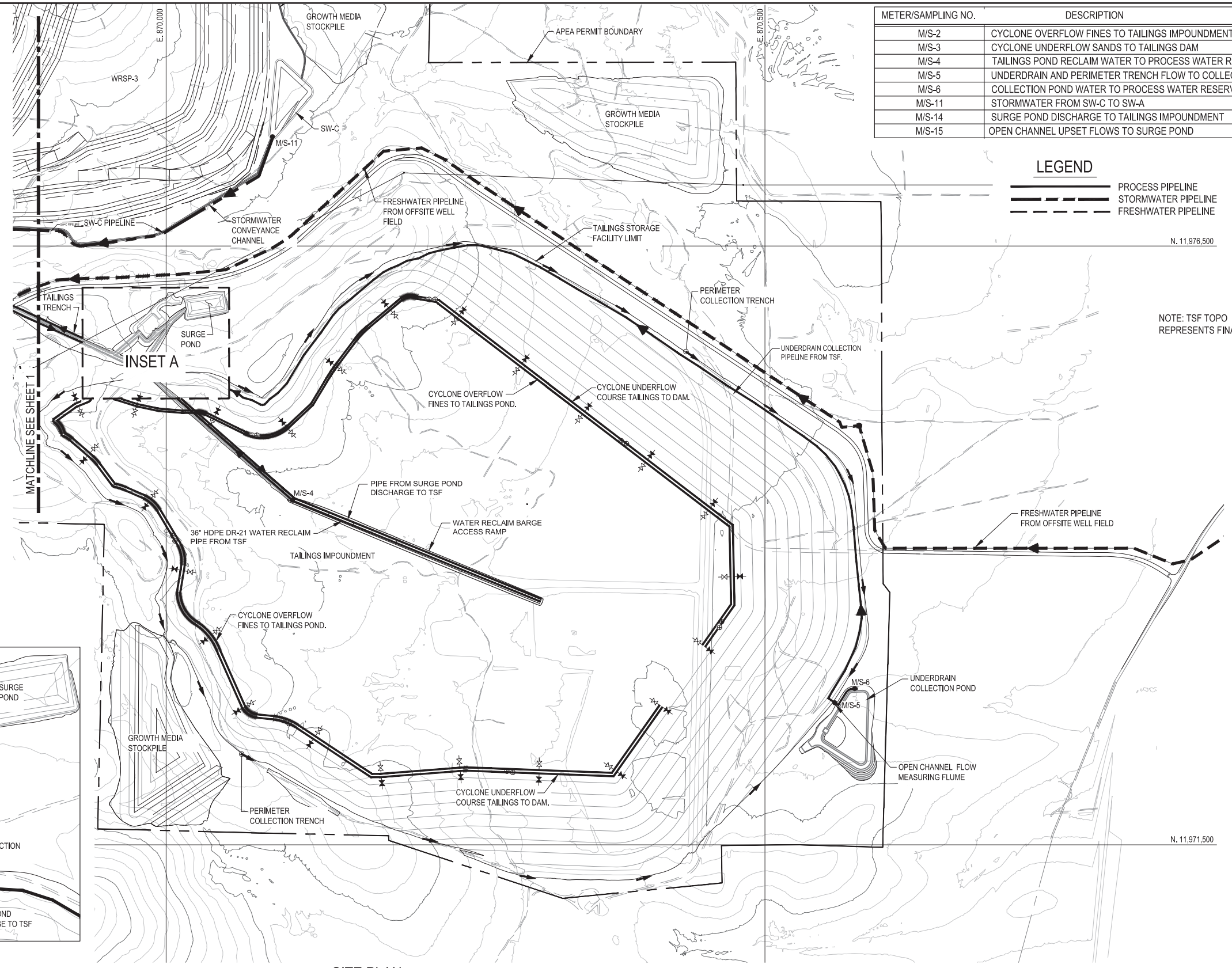
The TSF will also have an associated surge pond locate at the cyclone plant. The purpose of the surge pond will be to capture and temporarily retain tailings materials in the event of a



METER/SAMPLING NO.	DESCRIPTION
M/S-2	CYCLONE OVERFLOW FINES TO TAILINGS IMPOUNDMENT
M/S-3	CYCLONE UNDERFLOW SANDS TO TAILINGS DAM
M/S-4	TAILINGS POND RECLAIM WATER TO PROCESS WATER RESERVOIR
M/S-5	UNDERDRAIN AND PERIMETER TRENCH FLOW TO COLLECTION POND
M/S-6	COLLECTION POND WATER TO PROCESS WATER RESERVOIR
M/S-11	STORMWATER FROM SW-C TO SW-A
M/S-14	SURGE POND DISCHARGE TO TAILINGS IMPOUNDMENT
M/S-15	OPEN CHANNEL UPSET FLOWS TO SURGE POND

LEGEND

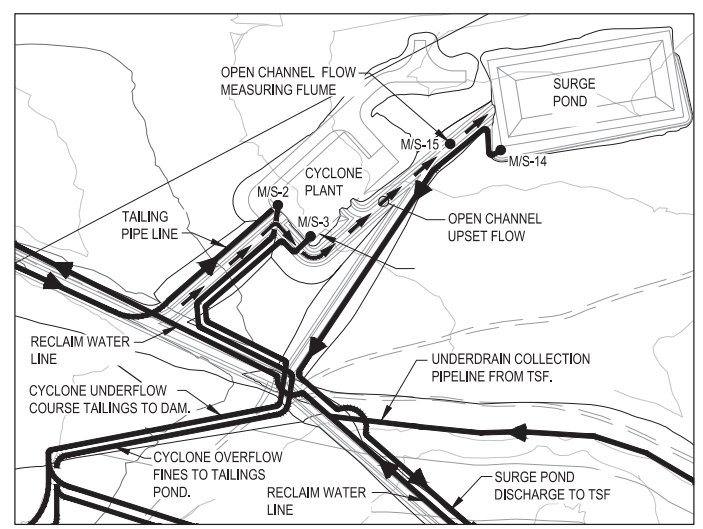
	PROCESS PIPELINE
	STORMWATER PIPELINE
	FRESHWATER PIPELINE



N. 11,976,500

NOTE: TSF TOPO REPRESENTS FINAL PHASE

N. 11,971,500



INSET A
SCALE IN FEET
150 75 0 150 300

SITE PLAN
SCALE IN FEET
400 200 0 400 800

PRELIMINARY
FOR AGENCY REVIEW



COPPER FLAT PROJECT

SITE GENERAL CIVIL
PIPELINES AND FLOWMETER LOCATIONS
SHEET 1

JOB NO. M3 PN-120885
DWG NO. **FIGURE 4-8**
REVNO. DATE
P2 09 JUN 16

DO NOT SCALE 11x17 DRAWINGS

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temporary upset in the cyclone plant and provide temporary storage in the event that an upset occurs in the cyclone circuit. The surge pond will be lined with a 60-mil HDPE liner. Under normal operating conditions the surge pond will be dry. However, it has been designed so that it can hold flows for one-half hour from maximum upset conditions that occur at the process facility plus maximum upset conditions at the cyclone plant both occurring during a 100-year, 24 hour precipitation event plus two feet of freeboard, all happening at the same time. Dedicated pumps will begin to evacuate the surge pond to the TSF at a pre-determined capacity of the pond within a half-hour of such an event occurring. In addition, the pumps will be tied into the emergency diesel power system located on-site in case of power is lost to the facility.

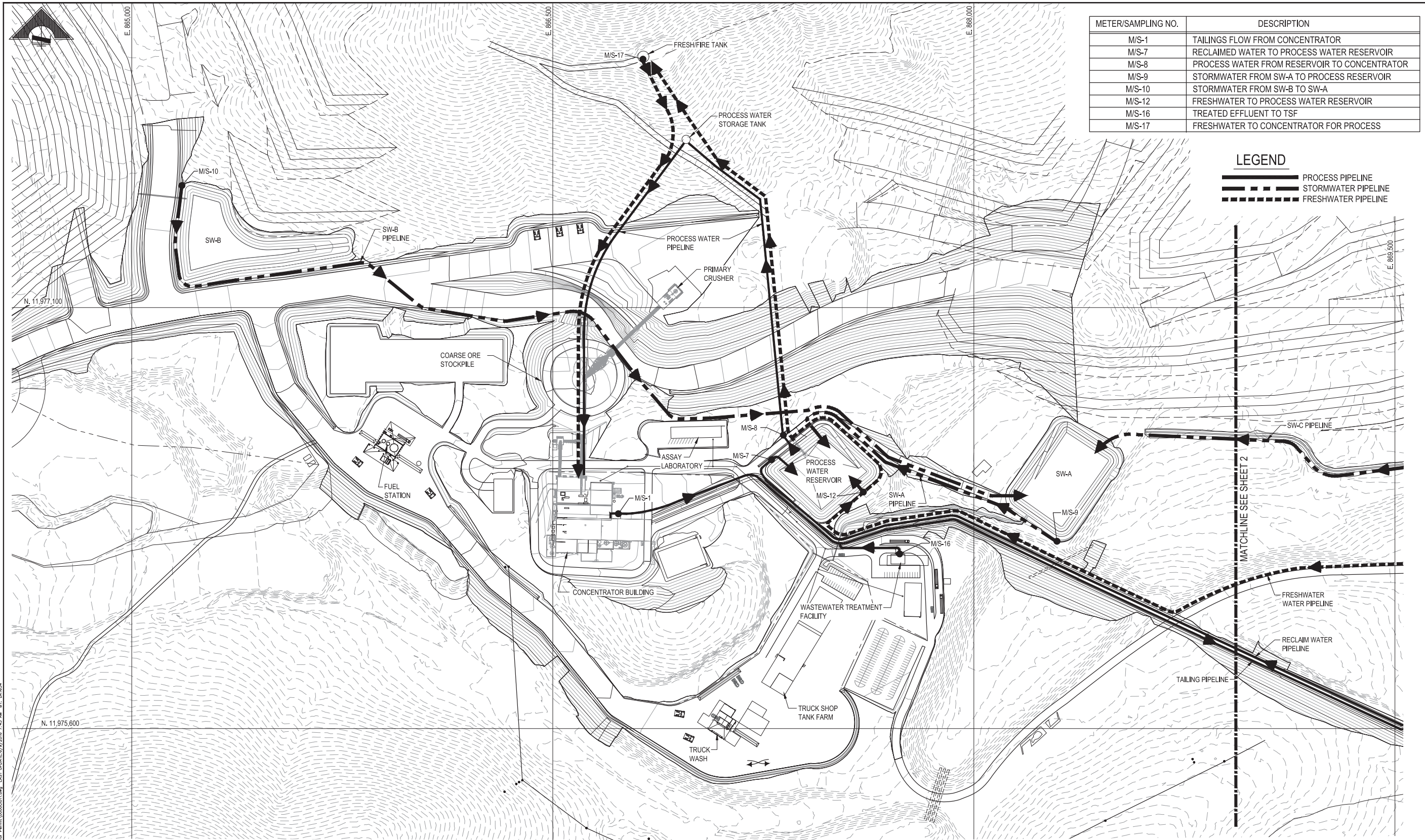
The use of MAT and BMPs at Copper Flat will extend to reclamation and closure of the TSF as described in detail in the Reclamation and Closure Plan. Surface re-grading of the tailings impoundment at closure will be performed that will provide a stable configuration to minimize ponding and promote conveyance of surface water. The final grade of the top surface of the re-contoured impoundment will be at least 1.0% after accounting for the magnitude and location of large-scale settlement of tailings.

Re-grading and contouring of the TSF to its post-mining configuration will begin at the end of the project life after it has been determined that sufficient water has been removed by the underdrain system. Because the Copper Flat TSF will be constructed with an engineered underdrain system, prior to conducting final grading activities NMCC will ensure that adequate drainage of the impoundment has occurred to ensure that large-scale settlement following grading is minimized. Consolidation drainage into the underdrain system is anticipated to continue at declining rates for a period following the cessation of tailings discharge operations. Underdrain water collected will be pumped from the underdrain collection pond and disposed of via evaporation utilizing active and passive systems as described in the TSF Post-Operations Water Management Plan, Attachment E2, of Appendix E.

A 36-inch topdressing soil cover will be placed on the top surfaces of the tailings impoundment and embankment out-slopes as described in Appendix E. The cover area will be seeded. Riprap and other erosion control structures will be placed as necessary in the drainage channels.

Other Impoundments, ponds and reservoirs

All of the impoundments, ponds and reservoirs will be lined with synthetic liner materials as described in more detail above and in Appendix B. In brief, the three impacted storm water impoundments and the surge pond will be lined with 60 mil HDPE liners. The process water reservoir will be double-lined with 60 mil HDPE liners and a leak collection system. There will also be a number of runoff collection trenches and water conveyance ditches constructed at Copper Flat to collect and convey water to the various water retaining structures as shown in Figures 4-8 and 4-9 and described in Appendix A and B. The trenches will have a 60 mil HDPE

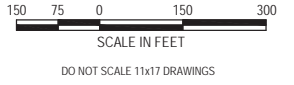


METER/SAMPLING NO.	DESCRIPTION
M/S-1	TAILINGS FLOW FROM CONCENTRATOR
M/S-7	RECLAIMED WATER TO PROCESS WATER RESERVOIR
M/S-8	PROCESS WATER FROM RESERVOIR TO CONCENTRATOR
M/S-9	STORMWATER FROM SW-A TO PROCESS RESERVOIR
M/S-10	STORMWATER FROM SW-B TO SW-A
M/S-12	FRESHWATER TO PROCESS WATER RESERVOIR
M/S-16	TREATED EFFLUENT TO TSF
M/S-17	FRESHWATER TO CONCENTRATOR FOR PROCESS

LEGEND

	PROCESS PIPELINE
	STORMWATER PIPELINE
	FRESHWATER PIPELINE

SITE PLAN



PRELIMINARY
FOR AGENCY REVIEW



COPPER FLAT PROJECT

SITE GENERAL CIVIL
PIPELINES AND FLOWMETER LOCATIONS
SHEET 2

JOB NO. M3 PN-120085
DWG NO. **Figure 4-9**
REV. NO. P2
DATE 09 JUN 16

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																TDL	FEB 13
																RKZ	FEB 13



liner, with exception of those trenches located at the WRSPs that are constructed into the impermeable bedrock as discussed earlier herein. The use of this MAT and BMPs will significantly mitigate against the potential for these structures being a pathway for migration. All of the impoundments, ponds, reservoirs, and conveyance ditches are designed to eliminate run-on and capture runoff to the maximum extent possible and contain storm events in accordance with the requirements of the NMED Copper Rules as described in the NMCC DP application during operations.

At reclamation/closure the impoundments, ponds and reservoirs will be decommissioned, the liners ripped and buried in place and the areas regraded, re-contoured, covered with top dressing and vegetated. The details of the reclamation/closure actions to be taken are described in Appendix E.

Waste Rock Stockpiles

As discussed in more detail above, the waste rock stockpiles will be constructed over bedrock that has a permeability of 10^{-6} cm/sec. As such, these areas will not require lining. While the waste rock stockpiles represent a potential source of migration on constituents to ground water, that potential is significantly mitigated by the natural barrier to migration that exists. The waste rock stockpiles also represent sources of potential pathways for migration of constituents to surface water. However, as discussed in detail above, each of the waste rock stockpile areas will be constructed within a developed watershed designed to eliminate run-on, control and capture runoff in a lined impoundment.

Surface water diversion ditches and swales will be constructed within the waste rock stockpile areas to divert water to the impoundments at a controlled rate while reducing the potential for ponding and infiltration. In addition, the WRSPs have been designed and will be constructed in a manner to promote contemporaneous reclamation to the extent practicable such that 3 feet of cover materials will begin to be placed on them, as discussed in Sections 3.4 and 4.2, and Appendix E.

These MATs and BMPs utilized at the WRSPs will provide appropriate protection of human health and safety, the environment, wildlife and domestic animals. During operations the waste rock stockpiles will be constructed to facilitate re-grading during reclamation such that inter-bench slope faces will be 3H:1V or flatter and shaped to enhance runoff and prevent infiltration and ponding. Inter-bench slopes lengths will be no longer than 200 ft. The composite overall slope, which includes the inter-bench slopes and benches, will be 3H:1V or flatter.

All of the WRSPs will be reclaimed in accordance with the approved Reclamation and Closure Plan. The side-slopes will be reconfigured, as needed to meet the requirements of the NMED



Copper Rules and the Mining Act regulations. The details of the reclamation/closure actions to be taken are described in Appendix E.

As discussed above, waste rock will be managed during operations based on NMCC's operations material characterization program and predictive geochemical modeling. Contemporaneous reclamation to the extent practicable, materials management and surface water control measures will be used to maximize runoff, reduce infiltration, and reduce contact with the air, thus minimizing the potential for acid generation. The top surfaces of the waste rock stockpiles will be designed and constructed to a minimum final grade of 1%. The potential for ponding on the final surface will be reduced by careful contouring of the surface.

The waste rock stockpiles will be reclaimed at the end of the life of operations by re-grading re-contoured, as necessary, and a 36-inch soil cover will be placed over the stockpiles, unless NMCC can demonstrate a thinner cover will resist erosion, sustain vegetation and be equally protective of groundwater considering site-specific reclamation plans for the facility in conformance with the NMED Copper Rules. The cover area will be seeded thereafter as soon as practicable. The top surfaces of the stockpiles will be graded to promote positive drainage to storm water conveyance channels. These channels and hydraulic structures will be designed to control erosion on the top surfaces and out-slopes and safely convey storm-water off of the stockpile areas. Cross bench drainages will be trapezoidal and constructed to safely convey storm-water off reclaimed slopes for a 100-year precipitation event that results in the peak discharge. Longitudinal slopes for these drainages will be 1 to 5 percent. Energy-dissipation structures will be constructed at channel outlets to reduce erosive velocities where necessary. Where possible, channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock, which will promote long-term integrity of the structures. The final designs will be adjusted for local conditions. Temporary erosion control measures will be provided during the reclamation construction and early vegetation establishment periods. These control measures include mulch, straw bales, silt fences and minor corrective re-grading, as necessary.

Process Area

The process area also represents a source of potential migration of constituents to ground water and surface water. With respect to ground water, the potential pathways for migration exist largely in the form of process water handling within the process, in particular at sumps that hold and transfer water from one part of the process to another. The sumps in the process area, including the crushing and grinding and flotation and concentrating areas, are designed and will be constructed using MAT techniques such as water-stops and single monolithic pours to reduce the likelihood of release.

With respect to the potential for release to surface water, the process area will be constructed



within a developed watershed that will be graded such that run-on will be eliminated and runoff will be controlled and captured in the lined impacted storm water impoundment A, as shown in Figure 4-9. Water from all of the process facilities, material handling areas, parking areas, storage areas, chemical and fuel inventory areas and pipeline surface areas will be routed to the impoundment.

Also, as noted above, tailings from the process facility will be transported through a pipeline to the cyclone plant. The tailings pipeline will be laid within the same trench as described above that will contain the water reclaim line from the TSF to the process water reservoir. This trench, shown in Figures 4-8 and 4-9, will provide secondary containment for all process water in the event of an upset condition in the process area and/or a significant precipitation event that may cause the process water reservoir to overflow. As discussed in detail in Appendix C, the process area has been designed such that all process water can be captured into this “secondary containment trench” and routed to the surge pond described above. From the surge pond, captured process fluids will be pumped to the TSF.

4.2 Contemporaneous Reclamation – 19.10.6.603.B

The concept of contemporaneous reclamation, operating-for-closure, or “designing for closure”, is a means by which NMCC will reduce erosion, provide early impact mitigation, limit costs and reduce final reclamation work. NMCC is committed to maximizing this type of reclamation at the Copper Flat Project where feasible. It has designed mine facilities to employ contemporaneous reclamation to the extent appropriate and practicable. Section 3.4 of this MORP update and the Reclamation and Closure Plan contain a detailed discussion of the contemporaneous reclamation activities that NMCC will undertake to take maximal advantage of site-specific conditions accomplish optimal closure of the site at the end of the mine’s life.

4.3 Assure Protection – 19.10.603.C

NMCC has designed its Operations and its Reclamation and Closure Plan to assure protection of human health and safety, the environment, wildlife and domestic animals. Mine development and operation activities will also be implemented to assure that protection.

4.3.1 Signs, Markers and Safeguarding – 19.10.6.603.C.(1)

NMCC will implement and maintain safeguarding measures such as signs, markers, fences and barricades to protect the public, wildlife and domestic animals from potentially dangerous areas associated with the Project. Access to the permit area will be controlled at all times during mining operations and reclamation phase to protect the public from possible injury due to operating conditions such as heavy equipment and truck traffic. All personnel entering the



site will be checked in, receive site-specific safety training and will be escorted by trained personnel.

Shafts, Adits or Tunnels – 19.10.603.C.(1)(a)

This Section does not apply to the Copper Flat Project as no underground workings are proposed. However, some historic underground mine workings exist within the permit area and will remain at closure. These historic workings have been and remain the responsibility of federal government. Nonetheless, because the existence of these workings have the potential to attract interest as a result of the increased activity at the site, NMCC will work with the BLM to ensure that they are appropriately safeguarded from unauthorized entry.

Warning Signs – 19.10.6.603.C.(1)(b)

NMCC will comply with the specific standards and regulations with respect to warning signage for mine operations and visitors as required by MSHA. Appropriate warning signs will be posted at strategic locations at the Copper Flat Project site around the perimeter and across the mine permit area beginning with the initial construction period, through mine operations until the completion of reclamation, as appropriate. Other markers or signs may be posted based on the facilities or activities at specific times.

Access Restriction to Hazardous Areas – 19.10.6.603.C.(1)(c)

All hazardous areas within the perimeter of the proposed permit area, such as ponds, electrical installations, power lines, reclaimed areas, explosives storage areas, etc., will be fenced and posted with appropriate warning signs.

Permit Area Boundaries – 19.10.6.603.C.(1)(d)

The mine permit boundary will be fenced and posted with signs warning of unauthorized entry and stating the appropriate hazard warning. A single public access point will be established at the main entrance to the mine site. Fences and locked gates will be placed at all secondary road entrances to the proposed permit area.

Main Entrance Signage – 19.10.6.603.C.(1)(e)

The main entrance to the site will have a security guard in a gatehouse to stop and check in personnel and visitors. Signs will be posted at the main entrance identifying the Project, the operator and a telephone number and other contact information in the event of emergencies related to the mining operation.

4.3.2 Wildlife Protection – 19.10.6.603.C.(2)

Construction, operations and reclamation phases of the Project will not impact critical habitat for wildlife based on wildlife studies conducted on site. Physical disturbances will be limited to only those areas needed for mine facilities and access, minimizing impacts to surrounding



habitat that may be used by wildlife. Construction activities such as land clearing and surface disturbance will be conducted, to the extent practicable, in a manner to minimize disturbance to active bird nests and/or birds' young, if possible, and to prevent habitat destruction whenever possible. The services of qualified biological consultants will be retained to provide assistance to NMCC in evaluating areas prior to the occurrence of construction disturbances to provide recommendations on the manner to proceed in the event of the presence of active nests during construction. Reasonable attempts will be made to minimize disturbances to active nests and/or mating bird pairs. Electric transmission or physical power poles will be constructed in a manner to protect raptors from potential electrocution hazards.

NMCC will take measures to minimize adverse impacts on wildlife and important habitat, including the installation of fencing around the perimeter of the permit area boundary to restrict humans and livestock entry to the Project area. Special wildlife exclusion fencing will be used in specific high hazard areas such impoundments, electrical substations and reagent areas. Gates and/or cattle guards will be installed along roadways within the proposed permit area, as appropriate. Because all of the impoundments and ponds are lined with HDPE it will be necessary to make every effort to deter access by wildlife and domestic animals and keep them off of the liner in order to protect its integrity and minimize the potential of puncture. At more remote locations at the site where there is less human activity and wildlife and domestic animals may be more likely to be enticed, it may be necessary to provide additional barriers such as individual exclusionary fencing to deter trespass. All of the ponds and impoundments (with the exception of the tailings impoundment) have been designed with an escape ramp so that an animal that may find its way into the pond may be able to get out.

Reclamation of the site will be performed with the goal of re-establishing the land use of the site as grazing and wildlife habitat, as it is today. Grading, re-contouring, covering and re-establishing vegetation at the site in accordance with the approved Reclamation and Closure Plan will allow NMCC to meet this goal.

4.3.3 Cultural Resources – 19.10.6.603.C.(3)

NMCC conducted cultural resource investigations as part of its baseline data gathering efforts to develop the information needed to assess the potential impacts of the proposed project on cultural resources and to meet compliance requirements for applicable State and Federal regulations, particularly Section 106 of the National Historic Preservation Act as part of its baseline data gathering efforts (Intera 2012). These investigations were conducted in accordance with State and Federal standards, and included survey and tribal consultation. Section 3.13 of the BLM DEIS contains a detailed discussion of the cultural resource considerations applicable to the Copper Flat Project.

In its evaluation in the DEIS, the BLM determined that there would be a significant impact to



historic properties from development of the Copper Flat Project that would result in an adverse effect to historic properties. The majority of these impacts would occur due to facility construction, surface activities at the mine area, removal of mineralized ore, and traffic. BLM has determined that prior the commencement of any mine development activities that it will complete Section 106 consultation with the Advisory Council on Historic Preservation, State Historic Preservation Office (SHPO), Tribes, and NMCC. The purpose of the consultation will be to develop measures to avoid, minimize, or mitigate the adverse effects to historic properties. A Programmatic Agreement (PA) will be developed for signature by the parties, which will document the measures to be implemented. The following measures to avoid, minimize, or mitigate adverse effects are examples that may be considered and included in the PA:

- Conducting data recovery excavations of archaeological sites;
- Fencing of sites and activity areas to prevent impacts;
- Implementing a monitoring program to ensure avoidance measures are effective, and to modify such measures if not effective;
- Implementing standard best management practices during construction and operations activities to control erosion and changes to erosion patterns;
- Training of NMCC construction, operations, and reclamation personnel and contractors to recognize when archaeological resources or human remains have been discovered, to recognize when inadvertent damage has occurred to a resource, to halt ground disturbing activities in the vicinity of the discovery, and to notify appropriate personnel; and
- Educating NMCC personnel and contractors on the importance of cultural resources, the laws and regulations protecting cultural resources, the need to stay within defined work zones, and the legal implications of vandalism and looting.

The PA will describe the processes to be followed in the event that previously unknown cultural resources or human remains are discovered during construction or operation of the selected alternative, and will address processes to be followed in the event that inadvertent physical damage to an historic property occurs. While the effects to the resources will remain, the PA and the measures contained within it will resolve these effects and reduce the significance of the impacts. The PA will address all effects to historic properties, and will document NMCC's, the BLM's, and other regulatory agencies' commitment to ensure that the mitigation measures are implemented.

NMCC is committed to this PA process. Roads and project facilities at Copper Flat will be sited as much as possible to avoid cultural resource impacts. If avoidance is not possible or is not adequate to prevent adverse effects, NMCC will undertake data recovery from such sites. Development of a treatment plan, data recovery, archeological documentation and report preparation will be based on the Secretary of the Interior's "Standards and Guidelines for



Archeology and Historic Preservation,” 48 CFR § 44716 (September 29, 1983), as amended or replaced. If an unevaluated site cannot be avoided, additional information will be gathered and the site evaluated. If the site does not meet eligibility criteria as defined by the New Mexico SHPO, no further cultural work will be performed. If a site meets eligibility criteria, a data recovery plan or appropriate mitigation will be completed.

An archaeologist will be consulted prior to and during construction to advise NMCC and its contractors and to issue clearances, as necessary, for construction activities and provide guidance and expertise to ensure the protection of cultural properties. The appropriate agency will be notified immediately if additional cultural sites are discovered during these activities. Mitigation strategies will be developed in consultation with the agency.

4.3.4 Hydrologic Balance – 19.10.6.603.C.(4)

NMCC has planned and designed operations of the Copper Flat facility in such a manner to minimize change to the hydrologic balance in the permit area and potentially affected areas. As discussed in Section 3, above, the resulting post-mining reclamation hydrologic balance will be similar to the pre-mining hydrologic balance. The following discussion addresses the methods by which NMCC will ensure this balance occurs.

Non-point Source Surface Releases of Acid or Other Toxic Substances – 19.10.6.603.C.(4)(a)

NMCC has designed and will operate the Copper Flat facility such that no releases of acid or other toxic substances will occur. To the extent that unanticipated releases may occur, all such releases will be contained within the permit area. Because the design of the facility contemplates retaining all surface flows in impoundments and/or ponds with no release of surface flows from the disturbed area, no treatment will be required. Sources of potential non-point source surface water drainage at Copper Flat include; the mine pit, the process plant area, the WRSPs and the TSF. All of these areas and their surface water protection features have been discussed earlier herein.

Surface water drainage in the area of the open pit is into the OPSDA. As such, no non-point source or releases off of the permit area from this location can or will occur as the open pit will act as a hydrologic sink for surface and ground water during and after operations.

The likelihood of acid generation from waste rock is very small (as discussed in Section 3.3, above); therefore, its release as a non-point source to the surface unlikely. NMCC has designed waste handling and disposal plans for the WRSPs as discussed in Sections 2.2.11 and 3.3.2. Surface water runoff and surface water that may manifest itself as a “seep” in the WRSP that could result from percolation of precipitation through the stockpile will be captured in runoff collection trenches and routed to lined impoundments. Water captured in these impoundments will, in turn, be evacuated to the double-lined process water reservoir located



in the process plant area, within thirty days of capture (per the NMED Copper Rules) for use in the process. Therefore, non-point source releases from the WRSPs will not occur.

In addition to the WRSPs that will be constructed per NMCC's proposal, there are four existing waste rock stockpiles on-site, i.e., EWRSP-1, EWRSP-2A and 2B, EWSRP-3 and EWRSP-4 as shown in Figure 2-2. EWRSP-1 and EWRSP-2B are located in the OPSDA and, as such, will not contribute non-point source releases during operations. EWRSP-2A is located in the area where WRSP-1 will be constructed and will be incorporated into it during operations. Thus, it will not contribute non-point source releases during operations. However, currently the westernmost edge of EWRSP-1 and the northernmost edge of EWSRP-2A may be contributing surface water runoff away from the site. NMCC will reclaim these areas as described in the Reclamation and Closure Plan so that no non-point source releases occur from them.

As shown in Figure 2-21, the TSF is designed so that no non-point source releases occur from the facility. Run-on into the location of the TSF will be diverted around and away from the TSF. Tailings water will be retained in the lined impoundment behind the dam and will be recycled utilizing a floating water reclaim barge in the impoundment and an underdrain collection system at the bottom of the impoundment. Drainage collection trenches will be constructed on the out-slope side of the tailings dam to capture and direct runoff to the lined underdrain collection pond. An underdrain collection system will also be placed under the dam to collect water that drains from the dam and routed to the pond. The water in the pond will be pumped back to the process water reservoir for reuse in the process. Therefore, there will be no non-point source releases from the TSF.

Surface water runoff from the process plant area will be captured in a lined impoundment. That water will also be pumped to the process water reservoir for use in the process. Sources of potential non-point source releases in this area include the primary crusher and ore storage pile, the grinding and concentrator facilities, the reagent storage areas and two existing waste rock stockpiles, EWRSP-3 and EWRSP-4. . All surfaces will be graded to drain into contained area or impoundment. Therefore no non-point source releases will occur from plant area.

EWRSP-4 will be graded and contoured to be utilized as an equipment lay-down area. Runoff from EWRSP-4 will be routed to the mine pit during operations. The southern exterior of EWRSP-4 will be reclaimed during operations as described in the Reclamation and Closure Plan to ensure that no non-point source surface releases of acid or other toxic substances occur.

Control of Suspended Solids – 19.10.6.603.C.(4)(b)

Sediment control will be achieved by the use of BMPs to stabilize the site and ensure that the permit area does not contribute suspended solids above background levels, or if applicable above the Water Quality Control Commission's standards, to intermittent and perennial



streams. All of the water courses in the permit area and the potentially affected areas are ephemeral, flowing only in response to precipitation events. This condition notwithstanding, the following types of BMPs may be utilized and maintained, as needed during construction, operation and reclamation/closure of the facilities:

- Fiber rolls (waddles)
- Ditches/swales
- Diversion channels
- Energy dissipaters
- Slope drains
- Sediment traps and fences
- Stilling basins
- Impoundments and ponds

A SWPPP will be developed for the permit area that outlines the mechanisms to control storm water run-on and runoff from disturbed areas. The SWPPP will be based on the final mine design. Mine employees will be trained to its requirements prior to commencement of construction and/or mining operations.

Roads will be constructed with a center crown to direct storm water to the side ditches. The ditches will have sediment control features such as water bars, fiber rolls or other traps to reduce sediment if the water enters an arroyo. These sediment control devices will be placed around construction or operational areas for temporary surface disturbance activities. Grading, revegetation, where appropriate and/or necessary and concurrent or contemporaneous reclamation will also be utilized as sedimentation control mechanisms.

Background Surface Water Quality – 19.10.6.603.C.(4)(c)

As part of the baseline study previously submitted, NMCC installed a surface water sampler where Grayback Arroyo enters the proposed mine permit boundary. The sampler is designated as SWQ-1 in Figure 8-7 of the BDR. This sampler was dry on each quarterly sampling event during the baseline study. However, continued sampling has continued since that time and any results that become available will be reported to establish background surface water quality.

Diversions of Overland Flow – 19.10.6.603.C.(4)(d)

NMCC's design of the Copper Flat facility includes several diversion structures to control overland flow of water at the site including runoff control and diversion ditches at the WRSP facilities, in the plant process area and at the TSF. The design specifications for these structures can be found in the various Appendices included in this MORP update. These structures have been designed and will be constructed and maintained to minimize adverse impacts to the hydrologic balance and assure public safety in accordance with the requirements of the



performance and reclamation standards and requirements of the Mining Act regulations and the requirements of the NMED Copper Rules.

Slope stability is of utmost importance in construction and operation of the Copper Flat facility. None of these structures will be located so as to increase the potential for landslides. All of the structures are designed to safely pass the peak runoff from a 100 year, 24 hour precipitation event and have been certified by a professional engineer registered in New Mexico as having been designed in accordance with 19.10 NMAC and the NMED Copper Rules. A complete set of diversion design documents will be kept on-site for inspection by the Director or his designee. When no longer needed, upon completion of reclamation/closure of the site, temporary diversions will be removed.

4.3.5 Stream Diversions – 19.10.6.603.C.(5)

NMCC does not propose to construct any stream diversions at the Copper Flat facility. There is a permanent existing diversion of Grayback Arroyo, an ephemeral watercourse that transects the site from west to east that will be maintained to continue to divert water safely around and through the site. NMCC has performed a diversion analysis to determine the ability of this diversion to protect the site. The report of this analysis is contained in Appendix D of the MORP update. The 24-hour storm flows for 100-year, 200-year and 550-year return periods were evaluated for the existing diversion structure. The report concludes that the existing diversion structure is protective of the site. The report also recommended minor repair and maintenance of the upstream inlets of the culverts located at the roadway and pipeline crossing (see Drawing 0000-CI-103 in Appendix D). NMCC will conduct said repair and maintenance at the beginning of construction.

4.3.6 Impoundments – 19.10.6.603.C.(6)

NMCC's design of the Copper Flat facility includes several impoundments, reservoirs and ponds, as shown on Figures 2-2 and 2-14, including three impacted process water impoundments, one process water reservoir, one tailings storage impoundment, one underdrain collection pond and one surge pond. The design specifications for these structures can be found in Appendix A and B. In addition, there will be an evaporation pond constructed after the end-of-life of the mine to provide long-term passive capture and evaporation of residual fluids that may be produced from the tailings impoundment. This pond will be constructed essentially as an extension of the underdrain collection pond, expanding its size and capacity as described in Appendix E. The liner system of the underdrain collection pond will be extended to the constructed evaporation pond. As such, the design of the pond is the same as that for the underdrain collection pond, except that it will be more shallow in depth as described in Appendix E. The design of the underdrain collection pond is described in Appendix A.



These impoundments have been designed and will be constructed and maintained to minimize adverse impacts to the hydrologic balance, protect adjoining property and assure the safety of the public in accordance with the requirements of the performance and reclamation standards and requirements of the Mining Act regulations and the requirements of the NMED Copper Rules.

As shown in the design documents provided herewith, all of the impoundments, ponds and reservoirs (except the tailings impoundment) meet or exceed the design requirements of Section 19.10.6.603.C.(6)(a)(i through ix). The tailings impoundment meets or exceeds the design requirements specified by the OSE. When no longer required, the impoundments, ponds and reservoirs will be reclaimed in accordance with the Reclamation and Closure Plan.

4.3.7 Minimization of Mass Movement – 19.10.6.603.C.(7)

All slopes, embankments and the stockpiles will be designed, constructed and maintained to prevent the potential for mass movement both during operations and following closure. Details of the WRSP and TSF designs are presented in Section 2.

4.3.8 Riparian and Wetland Areas – 19.10.603.C.(8)

Disturbance to riparian and wetland areas at Copper Flat will be minimal. The Copper Flat Project area is primarily a terrestrial habitat with limited riparian and wetland habitats. The primary riparian areas are associated with the Grayback Arroyo and the established diversion. Arroyo areas within the proposed permit area boundary occur along Grayback Arroyo, the diversion channel, and pit lake. The arroyo vegetative cover has the highest woody plant density within the proposed mine area. The majority of vegetation within this land cover consists of shrubs, with Emory's baccharis (*Baccharis emoryi*) being the most abundant. Burro bush (*Hymenoclea monogyra*) is also frequent in Grayback Arroyo. Grasses make up 24 percent of the relative vegetation cover, with vine mesquite (*Panicum obtusum*) being the most abundant. Other vegetation found in Grayback Arroyo includes desert willow (*Chilopsis linearis*), Goodding's willow (*Salix gooddingii*), cottonwood, four wing saltbush (*Atriplex canescens*), and salt cedar (*Tamarix* spp.) (BLM DEIS 2015, Section 3.11.1.1, pg 3-142,143). This variety and distribution of vegetation in this area of the arroyo is quite typical of the riparian habitat throughout the area in the arroyos in the vicinity of the site.

NMCC operations will not change the existing surface water flow conditions and will maintain the existing hydrologic conditions that support the riparian areas. All riparian areas will be managed appropriately according to state and federal requirements.

Mining operations will involve the drawdown of groundwater from the pumping of water from the fresh water well field that will provide a portion of the water required for processing. However, none of the hydric soils at the mine site or elsewhere in the potentially affected area



will be affected by that drawdown. Hydric soils in the wetlands along the arroyos and other water courses in the area do not rely on groundwater. They have an alternative source of water, such as flooding from surface water runoff or a perched water table.

The BDR (Intera 2012) indicates that during mine area surveys, two locations within the proposed mine area boundary appeared to meet wetland conditions as defined by the Clean Water Act (i.e., dominance by hydrophytic vegetation, hydric soils, and wetland hydrology). One of these areas is a small cattail wetland adjacent to the pit lake. The second wetland area, a patch dominated by Gooding's Willow, is estimated to be 1.5 acres in size. It is located within the mine at the bottom of Grayback Arroyo just below the culvert where the pit access road crosses Grayback Arroyo. Seep willow (*Baccharis salicifolia*) also occurs here (BLM 2015 DEIS, Section 3.11.1.1, pg 3-145).

Hydric soils of the small cattail wetland adjacent to the pit lake will be removed at the outset of operations since pumping of the pit lake will be necessary prior to mining and continuously throughout the life of the mine. This small wetland will be mined out when the pit is deepened so no surface soils will remain. As discussed in Appendix E, this small wetland will be replaced with the addition of two small water retention basins east of EWRSP-1. The second wetland area near the main mine entrance will not be affected by drawdown associated with activities at Copper Flat because it will be outside of the drawdown area. This area overlies the andesite bedrock of the Animas Uplift. As a result, there is no aquifer underlying the surface (BLM DEIS, November 2015, Section 3.8.2.1.1, page 3-110).

Engineering designs provided in the Reclamation and Closure Plan have set a minimum 50-foot set-back from Grayback Arroyo for the final reclamation footprints for GMSP-2, GMSP-3, WRSP-2, and WRSP-3 footprints and a 25-foot set-back for EWRSP-1. Riparian areas delineated at the plant area will not be disturbed and the land bridge will remain which will be protective of the Gooding's Willow community in Grayback Arroyo east of the mine entrance.

4.3.9 Roads – 19.10.6.602.C.(9)

For the most part, existing haul roads will be utilized to haul material to the crusher, stockpiles and WRSPs. Some minor realignment of these roads may be necessary and road widths will vary. Roads will be constructed and maintained to control erosion. Drainage control structures will be used, as necessary, to control runoff and minimize erosion, sedimentation and flooding. Drainage facilities will be installed as road construction or extension progresses and will be capable of passing a 10-year, 24 hour precipitation event. Culverts and drainage pipes will be constructed and maintained to avoid plugging, collapsing or erosion.

Haul roads are not expected to create new disturbances, as they will be constructed on previously disturbed land. Mined material will be hauled to the WRSPs and the mill using



conventional mining haul trucks. The on-site service roads will be designed for easy access and traffic movement within the operations area. No roads will be constructed that cross intermittent or perennial streams. Access to the project area is via an existing county road (Gold Dust Rd./Co. Rd. Bo27) which will remain following closure. Prior to final closure, the State of New Mexico and the BLM will determine which other roads will be made permanent in the project area to conduct post-closure monitoring or provide adjacent landowner access. A number of pre-1981, primitive roads currently exist within the proposed project boundary. Some of these roads will not be utilized during the proposed operations and will remain.

4.3.10 Subsidence Control – 19.10.6.603.C.(10)

No underground or in situ mining are proposed to be conducted at Copper Flat. Therefore, subsidence control is not a consideration for the Copper Flat Project.

4.3.11 Explosives Blasting – 19.10.6.603.C.(11)

Blasting will be conducted in a manner to prevent injury to persons or damage to property not owned by the operation. The generation of fly rock will be minimized to ensure that it is confined to the permit area. Blasting will be limited to the daylight hours and performed by trained and certified blasters. Safe seismic disturbance and air blast limits will be established to prevent damage to buildings.

Blasting agents and explosives such as ammonium nitrate and diesel fuel will be stored onsite in bins and tanks. Detonators, detonating cords, boosters, caps and fuses will be stored in two separate magazines. Ammonium nitrate will be stored in a 75-ton capacity, 3,000 ft³ silo. All explosive materials will be stored away from the plant site in compliance with MSHA, New Mexico State Mine Inspector's regulations, Bureau of Alcohol, Tobacco and Firearms (BATF), and U.S. Department of Homeland Security requirements. The magazines will be situated away from occupied buildings in compliance with the BATF and each magazine will be secured with two locks (see Figure 2-2).

Appropriate warning signs will be placed in such a way that a bullet passing through the sign will not strike the magazines. NMCC employees who use and handle explosives will do so in accordance with MSHA regulations and will meet all BATF, MSHA and state qualification and certification requirements. All transportation of explosives will meet MSHA and state requirements. An inventory will be kept of all explosives received into and distributed out of the magazines.

4.4 Site Stabilization & Configuration – 19.10.6.603.D

The permit area will be stabilized to minimize future impact to the environment and protect air and water resources. The final surface configurations of the disturbed areas subject to reclamation/closure will be suitable to achieve the post-mining land uses in accordance with



the approved Reclamation and Closure Plan. Contemporaneous reclamation actions undertaken at Copper Flat during operations will have a positive impact on site stabilization, configuration, and final reclamation. Contemporaneous reclamation is discussed in more detail in Section 3.4. Appendix E contains a more detailed discussion of the steps to be taken to stabilize and configure the site so as to achieve the post-mining land use. These steps are summarized below.

4.4.1 Final Slopes and Drainage Configuration – 19.10.6.603.D.(1)

The final slopes and drainage configuration of the reclaimed areas are designed to be compatible with the post-mining land use. The reclamation design is driven by the requirements of the NMED Copper Rules as well as the Mining Act regulations to promote slope and drainage stability. Consideration will be given to providing a diversity of topographic relief that assists in promoting vegetation diversity within the prescriptive context of the NMED Copper Rules thus providing a geomorphic component to the design. For example, the topographic disturbances, slopes, and other aspects of the disturbed project areas will be contoured to blend in with the surrounding topography to the extent practicable within the constraints of the NMED Copper Rule. Final slopes will be 3H:1V or shallower and will be restructured to resemble existing topography to the extent practicable. A few areas may have steeper slopes and would be stabilized by physically with coarser materials to add to general diversity and stability. Flatter disturbed areas (slopes of 4H:1V or less) will be minimally regraded to restore an appropriate drainage system and revegetated. Re-grading will be completed to direct water away from out-slopes, particularly on the WRSPs and the TSF. Where possible, the size and shape of new channels will approximate former drainages. All drainage channels, ditches and earthen water control structures will be revegetated and/or protected from erosion by riprap, sediment traps or other BMPs.

4.4.2 Backfilling – 19.10.6.603.D.(2)

The impoundments, ponds and reservoirs constructed at the site will be backfilled with embankment materials and re-contoured and reclaimed to blend into the natural topography. Because the Copper Flat deposit must be mined sequentially from top to bottom, NMCC will not backfill the pit.

4.4.3 Minimizing Mass Movement – 19.10.6.603.D.(3)

All reconstructed slopes and embankments of the WRSPs and TSF are designed and will be constructed and maintained to prevent the potential for mass movement. The Reclamation and Closure Plan contains the design details in Section 2 of Appendix E.

4.4.4 Acid and other Toxic Drainage Formation – 19.10.6.603.D.(4)

Section 3.3 provides a discussion of the measures that will be taken to reduce, to the extent practicable, the formation of acid and other toxic drainage that may otherwise occur following



closure to prevent releases that cause federal or state standards to be exceeded. NMCC's waste characterization efforts confirm that the likelihood of the formation of acid from the Copper Flat Project is very small (SRK 2013). NMCC's material handling, waste classification, waste rock flagging and routing and transitional waste materials disposal plans, as discussed in Section 3.3, to be implemented during operations, will prevent the formation of acid and other toxic drainage following reclamation/closure.

4.4.5 Non-Point Source Releases – 19.10.6.603.D.(5)

Section 4.3.4 provides a discussion of the measures that will be taken to ensure that there will be no non-point source surface releases of acid or other toxic substances during operations.

4.5 Topsoil (Topdressing or Cover Material) – 19.10.6.603.E

An important feature of “topsoil” is the presence of decomposed organic matter and bacterial, fungi, and other organisms that make the topsoil biologically active. These organisms are important to critical soil processes such as decomposition of organic matter and rendering nitrogen and other nutrients into plant-available forms. The alluvial sediments that will be stockpiled at Copper Flat for use in reclamation are unlikely to contain sufficient organic matter, nutrients and biological activity to support reclamation at the time of stockpiling but they are likely to contain adequate fine grained sediments (i.e., silts and clay) to provide water holding capacity when used as growth media (BLM DEIS page 3-41). As such, the discussion of “topsoil” in the context of salvaging materials for use in reclamation of the Copper Flat site reflects the nature of the materials available to stockpile. It is, therefore, more appropriate to refer to the materials utilized as cover material for reclamation and closure of the site as “topdressing” or “growth media.” Additional information regarding the cover growth material requirements for reclamation is provided in Section 3.1 and 5.5 of Appendix E.

4.5.1 Topdressing Suitability – 19.10.603.E.(1)

The suitability of topdressing/cover materials is based on the material's ability to provide erosion control, sustain vegetation, and reduce net infiltration. In general, soils and underlying colluvial and alluvial materials in the permit area are considered suitable and have no chemical limitations for growth of native and adapted reclamation species.

The NMED Copper Rules, 20.6.7.33.F.(2) NMAC, require that the proposed soil cover system be designed to limit net-percolation by having the capacity to store at least 95 percent of the long-term average winter (December, January and February) precipitation or at least 35% of the long-term average summer (June, July and August) precipitation, whichever is greater as determined by utilizing field or laboratory test results or published estimates of available water capacity. The suitability of topdressing cover materials to meet this standard is discussed in detail in Section 5.5.1 of Appendix E.



The available water capacity (AWC) for the salvageable growth media within the limits of the TSF and WRSP-2 and -3 were estimated utilizing the laboratory results of soils samples taken during Golder’s Supplemental Soils Investigation at Copper Flat (see Subsection 3.3.1 and Table 3, Golder, 2013). These estimates show an average AWC of approximately 0.9 inches of water per 1 foot of soil for the salvageable growth media within the footprint of WRSP-2 and -3, with a range of between 0.6 and 1.3 inches of water per 1 foot of soil. The AWC estimates for the salvageable growth media within the footprint of the TSF show an average AWC of approximately 1.2 inches of water per 1 foot of soil, with a range of between 0.4 and 2.2 inches of water per 1 foot of soil.

While the actual water retention of the salvaged soils will vary based on the types of soil materials that are placed in the GMSPs, the range of materials identified as suitable cover at the site indicates that the proposed cover system at Copper Flat will meet the storage requirements of the NMED Copper Rules.

4.5.2 Topdressing Salvage – 19.10.603.E.(2)

NMCC will salvage as much material as can be safely and practicably recovered and safely stored in the planned stockpiles. As part of the proposed operations, NMCC will bulk salvage suitable soils and near-surface alluvial materials from within the TSF, WRSP-2 and WRSP-3 footprints. Topdressing materials will be carefully recovered and stockpiled during the preproduction phase of the Project. Surficial soil materials will be salvaged in association with the construction of the plant, pipeline corridor, access roads and ancillary facilities. The salvaged growth media in these locations will be windrowed for local redistribution during final reclamation of the site. Suitable soils and other suitable cover materials including unconsolidated subgrade materials, colluvium and overburden will be salvaged to meet the volumetric requirements for final cover construction at closure as discussed in Section 3.1 and 5.5.2 of Appendix E.

4.5.3 Topdressing Stockpiling - 19.10.603.E.(3)

Salvaged reclamation cover materials will be stored in the three GMSPs shown in Figure 2-2. The GMSPs are located so as not to be disturbed or impacted by mining operations. The surfaces of the stockpile will be shaped after construction with overall slopes of 3H:1V or shallower to minimize soil loss. To further minimize erosion and the establishment of undesirable weeds, the GMSPs will be seeded with the interim seed mix. Additional information regarding stockpiling of growth media is contained in Section 5.5.3 of Appendix E.

4.5.4 Topdressing Re-Distribution - 19.10.603.E.(4)

Topdressing will be distributed in a manner to establish and maintain vegetation. Details regarding the manner in which it will be distributed and applied on regraded areas during for reclamation are discussed in Section 3.1, Growth Media Placement, of Appendix E.



4.5.5 Topdressing Stabilization - 19.10.603.E.(5)

Cover materials will be stabilized after distribution. Seedbed preparation, including scarification and disking along the contour, will be performed prior to seeding and mulching operations as described in the Section 3.0 of Appendix E.

4.5.6 Topdressing Amendment - 19.10.603.E.(6)

As discussed in Section 5.5.1 of Appendix E, most semi-arid native plants have adapted to low to moderate soil fertility conditions and are relatively unresponsive to increased soil fertility compared to crop plants. Fertilizer additions have been shown to have negative impacts in reclamation including increases in weedy annuals, shifts in species composition and decreases in drought, disease and pest resistance. Topdressing amendment requirements are discussed in Section 5.5.6 of Appendix E.

4.6 Erosion Control – 19.10.6.603.F

Reclamation activities described in Sections 3.0 and 5.6 of Appendix E will stabilize disturbed areas to a condition that protects against erosion. All disturbed areas will be regraded and shaped to a final contour that achieves positive drainage, reconstructs slopes with lengths and gradients that will provide long-term stability and seeded and mulched to establish a vegetative cover. Storm water will be diverted away from facilities. Drainage channels will be designed to regulate the velocity of water and minimize the potential for channel erosion. BMPs for storm water diversions, drainage and other water conveyance channels may include lining the channel with rock, riprap, vegetation or other geotechnical materials.

4.7 Revegetation – 19.10.6.603.G

As discussed in Section 2.5 of this updated MORP, NMCC's Copper Flat Project will take place on a combination of federal land administered by the BLM and private land owned by NMCC (by virtue of patented mining claims). The current land uses of federal lands administered by the BLM in the area of the Copper Flat facility have been identified previously in BLM's 1986 White Sands Resource Management Plan (BLM 1986); e.g., grazing, wildlife habitat, recreation and mining.

Revegetation of the site will be consistent with the requirements of 19.10.6.603.G.(3) NMAC; i.e., the previously accepted historic post-mining land uses as identified by the BLM in its land management plan consistent with the surrounding land uses of the Copper Flat site. The Reclamation and Closure Plan is designed to re-establish grazing in the area and allow for long-term use of the reclaimed areas by wildlife known to historically use the area without affecting



the potential for other uses such as mining and recreation. Revegetation success is discussed in Section 5.7 of Appendix E.

4.8 Perpetual Care – 19.10.6.603.H

The Copper Flat facility will be reclaimed in conformance with the Reclamation and Closure Plan, as approved. The Plan is designed to meet all of the applicable environmental requirements of the Act, 19.10.6 NMAC, the NMED Copper Rules and other laws following closure. As indicated above, NMCC will reclaim the disturbed areas consistent with the BLM's land management plan as currently approved. The lands surrounding the site are currently self-sustaining and do not require perpetual care. After the lands disturbed by NMCC's mining activities are reclaimed, the land will return to being self-sustaining requiring no perpetual care following closure.



5.0 REFERENCES

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Appendix A

Feasibility Level Design, 30,000 TPD Tailings Storage Facility
And
Tailings Distribution and Water Reclaim Systems

Copper Flat Project

Sierra County, New Mexico

Golder Associates Inc.

Revised, June 2016

Appendix A is contained in a separate volume binder, submitted with this Mine Operating and Reclamation Plan and the Copper Flat Mine Discharge Permit Application, dated December 2015, and revised June 2016.

Appendix B

Impoundment Design Report

M3 Engineering & Technology Corp.

November, 2015

Appendix B is contained in a separate volume binder, submitted with this Mine Operating and Reclamation Plan and the Copper Flat Mine Discharge Permit Application, dated December 2015.

Appendix C

Copper Flat

Process Facility Containment Report

M3 Engineering & Technology Corporation

December, 2015

Appendix C is contained in a separate volume binder, submitted with this Mine Operating and Reclamation Plan and the Copper Flat Mine Discharge Permit Application, dated December 2015.

Appendix D

Copper Flat

Site Diversion Analysis

M3 Engineering & Technology Corporation

Revised, June 2016

Appendix D is contained in a separate volume binder, submitted with this Mine Operating and Reclamation Plan and the Copper Flat Mine Discharge Permit Application, dated December 2015, and revised June 2016.

Appendix E

Mine Reclamation and Closure Plan

Copper Flat Mine

Golder Associates Inc.

October, 2016



FEASIBILITY LEVEL DESIGN REPORT

FEASIBILITY LEVEL DESIGN, 30,000 TPD TAILINGS STORAGE FACILITY

COPPER FLAT PROJECT
SIERRA COUNTY, NEW MEXICO

Submitted To: New Mexico Copper Corporation
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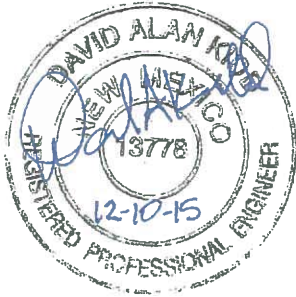
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November 30, 2015



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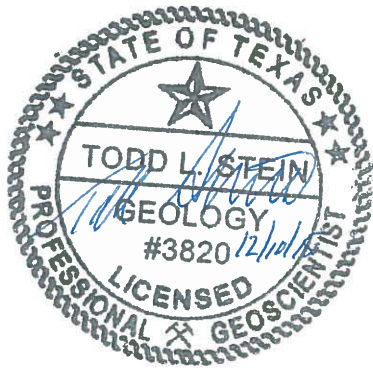
This report documents the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The design included herein was developed at a level consistent for agency review. Development of this report and associated TSF design was conducted under the oversight of the following Golder staff:



David A. Kidd, PE
(Feasibility Design Drawings and Associated Engineering Calculations)

12-10-2015

Date



Todd Stein, PG
(Preparation of Feasibility Level Design,
30,000 TPD Tailings Storage Facility Report)

12-10-15

Date



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List of Acronyms

1D	one dimensional
3D	three dimensional
ASTM	American Society of Testing and Materials
cm/sec	centimeters per second
F	Fahrenheit
FEMA	Federal Emergency Management Agency
FOS	factor of safety
ft-bgs	feet below the ground surface
ft-msl	feet above mean sea level
gpm	gallon per minute
HDPE	high density polyethylene
HSA	hollow stem auger
kg	kilograms
LCRS	leakage collection and recover system
MAG	Mineral Advisory Group
MDE	Maximum Design Earthquake
mm	millimeter
NMCC	New Mexico Copper Corporation
NMDSB	Dam Safety Bureau
NMED	New Mexico Environment Department
NOAA	National Oceanic and Atmospheric Administration
OSE	New Mexico Office of the State Engineer
PCPE	polyethylene pipe
pcf	pounds per cubic foot
PGA	peak ground acceleration
PI	plasticity index
PMP	probable maximum precipitation
PSD	particle size distribution curve
psi	pound per square inch
SPT	standard penetration test
TPD	tons per day
TPH	tons per hour
TPY	tons per year
TSF	tailing storage facility
USCS	Universal Soil Classification System
wt%	percent by total weight
yd ³	cubic yard



1.0 INTRODUCTION

1.1 Scope

Golder Associates Inc. (Golder) has been contracted by the New Mexico Copper Corporation (NMCC) and its parent company THEMAC Resources Group Ltd (THEMAC), to complete the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The TSF design presented herein has been completed in support of an overall Copper Flat project feasibility study as well as to support the various regulatory processes leading to permit approval of NMCC's project.

The TSF feasibility study report addresses geotechnical aspects of the project and presents the feasibility-level design of the TSF and tailings distribution and water reclaim systems. The individual components of the TSF feasibility study include: (1) the TSF design; (2) whole tailings delivery and distribution systems from the process plant and cyclone plant areas; (3) tailings delivery systems on the TSF; (4) underdrain collection system beneath the TSF; (5) TSF underdrain collection pond designs; (6) surge pond designs; (7) tailings reclaim water collection and delivery systems; and (8) systems for handling potential upset flow conditions (including secondary containment).

The location of the proposed facility is shown on Drawing 1 (Appendix J). Copper Flat is a proposed porphyry copper mining operation at a property that was briefly operated by Quintana Resources in 1981 and 1982. During the former mining operation, open pit pre-stripping was completed and a TSF with a design capacity of approximately 60 million tons was constructed and operated. Shortly after mining operations started, the mine was closed due to adverse economic conditions and depressed copper prices. In Drawing 2, which shows existing site conditions, the remains of the starter dam and splitter dike from the Quintana mining operation can be seen.

A new TSF will be constructed at Copper Flat in the same location as the former (old) Quintana Resources facility. The new TSF will extend approximately 1,000 feet to the east of the old starter dam (the tailings expansion area) as shown on Drawing 2.

A centerline construction method using cycloned tailings sand (cyclone underflow) for tailings dam construction will be utilized. A starter dam will be constructed using borrow material to provide initial storage capacity and to provide a location for initial discharge of tailings. The centerline approach allows construction of a stable, drained tailings dam using the cyclone underflow (i.e., tailings sand), while reducing the quantity of fill material required for dam construction. Tailings slimes (cyclone overflow) will be discharged into the interior of the TSF impoundment. The use of sand tailings for dam construction are such that the cyclone plant will be operated continually to produce the construction material.



The new TSF design will comply with the design and dam-safety guidelines and regulations of the New Mexico Office of the State Engineer (OSE) Dam Safety Bureau (NMDSB, 2010). Stormwater that cannot be diverted will be accommodated inside the impoundment by maintaining a dam crest elevation that provides adequate freeboard for containment of direct precipitation and run on.

The Mining and Environmental Compliance Section of the Ground Water Quality Bureau of the New Mexico Environment Department (NMED) will be the permitting authority for the groundwater discharge permit. NMED has provided guidance on anticipated design requirements for the TSF's liner system, which have been incorporated in this feasibility level design. Golder has also provided the feasibility level design of the tailings distribution and water reclaim systems.

Design drawings (Appendix J) to be read in conjunction with this report include the following:

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1.2 Terminology/Definitions

In this report, the TSF is described as consisting of the impoundment and the tailings embankment or dam. The impoundment refers to the interior tailings storage area located upstream of the embankment or dam. Tailings with different gradations will be placed inside the impoundment and on the dam.



Whole tailings refer to the process tailings delivered to the cyclone plant from the flotation plant outlet. At the cyclone plant, the tailings will be separated into cyclone underflow and cyclone overflow. The cyclone underflow, which consists primarily of the coarse sand fraction of the tailings, will be used to construct the dam. The cyclone overflow, which consists primarily of the fine fraction of the tailings, will be placed in the impoundment. Each of these tailings products will be transported and discharged as a slurry, with varying concentrations of tailings solids suspended in process water.

The cyclone overflow discharged into the impoundment will form a surface that gently slopes away from the point of discharge. The beach refers to the area near the point of discharge where the coarsest particles in the cyclone overflow will tend to settle. The slimes refer to the finer fraction of the cyclone overflow, which will flow down the beach with the majority of the process water to the distal portion of the impoundment surface and settle in the vicinity of the free water pond.

The TSF will have two separate underdrain systems. The dam underdrain will underlie the dam and overlie the dam liner and collect drainage from the cyclone underflow. The impoundment underdrain will overlie the impoundment liner and will collect drainage from the tailings beach and slimes (cyclone overflow).



2.0 SITE DESCRIPTION

2.1 Topography

Existing surface conditions at Copper Flat are shown on Drawing 2. The starter dam, splitter dike, and approximately 1.2 million tons of tailings from the Quintana Resources operation remain on the property.

The TSF site consists of a broad, shallow basin located at the head of a natural drainage that discharges to Grayback Arroyo. Elevation ranges from 5,170 to 5,435 feet above mean sea level (ft-msl) within the proposed TSF footprint. Topography is gently sloping over most of the site with the steepest slopes located around the west and southwest periphery of the facility. Because the site is near the head of a natural basin, requirements to divert water from upstream catchment areas will be minimal. Surface water diversion is discussed in Section 6.6.

2.2 Climate

The property is located within an arid, high desert area in the Basin and Range physiographic province subject to hot summers and relatively mild winters. Maximum summer temperatures can exceed 100 degrees Fahrenheit (F) while the average maximum daily temperature during winter months is approximately 40 degrees F. Average annual rainfall is approximately 13 inches and the property receives snow periodically.

Most rainfall occurs in July through September and is associated with high intensity, short duration, convective storms and moisture from the Gulf of Mexico. Winter precipitation is associated with west to east moving Pacific frontal storms. These storms typically produce less intense precipitation over a longer duration.

Based on the National Oceanic Atmospheric Administration Atlas 14 (NOAA, 2006), the 100-year, 24-hour storm event is estimated to be approximately 3.73 inches. Hydrometeorological Report 55A (US Department of Commerce, 1988), which provides probable maximum precipitation (PMP) estimates for areas located between the continental divide and the 103rd meridian, indicates a 72-hour PMP depth of 26 inches.

2.3 TSF Area Subsurface Conditions

2.3.1 Geology

The proposed TSF site is located in Sierra County, within the southern Basin and Range physiographic province (Parsons, 1995). The Basin and Range province is described as a broad, highly extended terrain that extends from Canada, through the western United States, and across much of Mexico (Parsons, 1995). The name is derived from the type of extensional block-faulting that left the characteristic pattern of alternating basins and ranges across the province.



The site lies near the eastern edge of the Black Range, on the Piedmont slopes of the Palomas Basin. The Black Range is a Late Cretaceous to early Tertiary (Laramide) volcanic-plutonic arc (Oniell et al, 2002). The Basin and Range province in this region is cut by the Rio Grande Rift zone. The property sits in the westernmost basin of the Rio Grande rift zone, which is made up of three parallel north-trending basins separated by intra-rift horsts (Chapin, 1971).

The Palomas Valley is a relatively narrow flood plain flanked on the east by the piedmont slopes of the Caballo Mountains and on the west by the long gentle slopes ascending to the base of the Hillsboro-Animas Hills (Hawley, 1965).

Detrital material derived from the erosion of the adjacent slopes fills the Rio Grande valley to a depth of up to 9,000 feet (Harley, 1934). Much of this material is gravel, which consists of boulders and pebbles of quartzite, limestone, granite, rhyolite, andesite, and basalt, derived from the surrounding rock complex. They are coarser along the valley sides, grading to finer material toward the middle of the basins. In the higher regions, as in the sides of the valley between Fairview and Hillsboro and at Hermosa and elsewhere, alluvial deposits of Quaternary age are comprised of boulders and pebbles in a finer matrix of the same material, loosely cemented into a firm hard conglomerate known as Palomas gravel (Harley, 1934). Younger sand and gravel deposits, which form some of the intermediate terraces, represent a filling of channels eroded in the older deposits of Santa Fe and Palomas age (Harley, 1934).

The principal fan in the district is that formed by the drainage toward the east out of Copper Flat, principally through Grayback Arroyo, but in part through Dutch Gulch. The basal part of the original fan is composed of fine to coarse fragments of rhyolite derived from the late flows that once covered the Animas Hills. The intermediate part of the fan is composed principally of andesite and latite fragments. The topmost portion of the fan is composed of basalt, andesite, and latite fragments (Harley, 1934).

The Animas Hills consists predominantly of andesite flows and breccias of Late Cretaceous age with minor interbeds of sandstone. The andesites and sandstone are intruded by a stock of quartz monzonite of Late Cretaceous age centered at Copper Flat, and by quartz latite dikes radial to the stock (Seegerstrom et al, 1975).

Upper Cenozoic basin fill in the Rio Grande rift is generally referred to as the Santa Fe Formation or Group. The Santa Fe Group (middle Pleistocene to uppermost Miocene), is comprised of multiple formations including the Camp Rice, Fort Hancock, Palomas, Sierra Ladrones, Arroyo Ojito, Ancho, Puye, and Alamosa Formations.

The Santa Fe consists of basal conglomerate and interbedded sand and clay beds. The cobbles and boulders are mainly andesite. Thickness is variable but generally in tens of feet in the subject area. Two facies in the study area include: 1) the piedmont facies consisting of brown, poorly sorted, weakly



stratified conglomerates and fan gravels and brown sandy silt, conglomerate and clay; and 2) the axial-river facies of cross-stratified sandstone, sand, pebble conglomerate, gravel, and clay lenses. The piedmont facies comprises the major portion of the Palomas Formation. It rests in angular unconformity on bedrock of varying lithologies along the major uplifts bordering the basins. However, in many central basin areas the piedmont facies appears to grade downward into coarse-grained deposits of the lower Santa Fe Group (Lozinsky, 1986).

2.3.2 Site Observations

The site geotechnical investigation program details are shown on Drawing 3. Drawings 4 through 9 contain TSF area geologic cross-sections developed on the basis of the recent drill holes and test pits, supplemented with subsurface information reported by Sergeant, Hauskins, and Beckwith (SHB, 1980).

Drill hole logs indicate that the foundation in the tailings area consists primarily of alluvial deposits that includes silt, sand and gravel underlain by clay. In the northwestern waste rock stockpile area, borings indicated the presence of gravelly silts and sands overlying conglomerate consisting primarily of andesite. The conglomerate is underlain by unweathered andesite.

Silts, sands, and gravels that occur in the proposed TSF area have been identified as piedmont alluvium and the older deposits of the Santa Fe Group (SHB, 1980) on which the piedmont alluvium was deposited. The Santa Fe Group is reported to consist of interfingering alluvial fan (gravel) and clay facies. Basalt flows are reported to occur in channels and arroyos cut into the piedmont and Santa Fe sediments. Basalt outcrops have been identified in an arroyo in the center of the TSF and locally around the site.

Drilling logs and geologic cross-sections indicate a high degree of variability in near surface materials both vertically and laterally, within the impoundment area and beneath the proposed dam. Silty/clayey horizons alternating with gravelly sand layers could potentially represent either the interfingering of the Santa Fe Group facies, or the more recent effects of local erosion and deposition.

In general, the interior of the impoundment is underlain by silty, clayey and gravelly sand, and cemented gravelly sand with a near surface layer of silty, wind-blown material. Eastward, toward the future dam site, interbedded clays and silts occur at depths typically greater than 20 feet; however, the composition of the foundation remains highly variable with interfingering, silty, sandy and clayey gravel units.

Groundwater is typically encountered at depths greater than 50 feet below ground surface (ft-bgs) in the vicinity of the TSF. A small zone of perched water has been identified in the vicinity of the old Quintana dam; however, recent drill holes completed in the perched water area to a depth of 50 ft-bgs did not intercept water. Groundwater and local perched water are not anticipated to impact the design and operation of the TSF.



3.0 SITE INVESTIGATIONS

3.1 Previous Site Studies

Portions of the TSF site were investigated by Sergeant, Hauskins, and Beckwith (SHB, 1980) prior to construction of the Quintana Resources TSF. The SHB investigation focused on the dam alignment for a 60 million ton facility, which does not coincide with the alignment currently proposed for the over 1 million ton facility. SHB coverage of the potential borrow areas in the impoundment interior was relatively extensive.

3.2 Site Geotechnical Investigation Program Description

Golder conducted a site geotechnical investigation between December 2012 and January 2013 to expand the coverage of the former SHB site investigation to include the new dam alignment. The field investigation consisted of 31 test pit excavations and 28 drill holes. Drawing 3 illustrates test pit and drill hole locations, and the location of geologic cross-sections developed to show the geology of the dam foundation.

Test pits were excavated with a Case CX210B or Terex 7606 hydraulic backhoe to depths up to 20 feet. Test pits were logged and photographed. Soil samples collected from test pits included bulk 5-gallon bucket and bag grab samples. Bulk samples are suitable for geotechnical soil classification, strength, consolidation, compaction and permeability testing. Bag samples are suitable for soil classification and moisture content testing. Test pit logs are contained in Appendix A.1.

Most of the drilling was completed with a track mounted CME 75 drill rig using hollow stem augers (HSA). The rig was equipped for conversion to diamond core drilling if bedrock was encountered. The drill hole target depth was 50 ft-bgs. Due to the presence of cemented gravels that were not amenable to drilling by either HSA or diamond core methods, down-hole percussion (Tubex) equipment was also used.

Standard penetration testing (SPT) was carried out at 5-foot intervals during HSA drilling. When percussion drilling was required, SPT test frequency was reduced to about 10 feet. Samples collected during drilling included bulk 5-gallon bucket cuttings samples from auger drilling, bagged Tubex rig cuttings samples, and bagged samples recovered from the SPT split-spoon sampler. Drill hole logs are contained in Appendix A.2.

3.3 Geotechnical Testing

Bulk and bag samples collected during site exploration were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for soil classification, compaction, permeability, consolidation, and strength testing. Laboratory reports for the various tests are contained in Appendix A.3. Soils were classified according to the Universal Soil Classification System (USCS). All geotechnical tests were completed in



accordance with applicable American Society of Testing and Materials (ASTM) standards. No undisturbed samples were collected during the site exploration. Geotechnical strength, consolidation, and permeability tests were conducted on remolded samples.

3.3.1 Soil Classification

3.3.1.1 Tailings Impoundment Interior

Within the interior, central and western portions of the impoundment area, site soils consist predominantly of clayey sand with gravel (SC), well-graded silty sand with gravel (SW-SM) with lesser clayey and well-graded silty gravel (GC, GW-GM). Silty and clayey soils (CL-ML) also occur locally. The fine fraction (finer than 75 microns or the material passing the No. 200 standard sieve) in sandy samples from the impoundment interior ranges from 8 to 29 percent and averages 20 percent. Plasticity indices (PI) average 17 percent. The specific gravity of interior area soils ranged from 2.67 to 2.75.

Four composite samples of near surface sandy, gravelly soils were prepared to evaluate materials potentially available for the construction of processed (crushed and screened) drainage material. Each composite sample was composed of two to three 5-gallon bucket samples of near surface materials. Composite samples were initially crushed to 100 percent finer than 1 inch to simulate material that would be suitable for placement against a geomembrane liner. Three of the four composites were classified as clayey sand (SC), while the other was classified as well graded silty gravel (GW-GM). The fine fraction in the crushed composite samples ranged from 8 to 20 percent and the average PI was 17 percent. It is anticipated that screening will be required to reduce the fines content of this material if it is to be used for highly permeable, manufactured drain fill adjacent to critical drainage pipes.

3.3.1.2 North Cell Tailings

The north cell of the old Quintana TSF contains tailings from mining conducted in the early 1980s. Old tailings samples were classified as silty sand (SM) and low plasticity silt (ML). The sample classified as sand was located adjacent to the old dam and presumably, near the point of discharge. It contained a minus 75-micron fine fraction of 49 percent and was non plastic. The silty sample was obtained from the center of the north cell and had a minus 75-micron fraction of 71 percent and a PI of 5 percent. Differences in old tailings properties are presumed to be the result of tailings segregation on the former tailings beach. Moisture content in the old tailings samples ranged from 6.0 to 11.3 percent.

3.3.1.3 Tailings Dam Footprint

Soils encountered in the footprint of the proposed dam are highly variable. Clayey sand and gravel (SC,GC) generally occur at shallow depth with interbedded high and low plasticity clays (CH,CL) and silts (ML,MH) occurring at depths below 20 ft-bgs. Clay intercepts indicate that the clay occurs in



discontinuous lenses or in eastward dipping strata. The fine fraction in the sandy and gravelly soils ranges from 17 to 39 percent and the PI averages 17 percent.

3.3.2 Foundation Strength Testing

Foundation samples were subjected to consolidated undrained triaxial testing with pore pressure measurement to determine effective shear strength for use in supporting stability analyses. Confining pressures were selected to represent anticipated foundation pressures associated with tailings embankment construction. Granular foundation materials are assumed to be cohesionless. Strength data reported below represent the effective internal friction angle. Triaxial test reports are contained in Appendix A.3.

It is anticipated that the old tailings will be placed beneath the new TSF geomembrane liner as a liner bedding fill layer. A 12-inch by 12-inch direct shear test was conducted to evaluate the interface friction at the TSF liner interface. The direct shear set-up included a layer of compacted old tailings from Test Pit 10 in the center of the north cell, a textured 80-mil high density polyethylene (HDPE) geomembrane, and a layer of material from the drainage composite samples 1 through 4. Tailings underlying the liner were placed at optimum moisture content based on ASTM D698 compaction testing and maintained in an unsaturated state during shearing. The over-liner drainage materials were wetted prior to testing but maintained in an unsaturated state during the test. The test was conducted in a manner that allows the failure through the lower strength interface (i.e., liner against old tailings or liner against drain fill). Direct shear test results are contained in Appendix A.3.

Table 1 summarizes the strength test samples, test objectives and strength test results.

Table 1: Summary of Strength Test Results

Sample	Material, Objective, Test Method	Internal Friction Angle
TP-10-3-13	Old tailings, strength of liner bedding triaxial CU ⁽¹⁾	34 degrees
TP-10-3-13	Old tailings, liner interface shear strength, direct shear ⁽²⁾	26.5 degrees
Composite 1-4	TSF Interior borrow for structural fill and drain material, triaxial CU	40 degrees
BH-16-0-8.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	28 degrees ⁽³⁾
BH-25-0-12.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	43.5 degrees
BH-10-0-14.5	Clayey sand, structural fill and embankment foundation strength, triaxial CU	32 degrees

Notes:

⁽¹⁾ CU = consolidated, undrained

⁽²⁾ Liner interface testing included old tailings and drain material in contact with textured, 80-mil HDPE geomembrane.

⁽³⁾ Sample was 91 percent finer than ¾ inch. Represents a matrix shear strength and does not account for interlocking of the coarse fraction.

No undisturbed samples were collected during site exploration. Tests were conducted on remolded samples.



3.3.3 Drainage Material Permeability Testing

Drainage material permeability testing was performed to support the design of the impoundment and dam underdrains. Testing was completed on composite samples 1 through 4, which represent the near surface materials that will be available for preparation of drainage material. Equal volumes of composite samples 1 through 4 were blended to prepare a sample for permeability testing. Samples were tested in a 10-inch diameter rigid wall cell with a 150-pound-per-square-inch (psi) load applied to simulate loading under field conditions. Permeability test reports are contained in Appendix A.3.

Two drainage materials are considered in the TSF design. These include crushed and screened materials with reduced fines content for the dam underdrain and primary drains in the impoundment interior (primary drain fill), and material obtained from selective borrowing of sandy soils within the TSF footprint (select native drain fill). Processing of the select native drain fill is anticipated to be limited to crushing to reduce the maximum particle size. No washing to reduce the fines content is anticipated to be required to produce suitable drainage materials.

3.3.3.1 Primary Drain Fill

A primary drain fill sample was prepared for testing the performance of the dam underdrain. The minus No. 40 standard sieve fraction of the sample was removed to simulate a prepared drainage material with low fines content that is filter compatible with the cyclone underflow sand. The hydraulic conductivity of the primary drain fill sample was 9.1×10^{-2} centimeters per second (cm/sec).

3.3.3.2 Select Native Drain Fill

The composite sample was tested without additional modification to estimate the permeability of the sandy site soils. As tested, the minus 75-micron fraction in the composite samples was 9 percent. The hydraulic conductivity of the select native drain fill sample was 3.8×10^{-5} cm/sec.

3.3.4 Foundation Sample Consolidation Testing

Selected foundation samples were subjected to conventional one-dimensional consolidation testing to support estimation of settlement potential in the proposed dam foundation. Samples were selected to evaluate silty and clayey horizons where changes in loading conditions could result in additional consolidation. Samples were remolded to dry densities ranging from 88 to 95 pounds per cubic foot (pcf) at natural moisture content to reflect in-situ density estimated from standard penetration tests. Foundation sample consolidation test reports are contained in Appendix A.3. Foundation settlement potential is discussed in Section 11.0.



4.0 TAILINGS TESTING

4.1 Program Description

The TSF embankment proposed in this feasibility study is to be constructed by the centerline raise method using cyclone underflow. Whole tailings from the flotation plant will be routed to a cyclone plant where the tailings will be separated into underflow (sand) and overflow (slimes) fractions. The cyclone underflow will be routed to the dam centerline and used to construct the tailings dam. The overflow will be discharged into the impoundment interior.

Primary considerations for effective centerline sand dam construction include adequate drainage and compaction of the cyclone underflow sand. Drainage requirements are typically met by:

- Producing a relatively free draining sand material. This is usually achieved when the minus 75-micron fraction of the underflow does not exceed 20 percent by weight.
- Having the hydraulic conductivity of the material placed in the dam two orders of magnitude greater than the material placed in the impoundment.

These two conditions generally result in a well-drained structure. A fixed cyclone station maintains optimum and consistent conditions at all cyclones and facilitates meeting gradation objectives. A blanket drain will be placed beneath the embankment to facilitate collection of cyclone sand drainage and minimize saturated conditions at the base of the embankment.

Industry experience at operating mines utilizing cyclone sand for dam construction indicates that hydraulic placement and self-weight compaction is generally sufficient to minimize liquefaction potential in cyclone sand dams located in regions with low seismic risk, such as at Copper Flat. Where it is required, compaction to a relative density of 60 percent (equivalent to approximately 90 percent of ASTM D698 maximum dry density) will result in low potential for liquefaction under static and seismic loading conditions (CANMET, 1977). At Copper Flat, tailings placed on the dam crest will be compacted. Some compaction of tailings placed on the dam out-slope will occur as a result of dozer spreading operations.

An initial cyclone test was conducted on a 55-kilogram (kg) tailings sample to determine the quantity and quality of sand available for dam construction, and produce samples of future tailings products for geotechnical testing. The initial cyclone test sample was produced by Metcon Research in September 2011. The sample was run through a 4-inch cyclone at the FL Smidth-Krebs (Krebs) facility in Tucson, Arizona. On the basis of the initial 4-inch cyclone test, Krebs, utilizing proprietary software, predicted that a gMAX15U-20 (15-inch) cyclone could recover 46 percent of the whole tailings stream with a minus 75-micron fraction of less than 20 percent. The quantity and quality of the cyclone underflow predicted on the basis of the 4-inch cyclone test were consistent with industry guidelines for sand dam construction. The quantity of sand recovered met anticipated construction requirements.



An additional pilot scale metallurgical study was conducted by the Mineral Advisory Group (MAG). Approximately 255 kg of tailings solids (whole tailings) were provided by MAG in five sealed 55-gallon steel drums. Drums were delivered to Krebs in July 2012 and the tailings were passed through a gMAX15U-20 cyclone in the Krebs laboratory on August 4, 2012. The sand recovery was 41 percent and the minus 75-micron fraction was under 16 percent. Cyclone underflow, and cyclone overflow produced during the test, and residual whole tailings were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for testing.

Appendix B.1 contains Krebs analysis of the August 2012 cyclone run and the simulation (prediction) of full field scale cyclone performance. The cyclone plant is estimated to have a recovery of 45 percent with a minus 75-micron fraction of 18 percent.

4.2 Tailings Test Program Description

Table 2 contains a test matrix for the tailings products produced during the cyclone test. Gradation analyses were completed on the cyclone underflow, cyclone overflow and whole tailings. The cyclone overflow and whole tailings were subjected to flume testing to simulate discharge into the impoundment. Flume testing involves the discharge of tailings slurry at the field anticipated solids content into a 12-inch flume at low velocity. The tailings flow down and settle in the flume. Samples are collected at various flume locations to evaluate changes in gradation and solids content. Gradation and slurry consolidation tests were conducted on samples from the head and tail of the flume to evaluate the characteristics of tailings found on the beach and in the interior of the impoundment (slimes). Laboratory data sheets for tailings tests are contained in Appendix B.

Table 2: Test Matrix for Cyclone Underflow, Cyclone Overflow and Whole Tailings

Test Method	Underflow	Overflow	Whole Tailings
Gradation and Atterburg Limits (ASTM D4221, D4318)	1	2 ⁽¹⁾	2 ⁽²⁾
Specific Gravity (ASTM D854)			1
Compaction Test (ASTM D-698)	1		
One-Dimensional Consolidation (ASTM D2435)	1		
Flume Test (flumes samples taken from head and tail of flume to evaluate segregation and settling) (specialty test)		1	1
Column Settling Test (single drain) (specialty test)		1	1
Slurry Consolidation Test (specialty test)		2 ⁽¹⁾	2 ⁽²⁾
Permeability (ASTM D2434)	1		
Triaxial Shear Strength, Consolidated-Undrained (ASTM D2850)	1	1	

Notes:

- (1) Testing completed on head and tail section samples from the flume to simulate properties of the beach and slimes fraction of the cyclone overflow.
- (2) Testing completed on head and tail section samples from flume to simulate properties of the beach and slimes fraction of the whole tailings.



4.3 Test Results

4.3.1 Material Classification

Table 3 summarizes the characteristics of the tailings prior to and following cyclone separation. Gradation test results for the whole tailings cyclone feed, cyclone underflow and cyclone overflow are contained in Appendix B.2.

Table 3: Summary of Tailings Properties, Pre- and Post-Cyclone Separation

Material	P ₈₀ (microns) ¹	Minus 75-micron Fraction (percent) ²	USCS Classification
Whole Tailings (feed)	110	59	SM (silty sand)
Underflow (sand)	150	17.9	SM (silty sand)
Overflow (slimes)	5.7	90	ML (low plasticity silt)

Notes:

¹P₈₀ is the particle size for which 80 percent of the material is finer

²The minus 75-micron fraction is the percentage of clay and silt sized particles

Based on the gradation test results, the whole tailings sample produced in the MAG metallurgical study is slightly coarser than the flotation tailings that will be produced during operations (P₈₀=110 microns versus the design P₈₀ of 105 microns).

4.3.2 Cyclone Underflow Testing

Cyclone underflow sand will be delivered to the dam at a solids content of approximately 70 percent based on cyclone test results and cyclone performance predictions. Cyclone underflow sand discharged on the dam crest will be spread and compacted. Moisture density testing (ASTM D698) indicates a cyclone underflow maximum dry density of 97 pcf and an optimum moisture content of 16.8 percent. The moist weight of the compacted cyclone underflow will be on the order of 110 pcf. Underflow compaction test data are contained in Appendix B.2.

Cyclone underflow samples were subjected to one-dimensional consolidation and consolidated-undrained triaxial shear strength testing. A cyclone underflow sample compacted to within 5 percent of maximum dry density exhibited an effective internal friction angle of 40 degrees. Consolidation and triaxial test reports are contained in Appendix B.3.

4.3.3 Cyclone Overflow and Whole Tailings Testing

Flume tests were conducted on cyclone overflow and whole tailings in a 12-inch wide by 24-foot long test flume. The primary purpose of the flume tests is to provide data to support estimation of post deposition density. Samples were collected from the head and tail sections of the flume for gradation, settling and slurry consolidation testing. Test data were used to develop input parameters for one-dimension numerical consolidation modeling and impoundment filling rate studies. Flume test gradation, settling and



slurry consolidation test reports are contained in Appendix B.4. Tailings consolidation modeling is discussed in Section 5.0.

To support stability analyses, a sample of cyclone overflow collected from the head of the test flume (beach material) was subjected to consolidated-undrained triaxial shear strength testing. The sample was tested at a dry density of approximately 94 pcf and a moisture content of 27 percent. The measured effective internal friction angle was 37 degrees. The triaxial test report is contained in Appendix B.4.



5.0 TAILINGS CONSOLIDATION ANALYSES

5.1 Approach

Consolidation calculations were performed using the computer program FSConsol (GWP Software, 1999). FSConsol performs a one-dimensional (1D), large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967). For modeling purposes, the non-linear relationships used to express permeability and compressibility are those proposed by Abu-Hejleh and Znidarcic (1994 and 1996), and defined by Equations 5.1 and 5.2, which are used in the consolidation and desiccation numerical model.

$$k = C e^D \quad \text{Equation 5.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 5.2}$$

When using FSConsol model, Equation 5.1 remains the same; however, Equation 5.2 is rewritten by the modified power law form to represent compressibility, as shown in Equation 5.3.

$$e = A(\sigma')^B + M \quad \text{Equation 5.3}$$

Where:

e = void ratio of the tailings

σ' = the effective confining stress

k = the hydraulic conductivity of the tailings

A, B, M (or Z), C, and D are material parameters determined from laboratory slurry consolidation and column settling tests

Five material parameters, *A*, *B*, *C*, *D*, and *M*, were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 and 2. Fitted parameters are shown in Tables 4 and 5. Data are based on slurry consolidation testing of cyclone overflow samples derived from the head and tail sections of the test flume discussed in Section 4.3.3.

Table 4: Permeability Input Parameters for Cyclone Overflow

Sample	C (centimeters per second)	D (dimensionless) ¹
Slimes	1.380×10^{-7}	3.353
Overflow Beach	1.523×10^{-6}	3.035

Table 5: Compressibility Input Parameters for Cyclone Overflow

Sample	A (1/kilopascals) ^B	B (dimensionless) ¹	M (dimensionless) ¹
Slimes	3.144	-0.1952	-0.1424
Overflow Beach	1.787	-0.2983	0.5224

Note:

¹ B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second units



The tailings impoundment was modeled by running analyses on single tailings columns or volumes filled with materials representing either the beach or slimes components of cyclone overflow. Under the proposed mining plan, the operator will be required to maximize use of the cyclone plant to generate sufficient sand to construct the dam. Therefore, whole tailings discharge will have little impact on the filling rate of the TSF or the characteristics of tailings placed in the TSF.

In each model run, the modeled component is assumed to represent 100 percent of the inflow to the model storage volume. Models were run until the modeled storage volume reached capacity, or the entire mass of tailings was input. The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities (adjusted for errors as discussed in Section 5.3) from the final void ratio profile of each tailings component to obtain an overall dry density that is weighted based on laboratory grain size distribution mass balance results, as discussed in Section 5.2.

5.2 Tailings Beach versus Tailings Slimes Split

Grain size distribution tests were performed by the Golder soils laboratory in the Denver, Colorado, on beach and slimes samples from laboratory flume tests and on a cyclone overflow head sample discharged into the test flume. From the grain size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the beach and slimes that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the beach and slimes samples should equate to the percentage of sand and fines in the original cyclone overflow head sample. Table 6 depicts the results of the laboratory gradation tests and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes) by weight or mass.

Table 6: Laboratory Grain Size Distribution and Beach to Slimes Split Calculation

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids Recovered	% Sand recovered from overflow	% Fines recovered from overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head Sample	9.63	90.37	100	9.63	90.37

5.3 Consolidation Modeling Results

FSConsol model output and a detailed description of modeling procedures and interpretation are contained in Appendix C. If 100 percent of the inflow into the TSF impoundment is represented by the tail section flume sample (i.e., the finest fraction of the tailings), FSConsol predicts that the final dry density of the slimes fraction over the modeled profile will average approximately 33.5 pcf. With 100 percent of the



inflow representative of tailings overflow beach materials, FSConsol predicts a final average tailings dry density of 78.6 pcf

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall bowl shaped geometry of the TSF, and the fact that the mass and volume quantities are larger towards the top than at the bottom of the TSF. The average dry density of each tailings component is corrected based on the average of the calculated percent error between 1D checks of the height of solids, the volume of solids, and the mass of solids. Based on the TSF geometry, calculated material properties, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1 to 14 percent with an average error ranging between 2.7 and 5.4 percent. Based on Golder's experience, 1D analyses resulting in less than a 15 percent average error reasonably depict expected consolidation in the field; therefore, the three-dimensional (3D) impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error calculated for the following checks: height of solids, volume of solids, and mass of solids.

In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components. After accounting for 3D characteristics of the impoundment geometry and errors, the predicted dry density of the beach and slimes components are 74.6 and 31.7 pcf, respectively.

5.4 TSF Capacity

Based on model results and corrections based on error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow slimes and beach at the end of filling. The capacity of the TSF interior is 96.9 million cubic yards (yd³) with a crest elevation of 5,460 ft-msl and a maximum tailings surface elevation of 5,450 ft-msl. While the beach and slimes components are predicted to represent 60 and 40 percent, respectively, of the mass of cyclone overflow to be discharged into the TSF interior based on the mass balance as discussed in Section 5.2, these components will represent approximately 40 and 60 percent, respectively, of the storage volume. At the estimated densities predicted with FSConsol, the required storage capacity is 93.4 million yd³. This assumes near constant operation of the cyclone plant. With approximately 96.5 percent utilization of the cyclone plant, available storage capacity will equal required storage capacity.

Without consideration of managed deposition effects, there is a small excess in available storage volume. The estimated weighted dry density of tailings within the TSF impoundment is 48.4 pcf, which is low in comparison to copper industry experience. Managed deposition such as cycling discharge locations to promote desiccation and controlling the size of the free water pond can be expected to increase the post deposition dry density and reduce storage volume requirements. In addition, the FSConsol modeling runs for slimes inflow were run for a time period of 7.9 years. The slimes can reasonably be expected to



continue to consolidate and increase in dry density through the remainder of the 11.1-year mine life. Only a slight increase in the dry density of the slimes over the remainder of the mine life will result in an increase in storage capacity.

The inflow rate for cyclone overflow assumes near constant operation of the cyclone plant to produce sufficient sand to construct the dam to the elevation of 5,460 ft-msl. As such, the mill should not be operated unless the cyclone plant is operating, and maintenance of the cyclone plant should be performed concurrently with mill maintenance. The cyclone overflow distribution system has two operating legs that will facilitate near constant operation. During operations, filling rates and tailings post deposition dry density should be regularly monitored to evaluate consolidation characteristics. If the rate of consolidation is better than predicted, utilization of the cyclone plant could potentially be reduced, the final dam crest could be lowered, and a corresponding reduction in the amount of sand required to construct the dam could be realized.



6.0 FEASIBILITY LEVEL DESIGN

Table 6 summarizes key design criteria assumed in feasibility level design of the new TSF.

Table 6: Feasibility Study Design Criteria

Regional Design Factors	
Precipitation/ Evaporation	Based on NOAA weather data for Hillsboro and Caballo Dam, New Mexico
Design Storm Events	100 percent of the 72-hour general storm probable maximum precipitation (PMP), 26 inches
Stability FOS	Minimum 1.5 for static conditions and 1.1 for seismic loading conditions
Seismicity PGA	USGS MDE, 2475-year return period, 0.13 times gravitational acceleration (0.13g)
TSF Design Factors	
Storage Capacity	112 million tons (THEMAC)
Production/Delivery Schedule	1,333 tons per hour (TPH) net tailings to the TSF year 1-5, 1,222 TPH years 6 to 11.1, 125,000 tons per year (TPY) (THEMAC), post concentrate recovery
Mill utilization	92.5 percent (M3)
Operating Life	11.1 years (THEMAC)
Tailings Specific Gravity	2.64 (Golder test)
Tailings Solids Content (wt%)	29.2 percent solids by weight (whole tailings to cyclone plant). Tailings diluted in outlet sump as needed to optimize cyclone performance.
Production Rate	Varies, Net tailings to the TSF from 9,182 to 10,704 kiltons per year (25,156 to 29,326 tons per day) (NMCC)
Tailings post- deposition dry density	31.7 and 74.6 pounds per cubic foot (pcf) dry weight assumed for post-deposition cyclone overflow slimes and beach, respectively, 92 pcf dry weight for the cyclone underflow fraction. (Golder estimate)
Embankment Construction	Phase 1 earthen starter dam to an elevation of 5,250 ft-msl. Post Phase 1 peripheral earthen dam extension constructed to 10 feet above grade. Centerline raise construction using cyclone underflow sand. Cyclone underflow on dam crest compacted to minimum of 90 percent of American Society for Testing and Materials (ASTM) D698, relative density > 60 percent
Liner System	From bottom to top: Prepared foundation, 12-inch liner bedding fill, 80-mil HDPE geomembrane, overliner drainage collection layer with internal drainage pipe network beneath the tailings embankment and continuous beneath impoundment



TSF Design Factors (cont.)	
Earthworks Slopes (assumed)	Soil cut slopes = 1.5H:1V (1.5 horizontal to 1 vertical) Rock cut slopes = 1H:1V Fill slopes = 2H:1V Lined slopes = 3H:1V to 2.5H:1V max Embankment out-slope = 3H:1V nominal Starter Dam, 2.5H:1V inner, 2H:1V outer
Drainage/TSF Underdrain Collection Pond	Double-lined pond with LCRS to contain dam and impoundment under drainage and surface water runoff. Pond to be constructed as an OSE non-jurisdictional facility.
Collection Pond Reclaim	Submersible turbine pumps with 4,000-gallon per minute (gpm) capacity
Collection Pond Capacity	Normal inventory, 24 hours reserve capacity for underdrain for reclaim pump system upset, 100-year, 24-hour event (3.73 inches) stormwater storage capacity for runoff contributing areas
Tailings Management	Tailings routed through eighteen 15-inch cyclones at 83 TPH feed rate per cyclone. 45.2 to 45.6 percent underflow solids recovery (Krebs), 18.2 to 18.4 minus 200 fraction in underflow (Krebs). Cyclone overflow discharged from the dam crest into the impoundment interior.
Supernatant Reclaim	Floating barge with 12,978-gpm capacity
TSF Water Storage and Stormwater Diversion Design Factors	
Dam Safety Hazard Ranking	Significant, due to environment risks associated with a release of tailings (OSE)
TSF Pond Design Freeboard	As required to accommodate wave run-up and provide minimum freeboard for design storm
TSF Pond Required Stormwater Storage	Contain flows from 1.0 times the 72-hour PMP storm event plus normal inventory of supernatant water
Hydrology Runoff Curve Numbers	100 - Impounded tailings and lined areas 50 - Tailings embankment sand shell 92 - Native ground surfaces
Stormwater Diversion	Divert runoff from undeveloped areas inside ultimate footprint where feasible. Divert exterior area runoff where feasible.
Underdrain System	Continuous underdrain layer beneath dam and TSF interior. Collected water will be returned to the process via TSF underdrain collection pond reclaim pump system
TSF Water Pond Surface Area	40 percent of tailings impoundment interior or a maximum of 40 acres assumed for feasibility level water balance calculations
TSF Water Pond Surface Evaporation	75 percent of average Pan evaporation
Tailings Surface Evaporation	50 percent of average Pan evaporation

Notes:

FOS = factor of safety

MDE = maximum design earthquake

PGA = peak ground acceleration

HDPE = High density polyethylene



The following sections describe the various construction components of the TSF feasibility level design.

6.1 Earthworks

TSF construction activities will require borrowed structural fill for starter dam and toe berm construction, drain fill for constructing underdrain layers, and liner bedding fill material. Due to permit boundary and land ownership conditions, the majority of the construction materials must be derived from within the TSF footprint. Meeting fill, reclamation topdressing salvage, and reclamation cover material requirements for construction will necessitate stockpiling selected materials early in the life of the TSF because the borrow sources for these materials will be buried as the TSF footprint expands. Initial Phase 1 grading and liner installation will cover approximately 60 percent of the ultimate TSF footprint and much of the area available for borrowing construction material and reclamation cover.

The Phase 1 liner bedding fill material will be derived from the existing tailings produced during the Quintana Resources operation. All existing tailings lie within the Phase 1 construction footprint. Liner bedding fill needed for Phase 2 through Phase 5 construction will be derived from soil borrow areas.

Drain fill material will be produced by crushing and screening native soils and gravels. Drain fill material will be placed in contact with geomembrane liners. To meet drainage and liner compatibility requirements, a minus 1-inch gradation is assumed. The fine rejects (undersized) from drain fill material production will be suitable for liner bedding fill. Because Phase 1 liner bedding fill requirements will be met by using the existing tailings, the undersized materials produced in Phase 1 will be stockpiled for use as liner bedding fill in Phases 2 and 3. Liner bedding fill stockpiled in Phase 1 will be supplemented with additional material produced during construction. Approximately 100,000 cy³ of undersized material will be stockpiled in Phase 1.

6.1.1 Site Grading

The TSF grading plan for Phases 1 through 5 is illustrated on Drawing 10. The topographic surface shown on the grading plan reflects the over-excavation/removal of borrow materials required for cover material stockpiling and TSF construction. The approximate construction limits by phase are also indicated on Drawing 10. Drawing 11 illustrates the Phase 1 grading plan and Phase 1 construction. Site grading will include removal of the old starter dam and splitter dike for use as structural fill in the new starter dam and toe berm. Additional structural fill and drainage material will be borrowed from within the TSF footprint. In general, gravelly sands suitable for drain material lie on the interior impoundment slopes to the west of the dam. Materials suitable for structural fill are exposed on the surface over most of the TSF footprint. Borrow areas developed during phased construction will extend across construction phase limits but will lie within the ultimate TSF footprint.



6.2 Toe Berm and Starter Dam

The Phase 1 toe berm and starter dam are illustrated on Drawing 11. The TSF at final build-out is shown on Drawing 12, and the toe berm and dam sections and details are shown on Drawings 13 and 14.

A temporary toe berm will be constructed around the downslope TSF periphery in Phase 1. The temporary toe berm will be removed and reconstructed as the liner is extended outward and downslope in Phases 2 through 4. In Phase 5, a permanent toe berm will be constructed.

The primary purpose of the temporary toe berm is to contain runoff and sediment from the dam face, and direct dam drainage to the underdrain collection system and then to the TSF underdrain collection pond. Both the temporary toe berms and permanent toe berm will be constructed with structural fill and a geomembrane “flap” draped over the perimeter berm. The temporary berm liner flap will be anchored in a temporary perimeter anchor trench on the top of the berm. To relocate the temporary toe berm, the geomembrane flap will be folded inward over sandbags to divert drainage away from the temporary berm while it is removed. Once the temporary toe berm is removed, the liner extension will be installed and the original liner will be laid back over the liner extension. The seam between the two liners will then be extrusion welded in accordance with industry standards. This method will ensure that the liner seam is located in the downgradient flow direction for tailing drainage. At final build-out, a permanent toe berm will be constructed to contain runoff and sediment, buttress the dam toe and establish the limit for reclamation cover placement. The return water pipeline from the TSF underdrain collection pond will also run along the upstream side of the toe berm and above the geomembrane liner. The return water pipeline will be relocated in conjunction with the reconstruction of the temporary toe berms during Phases 2 through 4. In Phase 5, the return water pipeline will be placed in its final location along the upstream side of the permanent toe berm and above the geomembrane liner.

The Phase 1 starter dam will be constructed to an elevation of 5,250 ft-msl, with a 2.5H:1V inner slope and a 2H:1V outer slope. The purpose of the starter dam is to provide initial containment of tailing material, and to aid in tailings distribution from the dam crest. In the early stages of the operation, impounded water may periodically come in contact with the upstream face of the starter dam. The upstream face of the Phase 1 starter dam will be lined with an extension of the TSF geomembrane liner. The purpose of the liner extension is to prevent tailing drainage into the starter dam fill. The liner extension is illustrated on Drawing 13.

The starter dam will be constructed over the impoundment liner and underdrain collection systems. In Phases 2 through 5, starter dam extensions will be constructed to a height of 10 feet over the liner surface along the south, west and north boundaries.



6.3 TSF Liner System

Liner system details are shown on Drawing 14. The liner will consist of an 80-mil HPDE liner placed on a 12-inch thick liner bedding fill layer. In Phase 1, the liner bedding fill will consist of a minimum of 12 inches of tailings recovered from the north cell of the old starter dam. After Phase 1, liner bedding fill will consist of a 12-inch layer of crushed and screened native material, or selected local soil.

6.4 Tailings Drainage

Drainage from future tailings will be collected in two separate underdrain systems and transported to the TSF underdrain collection pond. Drainage from the TSF impoundment interior will be collected in a continuous underdrain (impoundment underdrain) constructed over the geomembrane liner. A separate blanket drain will underlie the tailings dam (dam underdrain). The layout of the underdrain systems is shown on Drawing 15. Underdrain details are shown on Drawings 16 and 17.

6.4.1 Drain Description

The impoundment underdrain system will consist of a system of primary 10-inch diameter drainage pipes placed in drainage channels, and a system of 4-inch diameter lateral drain pipes that cover the remainder of the TSF interior floor. Two types of drain fill will be used for the impoundment underdrain. These include primary drain fill placed as an envelope around primary drain pipes, and a continuous minimum 18-inch thick layer of selected native drain material that covers the impoundment liner and contains the lateral pipe network. The primary drain fill will be produced by processing native gravelly sand to reduce its content of fine sand, silt and clay sized particles. The native drain fill material will consist of selected site soils (gravelly sand).

Scour protection will be placed at points of cyclone overflow discharge to protect the impoundment underdrain system. The scour protection will consist of locally derived coarse material cover over the underdrain, or the incorporation of energy dissipation measures on discharge spigots. Scour protection details are provided in Drawing 29. The specific number and type of scour protection required will be determined based on estimated cyclone overflow discharge volumes and flow velocities.

The dam underdrain system will consist of a minimum 18-inch thick layer of primary drain fill material and a network of 4-inch diameter internal drainage pipes.

6.4.1.1 Impoundment Underdrain

Pipes in the impoundment and dam underdrains will be placed at a spacing that maintains minimum hydraulic head on the geomembrane liner and reduces the potential for leakage through the geomembrane liner. Pipe spacing is a function of the rate at which tailings drainage reports to the underdrain and the hydraulic conductivity of the drain fill.



Tailings placed in contact with the impoundment underdrain can be expected to rapidly consolidate and form a low permeability layer over the drain fill. Slurry consolidation tests indicate that the cyclone overflow that will be deposited in the impoundment interior will exhibit post-consolidation hydraulic conductivities ranging from 5×10^{-7} cm/sec for material deposited on the tailings beach, to 5×10^{-8} cm/sec for cyclone overflow slimes. Assuming a unit hydraulic gradient and an average tailings beach and slimes hydraulic conductivity of 2.75×10^{-7} cm/sec (9×10^{-9} feet/second), the rate of drainage through the tailings and into the drain layer will be on the order of 7.8×10^{-4} feet/day/ft². At final build-out with an impoundment floor area of 321 acres, total drainage collected in the impoundment underdrain will be on the order of 66 gallons per minute (gpm).

The drain pipe spacing is set to maintain a drain layer water depth and liner head that is less than the drain layer thickness of 1.5 feet. Because drainage into the impoundment underdrain will occur at a very low rate, the drain layer fill hydraulic conductivity can be relatively low and still maintain drainage and low liner head. Using the mound equation (Masada, 1988) and a hydraulic conductivity of 3.8×10^{-5} cm/sec for the native material drain fill layer, a spacing of 35 feet between impoundment lateral drain pipes will result in a maximum liner head of 1.33 feet. The spacing calculation assumes a 1 percent grade between drain pipes, which is the minimum grade on the TSF floor. Steeper slopes between drain pipes will reduce the head on the liner. Drainage mound and pipe spacing calculations are contained in Appendix D.1.

6.4.1.2 Dam Underdrain

The dam underdrain constructed beneath the cyclone underflow sand fill will be subject to different conditions. The sand will be relatively permeable and the drainage rate will be variable because sand deposition locations will change frequently. The water in the cyclone underflow will be delivered to the dam at an average flow rate of approximately 1,042 gpm. In order to determine dam underdrain pipe spacing, the following assumptions were used to estimate drain inflow rates:

- Approximately 42 percent of the water deposited with the cyclone overflow will be retained in the tailings pore space. The remainder (approximately 58 percent) will either drain through the sand dam and report to the drain, or be lost to evaporation. Approximately 15 percent of the underflow water is assumed to be lost to evaporation. The resulting maximum flow of approximately 448 gpm will report to the dam underdrain.
- Dam construction is assumed to occur over an area of approximately 100 by 600 feet (60,000 square ft²).
- The slope between drain pipes is one percent.
- The dam underdrain fill (primary drain fill) will be a relatively clean fill, with approximately 20 percent finer than the No. 4 standard sieve, produced by crushing and/or screening of native gravelly sand. It is assumed to have a hydraulic conductivity of 1×10^{-1} cm/sec. A permeability test of prepared drain fill material with 50 percent finer than the No. 4 sieve exhibited a hydraulic conductivity of 9.1×10^{-2} cm/sec.



Based on these assumptions, the rate of application of cyclone drainage to the dam underdrain will be approximately 5.42×10^{-4} cm/sec (1.5 feet/day/ft²). Using the mound equation, a pipe spacing of 45 feet will result in a maximum head of 1.41 feet on the geomembrane liner. Drain layer thickness will be a minimum of 1.5 feet.

6.4.2 Drain Fill and Tailings Filter Compatibility

6.4.2.1 General Requirements for Drain Fill

Drain fill materials shall meet hydraulic conductivity and stability requirements. The dam underdrain fill (primary drain fill) shall be capable of retaining the cyclone underflow sand while allowing the transfer of drainage, i.e., without clogging. Inside the impoundment, the primary drain fill shall also be compatible with the underdrain pipe slot size and shall be capable of retaining the select native drain fill material while allowing tailing drainage to pass.

The select native drain fill layer shall meet a number of conditions, including:

- It shall be erosion resistant to control the potential for scour while temporarily exposed on impoundment slopes.
- It shall be compatible with the drain pipe slot size.
- It shall be capable of retaining the cyclone overflow slimes.
- It shall be retained by the primary drain fill.

6.4.2.2 Dam Underdrain Fill/Primary Drain Fill

Figure 3 shows the particle size distribution curve (PSD) for the cyclone underflow that will be used to construct the TSF embankment and will be in contact with the dam underdrain. The cyclone underflow filter envelope defines the range of drain fill gradations that are filter compatible with the cyclone underflow, i.e., materials that will restrict the migration of the cyclone underflow sand into the drain fill. It is assumed that this material will be prepared on site by reducing (screening out) a portion of the fine fraction of the native gravelly sand. A drain material with a minus 150-micron (No. 100 standard sieve) fraction of less than 10 percent and a minus 75-micron (No. 200 standard sieve) fraction of less than 5 percent is anticipated.

Figure 3 shows the estimated average PSD for the native soil composite samples following removal of the fine fraction with approximately 10 percent finer 425 microns (approximately 10 percent passing the No. 40 sieve). The modified gradation falls within the filter envelope for the cyclone underflow.

Type N-12 dual wall perforated, corrugated pipes have 3-millimeter (mm) wide slots for pipe diameters up to 10 inches. For broadly graded drain fill, Federal Emergency Management Agency (FEMA) (2007) recommends that the ratio of the D₈₅ particle size (the particle size for which 85 percent is finer) to the drain pipe slot width be greater than 4. To satisfy this condition, a D₈₅ of at least 12 mm is recommended.



Gravelly sand composite samples that were collected to represent materials available for drain construction meet the D_{85} requirement. When screened to remove fine particles, the D_{85} particle size will increase and the material will continue to meet the D_{85} size recommendation.

The primary drain fill that will be placed as an envelope around the primary drain pipes in the TSF interior will meet the same D_{85} size requirements for compatibility with the drain pipe slot size. The gravelly sand processed for the dam underdrain will also meet the requirements for primary drain fill.

For field production of the dam drain fill and primary drain fill during TSF construction, the minus 0.19 inch fraction (material finer than the No. 4 sieve) will be screened out to produce a granular drainage material. Table 7 presents the specification for the primary drain fill.

Table 7: Primary Drain Fill Specification

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	100-40
No. 4	0-20
No. 100	<10
No. 200	<5

Based on review of native soil gradation test results, a primary drain fill recovery rate of 40 to 60 percent is estimated during processing of available gravelly-sand soils. Fine rejects will be suitable for liner bedding fill material.

6.4.2.3 Select Native Drain Fill

The cyclone overflow slimes represent the finest material that will be discharged into the impoundment and placed in contact with the impoundment underdrain. Figure 4 shows the PSD of slimes and the gradation of the filter envelope required to retain the slimes. The SD of the composite gravelly sand samples that are anticipated to be used for the select native material drain fill are also shown. The PSD intercept the slimes filter band in the critical D_{15} range. The select native drain fills will be capable of retaining the slimes. Some local clogging could be anticipated because the native material fines content is higher than required; however, the material is expected to function as needed because the drainage rate into the filter layer will be very low. Table 8 presents the gradation specifications for the select native drain fill.

**Table 8: Select Native Drain Fill Specification**

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	90-60
No. 4	80-40
No. 10	70-30
No. 100	10-30
No. 200	>10

If the native soils are crushed to minus 1 inch, the recovery of select native drain fill should be 100 percent during field processing for construction.

6.4.2.4 Primary and Select Native Drain Fill Compatibility

The primary drain fill shall be capable of retaining the select native drain fill, while allowing tailing drainage to pass from the select native fill into the primary drains. Figure 5 shows the filter envelope for the select native drain fill and the estimated PSD for the primary drain. The primary drain fill in the critical D_{15} to D_{50} range of the PSD falls within the select native drain material filter envelope and the two drain materials are expected to be filter compactible, i.e., the select native drain fill will not migrate into the primary drain fill.

6.4.3 Drain Piping

The primary drainage pipes in the impoundment underdrain will be 10-inch diameter Type N-12 dual wall perforated, corrugated polyethylene pipe (PCPE). The 10-inch PCPE pipe will be plain-end and joined with soil tight split couplings.

The lateral drain piping in the impoundment and dam underdrain will be 4-inch diameter Type N-12 PCPE pipe with plain ends and soil tight split couplings.

6.4.4 Pipe Placement

The primary drain pipes will be placed in a constructed channel or drainage swale that is a minimum of 16 inches deep and inside an envelope of primary drain fill as shown in Drawings 16 and 17. After the primary drain fill is placed, the lateral drain pipes will be placed on the liner surface and oriented to drain into the primary drain pipe channel. In the primary pipe channel, lateral pipes will lie in select native fill and run parallel to the primary drain fill envelope for a length of at least 4 feet. The select native drain material fill will be placed over and around the primary drain fill and lateral drainage pipes. The final cover of primary and select native drain fill placed over the primary pipes will be a minimum of 24 inches thick. Pipe placement is such that collapse or damage of an individual lateral drain pipe will not result in the transfer of tailings or select native drain fill into the primary drain pipes. The cover of select native drain fill



over the primary drain fill will prevent the transfer of soil or tailings from the lateral to primary pipe network.

Dam underdrain pipes will be placed within the drain fill and oriented to discharge outside the toe of the dam in the dam underdrain and runoff collection channel. As the dam is raised, the dam underdrain and runoff collection channel will be relocated and the dam underdrain pipes will be extended to the new perimeter collection channel.

6.4.5 Drain Pipe Deflection

Deflection analyses were conducted to evaluate the performance of the underdrain pipes under the loads imposed by a tailings cover of approximately 240 feet. The Type N-12, dual walled PCPE pipes used for the underdrain collection system are considered to be flexible and can resist damage by distorting sufficiently to shed overburden loads to the surrounding underdrain fill.

As pressure on the top of the pipe is increased through an increase in tailings height, an increasing proportion of the vertical pressure on the pipe is transferred to the surrounding fill. This process is commonly called bridging. Therefore, the key parameter in assessing deflection of the underdrain collection system is the stiffness (modulus) of the fill in contact with the pipe. This bridging phenomenon was first accurately modeled by Burns and Richards in the paper *Attenuation of Stress for Buried Cylinders* (Burns and Richards, 1964). Golder has analyzed the pipe stresses and deformations based on the work of Burns and Richards and Hoeg (1968), with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression.

Golder analyzed the worst-case scenario with an entire column of tailings underflow cyclone sand of maximum height placed over the underdrain collection system. Based on Golder laboratory test results for the tailings underflow cyclone sand, a friction angle of 39 degrees and soil density of 120 pcf was used to model the tailings properties used in the analysis. The supporting fill was assumed to consist of the select native drain fill material with a similar stress versus strain relationship for a silty sand compacted to 80 percent of maximum dry density. The primary drain fill used beneath the dam and in the impoundment primary drains will have a reduced fines content compared to the select native drain fill, and will exhibit a strength that is at least that of the select native drain fill. Modeling deflection based on the select native drain fill is conservative.

Drain fill properties, along with pipe dimension and properties, were used to determine the maximum deformation that may be expected for the pipes. The maximum vertical pipe deflections are estimated to be between 11 and 14 percent. Golder's observations have been that pipe deflections greater than 15 to 20 percent often result in plastic deformation of the pipe at the springline. A vertical deformation of 15 percent is assumed as the maximum limiting deflection for flexible pipe. Therefore, the estimated



deformations of 11 to 14 percent are within the acceptable performance criteria established by Golder. Detailed data worksheets and calculations are provided in Appendix D.2.

6.4.6 Drainage Outlet Works

Tailing drainage from the impoundment underdrain will be routed beneath the tailings starter dam and cyclone sand dam to the TSF underdrain collection pond. Drainage outlet works details are shown on Drawings 16 and 17. The primary drain pipe network in the impoundment underdrain will be reduced to three pipes to transmit drainage to the outlet works. Three primary drainage pipes will be routed into a 12-inch diameter Type N-12 PCPE manifold at the upstream toe of the starter dam, which will in turn be connected to a 14-inch diameter, Schedule 80 carbon steel drain pipe. The steel drain pipe will be routed from the upstream toe of the starter dam to the TSF underdrain collection pond in a 42-inch deep by 42-inch wide ditch that will be backfilled with concrete.

The steel outlet pipe will pass through a valve vault. The valve vault will consist of a 72-inch diameter prefabricated concrete manhole base unit placed on an 18-inch thick reinforced concrete foundation mat. The manhole base unit will be fabricated with inlet and outlet openings for the steel drain pipe. Prefabricated manhole riser sections will be used to extend the valve vault vertically to a maximum height of 100 feet, to an elevation of approximately 5,300 ft-msl.

A 14-inch diameter, hydraulically actuated knife gate valve will be installed on the steel outlet pipe in the valve vault. The hydraulic actuator lines will be 100 feet long and will be routed up through the riser sections to the top surface of the dam for connection to a portable hydraulic power pack. As the manhole riser sections are added, the outlet valve hydraulic lines will be secured to the man-way ladder steps cast into the manhole riser sections. This arrangement will enable valve operation without manhole entry.

The purpose of the outlet valve is to prevent the drainage of excess water into the TSF underdrain collection pond in the early stages of impoundment operation (i.e., before the impoundment underdrain is covered with cyclone overflow). As positioned, the valve will be upstream of the main body of the dam, and its use will not result in pressurization of the underdrain pipe inside the dam. Once the underdrain is covered with cyclone overflow, the flow into the impoundment underdrain will be limited by the low hydraulic conductivity of the tailings slimes overlying the drain. When the dam reaches a height of 100 feet (approximately 5,300 ft-msl), the outlet valve will be fully opened and the valve vault will be backfilled with cement grout and granular fill materials.

6.5 TSF Underdrain Collection Pond

6.5.1 Pond Description

The location of the TSF underdrain collection pond is shown on Drawings 2, 10, 11, and 12. TSF underdrain collection pond details and sections are shown on Drawings 13, 18, 19 and 20. Figure 6



illustrates total TSF underdrain collection pond capacity, the maximum operating water required to preserve upset and stormwater storage capacity, and the maximum stormwater storage level.

The TSF underdrain collection pond will be double-lined with minimum 60-mil HDPE geomembrane liners. An HDPE geonet will be placed between the liners to serve as the collection pond leakage collection and recovery system (LCRS) and minimize the head on the lower pond liner. The pond will be fitted with a primary drain material filled sump and LCRS pump to recover any leakage through the upper geomembrane.

TSF underdrain collection pond reclaim pumps will be submersible, vertical turbine pumps supported in a reinforced concrete sump and headwall structure. The sump will allow the water level in the pond to be drained to the pond floor level and no dead storage will be required. The use of submersible turbine pumps mounted in a concrete sump will eliminate the potential for liner damage associated with a barge mounted pump coming to rest on the pond floor. The reinforced concrete sump is shown in cross section on Drawing 19.

Impoundment underdrain flows will be transported to the pond via a buried steel pipe. Runoff and dam underdrainage will be routed to the pond via an HDPE lined open ditch constructed at the toe of the dam.

6.5.2 TSF Underdrain Collection Pond Sizing

The TSF underdrain collection pond will contain drainage water from the TSF impoundment and dam underdrains, as well as runoff from the downstream face of the tailings dam. The pond is sized to contain 24 hours of tailing drainage flow at maximum estimated drainage rates, runoff from the 100-year, 24-hour storm event of 3.73 inches (National Oceanic and Atmospheric Administration [NOAA] 2006) incident on the downstream dam face, and an additional minimum 2-feet of freeboard. Underdrain flow rate calculations and runoff estimates are contained in Appendix E.

Underdrain flow rate estimates are based on the assumption that materials representative of the consolidated, cyclone overflow will be in contact with the impoundment underdrain and will control the rate of tailing drainage reporting to the TSF underdrain collection pond. The hydraulic conductivity of materials representative of beach and slimes samples are 5.0×10^{-7} and 5.0×10^{-8} cm/sec, respectively. If it is assumed that the more permeable beach-like material cover 60 percent of the impoundment underdrain at final build-out, the maximum underdrain flow rate will be on the order of 66 gpm.

Approximately 1,042 gpm of water will be delivered to the dam in cyclone underflow with a moisture content of approximately 30 percent. An estimated 42 percent of the water will be permanently bound or entrained within the pore space of the sand fill, and an additional 15 percent is estimated to be lost to evaporation. The remaining 28 percent (448 gpm) is assumed to report to the dam underdrain and TSF underdrain collection pond.



A storage allowance is provided for potential inflows associated with the free water pond coming in direct contact with the impoundment underdrain system. In this case, the drainage rate will be controlled by the hydraulic conductivity of the select native drain fill material that will cover the impoundment floor and drain pipe network. Permeability testing of a representative sample of select native drain fill indicated a hydraulic conductivity of 3.8×10^{-5} cm/sec. If it is assumed that a 20-acre area of impoundment drain will be inundated to an average depth of 2.5 feet, the estimated drainage rate will be on the order of 1,220 gpm.

The maximum contribution of stormwater runoff to the TSF underdrain collection pond will be from the combination of dam out-slope area and exposed toe area liner and underdrain occurring in Phase 4. The 100-year, 24-hour storm event incident on this area is estimated to produce a runoff volume of 3.94 million gallons.

Table 9 summarizes the TSF underdrain collection pond storage capacity requirements. The pond capacity is approximately 12.24 million gallons with 2 feet of dry freeboard below the crest of the pond (top of pond liner). The pond has the capacity to store up to approximately 5.8 million gallons of process water for facilitating process water make-up or storage of extra water during wet periods. A maximum water surface elevation of 5,157 should be maintained in order to provide sufficient storage for stormwater associated with the 100-year, 24-hour storm event and a coincident 24-hour upset period.

Table 9: TSF Underdrain Collection Pond Storage Capacity Requirements

Source	Type of Inflow	Volume (gal)
Dam Face Runoff	Storm Event Runoff	3,942,528
Dam Underdrainage	24-hour upset volume	645,206
Impoundment Underdrainage	24-hour upset volume	95,074
Free water pond direct drainage	24-hour upset volume	1,754,857
Total		6,437,666

6.6 Surface Water Management

6.6.1 Control of Impoundment Runon

The TSF will be required to contain inflows and direct precipitation associated with the 72-hour PMP of 26 inches. Diversion ditches constructed for impoundment runon control have been sized to carry the peak discharge associated with the prescribed PMP event using a rainfall intensity versus time distribution defined in Hydrometeorological Report 55A (US Department of Commerce, 1998). Runoff estimation and ditch sizing calculations are contained in Appendix F.1.

Diversion ditches will be constructed to divert runon away from the impoundment where possible. Peripheral catchment and runoff contributory areas are limited because the TSF lies in the head of a hydrologic catchment area. The Phase 1 grading plan (Drawing 11) indicates the location of diversion ditches. The Phase 1 ditches are located outside the Phase 2 construction area and will be functional



during Phases 1 and 2. In Phase 3, a permanent diversion ditch will be constructed on the west periphery of the TSF as shown on Drawing 11. Table 10 summarizes peak discharge estimates for the Phase 1 and 3 diversion ditches.

Table 10: Summary of Impoundment Runoff Diversion Ditch Capacity and Size Requirements

Phase/Location	Peak Discharge (cfs)	Ditch Width (ft)	Ditch Depth (ft) ⁽³⁾
Phase 1, TSF northeast	525	10 ⁽¹⁾	5.5
Phase 1, TSF southwest	340	10 ⁽²⁾	5.1
Phase 3/ TSF southwest periphery	205	10 ⁽²⁾	4.2

Notes

⁽¹⁾ 2H:1V side slopes assumes on the downslope side, slope on upstream side varies

⁽²⁾ 2H:1V slopes

⁽³⁾ Depth at the lowest channel slope, includes 1 foot of freeboard.

6.6.2 Dam and TSF Underdrain Collection Pond Surface Water Management

Surface water management facilities other than impoundment diversions are designed to contain and transport flows associated with the appropriate 100-year storm event. Hydrologic calculations for the dam and TSF underdrain collection pond surface water management facilities are contained in Appendix F.2.

Runoff from the downstream face of the dam will be routed to the TSF underdrain collection pond. The time of concentration (T_c) for the toe ditch catchment area is estimated to be three hours. The dam underdrain and runoff collection channel at the toe of the dam has been sized to carry the peak discharge associated with the 100-year, 3-hour storm. The 100-year, 3-hour storm will produce the peak 100-year storm runoff of 71 cubic feet per second. The flow depth at peak discharge is estimated to be a maximum of 0.5 feet. The perimeter toe berm height will be 3 feet (temporary berms) to 4 feet (permanent berms) high and will provide 2.5 to 3.5 feet of dry freeboard in the toe area dam underdrain and runoff collection channel.

6.7 Cyclone Plant Area

Excavation and site preparation will be required for the cyclone plant pad, the pump equipment pad and the surge pond. The cyclone plant general arrangement plan and site grading plan are shown on Drawing 21. The purpose of the cyclone plant is to separate whole tailings into sand and slimes fractions. Its' design and purpose are described in more detail in Section 7.0.

Surge pond cross sections and details are shown on Drawing 22. The purpose of the surge pond is to contain discharges (tailings, process, and reclaim water) from various processing locations under upset conditions, due to a pipe failure or shutdown of the cyclone plant. Upset flows from the cyclone plant will discharge by gravity to the surge pond within a secondary containment ditch lined with a minimum 60-mil HDPE geomembrane liner placed over 6 inches of liner bedding fill. Further details of the secondary containment ditch are provided below in Section 7.4.



7.0 TAILINGS DELIVERY AND DISTRIBUTION SYSTEM DESIGN

7.1 General System Description

The tailings delivery and distribution system design consists of pipeline system that delivers whole tailings from the processing plant to the tailings storage facility. Whole tailings will be separated into fine material and sand material in the cyclone plant. The sand fraction will be transported to the TSF and used for dam construction while fine material will be deposited into the TSF. The tailings surge system is designed for tailings management in case of unanticipated shutdown of any of the tailings stations or surges or overflows from station sumps. Return or reclaim water will be collected from the TSF surface pond and TSF underdrain water collection pond and transported back to the process plant. A general process flow diagram for the tailing delivery and distribution system is provided on Drawing 23.

Process equipment for the tailings delivery and distribution system will be located in four main stations as listed below:

- Cyclone Station: including the cyclone cluster, slurry pumps, slurry transfer sumps, gland seal water system, and electrical equipment;
- Surge Discharge System: including the surge pond evacuation pumps and lined secondary containment ditches;
- TSF Return Water Pond Barge Station: including a floating barge and barge mounted vertical turbine pumps and electrical equipment; and
- TSF Underdrain Collection Pond Pump Station: including vertical turbine pumps in a permanent structure and electrical equipment.

Tailings distribution will include whole tailings transport from the process area to the cyclone station and sand and fine tailings transport to the TSF. Return water will include tailing drainage water and TSF return water transported to the process plant. The major pipelines are listed below, and their interactions are shown in the overall system process flow diagram on Drawing 23.

- Cyclone Feed Line
- Cyclone Overflow Line
- Cyclone Underflow Line
- Cyclone Whole Tailings Bypass Line
- TSF Return Water Line
- TSF Underdrain Collection Return Water Line
- Main Surge Discharge Line

The major pipelines will be installed within secondary containment ditches lined with a minimum 60-mil HDPE geomembrane liner placed over six inches of liner bedding fill. The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23



NMAC, and will include secondary containment. Further details of the secondary containment ditches are provided below in Section 7.4.

The arrangement of the major components of the tailings delivery and distribution system is shown on Drawings 21 and 24. Drawings 25 and 26 present the tailings delivery and distribution system plan and profile. Whole tailings produced at the flotation plant will be transported via a 30-inch HDPE DR17 pipeline to the cyclone plant at the northwest side of the TSF. The cyclone plant will separate the sands fraction, which represents approximately 45 percent of the whole tailings stream, from the slimes fraction, which represents approximately 55 percent of the whole tailings stream. The sands fraction or the “underflow” of the cyclone plant will produce an underflow slurry which will be transported to the TSF in a separate 12-inch HDPE DR9 pipeline and discharged on the dam. The cyclone underflow sand placed on the dam crest will be then be spread, graded and compacted, as necessary to push sand down the dam out-slope and continually build the dam.

The cyclone overflow (slimes) from the cyclone plant will be routed to the TSF in a separate 30-inch HDPE DR17 pipeline and discharged into the impoundment. The cyclone overflow water will be returned to the process water reservoir at a rate of up to 13,000 gpm. The cyclone plant will operate continuously to produce the sand material needed for continuous construction of the dam. In the event of upset conditions when the cyclone plant is not in operation, whole tailings will be discharged via gravity into the surge pond through a lined secondary containment ditch (see Section 7.4).

7.2 Tailings Delivery

7.2.1 Underflow Sand

The cyclone underflow pipeline will deliver sand to the top of the embankment for tailings dam construction. Two underflow pipelines will be used. The east leg will be routed around the north side of the TSF, and the south leg will be routed around the south side of the TSF as shown on Drawing 24. Each leg is sized to transport 100 percent of the cyclone underflow at up to 45.6 percent sand recovery. This allows for 100 percent availability of sand delivery to the dam.

Cyclone underflow will be discharged through 4-inch spigots placed every 333 feet. Each spigot will include one 4-inch manual pinch valve. The underflow pipelines will also have in-line knife-gate isolation valves every 2,000 feet to allow for isolation and relocation of the pipe as the dam rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pick-up truck.

The north and south cyclone underflow pipelines will be operated independently. When one is in operation, the other can be serviced or broken down and relocated. Cyclone underflow pipes will be flanged each 500 feet to facilitate breakdown and relocation.



7.2.2 Cyclone Overflow

Two cyclone overflow delivery pipelines, one leg to the north side and one leg to the south side of the TSF, will transport the cyclone overflow to the TSF interior (impoundment interior) as shown on Drawing 24. The cyclone overflow will be discharged via spigots placed every 667 feet. Each spigot will include a manual pinch valve. Each pipe is sized to carry 100 percent of the cyclone overflow to permit pipeline relocation without interrupting operation as the TSF elevation rises. One leg will remain active while the other is serviced or relocated.

The cyclone overflow pipelines will also have knife-gate isolation valves placed every 2,000 feet to allow for isolation and relocation of the pipe as the impoundment rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pickup truck. The cyclone overflow delivery pipelines will be flanged every 500 feet to allow for breaking down and relocating the pipe.

7.3 Deposition Management

7.3.1 Dam Construction

Figure 7 illustrates height versus capacity and surface area relationships and rate of rise for the tailings impoundment. Near continuous operation of the cyclone plant will be required to produce sufficient sand to construct the dam to the ultimate elevation of 5,460 ft-msl. The difference in elevation between the dam crest and the head of the cyclone overflow impoundment beach will be maintained at 10 feet. Maintenance of this elevation difference will place the transition between the cyclone underflow sand dam fill and the interior cyclone overflow a distance of approximately 30 feet upstream of the inside dam crest. The elevation differential of 10 feet will be maintained in order to maintain adequate freeboard for stormwater storage. Maintaining this elevation differential also allows for maximum storage capacity for cyclone overflow, and thus maximum production of sand needed for dam construction.

In the early stages of operation, there will be more sand available than needed to maintain the elevation differential between the dam crest and the head of the beach. Excess sand will be pushed down the out-slope of the dam and used to construct the dam base. In the later stages of operation, the sand previously used to construct the dam base will reduce the sand requirements for raising the dam crest, and facilitate maintaining the crest to beach elevation differential.

7.3.2 Cyclone Overflow Discharge

The storage volume for the cyclone overflow will be maximized through managing deposition and practicing sub-area, thin lift deposition. Discharge spigot locations will be frequently cycled so that a thin lift of tailings is placed on the tailings beach. Exposure to evaporation prior to burial with a subsequent lift of tailings will allow the tailings to desiccate and consolidate. The degree to which thin lifts can be placed



and consolidation can occur under managed deposition is primarily a function of rate of tailings rise. It is also influenced by tailings properties, climatic conditions, surface water management, and operator effort.

7.4 Management of Upset Flows

Potential upset flows from the process area, cyclone plant, and TSF will be controlled through a series of secondary containment ditches, the surge pond, and the TSF underdrain collection pond (see Section 6.5). The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23 NMAC. The secondary containment ditches will run from the process area to the TSF (the main ditch), from the main ditch to the cyclone area, and from the cyclone area to the surge pond. The secondary containment ditches are designed to contain and transport flows via gravity that are related to potential upset conditions and direct precipitation onto the ditches associated with the 25-year 24-hour storm event (2.88 inches). Maximum upset flow conditions would be associated with overtopping of the process water reservoir (as estimated by M3, the design contractor for the process water reservoir). This maximum upset flow was assumed to be 18,000 gpm over a 30-minute period, at which point the process area pumps would be shut down. The secondary containment ditches are designed for these maximum upset flows, direct precipitation, and an additional 2 feet of freeboard. The main ditch is designed to flow to the TSF by gravity for the first six years. After year six, gravity flow to the TSF is no longer possible because of the increased height of the TSF and upset flows will then discharge to the surge pond via gravity in a lined ditch through year 11.1. The alignment of the secondary containment ditches is shown on Drawings 2, 3, 10, 11, 12, 21, and 24 through 26. Details of the secondary containment ditches are provided in Drawing 29.

Surge pond cross sections and details are shown on Drawing 22. The surge pond liner system will consist of a liner bedding fill layer overlain with a minimum 60-mil HDPE geomembrane liner. The surge pond is located at an elevation of 5,340 feet and is sized for a surge retention time of half an hour with an additional reserve capacity of over one million gallons. The pond is sized for the retention of approximately 1,610,000 gallons of slurry with an additional 2 feet of freeboard. The use of the surge pond will be intermittent and temporary and the pond will be empty under normal operating conditions. The pond will be equipped with dedicated hard-wired pumps that will automatically evacuate its contents. Emergency power for the pumps will be provided by the emergency diesel power generation system located on-site in the event of a power outage. The process facility control room will be equipped with emergency alarms that notify the operator of an upset condition allowing the operator to make necessary adjustments in the process, as needed. The pumps at the surge pond will be automatically activated upon the pond reaching a predetermined level. Water and solids collected from the surge pond will be discharged through a 12-inch HDPE DR17 pipeline to the top of the TSF. The solids handling pump is designed to evacuate the surge pond within 12 hours.



8.0 WATER RECLAIM SYSTEM DESIGN

The water reclaim system is a significant part of NMCC's water conservation program. It will provide approximately 75 percent of the water used in the process in the form of recycled water. The purpose of the water reclaim system is to recycle supernatant water stored in the TSF and water captured in the underdrain collection gallery and stored in the TSF underdrain collection pond. The TSF water reclaim system will recover water released as the cyclone overflow consolidates within the TSF. The underdrain water collection system will recover water from the bottom of the tailings impoundment through the TSF underdrains into the TSF underdrain collection pond. The underdrain pond will also store water captured by the dam underdrains from the downstream side of the dam as sand is deposited and compacted as well as precipitation run-off water from the face of the dam. All of this water will be transported from the TSF and TSF underdrain collection pond to the process water reservoir located at the plant via a 20-inch HDPE variable DR return water pipeline. Water from the TSF underdrain collection pond may also be pumped back directly to the TSF. The major components of the water reclaim system are shown on Drawing 27. Water reclaim system details are shown on Drawing 28.

The TSF reclaim system will be a barge-mounted pump station in the impoundment equipped with four pumps (three operating and one spare) with a design flow from this station of 13,000 gpm. This is equal to the maximum design rate at which water will be delivered to the TSF from the cyclone plant during normal operation.

The TSF underdrain collection pond system will be a pump station of two pumps (one operating and one spare) installed in a sump within the pond. The design flow from this station will be 4,000 gpm. This is the maximum design flow of the TSF underdrain collection gallery. It is anticipated that up 4000 gpm of flow will be captured by the underdrains in the initial stages of operation of the TSF. This flow rate will become less over time as the tailings impoundment fills and the underdrains become overlain with tailings materials.

The water reclaim barge in the TSF impoundment will be accessed from a ramp constructed over the impoundment liner as shown on Drawing 28. The ramp will be approximately 35 feet wide and initially constructed to a height of 10 feet above the impoundment floor. As the tailings level rises during operations, the position of the barge will migrate up the ramp northwestward along the reclaim pipeline alignment. In each construction phase, ramp construction will be completed with borrowed structural fill material.



9.0 WATER BALANCE

Water balance calculations are included in Appendix G. Figure 8 summarizes the results of a water balance analysis of the proposed TSF for average rainfall conditions. The water balance model incorporates water input from slurry water inflow, direct precipitation on the impoundment surface and runoff from un-diverted upgradient areas. On-site meteorological data collection at Copper Flat was initiated in August 2010. Due to the short duration of record keeping and recording gaps, NOAA data from Hillsboro and Caballo Dam, New Mexico, were used in conjunction with Copper Flat data to estimate monthly precipitation and evaporation rates. The ratio of site evaporation to Hillsboro evaporation for months where data are available from both sites was used to estimate site evaporation for months where data were not collected at the site.

Water balance model losses include entrainment of water within the tailings solids, evaporation from the TSF supernatant pool, evaporation from the exposed tailings beach, and evaporation of water from the dam. Entrainment represents the most significant water loss and is calculated on the basis of the estimated final post-deposition dry density of the cyclone underflow and cyclone overflow. An average post-deposition dry density of 57 pcf is assumed for the cyclone overflow. Over the life of the facility, approximately up to 49 million tons of cyclone underflow will be produced assuming near constant operation of the cyclone plant and mill.

The water balance model does not identify reclaim rates from specific locations because operation of the impoundment will impact where water accumulates. Water that is not lost to evaporation or bound within the tailings is assumed to be recovered from either the impoundment free water pond or the TSF underdrain collection pond.

The impoundment underdrain will be equipped with a shutoff valve at its inlet during the initial years of operation so that when the water level in the TSF underdrain collection pond exceeds the normal operating level, the underdrain will be closed to utilize the TSF supernatant pool for storage and the TSF undergrain collection pond will be pumped down.

A total process water inflow of 12,978 gpm is estimated based on average operating conditions. This inflow includes water contained in the cyclone overflow slurry and water delivered to the dam with the cyclone underflow. As shown in Figure 8, the estimated process water reclaim rate averages 9,215 gpm. The average make-up water requirement, calculated as the difference between the process water delivered and the water reclaimed, is estimated to be 3,169 gpm or approximately 152 gallons per ton of tailings placed in the TSF. The maximum estimated make-up water rate is 3,676 gpm.

The water balance examines water reclaim rates for average rainfall conditions. If the site experiences precipitation that is less than or exceeds average conditions, water reclaim rates and make-up water



requirements can also be expected to vary. The water reclaim system is capable of recovering water at a rate adequate to account for all water in the tailings slurry discharged from the flotation plant. Maximum reclamation of water following storm events can temporarily reduce demand on external water sources. The water balance does not consider additions from the open pit or waste rock stockpile stormwater ponds. Water available from other on-site sources is not expected to be significant.



10.0 TAILINGS DAM STABILITY ANALYSIS

10.1 Methods

Slope stability analyses were performed in support of the feasibility design of the Copper Flat TSF with an ultimate crest elevation of 5,435 ft-msl. The analyses were performed using limit-equilibrium slope stability software and Spencer's (Spencer, 1967) method of slices to compute the theoretical factors of safety (FOS) for various potential failure surfaces. Material properties used in the stability calculations for native soil and tailings materials were based on laboratory geotechnical testing performed in Golder's in Denver, Colorado laboratory and discussed previously in Sections 3.3 and 4.3.

Slope stability analyses were conducted to determine the FOS against failure for the critical stability section along the highest embankment section and most adverse subsurface topography on the downstream slope. The critical factors of safety assumed for stability analysis are 1.5 for static conditions and 1.1 for pseudo-static conditions, according to NMAC 19.25.12.11.12. Stability analyses were performed for static (steady-state) and dynamic (pseudo-static) loading conditions. Steady-state loading conditions represent the long-term stability of the TSF and pseudo-static loading conditions represent the stability of the TSF during the design earthquake loading event.

The computer package SLIDE™ (Version 6.021) was used to conduct the stability analyses (Rocscience, 2013). An arcuate (circular) failure mode was used to analyze the critical section with the shear surface failing through the tailings and/or foundation materials. A block failure mode was used to analyze the critical section with a potential failure at the liner interface. Both methods are based on the principle of limit equilibrium, i.e., the method calculates the shear strengths that would be required to maintain equilibrium, and then calculates the FOS by dividing the available shear strength by the shear strength required to maintain stability.

Analyses were limited to the investigation of global failures that can affect the full height of the embankment, are deeper seated, and may result in lowering the embankment crest and loss of containment. Local stability analyses associated with shallow slope failures were not investigated.

A pseudo-static analysis approach was used for the seismic loading case. With this method, a lateral force is added to a potential failure mass, with magnitude equal to some fraction of the weight of the sliding mass. The fraction is defined in the form of a pseudo-static coefficient and is expressed as a percentage of gravity. Selection of the pseudo-static coefficient is discussed below. Stability analysis supporting data and computer-generated outputs are contained in Appendix H.



10.2 Seismic Design Criteria

According to the regulations set by the NMDSB, the TSF can be classified as having a significant hazard potential. Dams assigned the significant hazard potential classification are those where failure results in no probable loss of human life but can cause economic loss, environmental damage, and/or disruption of lifeline facilities. The NMDSB requires that structures such as the Copper Flat TSF be designed to withstand the seismic loading from the Maximum Design Earthquake (MDE) with a 2 percent probability of exceedance in 50 years (approximately 2,475-year return frequency). The peak ground acceleration (PGA) for the Copper Flat property was obtained using the US Seismic “Design Maps” Web Application developed by the United States Geological Survey (USGS) Geologic Hazards Science Center (USGS, 2011). Considering the 2009 National Earthquake Hazards Reduction Program provisions for a Site Class C and a site location of 32.96° North latitude and 107.5° West longitude, the resulting PGA for the 2,475-year return MDE is approximately 0.13 times gravitational acceleration (0.13g).

The method developed by Hynes-Griffin and Franklin (1984) and Jansen (1985) was used to evaluate pseudo-static loading conditions, as outlined by the NMDSB guidelines (2010). This method recommends that the pseudo-static coefficient selected for analysis must be at least 50 percent of the predicted PGA, but not less than 0.05g, and the FOS under pseudo-static analysis should be 1.1 or greater. A coefficient of 0.087g, corresponding to two-thirds of the design PGA, was conservatively used for the analyses.

The results of the previous seismic liquefaction potential evaluation of the Copper Flat TSF by SHB (1980) indicated that the probability of liquefaction affecting the Quintana TSF was extremely remote based on the seismic hazard potential of the site and empirical data derived from case histories of tailings dams and natural, saturated, loose sandy deposits subject to earthquake-induced ground motions. Golder anticipates that a drained response to seismic loading will dominate the pore water conditions in the TSF during and following its active life based on the material permeabilities, boundary drainage conditions, and construction practices. Given the seismic hazard potential of the site and the proposed TSF construction and operating practices, the liquefaction potential of the new TSF is also considered low.

10.3 Tailing Drainage and Phreatic Conditions

The primary source of tailing drainage is drain-down water associated with the centerline-constructed cycloned underflow tailings portion of the embankment, with a minor contribution from consolidation and drainage of the impounded cyclone overflow upstream of the embankment crest. The drainage control system consists of a continuous granular fill blanket drain beneath the cyclone underflow sand embankment and toe area, with lateral underdrain pipes spaced at 45-foot centers across the embankment footprint. Lateral drain pipes will connect to an open channel, which gravity drains to the HDPE-lined TSF underdrain collection pond.



The phreatic surface was assigned to be coincident with the drainage control system at the base of the final embankment, based on performance of similar structures. Because of the large difference in the hydraulic conductivity of the cyclone underflow and cyclone overflow, a near vertical phreatic surface is assumed at the dam/beach interface. This condition will result in the main body of the dam being well-drained and unsaturated.

10.4 Pore Pressure Conditions and Liquefaction Potential

The response of tailings material to loading can be either drained or undrained, and is associated with the development of pore water pressures. For stability analyses, a phreatic surface was assumed at the beach material surface level upstream of cycloned underflow tailings embankment and an undrained response was evaluated. Undrained analyses were performed by applying an undrained strength to the cyclone overflow on and beneath the beach.

Susceptibility to liquefaction potential is assumed to be limited to the saturated beach areas upstream of the dam. Steady-state (residual) undrained strength was applied to the tailings beach upstream of the cyclone sand dam fill to evaluate post-liquefaction stability.

The risk of static and seismic liquefaction triggering will likely be low if appropriate control over embankment construction and the phreatic-surface elevation is exercised, and the cyclone underflow sand behaves as anticipated. Sand placed on the dam crest will be spread and compacted. Operating experience at mine sites utilizing cyclone underflow fill indicates that self-weight compaction of sand on the tailings out-slope is typically adequate for minimizing liquefaction potential. At Copper Flat, some compaction will be realized on the dam out-slope as a result of dozer spreading operations.

10.5 Material Properties

The material properties used in the stability analyses are based on a review of the properties used in previous site studies (SHB 1980), new data derived from testing of samples collected during the site geotechnical investigation, and from tests conducted on tailings characterization study samples. The components of the slope stability model include:

- Foundation Materials
- Liner Interface Zone
- Cyclone Underflow sand
- Cyclone overflow (beach material)
- Structural Fill

Table 11 summarizes the strength parameters used in the preliminary slope stability analyses.

**Table 11: Summary of Properties Used in Stability Analyses**

Component	Unit Weight (pcf)	Drained Strength		Undrained Strength (S_u/σ'_v)
		Cohesion (psf)	ϕ' (degrees)	
Foundation Materials	120	150	29	NA
Liner Interface Zone	120	0	26.5	NA
Cyclone Underflow	113	0	39	NA
Cyclone overflow	108	NA	NA	0.05
Structural Fill	120	0	29	NA

Notes:

pcf = pounds per cubic foot

psf = pounds per square foot

 ϕ' = Effective stress friction angle S_u/σ'_v = Residual undrained shear strength normalized by the effective overburden stress

NA = Not applicable

10.5.1 Foundation Materials

The foundation underlying the proposed TSF embankment has been characterized as an alluvial deposit comprised of predominately silty and clayey sands and gravels. Foundation material strength test results ranged from 28 to 32 degrees (effective friction angle). Based on the results of consolidated-undrained triaxial tests completed by Golder, an effective stress friction angle (ϕ') of 29 degrees and effective cohesion of 150 pounds per square foot were applied for the shear strength of the foundation materials.

10.5.2 Liner Interface Zone

The shear strength of the cycloned underflow tailings/geomembrane liner/liner bedding interface will be the controlling factor for possible sliding block-type failure surfaces along the base of the TSF embankment. The interface shear strength was evaluated in a direct shear test performed by Golder. Old (Quintana) tailings, and drain fill materials were placed in contact with a sample of textured, 80-mil HDPE geomembrane. The composition of the test sample is representative of the Phase 1 interface, when the old tailings will be utilized for liner bedding fill. The use of coarser, higher strength materials (crushed and screened native gravelly sand) is anticipated in later construction phases. The direct shear test indicated an interface friction of 26.5 degrees. This strength was assigned to the interface zone comprised of liner bedding, geomembrane and drainage materials.

10.5.3 Cyclone Underflow

The sloping, cyclone underflow embankment was considered to be fully drained and cohesionless because of the anticipated low phreatic-surface elevation and the high permeability of the cyclone underflow in the embankment, and the effect of the drainage control system. Mittal and Morganstern (1975) evaluated the ϕ' for cycloned copper tailings sands. The effects of particle crushing and sand dilatancy were most pronounced at stresses up to 40 psi, while at higher stresses ϕ' remained relatively constant and approximately equal to 34 degrees. Volpe (1975) reported ϕ' values for copper sands and slimes between 33 to 37 degrees. A consolidated-undrained triaxial test conducted by Golder on a



cyclone underflow sample indicated an internal friction angle of 40 degrees. An effective friction angle of 39 degrees was assumed in stability analyses.

10.5.4 Cyclone Overflow

Cyclone overflow in the impoundment is assumed to exhibit a drained response for most loading conditions except for the conditions occurring after static or seismic liquefaction. However; for conservatism, an undrained shear strength behavior was assumed for the cyclone overflow upstream of the embankment.

Undrained shear strength implicitly accounts for the effects of shear-induced pore pressures. Based on experience from similar mining projects and published data (Mittal and Morganstern 1975, Vick 1990), the peak undrained shear strength normalized by the effective overburden stress (S_u/σ'_{vo}) was estimated to be 0.20 for the impoundment area extending upstream from the embankment crest.

Residual undrained shear strengths were used in a static analysis to evaluate embankment stability following liquefaction in the beach area. Similar to that for peak shear strength, the residual undrained shear strength normalized by the effective overburden stress (S_u/σ'_{vo}) was estimated to be 0.05 for the tailings upstream of the cyclone underflow dam.

10.5.5 Structural Fill

Strength parameters for the starter dam structural fill are based on testing of representative materials recovered from the TSF borrow areas during the site exploration. Foundation material strength test results ranged from 28 to 32 degrees. The material was conservatively classified as cohesionless with a ϕ' of 29 degrees with a moist unit weight of 120 pcf.

10.5.6 Liner Material

The liner interface direct shear test was conducted with a sample of textured geomembrane. The use of un-textured geomembrane may be feasible, but testing of interface strength with un-textured liner has not been performed. To evaluate the potential for use of smooth liner, the interface strength was varied to find the interface friction angle required to meet the minimum pseudostatic FOS of 1.1. This analysis was performed for the block failure mode.

10.5.7 Fissured Clay Foundation Analysis

Stability analyses presented in Section 10.6 for the maximum embankment section address the embankment, the cyclone underflow beach and the liner interface zone on a maximum height embankment section. Additional sections corresponding to geology sections B-B' and D-D' were evaluated to determine the effects of clay foundation soils on the stability of the TSF.



Drill holes in the vicinity of these sections intercepted a clay layer that appears to be dipping to the eastward based on the first high plasticity clay intercepts identified during drilling. At several locations in the TSF expansion area, the top of this clay layer exhibited characteristics of a softened clay, with locally high moisture content and corrected SPT blow counts, in the range of 12 to 25, that were lower than those in overlying and underlying soils.

Stark and Eid (1977) state that fissured clays in first time slides (or first time slope failures) may exhibit a mobilized shear strength that is lower than the strength of fully softened clay, and suggest the use of the average of the fully softened and residual (large strain) clay strength in evaluating stability. Clay shear strength was estimated based on an empirical method presented by Mesri and Shahien (2003) that relates shear strength under varying normal stress to plasticity index. The method provides a non-linear shear strength envelope used to estimate the fully softened, residual and average strength of hard, fissured clays. The resulting shear strength versus normal stress envelope is used for clay strength input in the slope stability model.

The highest plasticity index (I_p) of 42 percent in Copper Flat samples, which was associated with a softened clay sample recovered from drill hole BH-18 at a depth of 43 feet, was assumed for estimating the strength of the high plasticity clay layers. In section B-B', clay interbedded clay and granular soil layers were modeled. In Section D-D', the clay layer was assumed to extend from the first high plasticity clay intercept to the base of the model section.

10.6 Stability Analysis Results

The results of stability analyses for static and pseudo-static loading conditions are summarized in Table 12.

Table 12: Calculated Slope Stability Factors of Safety

Failure Mode	Method	Static FOS (Global)	Pseudostatic FOS (Global)
Maximum Section Circular	Spencer	2.53	1.92
Maximum Section Block	Spencer	2.24	1.69
Maximum Section Circular, Post Liquefaction	Spencer	2.53	NA
Maximum Section Block, min required interface strength = 13.6 degrees	Spencer	1.53	1.1
Fissured Clay Section B-B' Circular	Spencer	1.56	1.12
Fissured Clay Section B-B' Block	Spencer	2.50	1.90
Fissured Clay Section D-D' Circular	Spencer	1.53	1.13
Fissured Clay Section D-D' Block	Spencer	2.48	1.87

Notes:

FOS = factor of safety

NA = not applicable

Min = minimum



The conservative assumptions applied in the stability analyses suggest that the Copper Flat TSF will be stable. All factors of safety meet or exceed the minimum NMDSB requirements of 1.1 and 1.5 for static and pseudostatic conditions. The residual strength analysis suggests if liquefaction of saturated tailings upstream of the dam occurs, the embankment will remain stable. The evaluation of the sensitivity of pseudostatic stability to the friction angle of the liner interface indicates a relatively low interface friction angle is required to maintain stability, and that the required interface strength is likely to be achievable with an un-textured geomembrane liner.



11.0 TAILINGS DAM FOUNDATION SETTLEMENT POTENTIAL

11.1 Analysis Approach

The TSF will consist of an earthen starter dam constructed to a height of approximately 50 feet with the remainder of the dam constructed with sand recovered from the cyclone plant. A geotechnical investigation was performed in the embankment footprint, which included standard penetration testing and sample collection from the surface to a depth of 50 feet. Drilling indicated that in general, the tailings embankment foundation consists primarily of alluvial deposits that include silt, sand and gravel, which are underlain by clay.

Representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Selected samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because settlement of the embankment will be adequately mitigated by continuous fill placement during ongoing embankment construction. Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc., for the analysis of settlement and consolidation under foundations and embankments.

A detailed description of the settlement potential investigation, settlement calculations and supporting information are contained in Appendix I.1. Drill holes and the location of cross-sections used to evaluate subsurface conditions are shown on Drawing 3. Drawings 5 and 7 present geologic cross sections B-B' and D-D', respectively, which were developed to evaluate settlement perpendicular to the dam axis. The cross-sections also include information derived from the former geotechnical study conducted on behalf of Quintana by Sargent Hauskins and Beckwith (SHB, 1980). Drill hole logs are contained in Appendix A.2.

A differential settlement and geomembrane strain analysis was subsequently conducted by Golder and is included in Appendix I.2. Cross sections were developed to intercept the various geologic materials underlying the TSF site. The engineering properties of the foundation materials were derived from the 1980 Sargent, Hauskins and Beckwith (SHB) geotechnical study, the geotechnical investigation conducted as part of the TSF design report and experience with similar foundation materials.

11.2 Settlement Potential Analysis Results

Laboratory consolidation testing was conducted on remolded specimens of the fine fraction of samples recovered from the embankment foundation. As such, the settlement prediction does not account for the presence of the coarse fraction in the foundation soils, and associated inter-particle contact and support of



foundation loads. Settlement predictions based on the laboratory consolidation tests are therefore conservative.

Results of the settlement potential analysis are shown graphically on geologic sections B-B' and D-D'. The maximum calculated settlement beneath the embankment is approximately 2.1 feet in the area of the maximum dam (and tailings beach) foundation loads. Settlement decreases at a relatively uniform rate as the weight of post-construction loading decreases towards the outer toe of the embankment.

Settlement prediction based on the laboratory consolidation testing of the fine fraction of foundation samples is conservative. SPT testing conducted during drilling showed the foundation strata to generally be very dense to hard. On the basis of SPT test results, actual post-construction consolidation settlement of less than 1 foot is anticipated.

Dam construction will be more or less continuous during the life of the facility. The effects of foundation settlement include the potential for the loss of dry freeboard for stormwater storage. The potential loss of freeboard can be mitigated by elevating the dam crest with managed/targeted placement of cyclone underflow sand.

The analyses did not indicate the potential for differential settlement that could impact the integrity of the TSF geomembrane liner. Sections B-B' and D-D' indicate predicted settlement varies uniformly across areas subject to changing foundation loads.

The impoundment underdrain will pass beneath the dam in a steel pipe placed in a ditch backfilled with concrete near section F-F' (Drawing 9). The settlement will not adversely impact the impoundment underdrain outlet pipe. There is adequate grade and elevation change along the outlet pipe alignment to accommodate predicted settlement.

A basalt outcrop identified by SHB (SHB, 1980) may lie beneath or in the vicinity of the impoundment underdrain pipe inlet near the upstream toe of the dam. The outcrop occurred in an area that was disturbed during Quintana dam construction activities, and was not observed during the recent site exploration. If the inlet to the underdrain pipe bears on basalt, local differential settlement could occur along the pipe alignment, which could induce stress on the outlet pipe. If, during construction, a basalt outcrop is identified at the location of the inlet, an alignment change may be warranted to avoid the pipe bearing on basalt.

It should be noted that the settlement potential investigation was performed for a previously completed design study, and evaluated an embankment geometry that differs from that presented in this report. The new embankment is higher and the depth of embankment fill overlying the foundation is greater for this 30,000 tons per day design; however, the original analyses assumed a higher, more conservative embankment moist unit weight of 130 pcf. Tailings testing completed after the settlement potential study



was conducted indicates a post embankment fill placement moist unit weight of approximately 113 pcf. The foundation loads imposed by the higher embankment fill, when corrected for the moist unit weight determined by laboratory testing, are lower than those used in the settlement potential analysis. Therefore, the results of the settlement investigation presented above are conservative relative to the current design. As part of future detailed engineering studies, settlement calculations will be updated for final design conditions; however, the conclusions are anticipated to be consistent with those presented herein.

The results of the differential settlement and geomembrane strain analysis indicates that, in general, settlement potential across the TSF is predicted to be limited. As such, the potential for tearing of the HDPE liner due to potential differential settlement within the entire area of the TSF is considered to be low. The maximum settlement is estimated to be 0.72 feet, while the maximum tensile strain on the HDPE liner due to differential settlement is estimated to be 0.02 percent. The allowable tensile strain on an 80 mil HDPE geomembrane liner is 10 percent and the predicted tensile strain is well within acceptable conditions. Therefore, Golder does not expect tearing of the HDPE liner due to differential settlement to be an issue.



12.0 USE OF THIS REPORT

This feasibility level design report has been prepared by Golder exclusively for the use of THEMAC and NMCC. No third-party engineer or consultant shall be entitled to rely on any of the information, conclusions, or opinions contained in this report without the written approval of Golder, THEMAC or NMCC.

The conclusions and recommendations in this report have been prepared in a manner consistent with the level of care and skill ordinarily exercised by engineering professionals practicing under similar conditions, subject to the time limits and financial and physical constraints imposed on or otherwise applicable to the work.



13.0 REFERENCES

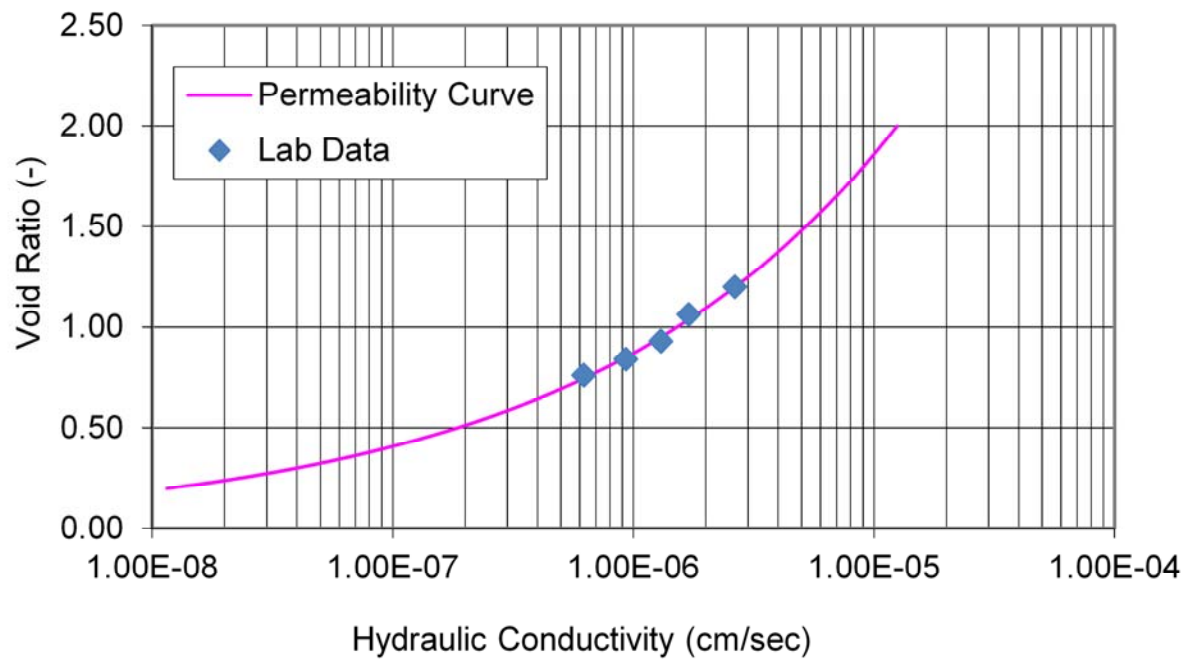
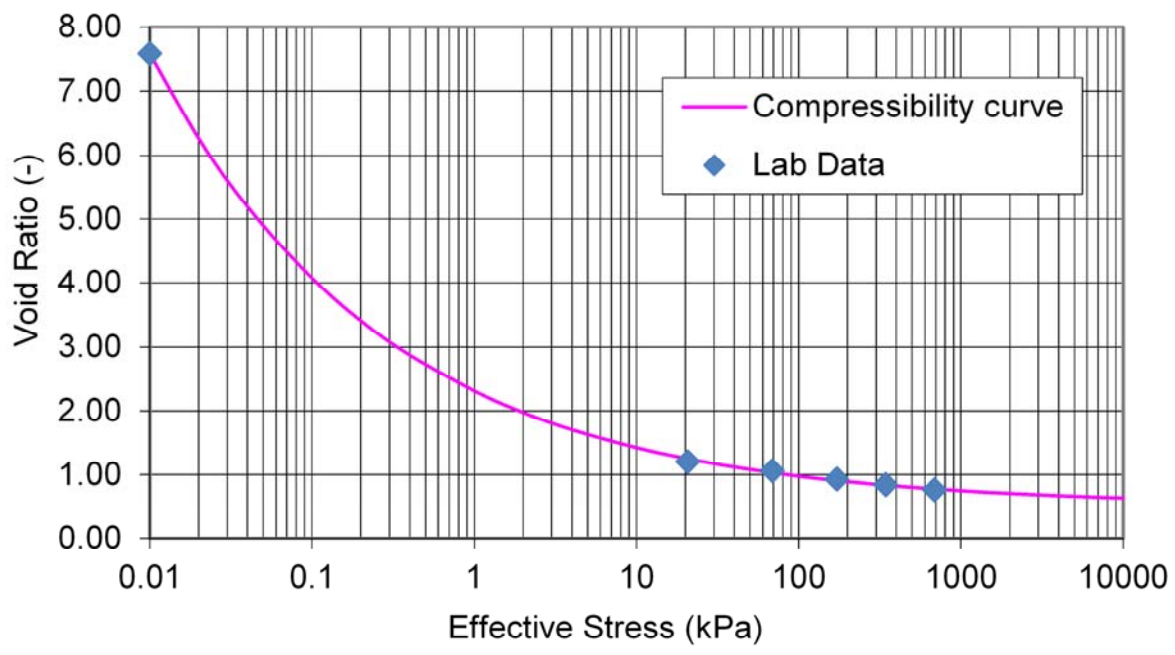
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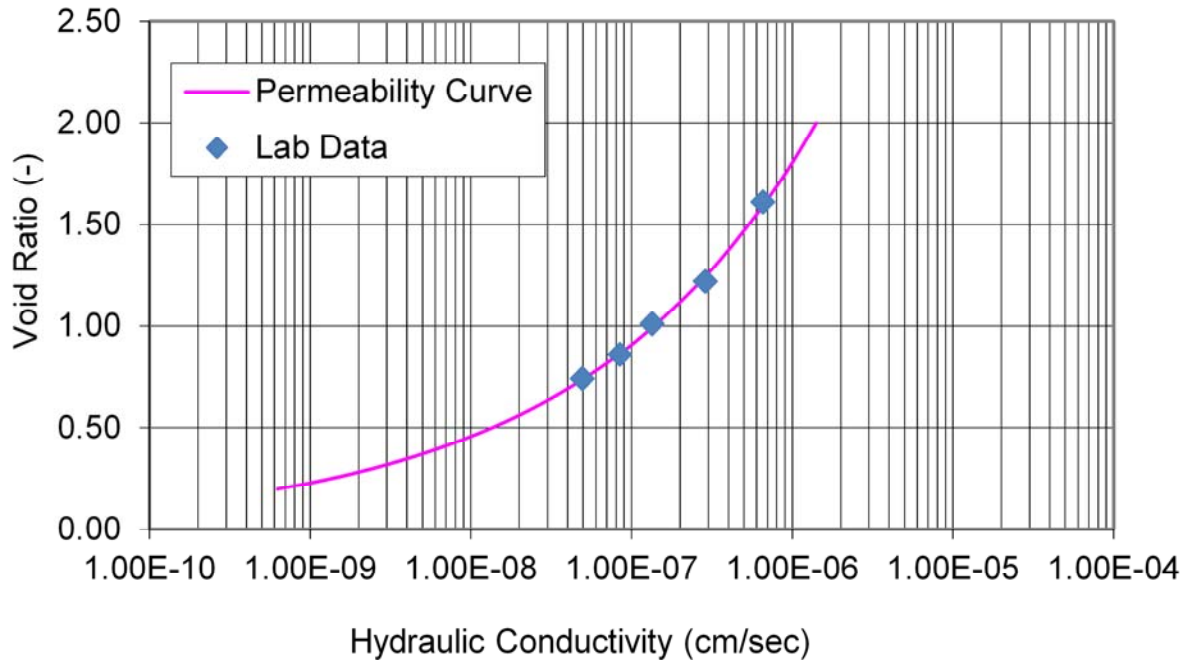
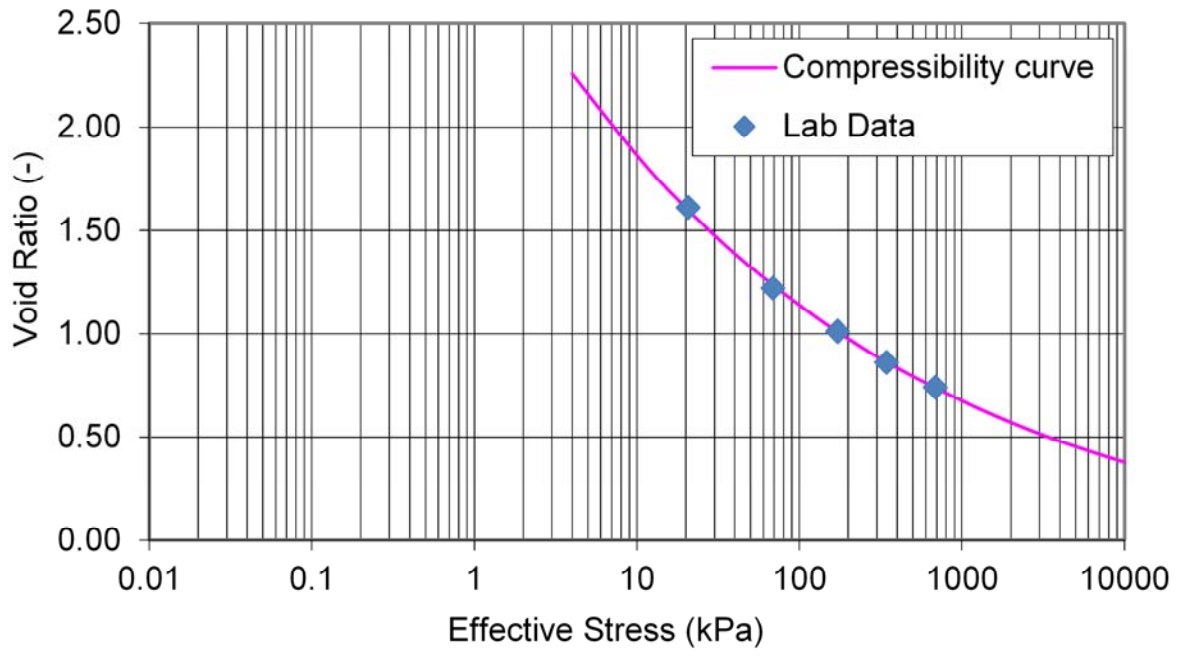
FIGURES

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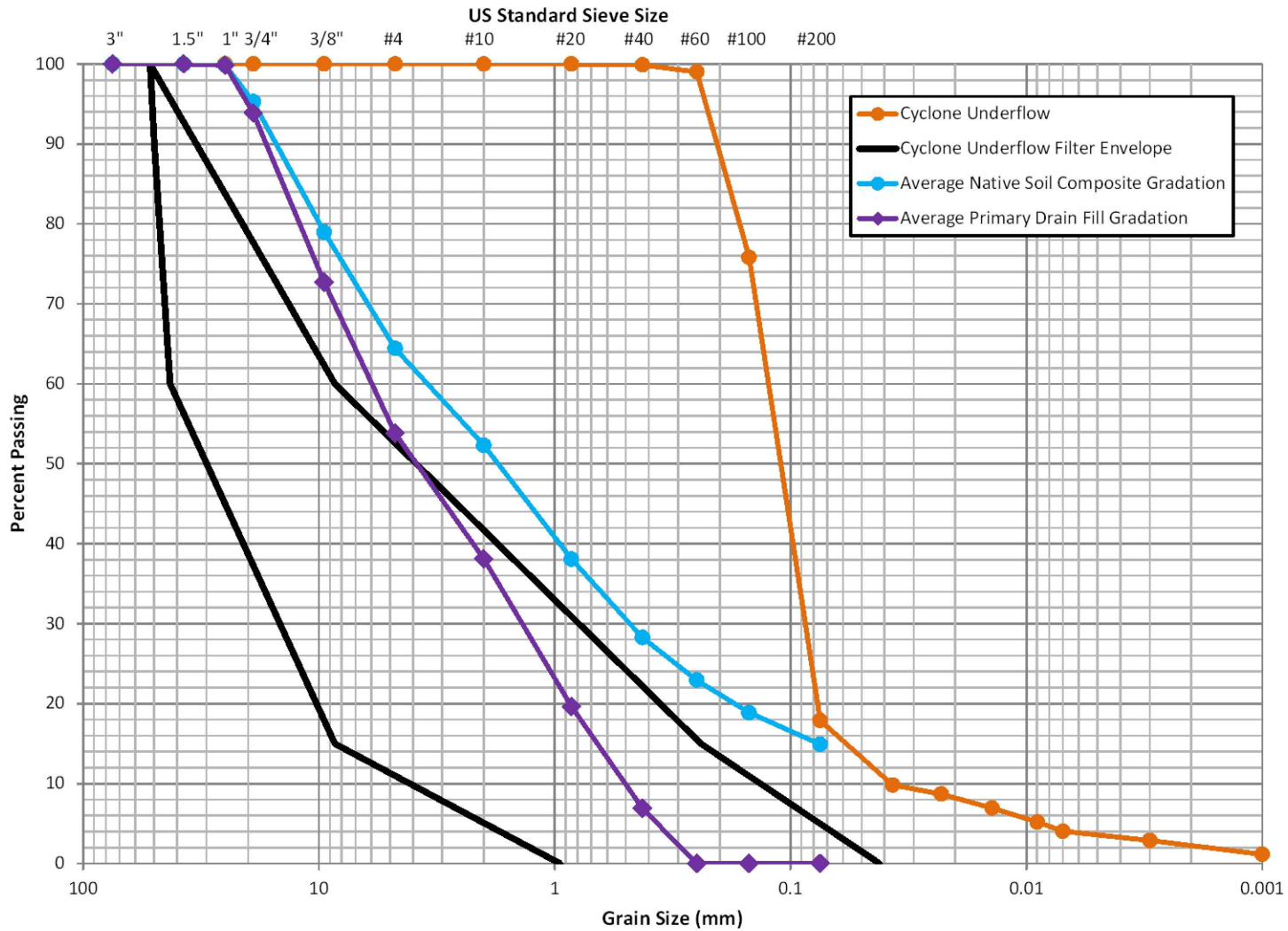





<p>PROJECT</p> <p>THEMAC RESOURCES</p> <p><small>Environmentally Responsible. Community-Minded. Local Opportunities.</small></p>	<p>NEW MEXICO COPPER CORPORATION</p>	<p>DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>																
<p>TITLE</p> <p>COMPRESSIBILITY AND PERMEABILITY VS. VOID RATIO THICKENED OVERFLOW- BEACH</p>																		
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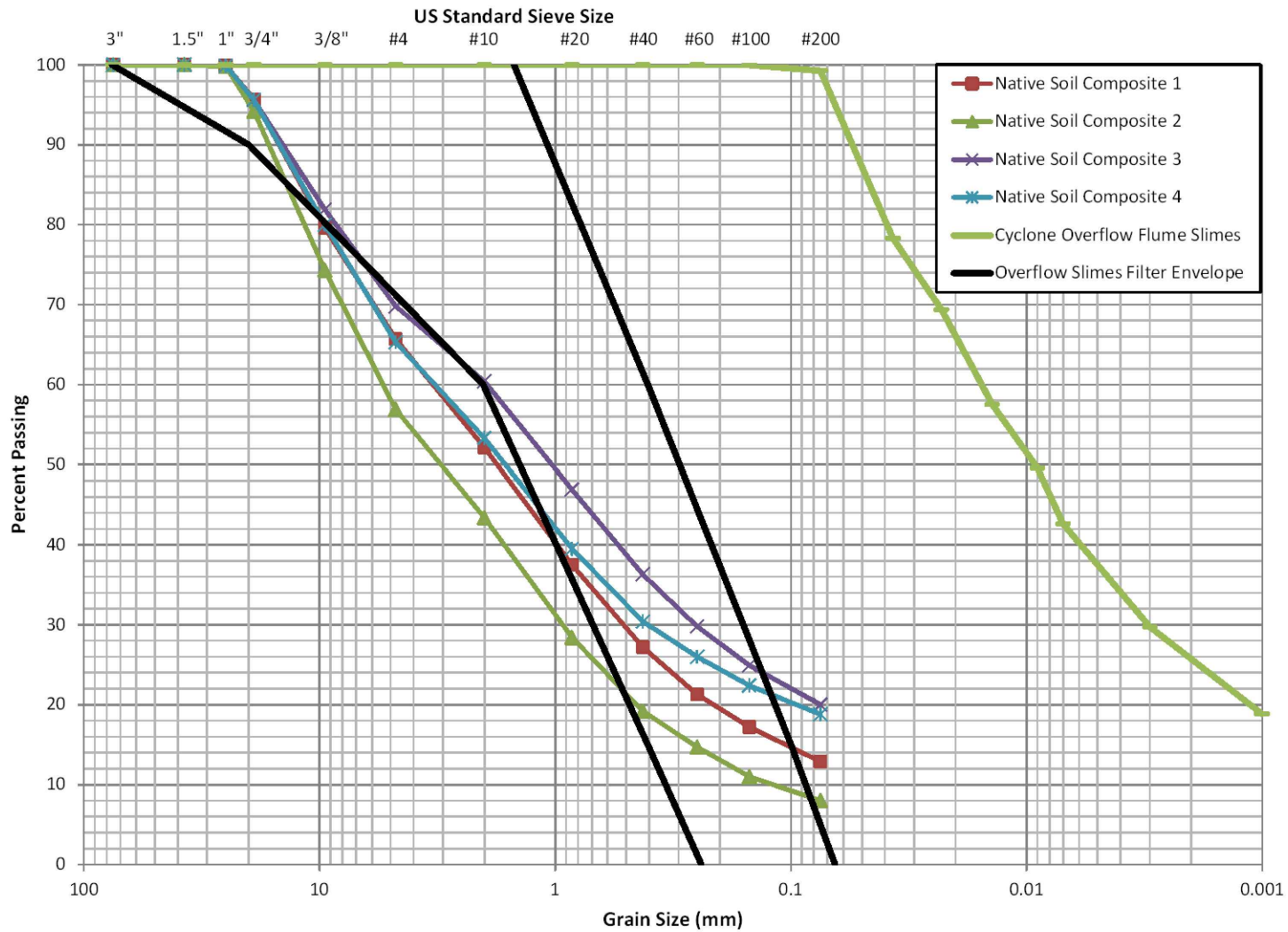
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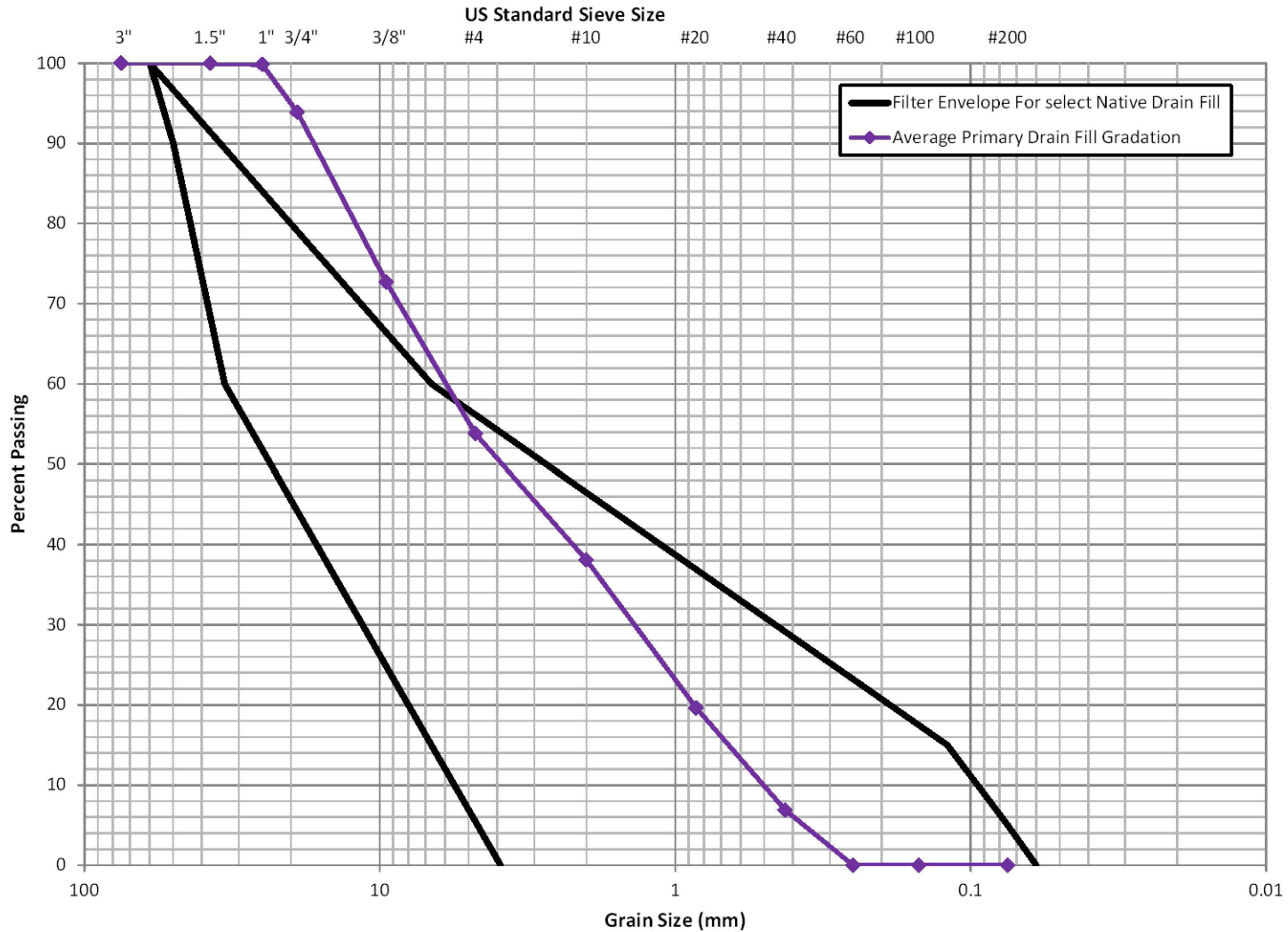
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<p>TITLE</p> <p>COMPRESSIBILITY AND PERMEABILITY VS VOID THICKENED OVERFLOW SLIMES</p>																				
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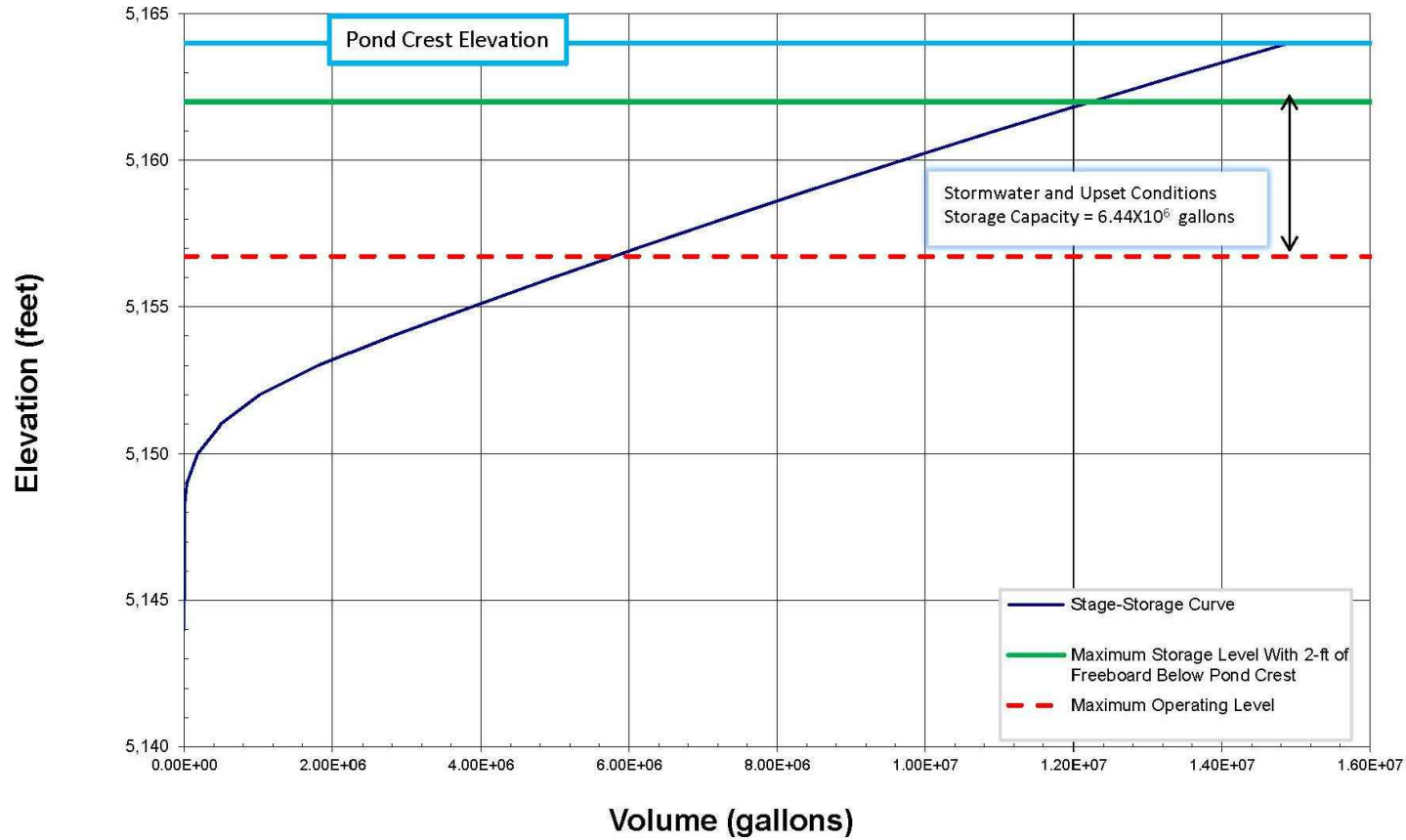
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DESIGN	DMW	2013-04-30	FIGURE		
CADD	JHR	2013-07-11	3		
CHECK	GM	2013-07-12			
REVIEW	DAK	2013-07-12			






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REVIEW	DAK	2013-07-12	



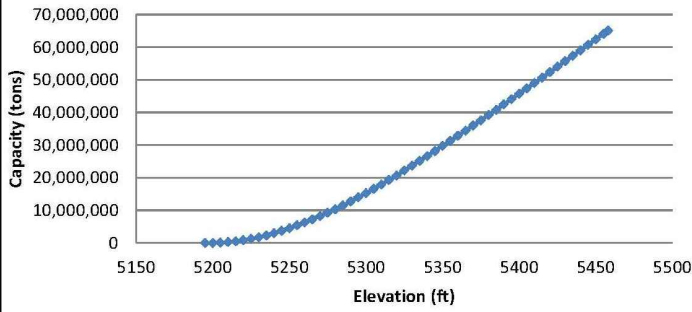
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CHECK	GM	2013-07-12			
REVIEW	DAK	2013-07-12			



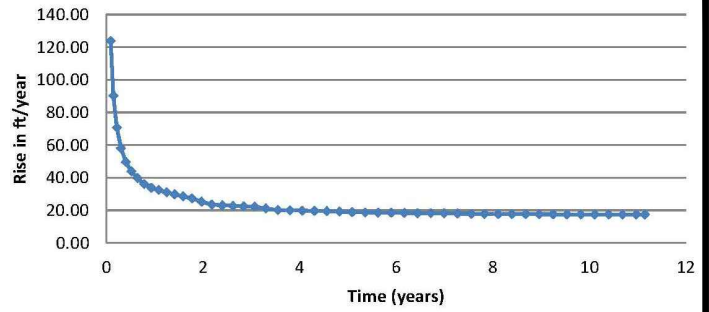
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REVIEW	DAK	2013-07-12			

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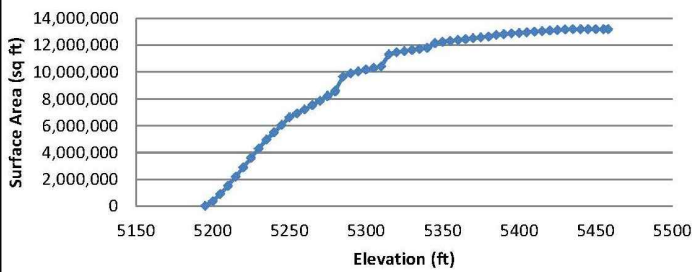
Impoundment Capacity vs. Elevation



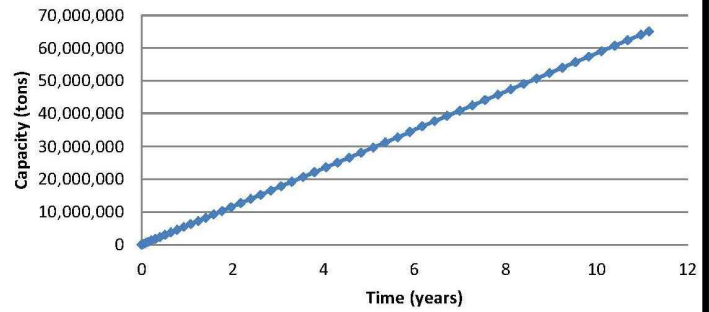
Rate of Rise (ft/yr)



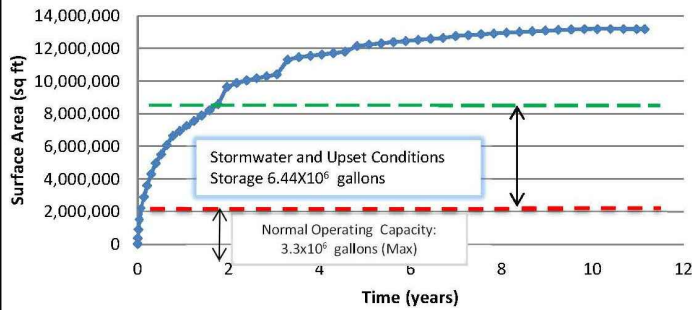
Impoundment Surface Area vs. Elevation



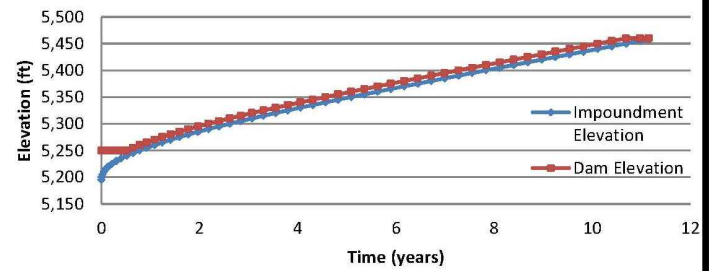
Impoundment Cumulative Storage





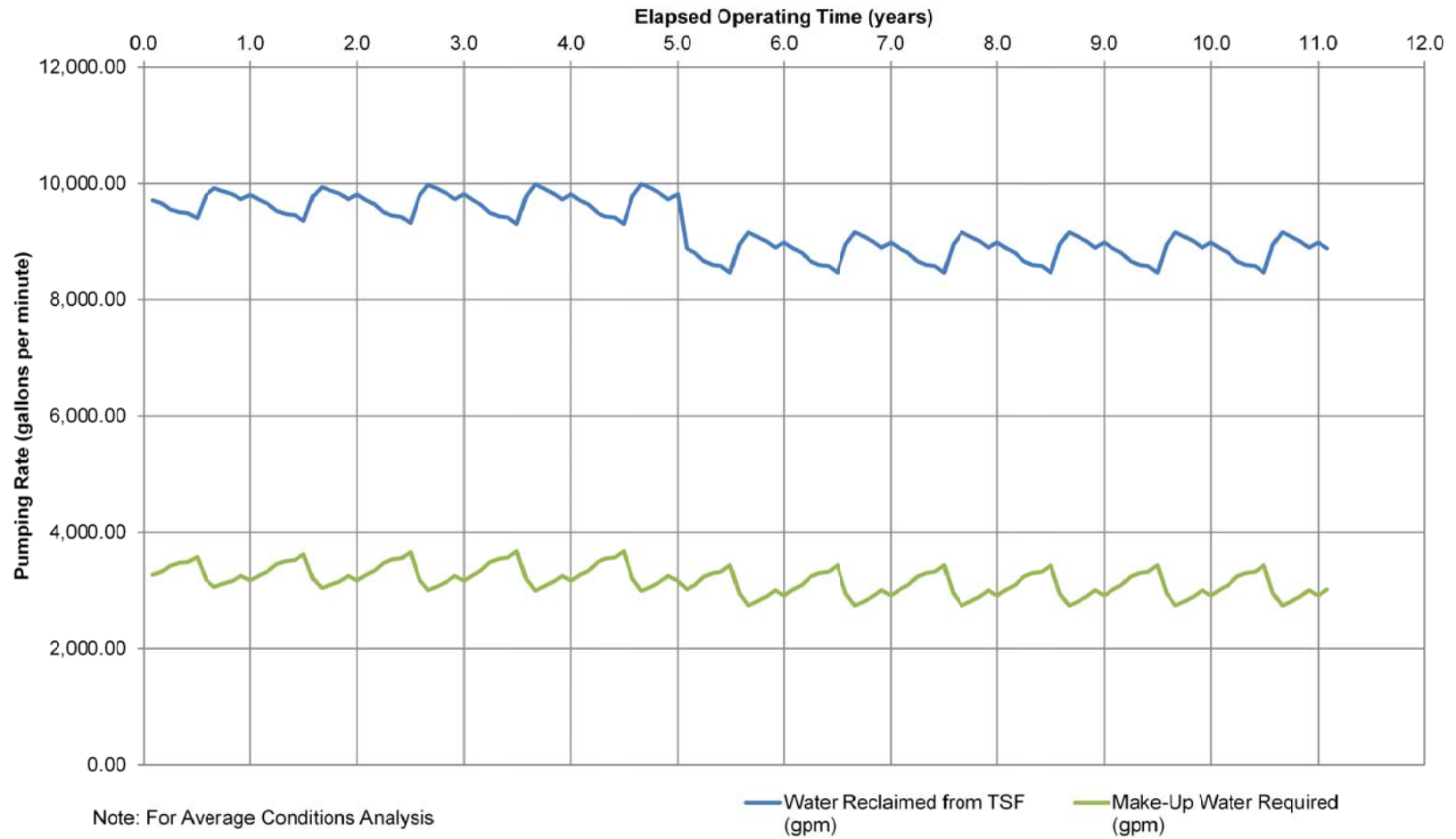
Impoundment Surface Area vs. Time






Dam & Impoundment Elevation vs. Time



 Environmentally Responsible. Community-Minded. Local Opportunities.	NEW MEXICO COPPER CORPORATION		DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO		
	TITLE TAILINGS STORAGE FACILITIES ELEVATION, SURFACE AREA, CAPACITY AND RATE OF RISE RELATIONSHIPS				
	PROJECT No.	153-1453	FILE No.	10392557K101_10232015	
	DESIGN	DMW	2013-04-30	SCALE	NTS
	CADD	JHR	2013-07-11	FIGURE	7
	CHECK	GM	2013-07-12		
REVIEW	DAK	2013-07-12			



				DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO	
TITLE <h2 style="text-align: center;">WATER BALANCE ANALYSIS RESULTS</h2>					
		PROJECT No. 153-1453 DESIGN DMW 2013-04-30 CADD JHR 2013-07-11 CHECK GM 2013-07-12 REVIEW DAK 2013-07-12	FILE No. 10392557K101_10232015 SCALE NTS FIGURE <h1 style="text-align: center;">8</h1>		

**APPENDIX A
SITE EXPLORATION**

**APPENDIX A.1
TEST PIT LOGS**



TEST PIT LOG: TP-1

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11977739.46 E: 867983.49

Date: 1/3/2013

Lithology:

Depth	USCS	Description
0 - 2 ft.	GM	sandy SILTY GRAVEL, medium, sub-angular, 35% coarse sub-rounded sand, 20% medium plasticity fines, 30% sub-angular cobbles (3-10"); pinkish gray (7.5YR 6/2), weak CaCO ₃ cementation; non-cohesive, dry, dense.
2 - 4 ft.	GP	GRAVEL, medium, and SAND fine to coarse, poorly graded, 10% non-plastic fines; 25% angular to sub-angular cobbles (3-10"), light brown (7.5YR 6/3), CaCO ₃ as cemented masses and disseminated; non-cohesive, dry, very dense.
4 - 8 ft.	SP	Friable/weathering rock (andesite); gravelly SAND, coarse, sub-rounded, 20% fine to medium sub-angular gravels; 10% non-plastic fines; 50% angular to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO ₃ coating rock fragments and fractures.



Samples:
None.

Special Notes:
Archaeologist present during excavation.
Pit located in road; top 2 ft. of original surface removed for road cut.
Native hillslope has ~70% surface rocks; with weathered/friable boulders exposed along road cut.
Test pit location immediately adjacent to existing waste rock disposal facility.



TEST PIT LOG: TP-2

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11976945.00 E: 869820.78

Date: 12/21/2012

Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 15% fine to coarse sub-rounded to sub-angular gravel, 40% low plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 4/3), blocky; cohesive, slightly moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded to sub-angular gravel, 35% low plasticity fines; 5% sub-rounded cobbles (3-6"), pinkish white (7.5YR 8/2), moderate CaCO ₃ cementation, blocky to platy; non-cohesive, dry, dense.
2 - 6 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded to sub-angular and SAND, fine to coarse, sub-rounded, 15% low to no plasticity fines; 20% sub-angular to sub-rounded cobbles and boulders (3-15"), pinkish gray (7.5YR 7/2), strong RXN with HCl, dry, dense.
6 - 7 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plasticity fines; 25% sub-angular cobbles and boulders (3-15"), light brown (7.5YR 6/3), strong RXN with HCl; non-cohesive, dry, very dense.
7 - 9 ft.	GW	Friable/weathering rock (andesite); GRAVEL, fine to coarse, angular, 10% coarse sand, 5% non-plastic fines; 50% angular cobbles and boulders (3- 15"), weak RXN with HCl.



Samples:

2-6 ft., bag
 6-7 ft., bag

Special Notes:

7 to 9 feet moderately strong rock, slightly weathered. Can slowly excavate with excavator (hard digging).



TEST PIT LOG: TP-3

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11977450.61 E: 869805.07

Date: 12/21/2012

Lithology:

Depth	USCS	Description
0 - 1 ft.	SM/ML	gravelly sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded to sub-angular gravels, 20% fine to coarse sub-rounded sand; 20% sub-angular cobbles and boulders (3-15");\, brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 20% low to non-plastic fines: 15% sub-angular cobbles, pinkish white (7.5YR 8/2), moderate CaCO ₃ cementation; non-cohesive, dry, dense.
2 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 25% sub-angular cobbles and boulders (3-20"), pink (7.5YR 7/3), strong RXN with HCl; non-cohesive, dry, dense.
7 - 9 ft.	GC	CLAYEY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% medium plasticity fines; 30% sub-angular cobbles and boulders (3-20"); brown (7.5YR 5/3), CaCO ₃ disseminated and as coatings on rocks; non-cohesive, dry, dense.
9 - 11 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 10% low to non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/4), CaCO ₃ as cemented masses, disseminated and coatings on rocks; non-cohesive, dry, very dense.



Samples:
BMI samples all layers (bag samples)

Special Notes:
Refusal at 11 ft., hit andesite. Fracturing andesite above.



TEST PIT LOG: TP-5

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11977574.94 E: 868955.59

Date: 1/3/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse sub-rounded, 20% fine to coarse sub-angular gravel, 25% low plasticity fines; 25% sub-angular cobbles (3-8"), yellowish brown (10YR 5/4), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 20% low plasticity fines; 5% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), CaCO ₃ as masses, weakly cemented in places, blocky; non-cohesive, dry, compact.
3 - 7 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 10% non-plastic fines; 5% sub-angular cobbles, light brown (7.5YR 6/3), moderate CaCO ₃ cementation in places (large plates excavated), clay fingering at 3 to 4 ft.; non-cohesive, dry, very dense.



Samples:

1-3 ft. bag, bucket
3-7 ft. bag, bucket
BMI samples (0-1ft, 1-3ft, 3-7ft)

Special Notes:

Refusal at 7 ft., hit bedrock (andesite).



TEST PIT LOG: TP-6

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11978206.72 E: 871778.25

Date: 1/4/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 25% fine to coarse poorly graded gravel, 20% non-plastic fines; 35% sub-rounded to sub-angular cobbles, brown (10YR 4/3), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 25% fine to coarse sub-rounded sand, 35% low plasticity fines; 15% sub-rounded to sub-angular cobbles (3-5"), very pale brown (10YR 7/3), blocky, CaCO ₃ as masses and disseminated; non-cohesive, dry, compact.
3 - 5 ft.	CL/GC	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded gravel, 25% fine to coarse sub-rounded sand; 5% sub-rounded to sub-angular cobbles (3-5"), strong brown (7.5YR 5/6), blocky, clay fingering, CaCO ₃ as masses and moderately cementation in places, large blocky plates excavated; non-cohesive, dry, stiff.
5 - 7 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-rounded, poorly graded, 20% fine to coarse sub-rounded sand, 15% medium plasticity fines; 15% sub-angular cobbles (3-7"), light brown (7.5YR 6/4); non-cohesive, dry, dense.
7 - 13 ft.	SM	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 25% med. plasticity fines; 45% sub-angular to angular cobbles and boulders (3-12"); light brown (7.5YR 6/4), CaCO ₃ coatings on rocks; non-cohesive, dry, very dense.



Samples:

- 1-3 ft. bag
- 3-5 ft. bag
- 5-7 ft. bag, bucket
- 7-13 ft. bag, bucket

Special Notes:

At 7 feet excavator broke through large boulder.



TEST PIT LOG: TP-7

Checked GM 1/29/2013

Client: THEMAC **Date:** 12/17/2012
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974576.20 E: 872338.77

Lithology:

Depth	USCS	Description
0 - 1.5 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% medium to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1.5 - 4 ft.	CH	CLAY and SAND, high plasticity, 45% medium to coarse poorly graded subrounded sand, 5% fine to coarse poorly graded gravels; reddish brown (5YR 4/4), blocky, some CaCO ₃ masses and disseminated; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 20% low plasticity fines; trace sub-angular cobbles (3-5"), pinkish gray (7.5YR 7/2), moderate CaCO ₃ cementation; non-cohesive, dry dense.
6 - 8 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, 10% non-plastic fines; 5% sub-angular cobbles (3-5"), pinkish gray (7.5YR 6/2), CaCO ₃ masses and disseminated; non-cohesive, dry, very dense.
8 - 10 ft.	GP	GRAVEL, fine, poorly graded, and SAND, poorly graded, fine to coarse, 5% non-plastic fines; trace cobbles (3-4"), brown (7.5YR 5/3), CaCO ₃ coating on rocks; non-cohesive, dry, very dense.
10 - 12 ft.	GP-GM	SILTY GRAVEL, fine to coarse, poorly graded, and SAND fine to coarse, sub-rounded, 10% low plasticity fines; 10% sub-angular cobbles (4-5"), brown (7.5YR 5/3), CaCO ₃ coating on rock fragments; non-cohesive, dry, dense.



Samples:
 0-1.5 ft. bag } 0-4ft. Bucket
 1.5-4 ft. bag }
 4-6 ft. bag
 6-10 ft. bag, bucket
 10-12 ft. bag, bucket
 BMI bag samples all layers

Special Notes:



TEST PIT LOG: TP-8

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974106.85 E: 872357.94

Date: 12/18/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy SILTY CLAY, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace cobbles (3-6"), brown (7.5YR 4/2), disseminated CaCO ₃ , platy; cohesive, moist, firm.
2 - 5 ft.	CI	sandy gravelly SILTY CLAY, high plasticity, 30% fine to coarse sand, 20% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), CaCO ₃ masses and weakly cemented in places, platy; cohesive, dry, stiff.
5 - 7 ft.	SC/CI	CLAYEY SAND, fine to coarse, sub-rounded, 45% medium plasticity fines, 5% fine to coarse poorly graded gravels; dark brown (7.5YR 3/4), CaCO ₃ masses, platy; cohesive, dry, stiff.
7 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse poorly graded gravels; trace cobbles (3-4"), reddish brown (5YR 5/4), moderate CaCO ₃ cementation, large plates excavated; cohesive, dry, hard.
13 - 16 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 5% sub-rounded cobbles (3-4"), light brown (7.5YR 6/3); CaCO ₃ coatings on rock fragments; non-cohesive, dry, compact to dense.



Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-7 ft. bag
- 7-13 ft. bag
- 13-16 ft. bag

Special Notes:



TEST PIT LOG: TP-9

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974362.19 E: 873288.54

Date: 12/17/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 45% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, firm.
2 - 6 ft.	SP	TAILINGS. poorly graded SAND, medium, sub-rounded, 5% non-plastic fines; pale yellow (2.5Y 7/3), no RXN with HCl; non-cohesive, dry, loose. Tailing thickness in pit is tapered east to west: lower depth 6 ft. (east) and 4 ft. (west).
6 - 8 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine poorly graded gravel, 15% medium plasticity fines; 5% sub-angular cobbles (3-4"); very pale brown (10YR 7/3), CaCO ₃ masses; non-cohesive, dry, dense.
8 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 5% angular cobbles (3-6"), pale brown (10YR 7/3), CaCO ₃ coatings on coarse fragments and disseminated; non-cohesive, dry, dense.
10 - 11 ft.	GM	sandy SILTY GRAVEL, poorly graded, fine, sub-rounded, 35% fine to coarse sand, 15% medium plasticity fines; 10% angular cobbles (3-4"), pinkish gray (7.5YR 6/2), moderate SiO ₂ /CaCO ₃ cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GP-GM	sandy SILTY GRAVEL, poorly graded, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 10% medium plasticity fines; pinkish gray (7.5YR 6/2), strong SiO ₂ /CaCO ₃ cementation; non-cohesive, dry, very dense.



Samples:
 6-8 ft. bag } 6-10 ft. bucket
 8-10 ft. bag }
 10-11 ft. bag } 10-14 ft. bucket
 11-14 ft. bag }

BMI bag samples: 6-8 ft., 8-10ft, 10-11 ft.

Special Notes:
 Reclaimed area on tailing dam.



TEST PIT LOG: TP-10

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974364.43 E: 873777.16

Date: 12/17/2012

Lithology:

Depth	USCS	Description
0 - 0.5 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% medium sub-angular gravel; trace sub-angular cobbles (3-6"), dark yellowish brown (10YR 4/4), friable, strong RXN with HCl; cohesive, slightly moist, soft.
0.5 - 3 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 40% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), dark brown (10YR 3/3), blocky, disseminated CaCO ₃ ; cohesive, dry, firm.
3 - 6 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 7/4), platy, no RXN with HCl; non-cohesive, dry, compact.
6 - 12 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 8/4), no RXN with HCl; non-cohesive, dry, loose.
12 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 25% medium plasticity fines, 25% fine to coarse sub-rounded gravels; 15% sub-rounded cobbles (3-10"), pale brown (10YR 6/3), CaCO ₃ masses and coatings on rock fragments, mixing with tailing at horizon contact; non-cohesive, dry, dense.



Samples:

- 0.5-3 ft. bag
- 3-6 ft. bag, bucket
- 6-12 ft. bag, bucket
- 12-13 ft. bag, bucket

Special Notes:

Reclaimed area on tailing dam.
Stop at 13 feet due to limit of backhoe, but appear to be on top of a layer of more gravels.



TEST PIT LOG: TP-11

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974375.04 E: 874235.34

Date: 12/17/2012

Lithology:

Depth	USCS	Description
0 - 0.83 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-10"), brown (10YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
0.83 - 5 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; light gray (2.5Y 7/2), platy, weak RXN with HCl; non-cohesive, dry, compact.
5 - 11 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; brownish yellow (10YR 6/8), no RXN with HCl; non-cohesive, dry, loose to compact.
11 - 13 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; grayish brown (2.5Y 5/2), no RXN with HCl; non-cohesive, dry, loose.



Samples:

- 0-1 ft. bag
- 1-5 ft. bag, bucket
- 5-11 ft. bag, bucket
- 11-13 ft. bag, bucket

Special Notes:

Reclaimed area on tailing dam.
Stop at 13 feet due to limit of backhoe.



TEST PIT LOG: TP-12

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974885.17 E: 875173.88

Date: 1/2/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	SILT and SAND, low plasticity, 45% poorly graded fine sand, 5% medium gravel; trace cobbles (4-6"), strong brown (7.5YR 4/6), blocky; cohesive, dry, firm.
1 - 3 ft.	CL	sandy SILTY CLAY, medium plasticity, 35% fine to coarse sub-rounded sand, 5% medium gravel; trace sub-rounded to sub angular cobbles (4-6"), pink (7.5 YR 7/3), clay fingering, CaCO ₃ masses and cemented in places, blocky; cohesive, dry, stiff.
3 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 40% sub-rounded to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO ₃ coatings on rock fragments; non-cohesive, dry, dense.
7 - 8 ft.	SM/GM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded; 20% low to medium plasticity fines; 10% cobbles (3-10"), pink (7.5YR 7/4), blocky, strong CaCO ₃ cementation; non-cohesive, dry, very dense.
8 - 11 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% non-plastic fines; 5% cobbles (3-10"), pale brown (10YR 6/3), weak to moderate SiO ₂ cementation, CaCO ₃ trace masses and disseminated; non-cohesive, dry, dense.
11 - 13 ft.	SM	SILTY SAND, fine to coarse, sub-rounded and GRAVEL, fine to coarse, sub-rounded; 20% low plasticity fines; trace cobbles (3-4"), yellowish brown (10YR 5/4), platy, moderate SiO ₂ /CaCO ₃ cementation, CaCO ₃ visible in pores and disseminated; non-cohesive, dry,
13 - 15 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% low to non-plastic fines; 15% cobbles (3-10"), yellowish brown (10YR 5/4), CaCO ₃ coatings on rock fragments; non-cohesive, dry, dense.



Samples:

BMI bag samples: 0-1 ft., 1-3 ft., 3-7 ft., 8-11 ft., 11-13 ft.

Special Notes:

Surface reworked by wind, small dunes around shrubs (*Flourensia cernua*, tarbush and *Prosopis glandulosa*, honey mesquite).



TEST PIT LOG: TP-13

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974438.98 E: 875447.89

Date: 1/2/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	SILT, non-plastic, 10% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1 - 3 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light yellowish brown (10YR 6/4), blocky, clay fingering, CaCO ₃ as masses and weakly cemented in places; cohesive, dry, soft.
3 - 5 ft.	CL	sandy SILTY CLAY, low to medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; light brown (7.5YR 6/4), blocky, CaCO ₃ as masses and disseminated; cohesive, dry, firm.
5 - 8 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light brown (7.5YR 6/4), strong angular blocky, CaCO ₃ along pores, weak RXN with HCl; cohesive, dry, stiff.
8 - 10 ft.	SM	SILTY SAND, fine to coarse, subrounded, and GRAVEL, fine to coarse, sub-rounded, 15% low plasticity fines; 15% sub-rounded to sub-angular cobbles and boulders (3-15"), brown (7.5YR 5/4), strong SiO ₂ /CaCO ₃ cementation, strong RXN with HCl; non-cohesive, dry, dense.
10 - 18 ft.	GW	sandy GRAVEL, fine to coarse, sub-rounded, 25% fine to coarse sub-rounded sand, 10% non-plastic fines; 30% sub-rounded to sub-angular cobbles and boulders (3-20"), dark yellowish brown (10YR 4/4), weak RXN with HCl; non-cohesive, dry, very, dense.



Samples:

- 5-8 ft. bag, bucket
- 5-10 ft. bag, bucket
- 10-18 ft. bag, bucket

Special Notes:



TEST PIT LOG: TP-14

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974366.80 E: 874917.98

Date: 1/2/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	CI	gravelly sandy SILTY CLAY, high plasticity, 20% fine to coarse sub-angular gravel, 15% fine to coarse sub-rounded sand; 20% sub-angular cobbles (3-12"); dark reddish brown (5YR 3/4), blocky, no RXN with HCl; cohesive, dry, firm.
1 - 4 ft.	CL	gravelly sandy SILTY CLAY, moderate plasticity, 25% fine to coarse sub-angular gravel, 25% fine to coarse sub-rounded sand; 15% sub-angular cobbles (3-12"); pink (7.5YR 7/3), blocky, CaCO ₃ masses and weak cementation in places, strong RXN with HCl clay fingering; cohesive, dry, firm.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 15% fine to coarse poorly graded gravels; trace cobbles (3"), pinkish white (7.5YR 8/2), strong CaCO ₃ cementation (large plates excavated); non-cohesive, dry, dense.
7 - 12 ft.	ML	sandy SILT, non-plastic, 20% fine to medium poorly graded sand, trace fine poorly graded gravels; brown (7.5YR 5/4), platy, CaCO ₃ lining pores and some masses, weak RXN with HCl; cohesive, dry, soft.
12 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 15% non-plastic fines; 15% sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), strong SiO ₂ /CaCO ₃ cementation, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16.5 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sand, 15% non-plastic fines; trace sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), weak SiO ₂ cementation, CaCO ₃ coatings on rocks, strong RXN with HCl; non-cohesive, dry, dense.



Samples:

- 0-1 ft. bag
- 1-4 ft. bag
- 4-7 ft. bag
- 7-12 ft. bag
- 12-14 ft. bag
- 14-16.5 ft. bag

Special Notes:

Offset pit location approximately 25 feet to the west to keep disturbance on tracked road.



TEST PIT LOG: TP-15

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973832.93 E: 874871.54

Date: 12/20/2012

Lithology:

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse sub-angular gravels; 5% sub-angular cobbles (3-10"), pink (7.5 YR 7/4), blocky, strong RXN with HCl; cohesive, dry, firm.
4 - 8 ft.	CI	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to medium poorly graded gravel; light reddish brown (5YR 6/4), angular blocky (breaking to fine aggregates), CaCO ₃ nodules and masses; cohesive, dry, firm.
8 - 10 ft.	CH	sandy CLAY, high plasticity, 15% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/4), angular blocky (breaking to gravel sized aggregates), weak RXN with HCl, clay pressure faces; cohesive, moist, firm.
10 - 20 ft.	CH	CLAY, high plasticity, 5% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/3), angular blocky (rock structure), weak RXN with HCl, clay pressure faces; cohesive, moist, stiff.



Samples:

8-10 ft. bag, bucket
 10-20 ft. bag, bucket

Special Notes:

Excavator leaves slick sidewalls at 8+ feet. Clays formed in place (not illuvial) from weathering primary minerals.



TEST PIT LOG: TP-16

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973732.56 E: 875481.72

Date: 12/20/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; 10% sub-angular cobbles (3-6"), brown (7.5YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 4 ft.	ML	sandy CLAYEY SILT, low plasticity, 20% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; trace sub-angular cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, stiff.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 20% fine poorly graded sub-angular gravels; brown (7.5YR 5/3), friable, CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
7 - 10 ft.	ML	sandy SILT, non-plastic, 30% fine to coarse sub-angular sand, 5% fine poorly graded sub-angular gravels; light brown (7.5YR 6/3), blocky, CaCO ₃ as masses and disseminated, strong RXN with HCl, thin layer (<1ft) of moderate cementation; non-cohesive, dry, hard.
10 - 17 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 20% sub-rounded cobbles (3-12"); brown (7.5YR 5/3), weak to strong CaCO ₃ cementation (stratified), weak to strong RXN with HCl; non-cohesive, dry, very dense.



Samples:

- 0-2 ft. bag
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-10 ft. bag, bucket
- BMI bag samples same as above

Special Notes:

10 to 17 foot interval has varying degrees of CaCO₃ cementation (none to strong), but grouped together due to particle size similarities.



TEST PIT LOG: TP-17

Checked GM 1/29/2013

Client: THEMAC

Date: 12/18/2012

Project: Copper Flat

Project No.: 103-92557

Location: Sierra County, NM

NAD 83: N: 11973205.01 E: 873937.67

Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	sandy SILT, low plasticity, 40% fine to coarse sub-rounded sand, trace fine poorly graded gravels; trace sub-angular cobbles (3-6"), yellowish brown (10YR 5/4), platy, weak RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	CH	sandy CLAY, high plasticity, 35% fine to coarse poorly graded sand, 5% fine to coarse poorly graded gravels; trace cobbles (3"), reddish brown (5YR 4/4), blocky, disseminated CaCO ₃ and masses, strong RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), pink (7.5YR 7/3), blocky, moderate CaCO ₃ cementation and masses, strong RXN with HCl; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, poorly graded, 25% low plasticity fines, 25% fine poorly graded gravels; trace cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO ₃ masses and coating rock fragments, strong RXN with HCl; cohesive, dry, firm.
6 - 14 ft.	GW	sandy GRAVEL, fine to coarse, poorly graded, 40% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), stratified, thickly bedded, some weak CaCO ₃ cementation at 8 feet moderate cementation at 14 feet, CaCO ₃ masses, strong RXN with HCl; non-cohesive, dry, dense



Samples:

- 0-2 ft. bag
- 2-4 ft. bag
- 4-6 ft. bag
- 6-14 ft. bag
- BMI bag samples: 0-2 ft., 2-4 ft., 4-6 ft., 6-10 ft.

Special Notes:

Disturbed surface, pit located in depression on tailing dam (cow lay-down area). Salt cedar (*Tamarix chinensis*) and seep willow (*Baccharis salicina*) stand.



TEST PIT LOG: TP-18

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973182.90 E: 874892.73

Date: 12/20/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	GM/SM	sandy SILTY GRAVEL, poorly graded, fine to medium, sub-angular, 25% fine to medium poorly graded sub-rounded sand, 45% low plasticity fines; trace sub-angular cobbles (3-4"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; trace sub-angular cobbles (3-4"), light brown (7.5YR 6/3), blocky, moderate CaCO ₃ cementation, strong RXN with HCl; non-cohesive, dry, dense.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 30% low plasticity fines; pink (7.5YR 7/3), blocky, moderate to weak CaCO ₃ cementation in places, disseminated, and coatings on rocks; non-cohesive, dry, dense.
5 - 7 ft.	SM/GM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; reddish brown (5YR 5/3), large plates excavated, moderate SiO ₂ cementation, strong reaction with HCl; non-cohesive, dry, dense.
7 - 9 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded gravel, 30% low plasticity fines; reddish brown (5YR 5/4), large plates excavated, strong SiO ₂ cementation, weak reaction with HCl; non-cohesive, dry, very dense.
9 - 15 ft.	SP	SAND, poorly graded, medium, sub-rounded, 5% fine poorly graded gravel, 5% non-plastic fines; reddish brown (5YR 5/4), large plates excavated, moderate SiO ₂ cementation, weak reaction with HCl; non-cohesive, dry, very dense.



Samples:

7-9 ft. bag, bucket
 9-15 ft. bag, bucket

Special Notes:



TEST PIT LOG: TP-19

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973045.18 E: 875451.64

Date: 12/19/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	GC	Disturbed/FILL. sandy CLAYEY GRAVEL, fine to coarse, sub-rounded, 45% high plasticity fines, 20% fine to coarse sub-rounded sand; 15% sub-rounded cobbles (3-6"), reddish brown (5YR 4/4), blocky, no RXN with HCl; cohesive, moist, firm.
2 - 3 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, poorly graded, sub-rounded, 40% high plasticity fines, 30% fine to coarse sub-rounded sand; 5% sub-rounded cobbles (3-6"), yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, dry, hard.
3 - 5 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% non-plastic fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), strong CaCO ₃ cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, subangular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO ₃ as masses, disseminated and weak cementation in places; non-cohesive, dry, dense.
10 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% medium poorly graded gravel, 25% low to medium plasticity fines; trace cobbles (3-10"), white (7.5YR 8/1), blocky, CaCO ₃ as masses, disseminated and moderate cementation in places; non-cohesive, dry, dense.
11 - 14 ft.	GM-GC	sandy GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 25% low to medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO ₃ as masses, disseminated and strong cementation in places; non-cohesive, dry, dense.



Samples:

None.

Special Notes:

stratified gravels and sands, thickly bedded, from 5 to 14 feet.



TEST PIT LOG: TP-20

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972549.21 E: 875734.91

Date: 12/19/2012

Lithology:

Depth	USCS	Description
0 - 0.5 ft.	ML	sandy SILT, non-plastic, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; brown (7.5YR 4/2), friable, no RXN with HCl; cohesive, dry, soft.
0.5 - 2 ft.	CH	CLAY and SAND, high plasticity, 40% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC-SP	gravelly CLAYEY SAND, fine to coarse, poorly graded, 30% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 10% sub-rounded cobbles (3-7"), brown (7.5YR 5/4), CaCO ₃ as masses and disseminated; strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% medium plasticity fines, 15% fine to coarse poorly graded sub-rounded gravels; trace sub-rounded cobbles (3"), white (7.5YR 8/1), blocky, weakly cemented (CaCO ₃); non-cohesive, dry, very dense.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO ₃ as masses and disseminated; non-cohesive, dry, dense.
7 - 11 ft.	GP	GRAVEL, fine to coarse, poorly graded, and SAND, fine to coarse, sub-round SAND, 5% non-plastic fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), weakly cemented (CaCO ₃); non-cohesive, dry, dense.
11 - 18.5 ft.	SW-SM	SILTY SAND fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 15% non-plastic fines; 10% sub-rounded cobbles (3-6"), grayish brown (10YR 5/2), blocky, CaCO ₃ as masses, disseminated and coatings on rock fragments; non-cohesive, dry, dense to compact.



Samples:
 0-2 ft. bag } 0-4 ft. bucket
 2-4 ft. bag }
 4-5 ft. bag } 4-7 ft. bucket
 5-7 ft. bag }
 7-11 ft. bag, bucket
 11-18.5 ft. bag, bucket

Special Notes:
 Surface disturbed.



TEST PIT LOG: TP-21

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972274.72 E: 875755.12

Date: 12/19/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 45% fine to coarse sub-rounded sand, 5% coarse poorly graded sub-angular, gravels; 5% sub-angular cobbles (3-4"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC/SM	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% medium plasticity fines, 30% medium poorly graded sub-angular gravels; trace cobbles (3"), white (7.5YR 8/1), blocky, weak CaCO ₃ cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 25% fine to medium poorly graded sub-angular gravels; trace cobbles (3"), brown (7.5YR 5/3), blocky, CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded sub-angular gravels, 25% medium plasticity fines; 5% sub-rounded cobbles (3-5"), brown (7.5YR 4/3), disseminated CaCO ₃ , masses and coatings on rock fragments, strong RXN with HCl; cohesive, dry, stiff.
7 - 11 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 20% medium plasticity fines; 10% sub-angular cobbles (3-6"), brown (7.5YR 5/4), weak RXN with HCl, moderate SiO ₂ cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, sub-angular, 40% fine to coarse sub-rounded sand, 15% low plasticity fines; 5% sub-rounded cobbles (3-6"); brown (7.5YR 5/4), weak RXN with HCl, weak SiO ₂ cementation; non-cohesive, dry, dense.
14 - 18 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 20% low plasticity fines; trace cobbles (3-4"); light brown (7.5YR 6/4), large plates excavated, weak RXN with HCl, weak SiO ₂ cementation; non-cohesive, dry, dense.



Samples:

- 7-11 ft. bag, bucket
- 11-14 ft. bag, bucket
- 14-18 ft. bag, bucket
- BMI bag samples: 7-11 ft., 11-14 ft., 14-18 ft.

Special Notes:

CaCO₃ masses and cementation confined to upper layers (2-7 ft.). Disseminated CaCO₃ and coatings on coarse fragments at 7+ feet. SiO₂ cementation/conglomerate at 7-18 feet.



TEST PIT LOG: TP-22

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972597.58 E: 875051.00

Date: 12/20/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse sub-angular gravel; 5% cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% low to medium plasticity fines, 25% fine to coarse sub-angular gravel; 5% cobbles (3-5"), pinkish white (7.5YR 8/2), weak CaCO ₃ cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 10% non-plastic fines; 10% cobbles (3-12"), light brown (7.5YR 6/3), CaCO ₃ as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
5 - 8 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sub-rounded sand, 10% non-plastic fines; 20% cobbles and boulders (3-15"), brown (7.5YR 5/4), CaCO ₃ as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
8 - 11 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 25% medium plasticity fines; 20% cobbles and boulders (3-15"), reddish brown (5YR 4/4), CaCO ₃ as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
11 - 13 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 25% medium plasticity fines; trace cobbles (3-4"), reddish brown (5YR 5/4), CaCO ₃ as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, 10% fine to medium poorly graded gravels, 10% low plasticity fines; yellowish red (5YR 5/6), weak RXN to HCl, moderate cementation (SiO ₂), large plates excavated; non-cohesive, dry, very dense.



Samples:

None.

Special Notes:

Stratified gravels at 5 to 11 feet, thickly bedded.
Disturbed surface; A horizon has been removed.



TEST PIT LOG: TP-23

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972621.41 E: 874709.99

Date: 12/20/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 35% fine to medium poorly graded sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; trace sub-angular cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	ML	CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/3), blocky, weak CaCO ₃ cementation, strong RXN with HCl; cohesive, dry, very stiff.
3 - 5 ft.	ML-SM	CLAYEY SILT and SAND, low to medium plasticity, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; light brown (7.5YR 6/3), blocky, CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 8 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low to non-plastic fines, 5% fine to medium poorly sub-angular graded gravel; pink (7.5YR 7/4), moderate CaCO ₃ cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
8 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 25% low plasticity fines, 25% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/4), strong CaCO ₃ cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.
11 - 12 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 30% low plasticity fines, 30% fine to coarse sub-rounded sand; 5% cobbles (3-5"), brown (7.5YR 5/2), strong SiO ₂ /CaCO ₃ cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.



Samples:

None.

Special Notes:

very hard digging at 10 feet. Refusal at 12 feet.



TEST PIT LOG: TP-24

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972670.75 E: 873699.23

Date: 12/18/2012

Lithology:

Depth	USCS	Description
0 - 3 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse poorly graded sub-rounded gravel; 20% cobbles (4-10"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded sand, 30% medium plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; 5% sub-angular cobbles (3-5"); brown (7.5YR 5/4), blocky, CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
5 - 10 ft.	SM/SC	SILTY SAND, fine to coarse, well graded, sub-rounded, 25% medium plasticity fines, 10% fine to medium poorly graded sub-angular gravel; trace cobbles (3-5"); light brown (7.5YR 6/3), large blocky plates excavated, CaCO ₃ as masses and weakly cemented (SiO ₂ /CaCO ₃) in places, strong RXN with HCl; non-cohesive, dry, dense.
10 - 14 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; 15% sub-angular cobbles; brown (7.5YR 5/3), CaCO ₃ coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16 ft.	GP-GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, 30% fine to coarse well graded sand, 10% low plasticity fines; 20% sub-angular cobbles (3-10"), brown (7.5YR 5/3), CaCO ₃ coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.



- Samples:**
 0-3 ft. bag
 3-5 ft. bag, bucket
 5-10 ft. bag, bucket
 10-14 ft. bag, bucket
 14-16 ft. bag, bucket
 BMI samples all layers (bag samples)

Special Notes:
Disturbed surface.



TEST PIT LOG: TP-25

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972408.78 E: 872794.31

Date: 12/13/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded sub-rounded gravel, 20% medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky; cohesive, dry, firm.
2 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravel, 10% low plasticity fines; 20% cobbles and boulders (3-15"), light brown (7.5YR 5/4), CaCO ₃ as masses and disseminated; non-cohesive, dry,
5 - 6 ft.	GP	sandy GRAVEL, fine to coarse, poorly graded, sub-angular, 25% fine to coarse sub-rounded graded sand, 5% non-plastic fines; 15% cobbles and boulders (3-12"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO ₂ /CaCO ₃ cementation (conglomerate); non-cohesive, dry, very dense.
6 - 7 ft.	GP	sandy GRAVEL, fine to coarse, poorly grades, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 15% cobbles (3-10"), light brown (7.5YR 6/3), large blocky plates excavated, moderate SiO ₂ /CaCO ₃ cementation (conglomerate); non-cohesive, dry, very dense.



Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-6 ft. bag
- 6-7 ft. bag

BMI sample: 2-5 ft.

Special Notes:

Pit located in old borrow area. Hard to dig with backhoe at 5+ feet.



TEST PIT LOG: TP-26

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973262.64 E: 872782.60

Date: 12/13/2012

Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	FILL. gravelly SILTY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse sub-rounded gravel; 5% sub-angular cobbles (3-4"), brown (10YR 5/3), friable; cohesive, dry, soft.
1 - 3 ft.	CL/SM	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 20% fine to coarse sub-rounded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), blocky, CaCO ₃ as masses and disseminated; cohesive, dry, stiff.
3 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-rounded graded gravel, 15% low plasticity fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), large blocky plates excavated, moderate SiO ₂ /CaCO ₃ cementation (conglomerate); non-cohesive, dry, very dense.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-angular gravel, 15% low plasticity fines; 5% sub-angular cobbles (3-5"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO ₂ /CaCO ₃ cementation (conglomerate); non-cohesive, dry, very dense.



Samples:

- 0-3 ft. bag
- 3-4 ft. bag
- 4-5 ft. bag

Special Notes:

Pit located in old borrow area. Hard to dig with backhoe; refusal at 5 feet.



TEST PIT LOG: TP-27

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11972823.18 E: 871169.18

Date: 12/19/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-5"), dark brown (10YR 3/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse poorly graded sub-angular gravel; 10% sub-angular cobbles and boulders (3-20"); brown (7.5YR 4/3), blocky, CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
3 - 7 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-angular, 45% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% sub-angular cobbles (3-10"), pinkish gray (7.5YR 7/2), CaCO ₃ coatings on rock fragments, weak RXN with HCl, moderate SiO ₂ cementation at 5 feet; non-cohesive, dry, dense.
7 - 13 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), pinkish gray (7.5YR 6/2), moderate SiO ₂ cementation, CaCO ₃ coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.
13 - 14 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 10% sub-angular cobbles (3-10"), pinkish gray (7.5YR 6/2), strong SiO ₂ cementation (conglomerate), CaCO ₃ coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.



Samples:

- 0-2 ft. bag, bucket
- 2-3 ft. bag, bucket
- 3-7 ft. bag, bucket
- 7-13 ft. bag, bucket
- 13-14 ft. bag
- BMI samples all layers (bag samples)

Special Notes:

Hard digging due to oversize at 7 feet and cemented conglomerate at 13 feet.



TEST PIT LOG: TP-28

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11973129.48 E: 871528.89

Date: 1/3/2013

Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-rounded gravel; 10% sub-angular cobbles (3-8"), brown (7.5YR 4/2), blocky, weak RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM/CL	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 30% fine to coarse sub-rounded gravel; trace sub-angular cobbles (3-8"), pinkish gray (7.5YR 6/2), blocky, weak CaCO ₃ cementation in places and masses strong RXN with HCl; cohesive, dry, firm.
4 - 6 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-rounded and SAND, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; trace cobbles (3-8"), brown (7.5YR 5/2), CaCO ₃ coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
6 - 9 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), light brown (7.5YR 6/4), moderate SiO ₂ cementation, very weak RXN with HCl; non-cohesive, dry, very
9 - 14.5 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 20% sub-angular cobbles (3-8"), light brown (7.5YR 6/4), strong SiO ₂ cementation (conglomerate), very weak RXN with HCl; non-cohesive, dry, very dense.



Samples:
None.

Special Notes:
Difficult to excavate at 9 feet. Archaeologist present during excavation.



TEST PIT LOG: TP-29

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974098.42 E: 871178.07

Date: 12/18/2012

Lithology:

Depth	USCS	Description
0 - 1 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-12"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
1 - 2 ft.	CI	gravelly sandy SILTY CLAY, medium to high plasticity, 25% fine to coarse sub-angular gravel; 20% medium to coarse poorly graded sub-rounded sand; 5% cobbles (3-12"), reddish brown (5YR 4/4), blocky, disseminated CaCO ₃ , strong RXN with HCl; cohesive, dry, stiff.
2 - 4 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse sub-angular gravels; 5% cobbles (3-6"), pinkish gray (7.5YR 6/2), CaCO ₃ as masses and disseminated, strong RXN with HCl; cohesive, dry, very stiff.
4 - 7 ft.	SP	gravelly SAND, fine to coarse, poorly graded, sub-rounded, 25% fine to coarse poorly graded gravels, 5% non-plastic fines; trace sub-angular cobbles (3-6"), light brown (7.5YR 6/3), CaCO ₃ as masses, disseminated and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, compact.
7 - 12 ft.	SW-GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 10% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), strong SiO ₂ cementation (conglomerate) at 11 feet, strong RXN with HCl, CaCO ₃ as masses, disseminated and coatings on rock fragments; non-cohesive, dry, very dense.



Samples:

- 0-2 ft. bag, bucket
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-12 ft. bag, bucket

Special Notes:

Refusal at 12 feet due to cementation and oversized.



TEST PIT LOG: TP-30

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11974680.06 E: 871571.56

Date: 12/18/2012

Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-10"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
2 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% fine to coarse poorly graded sub-angular gravel, 15% low plasticity fines; 10% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO ₃ and masses, strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SW	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine poorly graded sub-angular gravels, 5% non-plastic fines; pinkish gray (7.5YR 6/2), CaCO ₃ as masses and coatings on gravels, moderate SiO ₂ cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 12 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 30% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO ₃ and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.



Samples:
None.

Special Notes:



TEST PIT LOG: TP-31

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11975597.72 E: 872172.62

Date: 1/3/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	CL	gravelly sandy CLAYEY SILT, medium to high plasticity, 30% fine to coarse sub-rounded sand, 25% fine to coarse sub-angular gravel; 10% sub-angular to sub-rounded cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 30% medium plasticity fines; 15% cobbles (3-9"), brown (7.5YR 5/3), blocky, CaCO ₃ masses, disseminated and weak cementation in places, clay fingering, strong RXN with HCl; cohesive, dry, stiff.
2 - 5 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, weak SiO ₂ cementation, weak RXN with HCl, CaCO ₃ coatings on rocks; non-cohesive, dry, dense.
5 - 8 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 5% non-plastic fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, mod. SiO ₂ cementation, no RXN with HCl; non-cohesive, dry, very dense.
8 - 13 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), brown (7.5YR 5/4), platy, weak SiO ₂ cementation, no RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), platy, strong SiO ₂ cementation, no RXN with HCl; non-cohesive, dry, very dense.



Samples:
 1-2 ft. bag
 2-5 ft. bag
 5-8 ft. bag
 8-13 ft. bag
 13-16 ft. bag
 BMI bag samples all layers

Special Notes:
 Surface disturbed- placer mining location. Pit located in drainage.



TEST PIT LOG: TP-32

Checked GM 1/29/2013

Client: THEMAC
Project: Copper Flat
Project No.: 103-92557
Location: Sierra County, NM
NAD 83: N: 11975136.61 E: 872876.80

Date: 1/3/2013

Lithology:

Depth	USCS	Description
0 - 1 ft.	CL-ML	sandy CLAYEY SILT, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 25% medium plasticity fines; 20% cobbles (3-10"), pinkish gray (7.5YR 7/2), blocky, CaCO ₃ masses, coatings on rock fragments and weak cementation in places, strong RXN with HCl; non-cohesive, dry, compact.
3 - 5 ft.	SM	SILTY SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 15% low plasticity fines; 5% cobbles (3-4"), light brown (7.5YR 6/3), weak SiO ₂ /CaCO ₃ cementation, strong RXN to HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, poorly graded, sub-rounded sand, 10% low plasticity fines; 20% cobbles and boulders (3-12"), pinkish gray (7.5YR 6/2), weak SiO ₂ cementation, CaCO ₃ coatings on rock fragments, weak RXN to HCl; non-cohesive, dry, very dense.
10 - 14 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% low plasticity fines; 15% cobbles (3-8"), light brown (7.5YR 6/3), large platy blocks excavated, strong SiO ₂ cementation, CaCO ₃ masses, weak RXN to HCl; non-cohesive, dry, very dense.



Samples:

3-5 ft. bag, bucket
5-10 ft. bag, bucket
10-14 ft. bag, bucket

Special Notes:

Hard

**APPENDIX A.2
DRILL HOLE LOGS**



REPORT OF BOREHOLE: SOIL KEY

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION: Copper Flat
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N , E
 CHECKED: DP DATE: 2/21/13 ELEVATION: ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer, Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
5										<p><u>Blows Per Six Inches</u> Number of sample hammer blows required to drive the sampler six inches, or recorded number of blows to drive the sampler the specified distance (e.g. 50/4" = 50 hammer blows to drive the sampler four inches).</p> <p><u>Blows Per Foot</u> Number of sample hammer blows required to drive the sampler twelve inches. Resolved using the final twelve inches of the sample or the amount of penetration upon sample refusal (50 blow counts).</p>				
15			SS1	4	11	17 / 18				<p><u>Sample Types</u> Standard Penetration Test - Full penetration with 17 of 18 inches recovered.</p>				
20			SS2	7	R	7 / 10				<p>Standard Penetration Test - Refusal at 10 inches, 7 of 10 inches recovered.</p> <p>Auger Bag/Bulk Sample - Bulk grab from auger cuttings to become a collective bag sample over an interval.</p>				
25														
30														
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT.GPJ TEMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:7/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations

UNIFIED SOIL CLASSIFICATION (ASTM D 2487-00)

MATERIAL TYPES	CRITERIA FOR ASSIGNING SOIL GROUP NAMES AND GROUP SYMBOLS USING LABORATORY TESTS			GROUP SYMBOL	SOIL GROUP NAMES & LEGEND	
COARSE-GRAINED SOILS >50% OF COARSE FRACTION RETAINED ON NO. 200 SIEVE	GRAVELS >50% OF COARSE FRACTION RETAINED ON NO. 4. SIEVE	CLEAN GRAVELS <5% FINES	$C_u > 4$ AND $1 < C_c < 3$	GW	WELL-GRADED GRAVEL	If soil contains >15% sand, add "with sand"
			$C_u > 4$ AND/OR $1 > C_c > 3$	GP	POORLY-GRADED GRAVEL	
		GRAVELS WITH FINES >12% FINES	FINES CLASSIFY AS ML OR CL	GM	SILTY GRAVEL	
			FINES CLASSIFY AS CL OR CH	GC	CLAYEY GRAVEL	
	SANDS >50% OF COARSE FRACTION PASSES ON NO. 4. SIEVE	CLEAN SANDS <5% FINES	$C_u > 6$ AND $1 < C_c < 3$	SW	WELL-GRADED SAND	If soil contains >15% gravel, add "with gravel"
			$C_u > 6$ AND/OR $1 > C_c > 3$	SP	POORLY-GRADED SAND	
SANDS AND FINES >12% FINES		FINES CLASSIFY AS ML OR MH	SM	SILTY SAND		
		FINES CLASSIFY AS CL OR CH	SC	CLAYEY SAND		
FINE-GRAINED SOILS >50% PASSES NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT <50		CL	LEAN CLAY	If soil contains coarse-grained soil from 15% to 29%, add "with sand" or "with gravel" for whichever type is prominent, or for >30%, add "sandy" or "gravelly"	
			ML	SILT		
			OL	ORGANIC CLAY OR SILT		
	SILTS AND CLAYS LIQUID LIMIT >50		CH	FAT CLAY		
			MH	ELASTIC SILT		
			OH	ORGANIC CLAY OR SILT		
HIGHLY ORGANIC SOILS	PRIMARILY ORGANIC MATTER, DARK IN COLOR, AND ORGANIC ODOR			PT	PEAT	

$$C_u = \frac{D_{60}}{D_{10}} \quad C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$$

Gravels or sands with 5% to 12% fines require dual symbols (GW-GM, GW-GC, GP-GM, GP-GC, SW-SM, SW-SC, SP-SM, SP-SC) and add "with clay" or "with silt" to group name. If fines classify as CL-ML for GM or SM, use dual symbol GC-GM or SC-SM.

DESCRIPTIVE TERMINOLOGY FOR PERCENTAGES (ASTM D 2488-00)

DESCRIPTIVE TERMS	RANGE OF PROPORTION
TRACE	0 - 5%
FEW	5 - 10%
LITTLE	15 - 25%
SOME	30 - 45%
MOSTLY	50 - 100%

LABORATORY TEST ABBREVIATIONS

AL Atterberg Limits	HY Hydrometer	SG Specific Gravity
CI Chloride Content	PT Proctor	SP Swell Potential
CO Consolidation	pH Soil pH	UC Unconfined Compression
CP Collapse Potential	RS Restivity	UU Triaxial Unconsolidated, Undrained
CU Triaxial Consolidated Undrained	RV R-Value	
DD Dry Density	SA Sieve Analysis	
DS Direct Shear	SC Soluble Sulfate Content	

CRITERIA FOR DESCRIBING MOISTURE CONDITION (ASTM D 2488-00)

DRY	Absence of moisture, dusty, dry to the touch
MOIST	Damp but no visible water
WET	Visible free water, usually soil is below water table

COMPONENT DEFINITIONS BY GRADATION

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. to 12 in.
GRAVEL	3 in. to No. 4 (4.76 mm)
COARSE GRAVEL	3 in. to 3/4 in.
FINE GRAVEL	3/4 in. to No. 4 (4.76 mm)
SAND	No. 4 (4.76 mm) to No. 200 (0.074 mm)
COARSE SAND	No. 4 (4.76 mm) to No. 10 (2.0 mm)
MEDIUM SAND	No. 10 (2.0 mm) to No. 40 (0.42 mm)
FINE SAND	No. 40 (0.42 mm) to No. 200 (0.074 mm)
SILT AND CLAY	Smaller than No. 200 (0.074 mm)
SILT	0.074 mm to 0.005 mm
CLAY	Less than 0.005 mm

RELATIVE DENSITY / CONSISTENCY ESTIMATE USING STANDARD PENETRATION TEST (SPT) VALUES

COHESIONLESS SOILS (GRAVEL, SAND, NONPLASTIC SILT)			COHESIVE SOILS (PLASTIC SILT, CLAY)		
DENSITY	N ₁ (BLOWS /FOOT)*	RELATIVE DENSITY (%)	CONSISTENCY	N ₁ (BLOWS /FOOT)*	COMPRESSIVE STRENGTH (TSF)
VERY LOOSE	0 - 4	0 - 15	VERY SOFT	0 - 2	0 - 0.25
LOOSE	4 - 10	15 - 35	SOFT	2 - 4	0.25 - 0.50
COMPACT	10 - 30	35 - 65	FIRM	4 - 8	0.50 - 1.0
DENSE	30 - 50	65 - 85	STIFF	8 - 15	1.0 - 2.0
VERY DENSE	OVER 50	> 85	VERY STIFF	15 - 30	2.0 - 4.0
			HARD	OVER 30	OVER 4.0

*Refer to ASTM D 1586-99 for a definition of N. Values shown are based on N values corrected for overburden pressures (N₁). N values may be affected by a number of factors including material size, depth, drilling method, and borehole disturbance. N values are only an approximate guide for consistency of cohesive soil.

GENERAL NOTES

Report of Borehole logs present material classifications, test data, and observations from subsurface explorations at the subject site as reported by the field geologist, engineer, or scientist. In some cases, the classifications may be made based on laboratory test data when available. It should be noted that the investigation methods only recover a small part of the subsurface materials at the exploration location. Therefore, actual conditions between borings and sampled intervals may differ from those presented on the Report of Borehole logs.

This key and Report of Borehole logs must be read together with the attached report. The information presented on the logs and in this key provide only a basis for an evaluation of the subsurface conditions. Any evaluation of the conditions reported on the Report of Borehole logs must be performed by Professional Engineers or Geologists.



REPORT OF BOREHOLE: BH-01

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 12/17/12
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-1 43.5 feet west
 XY COORDINATES: N 11,973,679, E 871,432
 ELEVATION: 5,298.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Compact, light gray, SILTY GRAVEL (GM) with sand, dry, some cobbles.				
5			SS1		18 17 32	49	18 / 18			Becomes dense, gray.				
				Bulk										
10			SS2		4 40 45	85	18 / 18		GM	Becomes very dense.				
				Bulk										
15			SS3		13 32 50	82	18 / 18							
				Bag										
	5280.5 17.5			Bulk						Hard, brown, GRAVELLY SILT (ML), dry, some cobbles.				
20			SS4		26 50/5"	R	11 / 11							
				Bulk										
25			SS5		45 30/2"	R	8 / 8		ML					
				Bag										
30			SS6		40 40/3"	R	3 / 6							
				Bag										
	5267.0 31.0			SS7	10 10/0"	R	0 / 0			Refusal at 31'. Backfilled with cuttings. No groundwater encountered in boring.				
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-02

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 12/18/12

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-2 2 feet east

XY COORDINATES: N 11,973,762, E 870,444

ELEVATION: 5,361.8 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: Hollow Stem Auger

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
	5358.3			B/Bag					GC	Compact, light gray, CLAYEY GRAVEL WITH SAND (GC), dry, some cobbles.				
	3.5			SS1	17 16 23	39	18 / 18			Dense, light gray, SILTY SAND (SM), little gravel, dry.				
				B/Bag					SM	Becomes light brown, some gravel, occasional cobbles.				
	5352.3			SS2	16 29 50	79	6 / 18							
	5359.8			Bulk					GW	6" lens of very dense, gray, GRAVEL (GW), dry.				
	10.0									Hard, brown, SANDY SILT and gravel (ML), dry, occasional cobble.				
				SS3	17 50/6"	R	12 / 12							
				Bulk					ML					
	5343.3			SS4	26 47 50/3"	R	15 / 15							
	18.5			Bulk					SM	Very dense, brown and gray, SILTY SAND (SM), some gravel, dry.				
	5340.3													
	21.5									Refusal at 21.5'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-03

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-3 5 feet south
 LOGGED: CMT DATE: 1/25/13 XY COORDINATES: N 11,977,597, E 868,520
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,468.6 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, gray, WELL-GRADED SAND WITH GRAVEL (SW), trace fines, dry.				
5									SW					
8.0	5460.6			SS1 Bulk	50/0"	R	0 / 0			Slightly weathered, dark gray, strong rock.	Becomes rock.			
15.0	5453.6			Core						Refusal at 8'. Bottom of borehole at 15'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-04

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/5/13 XY COORDINATES: N 11,977,281, E 870,076
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,355.3 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Diamond Coring
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					SW-SM	Compact, gray/light gray, SILTY SAND (SM), some gravel, cobbles, dry, cementation.				
5351.3 4.0			SS1		14 23 19	42	18 / 18			Dense, gray, SANDY SILT (SM), dry, some cementation.				
				Bag					ML					
5346.3 9.0			SS2 Core		25/0"	R	0 / 0			Weathered, dark gray, fragmented ROCK, trace cementation, trace silty sand.	Becomes rock. Switch to Diamond Coring.			
				Core						Becomes moderate cementation				
				Core						Become strong cementation. [CONGLOMERATE]				
5337.3 18.0				Core						Refusal of Hollow Stem Auger at 9'. Core from 9' to 18'. Bottom of borehole at 18'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-05

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/7/13 XY COORDINATES: N 11,977,243, E 869,166
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,385.6 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Diamond Coring
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Compact, gray/ light gray, SILTY SAND and gravel (SW-SM), dry, trace cementation, cobbles.				
5	5379.6 6.0			Bulk Core	12 24 41	65	18 / 18		SW-SM	Becomes very dense.				
10										Weathered, dark gray, fragmented ROCK, some sandy silt, strongly cemented.	Become rock. Switch to Diamond coring.			
15	5372.6 13.0									Refusal of Hollow Stem Auger at 6'. Diamond coring from 6-13'. Bottom of borehole at 13'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-06

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/25/13 XY COORDINATES: N 11,976,597, E 870,667
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,308.1 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer, Diamond Coring
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag							Very dense, brown, SILTY SAND and gravel (SW-SM), dry.				
5			Bulk						SW-SM	Becomes gray.				
5300.1 8.0			SS1 Bulk		50/0"	R	0 / 0			Slightly weathered, dark gray, strong ROCK.	Becomes rock.			
10														
15														
20			Core								Switch to Diamond Coring.			
25	5283.1 25.0									Diamond coring from 18-25'. Bottom of borehole at 25'. Backfilled with cuttings. No groundwater encountered in boring.				
30														
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-07

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 12/18/12
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,976,181, E 871,532
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Diamond Coring
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0	5371.5 0.5		Bag Bag						SM GP	Loose, brown, SILTY SAND (SM), little gravel, dry. Compact, light gray, SANDY GRAVEL (GP), dry.				
5	5367.0 5.0		SS1 Bag		14 28 33	61	18 / 18			Lens of brown/white, SILTY SAND, trace gravel, dry. Becomes very dense. Becomes well-graded gravel. Very dense, gray, SILTY SAND and gravel (SW-SM), dry.				
10			SS2 Bag		12 39 50/5"	R	17 / 17		SW-SM					
15	5357.0 15.0		SS3 Bag Bag		11 37 50/2"	R	14 / 14			Very dense, gray, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
20			SS4 Bag		25 30/2"	R	10 / 10		GW-GM					
25	5347.5 24.5		SS5 Bag		23 32/3"	R	9 / 9			Very dense, brown, SILTY SAND and gravel (SW-SM), dry, few cobbles.				
30	5342.0 30.0		SS6 Core		36/5"	R	5 / 5		SW-SM					
35			SS7 Core		50/0"	R	0 / 0			Weathered, dark gray, fractured ROCK, strong cementation.	Becomes Rock. Switch to Diamond Coring.			
40			SS8 Core		20 50/0"	R	6 / 6							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-07

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 12/18/12
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,976,181, E 871,532
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Diamond Coring
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40										Weathered, dark gray, fractured ROCK, strong cementation. (continued)				
45														
50	5324.0 48.0									Refusal of Hollow Stem Auger at 30'. Diamond coring from 30-48'. Bottom of borehole at 48'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-08

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/19/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,971,703, E 873,489
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Dense, brown, SILTY SAND (SW-SM), some gravel, slightly cohesive, trace cementation, trace clayey silt, dry.				
5														
10			SS1	18 19 23	42	18 / 18			SW-SM					
			Bulk							Becomes very dense.				
15			SS2	50 53 30	83	18 / 18								
			Bulk											
20	5200.0 18.0		Bulk							Hard/very dense, brown, SANDY SILT and gravel (ML), some clayey silt (low plasticity), dry, trace cementation.				
25			SS3	50/5"	R	5 / 5			ML					
			Bulk											
30	5190.0 28.0		Bag						SW-SM	Very dense, gray/brown, SILTY SAND and gravel (SW-SM), some clayey silt, dry.				
35	5185.0 33.0		SS4	50/5"	R	5 / 5			SW	Very dense, gray, SAND and gravel (SW), dry.				
			Bulk											
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-08

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/19/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,971,703, E 873,489
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5175.0 43.0								SW	Very dense, gray, SAND and gravel (SW), dry. <i>(continued)</i>				
45									GW	Very dense, gray, GRAVEL and sand (GW), trace clayey silt, dry.				
50	5168.0 50.0		SS5		50/0"	R	0/0							
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-09

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 12/21/13

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-9 2 feet east

XY COORDINATES: N 11,972,261, E 875,052

ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: HSA, Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, brown, SANDY SILT (ML), trace gravel, dry.				
5	5170.7 6.0		SS1	20 34 40	74	18 / 18		ML						
			Bulk											
			SS2	20/0"	R	0 / 1				Dense to very dense, light gray/brown, SAND and gravel (SW), dry.				
10	5165.7 11.0		SS3	20 25 24	49	18 / 18		SW						
			Bag(2)											
			Bag							Hard, light brown, SILT (ML), trace gravel, dry, slightly cohesive, trace cementation.				
			SS4	16 25 27	52	18 / 18				Becomes light reddish brown, little gravel.				
15			Bulk					ML						
20	5156.7 20.0		SS5	4 14 24	38	18 / 18								
			Bulk							Hard, reddish brown, CLAYEY SILT (MH), dry, cohesive.				
			SS6	22 14 19	33	18 / 18		MH						
25	5150.7 26.0		Bulk											
			B/Bag							Hard, red SILTY CLAY (CL-ML), dry, cohesive, moderate plasticity.				
			SS7	16 15 19	34	18 / 18								
			B/Bag					CL-ML						
30			SS8	6 4 13	17	18 / 18								
	5141.7 35.0		Bulk							Hard, red, CLAY (CL), dry.				
			SS9	14 21 31	52	18 / 18		CL						

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-09

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 12/21/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-9 2 feet east
 XY COORDINATES: N 11,972,261, E 875,052
 ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk					CL	Hard, red, CLAY (CL), dry. <i>(continued)</i>				
	5133.2 43.5		SS10		13 20 32	52	18 / 18		CL-ML	Hard, red, SILTY CLAY (CL-ML), dry.				
45	5131.7 45.0			Bulk					CH	Hard, red, CLAY, dry.				
50	5126.7 50.0		SS11		9 15 24	39	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-10

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/12/13 XY COORDINATES: N 11,972,513, E 874,813
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				B/Bag						Compact, light gray, CLAYEY SAND WITH GRAVEL (SC), dry, few cobbles.				
5			SS1	X	17 6 5	11	18 / 18							
				B/Bag					SC					
10			SS2	X	9 12 18	30	18 / 18							
				B/Bag										
15	5167.5 14.5		SS3	X	20 16 14	30	18 / 18							
				Bag					MH	Very stiff, red/light brown, CLAYEY SILT (MH), dry, little gravel.				
20	5163.0 19.0		SS4	X	8 15 20	35	18 / 18							
				B/Bag						Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity.				
25			SS5	X	3 10 17	27	18 / 18							
				B/Bag										
30			SS6	X	10 19 41	60	18 / 18							
				B/Bag					CL					
35			SS7	X	8 11 17	28	18 / 18			Becomes moderate plasticity				
				B/Bag						Becomes slightly moist, moderate plasticity.				
40			SS8	X	4 6 19	25	18 / 18							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TEMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-10

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/12/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,513, E 874,813
 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			B/Bag											
	5138.5 43.5		SS9		4 7 14	21	18 / 18		CL	Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity. <i>(continued)</i>				
45									CH	Very stiff, red, CLAY (CH), slightly moist, high plasticity.				
50	5132.0 50.0		SS10		7 10 16	26	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-11

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/3/13 XY COORDINATES: N 11,972,894, E 874,891
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff-very stiff, brown, SILT (ML), trace sand and gravel, dry.				
5178.0 2.5			Bulk											
			SS1		9 13 27	40	18 / 18			Dense, white/light gray, GRAVELLY SAND (SW), dry.				
			Bulk						SW					
5172.0 8.5			SS2		12 27 23	50	18 / 18		SW-SM	Very dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk											
5167.0 13.5			SS3		5 11 16	27	18 / 18		MH	Very stiff, brown, CLAYEY SILT (MH), dry, low plasticity.				
			Bulk											
5162.0 18.5			SS4		7 12 23	35	18 / 18			Hard, reddish brown, SILTY CLAY (CL-ML), white and black inclusions, dry.				
			B/Bag							Becomes red, trace gravel, moderate plasticity, slightly moist.				
			SS5		24 16 23	39	18 / 18							
			B/Bag						CL-ML					
			SS6		7 15 25	40	18 / 18							
			B/Bag											
5147.0 33.5			SS7		11 13 18	31	18 / 18			Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity.				
			Bulk						CH					
			SS8		12 18 30	48	18 / 18							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-11

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/3/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,894, E 874,891
 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9	11 13 18	31	18 / 18			CH	Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity. (continued)				
50	5130.5 50.0		SS10	11 12 21	33	18 / 18					Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-12

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/4/13 XY COORDINATES: N 11,972,981, E 875,148
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff, brown, SANDY SILT (ML) and gravel, dry.				
5175.5	4.0		SS1		5	27	18 / 18		GW	Compact, light gray, GRAVEL AND SAND (GW), dry.				
5174.5	5.0		Bulk		12					Compact, gray/light brown, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
					15									
			SS2		39	93	17 / 17		GW-GM	Becomes very dense, trace cobbles.				
					43									
					50									
			SS3		13	83	16 / 16							
5164.5	15.0		Bulk Bag		33					Hard, brown, SANDY SILT and gravel (ML), dry.				
					50				ML					
			SS4		7	42	18 / 18							
5160.0	19.5		B/Bag		18					Hard, red, SILTY CLAY (CL-ML), trace gravel, dry, low plasticity.				
					24									
			SS5		7	30	18 / 18							
					10									
					20				CL-ML					
			SS6		12	41	18 / 18							
					15									
					26									
			SS7		12	38	18 / 18							
5146.0	33.5		Bulk		14					Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity.				
					24				CH					
			SS8		12	42	18 / 18							
					16									
					26									
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-12

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/4/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,981, E 875,148
 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity. <i>(continued)</i>				
45			SS9		10 11 18	29	18 / 18		CH	Becomes very stiff-hard.				
50	5129.5 50.0		SS10		10 12 20	32	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-13

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/17/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,776, E 875,471
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, light gray, SANDY SILT and gravel (ML), dry, slightly cohesive.				
5			SS1		12 11 13	24	18 / 18							
				Bulk										
10			SS2		28 28 33	61	18 / 18			Becomes hard, light brown, trace cementation.				
				Bag					ML					
15			SS3		13 29 35	64	18 / 18							
				Bag										
20			SS4		21 30 31	61	18 / 18							
				Bulk										
25	5146.3 23.5		SS5		28 50/3"	R				Very dense, brown, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines.				
				Bulk										
30			SS6		21 50/4"	R				Lens of reddish brown clayey silt encountered at 34'.				
				Bulk					GW-GM					
35			SS7		27 49 50/2"	R								
				Bulk										
40	5131.3 38.5		SS8		13 24 33	57	18 / 18			Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel.				
									CL-ML					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-13

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/17/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,776, E 875,471
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bag						Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel. <i>(continued)</i>				
45			SS9	Bulk	9 14 14	28	18 / 18		CL-ML	Becomes very stiff, red, dry.				
50	5119.3 50.5		SS10		25 32 28	60	18 / 18				Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-14

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/22/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,766, E 875,868
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, brown, SILT (ML), some gravel, dry.				
5			SS1	23 44 30	74	18 / 18			ML	Becomes hard, brown, some gravel, cementation at 4-5 ft.				
10	5149.7 8.5		SS2	15 19 23	42	18 / 18			GW-GM	Dense, light brown/gray, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines, few cobbles.				
15			SS3	22 20 25	45	18 / 18			GW					
20			SS4	17 50/3"	R	9 / 9			GW	Becomes very dense, light brown/gray, GRAVEL and silty sand.				
25	5134.7 23.5		SS5	18 35 50/4"	R	16 / 16			ML	Hard, light gray, SANDY SILT and gravel (ML), dry.				
30	5129.7 28.5		SS6	25 50/6"	R	12 / 18			SW-SM	Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry.				
35			SS7	33 32 50/5"	R	17 / 17			SW-SM					
40			SS8	16 50/5"	R	11 / 11								

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-14

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/22/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,972,766, E 875,868
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9 Bulk		50/1"	R	1 / 1		SW-SM	Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry. (continued)				
50	5108.2 50.0		SS10		31 29 44	73	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-15

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/4/13 XY COORDINATES: N 11,973,609, E 875,724
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, light gray/light brown, SILT (ML), little gravel, dry.				
5			SS1	9 15 26	41				ML					
8.0	5168.4			Bulk										
10	5166.4		SS2	38 50/6"	R	12 / 12			GW	Very dense, brown, SANDY GRAVEL (GW), dry.				
12.0	5164.4			Bag					SW	Very dense, gray, SAND and gravel (SW), dry.				
15			SS3	17 23 50	73				GW	Very dense, gray, SANDY GRAVEL (GW), dry.				
17.5	5158.9			Bulk										
20	5154.9		SS4	30 50/3"	R	6 / 9			SW	Very dense, light gray, SAND and gravel (SW), dry.				
21.5	5154.9			Bag(2)										
25			SS5	28 50/4"	R	10 / 10			ML	Hard, light gray, SANDY SILT and gravel (ML), dry.				
27.5	5148.9			Bag(2)										
30			SS6	22 50/6"	R	11 / 12			SW	Very dense, gray, SAND and gravel (SW), dry.				
31.5	5144.9			Bag(2)										
35			SS7	200"	R				SW-SM	Very dense, light gray, SILTY SAND (SW-SM), some gravel, dry.				
36.5	5139.9			Bag(2)										
40									SW	Very dense, light gray, SAND and gravel (SW), dry.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-15

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/4/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,973,609, E 875,724
 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5134.9 41.5		SS8 Bag(2)		50/2"	R			SW	Very dense, light gray, SAND and gravel (SW), dry. <i>(continued)</i>				
45									ML	Very dense, gray/brown, SANDY SILT (ML), and gravel, dry, slightly cohesive.				
50	5126.4 50.0								MH	Hard, reddish brown, CLAYEY SILT (MH), some gravel, slightly moist, slightly cohesive.				
55	5122.1 54.3		SS9		24 54 38/3"	R				Bottom of borehole at 54.25'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-16

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/22/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north
 XY COORDINATES: N 11,973,973, E 875,187
 ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown, CLAYEY GRAVEL with sand (GC), dry.				
5			SS1	SS1	9 32 30	62	18 / 18		GC					
10	5183.2 8.5		SS2	SS2	26 18 19	37	18 / 18		GP-GM	Very dense, gray, GRAVEL and sandy silt (GP-GM), dry.				
15	5178.2 13.5		SS3	B/Bulk	50/3"	R	3 / 3		GW	Very dense, light brown, GRAVEL and silty sand (GW), dry, trace CaCO3.				
20			SS4	B/Bulk	26 50/4"	R	10 / 10		GW					
25			SS5	B/Bulk	48 50/3"	R	9 / 9		GW					
30	5162.7 29.0		SS6	Bulk	21 50/3"	R	9 / 9		SC	Very dense, gray, CLAYEY SAND with gravel, trace CaCO3, dry, some cementation.				
35	5157.7 34.0		SS7	Bulk	13 8 12	20	18 / 18		MH	Very stiff, light reddish brown, CLAYEY SILT (MH), dry.				
40	5153.2 38.5		SS8		14 9 11	20	18 / 18		CL-ML	Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-16

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 1/22/13

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north

XY COORDINATES: N 11,973,973, E 875,187

ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist. <i>(continued)</i>				
45			SS9		17 11 13	24	18 / 18		CL-ML					
				Bulk										
50	5141.7 50.0		SS10		4 2 4	6	18 / 18			Becomes firm, caliche and little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-17

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/5/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,974,131, E 875,734
 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, gray/light brown, SANDY SILT and gravel (ML), dry.				
5			SS1	X	10 13 12	25	18 / 18		ML					
				Bulk										
	5177.5 8.5		SS2	X	4 45 50/4"	R	12 / 16			Very dense, gray, SANDY GRAVEL (GW), dry. Becomes brown/gray.				
10				Bulk										
15									GW					
20			SS3	X	7 28 39	67	18 / 18							
	5163.0 23.0			Bulk						Very dense, light brown, SAND and gravel (SW), dry.				
25									SW	Becomes light gray/ light brown.				
30	5158.0 28.0		SS4	X	24 50 50/4"	R	16 / 16		SW-SM	Very dense, brown, SILTY SAND and gravel (SW-SM), dry, slightly cohesive fines.				
	5154.0 32.0			Bulk						Very dense, white/gray CALICHE, dry.				
35														
40			SS5	Bulk	25/0"	R	0 / 0							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-17

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/5/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,974,131, E 875,734
 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40										Very dense, white/gray CALICHE, dry. <i>(continued)</i>				
45										very dense, white/gray CALICHE, dry, little gravel, seam of light brown silt at 44-45'				
50	5136.0 50.0		SS6		25/0"	R	0 / 0			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-18

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/23/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-18 2 feet east
 XY COORDINATES: N 11,974,701, E 874,701
 ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					ML	Stiff, light brown, SANDY SILT (ML), some gravel, dry.				
5205.3 2.0				Bulk										
				SS1	9 23 20	43	18 / 18		GC	Dense, light brown, CLAYEY GRAVEL with sand (GC), dry.	Switch to Air Rotary drilling at 6'.			
5199.3 8.0				Bulk										
				SS2										
				Bulk										
5189.3 18.0				SS2	50/4"	R			GW	Very dense, light brown/gray, GRAVEL and sand (GW), dry, little CaCO3.				
				Bulk										
5184.3 23.0				SS3	24 31 31	62	18 / 18		ML	Hard, light reddish white, SANDY SILT (ML), some gravel, dry, some CaCO3.				
				Bulk										
5173.8 33.5				SS4	13 8 12	20	18 / 18		CL	Hard, light reddish brown, sandy LEAN CLAY (CL), trace gravel, dry.				
				Bulk										
5168.8 38.5				SS5	38 50/3"	R	9 / 9		CL-ML	Hard, reddish brown, CLAY (CL-ML), some silty clay, little gravel, moderate plasticity, dry.				
				Bulk										
				SS6	11 21 24	45	18 / 18		CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist.				
				Bulk										

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-18

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 1/23/13

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-18 2 feet east

XY COORDINATES: N 11,974,701, E 874,701

ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: HSA, Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5163.8 43.5		Bulk						CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist. <i>(continued)</i>				
45			SS7	5 6 9	15	18 / 18			CH	Stiff-hard, reddish brown, CLAY (CH), high plasticity, slightly moist, blocky.				
50	5157.3 50.0		SS8	10 19 28	47	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-19

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/10/13 XY COORDINATES: N 11,974,564, E 875,328
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0										Soft, brown, SANDY SILT (ML), little gravel, dry.				
5193.4 3.0			SS1	11 21 22	43	18 / 18		ML		Dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk					SW-SM						
5187.4 9.0			SS2	19 38 50/5"	R	17 / 17				Very dense, brown/gray, SANDY GRAVEL (GW), dry.				
			Bulk											
			Bag							Becomes dark gray.				
			Bag(2)							Becomes brown/gray.				
			SS3	27 40 23	63	18 / 18		GW						
5174.4 22.0			B/Bag							Very dense, light gray/white, CALICHE, little gravel, dry.				
			SS4 B/Bag	50/0"	R	0 / 0								
			SS5 Bulk	25/0"	R	0 / 0				Seam of brown sandy silt at 41' - 42', dry.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-19

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/10/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,974,564, E 875,328
 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5153.4 43.0			Bulk						Very dense, light gray/white, CALICHE, little gravel, dry. <i>(continued)</i>				
45				Bag					CL-ML	Very stiff, red, SILTY CLAY (CL-ML), slightly moist, cohesive				
50	5146.4 50.0			SS6	5 13 14	27	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-20

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/20/13
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to this location
 XY COORDINATES: N 11,975,241, E 871,714
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)							Compact, brown, SILTY SAND (SM), little gravel, dry.				
5			Bag						SM	Becomes compact to dense, light gray, slightly cohesive, trace clayey silt.				
8.0	5284.0		SS1		16	60	18 / 18			Very dense, gray, GRAVEL (GW), some silty sand, dry, trace clayey silt lenses.				
10			Bulk		29				GW					
18.0	5274.0		SS2		50/3"	R	3 / 3			Very dense, light brown/gray, SILTY SAND and gravel, (SW-SM) dry, slightly cohesive, some light reddish brown clayey silt.				
20			Bulk											
30			SS3		50/3"	R	3 / 3		SW-SM					
35			Bulk											
40	5252.0		SS4		15	R	9 / 9							
			Bulk		50/3"									

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-20

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/20/13
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to new location
 XY COORDINATES: N 11,975,241, E 871,714
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0			Bulk										
45									SM	Very dense, light brown, SILTY SAND (SM), some gravel, slightly moist, slightly cohesive.				
50	5242.0 50.0		SS5	X	21 36 50/2"	R	14 / 14				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-21

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N 11,975,236, E 874,685
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,214.2 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown/gray, WELL-GRADED SAND with silt and gravel (SW-SM), dry.				
21			SS1		21	83	18 / 18		SW-SM					
36				Bulk	36									
47					47									
5196.2	18.0		SS2		25	68	18 / 18		SW-SM	Very dense, light brown/reddish white, SILTY SAND and gravel (SW-SM), trace clayey silt, cementation, dry.				
30			SS3		50/0*	R	0 / 0							
5184.2	30.0		Bag(2)							Hard, light reddish brown, CLAYEY SILT and silty clay (MH), trace gravel, dry.				
5177.7	36.5		SS4		15	65	18 / 18		MH					
										Bottom of borehole at 36.5'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-22

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/21/13 XY COORDINATES: N 11,974,757, E 873,750
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag Bulk						Loose, dark brown, SANDY SILTY CLAY (CL-ML), some gravel, dry. Becomes very stiff, yellowish/orange, trace gravel, dry (old tailings).				
5			SS1	16 8 8	16	18 / 18			CL-ML	Becomes orange/brown, slightly moist, cohesive, (old tailings).				
10			SS2	5 4 6	10	18 / 18			CL-ML	Becomes stiff, greenish-gray, moist, cohesive, (old tailings).				
15	5217.5 14.5		SS3	9 10 50/3"	R	15 / 15			GW	Very dense, light gray-gray, GRAVEL and sand (GW), dry, CaCO3 inclusions.				
25	5210.0 22.0		SS4	23 50/3"	R	9 / 9			ML	Hard, light gray/yellowish, SILT (ML), some gravel, slightly moist, slightly cohesive.				
30	5202.0 30.0			Bag Bulk					ML	Becomes brown, SANDY SILT, some clayey silt, trace gravel, moist, cohesive fines.		10.4		
35	5197.0 35.0		SS5	7 11 13	24	18 / 18			CL-ML	Very stiff, reddish brown, SILTY CLAY (CL-ML), moist, cohesive.				
40				Bulk Bulk					CL	Hard, reddish brown, CLAY (CL), dry, moderate plasticity.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-22

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/21/13 XY COORDINATES: N 11,974,757, E 873,750
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5188.5 43.5		SS6		20 45 50/3"	R	15 / 15		CL	Hard, reddish brown, CLAY (CL), dry, moderate plasticity. <i>(continued)</i>				
45	5186.0 46.0		Bulk						SM	Hard, gray/brown, SILTY SAND (SM), slightly moist.				
50	5182.0 50.0		SS7		10 17 50/5"	R	17 / 17		CL-ML	Hard, brown, SILTY CLAY (SM), slightly moist, little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-23

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/20/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest
 XY COORDINATES: N 11,974,757, E 873,750
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
	5228.0 2.0			Bag					SP	Loose, brown, SAND and gravel (SP), dry.				
				SS						Very stiff, yellowish, SILT (ML), trace gravel, dry (old tailings).				
				Bulk					ML	Becomes slightly cohesive.				
	5221.0 5220.0 9.5			SS1	13 38 36	74	18 / 18							
				Bag					GM	Very dense, gray/brown, GRAVEL and silt (GM), dry, slight cementation.				
										Hard, white, SANDY SILT (ML), little gravel, dry.				
									ML					
	5216.0 14.0			SS2	13 16 14	30	18 / 30							
				Bulk						Dense, gray, SILTY SAND and gravel (SM), dry, CaCO3 inclusions, slight cementation.				
				SS3	16 18 50/2"	R	14 / 14							
				Bag(2)						Becomes very dense.				
									SM					
				Bulk						Becomes slightly moist.				
				SS4	50/6"	R	6 / 4							
				Bulk						Becomes brown, some blocky and cohesive clayey silt (low plasticity), slightly moist.				
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-23

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/20/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest
 XY COORDINATES: N 11,974,757, E 873,750
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45									SM	Dense, gray, SILTY SAND and gravel (SM), dry, CaCO3 inclusions, slight cementation. (continued)				
48.0	5182.0													
49.5	5180.5		SS5	50/4"	R	4 / 5			ML	Very dense, brown, SANDY SILT (ML), wet, little gravel.				
50										Bottom of borehole at 49.5'. Backfilled with cuttings and bentonite. Groundwater encountered in boring at 45'.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-24

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/19/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,975,910, E 872,908
 ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						SP	Dense, gray/brown, SAND and gravel (SP), dry, trace CaCO ₃ .				
2.5	5264.6			Bulk										
5														
10			SS1	Bulk	50/4"	R	4 / 4							
15									SW-SM					
20			SS2	Bulk	50/3"	R	3 / 3							
25														
28.0	5239.1		SS3	Bulk	41 50/3"	R	9 / 9			Hard/very dense, brown, SANDY SILT and gravel (ML), trace clayey silt, dry.				
30									ML					
33.0	5234.1			Bulk						Very dense, gray/brown, SILTY SAND (SM), some gravel, trace clayey silt, dry.				
35									SM					
40	5227.1		SS4		44 50/3"	R	9 / 9							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-24

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/19/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,975,910, E 872,908
 ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0			Bulk										
45									ML					
50	5217.1 50.0		SS5		50/1"	R	1 / 1							
55														
60														
65														
70														
75														
80														

Hard, light reddish brown, SANDY SILT (ML), some gravel, trace clayey silt, dry, slightly cohesive.

Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.



REPORT OF BOREHOLE: BH-25

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION:
 LOGGED: CMT DATE: 1/18/13 XY COORDINATES: N 11,971,726, E 874,570
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMPT (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Dense to very dense, gray/brown, CLAYEY GRAVEL with sand (GC), well graded, dry.				
5			SS1		22 28	57	18 / 18		GC					
10			SS2		39 26 16	42	18 / 18		GC					
15	5199.7 12.5		SS3		18 21 20	41	18 / 18		ML	Hard, brown, SANDY SILT and gravel (ML), dry, slightly cohesive fines, trace CaCO3.				
20			SS4		15 15 33	48	18 / 18		ML					
25	5190.2 22.0		SS5		10 27 31	58	18 / 18		SC	Very dense, light reddish brown, CLAYEY SAND (SC), dry, trace gravel, trace CaCO3.				
30			SS6		5 32 35	57	18 / 18		SC					
35	5177.2 35.0		SS7		6 9 16	25	18 / 18		CL-ML	Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive.				
40			SS8		8 8	25	18 / 18		CL-ML					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-25

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/18/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,971,726, E 874,570
 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			Bulk		17					Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive. (continued)				
45	5167.2 45.0		SS9 Bulk		11 10 25	35	18 / 18		CL-ML		Hard, red, CLAY (CH), dry, sand seam at 44' to 44.5'.			
50	5161.7 50.5		SS10		10 18 26	44	18 / 18		CH		Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-26

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-26 2 feet south
 LOGGED: CMT DATE: 1/18/13 XY COORDINATES: N 11,971,618, E 872,048
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						GW	Dense, brown, GRAVEL (GW), some sand, dry.				
5310.0 2.0			Bulk						SW	Very dense, gray/brown, SAND and gravel (SW), dry, sheen.				
5304.0 8.0			SS1 Bulk	27 50/3"	R	9 / 9			GW	Very dense, dark gray, GRAVEL (GW), some sand, dry.				
5297.0 15.0			Bag(2)						SW	Very dense, brown/dark gray, SAND and gravel (SW), dry.				
			SS2 Bulk	50/4"	R	4 / 4			SW	Becomes gray/light brown, trace CaCO3.				
			SS3 SS	25 50/2"	R	8 / 8			SW					
5279.0 33.0			Bag(2)						SW-SM	Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt.				
			SS4 Bulk	21 50/5"	R	11 / 4			SW-SM					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-26

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/18/13
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-26 2 feet south
 XY COORDINATES: N 11,971,618, E 872,048
 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45														
50	5262.0 50.0		SS5	50/2"	R	2 / 5		SW-SM		Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt. <i>(continued)</i>				
55											Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-27

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/23/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,973,983, E 874,918
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Dense, brown, SILTY SAND and gravel (50%) (SM), dry, trace CaCO3, some cementation.				
5									SM					
10				SS1	17 16 15	31	18 / 18							
12.0	5186.1			Bulk										
15										Hard, light brown/white-gray, SANDY SILT (ML), some gravel, dry, some CaCO3, cementation.				
20				SS2	30 58/6'	R	12 / 12		ML					
23.0	5175.1			Bulk										
25										Very dense, brown, SAND and gravel (SW), dry.				
26.0	5172.1			SS3	20 24 33	57	18 / 18		CH	Hard, reddish brown, CLAY (CH), dry.				
27.5	5170.6			Bulk						Very stiff, light reddish brown, CLAYEY SILT (MH), some clay, dry, trace gravel.				
30														
35				SS4	10 7 13	20	18 / 18		MH					
38.5	5159.6			Bulk										
40				SS5	12 24 30	54	18 / 18		CL-ML	Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO3 inclusions.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13



REPORT OF BOREHOLE: BH-27

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 1/23/13
 CHECKED: DP DATE: 2/21/13

LOCATION:
 XY COORDINATES: N 11,973,983, E 874,918
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: HSA, Air Hammer
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO3 inclusions. <i>(continued)</i>				
45			SS6		10 12 17	29	18 / 18							
			Bulk											
50	5148.1 50.0		SS7		14 20 27	47	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



REPORT OF BOREHOLE: BH-28

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility
 PROJECT NO.: 103-92557
 CLIENT: New Mexico Copper Corp.
 LOGGED: CMT DATE: 12/18/12
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-28 2 feet east
 XY COORDINATES: N 11,975,241, E 870,785
 ELEVATION: 5,388.0 ft.

DRILLING CONTRACTOR: Yellow Jacket
 DRILL RIG: CME-1250
 DRILLING METHOD: Hollow Stem Auger
 HAMMER TYPE: Auto Hammer
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					GW	Dense, light gray, SANDY GRAVEL (GW), dry, few cobbles.				
5	5383.0 5.0		SS1	Bag	9 13 18	31			GP	Dense, light gray, SANDY GRAVEL (GP), dry.				
10	5378.0 10.0 5377.0 11.0		SS2	Bag	22 32 50	82			GW	Becomes very dense, white inclusions, sheen, dry.				
15	5374.5 13.5		SS3	Bag	30 30/3"	R			SM	Very dense, light gray, SANDY GRAVEL (GW), dry, few cobbles. Very dense, light brown and gray, gravelly SILTY SAND (SM), dry.				
20	5369.0 19.0 5367.0 21.0 5366.0 22.0		SS4	Bag	50/4"	R			SW	Very dense, light brown and gray, gravelly SAND (SW), sheen, dry.				
25			SS5	Bag	50/0"	R			GW	Seam of very dense, light gray, SANDY GRAVEL (GW), dry. Becomes light brown.				
30									SW	Very dense, light brown and gray, gravelly SAND, (SW) sheen, dry.				
40										Refusal at 22'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR_TUC2.GDT LIB:GLDR_TUC_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations

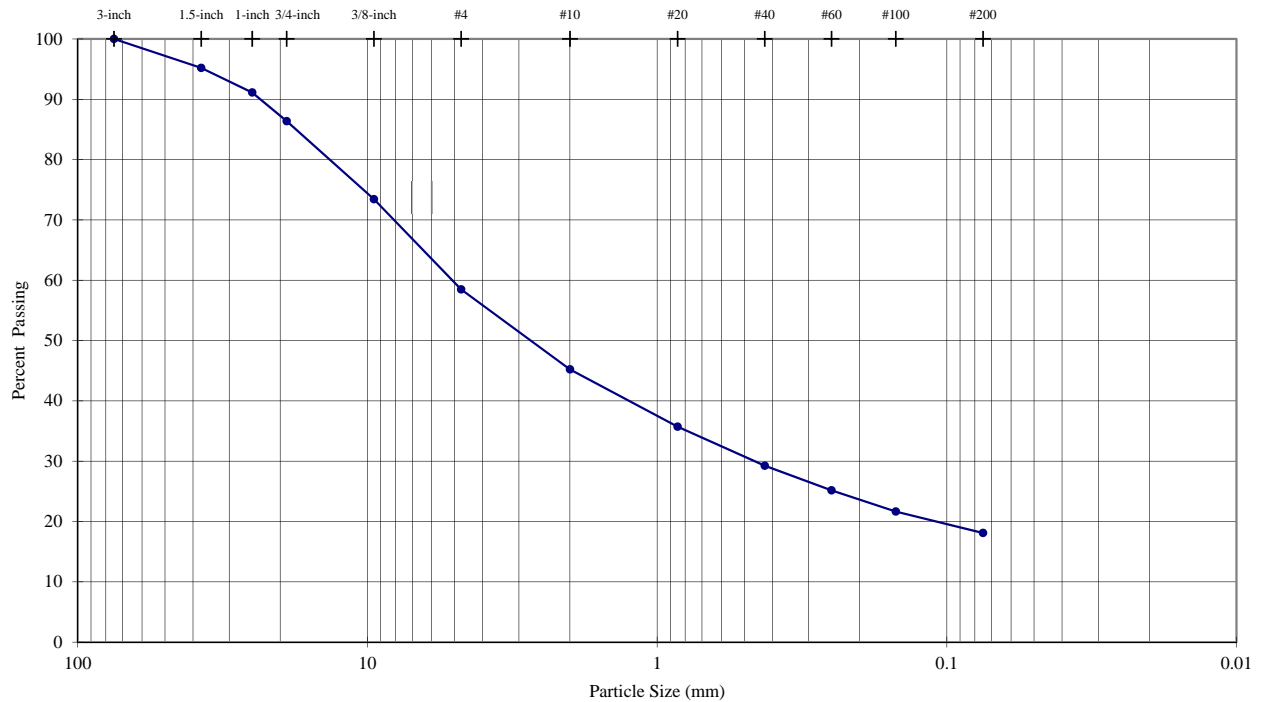
**APPENDIX A.3
GEOTECHNICAL TEST RESULTS**

APPENDIX A.3.1
GRADATION MOISTURE/DENSITY TEST REPORTS

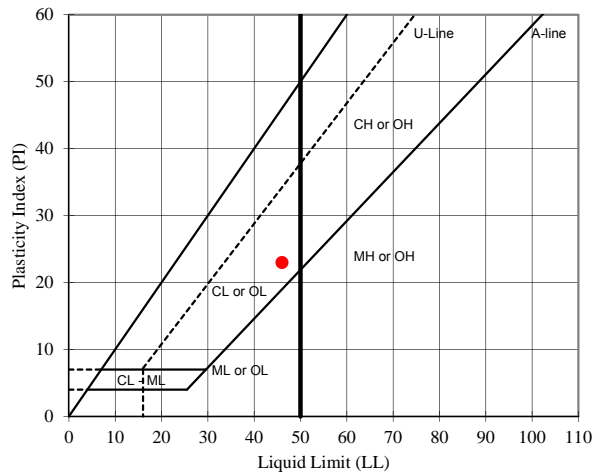
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-2**
 TYPE: **Pail/Bag**

DEPTH (ft): **0-3.5**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	13.63
1.5-inch	37.5	95.2		
1-inch	25.0	91.1		
3/4-inch	19.0	86.4	Fine Gravel	27.89
3/8-inch	9.5	73.4		
#4	4.8	58.5	Coarse Sand	13.26
#10	2.00	45.2		
#20	0.85	35.7	Medium Sand	15.98
#40	0.43	29.2		
#60	0.25	25.2	Fine Sand	11.15
#100	0.15	21.7		
#200	0.075	18.1	Silt or Clay Fines	18.10



USCS Description (ASTM D 2487):
 Clayey gravel with sand, olive brown, dry

LL	PL	PI
46	23	23

As-Received Moisture Content (%)
 --

USCS Group Symbol
 GC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

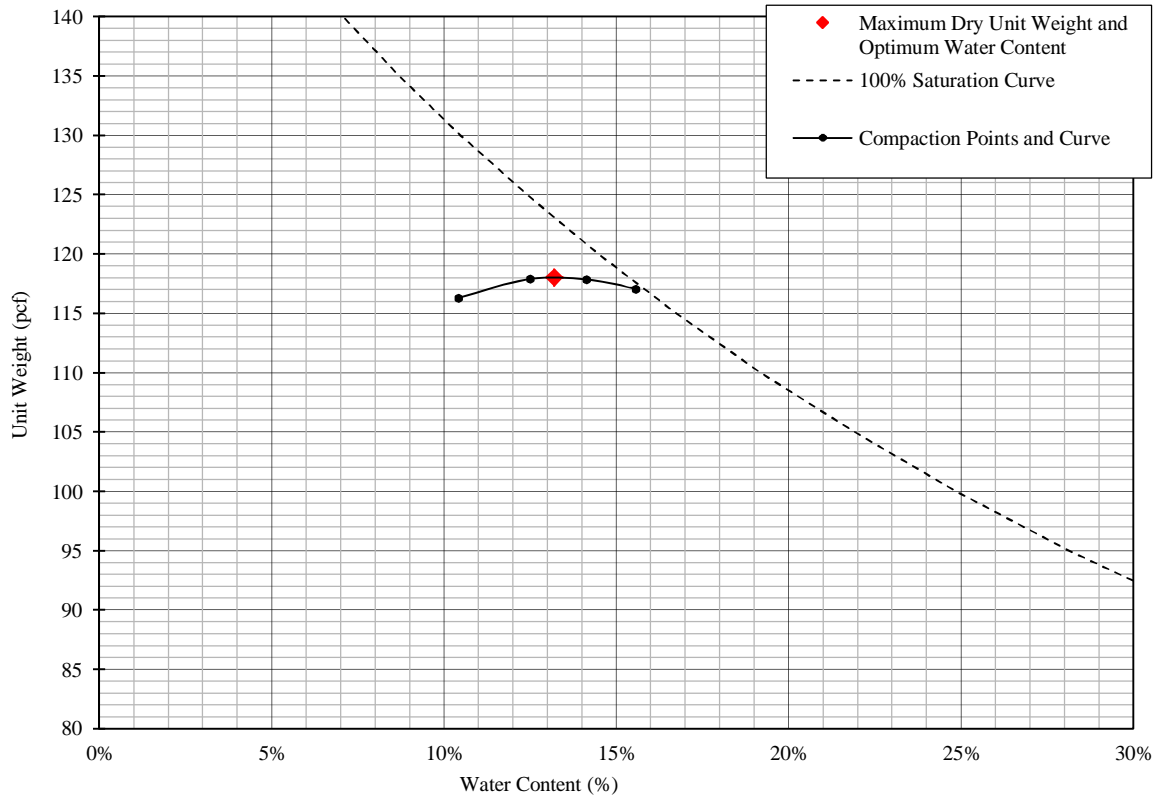
TECH	MGC
DATE	2/22/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method C

Manual Rammer Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-2**
 TYPE: **Pail/Bag**

DEPTH (ft): **0-3.5**



% Test Fraction Passing 3/4-inch Sieve	87%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.67

Maximum Dry Unit Weight (pcf)	118.0
Optimum Water Content (%)	13.2

Corrected Maximum Dry Unit Weight (pcf)	122.4
Corrected Optimum Water Content (%)	11.5

USCS Description (ASTM D 2487): Clayey gravel with sand, olive brown, dry

USCS GC

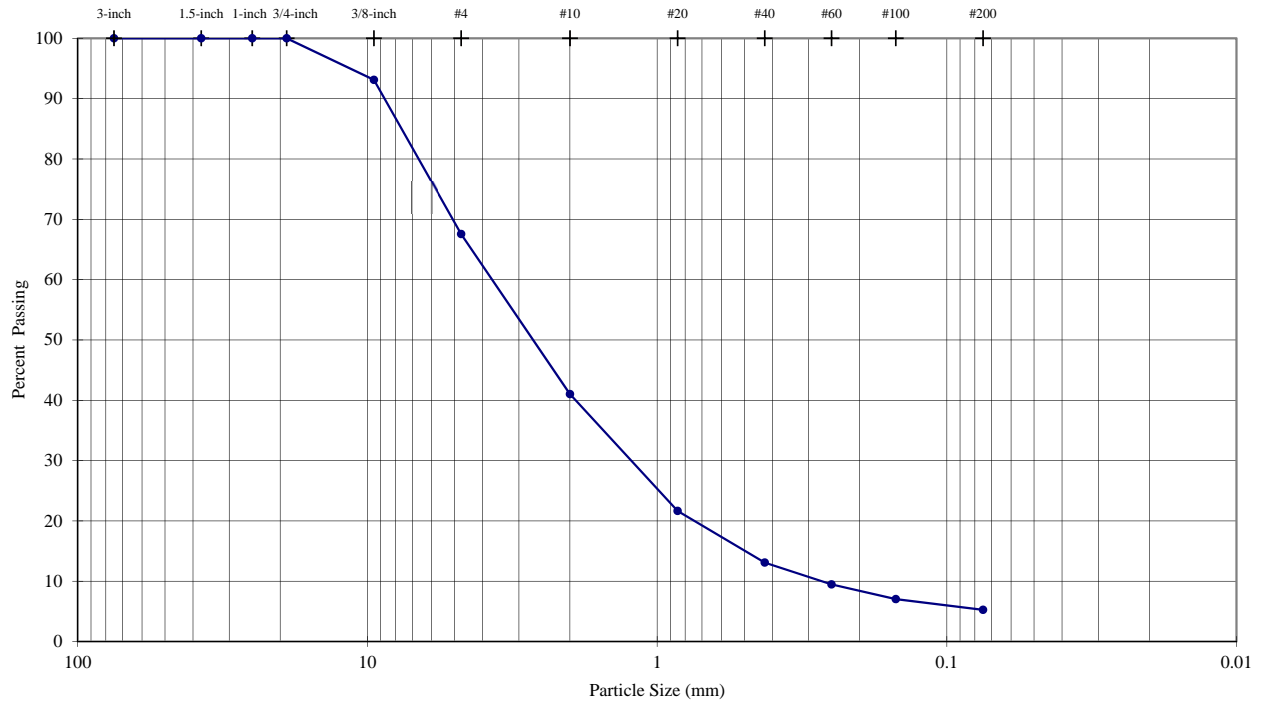
TECH	MGC
DATE	2-26-13
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

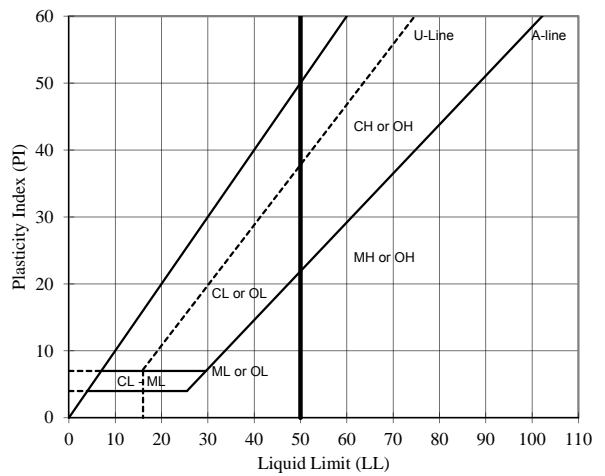
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-3**
 TYPE: **Pail**

DEPTH (ft): **0-8**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	32.44
3/8-inch	9.5	93.1		
#4	4.8	67.6	Coarse Sand	26.53
#10	2.00	41.0		
#20	0.85	21.6		
#40	0.43	13.1	Medium Sand	27.93
#60	0.25	9.5		
#100	0.15	7.0	Fine Sand	7.83
#200	0.075	5.3		
			Silt or Clay Fines	5.26



Visual Description (Golder Procedure):
Gravelly SAND, some fines, greenish gray, dry

LL	PL	PI
--	--	--
As-Received Moisture Content (%)		USCS Group Symbol
--		--

Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

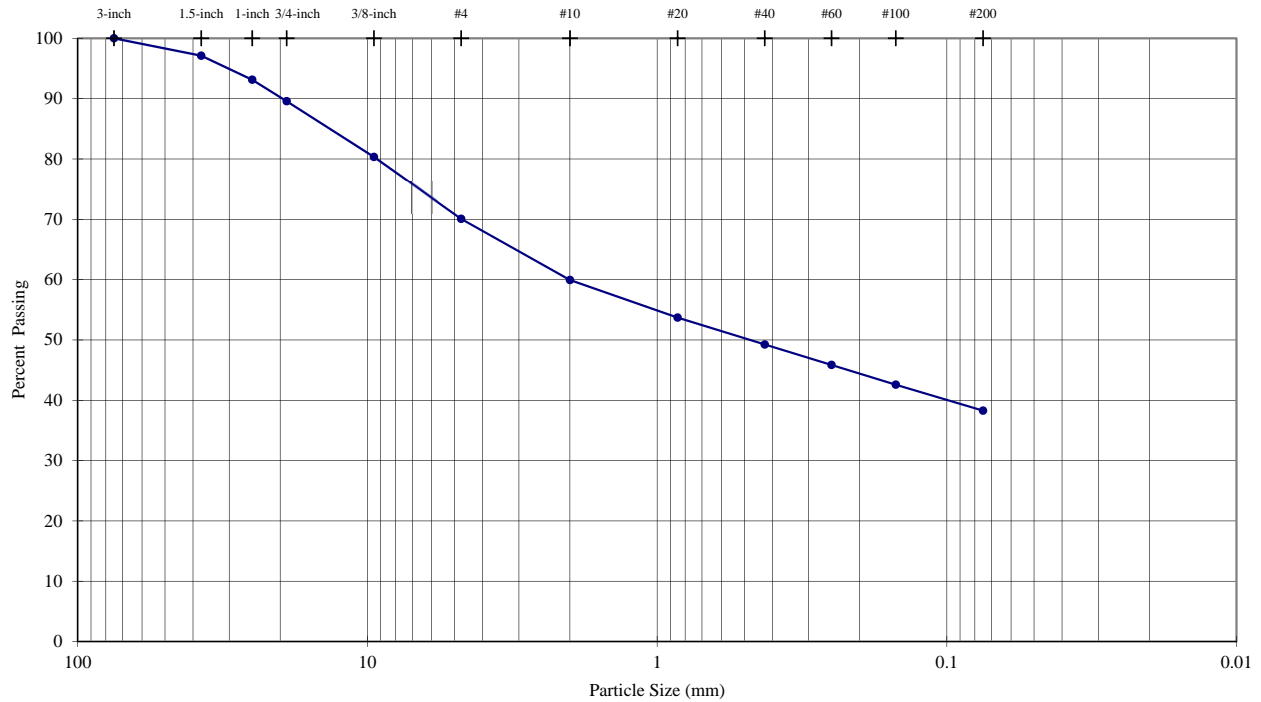
TECH **MGC**
 DATE **2/22/2013**
 REVIEW **MB**

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

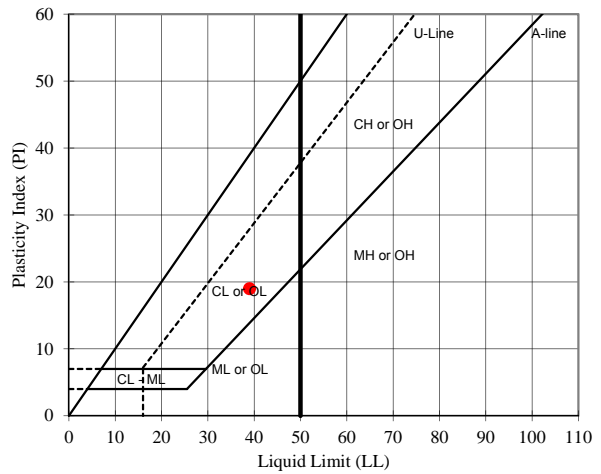
PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-10**
 TYPE: **Pail/Bag**

DEPTH (ft): **0-14.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	10.43
1.5-inch	37.5	97.1		
1-inch	25.0	93.1		
3/4-inch	19.0	89.6	Fine Gravel	19.50
3/8-inch	9.5	80.3		
#4	4.8	70.1	Coarse Sand	10.14
#10	2.00	59.9		
#20	0.85	53.7		
#40	0.43	49.2	Medium Sand	10.69
#60	0.25	45.9		
#100	0.15	42.6	Fine Sand	10.96
#200	0.075	38.3		
			Silt or Clay Fines	38.28

Sieve Analysis
(Initial Separation on No. 4 Sieve)



USCS Description (ASTM D 2487):

Clayey sand with gravel, light yellowish brown, dry

LL	PL	PI
39	20	19

As-Received Moisture Content (%)

--

USCS Group Symbol

SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

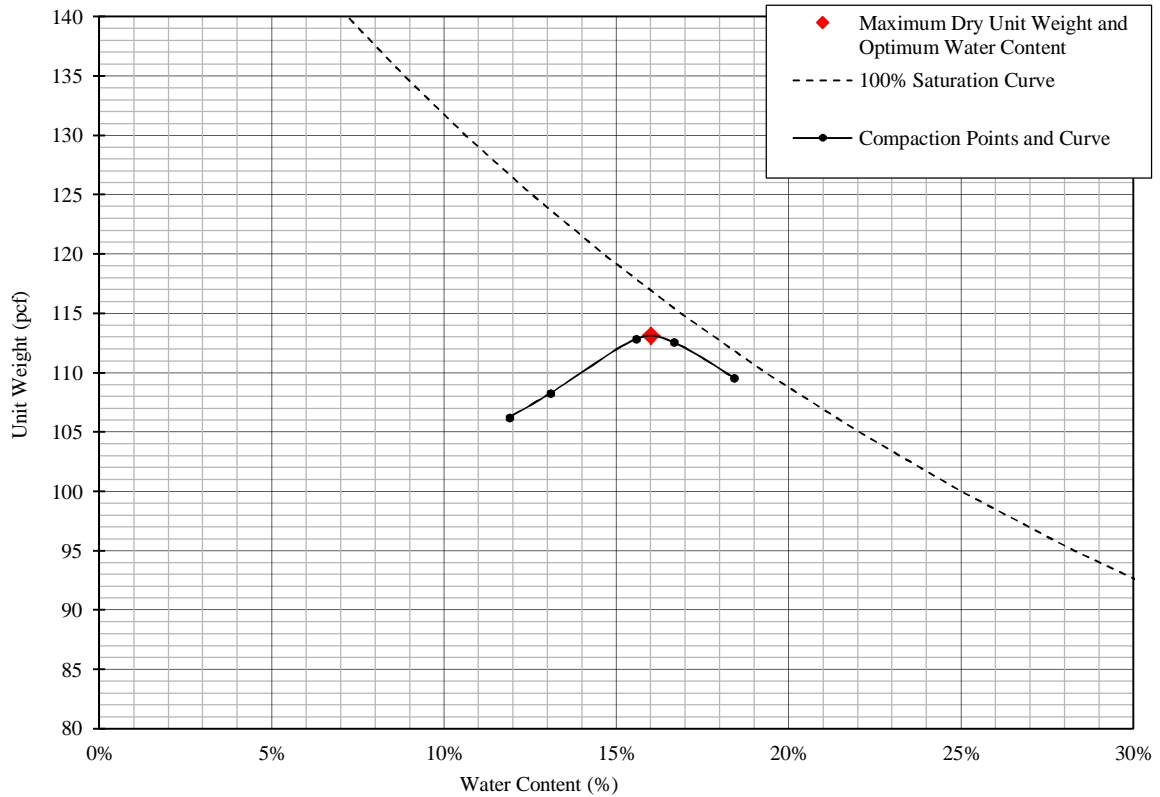
TECH	AMS/MGC
DATE	2/25/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-10**
 TYPE: **Pail/Bag**

DEPTH (ft): **0-14.5**



% Test Fraction Passing 3/8-inch Sieve	81%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.68

Maximum Dry Unit Weight (pcf)	113.1
Optimum Water Content (%)	16.0

Corrected Maximum Dry Unit Weight (pcf)	120.0
Corrected Optimum Water Content (%)	13.2

USCS Description (ASTM D 2487): Clayey sand with gravel, light yellowish brown, dry

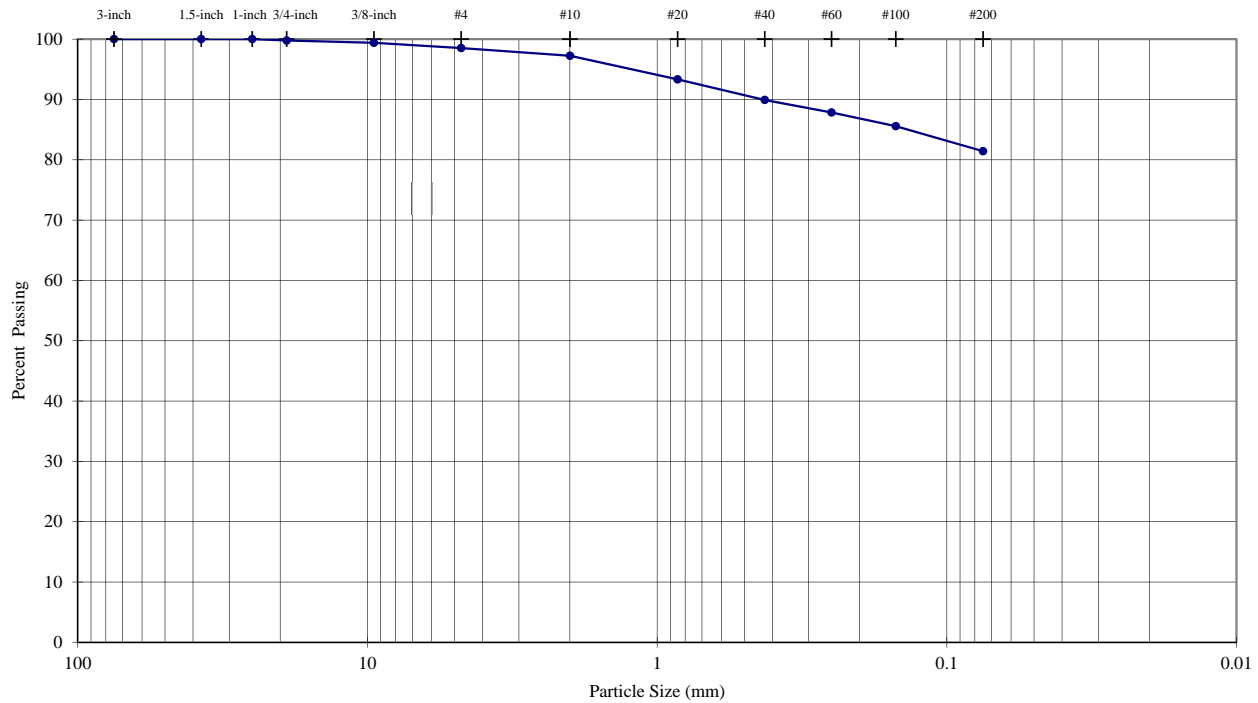
USCS SC

TECH	AMS
DATE	2-27-13
REVIEW	MB

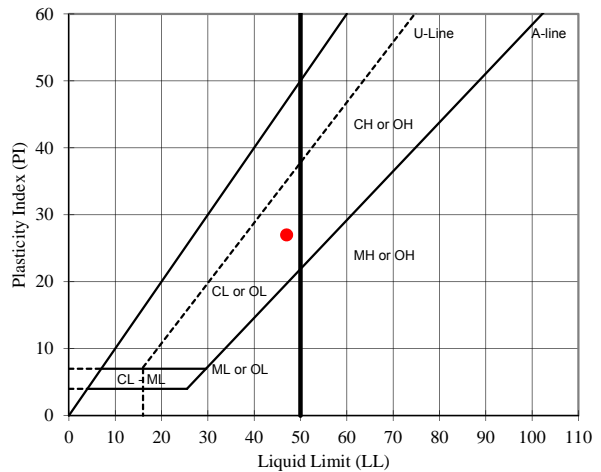
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-10**
 TYPE: **Pail**

DEPTH (ft): **19-33**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.23
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.8	Fine Gravel	1.25
3/8-inch	9.5	99.4		
#4	4.8	98.5	Coarse Sand	1.28
#10	2.00	97.2		
#20	0.85	93.3	Medium Sand	7.32
#40	0.43	89.9		
#60	0.25	87.8	Fine Sand	8.49
#100	0.15	85.6		
#200	0.075	81.4		
			Silt or Clay Fines	81.42



USCS Description (ASTM D 2487):

Lean clay with sand, yellowish red, dry

LL	PL	PI
47	20	27

As-Received Moisture Content (%)

14.4

USCS Group Symbol

CL

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

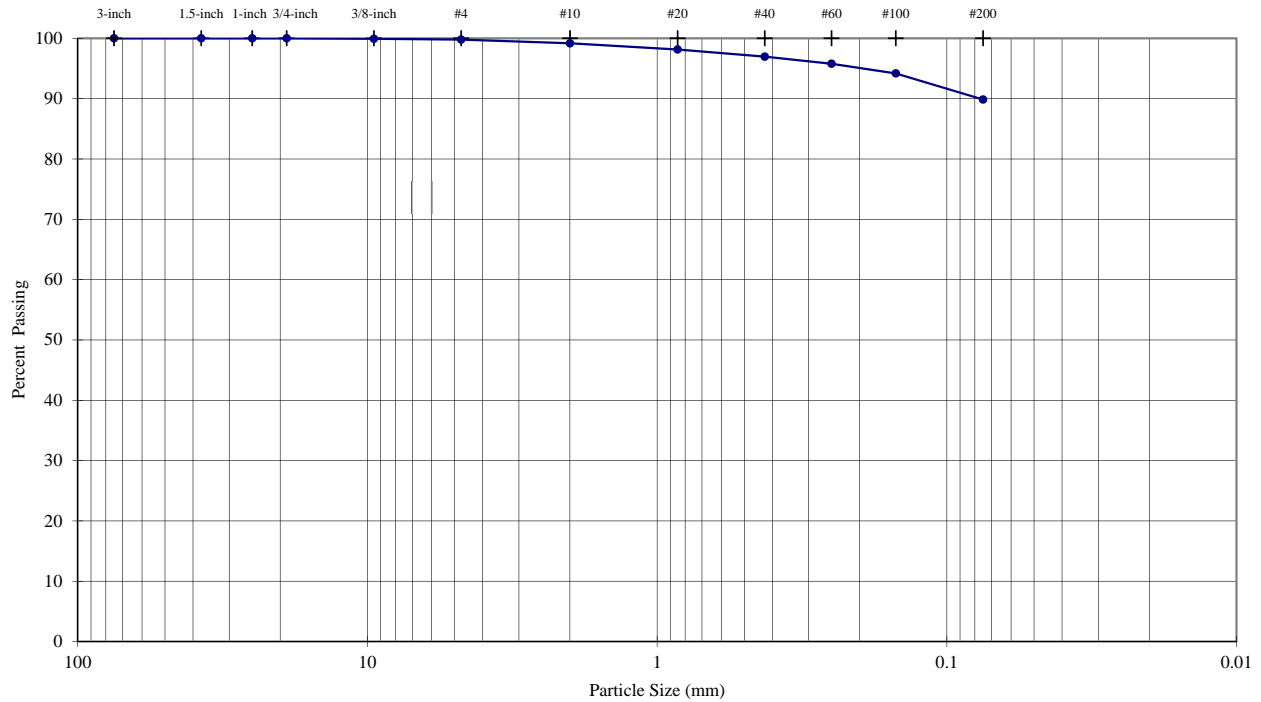
TECH	AMS
DATE	2/25/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

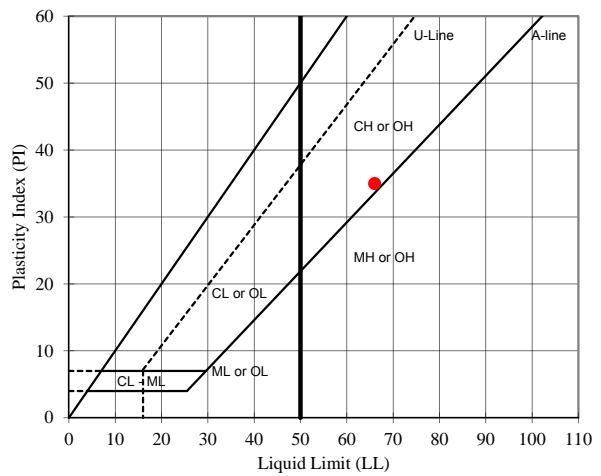
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-12**
 TYPE: **Pail**

DEPTH (ft): **33.5-48.5**



Sieve Analysis (Initial Separation on No. 4 Sieve)	Particle Size		Description	Percentage	
	Sieve	(mm)			% Passing
	3-inch	75.0	100.0	Coarse Gravel	0.00
	1.5-inch	37.5	100.0		
	1-inch	25.0	100.0		
	3/4-inch	19.0	100.0	Fine Gravel	0.21
	3/8-inch	9.5	99.9		
	#4	4.8	99.8	Coarse Sand	0.62
	#10	2.00	99.2		
	#20	0.85	98.1		
	#40	0.43	97.0	Medium Sand	2.21
	#60	0.25	95.8		
	#100	0.15	94.2	Fine Sand	7.10
	#200	0.075	89.9		
				Silt or Clay Fines	89.86



USCS Description (ASTM D 2487):
 Fat clay, dark red, moist

LL	PL	PI
66	31	35

As-Received Moisture Content (%)
 --

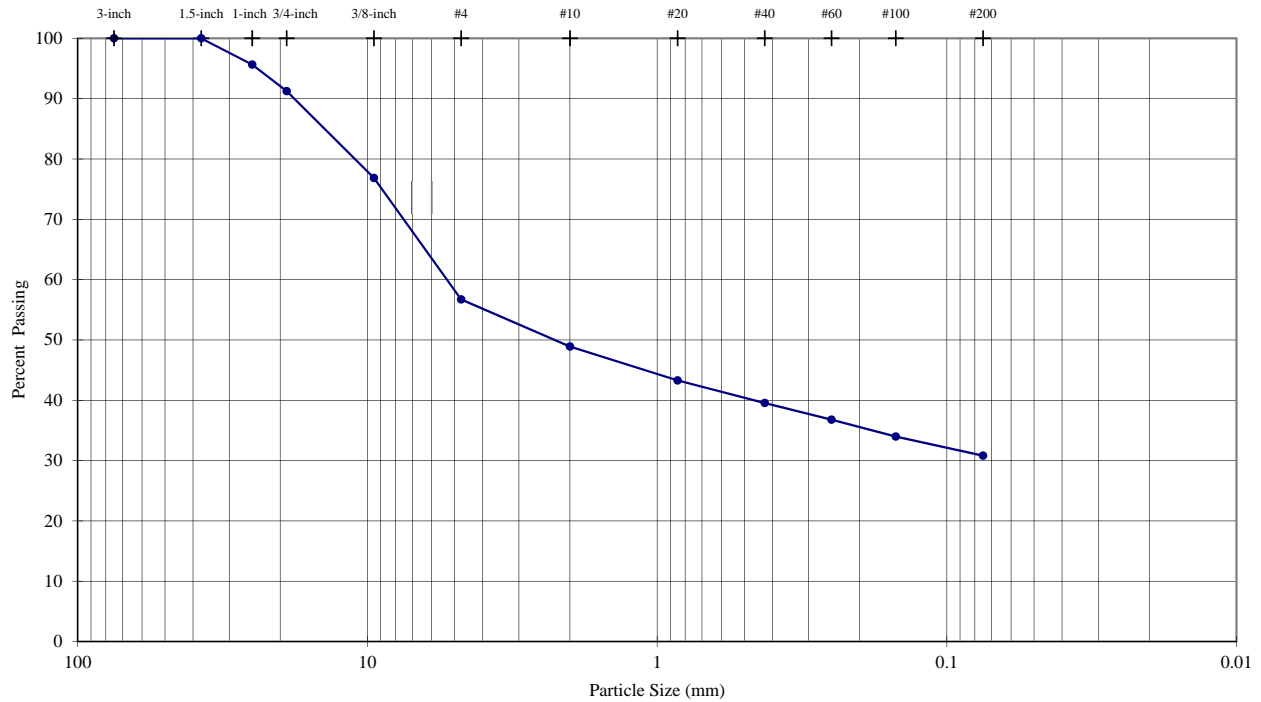
USCS Group Symbol
 CH

Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

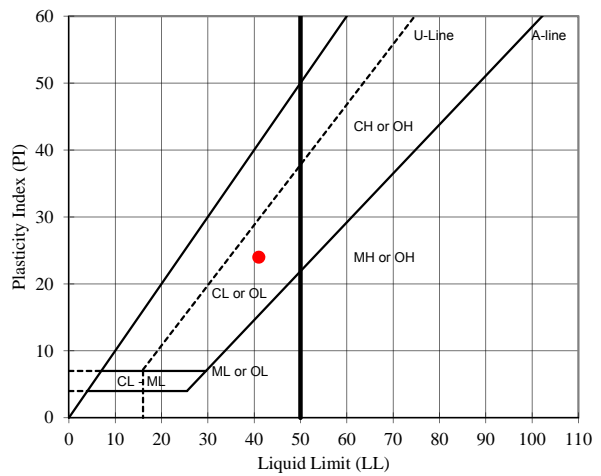
TECH AM
 DATE 2/26/2013
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-16** DEPTH (ft): **0-8.5**
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	8.75
1.5-inch	37.5	100.0		
1-inch	25.0	95.7		
3/4-inch	19.0	91.2	Fine Gravel	34.54
3/8-inch	9.5	76.9		
#4	4.8	56.7	Coarse Sand	7.80
#10	2.00	48.9		
#20	0.85	43.3		
#40	0.43	39.5	Medium Sand	9.36
#60	0.25	36.8		
#100	0.15	34.0	Fine Sand	8.73
#200	0.075	30.8		
			Silt or Clay Fines	30.81



USCS Description (ASTM D 2487):
 Clayey gravel with sand, reddish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)
 --

USCS Group Symbol
 GC

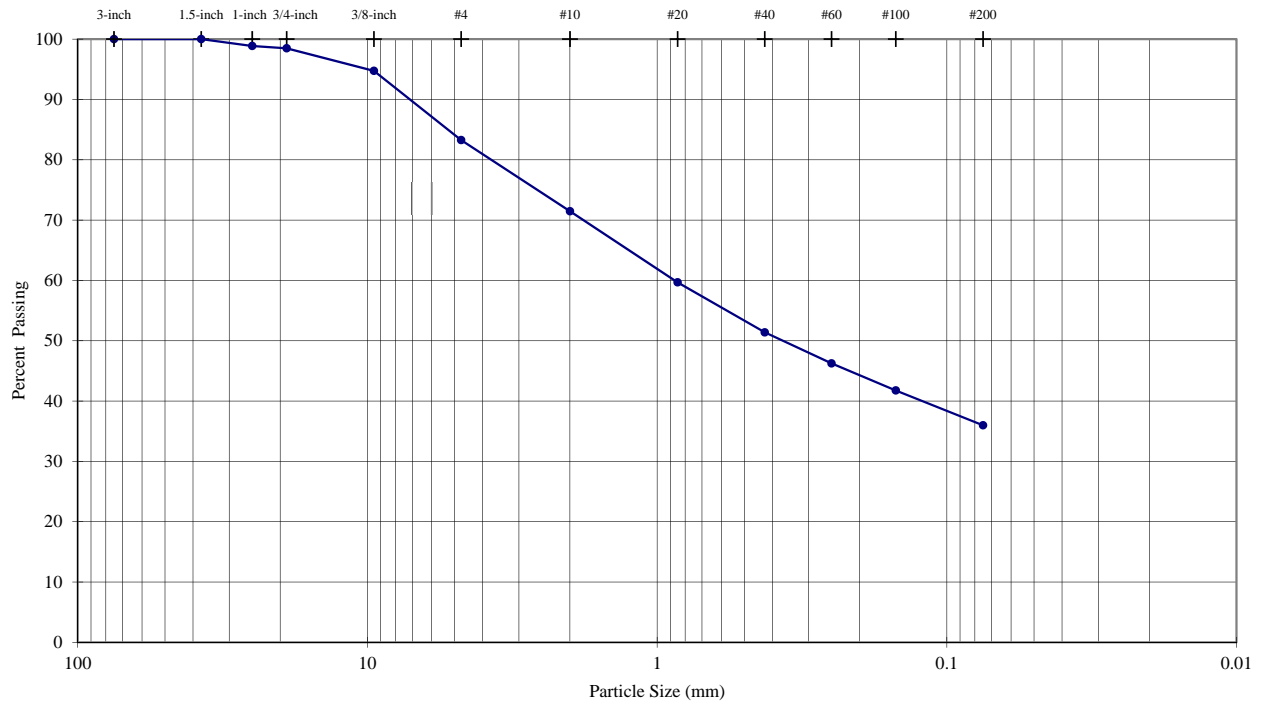
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/27/2013
REVIEW	MB

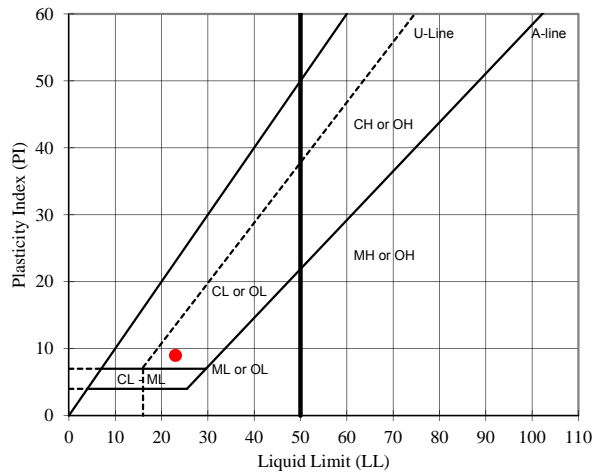
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-16**
 TYPE: **Pail**

DEPTH (ft): **29-34**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	1.52
1.5-inch	37.5	100.0		
1-inch	25.0	98.9		
3/4-inch	19.0	98.5	Fine Gravel	15.22
3/8-inch	9.5	94.7		
#4	4.8	83.3	Coarse Sand	11.79
#10	2.00	71.5		
#20	0.85	59.7		
#40	0.43	51.4	Medium Sand	20.08
#60	0.25	46.2		
#100	0.15	41.8		
#200	0.075	36.0	Fine Sand	15.41
			Silt or Clay Fines	35.98



USCS Description (ASTM D 2487):
 Clayey sand with gravel, yellowish brown, dry

LL	PL	PI
23	14	9

As-Received Moisture Content (%)
 --

USCS Group Symbol
 SC

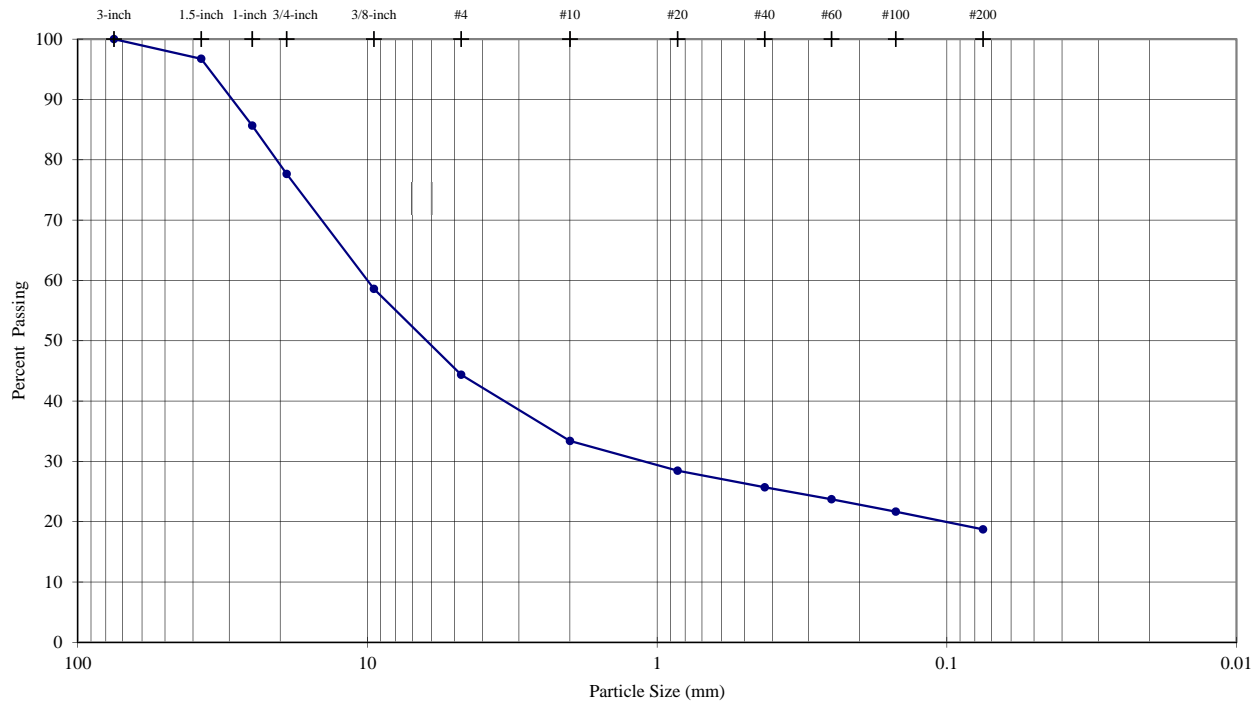
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	EH
DATE	2/26/2013
REVIEW	MB

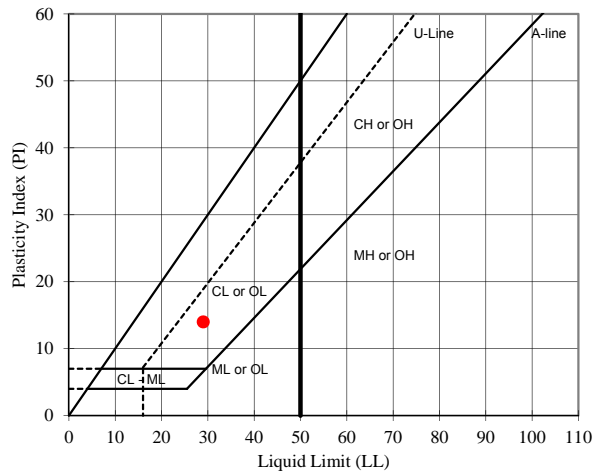
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-18**
 TYPE: **Pail**

DEPTH (ft): **2-8**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	22.35
1.5-inch	37.5	96.7		
1-inch	25.0	85.7		
3/4-inch	19.0	77.6	Fine Gravel	33.28
3/8-inch	9.5	58.6		
#4	4.8	44.4	Coarse Sand	10.98
#10	2.00	33.4		
#20	0.85	28.5	Medium Sand	7.68
#40	0.43	25.7		
#60	0.25	23.7	Fine Sand	6.98
#100	0.15	21.7		
#200	0.075	18.7	Silt or Clay Fines	18.73



USCS Description (ASTM D 2487):
 Clayey gravel with sand, yellowish brown, dry

LL	PL	PI
29	15	14

As-Received Moisture Content (%)
 --

USCS Group Symbol
 GC

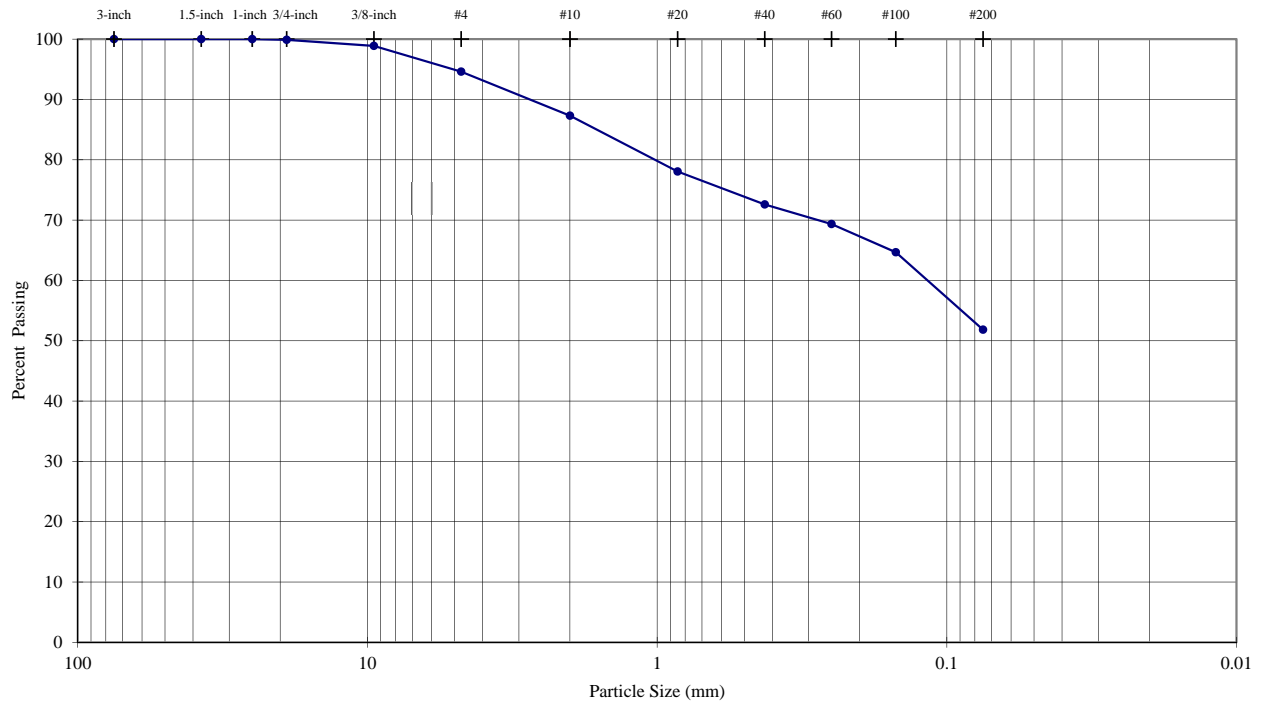
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AMS
 DATE 2/28/2013
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

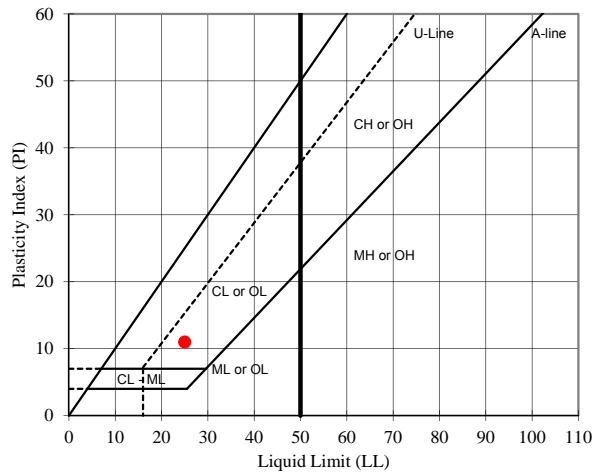
PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-18**
 TYPE: **Pail**

DEPTH (ft): **23-33.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.12
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	5.27
3/8-inch	9.5	98.9		
#4	4.8	94.6	Coarse Sand	7.30
#10	2.00	87.3		
#20	0.85	78.1	Medium Sand	14.72
#40	0.43	72.6		
#60	0.25	69.3	Fine Sand	20.76
#100	0.15	64.7		
#200	0.075	51.8		
			Silt or Clay Fines	51.84

Sieve Analysis (Initial Separation on No. 4 Sieve)



USCS Description (ASTM D 2487):
 Sandy lean clay, reddish brown, moist

LL	PL	PI
25	14	11

As-Received Moisture Content (%)

#DIV/0!

USCS Group Symbol

CL

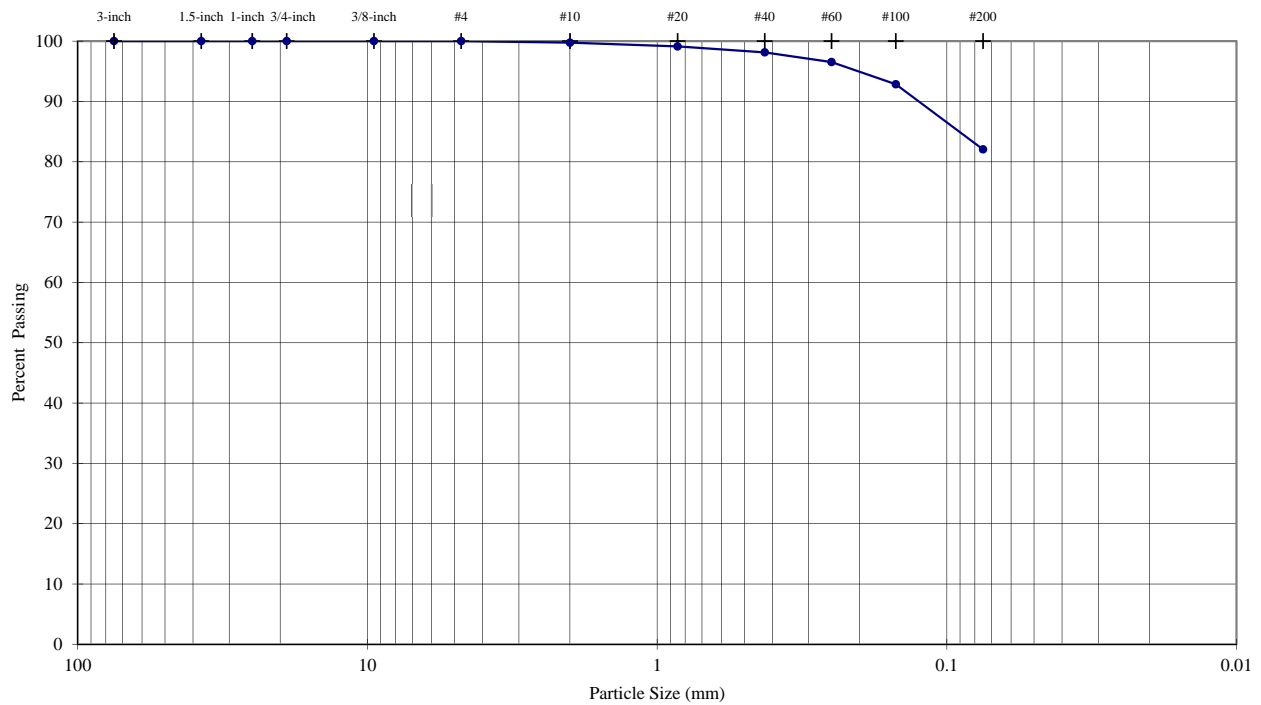
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH: AMS
 DATE: 2/25/2013
 REVIEW: MB

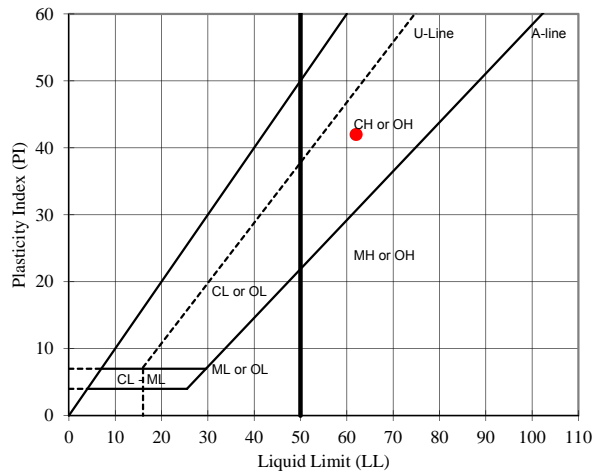
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-18**
 TYPE: **Bag**

DEPTH (ft): **43.5-48.5**



Sieve Analysis	Particle Size		Description	Percentage	
	Sieve	(mm)			% Passing
	3-inch	75.0	100.0	Coarse Gravel	0.00
	1.5-inch	37.5	100.0		
	1-inch	25.0	100.0		
	3/4-inch	19.0	100.0	Fine Gravel	0.00
	3/8-inch	9.5	100.0		
	#4	4.8	100.0	Coarse Sand	0.25
	#10	2.00	99.7		
	#20	0.85	99.1		
	#40	0.43	98.1	Medium Sand	1.61
	#60	0.25	96.5		
	#100	0.15	92.9	Fine Sand	16.07
	#200	0.075	82.1		
				Silt or Clay Fines	82.06



USCS Description (ASTM D 2487):
 Fat clay with sand, dark red, wet

LL	PL	PI
62	20	42

As-Received Moisture Content (%)
 29.6

USCS Group Symbol
 CH

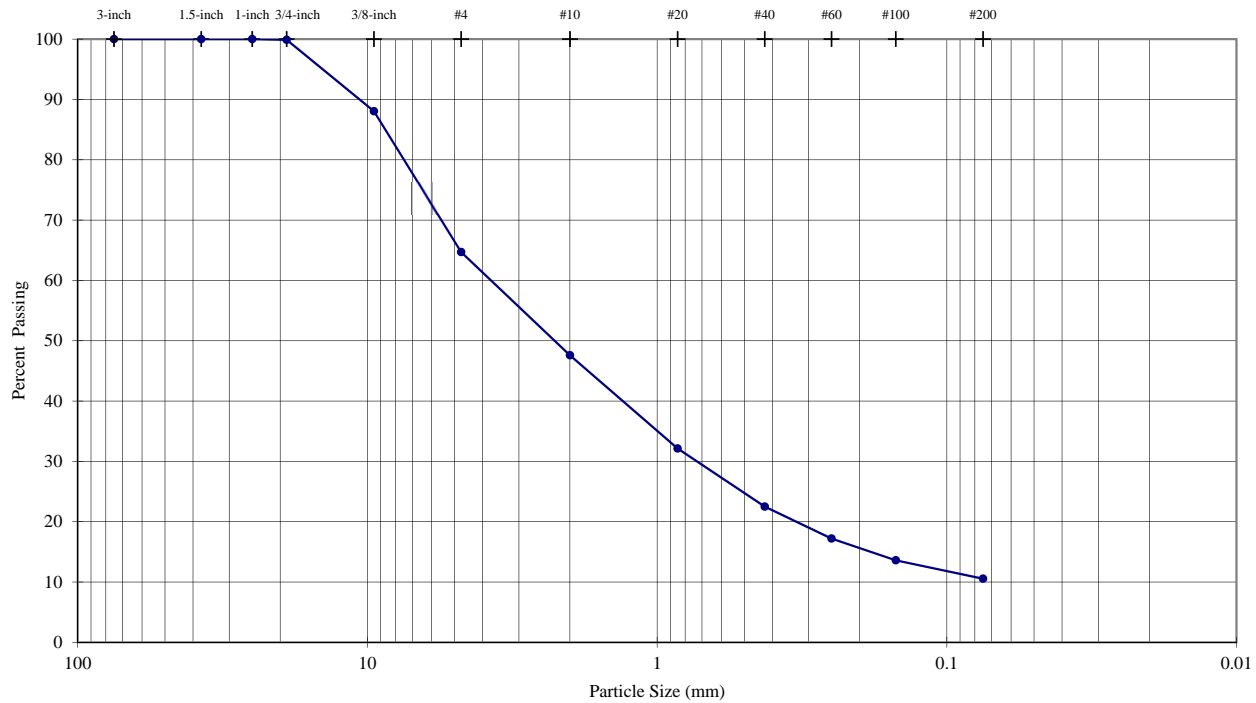
Notes: 0g of particles up to 4.8mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the wet method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AMS
 DATE 2/25/2013
 REVIEW MB

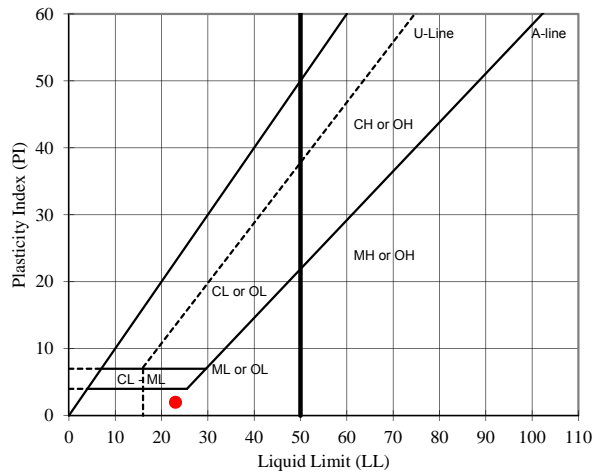
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-21**
 TYPE: **Pail**

DEPTH (ft): **0-18**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.12
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	35.18
3/8-inch	9.5	88.1		
#4	4.8	64.7	Coarse Sand	17.11
#10	2.00	47.6		
#20	0.85	32.1	Medium Sand	25.09
#40	0.43	22.5		
#60	0.25	17.2	Fine Sand	11.95
#100	0.15	13.6		
#200	0.075	10.6	Silt or Clay Fines	10.55



USCS Description (ASTM D 2487):

Well-graded sand with silt and gravel, yellowish red, dry

LL	PL	PI
23	21	2

As-Received Moisture Content (%)

--

USCS Group Symbol

SW-SM

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

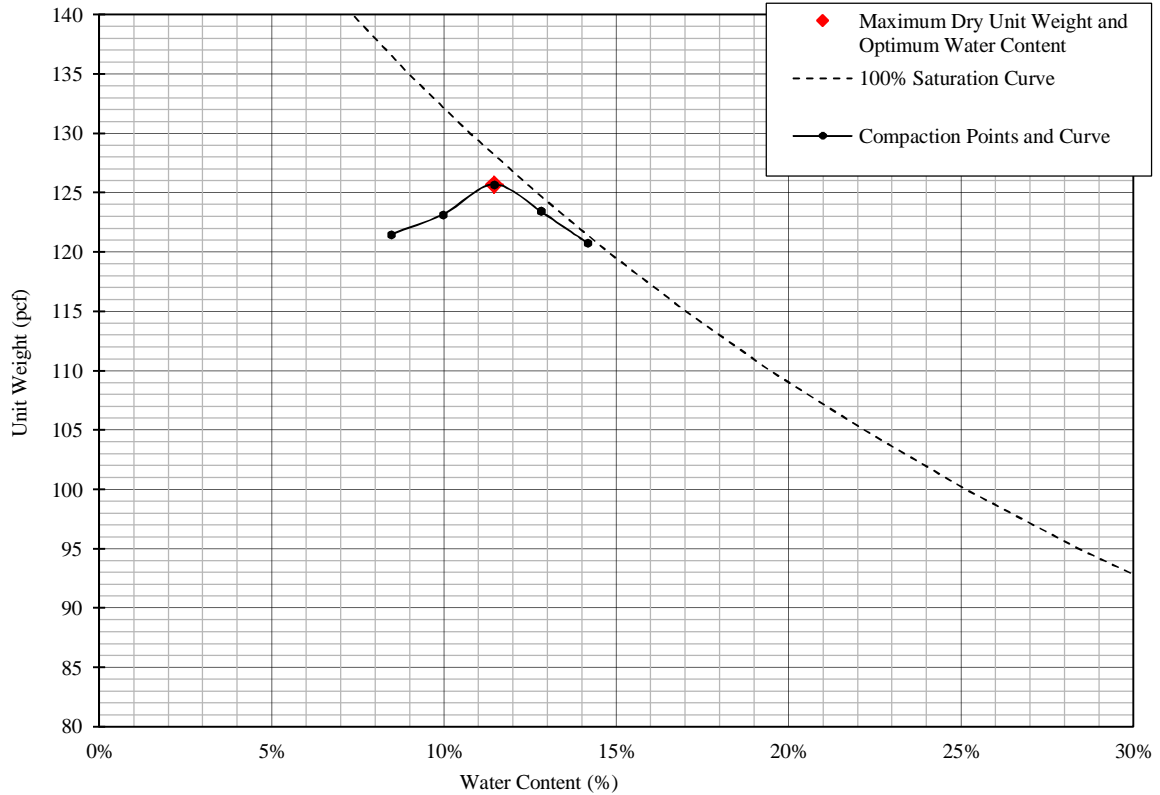
TECH	AMS
DATE	2/26/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-21**
 TYPE: **Pail**

DEPTH (ft): **0-18**



% Test Fraction Passing 3/8-inch Sieve	88%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.69

Maximum Dry Unit Weight (pcf)	125.7
Optimum Water Content (%)	11.4

Corrected Maximum Dry Unit Weight (pcf)	129.2
Corrected Optimum Water Content (%)	10.1

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, yellowish red, dry

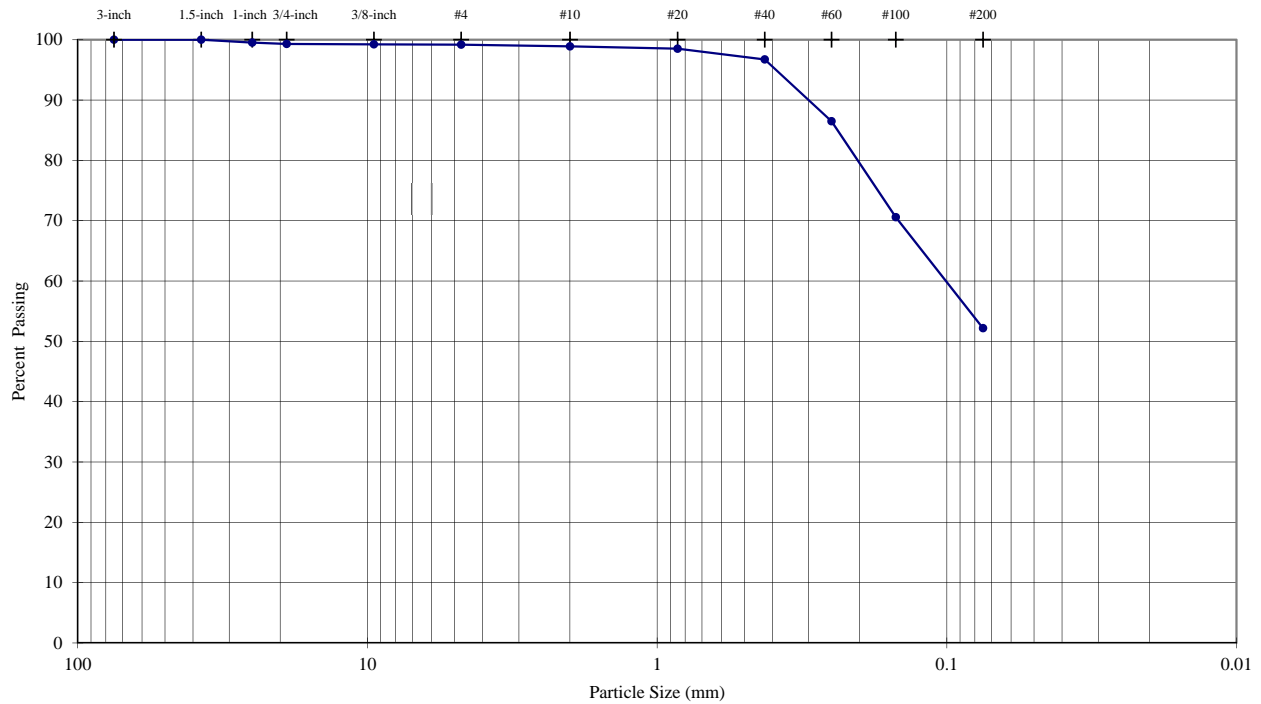
USCS SW-SM

TECH	AMS
DATE	2-27-13
REVIEW	MB

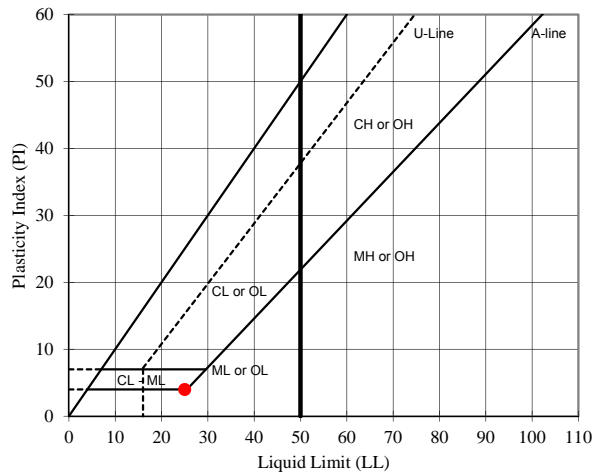
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-22**
 TYPE: **Pail**

DEPTH (ft): **5-8.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.70
1.5-inch	37.5	100.0		
1-inch	25.0	99.5		
3/4-inch	19.0	99.3	Fine Gravel	0.12
3/8-inch	9.5	99.2		
#4	4.8	99.2	Coarse Sand	0.28
#10	2.00	98.9		
#20	0.85	98.5	Medium Sand	2.17
#40	0.43	96.7		
#60	0.25	86.5	Fine Sand	44.55
#100	0.15	70.6		
#200	0.075	52.2		
			Silt or Clay Fines	52.18



USCS Description (ASTM D 2487):
 Sandy silty clay, brownish yellow, moist

As-Received Moisture Content (%): --

USCS Group Symbol: CL-ML

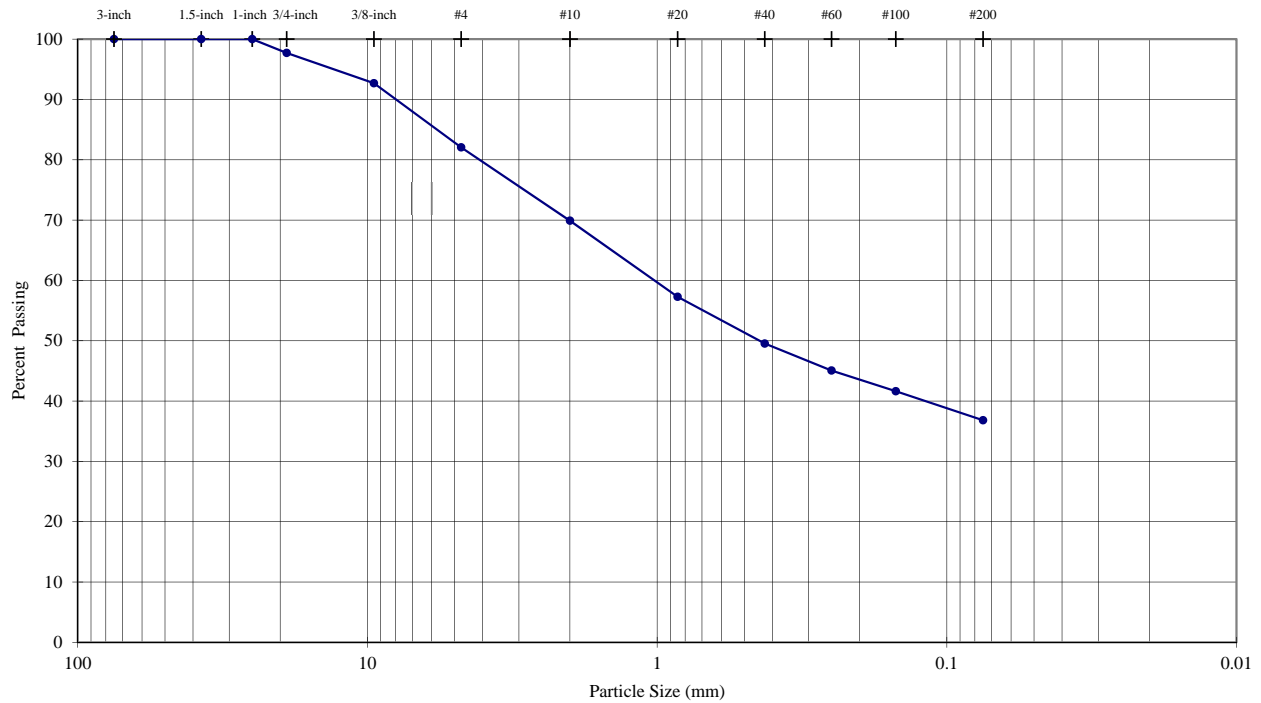
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	EH
DATE	2/26/2013
REVIEW	MB

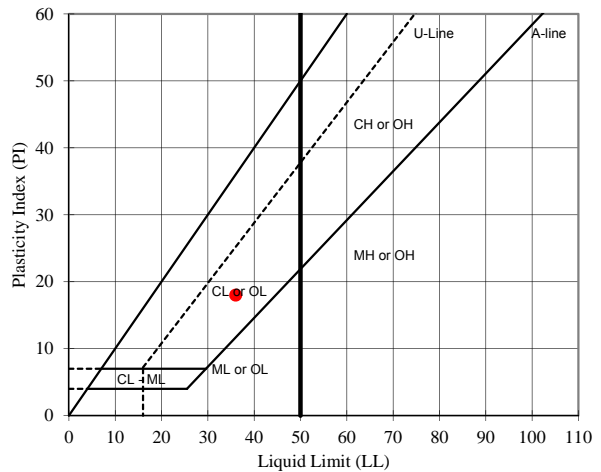
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-22**
 TYPE: **Bag**

DEPTH (ft): **28-30**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	2.29
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	97.7		
3/8-inch	9.5	92.7	Fine Gravel	15.65
#4	4.8	82.1	Coarse Sand	12.16
#10	2.00	69.9		
#20	0.85	57.3		
#40	0.43	49.5	Medium Sand	20.36
#60	0.25	45.1		
#100	0.15	41.6	Fine Sand	12.71
#200	0.075	36.8	Silt or Clay Fines	36.82



USCS Description (ASTM D 2487):
 Clayey sand with gravel, reddish brown, moist

LL	PL	PI
36	18	18

As-Received Moisture Content (%)
 10.4

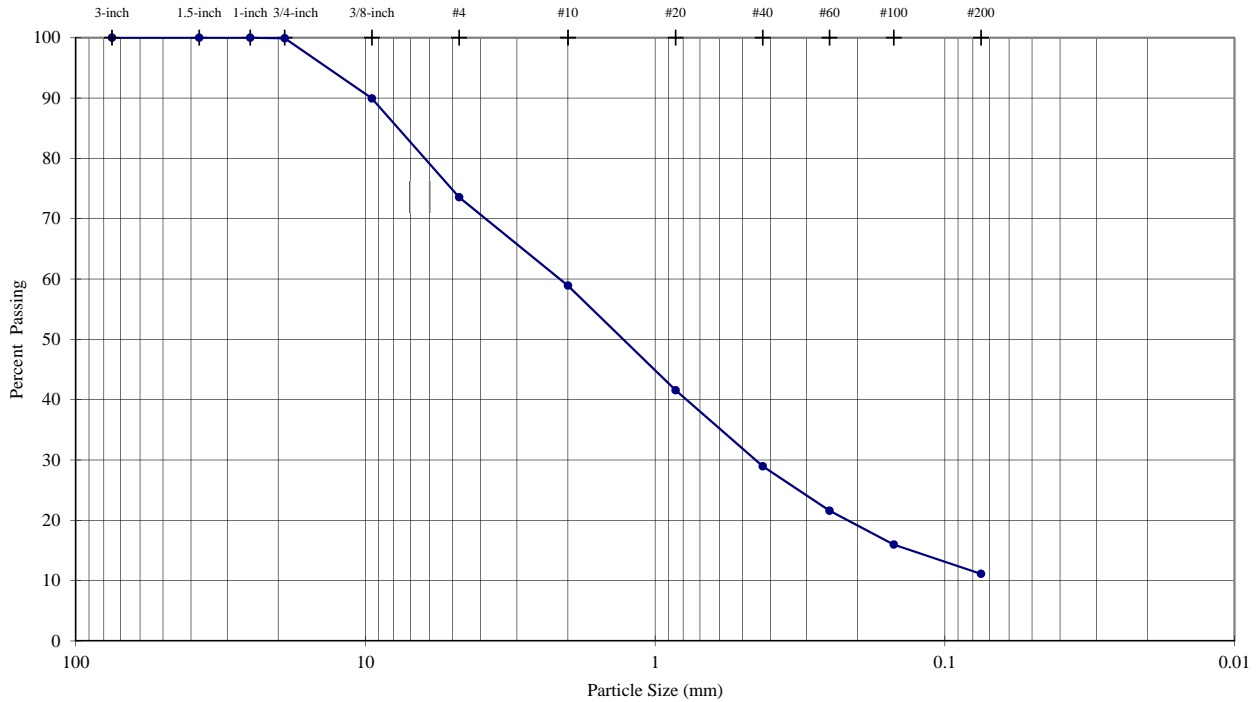
USCS Group Symbol
 SC

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

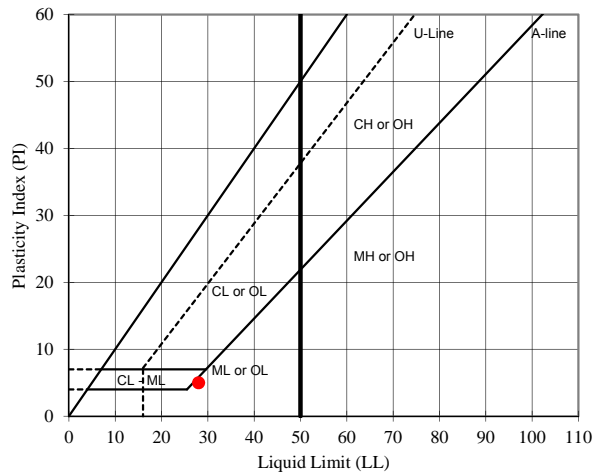
TECH: AMS
 DATE: 2/25/2013
 REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-24** DEPTH (ft): **2.5-18**
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.09
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	26.35
3/8-inch	9.5	89.9		
#4	4.8	73.6	Coarse Sand	14.65
#10	2.00	58.9		
#20	0.85	41.6	Medium Sand	29.96
#40	0.43	29.0		
#60	0.25	21.6	Fine Sand	17.83
#100	0.15	16.0		
#200	0.075	11.1		
			Silt or Clay Fines	11.12



USCS Description (ASTM D 2487):
 Well-graded sand with silt and gravel, weak red, dry

LL	PL	PI
28	23	5

As-Received Moisture Content (%)
 --

USCS Group Symbol
 SW-SM

Notes: Og of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

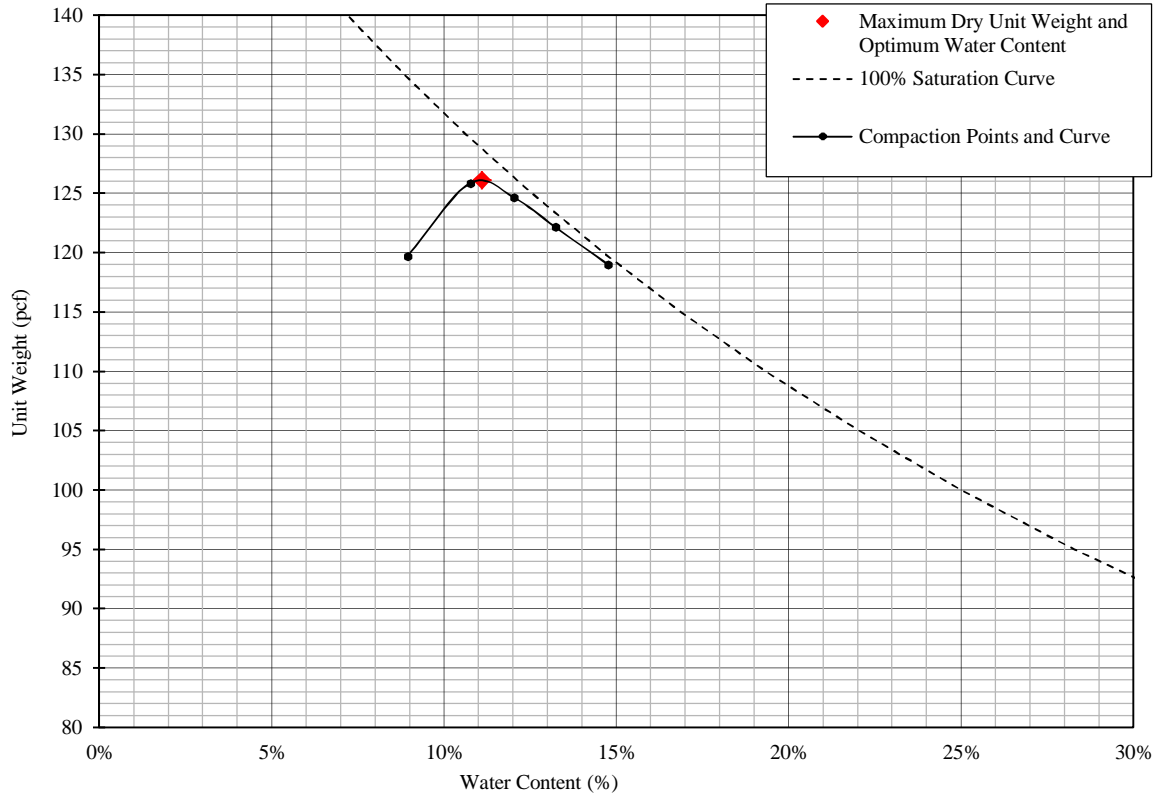
TECH	AMS
DATE	2/27/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-24**
 TYPE: **Pail**

DEPTH (ft): **2.5-18**



% Test Fraction Passing 3/8-inch Sieve	90%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.68

Maximum Dry Unit Weight (pcf)	126.1
Optimum Water Content (%)	11.1

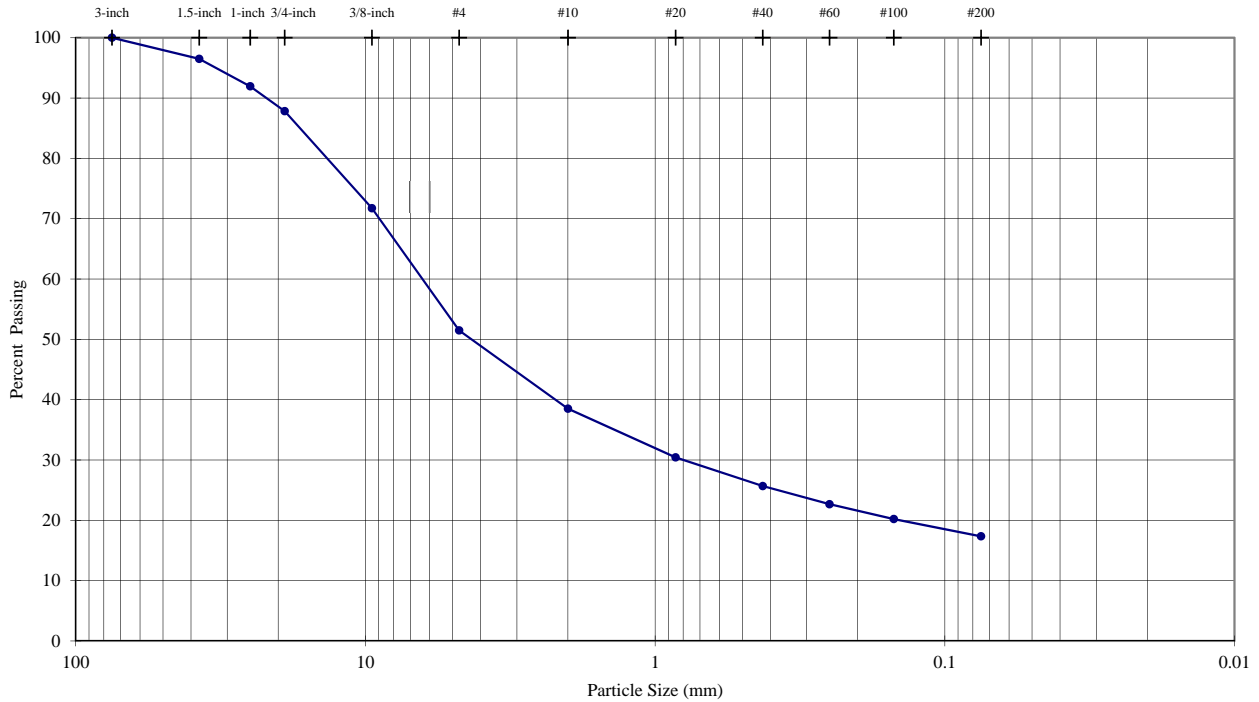
Corrected Maximum Dry Unit Weight (pcf)	128.9
Corrected Optimum Water Content (%)	10.0

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, weak red, dry
 USCS: SW-SM

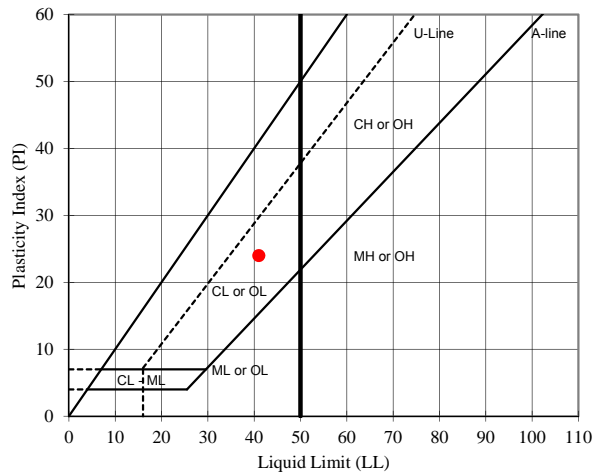
TECH	AMS
DATE	2-27-13
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
 ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-25** DEPTH (ft): **0-12.5**
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	12.17
1.5-inch	37.5	96.5		
1-inch	25.0	91.9		
3/4-inch	19.0	87.8	Fine Gravel	36.36
3/8-inch	9.5	71.7		
#4	4.8	51.5	Coarse Sand	12.97
#10	2.00	38.5		
#20	0.85	30.4	Medium Sand	12.83
#40	0.43	25.7		
#60	0.25	22.7	Fine Sand	8.33
#100	0.15	20.2		
#200	0.075	17.3		
Silt or Clay Fines				17.34



USCS Description (ASTM D 2487):
 Clayey gravel with sand, dark yellowish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)
 --

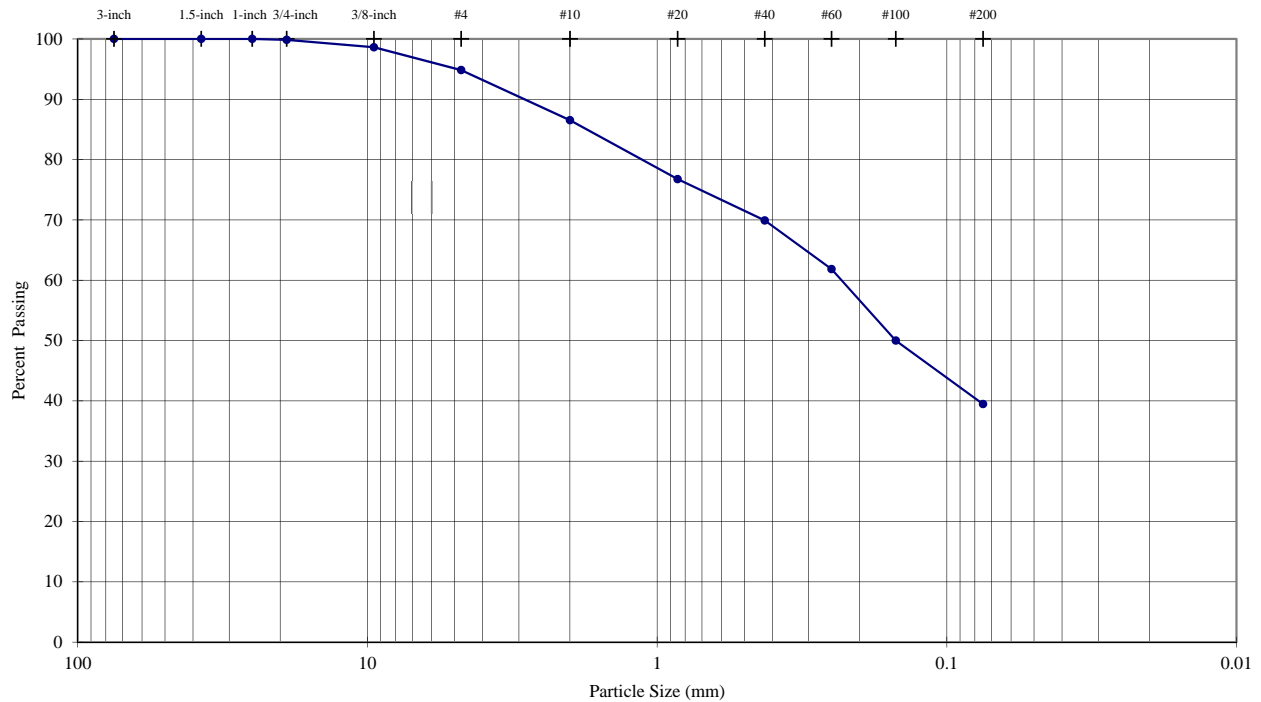
USCS Group Symbol
 GC

Notes: Og of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

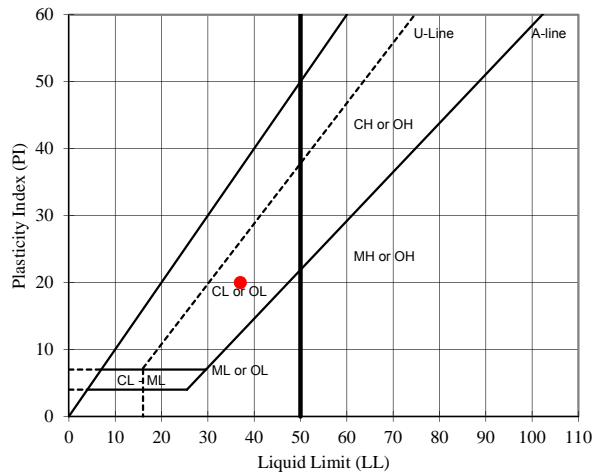
TECH **AMS**
 DATE **2/27/2013**
 REVIEW **MB**

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **BH-25** DEPTH (ft): **22-34**
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.16
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.8	Fine Gravel	5.01
3/8-inch	9.5	98.6		
#4	4.8	94.8	Coarse Sand	8.31
#10	2.00	86.5		
#20	0.85	76.8		
#40	0.43	69.9	Medium Sand	16.62
#60	0.25	61.8		
#100	0.15	50.0	Fine Sand	30.43
#200	0.075	39.5		
Silt or Clay Fines			39.47	



USCS Description (ASTM D 2487):
Clayey sand, reddish brown, moist

LL	PL	PI
37	17	20

As-Received Moisture Content (%)
#DIV/0!

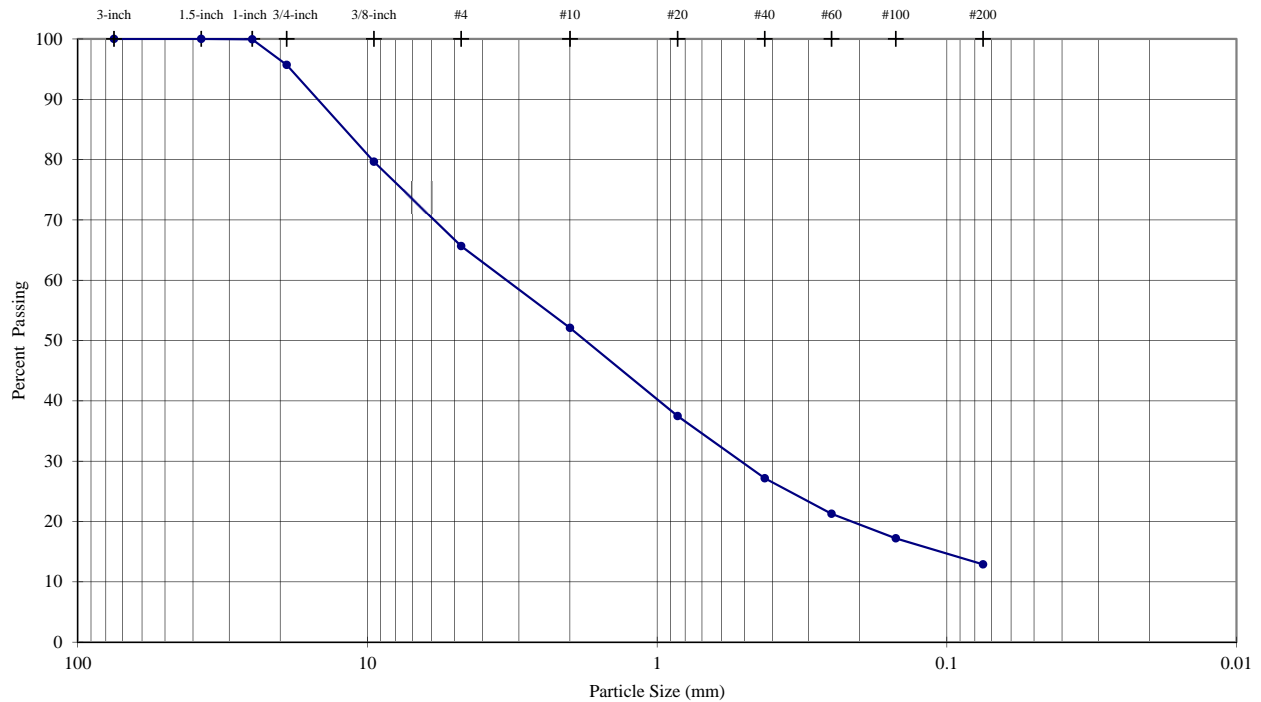
USCS Group Symbol
SC

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

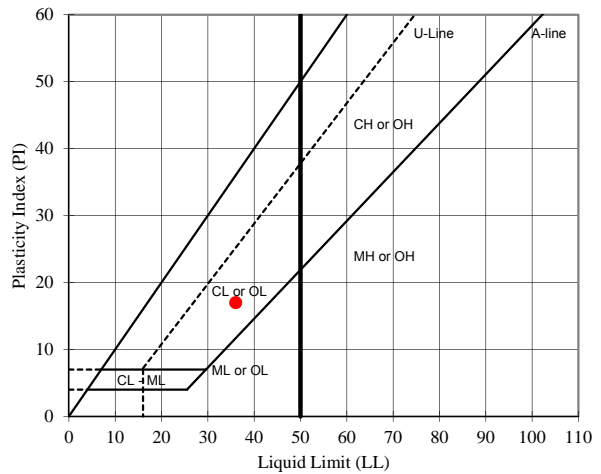
TECH	AMS
DATE	2/26/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Composite #1** DEPTH (ft): **5-13**
 TYPE: **Pails**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	4.30
1.5-inch	37.5	100.0		
1-inch	25.0	99.9		
3/4-inch	19.0	95.7	Fine Gravel	30.03
3/8-inch	9.5	79.6		
#4	4.8	65.7	Coarse Sand	13.57
#10	2.00	52.1		
#20	0.85	37.5		
#40	0.43	27.2	Medium Sand	24.93
#60	0.25	21.3		
#100	0.15	17.2	Fine Sand	14.27
#200	0.075	12.9		
			Silt or Clay Fines	12.90



USCS Description (ASTM D 2487):
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
36	19	17	2.75

As-Received Moisture Content (%) USCS Group Symbol
-- SC

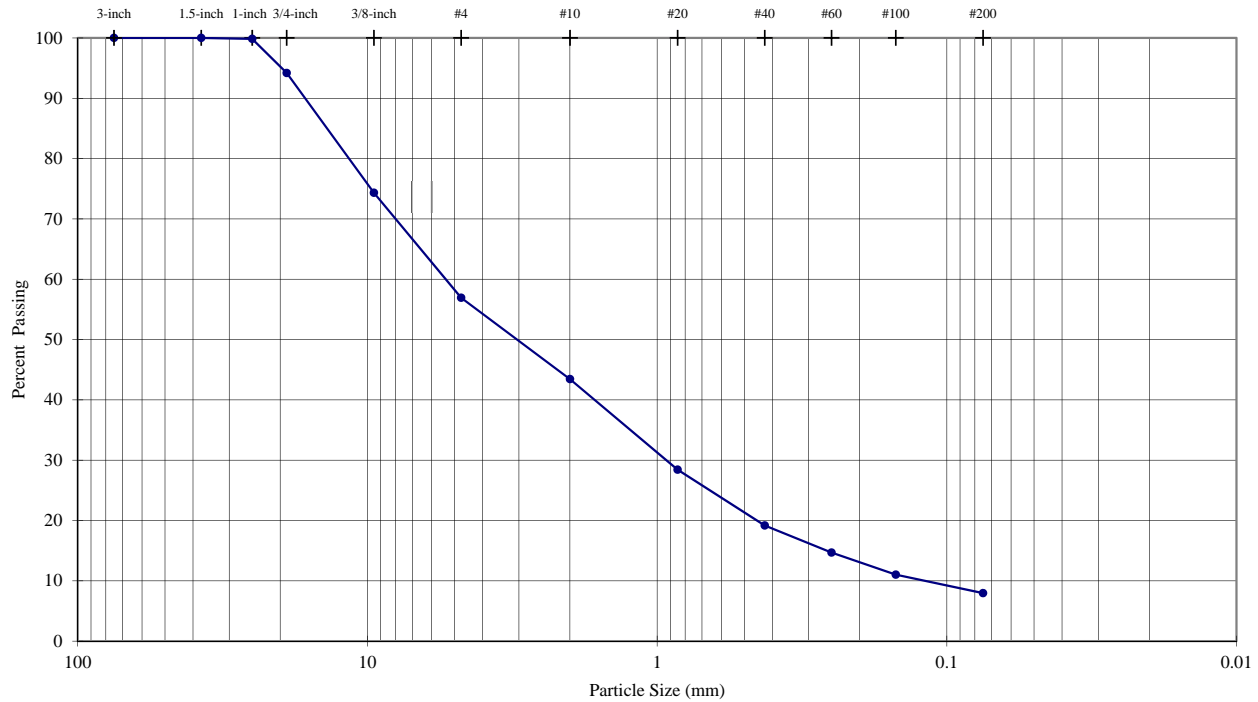
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

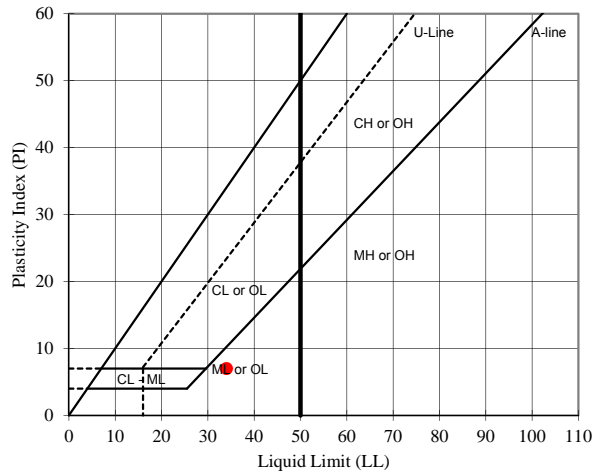
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Composite #2**
 TYPE: **Pails**

DEPTH (ft): **5-14**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	5.81
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	94.2	Fine Gravel	37.27
3/8-inch	9.5	74.3		
#4	4.8	56.9	Coarse Sand	13.48
#10	2.00	43.4		
#20	0.85	28.4	Medium Sand	24.28
#40	0.43	19.2		
#60	0.25	14.7	Fine Sand	11.20
#100	0.15	11.0		
#200	0.075	8.0		
			Silt or Clay Fines	7.96



USCS Description (ASTM D 2487):

Well-graded sand with silt and gravel, brown, dry

LL	PL	PI	SpG
34	27	7	2.74

As-Received Moisture Content (%)

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USCS Group Symbol

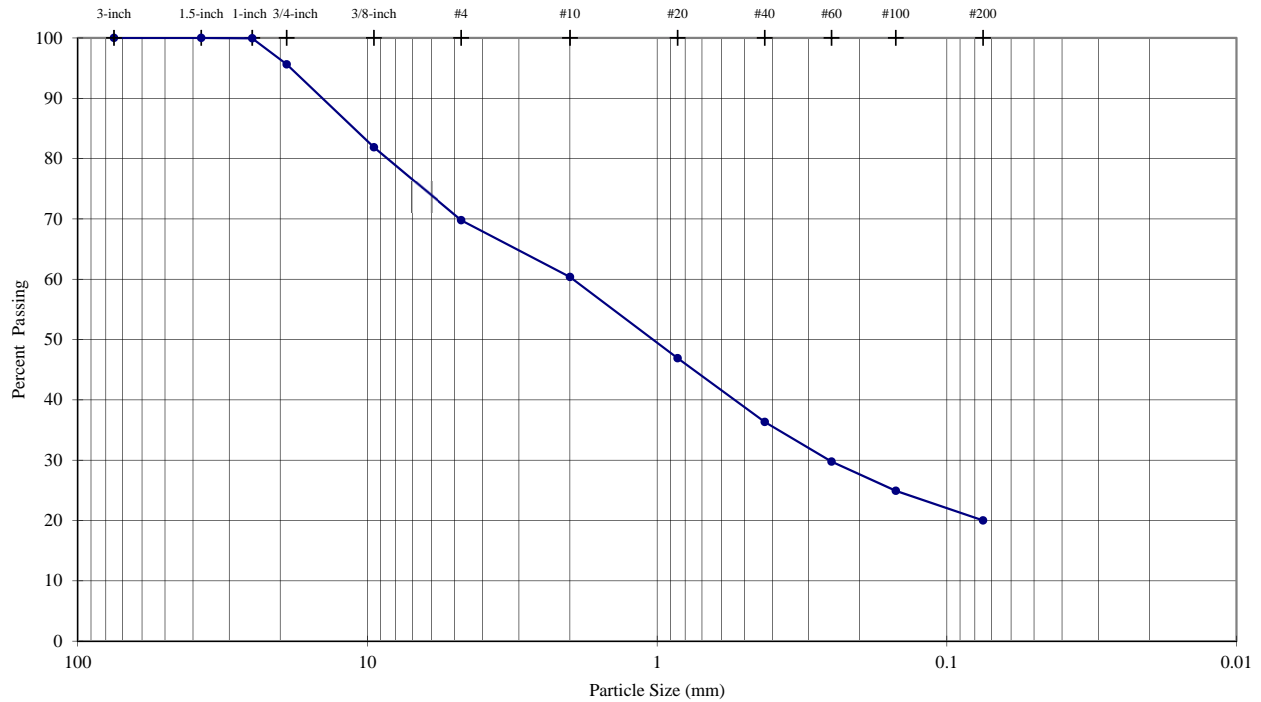
GW-GM

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

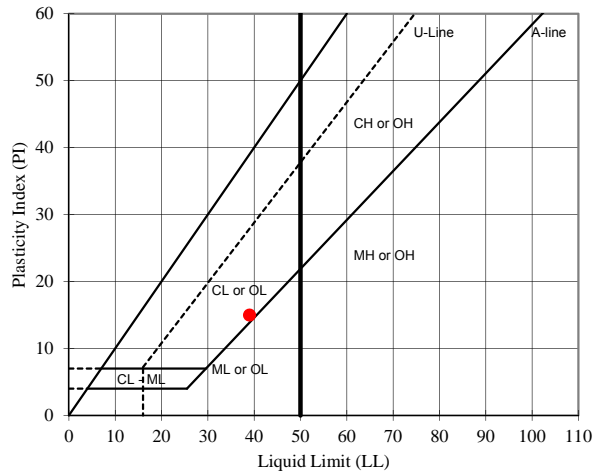
TECH	AMS
DATE	3/4/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
 ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Composite #3** DEPTH (ft): **2-3**
 TYPE: **Pails**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	4.38
1.5-inch	37.5	100.0		
1-inch	25.0	99.9		
3/4-inch	19.0	95.6	Fine Gravel	25.84
3/8-inch	9.5	81.9		
#4	4.8	69.8	Coarse Sand	9.40
#10	2.00	60.4		
#20	0.85	46.9	Medium Sand	24.04
#40	0.43	36.3		
#60	0.25	29.8	Fine Sand	16.32
#100	0.15	24.9		
#200	0.075	20.0		
			Silt or Clay Fines	20.01



USCS Description (ASTM D 2487):
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
39	24	15	2.67

As-Received Moisture Content (%)

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USCS Group Symbol

SC

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

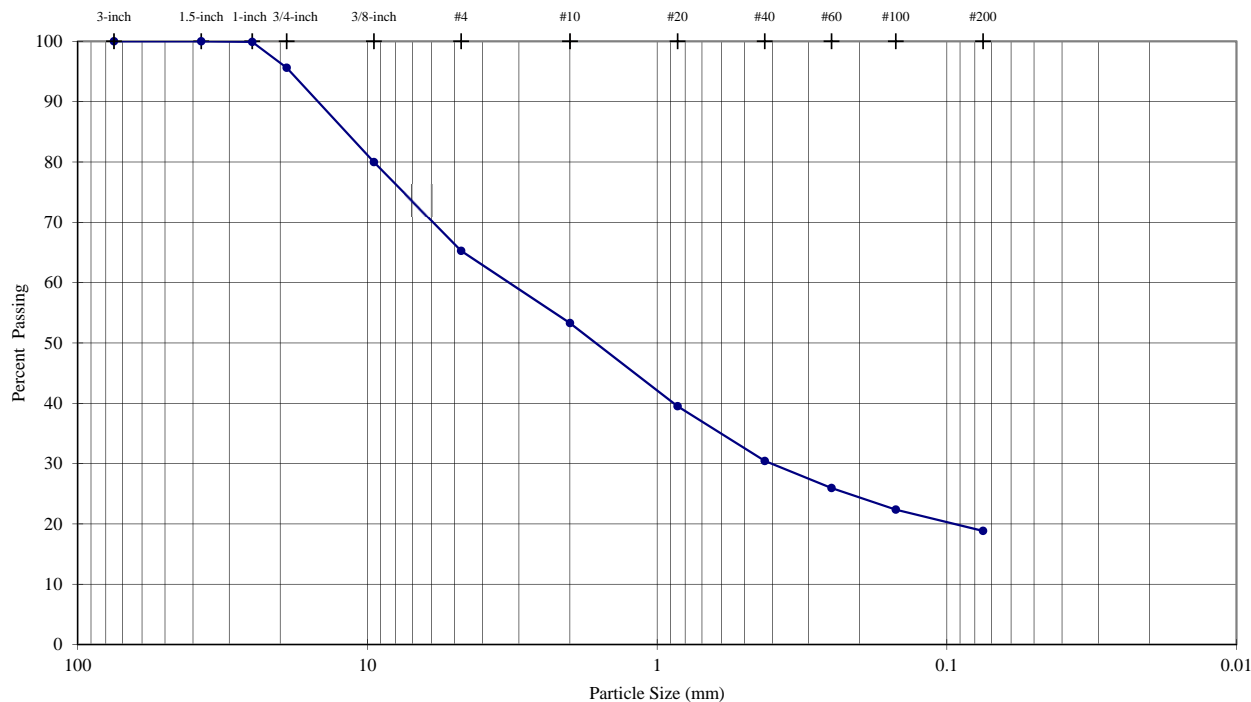
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**

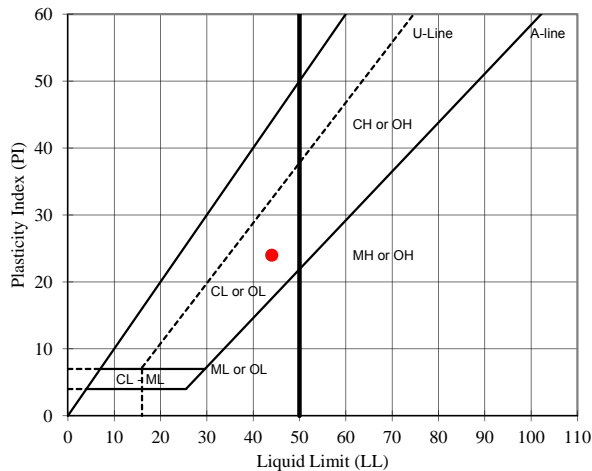
SAMPLE ID: **Composite #4**

DEPTH (ft): **0-10**

TYPE: **Pails**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	4.38
1.5-inch	37.5	100.0		
1-inch	25.0	99.9		
3/4-inch	19.0	95.6	Fine Gravel	30.35
3/8-inch	9.5	80.0		
#4	4.8	65.3	Coarse Sand	11.97
#10	2.00	53.3		
#20	0.85	39.5		
#40	0.43	30.4	Medium Sand	22.86
#60	0.25	26.0		
#100	0.15	22.4	Fine Sand	11.60
#200	0.075	18.8		
			Silt or Clay Fines	18.84



USCS Description (ASTM D 2487):
Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
44	20	24	2.70

As-Received Moisture Content (%)
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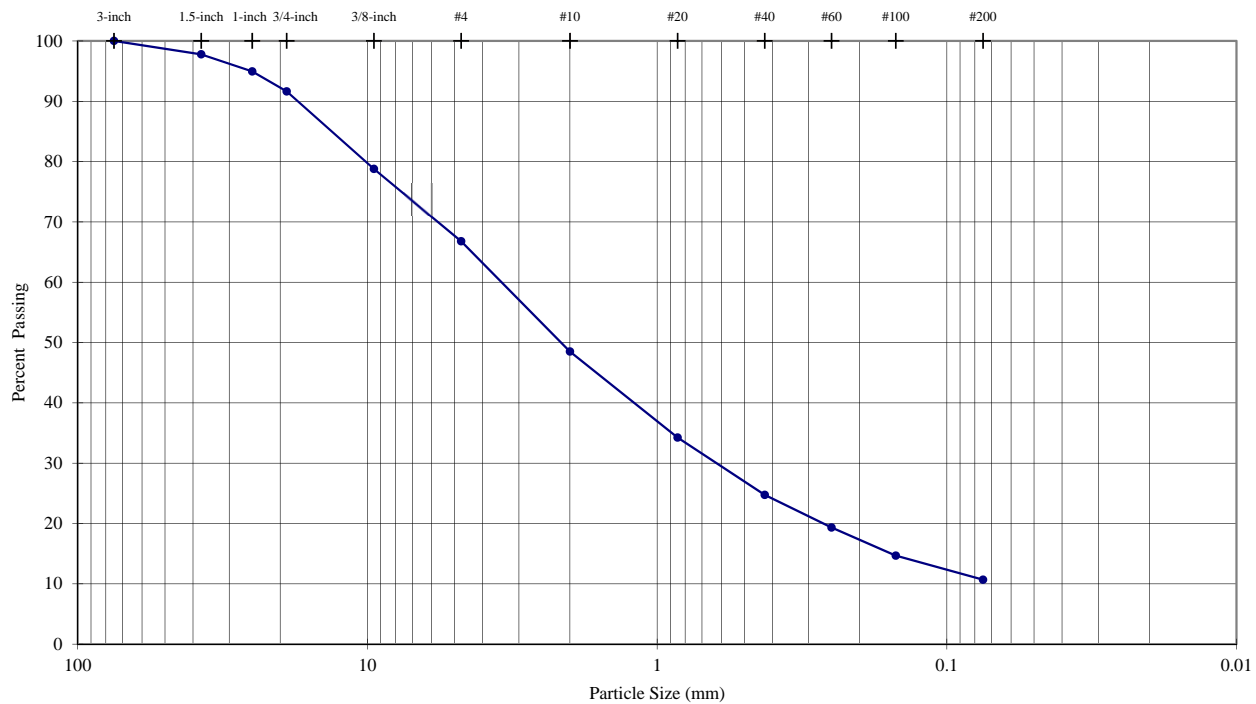
USCS Group Symbol
SC

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
Sample prepared for Atterberg Limits testing by the dry method
Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

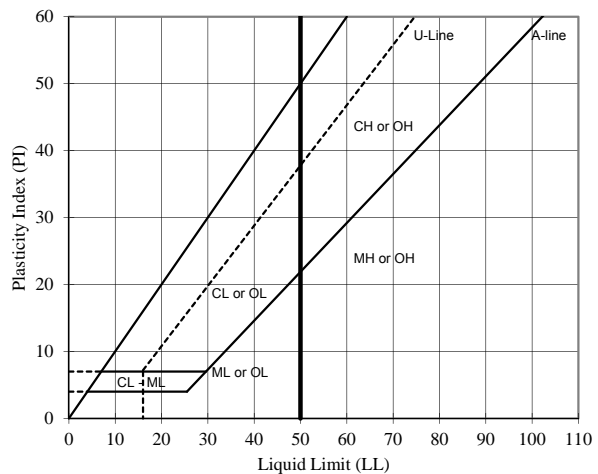
TECH	AMS
DATE	3/4/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-5** DEPTH (ft): **3-7**
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	8.37
1.5-inch	37.5	97.8		
1-inch	25.0	94.9		
3/4-inch	19.0	91.6	Fine Gravel	24.84
3/8-inch	9.5	78.8		
#4	4.8	66.8	Coarse Sand	18.28
#10	2.00	48.5		
#20	0.85	34.2		
#40	0.43	24.7	Medium Sand	23.77
#60	0.25	19.3		
#100	0.15	14.7	Fine Sand	14.07
#200	0.075	10.7		
			Silt or Clay Fines	10.68



Visual Description (Golder Procedure):
 gravelly SAND, some non-plastic fines, yellowish brown, dry

LL	PL	PI
#VALUE!	#DIV/0!	#VALUE!

As-Received Moisture Content (%)
 --

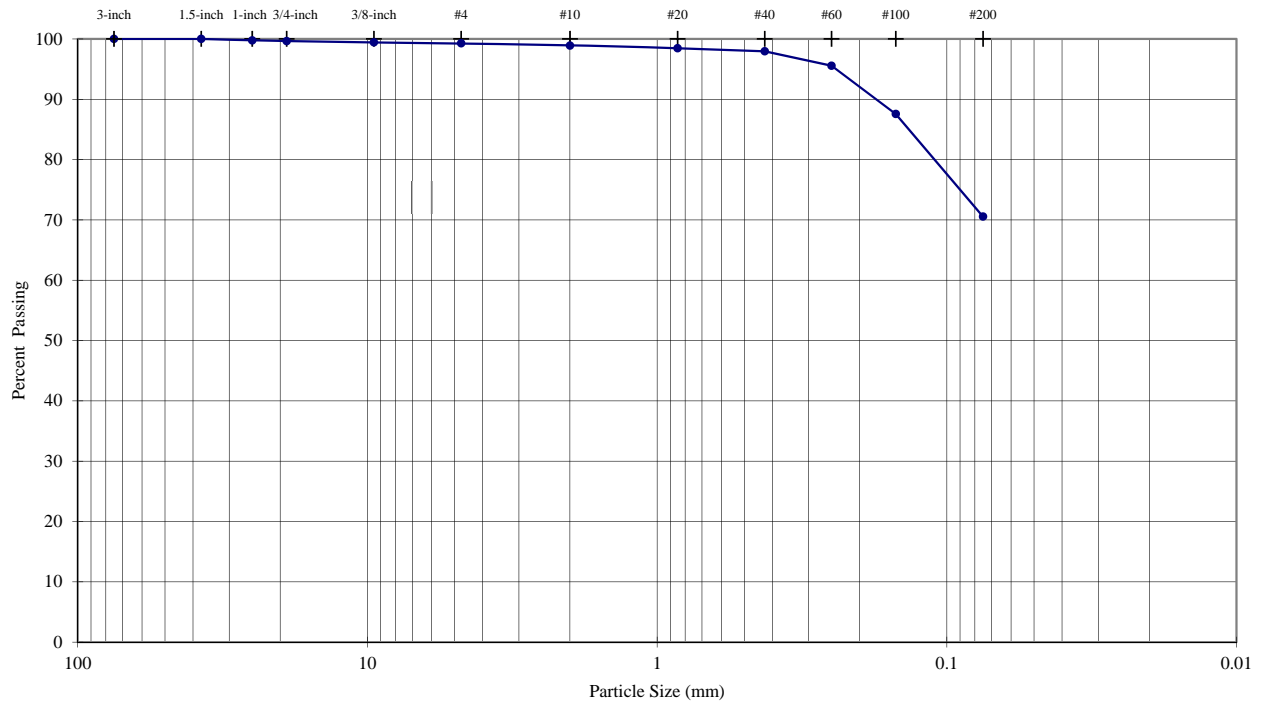
USCS Group Symbol
 --

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

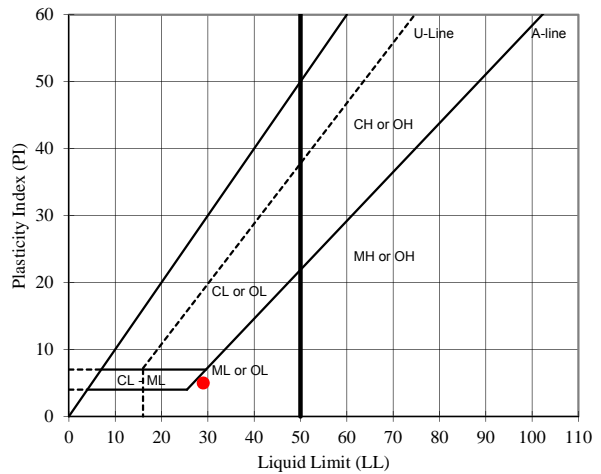
TECH	AMS
DATE	2/27/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-10**
 TYPE: **Pail**
 DEPTH (ft): **3-5(12)**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.37
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	99.6	Fine Gravel	0.36
3/8-inch	9.5	99.4		
#4	4.8	99.3	Coarse Sand	0.38
#10	2.00	98.9		
#20	0.85	98.5	Medium Sand	0.95
#40	0.43	97.9		
#60	0.25	95.6	Fine Sand	27.39
#100	0.15	87.6		
#200	0.075	70.6		
			Silt or Clay Fines	70.55



USCS Description (ASTM D 2487):
Silt with sand, yellowish brown, dry

LL	PL	PI
29	24	5

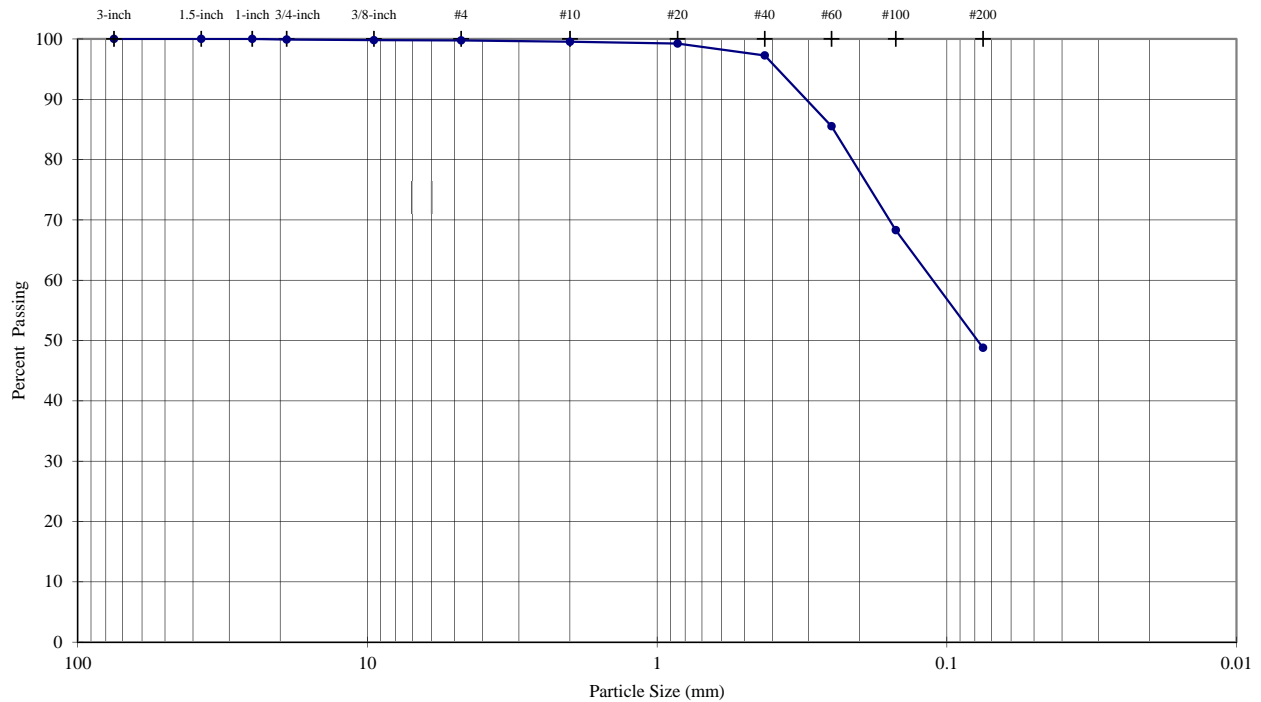
As-Received Moisture Content (%) USCS Group Symbol
-- ML

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

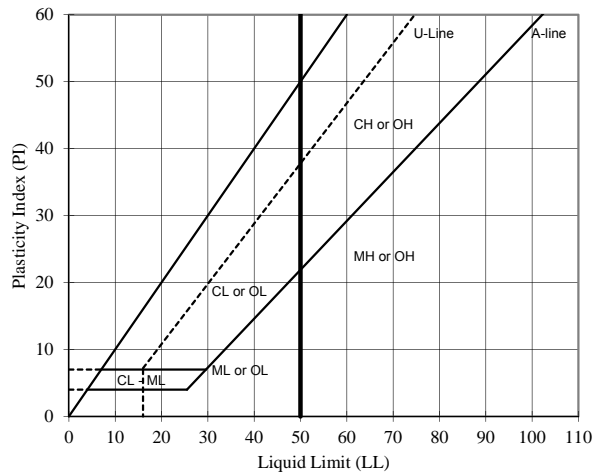
TECH	AMS
DATE	3/1/2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-11** DEPTH (ft): **3-11**
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.11
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	0.14
3/8-inch	9.5	99.8		
#4	4.8	99.8	Coarse Sand	0.22
#10	2.00	99.5		
#20	0.85	99.2		
#40	0.43	97.3	Medium Sand	2.27
#60	0.25	85.5		
#100	0.15	68.3	Fine Sand	48.47
#200	0.075	48.8		
			Silt or Clay Fines	48.80



USCS Description (ASTM D 2487):
Silty sand, brownish yellow, moist

LL	PL	PI	SpG
NP	NP	NP	2.74

As-Received Moisture Content (%) **--** USCS Group Symbol **SM**

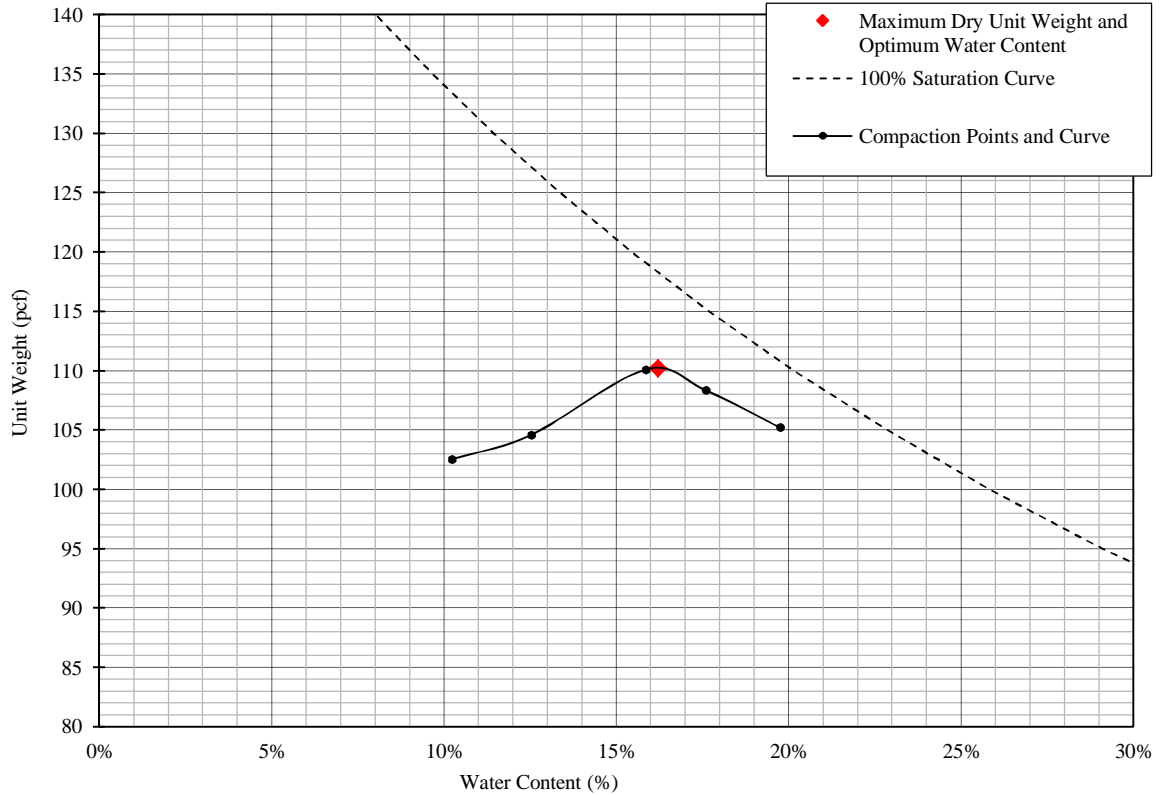
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM
DATE	2/20/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-11** DEPTH (ft): **3-11**
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	100%
As-Received Moisture Content	NA
Specific Gravity (ASTM D854)	2.74

Maximum Dry Unit Weight (pcf)	110.2
Optimum Water Content (%)	16.2

USCS Description (ASTM D 2487): Silty sand, brownish yellow, moist

USCS SM

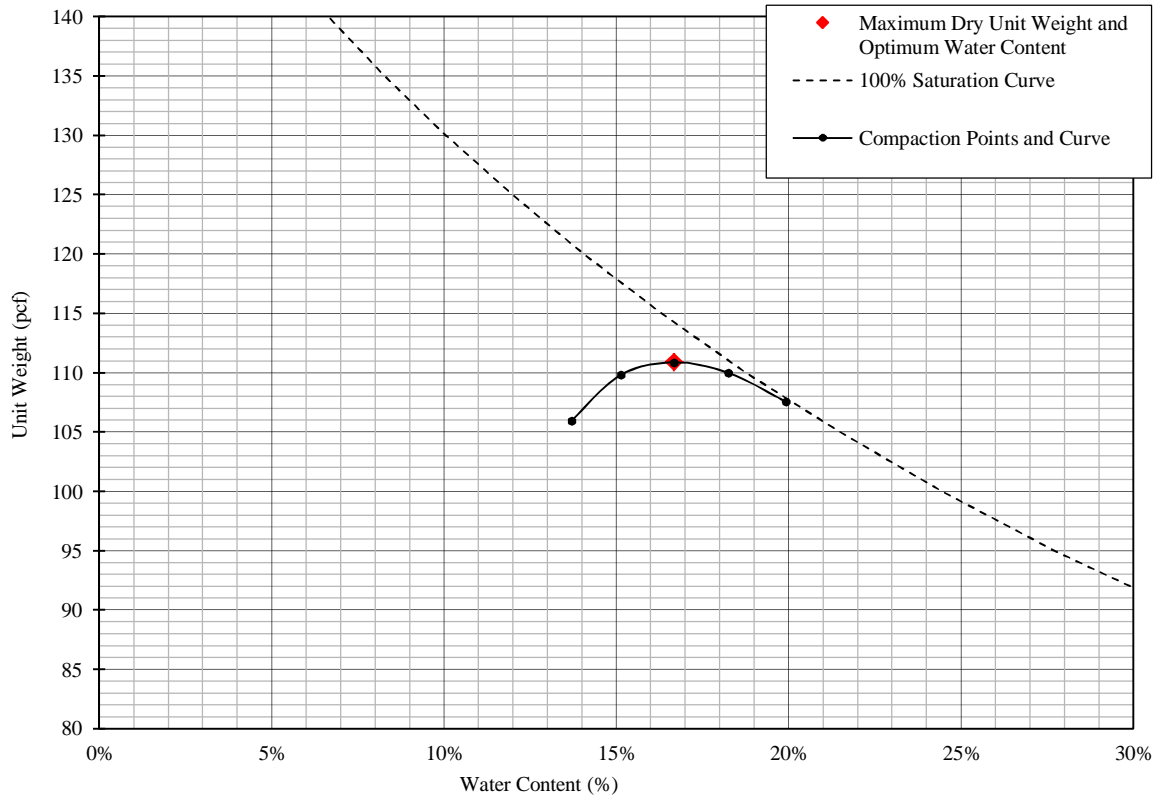
TECH	EH
DATE	2-22-2013
REVIEW	MB



LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-20** DEPTH (ft): **0-4**
 TYPE: **Pail/Bag**



% Test Fraction Passing #4 Sieve	82%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.64

Maximum Dry Unit Weight (pcf)	110.9
Optimum Water Content (%)	16.7

Corrected Maximum Dry Unit Weight (pcf)	116.9
Corrected Optimum Water Content (%)	13.9

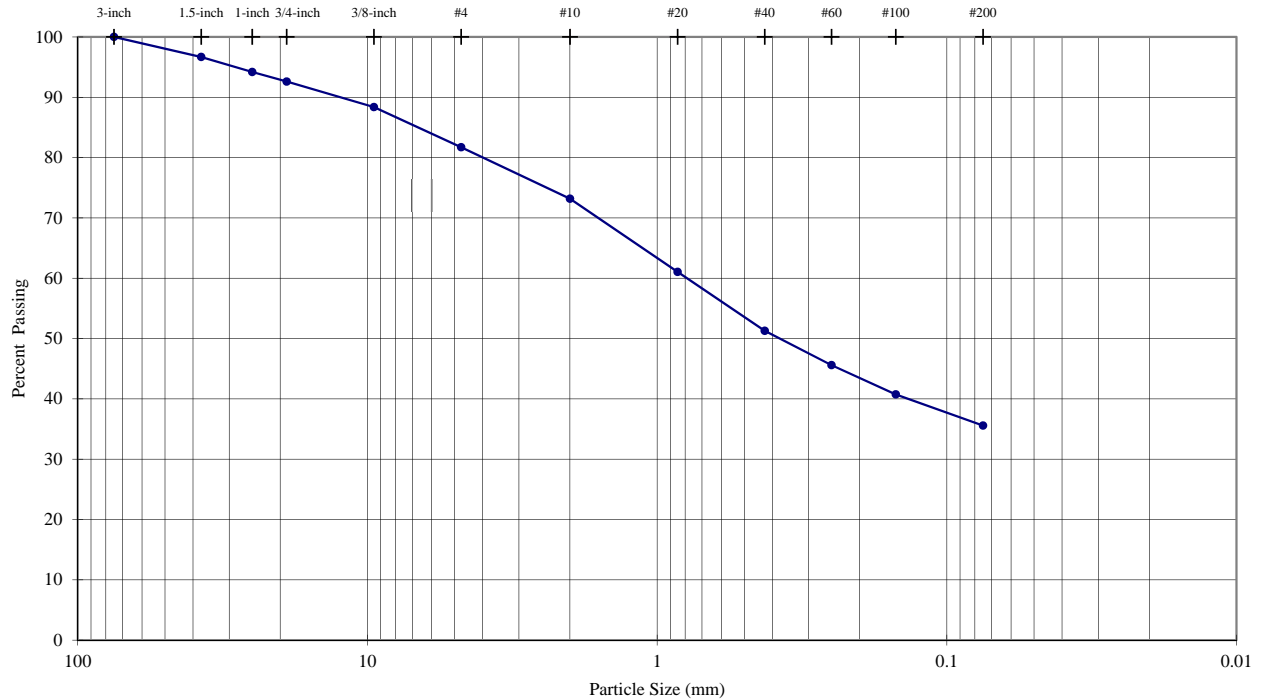
USCS Description (ASTM D 2487): Clayey sand with gravel, strong brown, wet

USCS	SC
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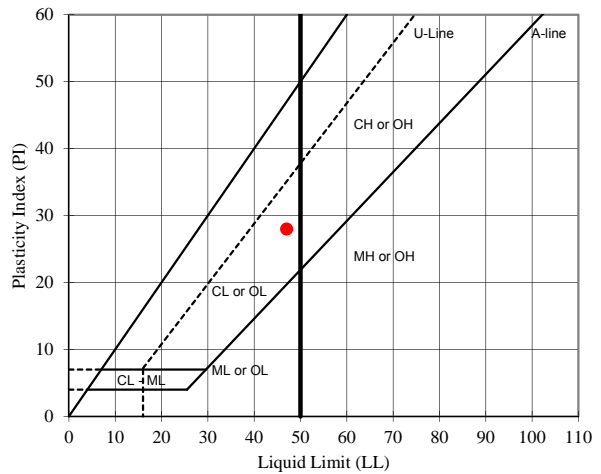
TECH	AM
DATE	2-27-13
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-20** DEPTH (ft): **0-4**
 TYPE: **Pail/Bag**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	7.40
1.5-inch	37.5	96.7		
1-inch	25.0	94.2		
3/4-inch	19.0	92.6	Fine Gravel	10.89
3/8-inch	9.5	88.4		
#4	4.8	81.7	Coarse Sand	8.53
#10	2.00	73.2		
#20	0.85	61.0		
#40	0.43	51.3	Medium Sand	21.90
#60	0.25	45.6		
#100	0.15	40.7	Fine Sand	15.70
#200	0.075	35.6		
			Silt or Clay Fines	35.57



USCS Description (ASTM D 2487):
 Clayey sand with gravel, strong brown, wet

LL	PL	PI
47	19	28

As-Received Moisture Content (%)
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USCS Group Symbol
 SC

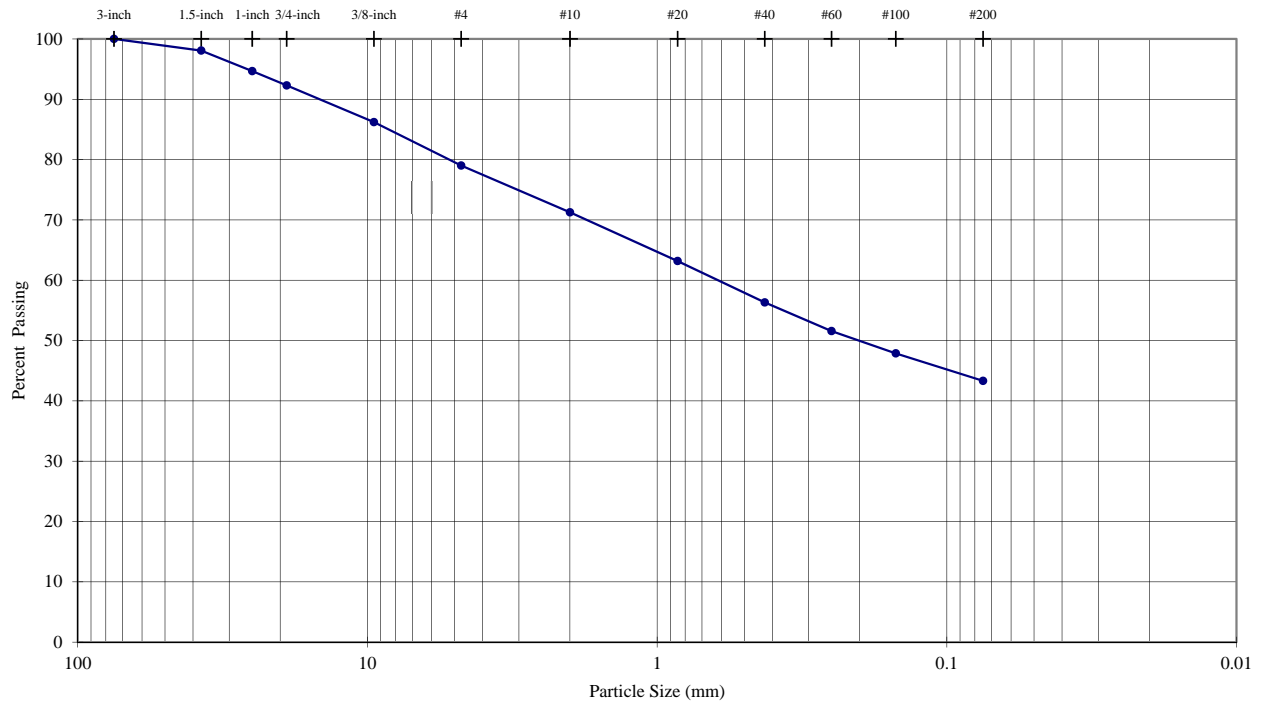
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/25/2013
REVIEW	MB

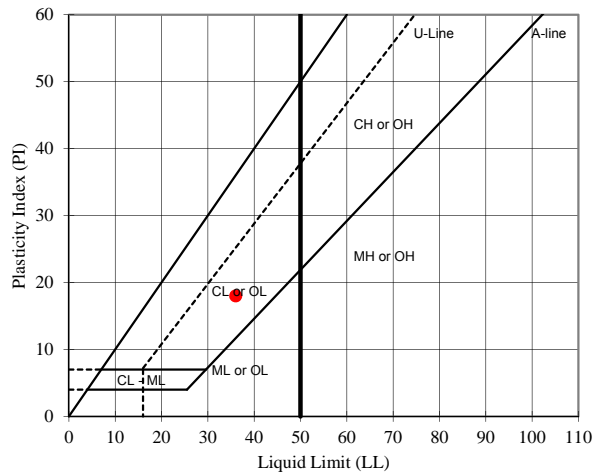
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-20**
 TYPE: **Pail/Bag**

DEPTH (ft): **4-7**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	7.71
1.5-inch	37.5	98.1		
1-inch	25.0	94.7		
3/4-inch	19.0	92.3	Fine Gravel	13.28
3/8-inch	9.5	86.2		
#4	4.8	79.0	Coarse Sand	7.75
#10	2.00	71.3		
#20	0.85	63.2	Medium Sand	14.95
#40	0.43	56.3		
#60	0.25	51.6	Fine Sand	12.99
#100	0.15	47.9		
#200	0.075	43.3	Silt or Clay Fines	43.32



USCS Description (ASTM D 2487):

Clayey sand with gravel, brown, dry

LL	PL	PI
36	18	18

As-Received Moisture Content (%)

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USCS Group Symbol

SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

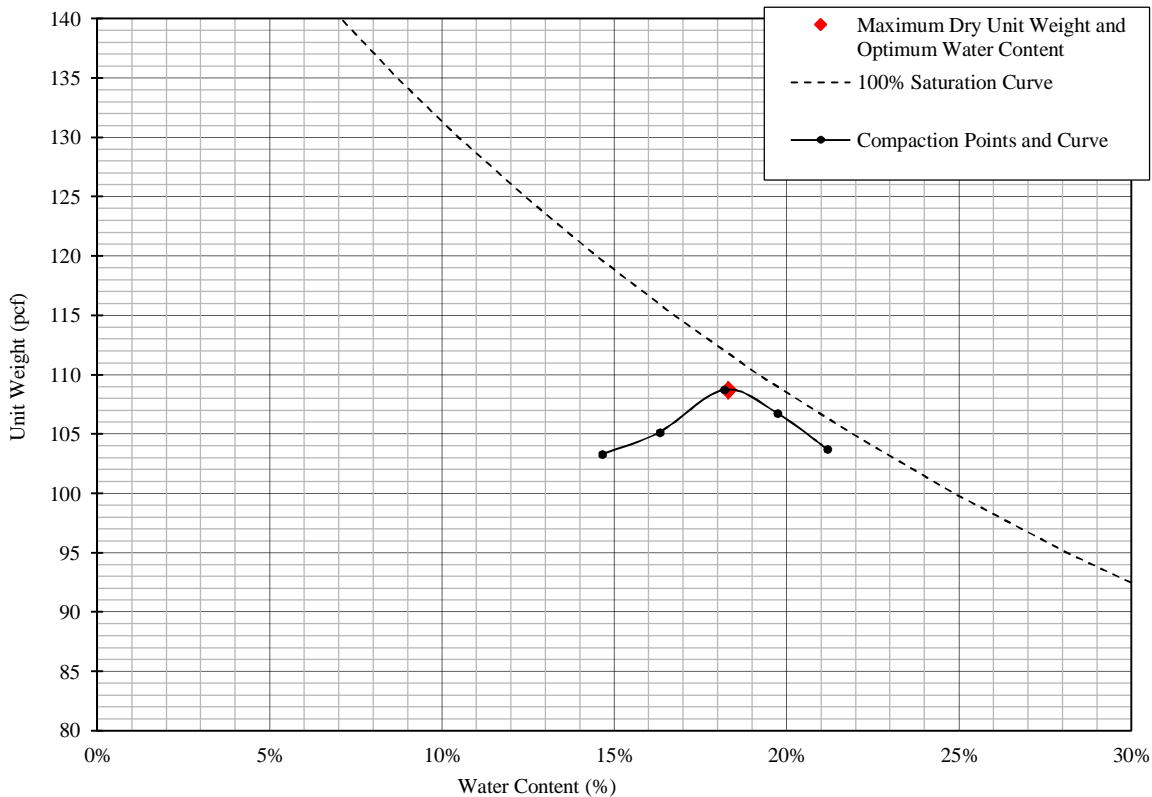
TECH	AMS
DATE	2/25/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-20**
 TYPE: **Pail/Bag**

DEPTH (ft): **4-7**



% Test Fraction Passing #4 Sieve	79%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.67

Maximum Dry Unit Weight (pcf)	108.7
Optimum Water Content (%)	18.3

Corrected Maximum Dry Unit Weight (pcf)	116.1
Corrected Optimum Water Content (%)	14.8

USCS Description (ASTM D 2487): Clayey sand with gravel, brown, dry

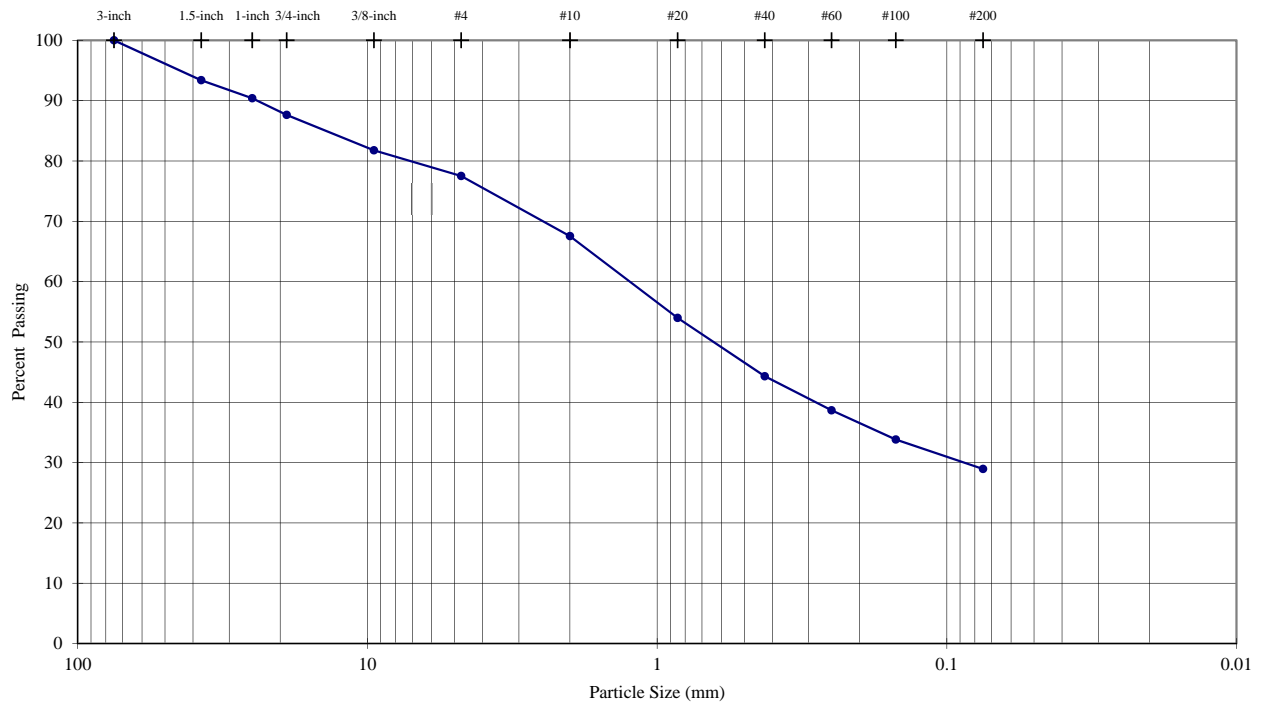
USCS SC

TECH	AMS
DATE	2-28-13
REVIEW	MB

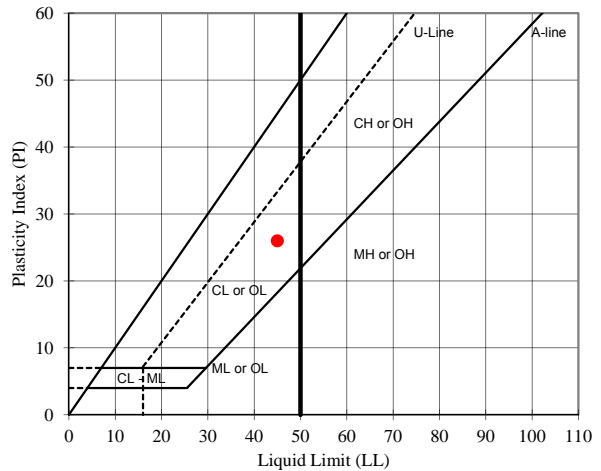
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-24**
 TYPE: **Pail**

DEPTH (ft): **3-5**



Sieve	Particle Size	% Passing	Description	Percentage
	(mm)			
3-inch	75.0	100.0	Coarse Gravel	12.37
1.5-inch	37.5	93.4		
1-inch	25.0	90.4		
3/4-inch	19.0	87.6	Fine Gravel	10.12
3/8-inch	9.5	81.8		
#4	4.8	77.5	Coarse Sand	9.97
#10	2.00	67.5		
#20	0.85	54.0		
#40	0.43	44.3	Medium Sand	23.23
#60	0.25	38.7		
#100	0.15	33.8	Fine Sand	15.37
#200	0.075	28.9		
			Silt or Clay Fines	28.94



USCS Description (ASTM D 2487):

Clayey sand with gravel, yellowish brown, dry

LL	PL	PI
45	19	26

As-Received Moisture Content (%)

--

USCS Group Symbol

SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

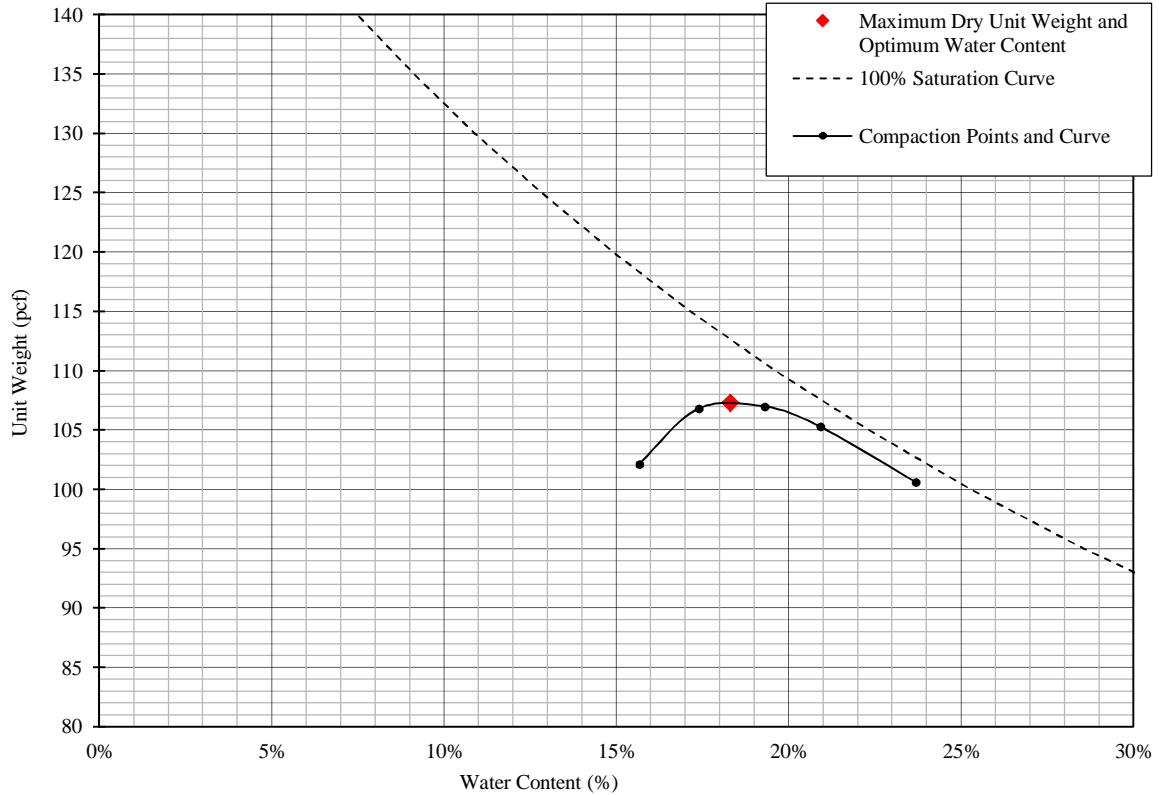
TECH	AM
DATE	2/20/2013
REVIEW	MB



LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-24** DEPTH (ft): **3-5**
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	79%
As-Received Moisture Content	NA
Specific Gravity (estimated)	2.70

Maximum Dry Unit Weight (pcf)	107.3
Optimum Water Content (%)	18.3

Corrected Maximum Dry Unit Weight (pcf)	114.7
Corrected Optimum Water Content (%)	14.4

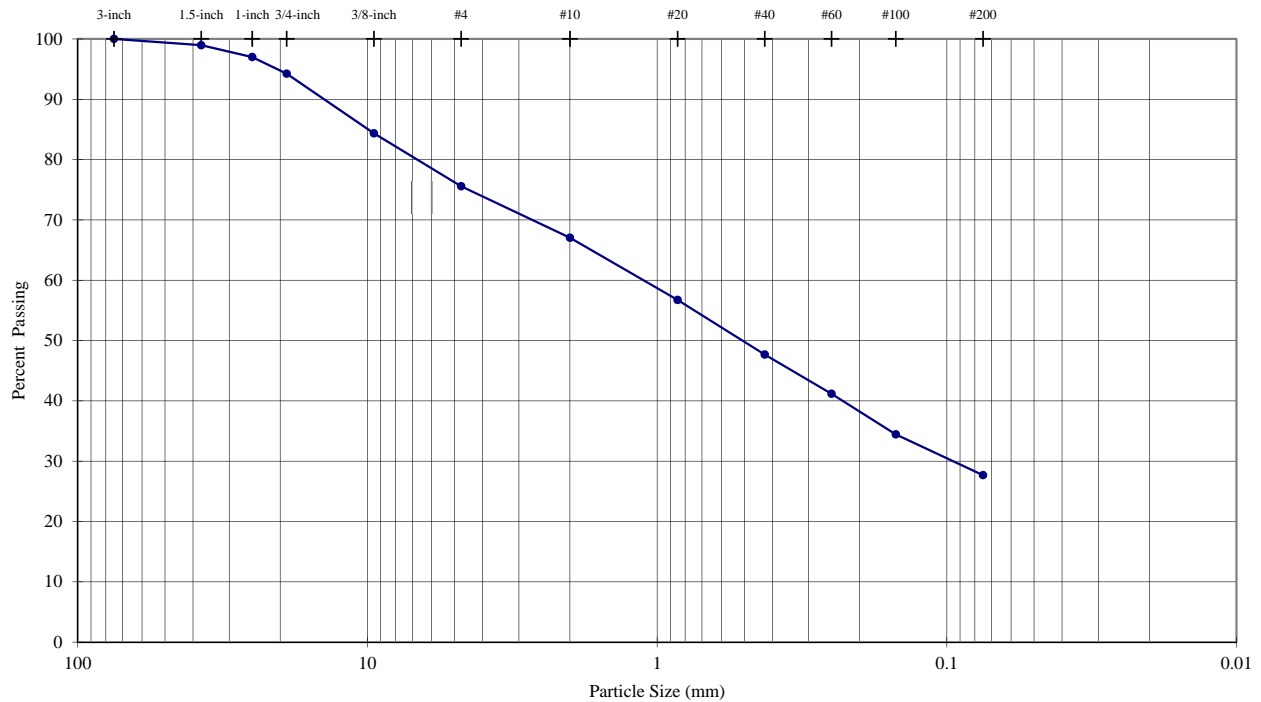
USCS Description (ASTM D 2487): Clayey sand with gravel, yellowish brown, dry

USCS SC

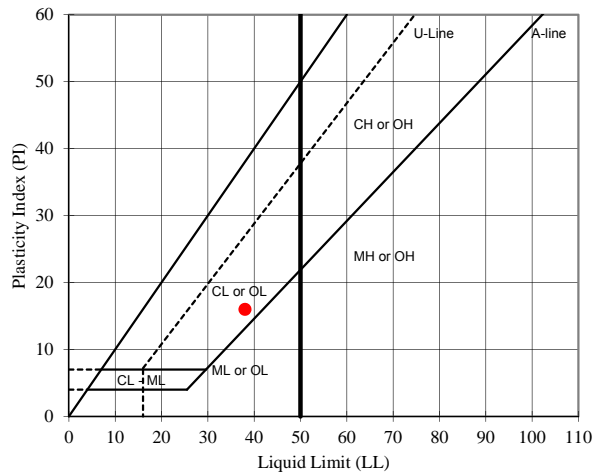
TECH	EH
DATE	2-22-2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-27** DEPTH (ft): **0-2**
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	5.76
1.5-inch	37.5	99.0		
1-inch	25.0	97.0		
3/4-inch	19.0	94.2	Fine Gravel	18.67
3/8-inch	9.5	84.4		
#4	4.8	75.6	Coarse Sand	8.53
#10	2.00	67.0		
#20	0.85	56.7		
#40	0.43	47.7	Medium Sand	19.37
#60	0.25	41.2		
#100	0.15	34.4	Fine Sand	19.99
#200	0.075	27.7		
Silt or Clay Fines				27.69



USCS Description (ASTM D 2487):
 Clayey sand with gravel, dark yellowish brown, dry

LL	PL	PI
38	22	16

As-Received Moisture Content (%)
 --

USCS Group Symbol
 SC

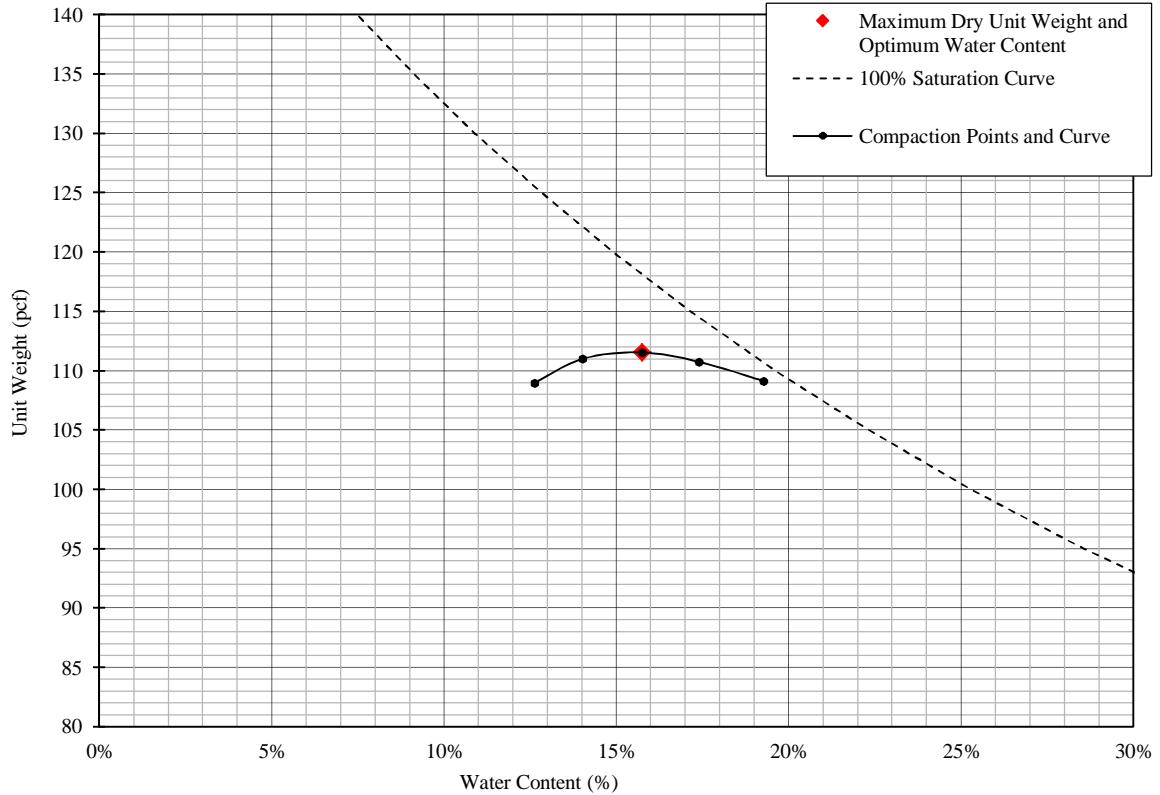
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH MGC
 DATE 2/22/2013
 REVIEW MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-27** DEPTH (ft): **0-2**
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	76%
As-Received Moisture Content	NA
Specific Gravity (estimated)	2.70

Maximum Dry Unit Weight (pcf)	111.5
Optimum Water Content (%)	15.7

Corrected Maximum Dry Unit Weight (pcf)	119.9
Corrected Optimum Water Content (%)	12.2

USCS Description (ASTM D 2487): Clayey sand with gravel, dark yellowish brown, dry

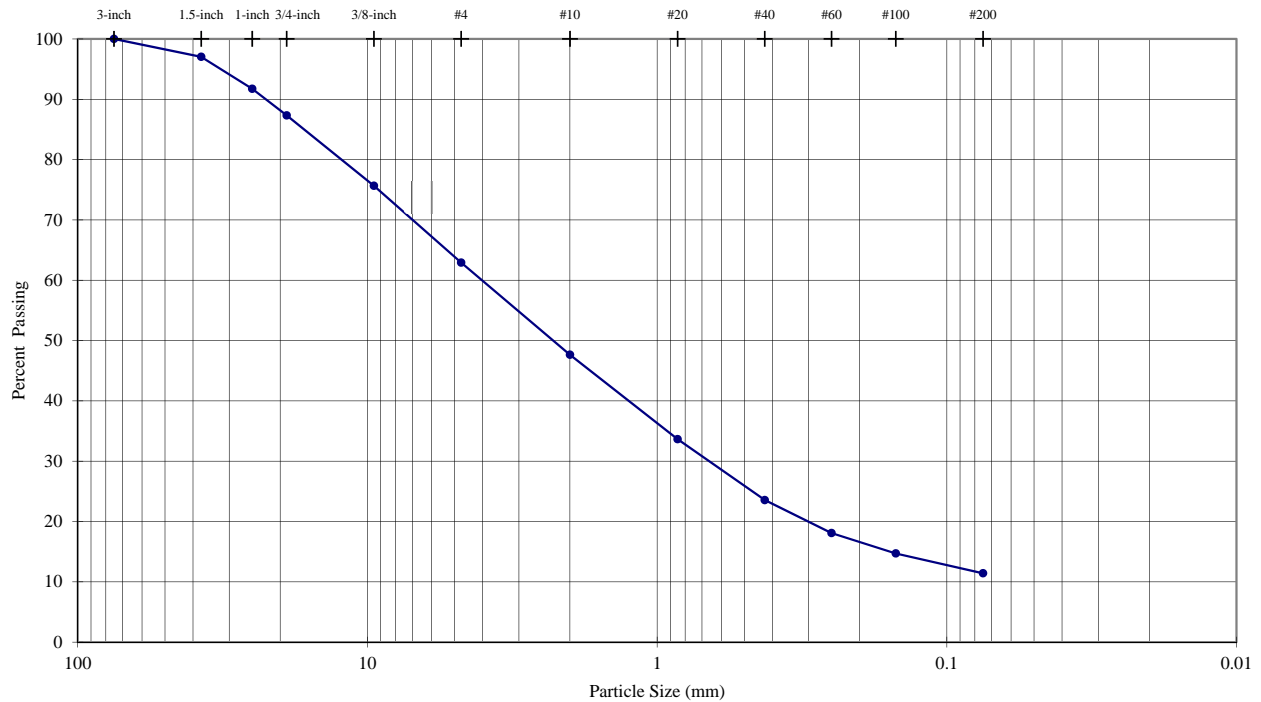
USCS SC

TECH	MGC/AMS
DATE	2-26-2013
REVIEW	MB

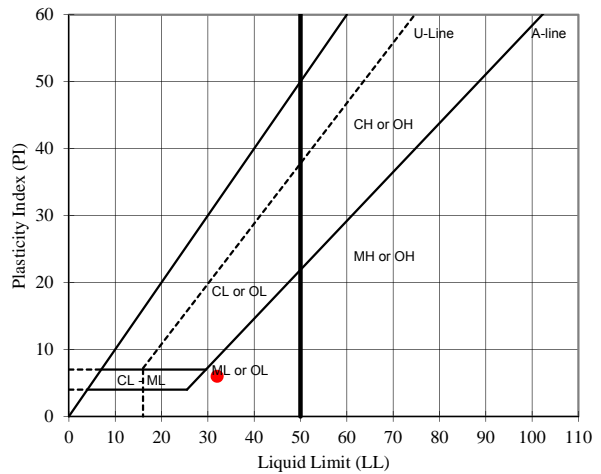
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-27**
 TYPE: **Bag/Pail**

DEPTH (ft): **3-7**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	12.68
1.5-inch	37.5	97.0		
1-inch	25.0	91.7		
3/4-inch	19.0	87.3	Fine Gravel	24.40
3/8-inch	9.5	75.6		
#4	4.8	62.9	Coarse Sand	15.27
#10	2.00	47.6		
#20	0.85	33.7	Medium Sand	24.09
#40	0.43	23.6		
#60	0.25	18.1	Fine Sand	12.14
#100	0.15	14.7		
#200	0.075	11.4	Silt or Clay Fines	11.42



USCS Description (ASTM D 2487):
 Well-graded sand with silt and gravel, dark yellowish brown, dry

LL	PL	PI
32	26	6

As-Received Moisture Content (%)
 --

USCS Group Symbol
 SW-SM

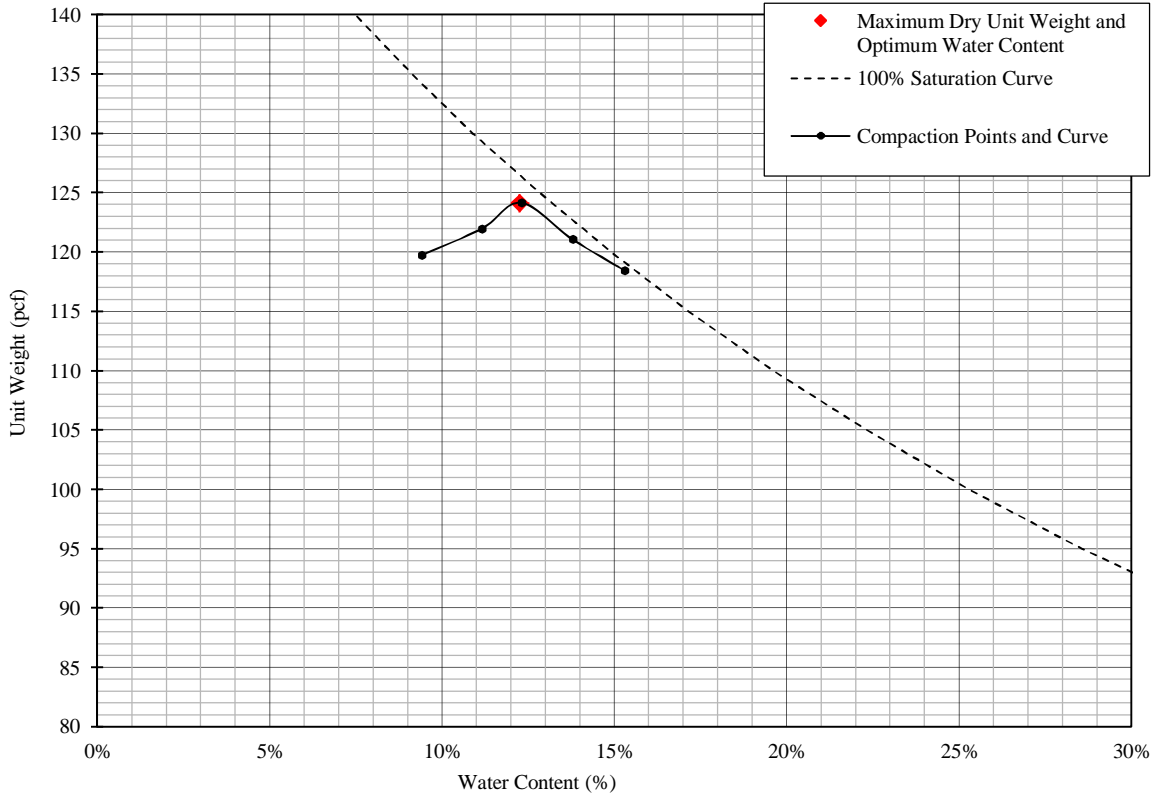
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	EH
DATE	2/20/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-27** DEPTH (ft): **3-7**
 TYPE: **Bag/Pail**



% Test Fraction Passing 3/8-inch Sieve	76%
As-Received Moisture Content	NA
Specific Gravity (estimated)	2.70

Maximum Dry Unit Weight (pcf)	124.1
Optimum Water Content (%)	12.3

Corrected Maximum Dry Unit Weight (pcf)	130.5
Corrected Optimum Water Content (%)	9.6

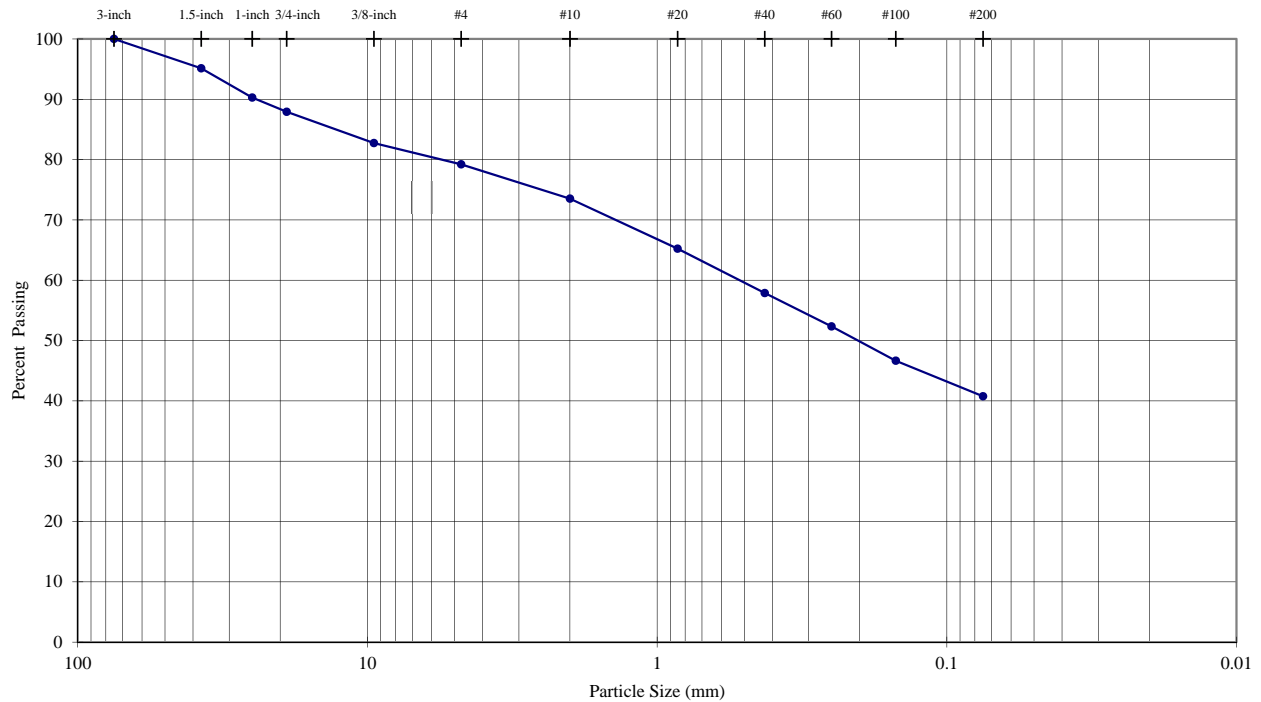
USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, dark yellowish brown, dry

USCS SW-SM

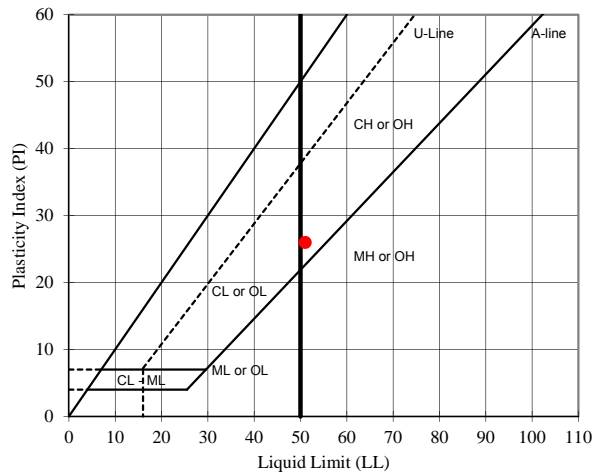
TECH	EH
DATE	3-8-2013
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-29** DEPTH (ft): **0-2**
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	12.08
1.5-inch	37.5	95.1		
1-inch	25.0	90.3		
3/4-inch	19.0	87.9	Fine Gravel	8.71
3/8-inch	9.5	82.7		
#4	4.8	79.2	Coarse Sand	5.68
#10	2.00	73.5		
#20	0.85	65.2		
#40	0.43	57.9	Medium Sand	15.66
#60	0.25	52.3		
#100	0.15	46.7	Fine Sand	17.10
#200	0.075	40.8		
			Silt or Clay Fines	40.76



USCS Description (ASTM D 2487):
 Clayey sand with gravel, strong brown, dry

LL	PL	PI
51	25	26

As-Received Moisture Content (%)
 --

USCS Group Symbol
SC

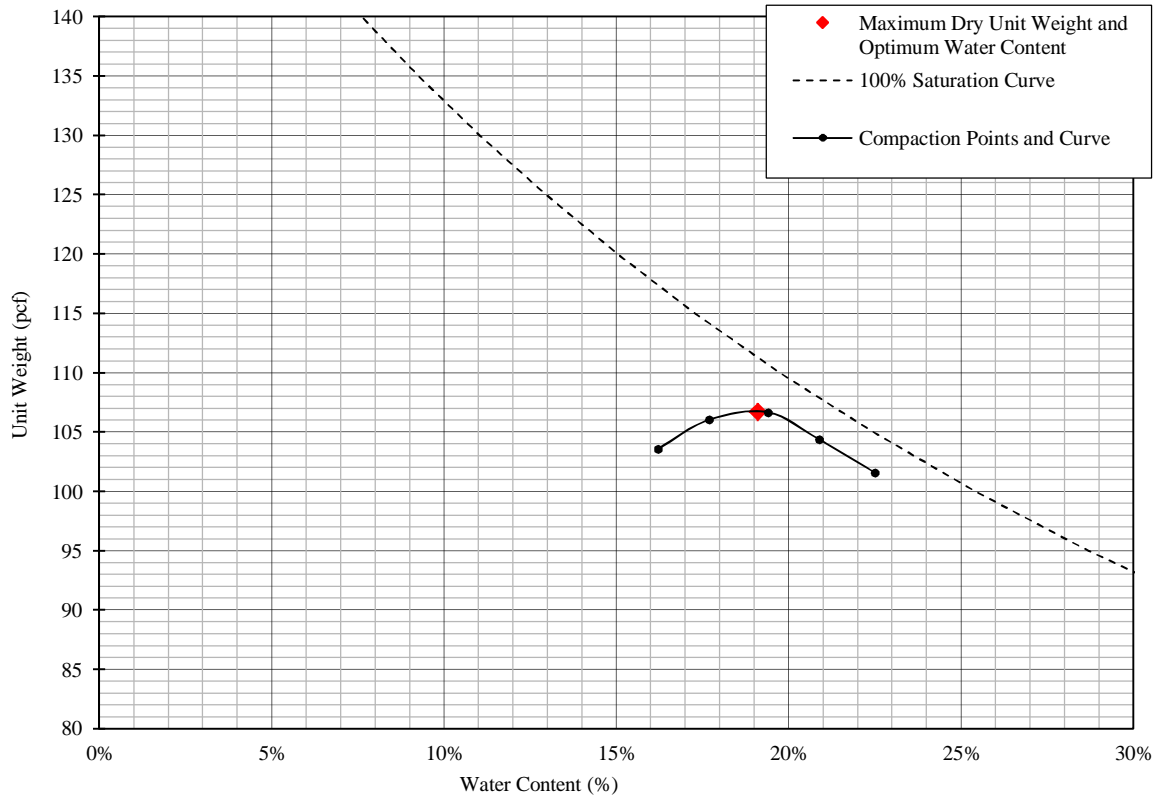
Notes: 0g of particles up to plus 75.0mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM
DATE	2/21/2013
REVIEW	MB

LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **TP-29** DEPTH (ft): **0-2**
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	80%
As-Received Moisture Content	NA
Specific Gravity (ASTM C127)	2.71

Maximum Dry Unit Weight (pcf)	106.7
Optimum Water Content (%)	19.1

Corrected Maximum Dry Unit Weight (pcf)	114.4
Corrected Optimum Water Content (%)	15.5

USCS Description (ASTM D 2487): Clayey sand with gravel, strong brown, dry

USCS SC

TECH	AM
DATE	2-25-2013
REVIEW	MB

**APPENDIX A.3.2
TRIAxIAL TEST REPORTS**

Boring or Test Pit: --
 Sample: Comp 1-4
 Depth: -- ft
 Point No.: 1

Boring or Test Pit: --
 Sample: Comp 1-4
 Depth: -- ft
 Point No.: 2

Boring or Test Pit: --
 Sample: Comp 1-4
 Depth: -- ft
 Point No.: 3

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.517 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 7.355 lb
 Moisture Content = 15.8%
 Wet Unit Weight = 126.6 pcf
 Dry Unit Weight = 109.3 pcf
 Void Ratio = 0.51
 Percent Saturation = 83%

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.506 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 7.332 lb
 Moisture Content = 16.0%
 Wet Unit Weight = 126.4 pcf
 Dry Unit Weight = 108.9 pcf
 Void Ratio = 0.51
 Percent Saturation = 83%

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.506 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 7.377 lb
 Moisture Content = 15.3%
 Wet Unit Weight = 126.4 pcf
 Dry Unit Weight = 109.6 pcf
 Void Ratio = 0.50
 Percent Saturation = 81%

After Consolidation
 Length = 9.173 in
 Diameter = 3.814 in
 Area = 11.424 in² (Method B)
 Volume = 104.789 in³
 Moisture Content = 13.5%
 Wet Unit Weight = 137.7 pcf
 Dry Unit Weight = 121.3 pcf
 Void Ratio = 0.36
 Percent Saturation = 100%

After Consolidation
 Length = 9.094 in
 Diameter = 3.821 in
 Area = 11.465 in² (Method B)
 Volume = 104.264 in³
 Moisture Content = 13.4%
 Wet Unit Weight = 137.8 pcf
 Dry Unit Weight = 121.5 pcf
 Void Ratio = 0.35
 Percent Saturation = 100%

After Consolidation
 Length = 9.057 in
 Diameter = 3.799 in
 Area = 11.333 in² (Method B)
 Volume = 102.647 in³
 Moisture Content = 12.3%
 Wet Unit Weight = 139.5 pcf
 Dry Unit Weight = 124.2 pcf
 Void Ratio = 0.32
 Percent Saturation = 100%

B Parameter = 0.97
 Shear Rate = 0.071% /min.
 t₅₀ = 5.6 min.
 Strain at Failure = 5.0%

B Parameter = 0.98
 Shear Rate = 0.083% /min.
 t₅₀ = 1.8 min.
 Strain at Failure = 5.0%

B Parameter = 0.96
 Shear Rate = 0.027% /min.
 t₅₀ = 14.5 min.
 Strain at Failure = 5.0%

Cell Pressure = 75 psi
 Back Pressure = 50 psi
 Confining Pressure = 25 psi

Cell Pressure = 100 psi
 Back Pressure = 50 psi
 Confining Pressure = 50 psi

Cell Pressure = 150 psi
 Back Pressure = 50 psi
 Confining Pressure = 100 psi

Notes: Sample description: Clayey sand with gravel, yellowish brown, moist
 Atterberg limits: LL = -- PL = -- PI = -- (-- indicates test was not performed)
 Percent finer: 3/4 in. = -- No. 4 = -- No. 200 = -- (-- indicates test was not performed)
 Specimen type:

	Intact	X
	Cuttings	
	Wet	
	(σ ₁ /σ ₃) _{max}	
	Corrected	

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture
 Moisture from:

X

 Entire specimen
 Saturation method:

X

 Dry
 Failure criterion:

--

 (σ₁-σ₃)_{max} 5 % strain
 Membrane effect:

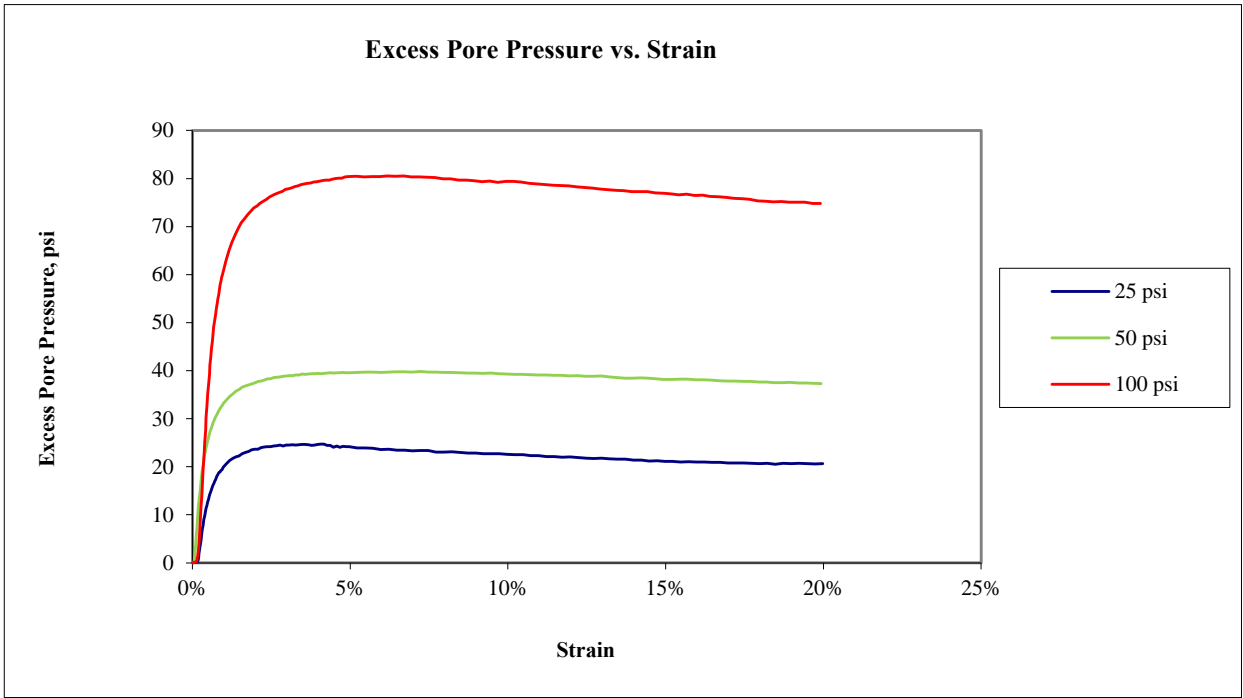
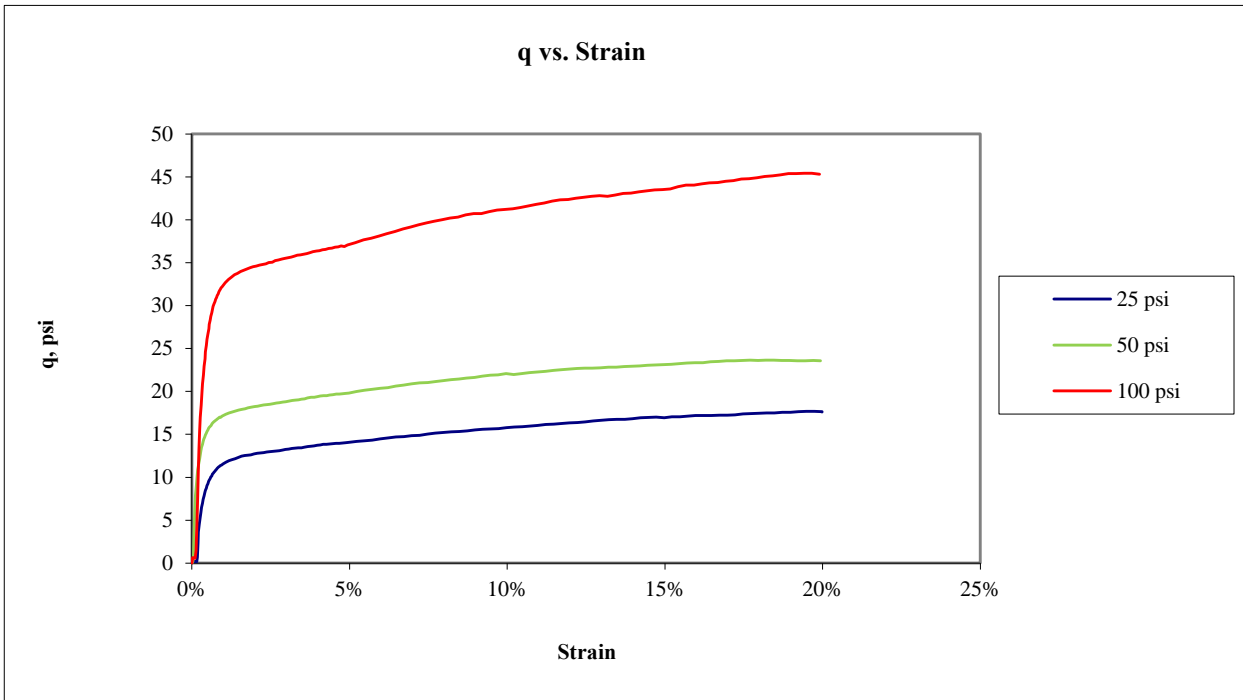
X

 Corrected

--

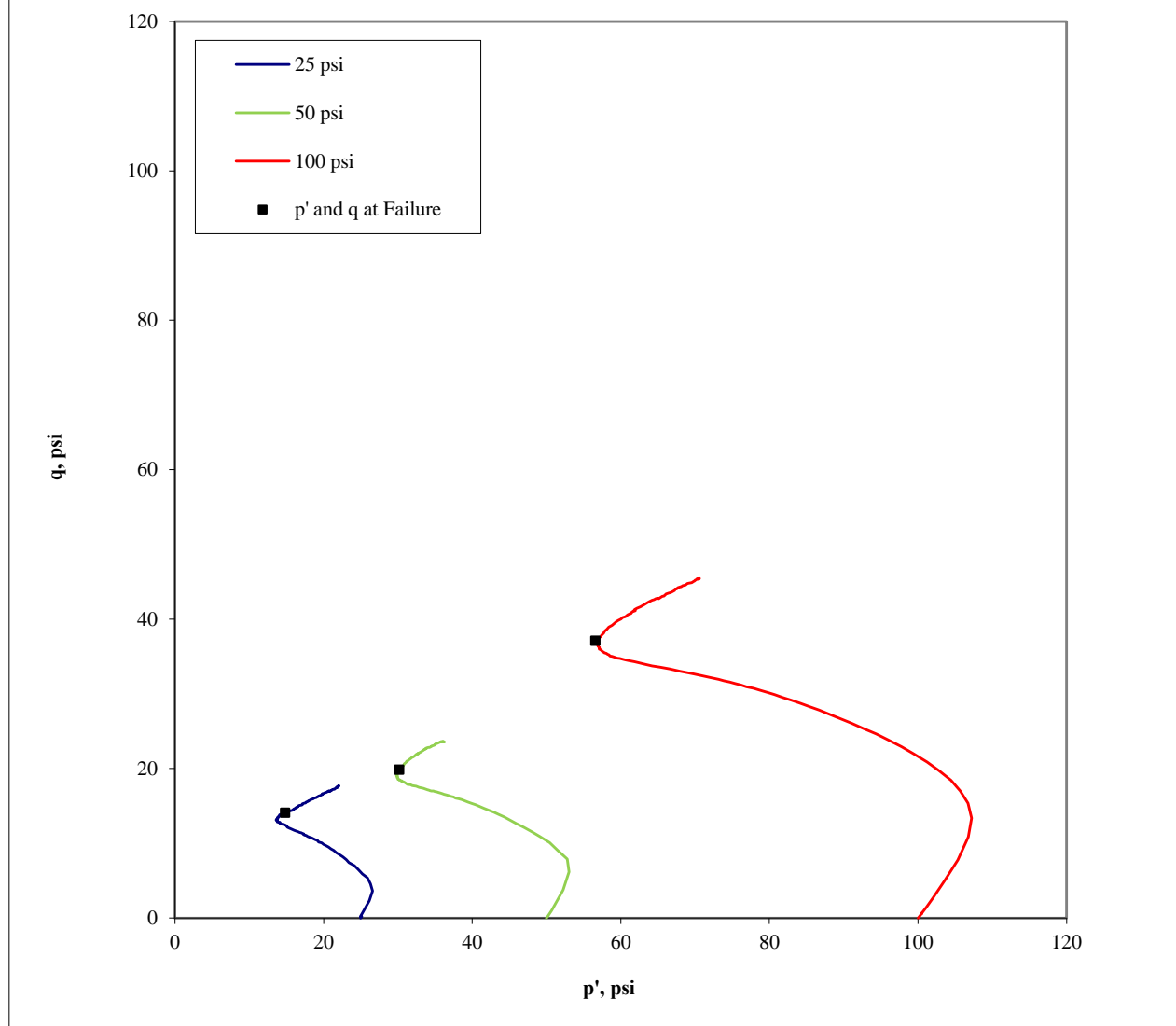
 Not Corrected

Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 2

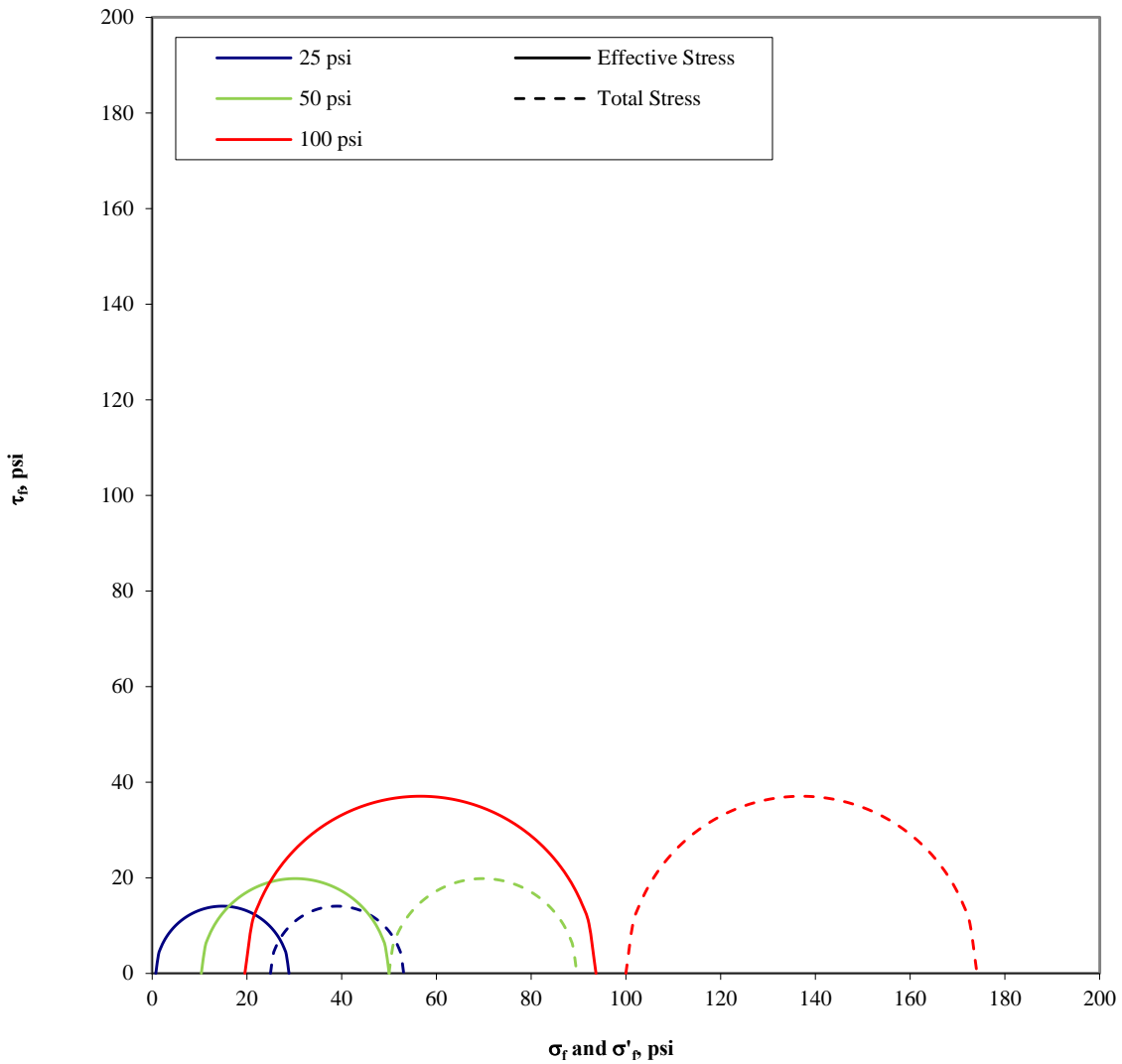
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	39.1	14.8	14.1
50	69.8	30.2	19.8
100	137.0	56.6	37.0

<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: Composite 1-4</p>	<p>Technician: RJM</p>	<p>Reviewed: CCS</p>	<p>Date: 5/3/2013</p>	<p>Job Number: 103-92557.006</p>	<p>Figure: 3</p>

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
25	28.9	0.8	53.1	25.0
50	50.0	10.4	89.6	50.0
100	93.7	19.6	174.1	100.0

Golder Associates Inc.
Denver, Colorado

Title:

ASTM D4767
 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT
 MOHR'S CIRCLE DIAGRAM

Job Short Title:
 Copper Flat Tailings Design Study

Sample: Composite 1-4

Technician:
 RJM

Reviewed:
 CCS

Date:
 5/3/2013

Job Number:
 103-92557.006

Figure:
 4



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 5



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 6



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 7

Boring or Test Pit: --
 Sample: BH-16
 Depth: 0.0 - 8.5 ft
 Point No.: 1

Boring or Test Pit: --
 Sample: BH-16
 Depth: 0.0 - 8.5 ft
 Point No.: 2

Boring or Test Pit: --
 Sample: BH-16
 Depth: 0.0 - 8.5 ft
 Point No.: 3

Initial
 Length = 5.751 in
 Diameter = 2.882 in
 Wet Mass = 2.743 lb
 Area = 6.523 in²
 Volume = 37.516 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.371 lb
 Moisture Content = 15.7%
 Wet Unit Weight = 126.3 pcf
 Dry Unit Weight = 109.2 pcf
 Void Ratio = 0.56
 Percent Saturation = 76%

Initial
 Length = 5.785 in
 Diameter = 2.812 in
 Wet Mass = 2.734 lb
 Area = 6.210 in²
 Volume = 35.927 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.367 lb
 Moisture Content = 15.5%
 Wet Unit Weight = 131.5 pcf
 Dry Unit Weight = 113.8 pcf
 Void Ratio = 0.50
 Percent Saturation = 85%

Initial
 Length = 5.764 in
 Diameter = 2.877 in
 Wet Mass = 2.724 lb
 Area = 6.501 in²
 Volume = 37.471 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.348 lb
 Moisture Content = 16.0%
 Wet Unit Weight = 125.6 pcf
 Dry Unit Weight = 108.3 pcf
 Void Ratio = 0.58
 Percent Saturation = 76%

After Consolidation
 Length = 5.697 in
 Diameter = 2.772 in
 Area = 6.035 in² (Method B)
 Volume = 34.380 in³
 Moisture Content = 15.8%
 Wet Unit Weight = 138.0 pcf
 Dry Unit Weight = 119.2 pcf
 Void Ratio = 0.43
 Percent Saturation = 100%

After Consolidation
 Length = 5.692 in
 Diameter = 2.760 in
 Area = 5.984 in² (Method B)
 Volume = 34.058 in³
 Moisture Content = 15.4%
 Wet Unit Weight = 138.6 pcf
 Dry Unit Weight = 120.1 pcf
 Void Ratio = 0.42
 Percent Saturation = 100%

After Consolidation
 Length = 5.579 in
 Diameter = 2.731 in
 Area = 5.859 in² (Method B)
 Volume = 32.690 in³
 Moisture Content = 13.7%
 Wet Unit Weight = 141.2 pcf
 Dry Unit Weight = 124.1 pcf
 Void Ratio = 0.38
 Percent Saturation = 100%

B Parameter = 0.95
 Shear Rate = 0.083% /min.
 t₅₀ = 2.8 min.
 Strain at Failure = 5.0%

B Parameter = 0.98
 Shear Rate = 0.083% /min.
 t₅₀ = -- (not computed)
 Strain at Failure = 5.0%

B Parameter = 0.95
 Shear Rate = 0.083% /min.
 t₅₀ = 0.8 min.
 Strain at Failure = 5.0%

Cell Pressure = 75 psi
 Back Pressure = 50 psi
 Confining Pressure = 25 psi

Cell Pressure = 110 psi
 Back Pressure = 60 psi
 Confining Pressure = 50 psi

Cell Pressure = 150 psi
 Back Pressure = 50 psi
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, reddish brown, moist
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)
 Percent finer: 3/4 in. = 91% No. 4 = 57% No. 200 = 31% (ASTM D422, refer to separate report for gradation curve)
 Specimen type:

	Intact	X
	Cuttings	
	Wet	
	(σ ₁ /σ ₃) _{max}	
	Corrected	

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture
 Moisture from:

X

 Entire specimen
 Saturation method:

X

 Dry
 Failure criterion:

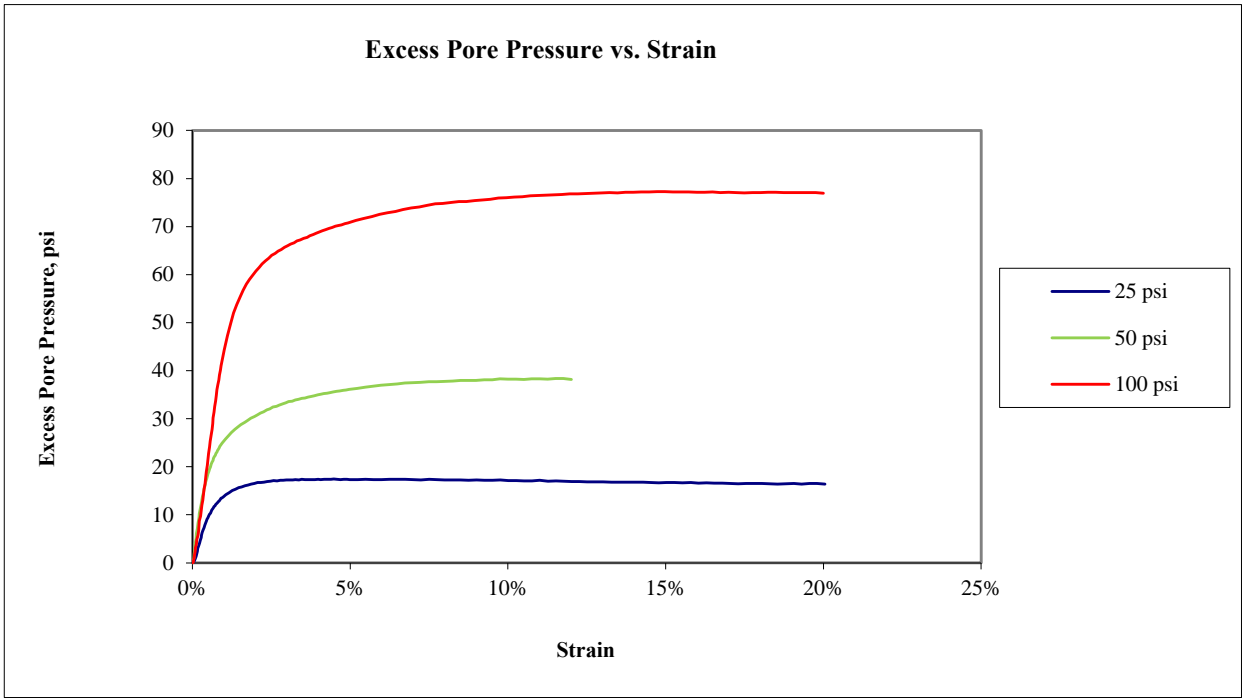
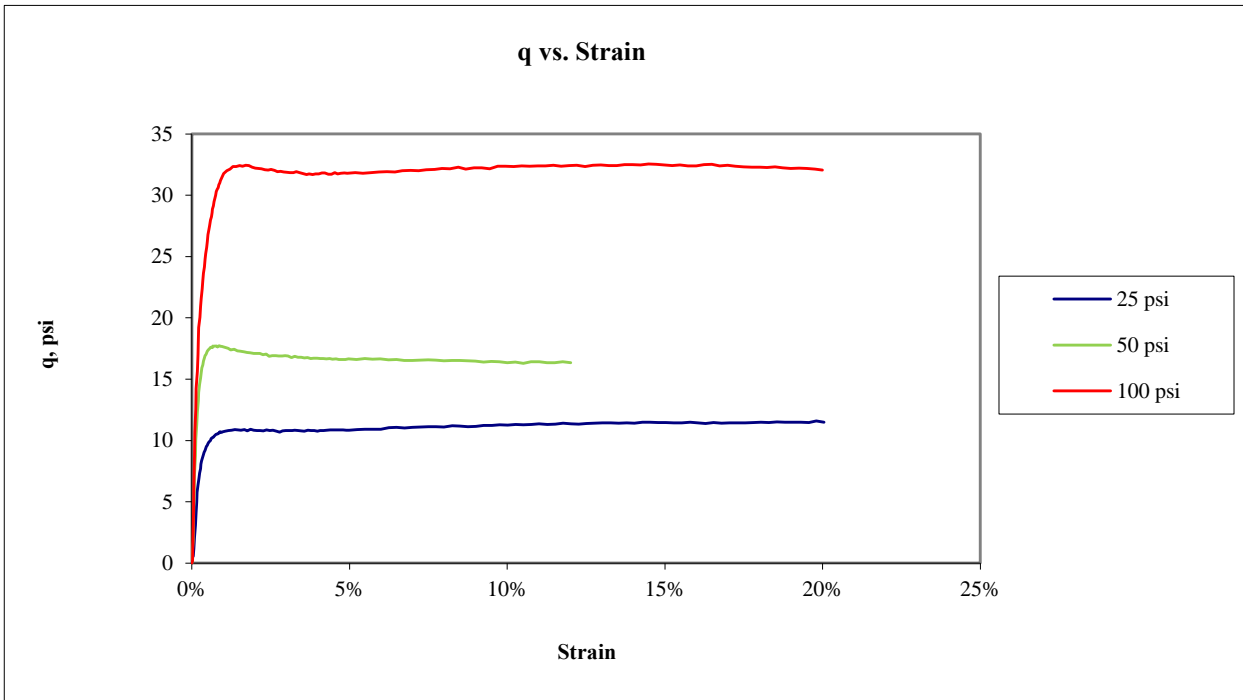
 (σ₁-σ₃)_{max} 5 % strain
 Membrane effect:

X

 Corrected

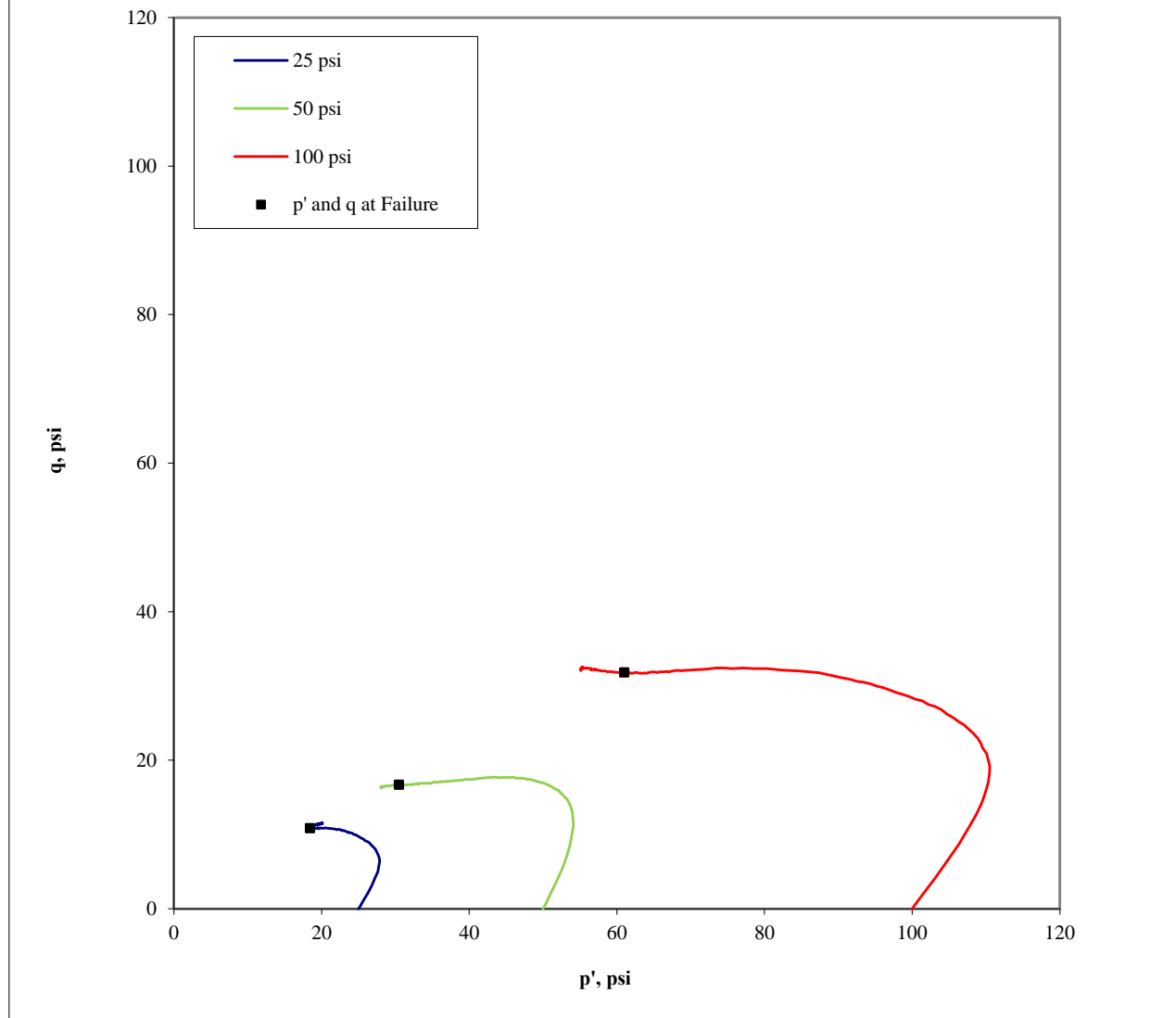
 Not Corrected

Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 2

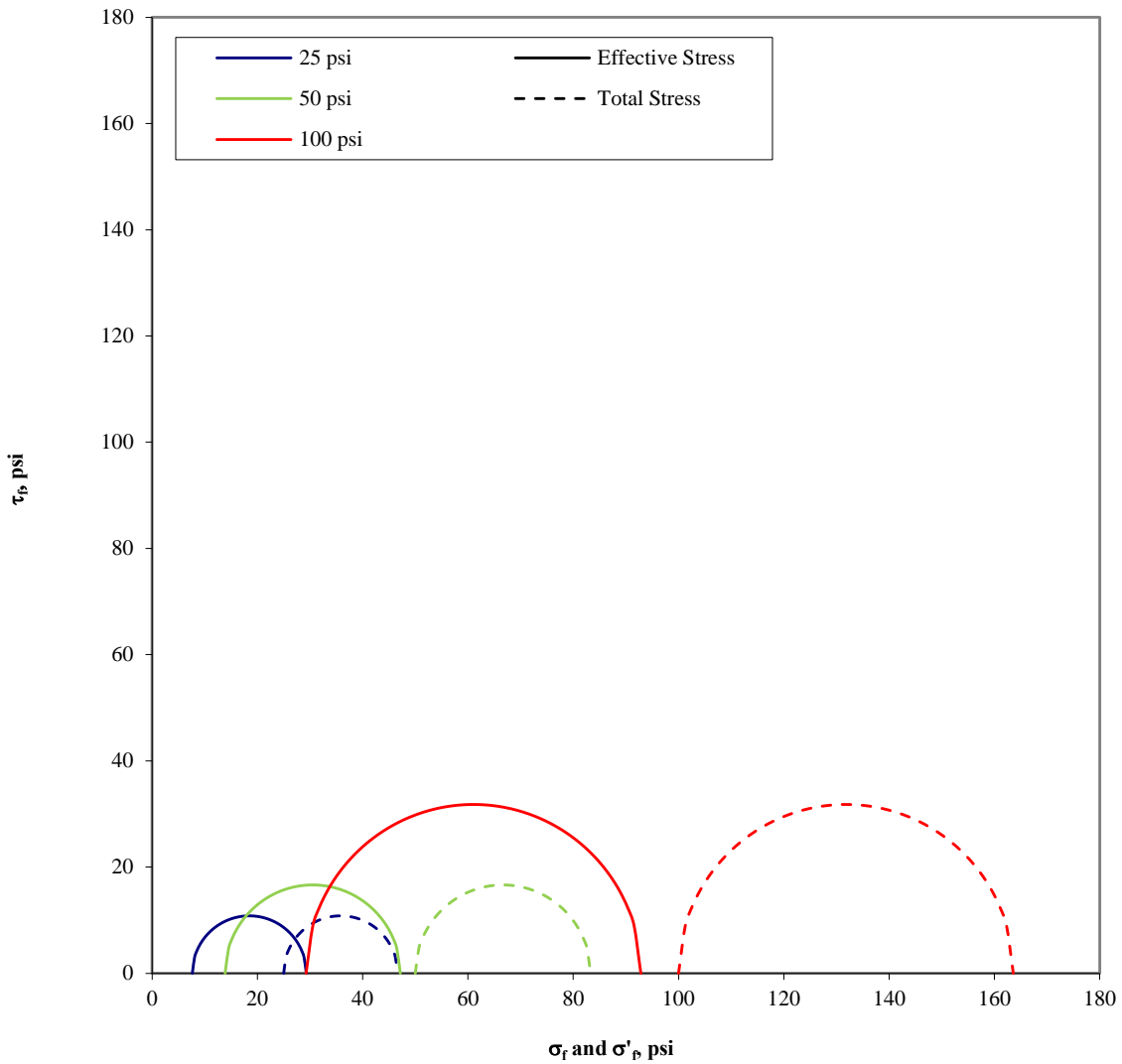
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	35.8	18.5	10.8
50	66.6	30.5	16.6
100	131.8	61.1	31.8

<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: BH-16 @ 0.0 - 8.5 ft</p>	<p>Technician: RJM</p>	<p>Reviewed: CCS</p>	<p>Date: 3/26/2013</p>	<p>Job Number: 103-92557.006</p>	<p>Figure: 3</p>

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
25	29.3	7.6	46.7	25.0
50	47.2	13.9	83.3	50.0
100	92.8	29.3	163.6	100.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 4



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 5



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 6



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
Job Short Title: Copper Flat Tailings Design Study		Technician: RJM		Reviewed: CCS	Date: 3/26/2013
Sample: BH-16 @ 0.0 - 8.5 ft		Job Number: 103-92557.006	Figure: 7		

Boring or Test Pit: --
 Sample: BH-25
 Depth: 0.0 - 12.5 ft
 Point No.: 1

Boring or Test Pit: --
 Sample: BH-25
 Depth: 0.0 - 12.5 ft
 Point No.: 2

Boring or Test Pit: --
 Sample: BH-25
 Depth: 0.0 - 12.5 ft
 Point No.: 3

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.589 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 7.443 lb
 Moisture Content = 15.4%
 Wet Unit Weight = 127.6 pcf
 Dry Unit Weight = 110.6 pcf
 Void Ratio = 0.54
 Percent Saturation = 78%

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.601 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 7.421 lb
 Moisture Content = 15.9%
 Wet Unit Weight = 127.8 pcf
 Dry Unit Weight = 110.3 pcf
 Void Ratio = 0.55
 Percent Saturation = 79%

Initial
 Length = 9.250 in
 Diameter = 4.001 in
 Wet Mass = 8.587 lb
 Area = 12.573 in²
 Volume = 116.297 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 7.416 lb
 Moisture Content = 15.8%
 Wet Unit Weight = 127.6 pcf
 Dry Unit Weight = 110.2 pcf
 Void Ratio = 0.55
 Percent Saturation = 79%

After Consolidation
 Length = 9.195 in
 Diameter = 3.788 in
 Area = 11.267 in² (Method B)
 Volume = 103.604 in³
 Moisture Content = 13.7%
 Wet Unit Weight = 141.2 pcf
 Dry Unit Weight = 124.1 pcf
 Void Ratio = 0.38
 Percent Saturation = 100%

After Consolidation
 Length = 9.131 in
 Diameter = 3.765 in
 Area = 11.133 in² (Method B)
 Volume = 101.651 in³
 Moisture Content = 12.9%
 Wet Unit Weight = 142.4 pcf
 Dry Unit Weight = 126.2 pcf
 Void Ratio = 0.35
 Percent Saturation = 100%

After Consolidation
 Length = 9.054 in
 Diameter = 3.749 in
 Area = 11.037 in² (Method B)
 Volume = 99.932 in³
 Moisture Content = 12.1%
 Wet Unit Weight = 143.7 pcf
 Dry Unit Weight = 128.2 pcf
 Void Ratio = 0.33
 Percent Saturation = 100%

B Parameter = 0.95
 Shear Rate = 0.010% /min.
 t₅₀ = 38.0 min.
 Strain at Failure = 5.0%

B Parameter = 0.98
 Shear Rate = 0.033% /min.
 t₅₀ = 12.0 min.
 Strain at Failure = 5.0%

B Parameter = 0.96
 Shear Rate = 0.047% /min.
 t₅₀ = 8.4 min.
 Strain at Failure = 5.0%

Cell Pressure = 65 psi
 Back Pressure = 40 psi
 Confining Pressure = 25 psi

Cell Pressure = 90 psi
 Back Pressure = 40 psi
 Confining Pressure = 50 psi

Cell Pressure = 145 psi
 Back Pressure = 45 psi
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, yellowish brown, moist
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)
 Percent finer: 3/4 in. = 88% No. 4 = 51% No. 200 = 17% (ASTM D422, refer to separate report for gradation curve)
 Specimen type:

Intact	<input checked="" type="checkbox"/>
Cuttings	<input type="checkbox"/>
Wet	<input type="checkbox"/>
(σ' ₁ /σ' ₃) _{max}	<input type="checkbox"/>
Corrected	<input type="checkbox"/>

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture
 Moisture from:

Entire specimen	<input checked="" type="checkbox"/>
Dry	<input type="checkbox"/>

 Saturation method:

Wet	<input checked="" type="checkbox"/>
Dry	<input type="checkbox"/>

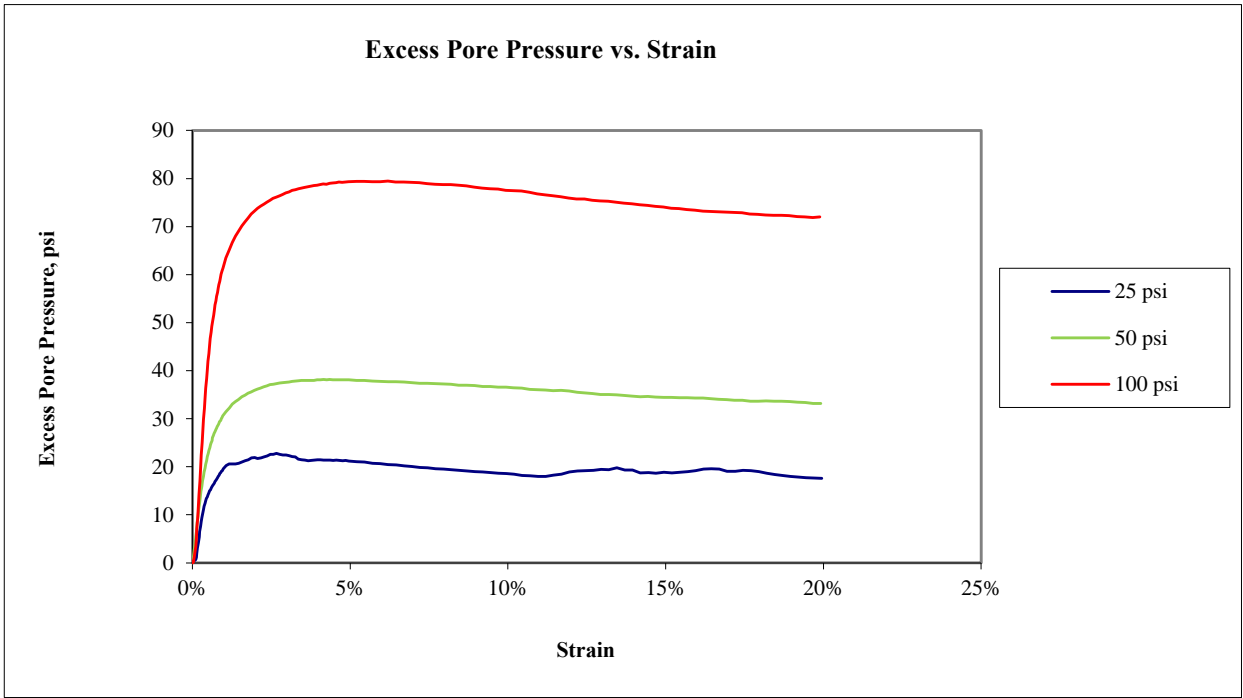
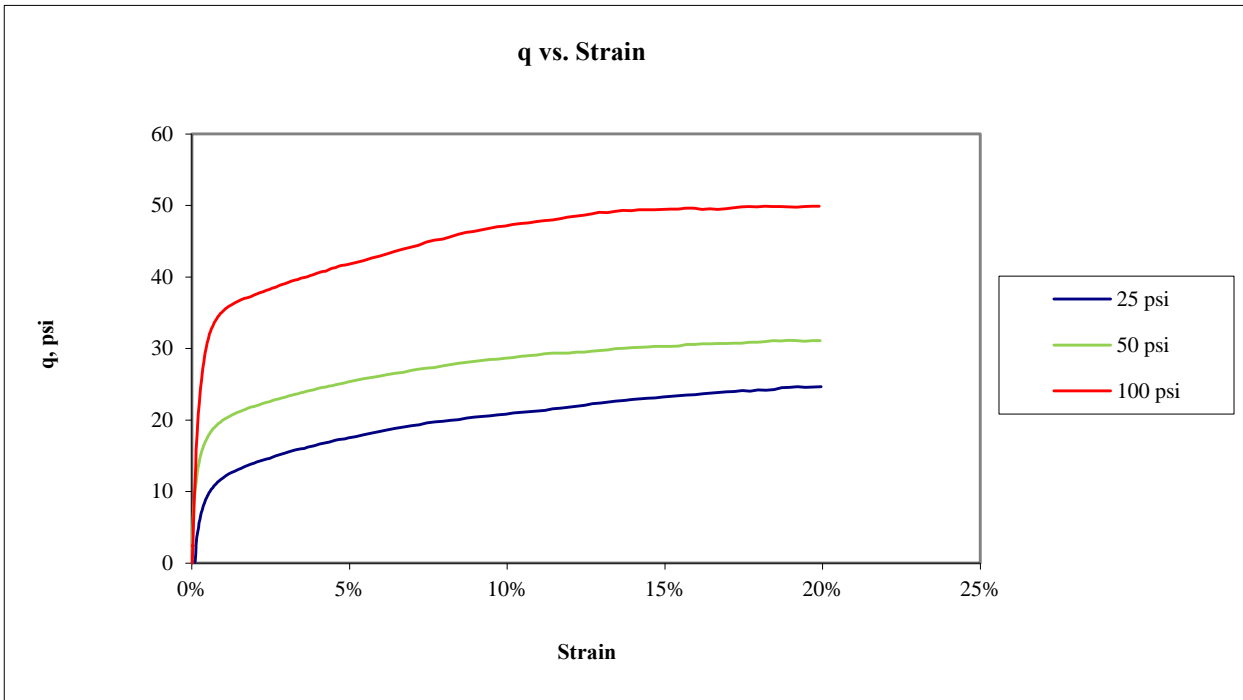
 Failure criterion:

(σ' ₁ -σ' ₃) _{max}	<input type="checkbox"/>
5 % strain	<input checked="" type="checkbox"/>

 Membrane effect:

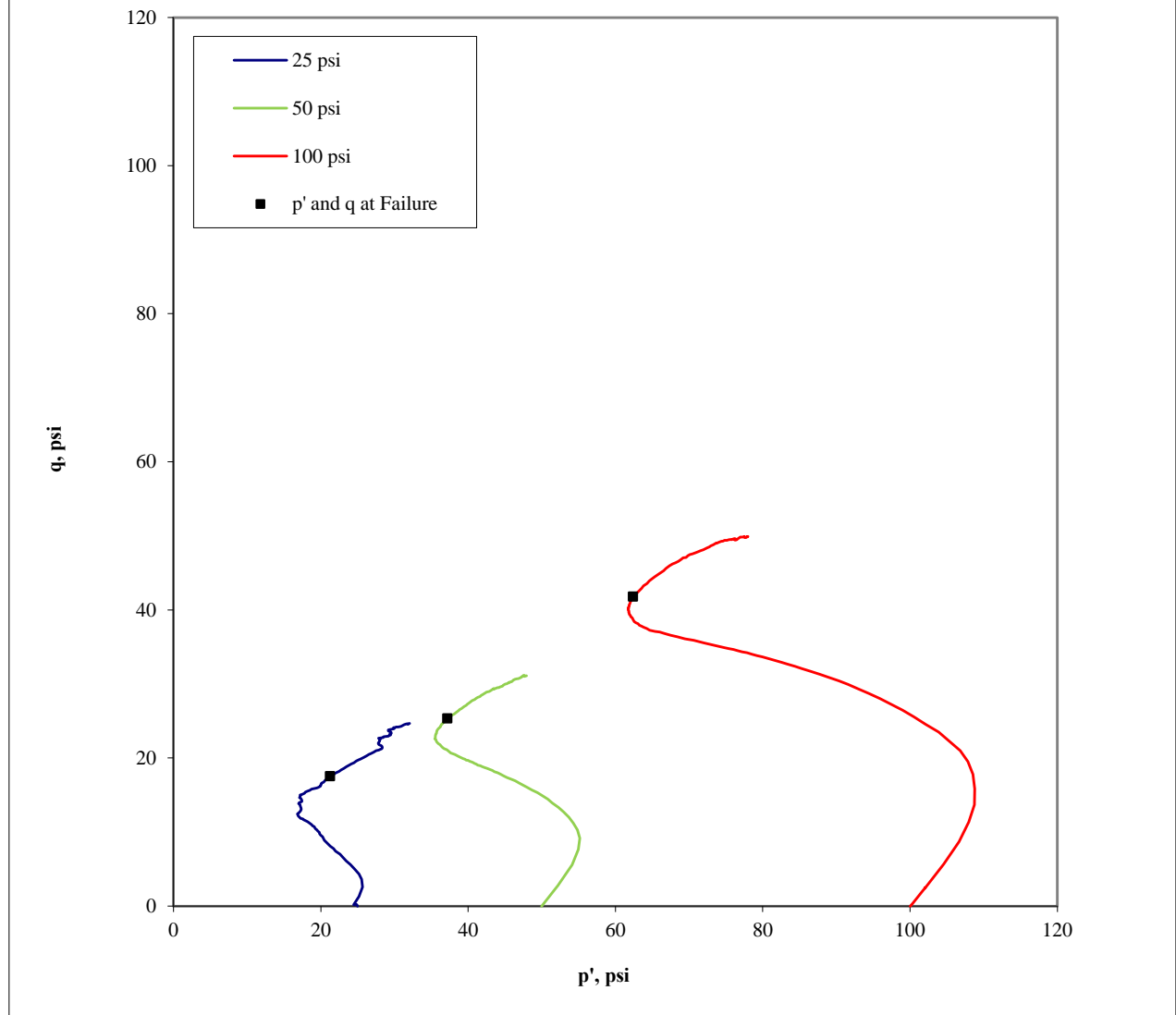
Corrected	<input checked="" type="checkbox"/>
Not Corrected	<input type="checkbox"/>

Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 2

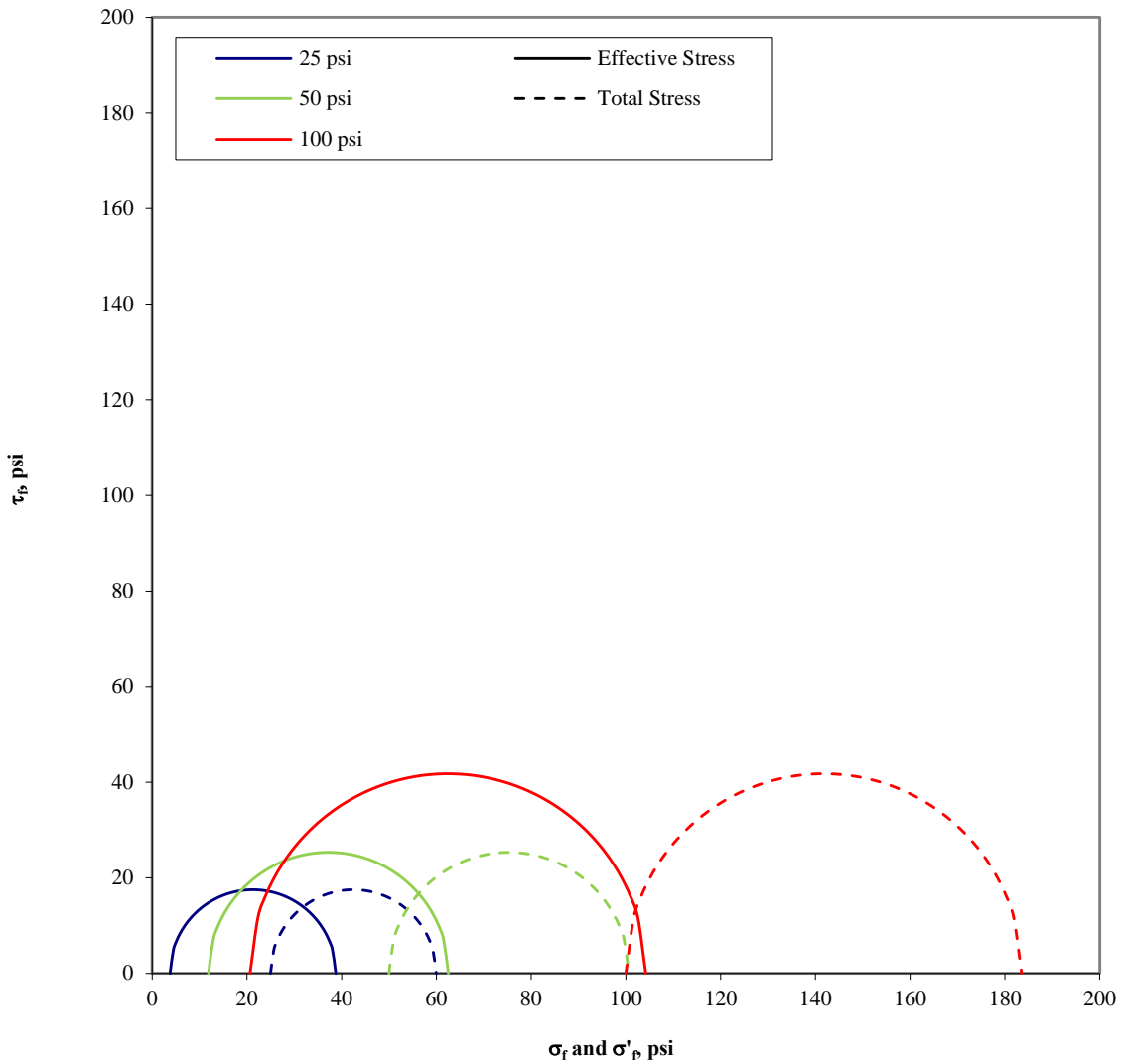
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	42.5	21.3	17.5
50	75.3	37.2	25.3
100	141.8	62.4	41.8

<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: BH-25 @ 0.0 - 12.5 ft.</p>	<p>Technician: RJM</p>	<p>Reviewed: CCS</p>	<p>Date: 5/1/2013</p>	<p>Job Number: 103-92557.006</p>	<p>Figure: 3</p>

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
25	38.8	3.8	60.0	25.0
50	62.5	11.9	100.6	50.0
100	104.2	20.7	183.5	100.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 4



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 5



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 6



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 7

Boring or Test Pit: --
 Sample: TP-10
 Depth: 3.0 - 12.0 ft
 Point No.: 1

Boring or Test Pit: --
 Sample: TP-10
 Depth: 3.0 - 12.0 ft
 Point No.: 2

Boring or Test Pit: --
 Sample: TP-10
 Depth: 3.0 - 12.0 ft
 Point No.: 3

Initial
 Length = 5.786 in
 Diameter = 2.886 in
 Wet Mass = 2.610 lb
 Area = 6.542 in²
 Volume = 37.850 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.256 lb
 Moisture Content = 15.7%
 Wet Unit Weight = 119.2 pcf
 Dry Unit Weight = 103.0 pcf
 Void Ratio = 0.66
 Percent Saturation = 65%

Initial
 Length = 5.784 in
 Diameter = 2.886 in
 Wet Mass = 2.611 lb
 Area = 6.542 in²
 Volume = 37.836 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.247 lb
 Moisture Content = 16.2%
 Wet Unit Weight = 119.2 pcf
 Dry Unit Weight = 102.6 pcf
 Void Ratio = 0.66
 Percent Saturation = 67%

Initial
 Length = 5.796 in
 Diameter = 2.886 in
 Wet Mass = 2.607 lb
 Area = 6.542 in²
 Volume = 37.915 in³
 Specific Gravity = 2.74 (Provided)
 Dry Mass of Solids = 2.240 lb
 Moisture Content = 16.4%
 Wet Unit Weight = 118.8 pcf
 Dry Unit Weight = 102.1 pcf
 Void Ratio = 0.67
 Percent Saturation = 67%

After Consolidation
 Length = 5.747 in
 Diameter = 2.842 in
 Area = 6.345 in² (Method B)
 Volume = 36.467 in³
 Moisture Content = 21.8%
 Wet Unit Weight = 130.2 pcf
 Dry Unit Weight = 106.9 pcf
 Void Ratio = 0.60
 Percent Saturation = 100%

After Consolidation
 Length = 5.724 in
 Diameter = 2.855 in
 Area = 6.400 in² (Method B)
 Volume = 36.631 in³
 Moisture Content = 22.3%
 Wet Unit Weight = 129.6 pcf
 Dry Unit Weight = 106.0 pcf
 Void Ratio = 0.61
 Percent Saturation = 100%

After Consolidation
 Length = 5.667 in
 Diameter = 2.884 in
 Area = 6.532 in² (Method B)
 Volume = 37.016 in³
 Moisture Content = 23.1%
 Wet Unit Weight = 128.7 pcf
 Dry Unit Weight = 104.6 pcf
 Void Ratio = 0.63
 Percent Saturation = 100%

B Parameter = 0.96
 Shear Rate = 0.083% /min.
 t₅₀ = 0.5 min.
 Strain at Failure = 5.0%

B Parameter = 0.95
 Shear Rate = 0.083% /min.
 t₅₀ = 0.6 min.
 Strain at Failure = 5.0%

B Parameter = 0.95
 Shear Rate = 0.083% /min.
 t₅₀ = 0.4 min.
 Strain at Failure = 5.0%

Cell Pressure = 95 psi
 Back Pressure = 70 psi
 Confining Pressure = 25 psi

Cell Pressure = 130 psi
 Back Pressure = 80 psi
 Confining Pressure = 50 psi

Cell Pressure = 190 psi
 Back Pressure = 90 psi
 Confining Pressure = 100 psi

Notes: Sample description: Silty sand, brownish yellow, moist
 Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 49% (ASTM D422, refer to separate report for gradation curve)
 Specimen type:

Intact	X
Cuttings	X

 Reconstituted Remold targets: 104.7 pcf (dry) at 16.2% moisture
 Moisture from:

Wet	
Dry	

 Entire specimen
 Saturation method:

X	Wet
	Dry

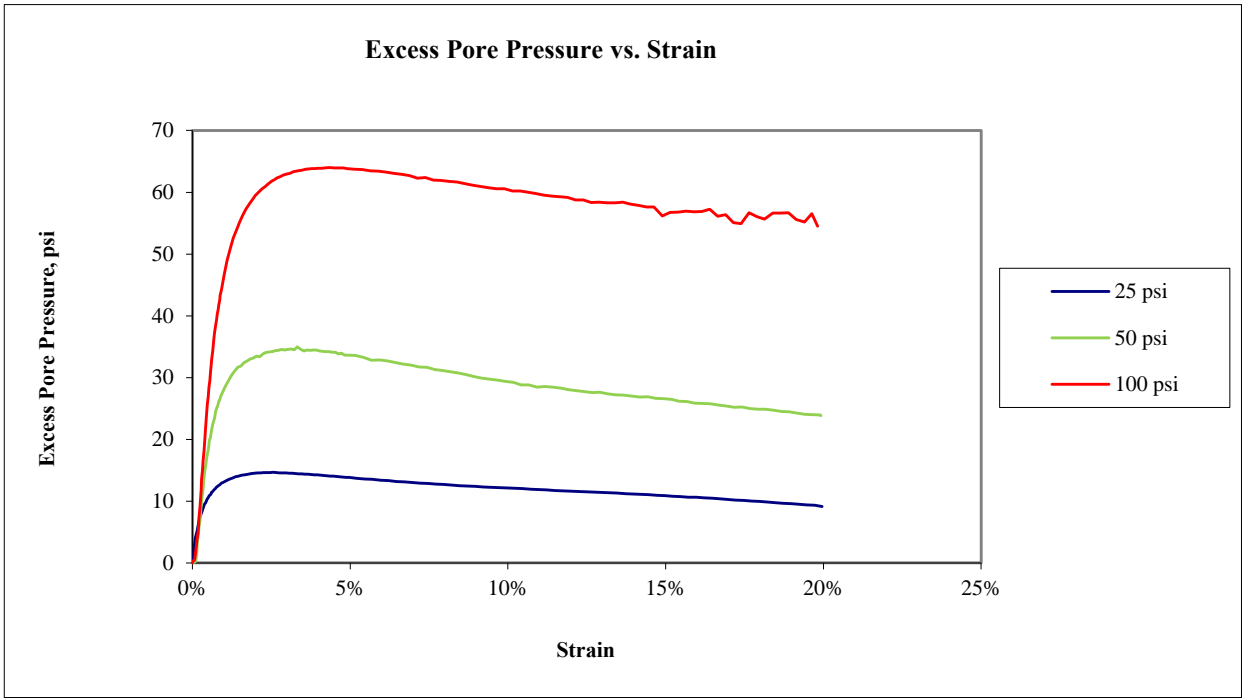
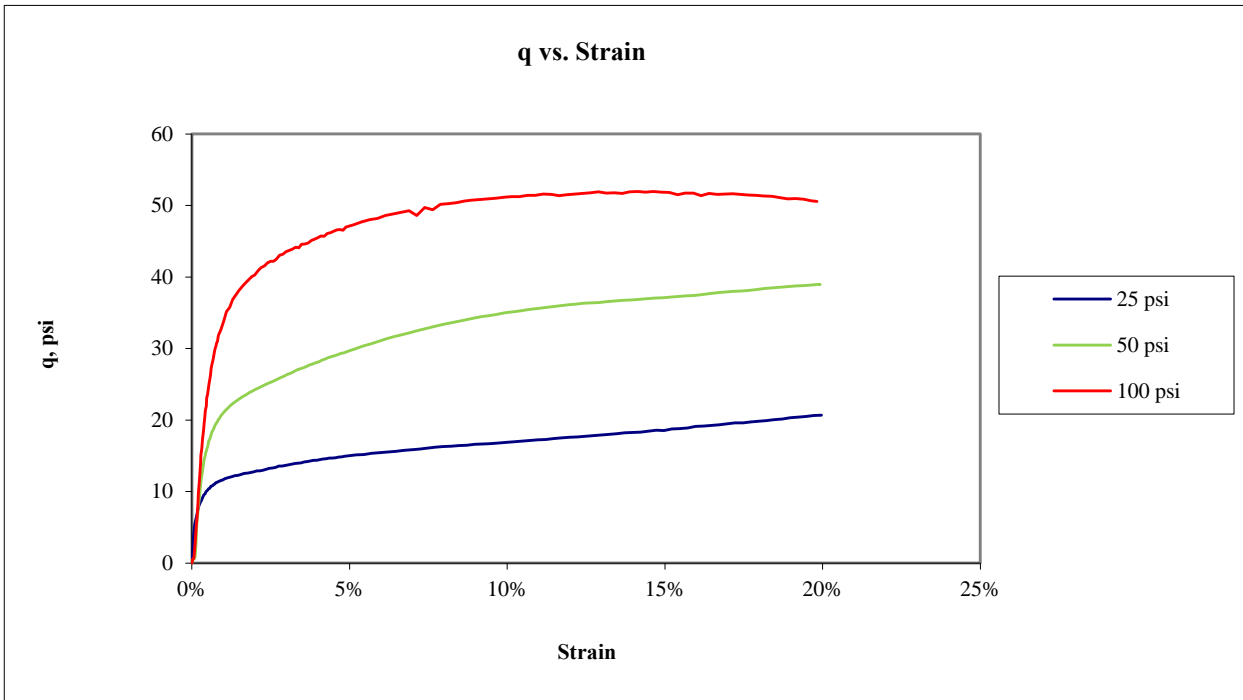
 Failure criterion:

(σ ₁ /σ ₃) _{max}	(σ ₁ -σ ₃) _{max}	5
--	--	---

 % strain
 Membrane effect:

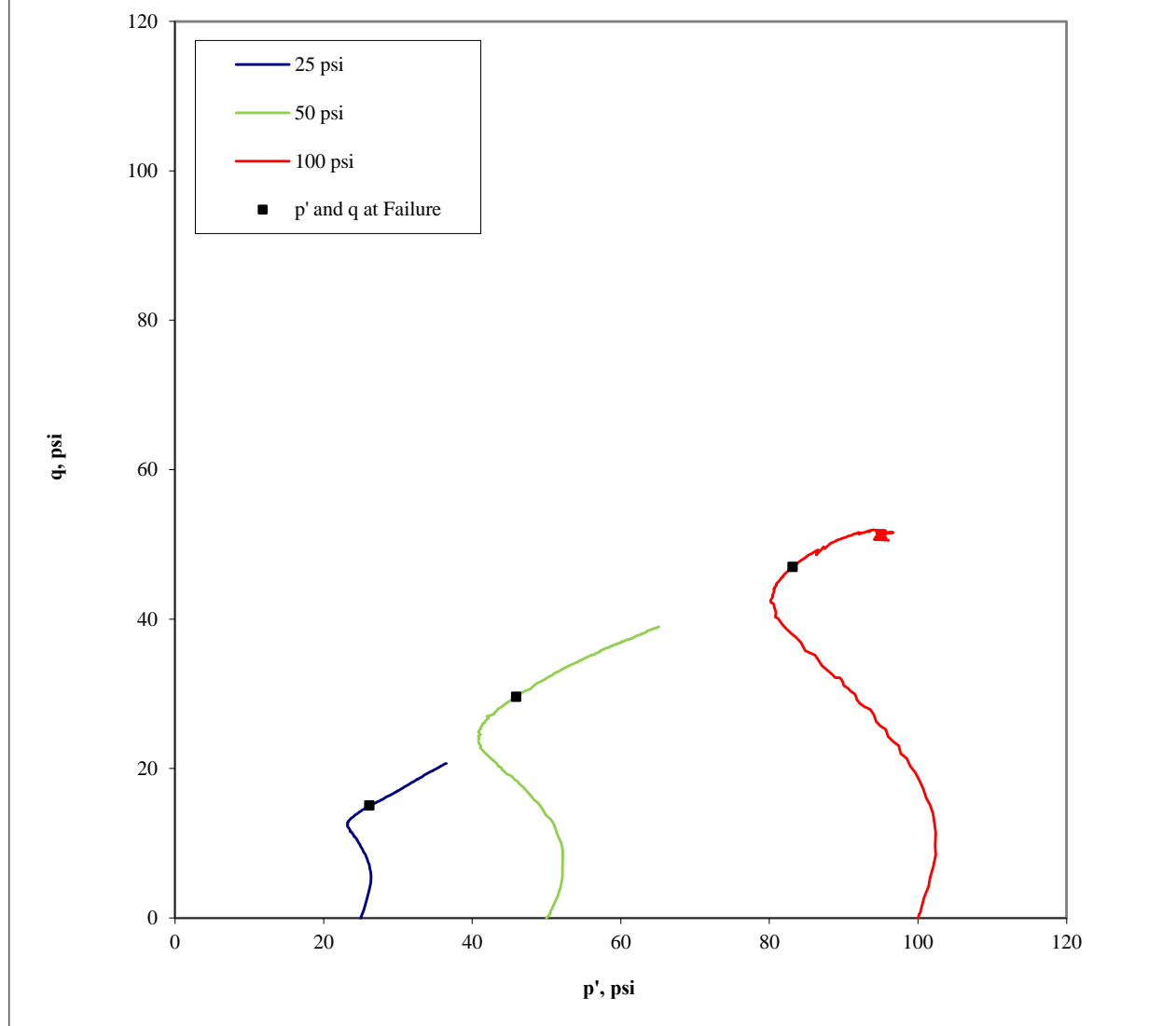
X	Corrected	Not Corrected
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Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA				
Job Short Title: Copper Flat Tailings Design Study					
Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013	Job Number: 103-92557.006	Figure: 2

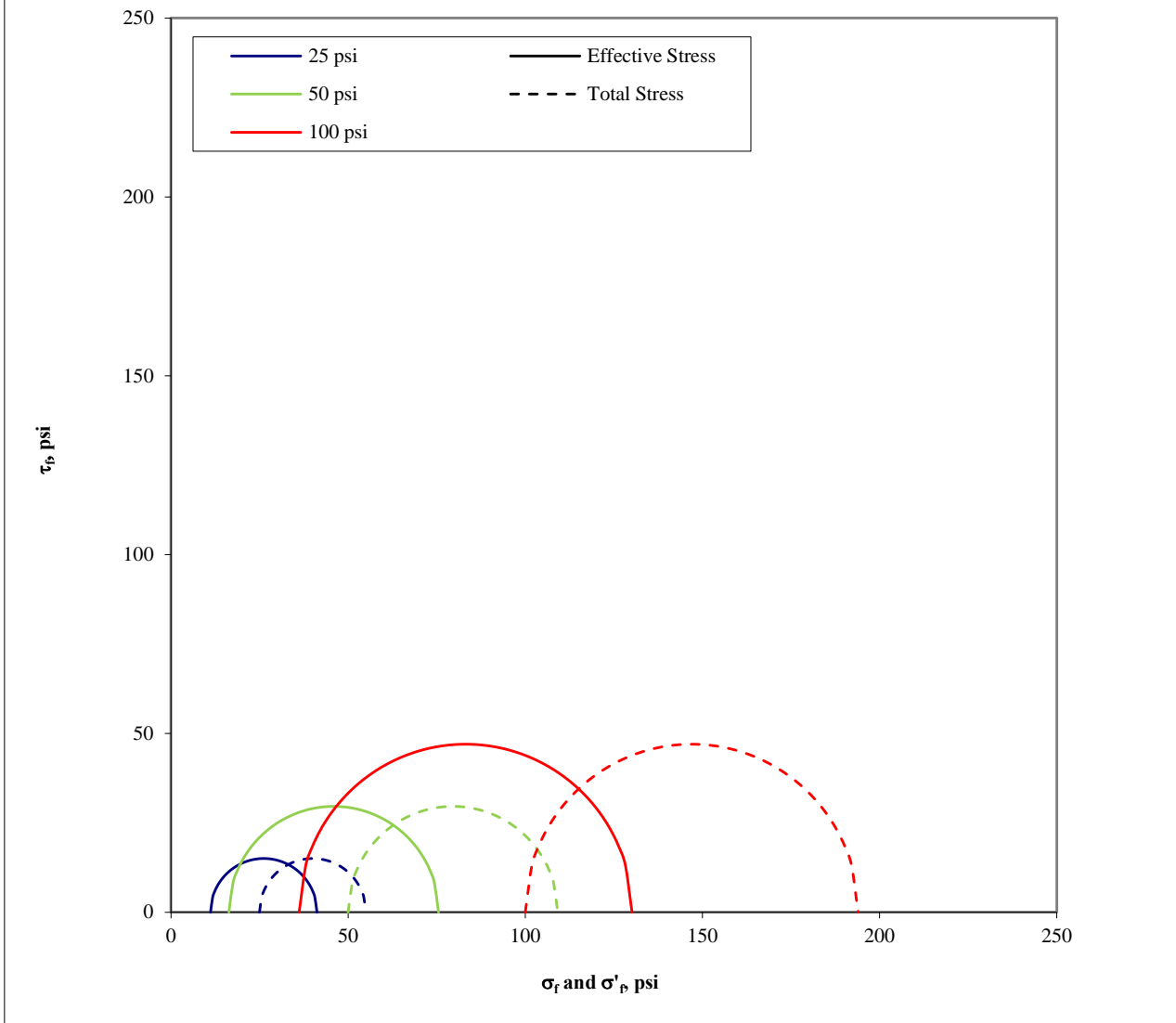
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	40.0	26.2	15.0
50	79.6	45.9	29.6
100	147.0	83.2	47.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT			
Job Short Title: Copper Flat Tailings Design Study					
Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013	Job Number: 103-92557.006	Figure: 3

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
25	41.2	11.2	55.0	25.0
50	75.5	16.4	109.2	50.0
100	130.1	36.2	193.9	100.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013	Job Number: 103-92557.006	Figure: 4



Golder Associates Inc. Denver, Colorado		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT			
		SPECIMEN PHOTOGRAPH - 25 psi			
Sample:	TP-10 @ 3 - 12 ft	Technician:	RJM	Reviewed:	CCS
		Date:	4/8/2013	Job Number:	103-92557.006
				Figure:	5



Golder Associates Inc. Denver, Colorado		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
		Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013
					Figure: 6



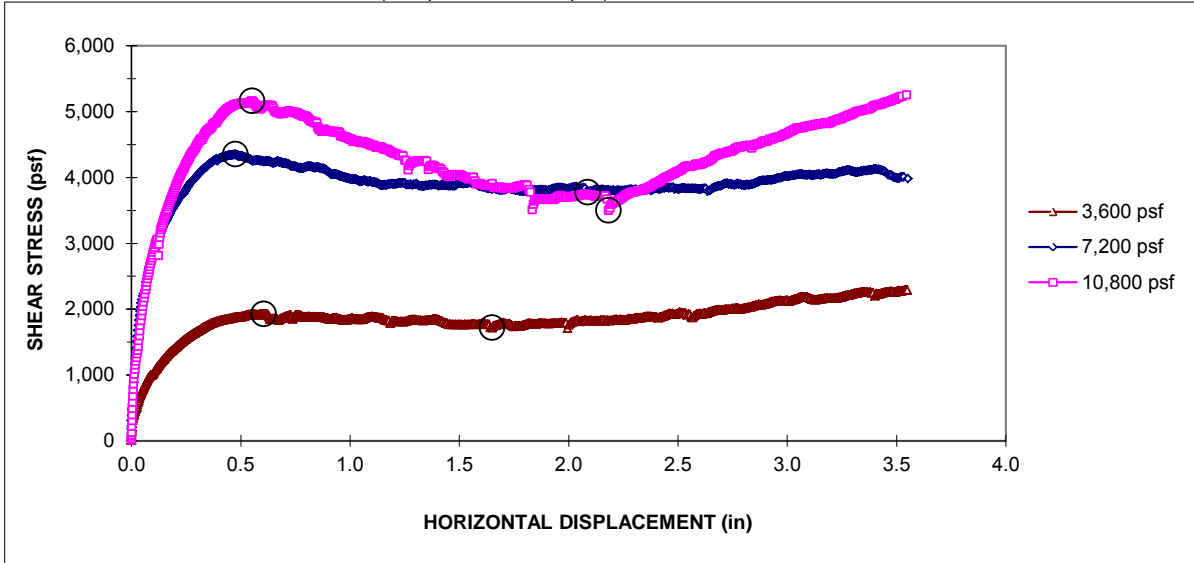
Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: TP-10 @ 3 - 12 ft	Technician: RJM	Reviewed: CCS	Date: 4/8/2013	Job Number: 103-92557.006	Figure: 7

DIRECT SHEAR TEST RESULTS

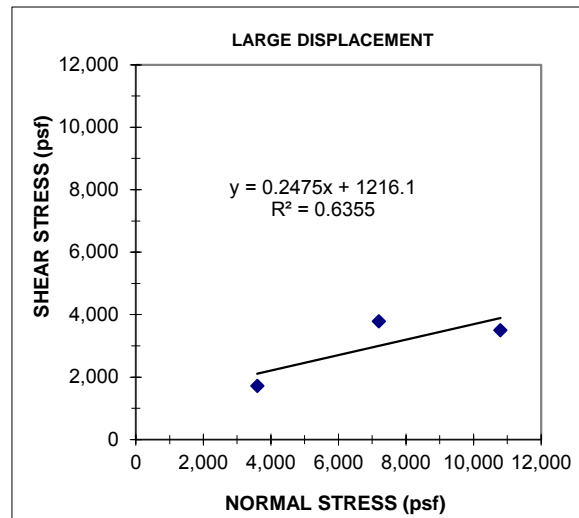
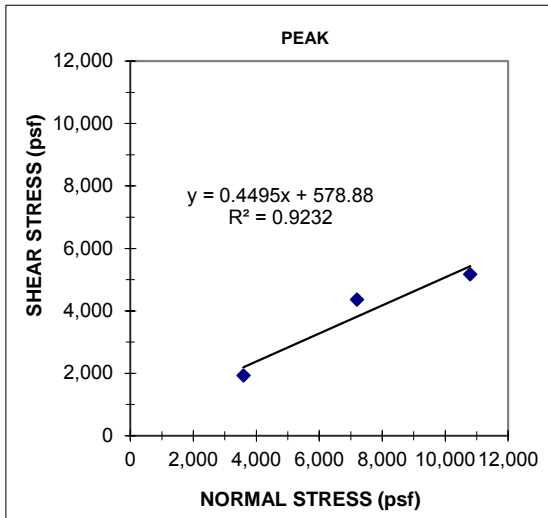
ASTM D5321

PROJECT NAME: Copper Flat Tailings Design Study
 SAMPLE NUMBER: Tailings / 80-mil Textured Geomembrane / Drain Rock

APPARTUS: 12.0 inch by 12.0 inch shear box; air bladder used to apply normal loads.
INTERFACE TESTED: Top: Tailings (remolded to 104.7 pcf @ 15.7% moisture content)
 Bottom: GSE 80-mil HDPE double-sided textured geomembrane
TEST CONDITIONS: Drain Rock inundated; consolidated overnight at normal load; Floating geomembrane
SHEAR RATE: 0.04 in/min
SUBSTRATE: Drain Rock (Composites 1-4, scalped)



Normal Stress (psf)	Shear Stress		Peak		Large Displacement	
	Peak ¹ (psf)	Lg. Displ. (psf)	Friction Angle	Adhesion ² (psf)	Friction Angle	Adhesion ² (psf)
3,600	1,927	1,717	24.2	578.9	13.9	1216.1
7,200	4,354	3,778				
10,800	5,164	3,500				



Observations After Test

Peak and residual shear strengths were chosen based on testing observations. Shear stresses measured at greater horizontal displacements than those chosen for the large-displacement shear strength appeared to have been affected by gravel particles pushing against the end of the bottom shear box.

- 3,600 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface
- 7,200 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface
- 10,800 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface
 Drain Gravel interfered with shear boxes at approximately 2.2 inches displacement and may have affected

Tech: PRH
 Review: CCS

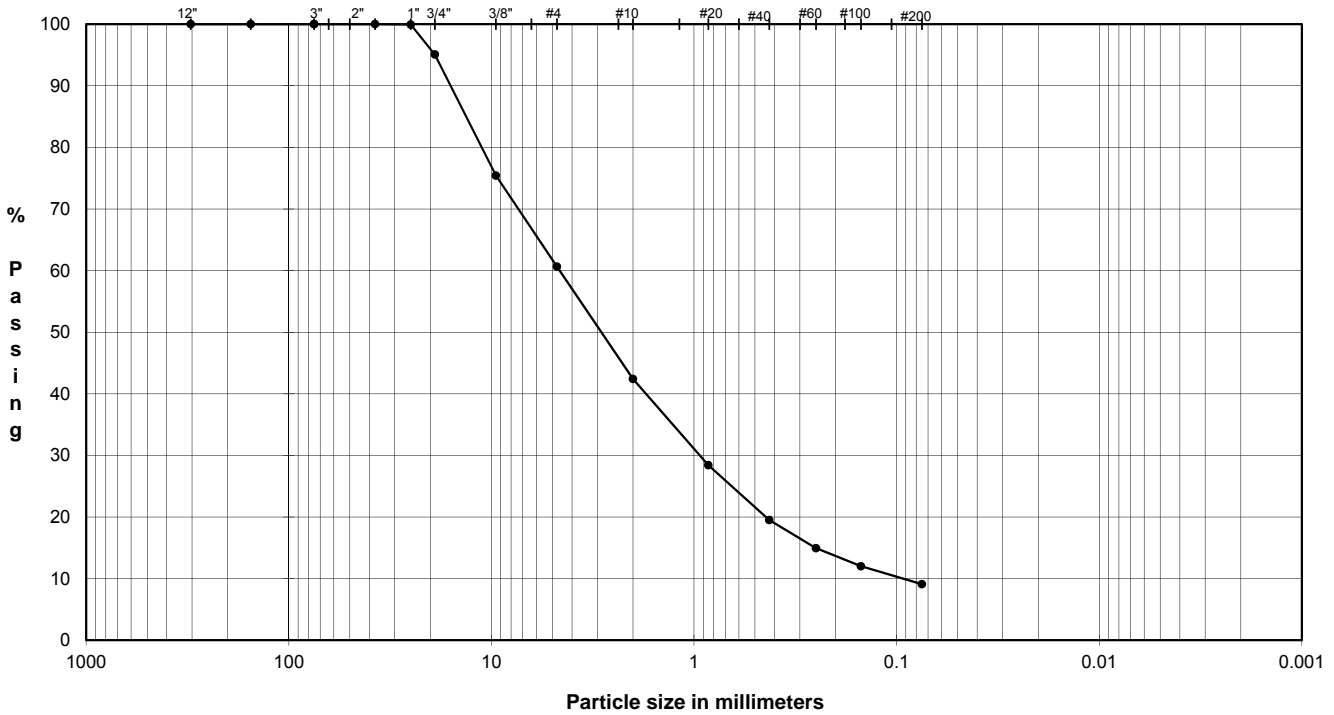
(1) Peak shear stresses for 3,600 psf, 7,200 psf, and 10,800 psf normal stresses were chosen at 0.606, 0.476, and 0.552 inches horizontal displacements, respectively.

(2) Interface shear parameters are based on the "best-fit" line per ASTM D5321. Interpretation of the test data by a qualified professional for the specific application is required.

**APPENDIX A.3.3
PERMEABILITY TEST REPORTS**

PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Compostie 1-4** Depth (ft) **--**
 TYPE: **Pail**

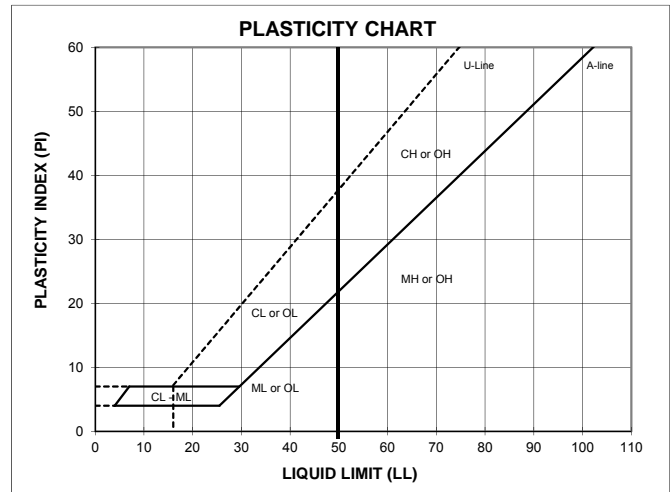


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

PRE-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		100.0
6.0"	154.2		100.0
3.0"	75.0		100.0
3.0"	75.0		100.0
1.5"	37.5		100.0
1.0"	25.0		99.9
0.75"	19.0	Coarse Gravel	4.93
0.375"	9.5		75.4
#4	4.8	Fine Gravel	34.42
#10	2.0	Coarse Sand	18.25
#20	0.9		28.4
#40	0.4	Medium Sand	22.89
#60	0.3		14.9
#100	0.2		12.0
#200	0.1	Fine Sand	10.42
		Fines	9.09



ATTERBERG LIMITS

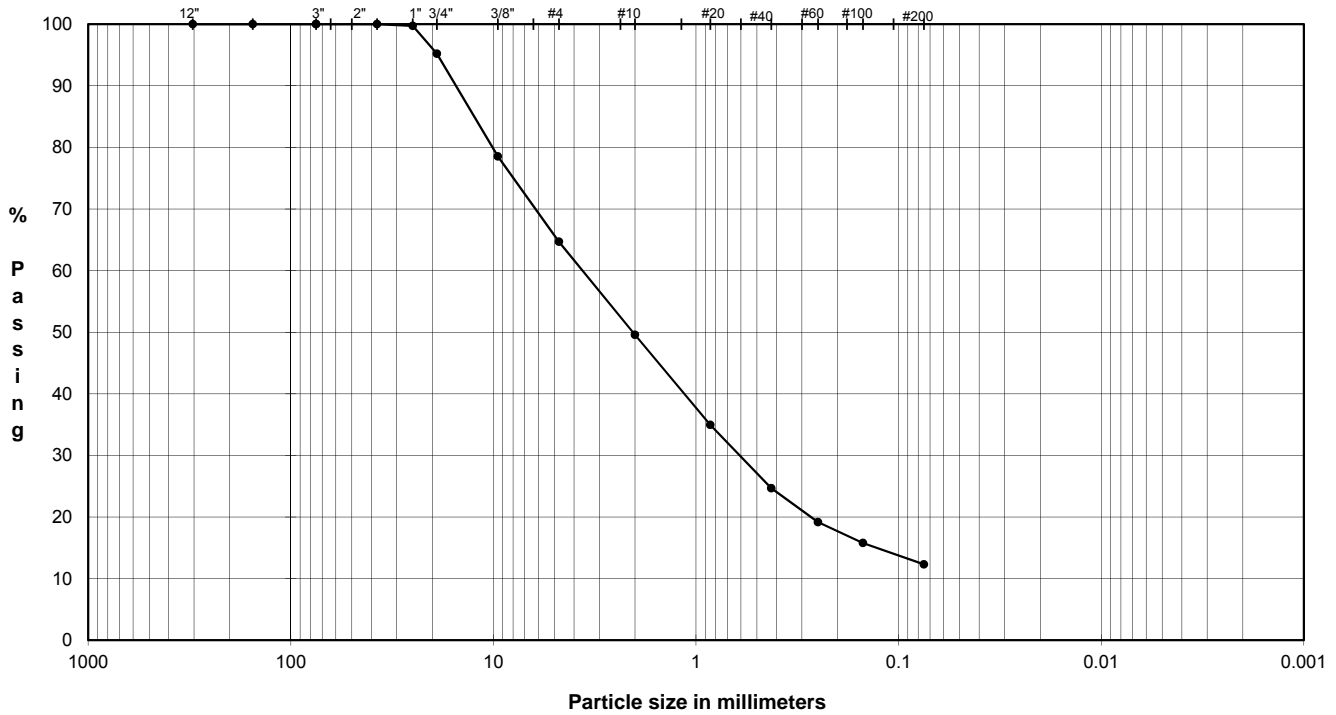
M_c	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown,
 (Golder Procedure): dry
 USCS: --

TECH: EH
 DATE: 4/10/13
 REVIEW: MB

POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Compostie 1-4** Depth (ft) --
 TYPE: **Pail**

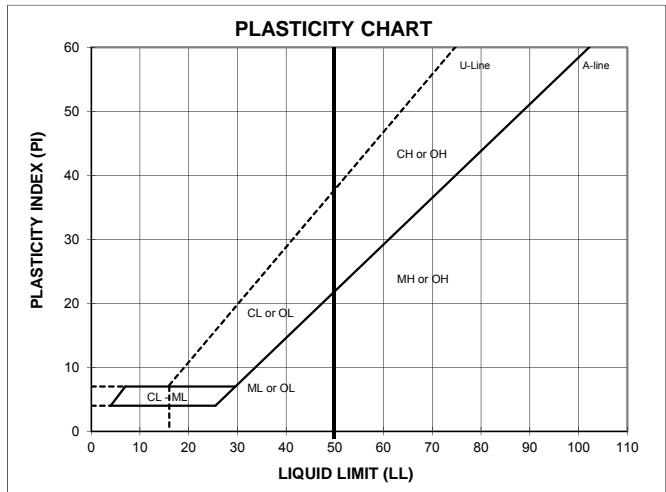


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

POST-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		100.0
6.0"	154.2	Cobbles	100.0
3.0"	75.0		100.0
3.0"	75.0		100.0
1.5"	37.5	Coarse Gravel	100.0
1.0"	25.0		99.7
0.75"	19.0		95.2
0.375"	9.5	Fine Gravel	78.5
#4	4.8		64.7
#10	2.0	Coarse Sand	49.6
#20	0.9	Medium Sand	34.9
#40	0.4		24.7
#60	0.3		19.2
#100	0.2	Fine Sand	15.8
#200	0.1		12.3
Fines			12.31



ATTERBERG LIMITS

M_c	LL	PL	PI	SG
--	--	--	--	--

Visual Description: **silty clayey SAND and GRAVEL, yellowish brown,**
 (Golder Procedure): **dry**
 USCS: **--**

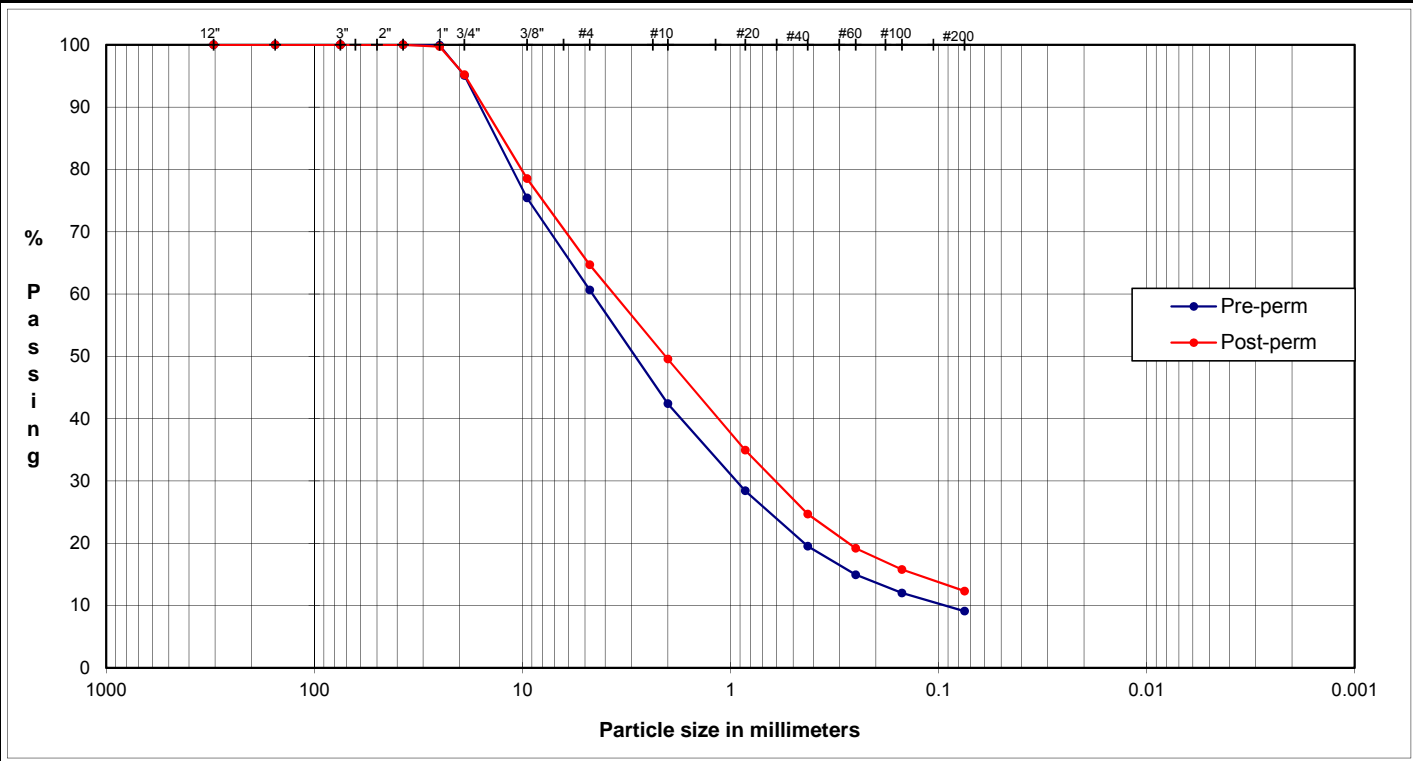
TECH: **EH**
 DATE: **4/16/2013**
 REVIEW: **MB**

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Compostie 1-4**
 TYPE: **Pail**

Depth (ft) --



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

		PRE-PERM			
		Particle Size (mm)	% Passing	Classification	Percentage
U.S. Standard Sieves Sizes and Numbers	12.0"	304.8	100.0	Cobbles	0.00
	6.0"	154.2	100.0		
	3.0"	75.0	100.0		
	3.0"	75.0	100.0		
	1.5"	37.5	100.0		
	1.0"	25.0	99.9	Coarse Gravel	4.93
	0.75"	19.0	95.1		
	0.375"	9.5	75.4		
	#4	4.8	60.7	Fine Gravel	34.42
	#10	2.0	42.4	Coarse Sand	18.25
	#20	0.9	28.4	Medium Sand	22.89
	#40	0.4	19.5		
	#60	0.3	14.9		
	#100	0.2	12.0	Fine Sand	10.42
#200	0.1	9.1			
				Fines	9.09

		POST-PERM			
		Particle Size (mm)	% Passing	Classification	Percentage
U.S. Standard Sieves Sizes and Numbers	12.0"	304.8	100.0	Cobbles	0.00
	6.0"	154.2	100.0		
	3.0"	75.0	100.0		
	3.0"	75.0	100.0		
	1.5"	37.5	100.0		
	1.0"	25.0	99.7	Coarse Gravel	4.79
	0.75"	19.0	95.2		
	0.375"	9.5	78.5		
	#4	4.8	64.7	Fine Gravel	30.52
	#10	2.0	49.6	Coarse Sand	15.12
	#20	0.9	34.9	Medium Sand	24.90
	#40	0.4	24.7		
	#60	0.3	19.2		
	#100	0.2	15.8	Fine Sand	12.36
#200	0.1	12.3			
				Fines	12.31

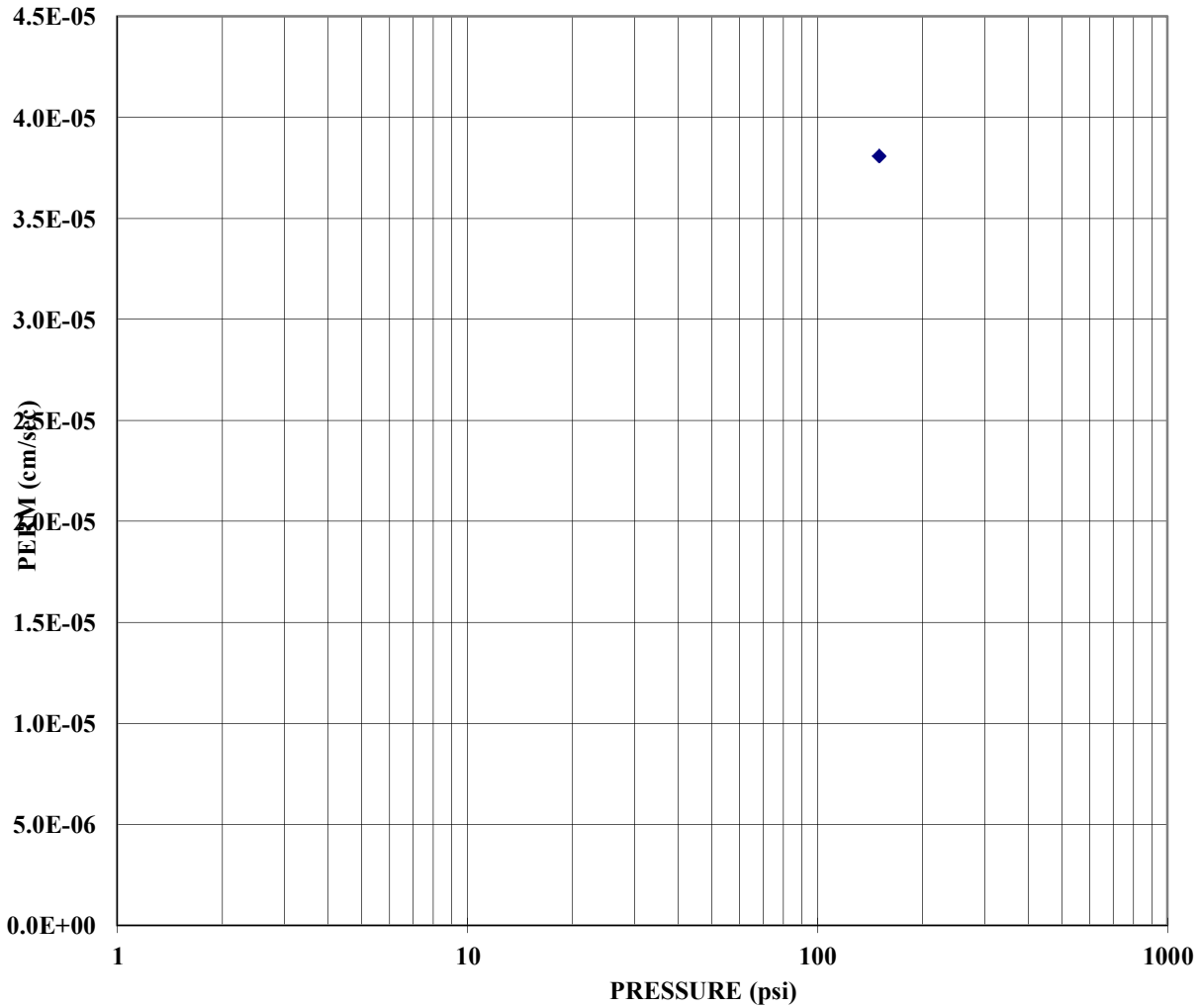
ATTERBERG LIMITS

M_c	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown, (Golder Procedure): dry
 USCS: --

TECH: EH
 DATE: 4/10/13
 REVIEW: MB

ONE-DIMENSIONAL CONSOLIDATION X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Repor



SAMPLE #: Composite 1-4

Visual Description: silty clayey SAND and
 (Golder Procedure): GRAVEL, yellowish brown, dry

DATE 4/11/2013

TECH MGC

REVIEW MB

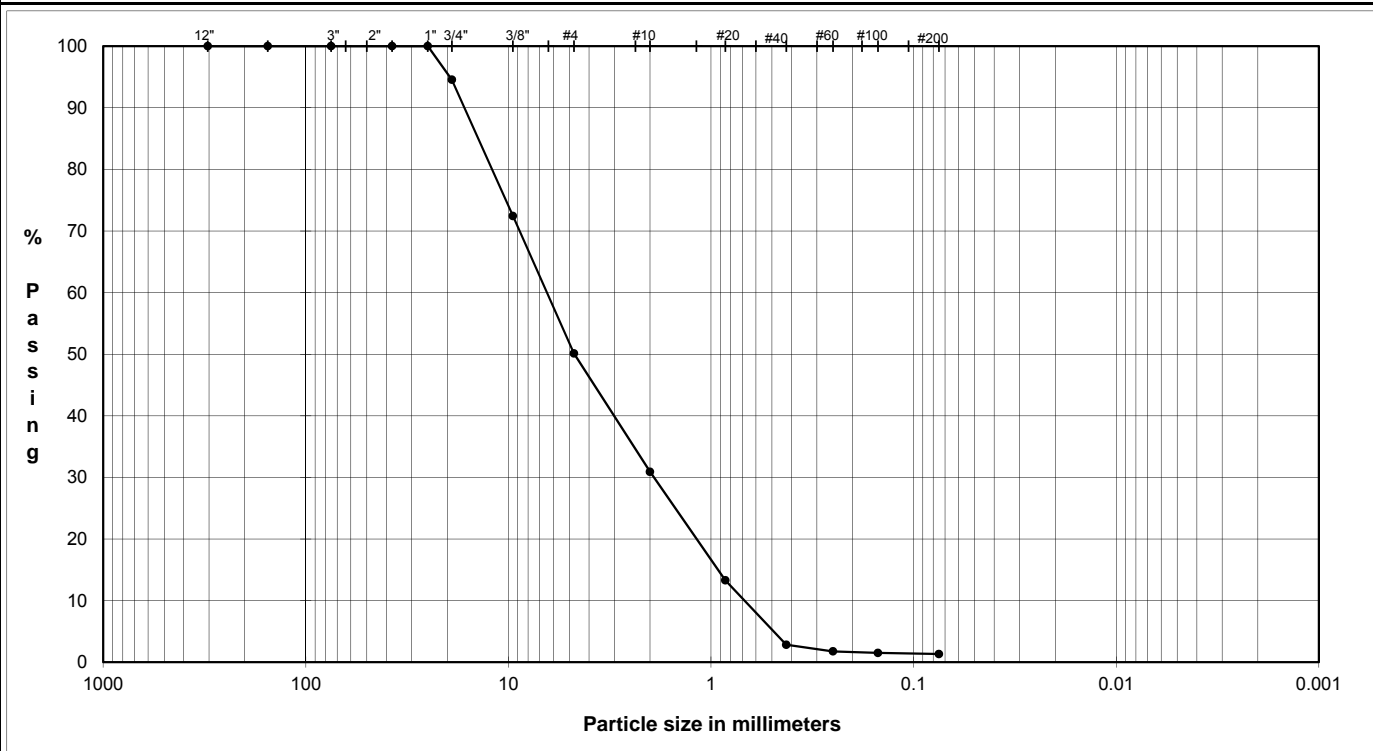
103-92557.006
 Copper Flat Tailings Design Study

GOLDER ASSOCIATES INC.
 LAKEWOOD, COLORADO



PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) --
 TYPE: **Pail**

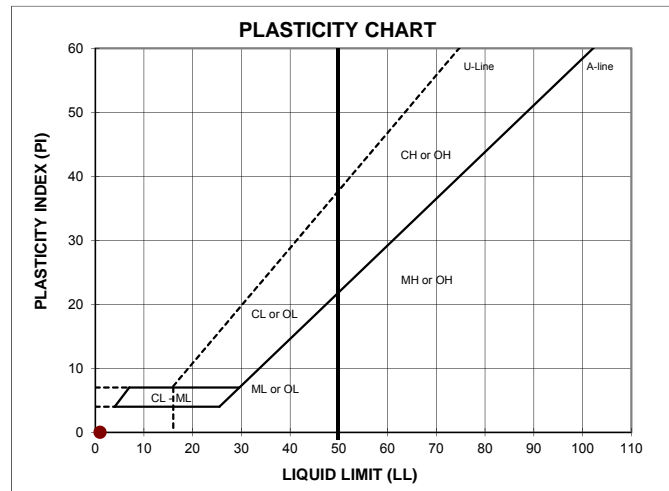


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

PRE-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		
6.0"	154.2		
3.0"	75.0	Cobbles	0.00
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	5.45
0.375"	9.5		
#4	4.8	Fine Gravel	44.42
#10	2.0	Coarse Sand	19.22
#20	0.9		
#40	0.4	Medium Sand	28.06
#60	0.3		
#100	0.2		
#200	0.1	Fine Sand	1.51
		Fines	1.33



ATTERBERG LIMITS

M_c	LL	PL	PI	SG
--	--	--	--	--

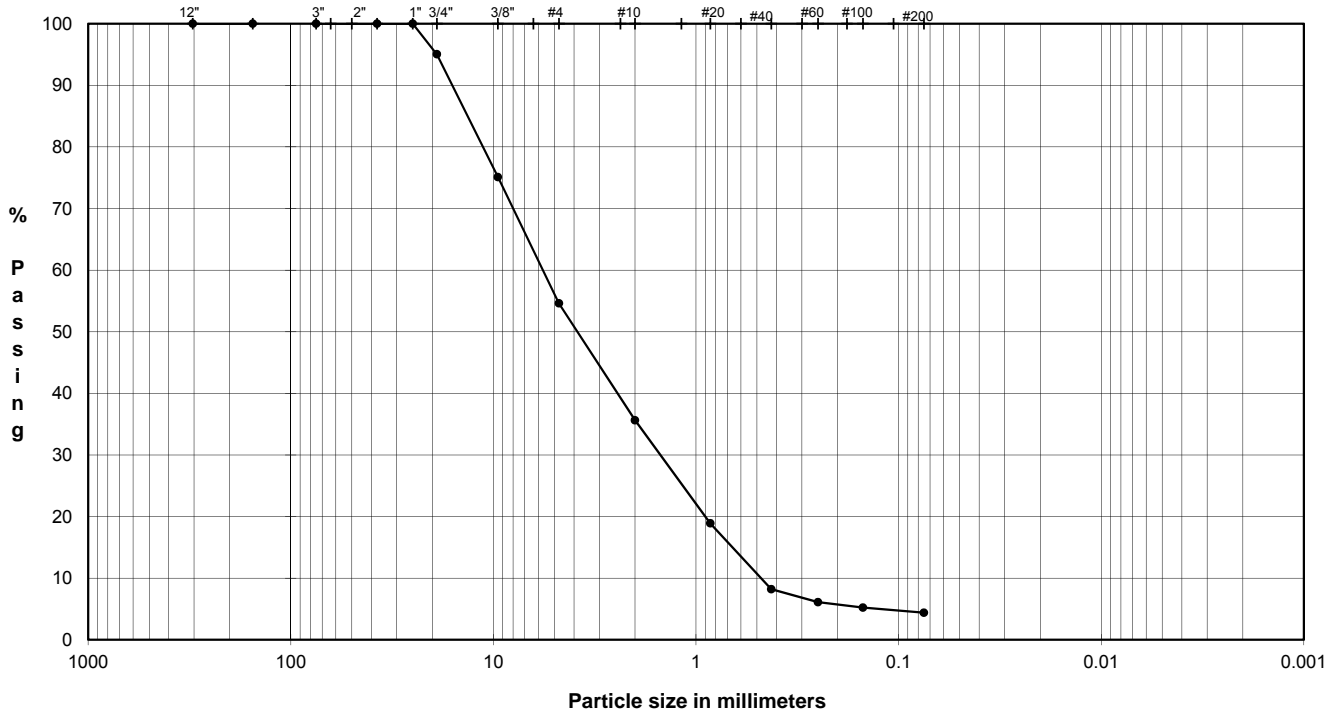
DESCRIPTION: SAND and GRAVEL, yellowish brown

USCS: --

TECH: AM
 DATE: 4/22/13
 REVIEW: PRH

POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) **--**
 TYPE: **Pail**

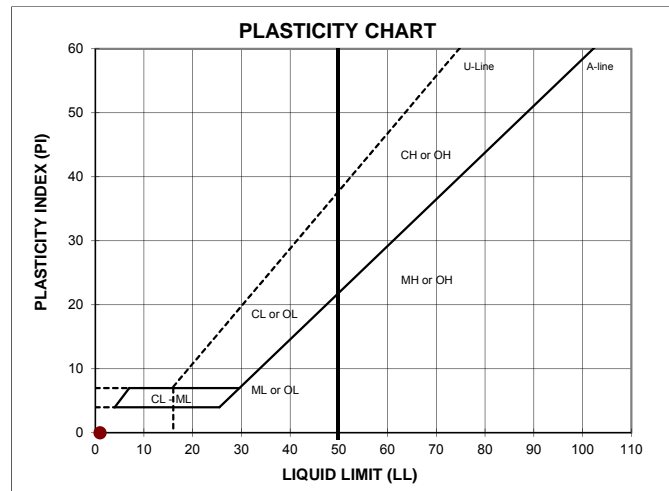


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

POST-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		100.0
6.0"	154.2		100.0
3.0"	75.0	Cobbles	0.00
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	4.96
0.375"	9.5		
#4	4.8	Fine Gravel	40.43
#10	2.0	Coarse Sand	18.97
#20	0.9		
#40	0.4	Medium Sand	27.43
#60	0.3		
#100	0.2		
#200	0.1	Fine Sand	3.82
		Fines	4.39



ATTERBERG LIMITS

M_c	LL	PL	PI	SG
--	--	--	--	--

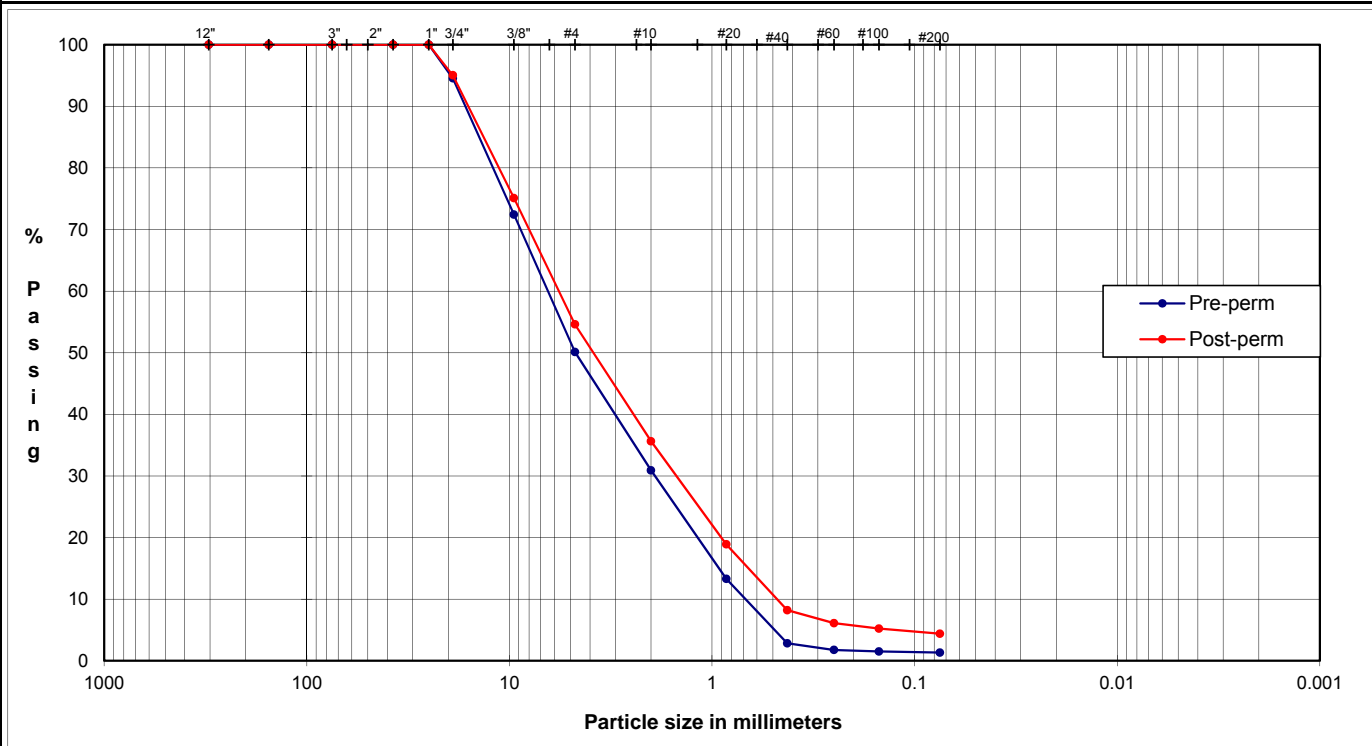
DESCRIPTION: SAND and GRAVEL, yellowish brown

USCS: --

TECH	EH
DATE	5/1/2013
REVIEW	PRH

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) --
 TYPE: **Pail**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

PRE-PERM					POST-PERM				
U.S. Standard Sieves Sizes and Numbers	Particle Size		Classification	Percentage	U.S. Standard Sieves Sizes and Numbers	Particle Size		Classification	Percentage
	(mm)	% Passing				(mm)	% Passing		
	12.0"	304.8	100.0			12.0"	304.8	100.0	
	6.0"	154.2	100.0			6.0"	154.2	100.0	
	3.0"	75.0	100.0	Cobbles	0.00		3.0"	75.0	100.0
	1.5"	37.5	100.0			1.5"	37.5	100.0	
	1.0"	25.0	100.0			1.0"	25.0	100.0	
	0.75"	19.0	94.5	Coarse Gravel	5.45		0.75"	19.0	95.0
	0.375"	9.5	72.4			0.375"	9.5	75.1	Coarse Gravel
	#4	4.8	50.1	Fine Gravel	44.42		#4	4.8	54.6
	#10	2.0	30.9	Coarse Sand	19.22		#10	2.0	35.6
	#20	0.9	13.3				#20	0.9	18.9
	#40	0.4	2.8	Medium Sand	28.06		#40	0.4	8.2
	#60	0.3	1.8				#60	0.3	6.1
	#100	0.2	1.5				#100	0.2	5.2
	#200	0.1	1.3	Fine Sand	1.51		#200	0.1	4.4
				Fines	1.33				
									Fines
									4.39

ATTERBERG LIMITS					
	M _c	LL	PL	PI	SG
	--	--	--	--	--
DESCRIPTION:	SAND and GRAVEL, yellowish brown				
USCS:	--				
TECH	AM				
DATE	4/22/13				
REVIEW	PRH				

Copper Flat Tailing Design Study
Table 1 - Rigid Wall Compression
Falling Head Permeability - 10 inch diameter cell

Project Title: Copper Flat Tailings Design Study
Project Number: 103-92557.006
Dates Tested: 4/26/2013 To: 4/29/2013
Boring: --
Sample: Comp 1-4 SCALPED
Depth (ft): --

Sample Setup

Initial Sample Height, in	9.194
Mold Diameter, in	10.00
Sample Area, in ²	78.54
Wet Sample Weight, g	19,998.9
Wet Sample Weight, lb	44.10
Dry Sample Weight, g	19,416.3
Dry Sample Weight, lb	42.81

Initial Sample:

Moisture Determination

Tare	PGC
Wet Weight and Tare, g	663.26
Dry Weight and Tare, g	647.65
Tare Weight, g	127.37
Moisture Content, %	3.0

Initial Sample Density and Void Ratio

Specific Gravity ¹	2.70
Initial Sample Volume, ft ³	0.418
Initial Wet Density, lb/ft ³	105.5
Initial Dry Density, lb/ft ³	102.5
Initial Void Ratio	0.64

Final Sample Density and Void Ratio

Final Sample Height, in	8.330
Final Sample Volume, ft ³	0.379
Final Dry Density, lb/ft ³	113.1
Final Void Ratio	0.49

Load (psi)	Height (in)	Dry Density (pcf)	Void Ratio	Flow Rate (ml/sec)	Gradient	Permeability (cm/sec)	Porosity
150	8.330	113.1	0.49	10.62	0.23	9.1E-02	0.33

NOTES: ¹Specific Gravity = Assumed Value

CONSTANT-HEAD PERMEABILITY (RIGID-WALL)

JOB NUMBER: 103-92557.006
 JOB NAME: Copper Flat Tailings Design Study
 DATE TESTED: 04/26/13

BORING NUMBER: --
 SAMPLE NUMBER: Comp 1-4 SCALPED
 SAMPLE DEPTH: --

Initial Moisture Content

Tare:	PGC
Wet Weight & Tare, g:	663.26
Dry Weight & Tare, g:	647.65
Tare Weight, g:	127.37
Moisture, %:	3.0

Final Moisture Content

Tare:	J11
Wet Weight & Tare, g:	662.43
Dry Weight & Tare, g:	607.20
Tare Weight, g:	82.33
Moisture, %:	10.5

Initial Height Determination (Inches)

(Height from mold rim to plate on sample)	
1.	3.447
2.	3.442
3.	3.404
4.	3.379
5.	3.404
6.	3.365
Average	3.406
Cell Height	12.600
Sample Height ¹	9.194

Density

Wet Weight:	19,998.90	g
Dry Weight:	19,416.35	g
Diameter:	10.000	in 25.40 cm
Area:	78.540	in ² 506.71 cm ²
Initial Height:	9.194	in 23.35 cm
Final Height ² :	8.330	in 21.16 cm
Initial Volume:	0.418	ft ³ 11,832.48 cm ³
Final Volume:	0.379	ft ³ 10,721.13 cm ³
Initial Wet Density:	105.6	pcf
Final Wet Density:	116.5	pcf
Initial Dry Density:	102.5	pcf
Final Dry Density:	113.1	pcf

TRIAL	TIME	OUTFLOW	BURETTE	Q	i	k
	seconds	ml	(mm)	(cm ³ /sec)		cm/sec
01	240	1704.9	35	7.1	0.165	8.48E-02
02	240	1704.5	35	7.1	0.165	8.47E-02
03	240	1705.2	35	7.1	0.165	8.48E-02
04	240	2513.2	48	10.5	0.227	9.11E-02
05	240	2508.93	48	10.5	0.227	9.09E-02
06	240	2507.75	48	10.4	0.227	9.09E-02
07	240	3436.38	62	14.3	0.293	9.64E-02
08	240	3434.01	62	14.3	0.293	9.64E-02
09	240	3435.08	62	14.3	0.293	9.64E-02
10						
Average	240.0	2549.996	48.3	10.6	0.228	9.07E-02

NOTES: ¹Sample Height = Cell Height - Average distance to rim of mold

²Final Height = Initial Height - Displacement of Final Reading *Before* Running Perm Test

CONSTANT-HEAD PERMEABILITY (RIGID-WALL)

JOB NUMBER: 103-92557.006
 JOB NAME: Copper Flat Tailings Design Study
 DATE TESTED: 04/26/13

BORING NUMBER: --
 SAMPLE NUMBER: Comp 1-4 SCALPED
 SAMPLE DEPTH: --

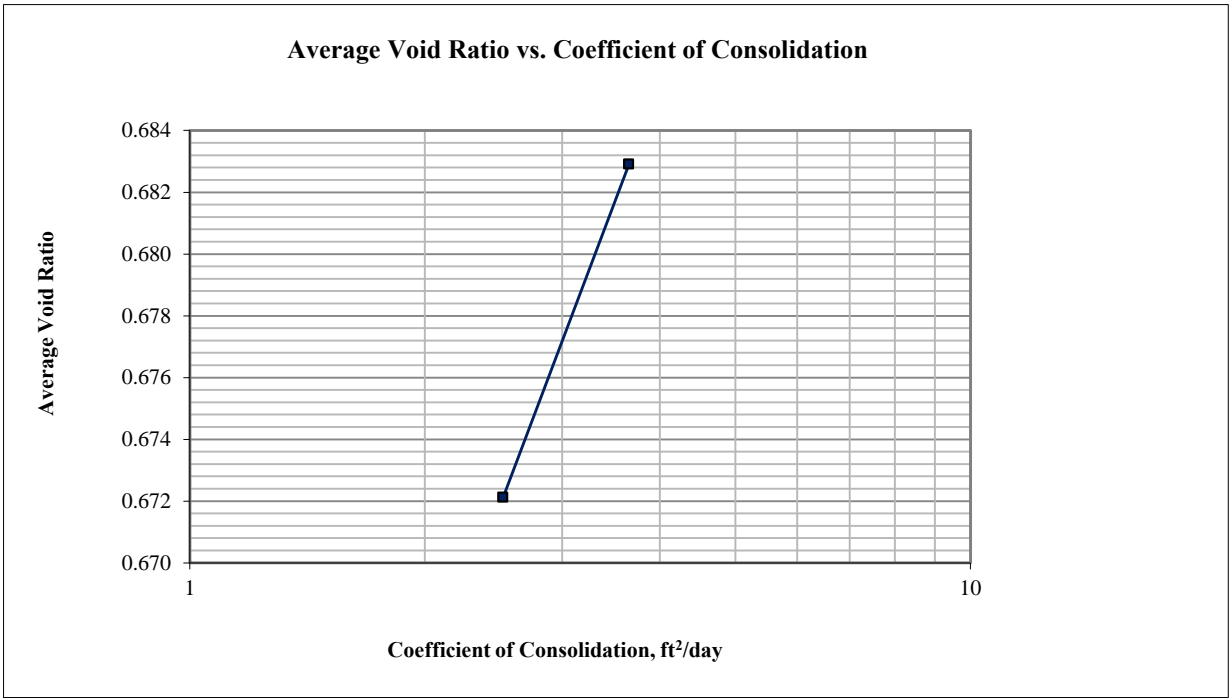
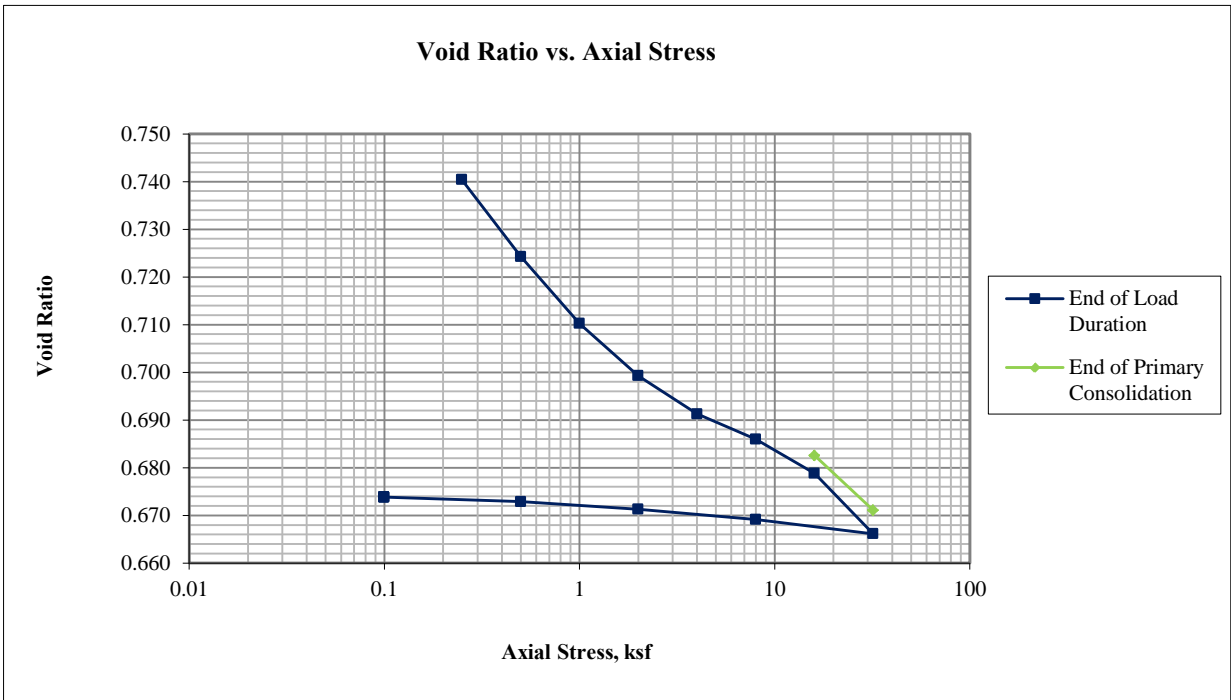
DATE	TIME	LOAD (psi)	DIAL (Left)	DIAL (Right)	AVERAGE	DISPLACEMENT
INITIAL	10:10 AM	0	0.005	0.004	0.005	--
SATURATE	10:25 AM	0	0.021	0.044	0.033	0.028
LOAD	12:40 PM	150	0.775	0.742	0.759	0.754
	8:55 AM	150	0.880	0.844	0.862	0.858
PERM	7:55 AM	150	0.886	0.850	0.868	0.864
END PERM	12:10 PM	150	0.887	0.851	0.869	0.865

**APPENDIX A.3.4
CONSOLIDATION TEST REPORTS**

	Initial		Final	Notes				
Height =	0.994	in	0.960	in	USCS description (ASTM D2487):	Lean clay with sand, yellowish red, moist		
Diameter =	2.499	in	2.499	in	Atterberg Limits (ASTM D4318):	LL = 47	PL = 20	PI = 27
Area =	4.905	in ²	4.905	in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 99%	No. 200 = 81%
Volume =	4.875	in ³	4.709	in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.4%		4.8%		Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.6%		
Height of Solids =	0.5641	in	0.5641	in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.762		0.702		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		18.5%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308	lb	0.282	lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)	
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.3	pcf	103.6	pcf	Final Differential Height:	-0.0158 in		
Dry Unit Weight =	95.5	pcf	98.9	pcf	Estimated Preconsolidation Stress:	Not Computed		

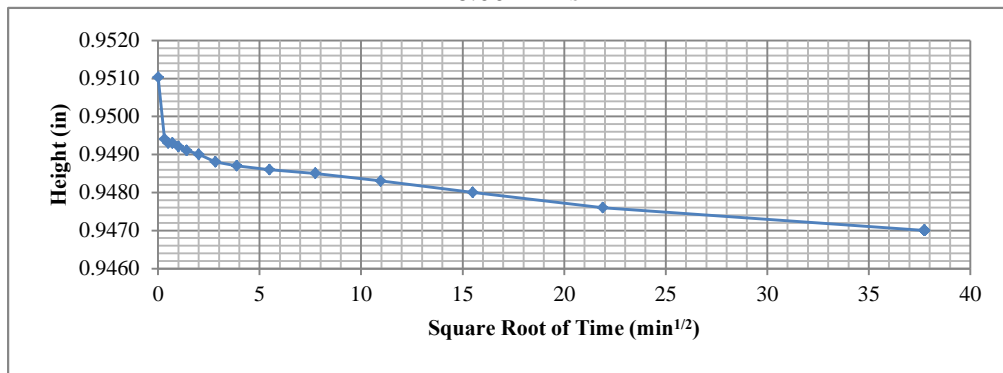
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-10 @ 19-33 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

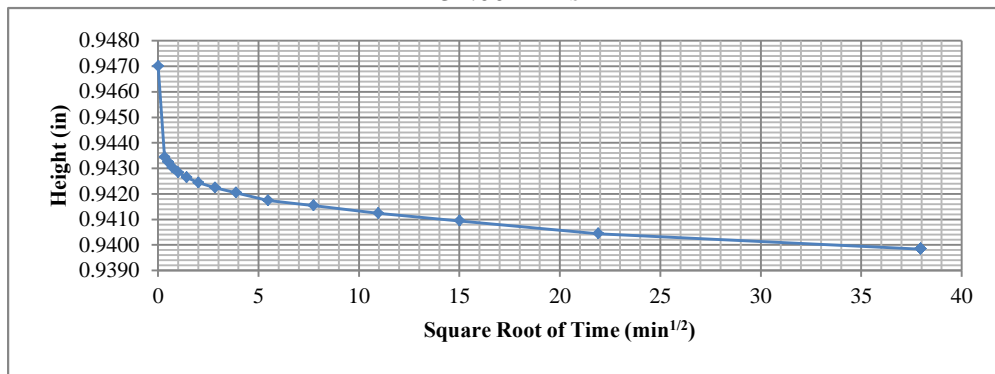


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-10 @ 19-33 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

16.00 ksf



32.00 ksf

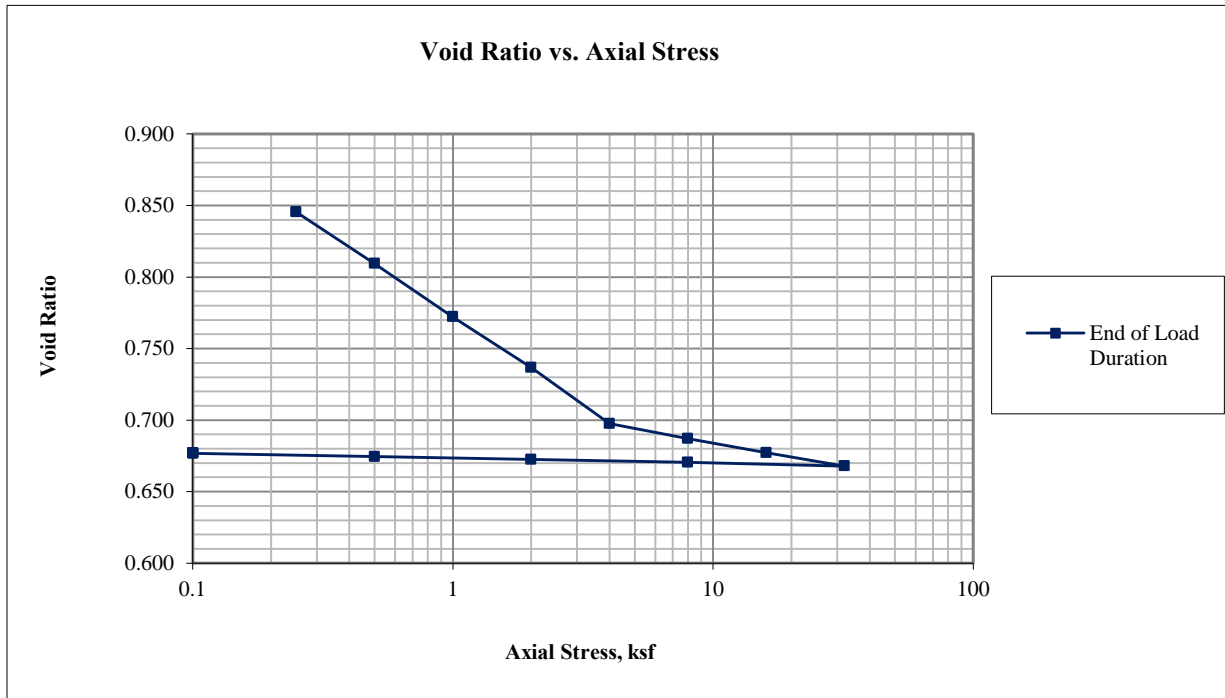


Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-10 @ 19-33 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3	

	Initial		Final	Notes			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in ²		4.893 in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in ³		4.428 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-12 @ 33.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

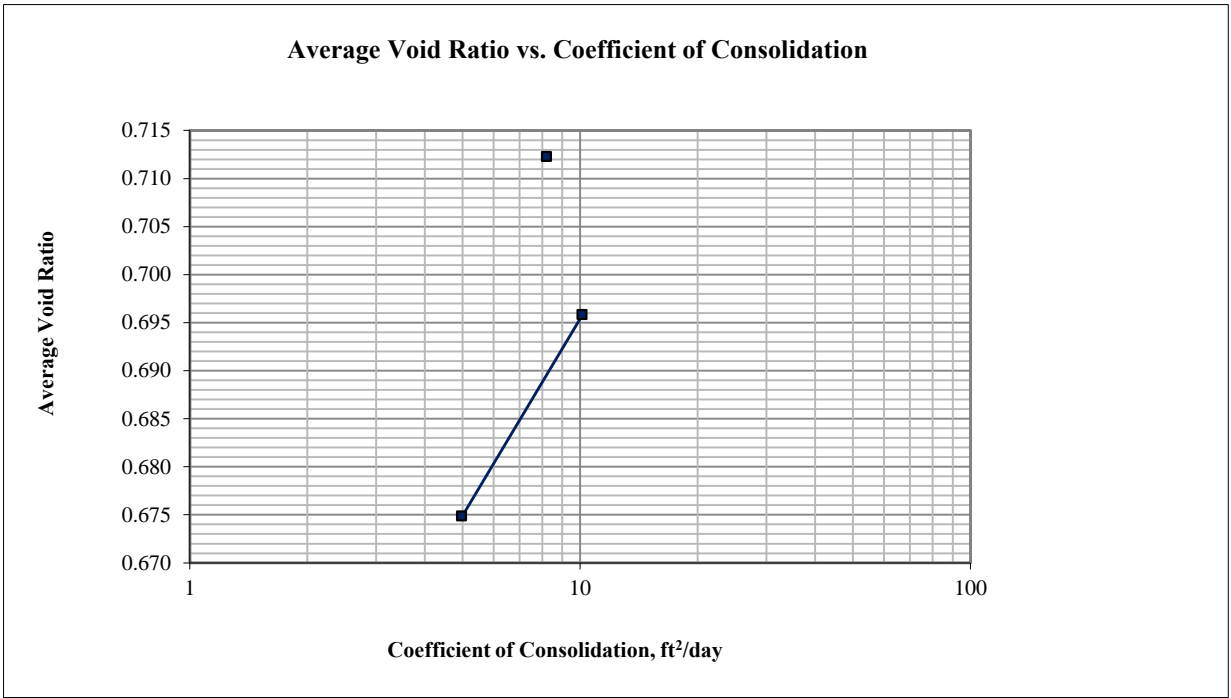
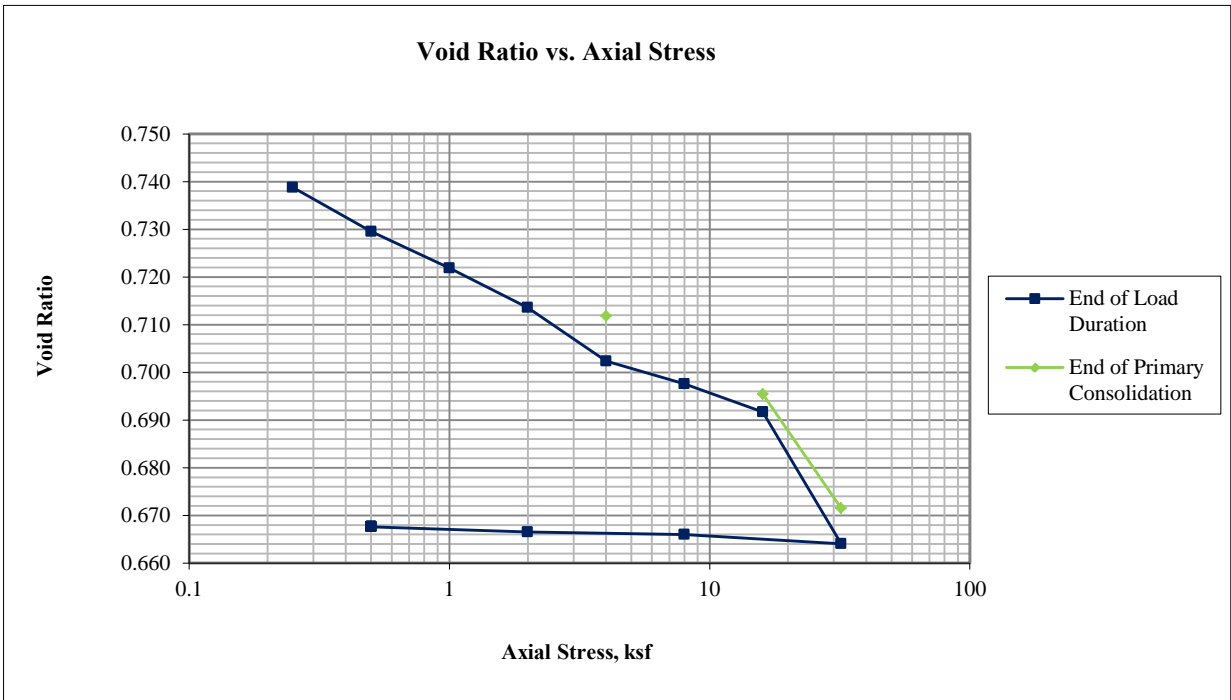


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-12 @ 33.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

	Initial		Final	Notes			
Height =	0.993 in		0.887 in	USCS description (ASTM D2487):	Clayey sand with gravel, yellowish brown, dry		
Diameter =	2.500 in		2.500 in	Atterberg Limits (ASTM D4318):	LL = 23	PL = 14	PI = 9
Area =	4.909 in ²		4.909 in ²	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 83%	No. 200 = 36%
Volume =	4.874 in ³		4.354 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.1%		1.7%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%		
Height of Solids =	0.5636 in		0.5636 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.762		0.574	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	49.8%		8.0%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.307 lb		0.274 lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)	
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	108.9 pcf		108.7 pcf	Final Differential Height:	0.0528 in		
Dry Unit Weight =	95.5 pcf		106.9 pcf	Estimated Preconsolidation Stress:	16.4 ksf		

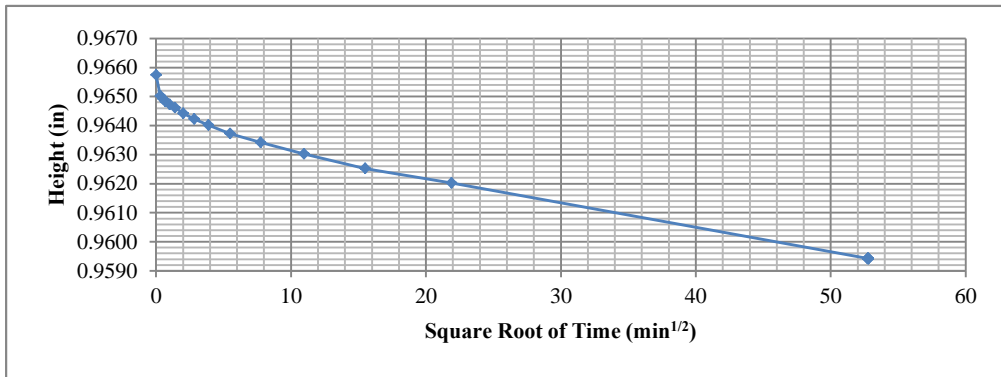
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1

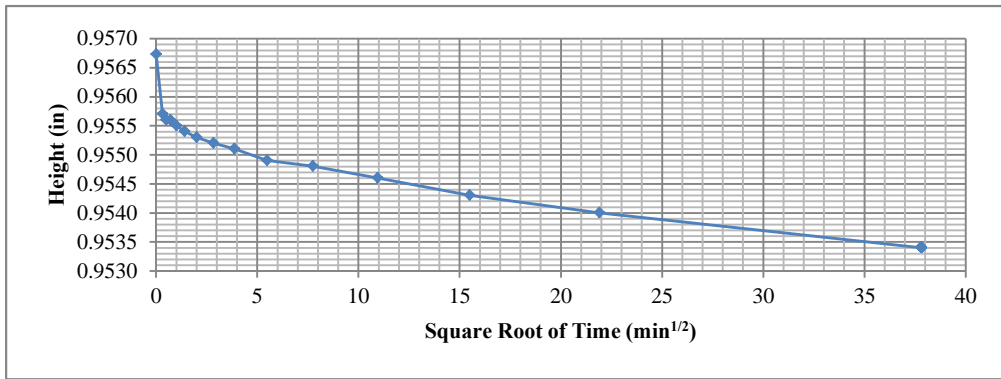


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2

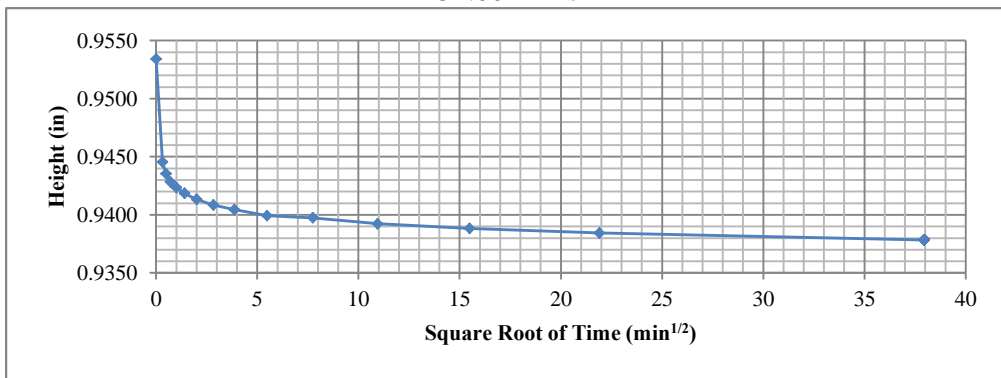
4.00 ksf



16.00 ksf



32.00 ksf



Golder Associates Inc.
Denver, Colorado

Title:

ASTM D2435
ONE-DIMENSIONAL CONSOLIDATION TEST REPORT
TIME-DEFORMATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-16 @ 29-34 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/25/2013

Job Number:

103-92557.006

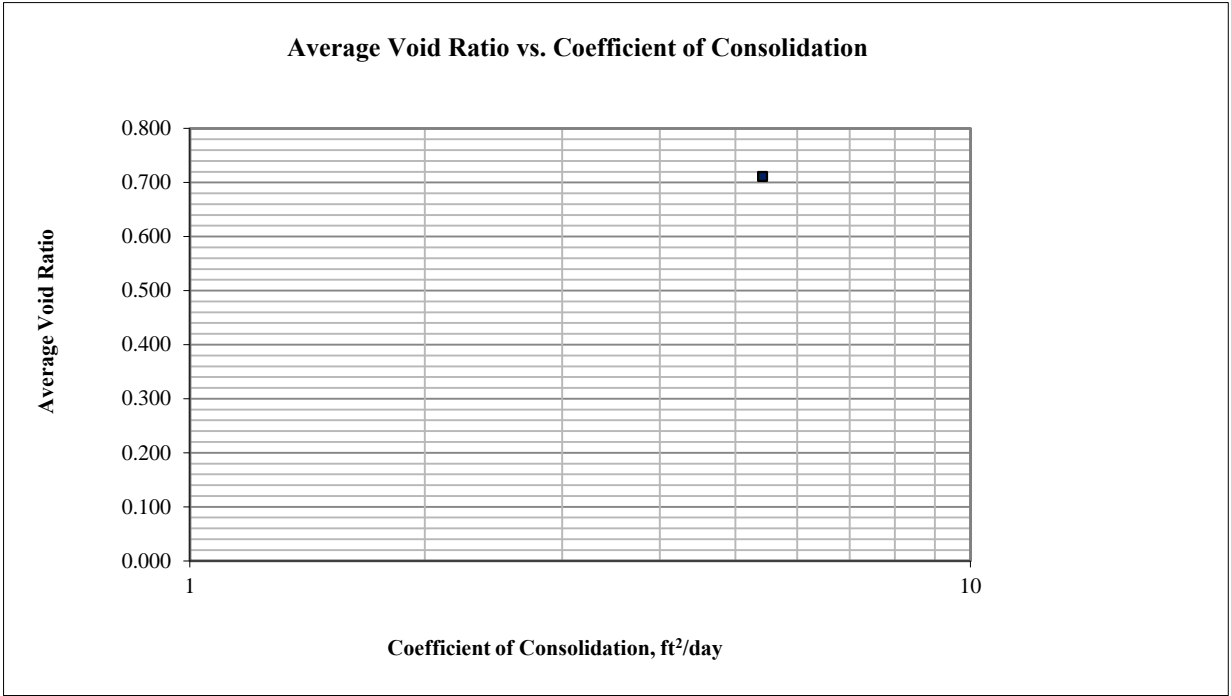
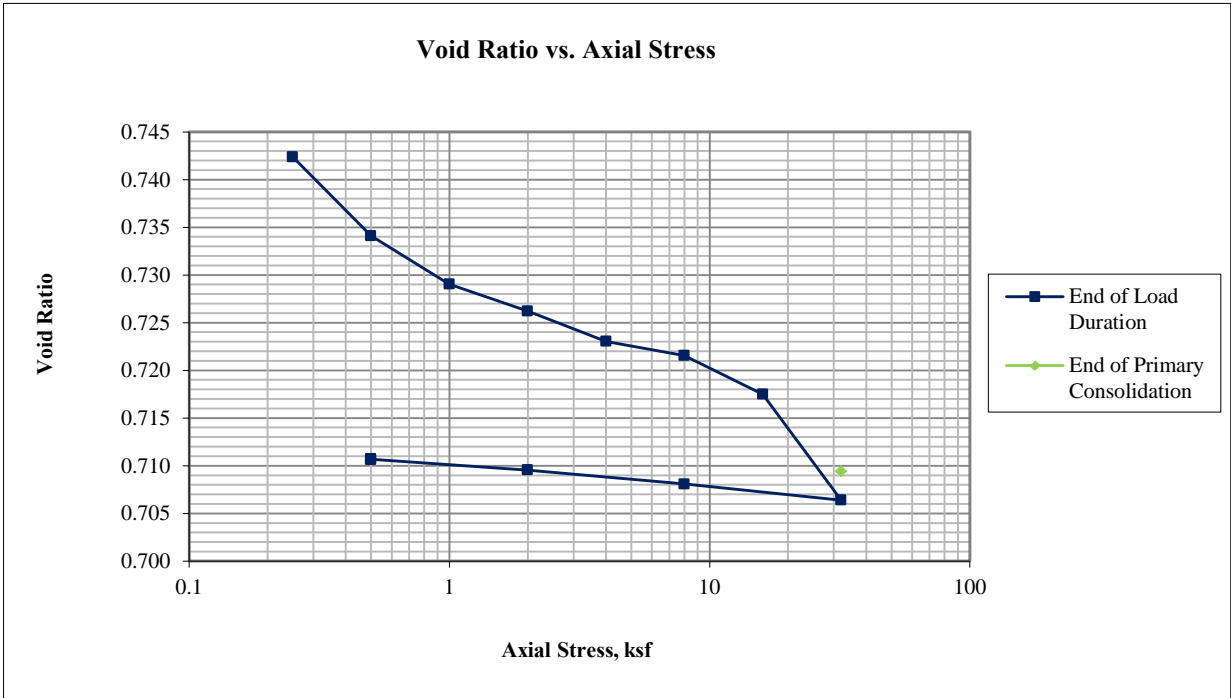
Figure:

3

	Initial		Final	Notes				
Height =	0.997	in	0.960	in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist		
Diameter =	2.498	in	2.498	in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 14	PI = 11
Area =	4.901	in ²	4.901	in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 52%
Volume =	4.886	in ³	4.705	in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.5%		2.7%		Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%		
Height of Solids =	0.5638	in	0.5638	in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.768		0.703		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		10.4%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308	lb	0.276	lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.0	pcf	101.5	pcf	Final Differential Height:	0.0045 in		
Dry Unit Weight =	95.1	pcf	98.8	pcf	Estimated Preconsolidation Stress:	14.7 ksf		

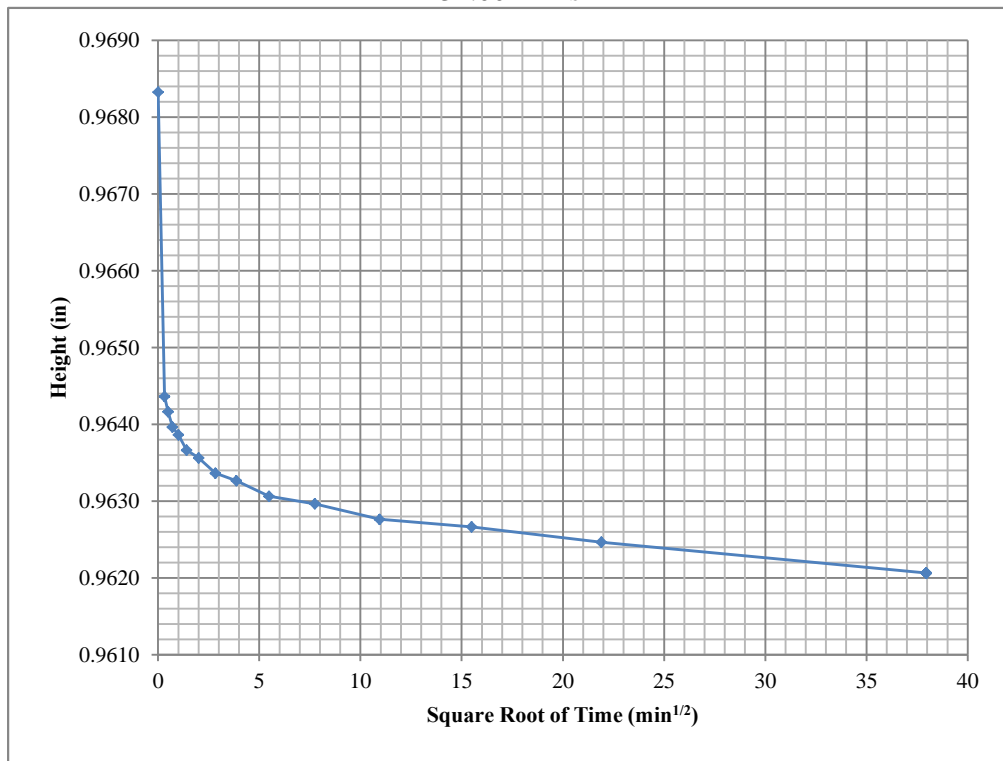
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2

32.00 ksf



Golder Associates Inc.
Denver, Colorado

Title: **ASTM D2435**
ONE-DIMENSIONAL CONSOLIDATION TEST REPORT
TIME-DEFORMATION PLOTS

Job Short Title:
Copper Flat Tailings Design Study

Sample:
BH-18 @ 23-33.5 ft

Technician:
RJM

Reviewed:
CCS

Start Date:
3/25/2013

Job Number:
103-92557.006

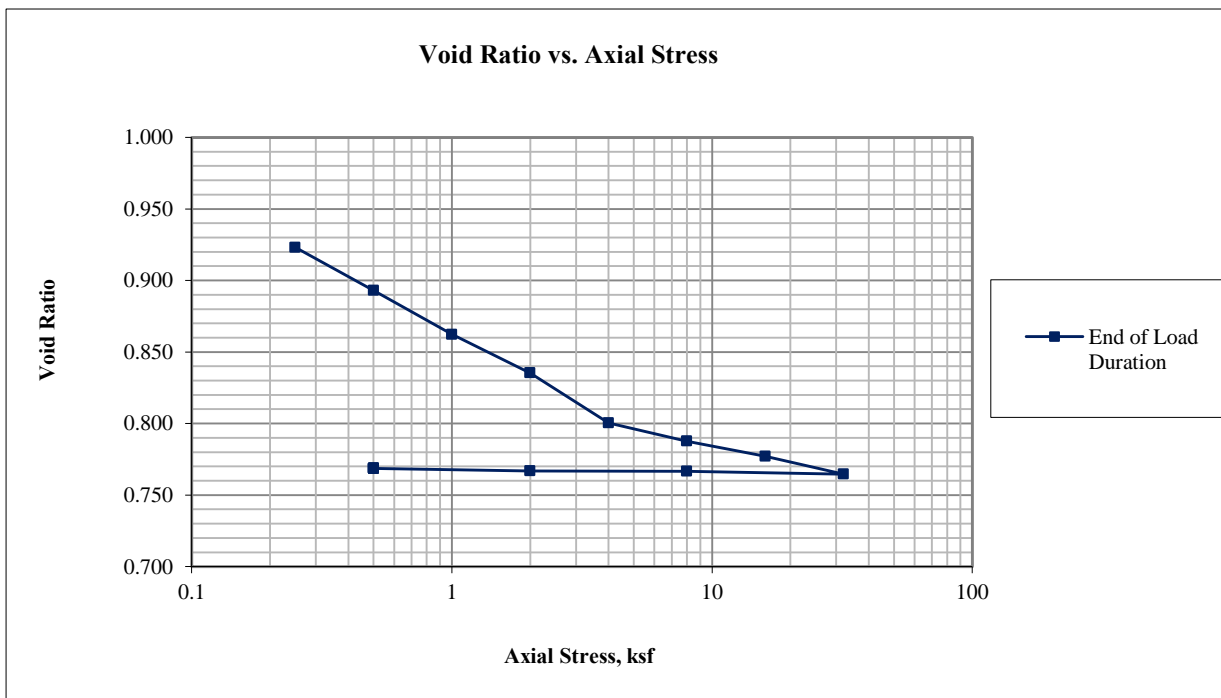
Figure:
3

	Initial		Final	Notes			
Height =	0.994 in		0.924 in	USCS description (ASTM D2487):	Fat clay with sand, dark red, wet		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 62	PL = 20	PI = 42
Area =	4.901 in ²		4.901 in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 82%
Volume =	4.871 in ³		4.528 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	28.9%		9.0%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	29.1%		
Height of Solids =	0.5085 in		0.5085 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.955		0.817	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	81.7%		29.7%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.243 lb		0.243 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.9 pcf		100.9 pcf	Final Differential Height:	-0.0246 in		
Dry Unit Weight =	86.1 pcf		92.6 pcf	Estimated Preconsolidation Stress:	-- ksf		

-- indicates test was not performed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-18 @ 43.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1

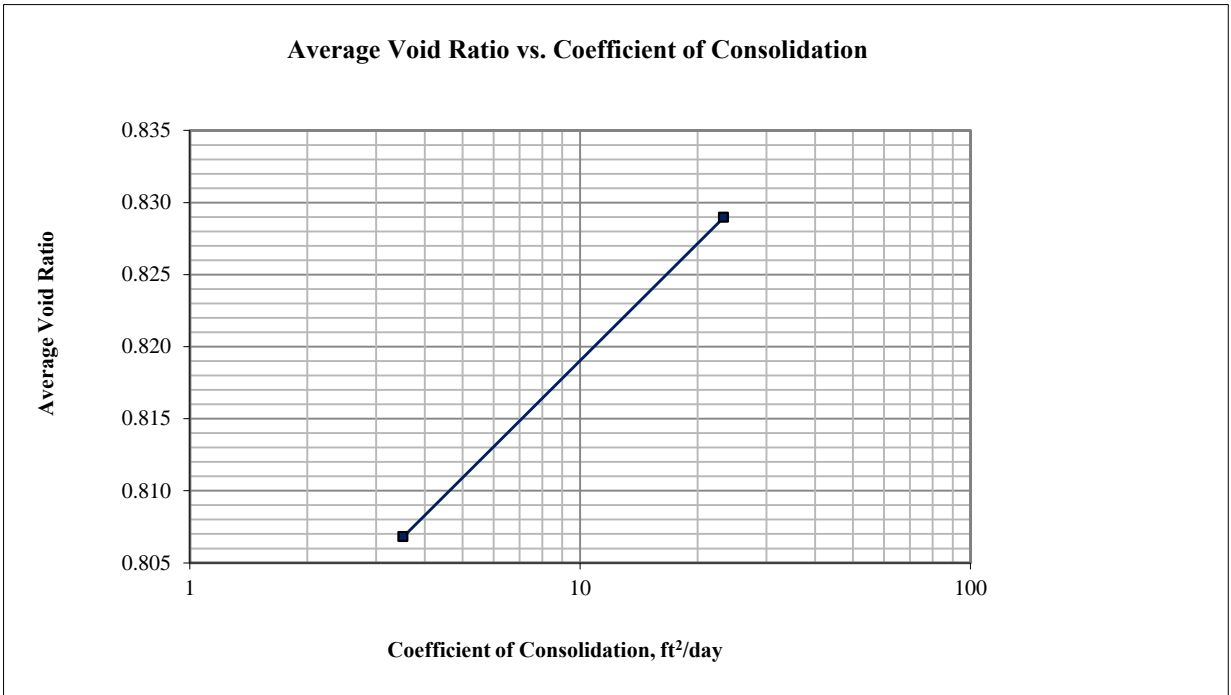
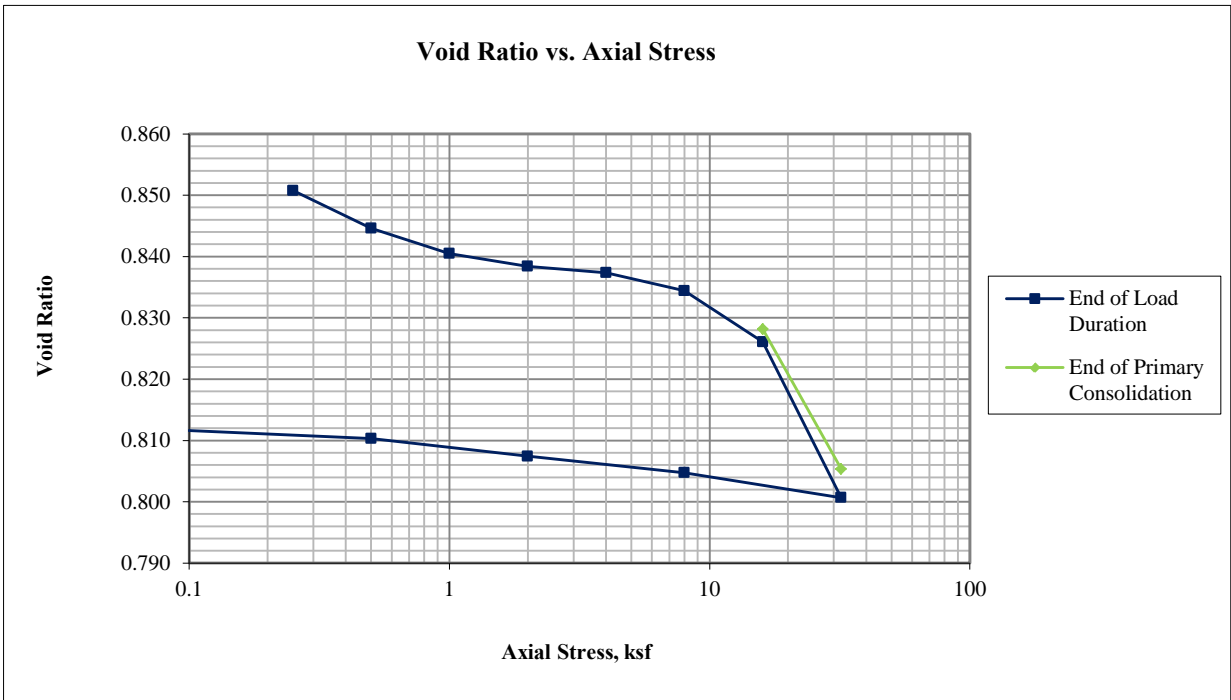


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-18 @ 43.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2

	Initial		Final	Notes			
Height =	1.000 in		0.981 in	USCS description (ASTM D2487):	Sandy silty clay, brownish yellow, moist		
Diameter =	2.497 in		2.497 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 21	PI = 4
Area =	4.897 in ²		4.897 in ²	Percent Finer (ASTM D422):	3/4 in. = 99%	No. 4 = 99%	No. 200 = 52%
Volume =	4.897 in ³		4.804 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	9.7%		1.1%	Remold Targets:	90.0 pcf (dry) at	10.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.4%		
Height of Solids =	0.5363 in		0.5363 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.865		0.829	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	30.3%		3.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.281 lb		0.259 lb	Apparatus:	Frame No. 6	(ELE C-320A)	
Dry Mass =	0.256 lb		0.256 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	99.0 pcf		93.0 pcf	Final Differential Height:	-0.0094 in		
Dry Unit Weight =	90.2 pcf		92.0 pcf	Estimated Preconsolidation Stress:	13.0 ksf		

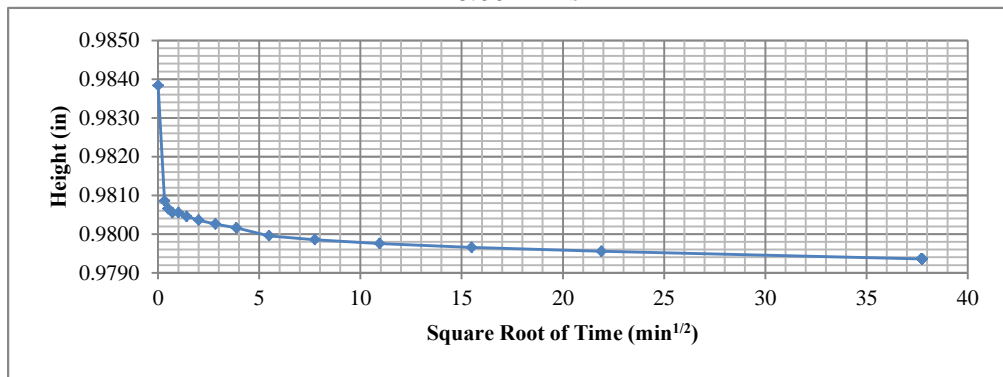
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

Golder Associates Inc. Denver, Colorado			Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
Job Short Title: Copper Flat Tailings Design Study							
Sample: BH-22 @ 0-8.5 ft			Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

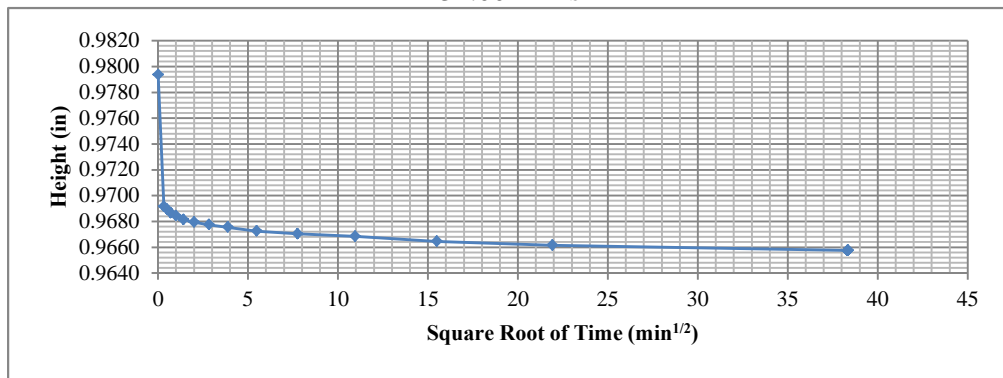


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 0-8.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

16.00 ksf



32.00 ksf

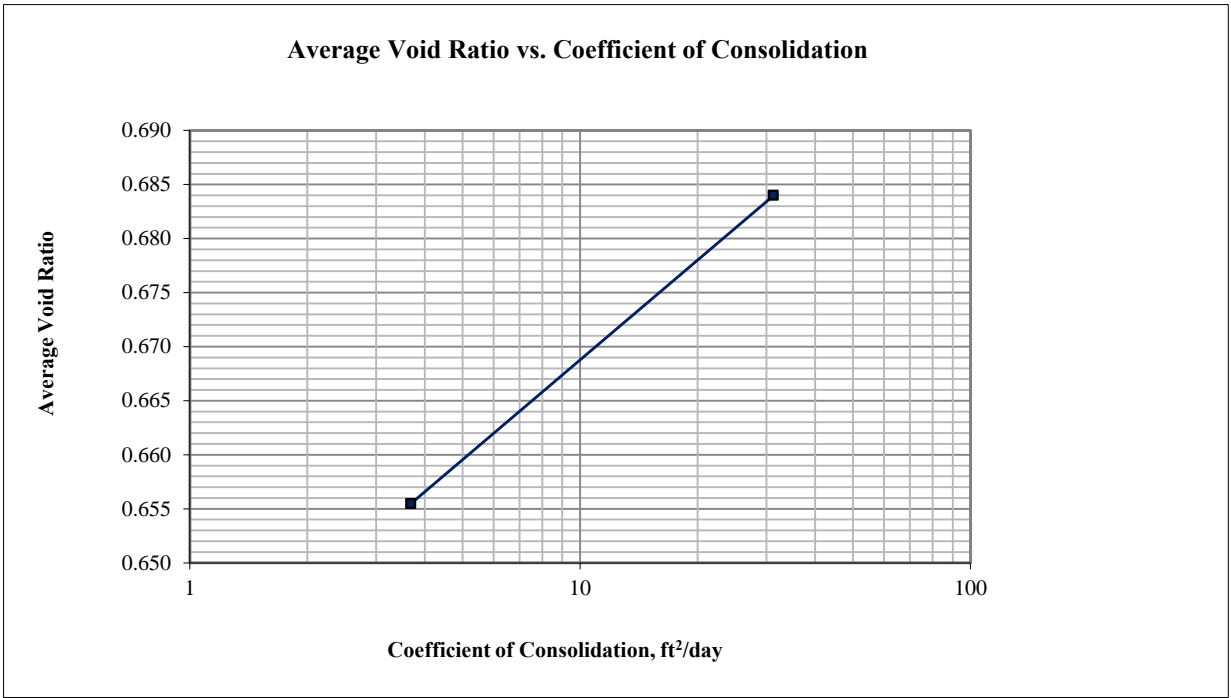
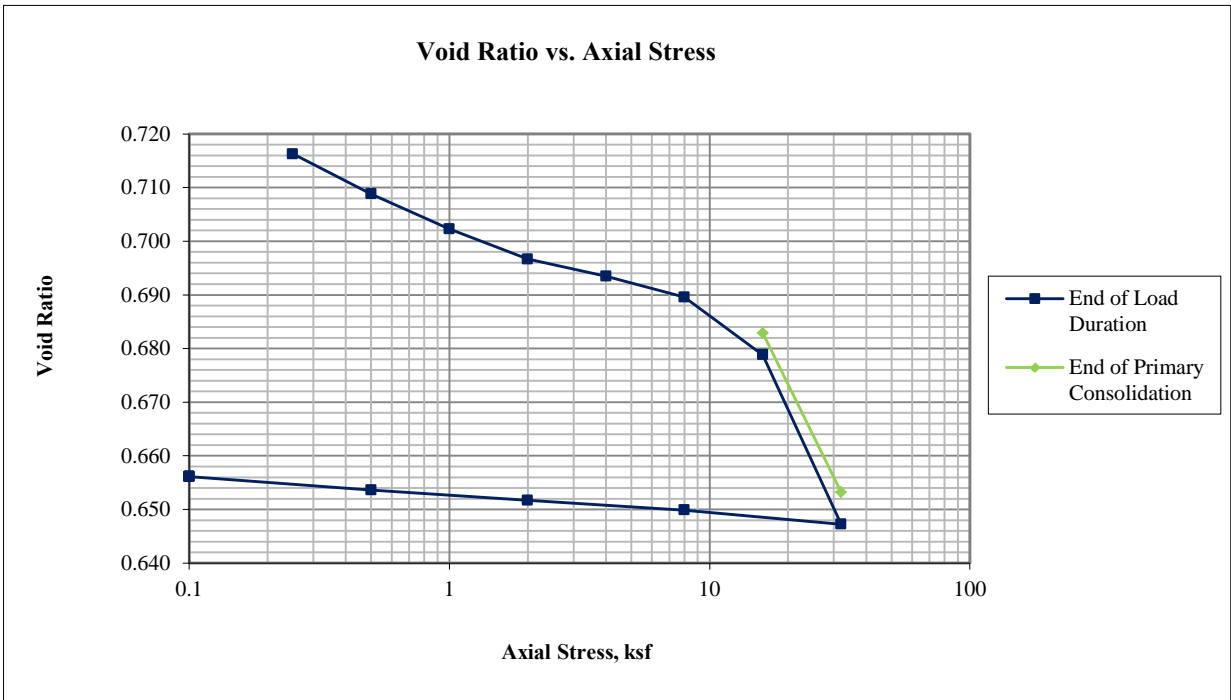


Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-22 @ 0-8.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3	

	Initial		Final	Notes			
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36	PL = 18	PI = 18
Area =	4.901 in ²		4.901 in ²	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 82%	No. 200 = 37%
Volume =	4.886 in ³		4.528 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%		
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in		
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf		

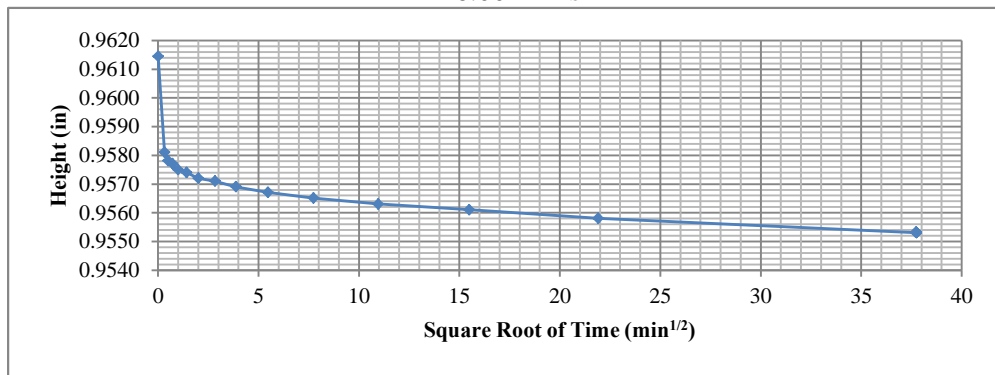
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

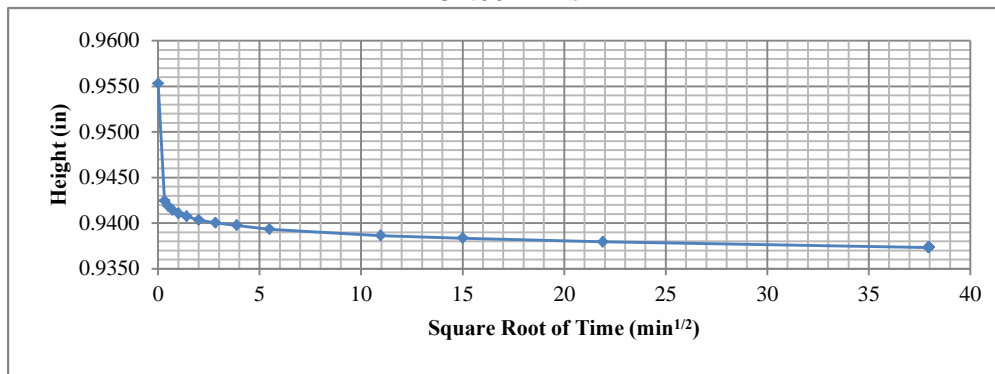


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

16.00 ksf



32.00 ksf



**Golder Associates Inc.
Denver, Colorado**

Title:

ASTM D2435
ONE-DIMENSIONAL CONSOLIDATION TEST REPORT
TIME-DEFORMATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-22 @ 28-30 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/11/2013

Job Number:

103-92557.006

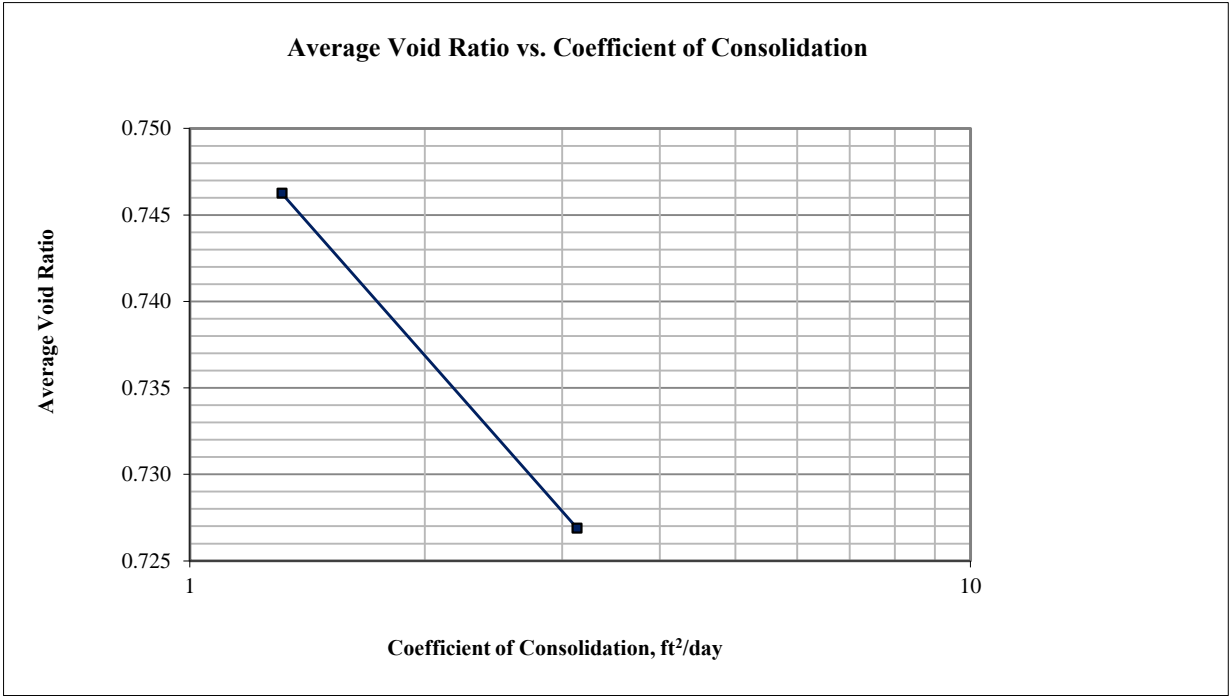
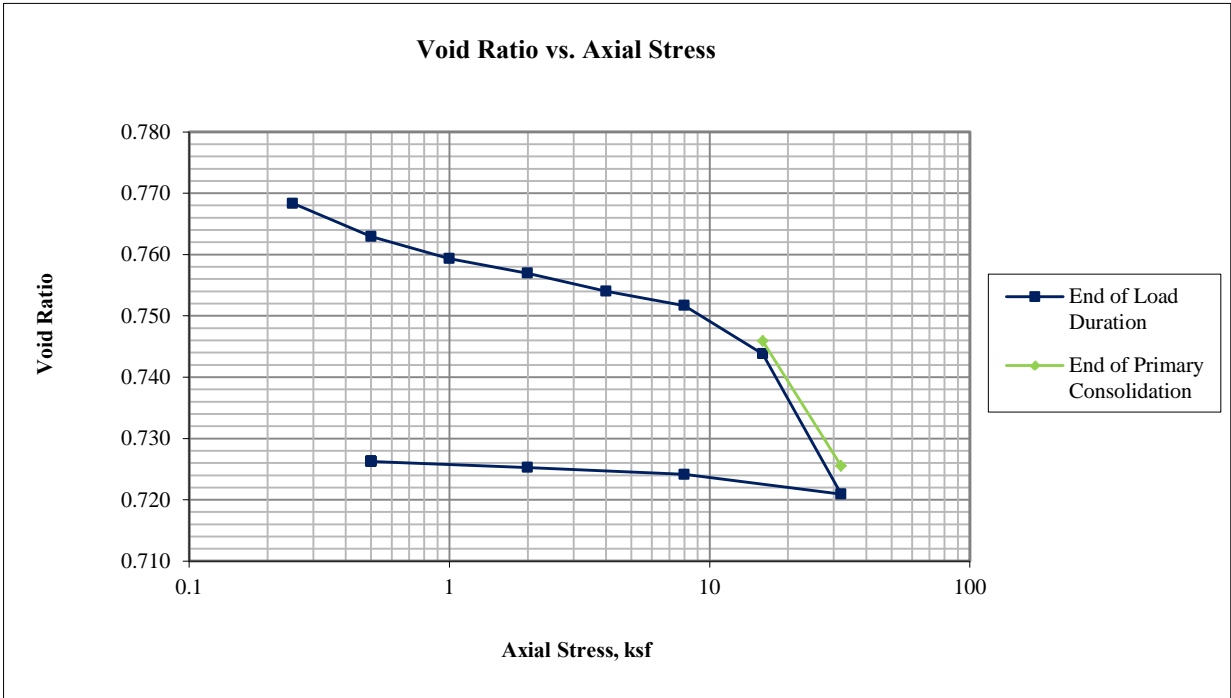
Figure:

3

	Initial		Final	Notes			
Height =	1.000 in		0.973 in	USCS description (ASTM D2487):	Clayey sand, reddish brown, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 37	PL = 17	PI = 20
Area =	4.901 in ²		4.901 in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 39%
Volume =	4.901 in ³		4.769 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	9.8%		4.2%	Remold Targets:	93.0 pcf (dry) at	10.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.9%		
Height of Solids =	0.5558 in		0.5558 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.799		0.751	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	33.0%		15.1%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.291 lb		0.276 lb	Apparatus:	Frame No. 6	(ELE C-320A)	
Dry Mass =	0.265 lb		0.265 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	102.7 pcf		100.1 pcf	Final Differential Height:	-0.0136 in		
Dry Unit Weight =	93.5 pcf		96.1 pcf	Estimated Preconsolidation Stress:	13.8 ksf		

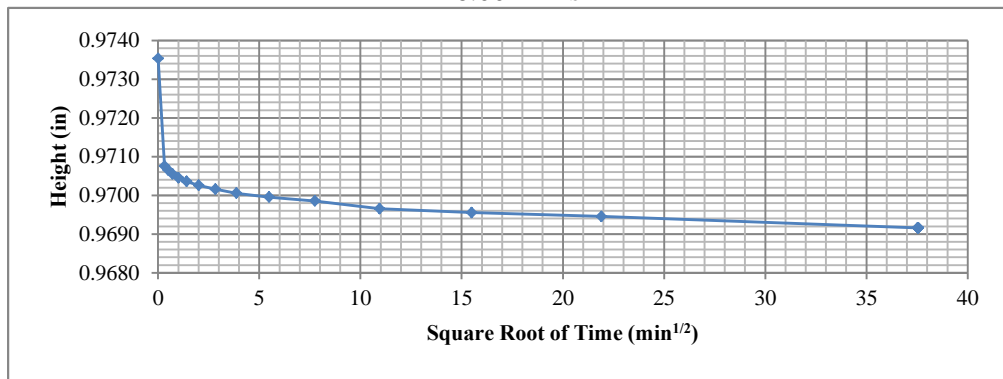
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1

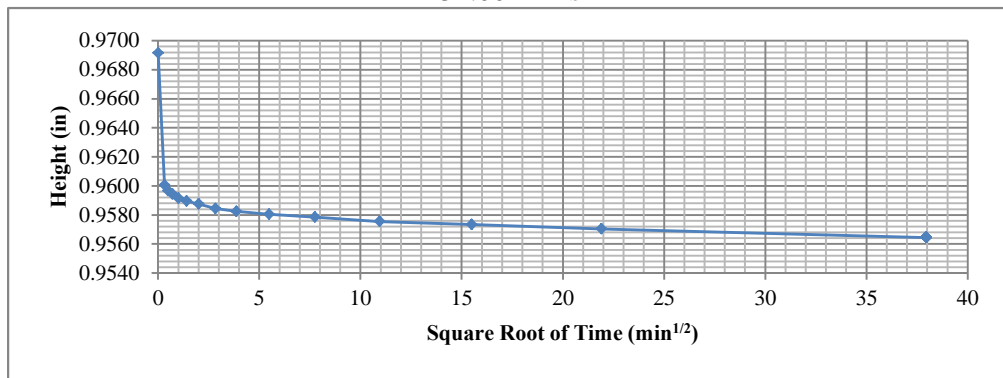


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2

16.00 ksf



32.00 ksf



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 3	

**APPENDIX B
TAILINGS TEST RESULTS**

**APPENDIX B.1
CYCLONE TEST RESULTS,
FULL SCALE CYCLONE PERFORMANCE SIMULATION**

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1222 STPH; 55.5% -200 mesh

U/F = 18.2% -200 mesh; 45.2% recovery

Number, Model Krebs Cyclones: 15 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 13-14 PSI

Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1222.00	669.86	552.15
STPH Liquids	2977.31	2740.68	236.63
STPH Slurry	4199.31	3410.53	788.78
Wt Solids	29.10	19.64	70.00
S.G. Slurry	1.221	1.139	1.773
Vol% Solids	13.41	8.44	46.82
GPM Slurry	13734.13	11956.71	1777.42
M3/Hr. Slurry	3119.34	2715.64	403.69

Ref: 72.3 4.5 38.9*

Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	22.0	0.00	0.00	0.0	3.98	3.98	22.0	100.0
100	149.0	18.50	16.70	204.1	0.23	0.23	1.6	40.66	36.68	202.5	99.2
150	104.0	30.60	12.10	147.9	2.69	2.46	16.5	64.46	23.79	131.4	88.9
200	74.0	44.50	13.90	169.9	13.74	11.05	74.0	81.81	17.36	95.8	56.4
270	53.0	56.40	11.90	145.4	29.20	15.46	103.5	89.40	7.59	41.9	28.8
325	45.0	61.10	4.70	57.4	35.93	6.73	45.0	91.64	2.24	12.4	21.6
400	37.0	63.70	2.60	31.8	39.89	3.97	26.6	92.58	0.94	5.2	16.4
-400	-37.0	100.00	36.30	443.6	100.00	60.11	402.6	100.00	7.42	40.9	9.2
TOTAL				1222.00			669.86			552.15	45.2

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1333 STPH; 55.5% -200 mesh

U/F = 18.4% -200 mesh; 45.6% recovery

Number, Model Krebs Cyclones: 16 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 14 PSI
 Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1333.00	724.53	608.47
STPH Liquids	3247.76	2986.98	260.77
STPH Slurry	4580.76	3711.51	869.24
Wt Solids	29.10	19.52	70.00
S.G. Slurry	1.221	1.138	1.773
Vol% Solids	13.41	8.39	46.82
GPM Slurry	14981.66	13022.93	1958.73
M3/Hr. Slurry	3402.68	2957.81	444.87

Ref: 71.4 4.5 38.9*

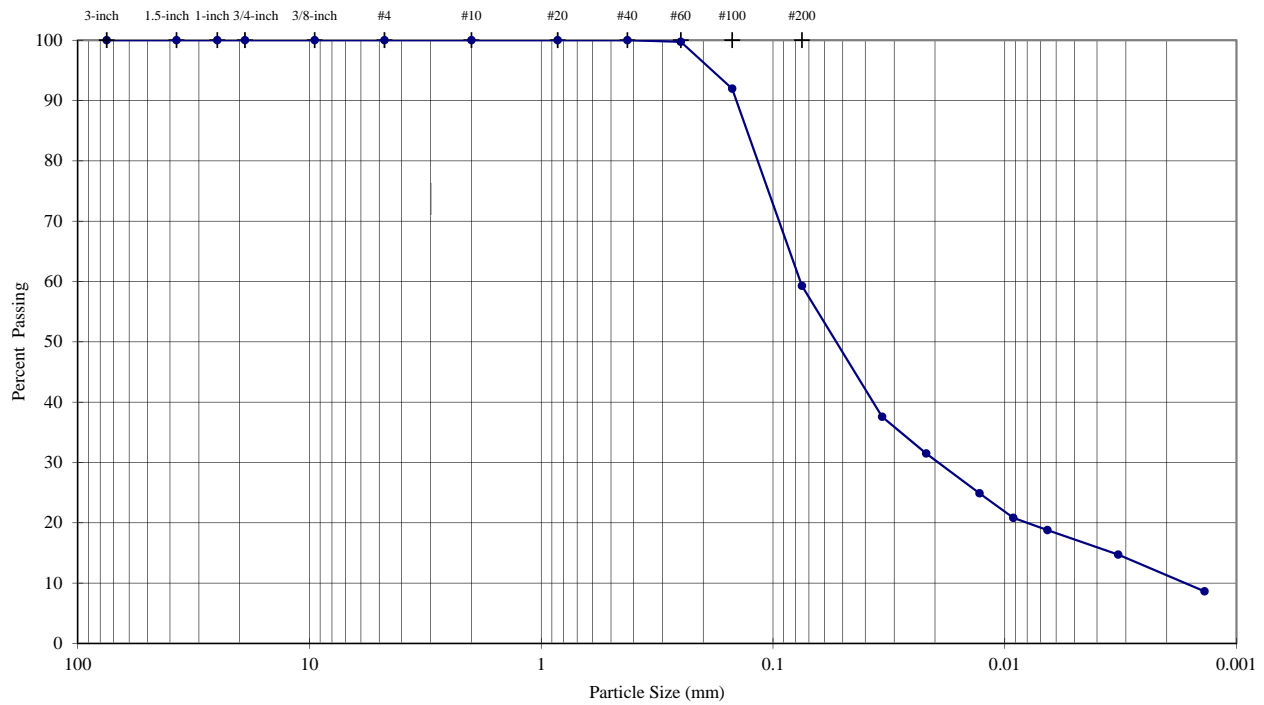
Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	24.0	0.00	0.00	0.0	3.94	3.94	24.0	100.0
100	149.0	18.50	16.70	222.6	0.21	0.21	1.5	40.28	36.34	221.1	99.3
150	104.0	30.60	12.10	161.3	2.52	2.31	16.7	64.04	23.76	144.6	89.6
200	74.0	44.50	13.90	185.3	13.31	10.79	78.2	81.64	17.60	107.1	57.8
270	53.0	56.40	11.90	158.6	28.73	15.42	111.7	89.35	7.71	46.9	29.6
325	45.0	61.10	4.70	62.7	35.47	6.74	48.8	91.62	2.27	13.8	22.1
400	37.0	63.70	2.60	34.7	39.45	3.98	28.9	92.58	0.95	5.8	16.7
-400	-37.0	100.00	36.30	483.9	100.00	60.55	438.7	100.00	7.42	45.2	9.3
TOTAL				1333.00			724.53			608.47	45.6

**APPENDIX B.2
TAILINGS GRADATIONS**

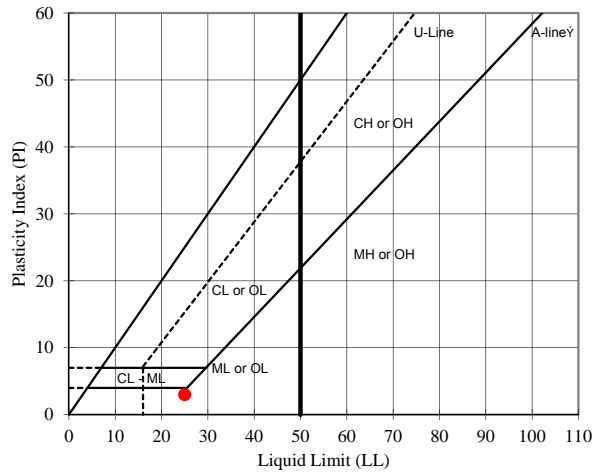
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Whole Tailings Drum**
 TYPE: **Drum**

DEPTH (ft): --



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	Fine Gravel
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Coarse Sand
		#10	2.0	100.0	
		#20	0.85	100.0	Medium Sand
		#40	0.425	100.0	
		#60	0.25	99.7	Fine Sand
	#100	0.15	92.0		
	#200	0.075	59.3		
		0.034	37.6		
Hydrometer Analysis			0.022	31.5	Silt or Clay Fines
			0.013	24.9	
			0.009	20.8	
			0.007	18.8	
			0.003	14.7	
		0.001	8.6		



USCS Description (ASTM D 2487):

Dry, yellow sandy silt

LL	PL	PI	SpG
25	22	3	2.64

As-Received Moisture Content (%)

--

USCS Group Symbol

ML

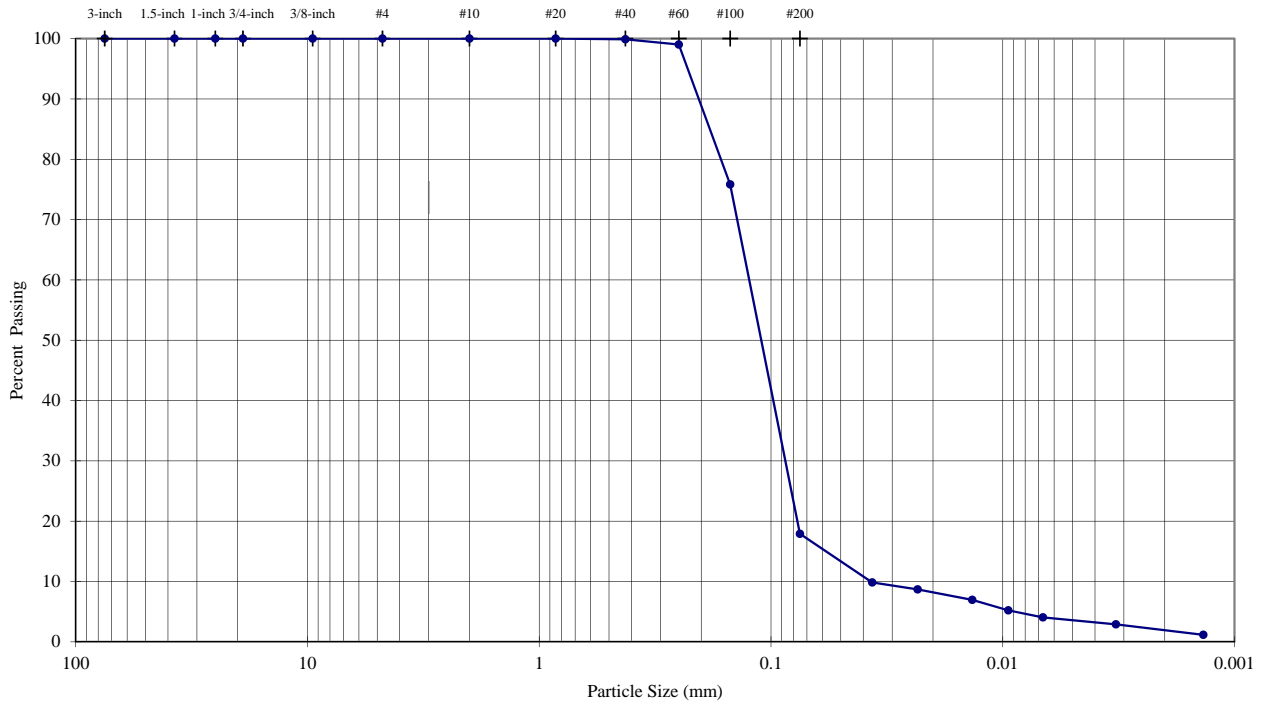
Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	11/13/2012
REVIEW	MB

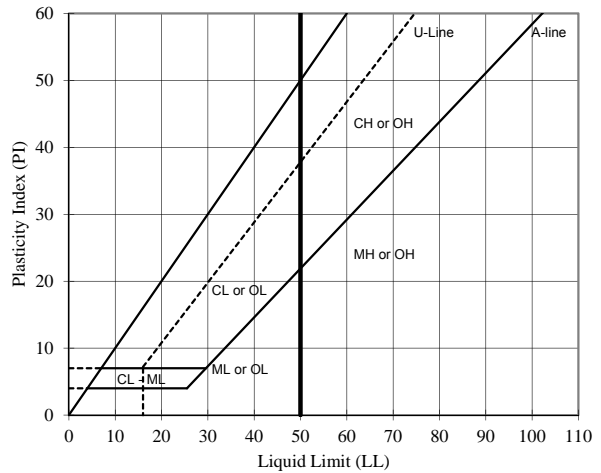
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Tailings Underflow**
 TYPE: **Pail**

DEPTH (ft): --



		Particle Size				
		Sieve	(mm)	% Passing		
Sieve Analysis (Initial Separation on No. 4 Sieve)	3-inch	75.0	100.0	Coarse Gravel	0.00	
	1.5-inch	37.5	100.0			
	1-inch	25.0	100.0			
	3/4-inch	19.0	100.0			
	3/8-inch	9.5	100.0	Fine Gravel	0.00	
	#4	4.75	100.0	Coarse Sand	0.00	
	#10	2.0	100.0			
	#20	0.85	100.0			
	Hydrometer Analysis	#40	0.425	99.9	Medium Sand	0.12
		#60	0.25	99.0		
#100		0.15	75.8	Fine Sand	81.98	
#200		0.075	17.9			
		0.0375	9.8			
		0.023	8.7			
		0.014	6.9			
		0.009	5.2			
	0.0075	4.1				
	0.003	2.9				
	0.001	1.2	Silt or Clay Fines	17.91		



USCS Description (ASTM D 2487):

Wet, light, yellowish brown silty sand

LL	PL	PI	Spg (assumed)
NP	NP	NP	2.7

As-Received Moisture Content (%)

--

USCS Group Symbol

SM

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	MC/SRS
DATE	10/24/2012
REVIEW	MB

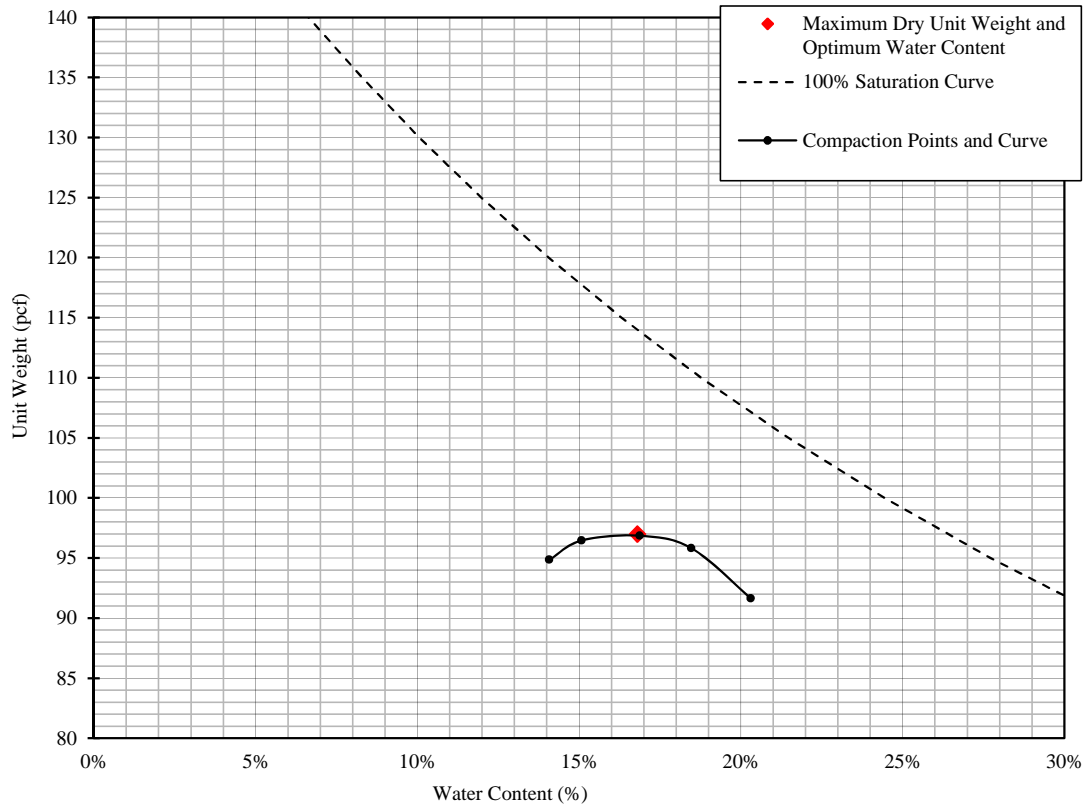
LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer

Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**
SAMPLE ID: **Tailings Underflow**
TYPE: **Pail**

DEPTH (ft): --



% Test Fraction Passing #4 Sieve	100%
As-Received Moisture Content	NA
Specific Gravity (ASTM D854)	2.64

Maximum Dry Unit Weight (pcf)	97.0
Optimum Water Content (%)	16.8

USCS Description (ASTM D 2487): Wet, light, yellowish brown silty sand

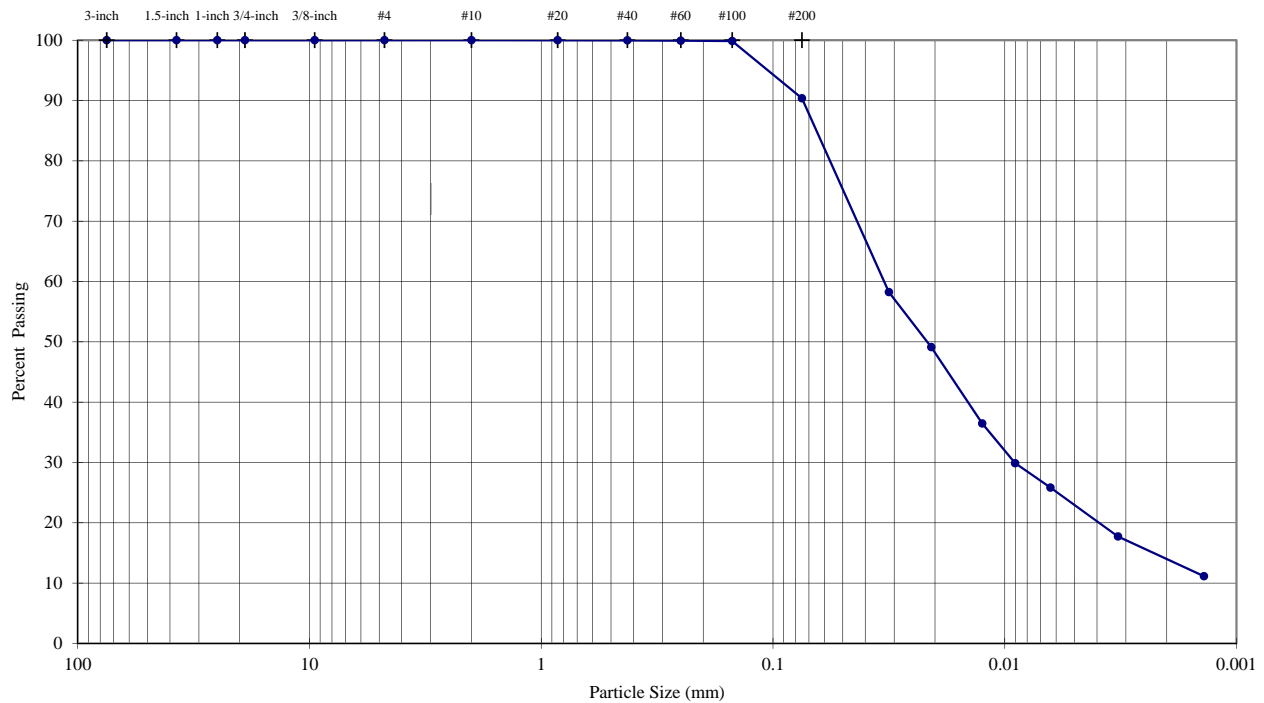
USCS SM

TECH	ACE/MC
DATE	10-25-12
REVIEW	MB

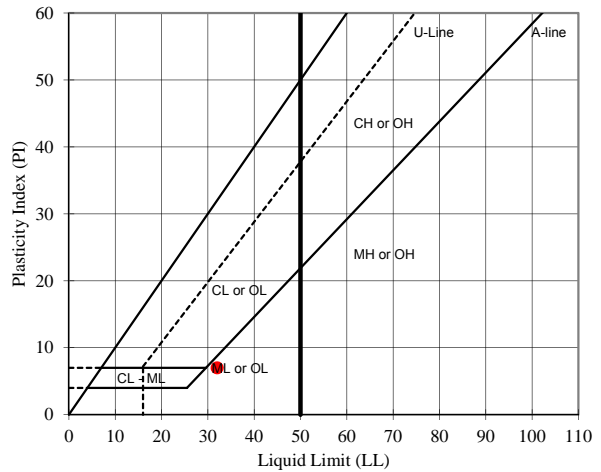
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Tailings Overflow**
 TYPE: **Drum**

DEPTH (ft): --



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	Fine Gravel
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Coarse Sand
		#10	2.0	100.0	
		#20	0.85	100.0	
		#40	0.425	100.0	Medium Sand
		#60	0.25	99.9	
	#100	0.15	99.9	Fine Sand	
	#200	0.075	90.4		
		0.032	58.2		
Hydrometer Analysis			0.021	49.1	Silt or Clay Fines
			0.013	36.5	
			0.009	29.9	
			0.006	25.8	
			0.003	17.7	
			0.001	11.1	



USCS Description (ASTM D 2487):

Dry, olive yellow SILT

LL	PL	PI	SpG (assumed)
32	25	7	2.64

As-Received Moisture Content (%)

--

USCS Group Symbol

ML

Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	1/2/2013
REVIEW	MB

**APPENDIX B.3
CYCLONE UNDERFLOW TEST RESULTS**

Boring or Test Pit: --
 Sample: Tailings Underflow
 Depth: -- ft
 Point No.: 1

Boring or Test Pit: --
 Sample: Tailings Underflow
 Depth: -- ft
 Point No.: 2

Boring or Test Pit: --
 Sample: Tailings Underflow
 Depth: -- ft
 Point No.: 3

Initial
 Length = 5.006 in
 Diameter = 2.500 in
 Wet Mass = 1.526 lb
 Area = 4.909 in²
 Volume = 24.573 in³
 Specific Gravity = 2.64 (Assumed)
 Dry Mass of Solids = 1.311 lb
 Moisture Content = 16.4%
 Wet Unit Weight = 107.3 pcf
 Dry Unit Weight = 92.2 pcf
 Void Ratio = 0.79
 Percent Saturation = 55%

Initial
 Length = 5.006 in
 Diameter = 2.500 in
 Wet Mass = 1.523 lb
 Area = 4.909 in²
 Volume = 24.573 in³
 Specific Gravity = 2.64 (Assumed)
 Dry Mass of Solids = 1.312 lb
 Moisture Content = 16.1%
 Wet Unit Weight = 107.1 pcf
 Dry Unit Weight = 92.3 pcf
 Void Ratio = 0.78
 Percent Saturation = 54%

Initial
 Length = 5.006 in
 Diameter = 2.500 in
 Wet Mass = 1.517 lb
 Area = 4.909 in²
 Volume = 24.573 in³
 Specific Gravity = 2.64 (Assumed)
 Dry Mass of Solids = 1.306 lb
 Moisture Content = 16.1%
 Wet Unit Weight = 106.7 pcf
 Dry Unit Weight = 91.9 pcf
 Void Ratio = 0.79
 Percent Saturation = 54%

After Consolidation
 Length = 4.989 in
 Diameter = 2.470 in
 Area = 4.792 in² (Method B)
 Volume = 23.906 in³
 Moisture Content = 27.9%
 Wet Unit Weight = 121.2 pcf
 Dry Unit Weight = 94.7 pcf
 Void Ratio = 0.74
 Percent Saturation = 100%

After Consolidation
 Length = 4.977 in
 Diameter = 2.469 in
 Area = 4.787 in² (Method B)
 Volume = 23.824 in³
 Moisture Content = 27.6%
 Wet Unit Weight = 121.4 pcf
 Dry Unit Weight = 95.2 pcf
 Void Ratio = 0.73
 Percent Saturation = 100%

After Consolidation
 Length = 4.971 in
 Diameter = 2.429 in
 Area = 4.633 in² (Method B)
 Volume = 23.033 in³
 Moisture Content = 25.7%
 Wet Unit Weight = 123.2 pcf
 Dry Unit Weight = 98.0 pcf
 Void Ratio = 0.68
 Percent Saturation = 100%

B Parameter = 0.98
 Shear Rate = 0.083% /min.
 t₅₀ = -- (not computed)
 Strain at Failure = 5.0%

B Parameter = 0.97
 Shear Rate = 0.083% /min.
 t₅₀ = -- (not computed)
 Strain at Failure = 5.0%

B Parameter = 0.96
 Shear Rate = 0.082% /min.
 t₅₀ = -- (not computed)
 Strain at Failure = 5.0%

Cell Pressure = 120 psi
 Back Pressure = 100 psi
 Confining Pressure = 20 psi

Cell Pressure = 150 psi
 Back Pressure = 100 psi
 Confining Pressure = 50 psi

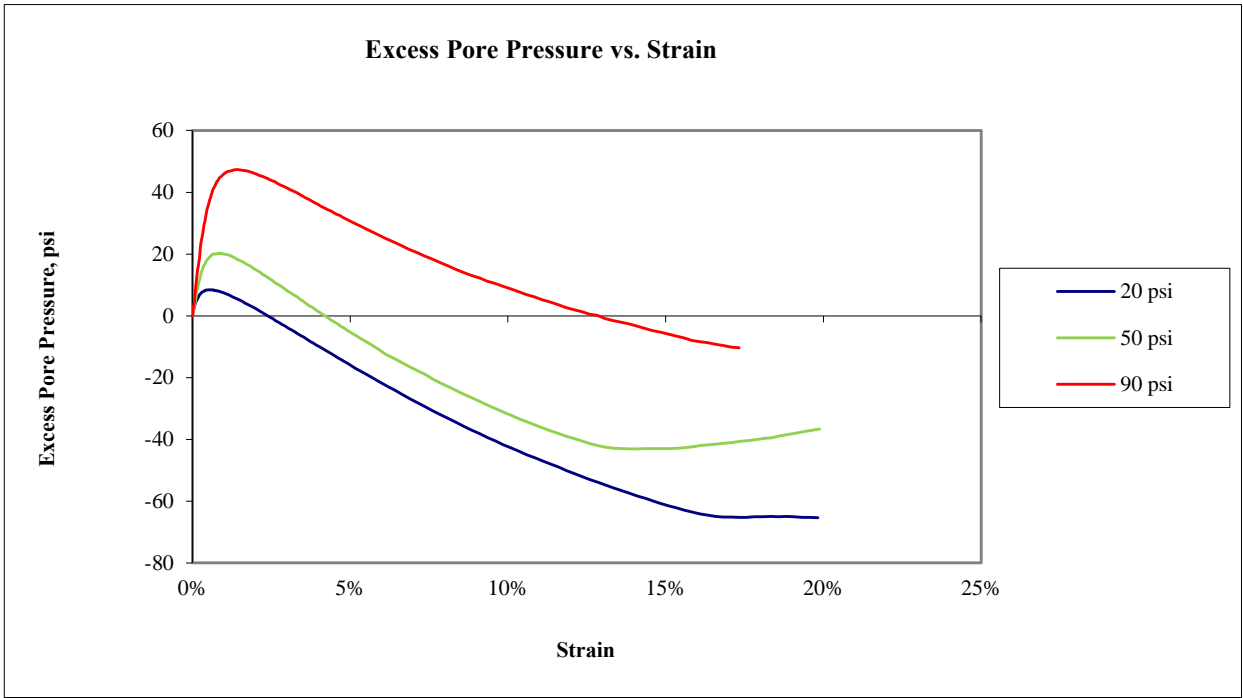
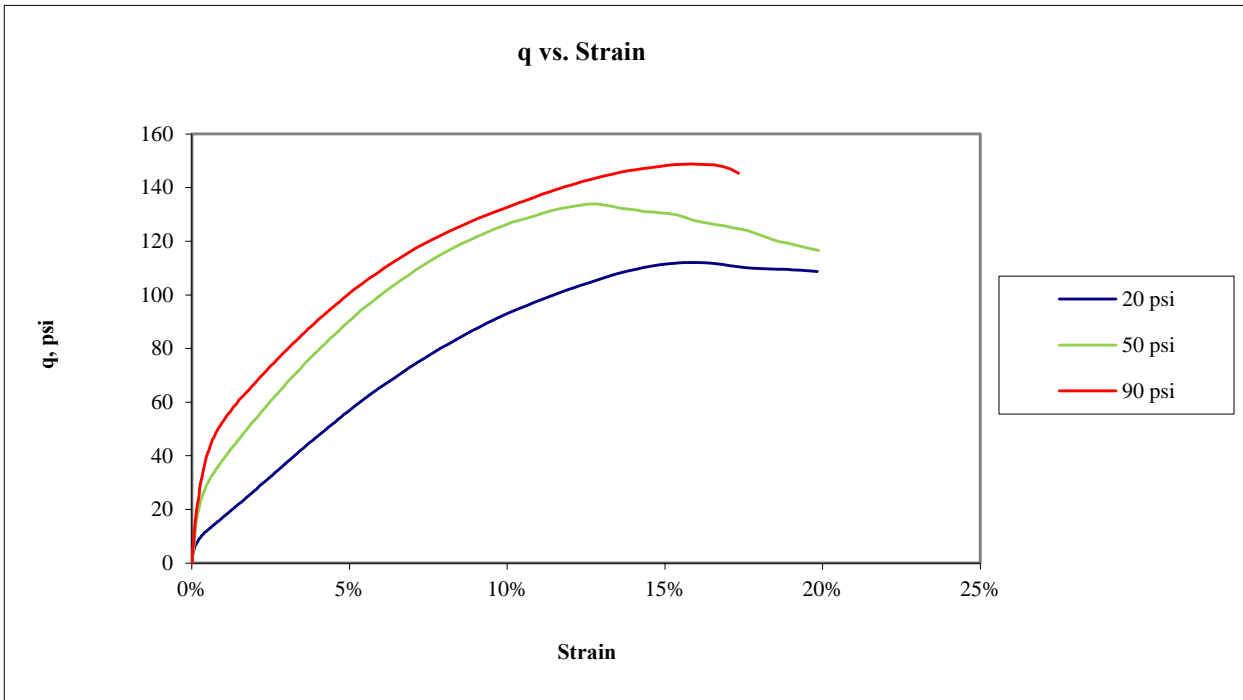
Cell Pressure = 170 psi
 Back Pressure = 80 psi
 Confining Pressure = 90 psi

Notes: Sample description: Light yellowish brown silty sand
 Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 18% (ASTM D422, refer to separate report for gradation curve)
 Specimen type:

Intact	X
Cuttings	X
Wet	
(σ ₁ /σ ₃) _{max}	
Corrected	

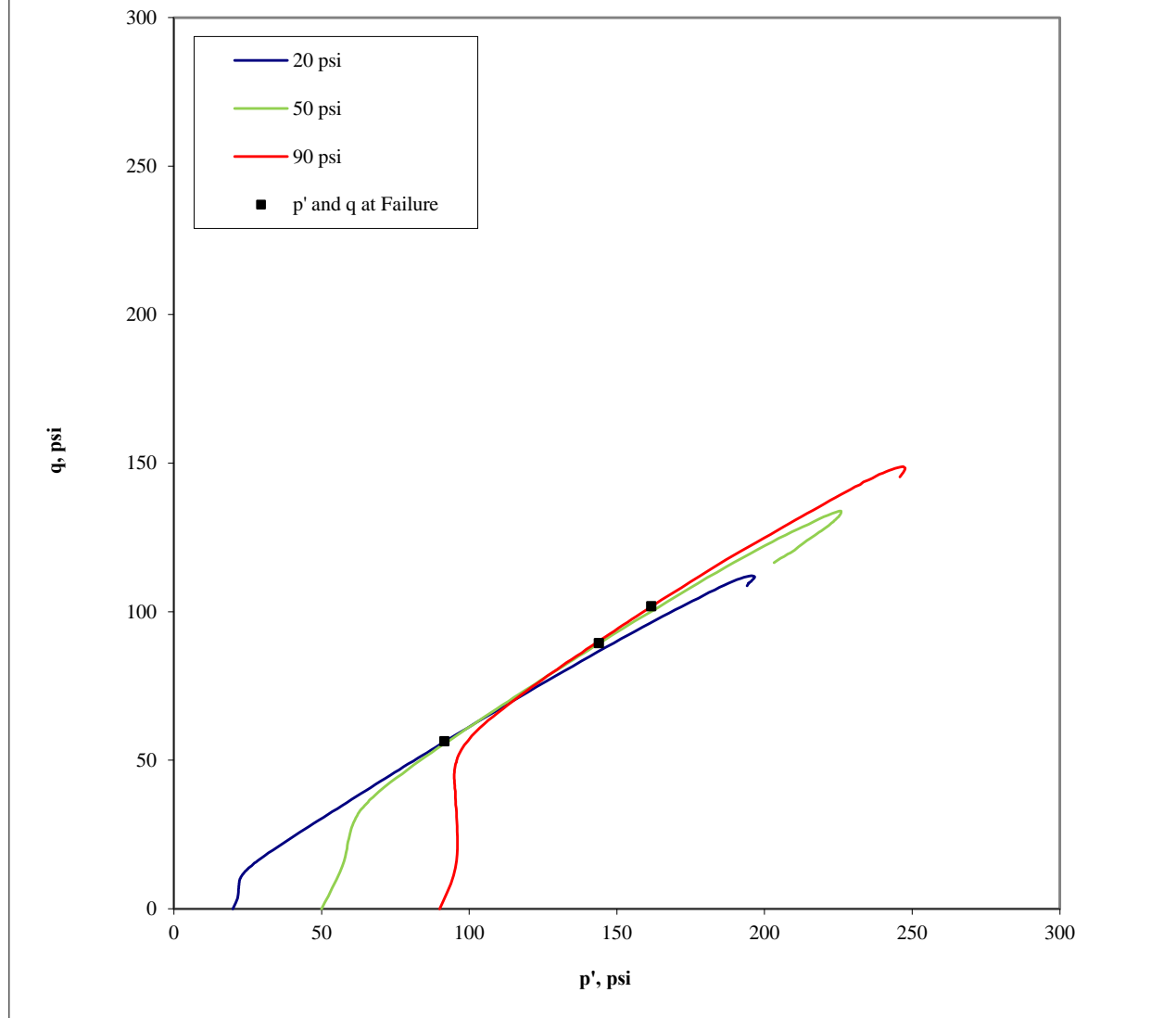
 Reconstituted Remold targets: 92.2 pcf (dry) at 16.8% moisture (+/- 2%)
 Moisture from: Entire specimen
 Saturation method: X Wet Dry
 Failure criterion: (σ₁-σ₃)_{max} 5 % strain
 Membrane effect: X Corrected Not Corrected

Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013	Job Number: 103-92557	Figure: 1



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013	Job Number: 103-92557	Figure: 2

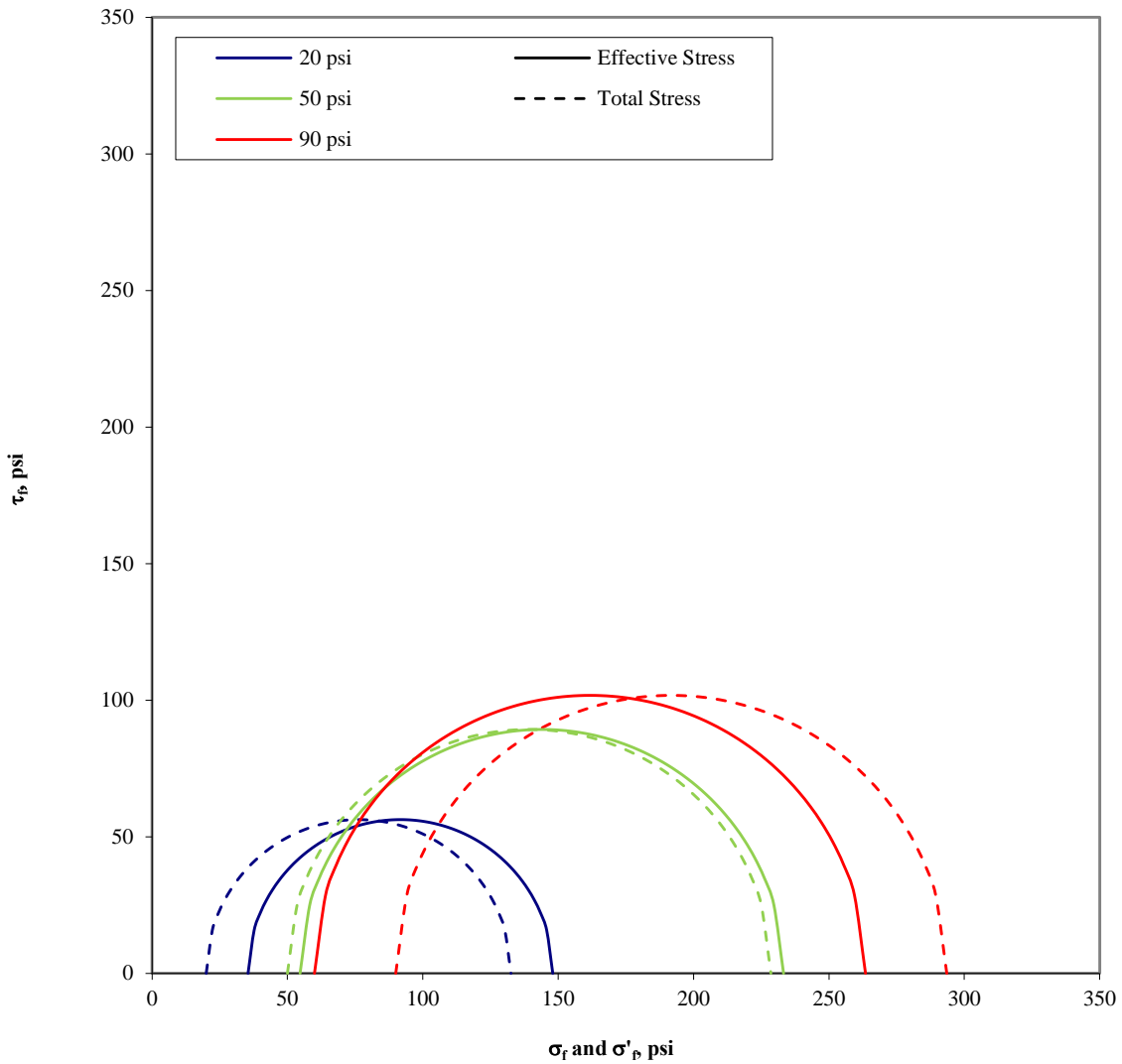
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	76.3	91.7	56.3
50	139.3	143.9	89.3
90	191.8	161.7	101.8

<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: Tailing Underflow</p>	<p>Technician: RJM/PRH</p>	<p>Reviewed: CCS</p>	<p>Date: 1/8/2013</p>	<p>Job Number: 103-92557</p>	<p>Figure: 3</p>

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
20	148.0	35.4	132.6	20.0
50	233.2	54.7	228.6	50.0
90	263.5	60.0	293.5	90.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013	Job Number: 103-92557	Figure: 4



Golder Associates Inc. Denver, Colorado		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
		Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013	Job Number: 103-92557	Figure: 6

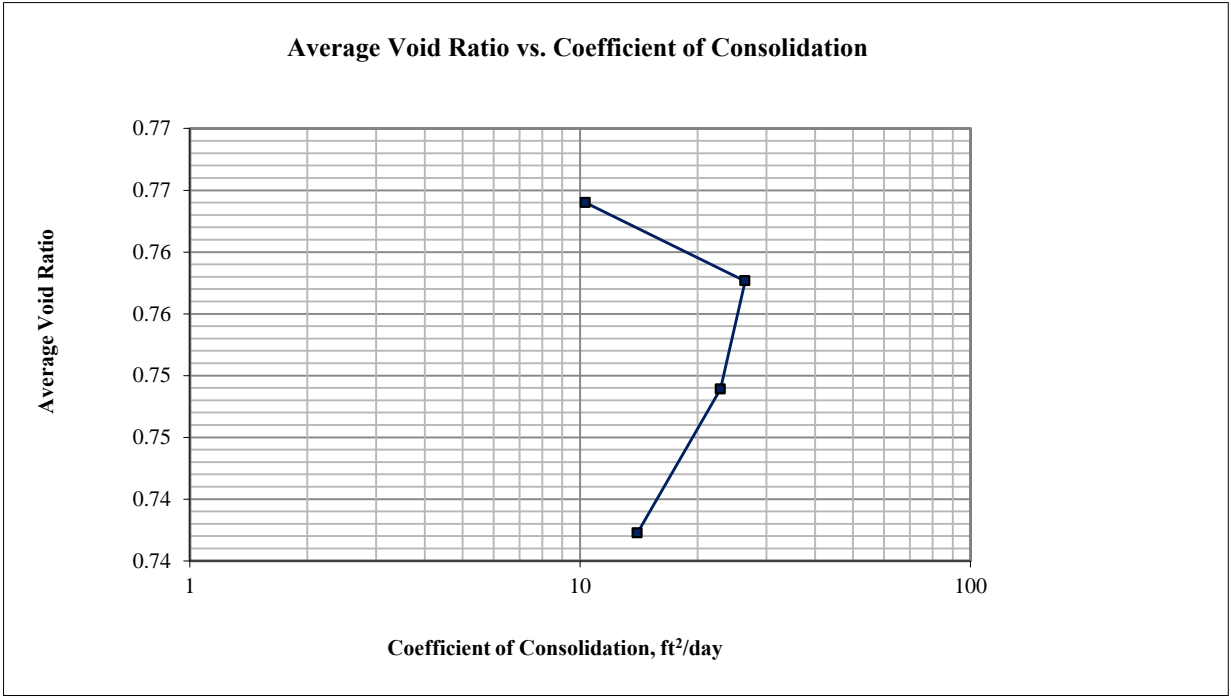
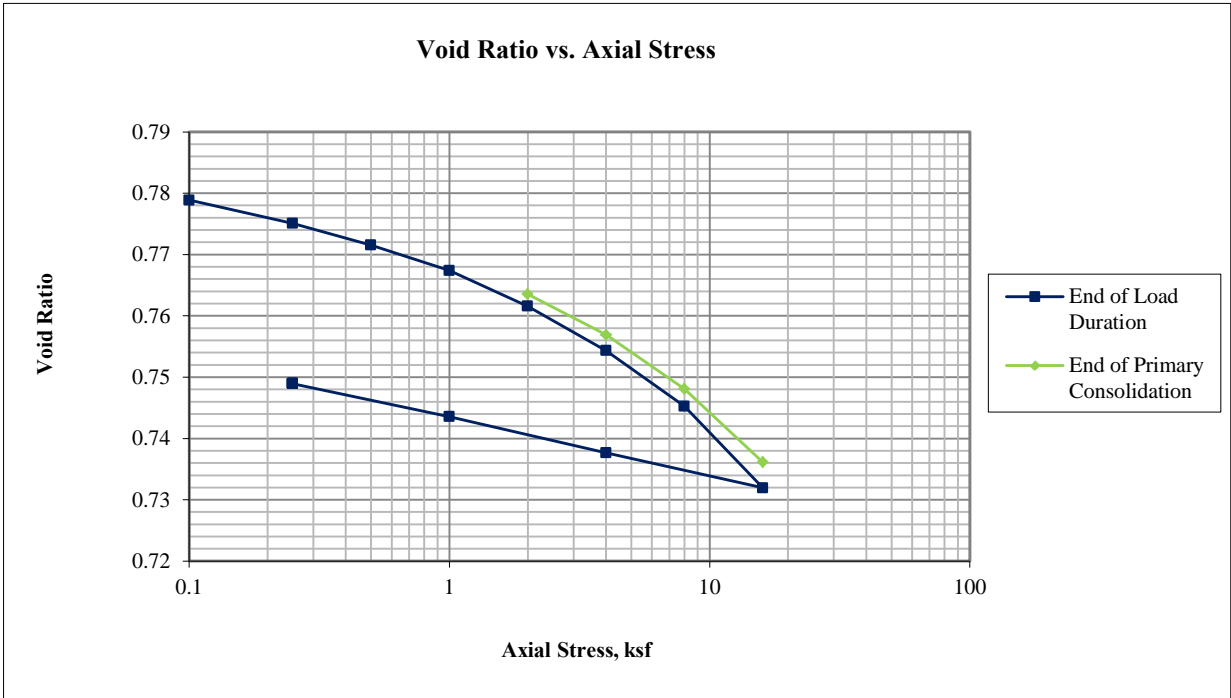


Golder Associates Inc. Denver, Colorado		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
		Sample: Tailing Underflow	Technician: RJM/PRH	Reviewed: CCS	Date: 1/8/2013
					Figure: 7

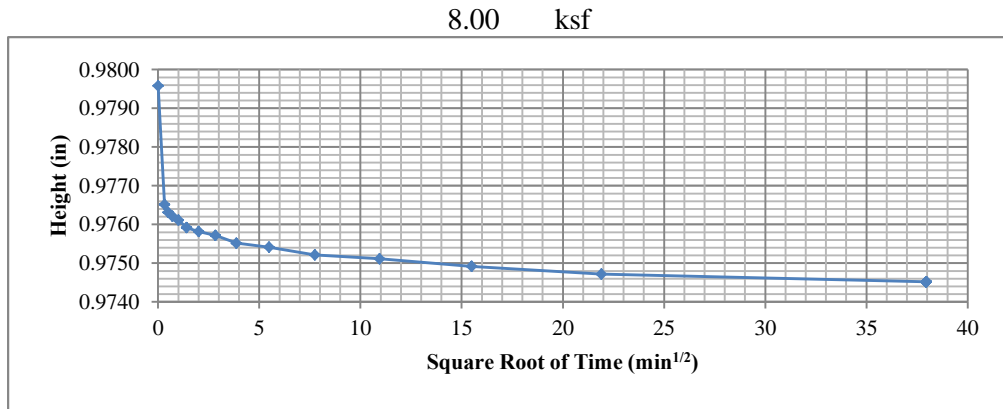
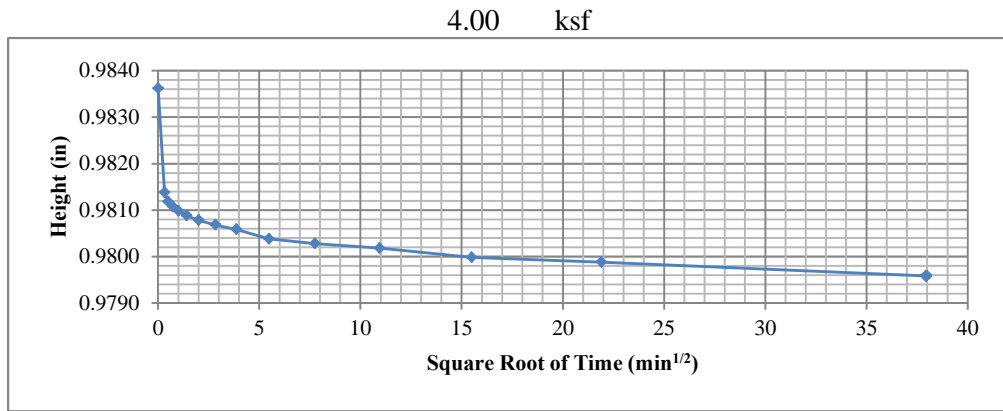
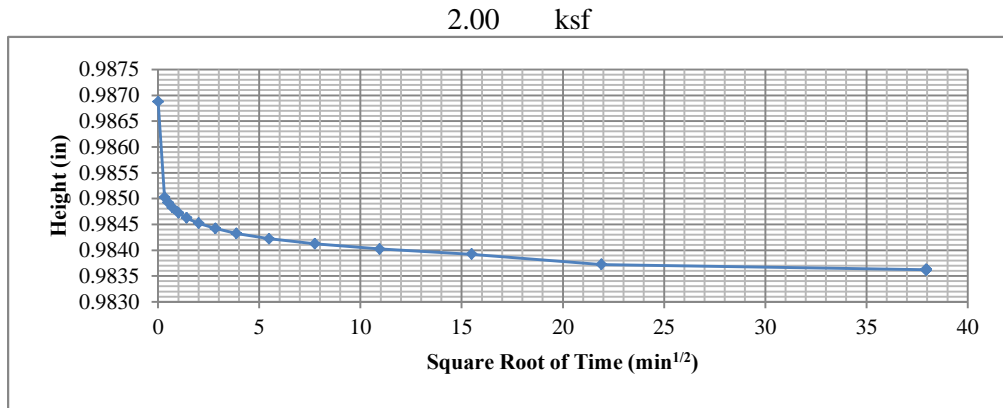
	Initial		Final	Notes	
Height =	0.994 in		0.973 in	Visual description (Golder procedure):	Damp, yellowish brown SILTY SAND
Diameter =	2.501 in		2.501 in	Atterberg Limits (ASTM D4318):	LL = NP PL = NP PI = NP
Area =	4.913 in ²		4.913 in ²	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 100% No. 200 = 18%
Volume =	4.883 in ³		4.780 in ³	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	16.5%		25.0%	Remold Targets:	92.2 pcf (dry) at 16.8% moisture (+/- 2.0%)
Specific Gravity =	2.64 (Assumed)		2.64 (Assumed)	Water Content of Trimmings (ASTM D2216):	16.6%
Height of Solids =	0.5584 in		0.5584 in	Trimming Procedure:	Specimen trimmed in ring
Void Ratio =	0.780		0.743	Inundation:	<input type="checkbox"/> Not inundated <input checked="" type="checkbox"/> Inundated at 0.1 ksf
Degree of Saturation =	55.8%		88.9%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.304 lb		0.326 lb	Apparatus:	Frame No. 2 (Wykeham Farrance 24251)
Dry Mass =	0.261 lb		0.261 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	107.6 pcf		118.0 pcf	Final Differential Height:	0.0036 in
Dry Unit Weight =	92.4 pcf		94.4 pcf	Estimated Preconsolidation Stress:	3.7 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	160					0.0000	0.9937	0.00	0.780				
1	0.10	1000					0.0004	0.9933	0.04	0.779				
2	0.25	1500					0.0025	0.9912	0.25	0.775				
3	0.50	1440					0.0045	0.9892	0.45	0.772				
4	1.00	1440					0.0068	0.9869	0.68	0.767				
5	2.00	1440	0.0090	0.9847	0.90	0.764	0.0101	0.9836	1.01	0.762	2 (Root time)	0.764	10.328	0.3
6	4.00	1440	0.0127	0.9810	1.27	0.757	0.0141	0.9796	1.42	0.754	2 (Root time)	0.758	26.471	0.3
7	8.00	1440	0.0176	0.9761	1.77	0.748	0.0192	0.9745	1.93	0.745	2 (Root time)	0.749	22.898	0.3
8	16.00	1440	0.0243	0.9694	2.44	0.736	0.0266	0.9671	2.68	0.732	2 (Root time)	0.737	14.013	0.4
9	4.00	150					0.0234	0.9703	2.35	0.738				
10	1.00	870					0.0201	0.9736	2.02	0.744				
11	0.25	1145					0.0171	0.9766	1.72	0.749				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailings Underflow	Technician: RJM	Reviewed: CCS	Start Date: 11/12/2012	Job Number: 103-92557	Figure: 1

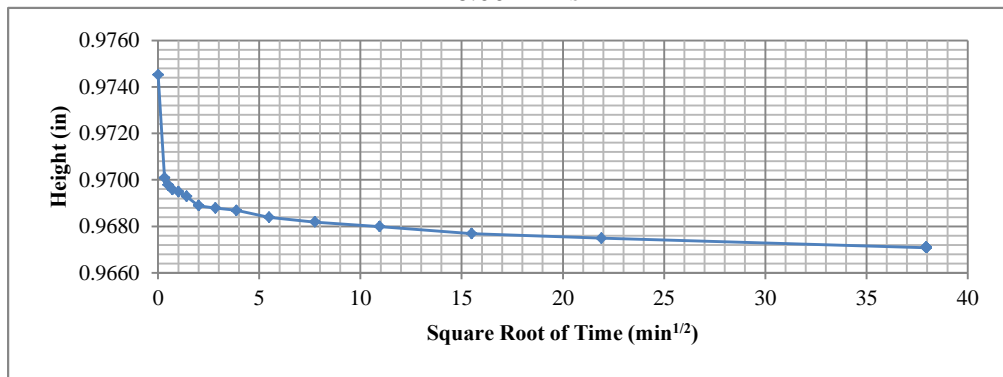


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailings Underflow	Technician: RJM	Reviewed: CCS	Start Date: 11/12/2012	Job Number: 103-92557	Figure: 2



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS (1)				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailings Underflow	Technician: RJM	Reviewed: CCS	Start Date: 11/12/2012	Job Number: 103-92557	Figure: 3

16.00 ksf



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS (2)			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailings Underflow	Technician: RJM	Reviewed: CCS	Start Date: 11/12/2012	Job Number: 103-92557	Figure: 4

**APPENDIX B.4
CYCLONE OVERFLOW AND WHOLE TAILINGS
TEST RESULTS**

**APPENDIX B.4.1
CYCLONE OVERFLOW TEST REPORTS**

Boring or Test Pit: --
 Sample: O/F Flume Beach
 Depth: -- ft
 Point No.: 1

Boring or Test Pit: --
 Sample: O/F Flume Beach
 Depth: -- ft
 Point No.: 2

Boring or Test Pit: --
 Sample: O/F Flume Beach
 Depth: -- ft
 Point No.: 3

Initial
 Length = 4.183 in
 Diameter = 1.924 in
 Wet Mass = 0.851 lb
 Area = 2.907 in²
 Volume = 12.162 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 0.666 lb
 Moisture Content = 27.7%
 Wet Unit Weight = 120.9 pcf
 Dry Unit Weight = 94.6 pcf
 Void Ratio = 0.74
 Percent Saturation = 99%

Initial
 Length = 4.127 in
 Diameter = 1.930 in
 Wet Mass = 0.832 lb
 Area = 2.926 in²
 Volume = 12.074 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 0.653 lb
 Moisture Content = 27.4%
 Wet Unit Weight = 119.1 pcf
 Dry Unit Weight = 93.5 pcf
 Void Ratio = 0.76
 Percent Saturation = 95%

Initial
 Length = 4.257 in
 Diameter = 1.929 in
 Wet Mass = 0.868 lb
 Area = 2.922 in²
 Volume = 12.441 in³
 Specific Gravity = 2.64 (Provided)
 Dry Mass of Solids = 0.682 lb
 Moisture Content = 27.3%
 Wet Unit Weight = 120.5 pcf
 Dry Unit Weight = 94.7 pcf
 Void Ratio = 0.74
 Percent Saturation = 98%

After Consolidation
 Length = 4.091 in
 Diameter = 1.894 in
 Area = 2.816 in² (Method B)
 Volume = 11.522 in³
 Moisture Content = 24.5%
 Wet Unit Weight = 124.4 pcf
 Dry Unit Weight = 99.9 pcf
 Void Ratio = 0.65
 Percent Saturation = 100%

After Consolidation
 Length = 4.080 in
 Diameter = 1.866 in
 Area = 2.734 in² (Method B)
 Volume = 11.156 in³
 Moisture Content = 23.7%
 Wet Unit Weight = 125.2 pcf
 Dry Unit Weight = 101.2 pcf
 Void Ratio = 0.63
 Percent Saturation = 100%

After Consolidation
 Length = 4.224 in
 Diameter = 1.849 in
 Area = 2.685 in² (Method B)
 Volume = 11.339 in³
 Moisture Content = 22.1%
 Wet Unit Weight = 126.9 pcf
 Dry Unit Weight = 103.9 pcf
 Void Ratio = 0.58
 Percent Saturation = 100%

B Parameter = 0.95
 Shear Rate = 0.051% /min.
 t₅₀ = 7.8 min.
 Strain at Failure = 5.0%

B Parameter = 0.99
 Shear Rate = 0.084% /min.
 t₅₀ = 3.7 min.
 Strain at Failure = 5.0%

B Parameter = 0.98
 Shear Rate = 0.083% /min.
 t₅₀ = 4.4 min.
 Strain at Failure = 5.0%

Cell Pressure = 60 psi
 Back Pressure = 40 psi
 Confining Pressure = 20 psi

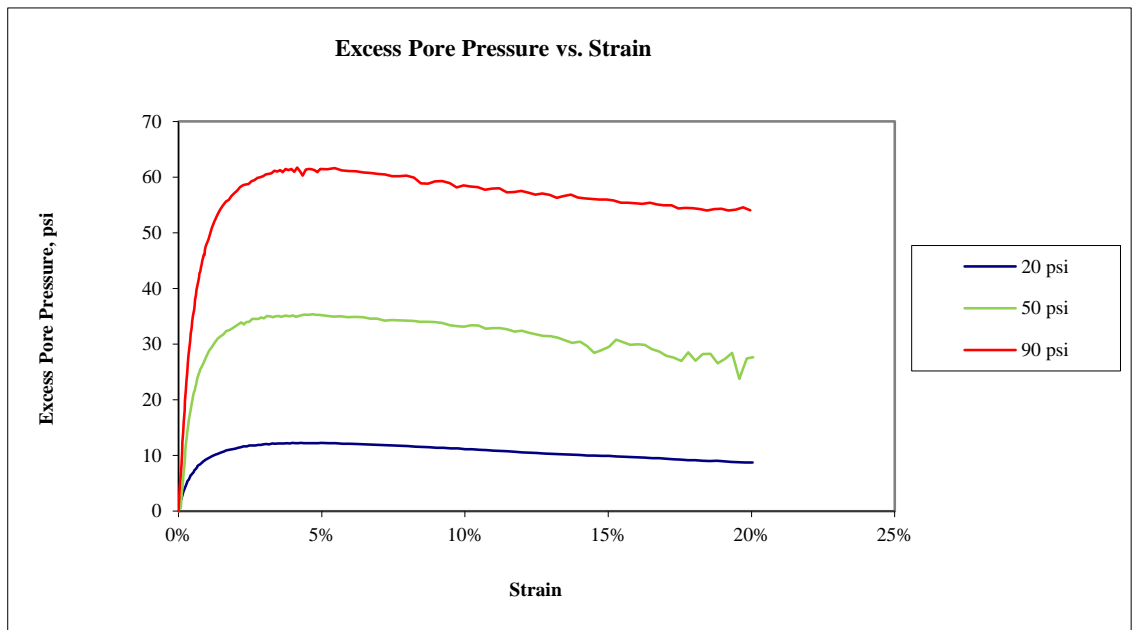
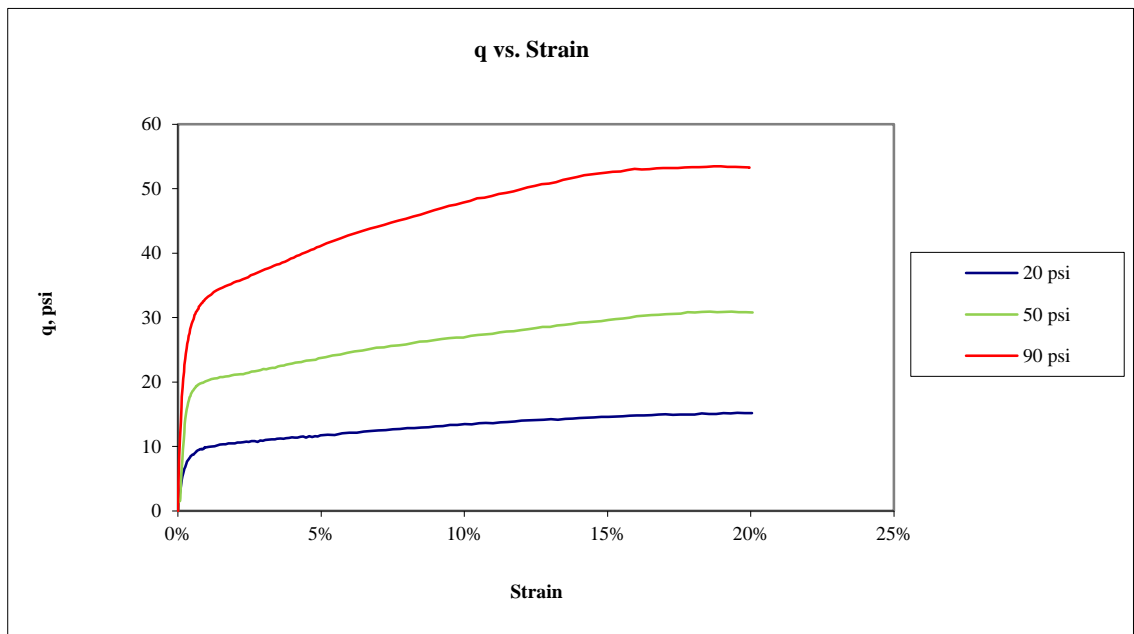
Cell Pressure = 90 psi
 Back Pressure = 40 psi
 Confining Pressure = 50 psi

Cell Pressure = 120 psi
 Back Pressure = 30 psi
 Confining Pressure = 90 psi

Notes: Sample description: Silt, pale yellow, moist

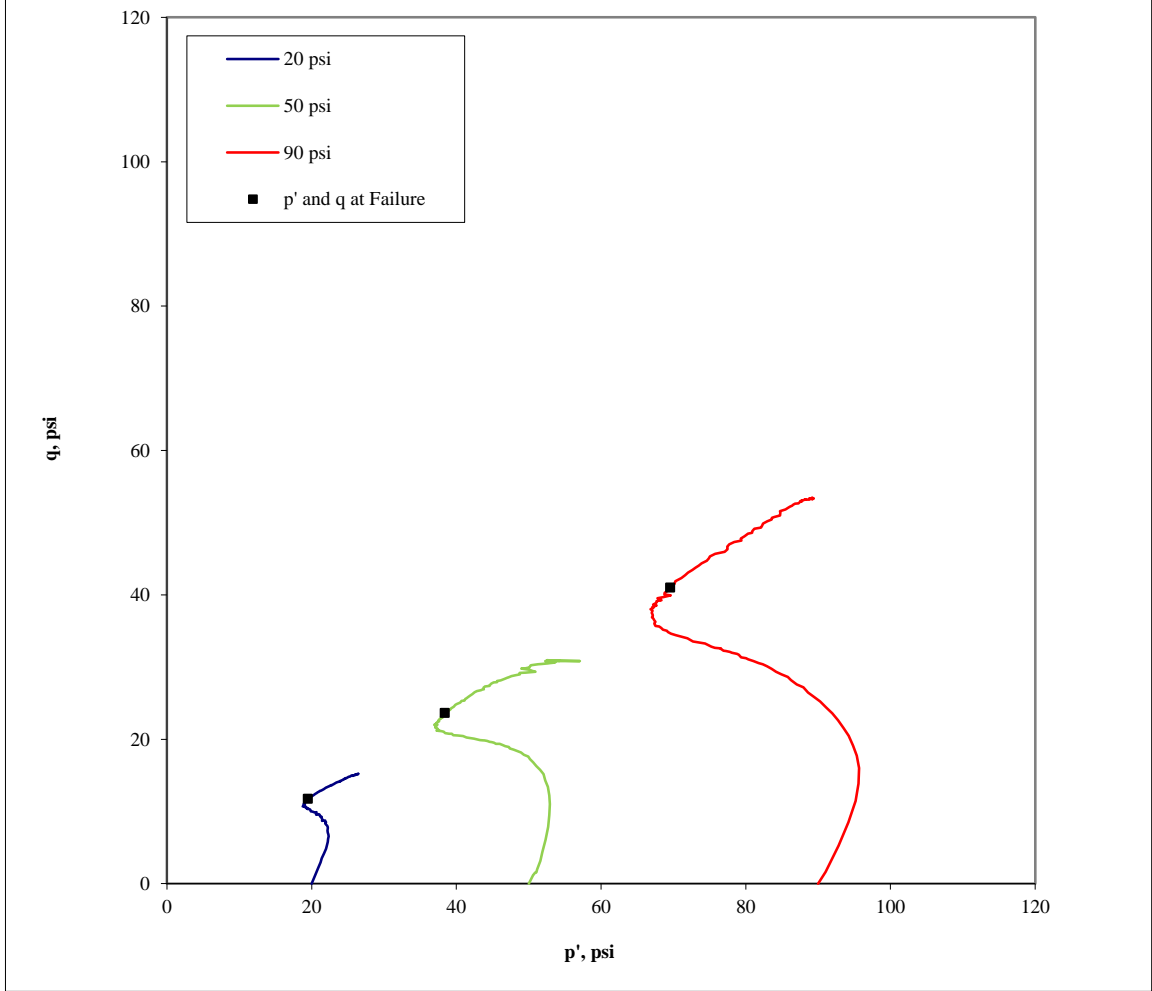
Atterberg limits: LL = 28 PL = 25 PI = 3 (ASTM D4318)
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 84% (ASTM D422, refer to separate report for gradation curve)
 Specimen type: Intact Reconstituted Slurry consolidated in tube and extruded
 Moisture from: Cuttings Entire specimen
 Saturation method: Wet Dry
 Failure criterion: (σ₁/σ₃)_{max} (σ₁-σ₃)_{max} 5 % strain
 Membrane effect: Corrected Not Corrected

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SAMPLE AND TEST DATA			
Job Short Title: Copper Flat Tailngs Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 2

Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	31.7	19.5	11.7
50	73.7	38.4	23.7
90	131.0	69.6	41.0

Golder Associates Inc.
Denver, Colorado

Job Short Title:
Copper Flat Tailings Design Study

Sample:
Overflow Flume Test Beach

Title:

ASTM D4767
CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT
STRESS PATH PLOT

Technician:
RJM

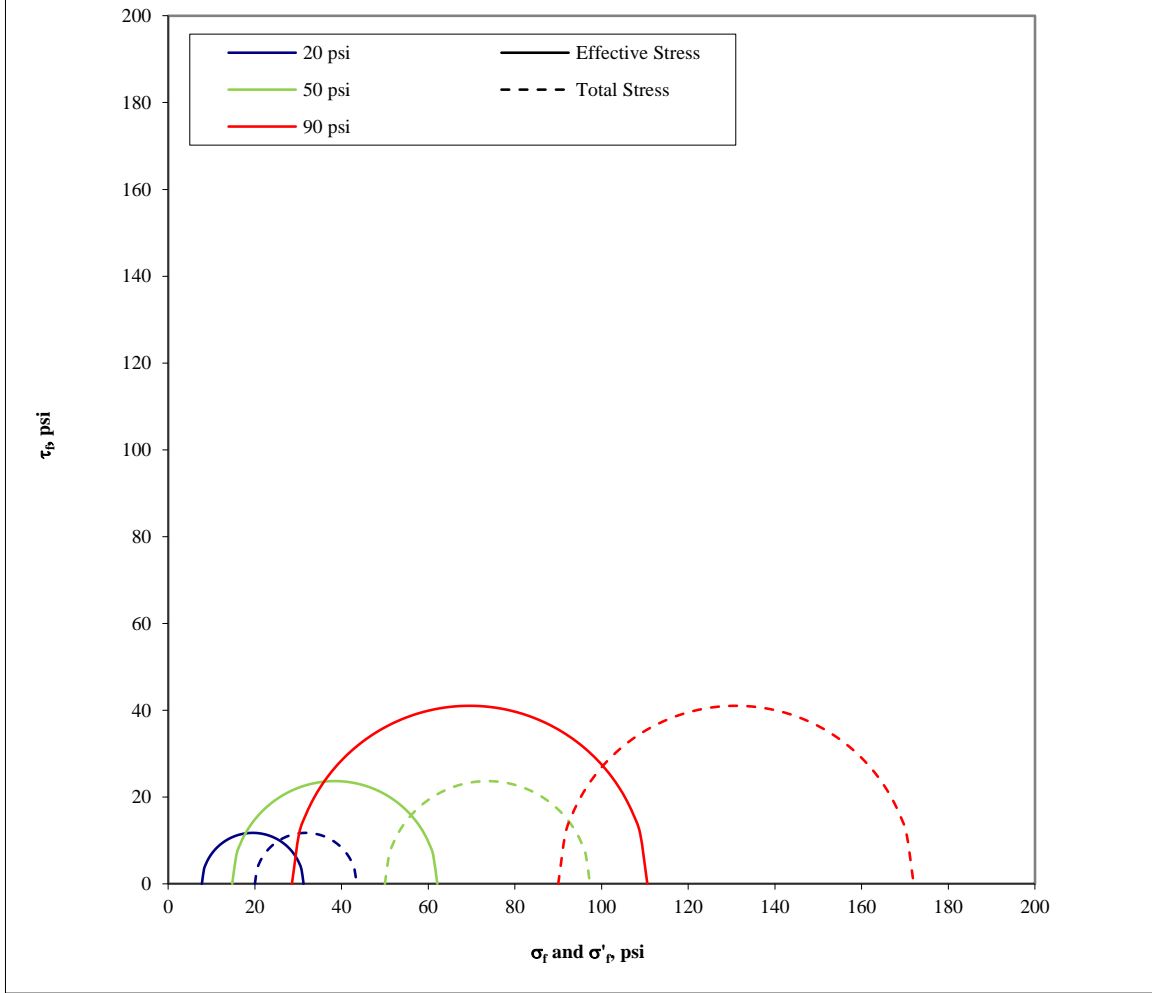
Reviewed:
DAR

Date:
3/12/2013

Job Number:
103-92557.010

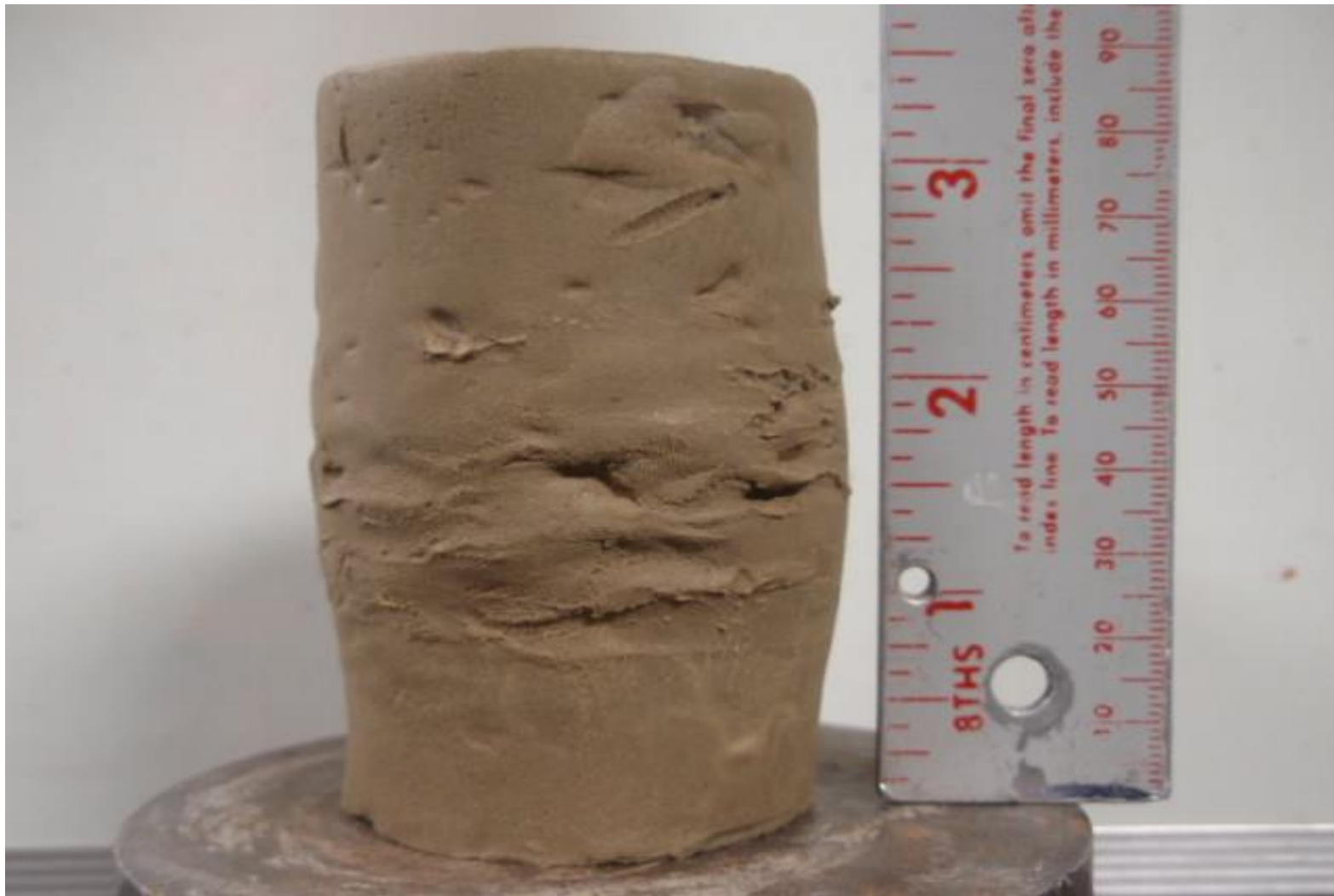
Figure:
3

Mohr's Circle Diagram



Confining Pressure (psi)	σ'_1 at failure (psi)	σ'_3 at failure (psi)	σ_1 at failure (psi)	σ_3 at failure (psi)
20	31.2	7.8	43.5	20.0
50	62.1	14.8	97.3	50.0
90	110.6	28.5	172.0	90.0

Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 4



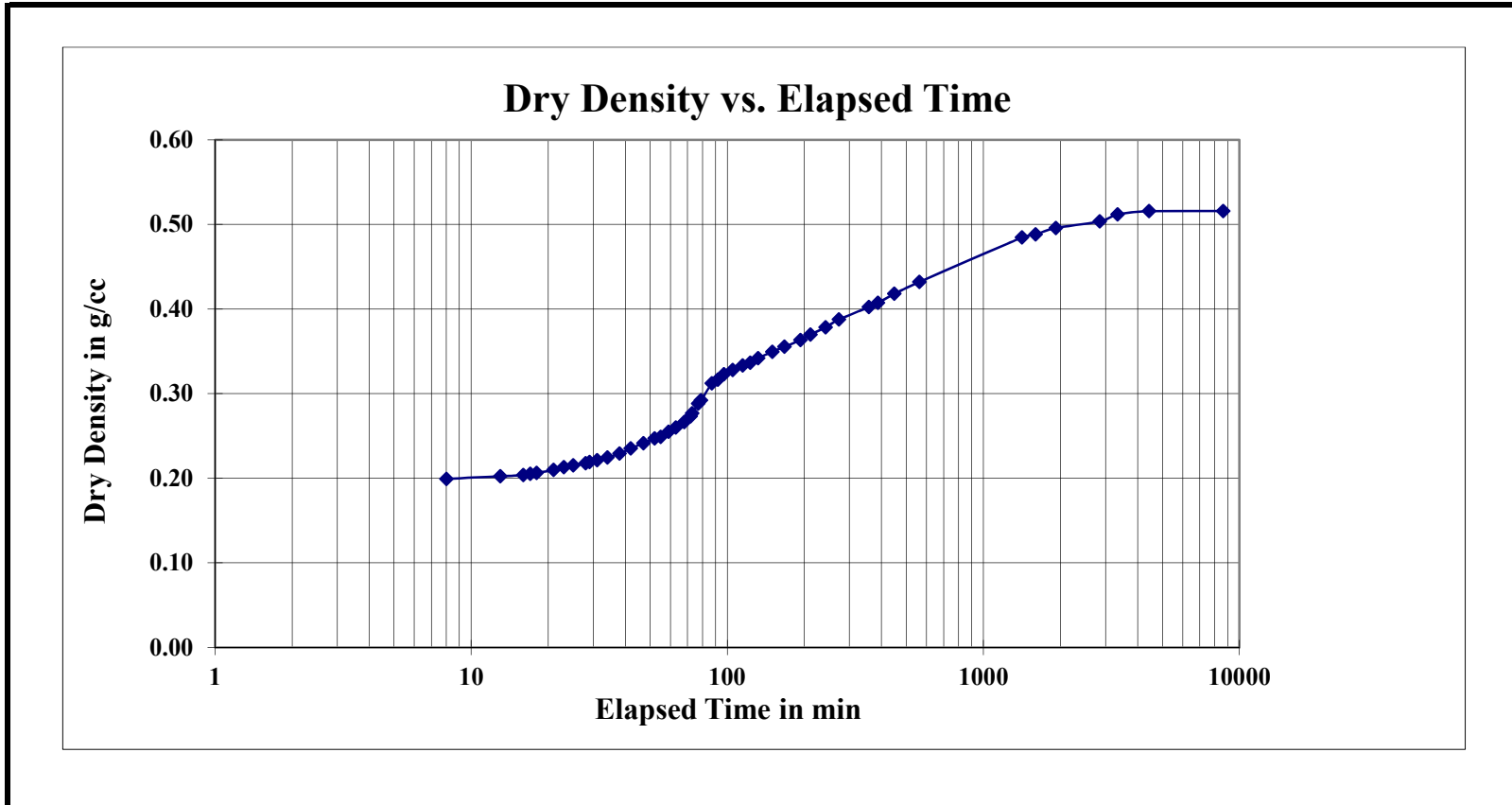
Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
Job Short Title: Copper Flat Tailngs Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 5



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
Job Short Title: Copper Flat Tailngs Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 6



Golder Associates Inc. Denver, Colorado		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 7



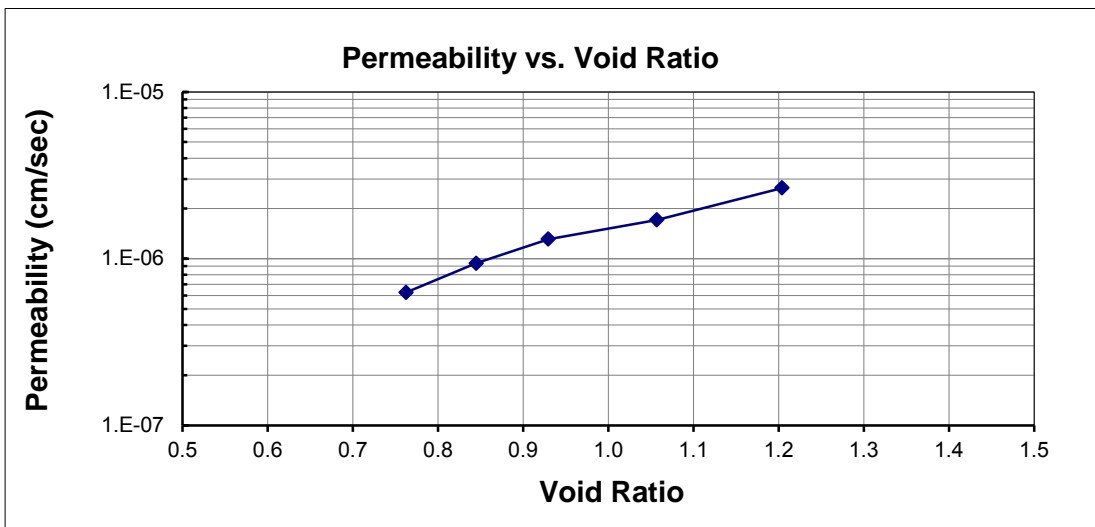
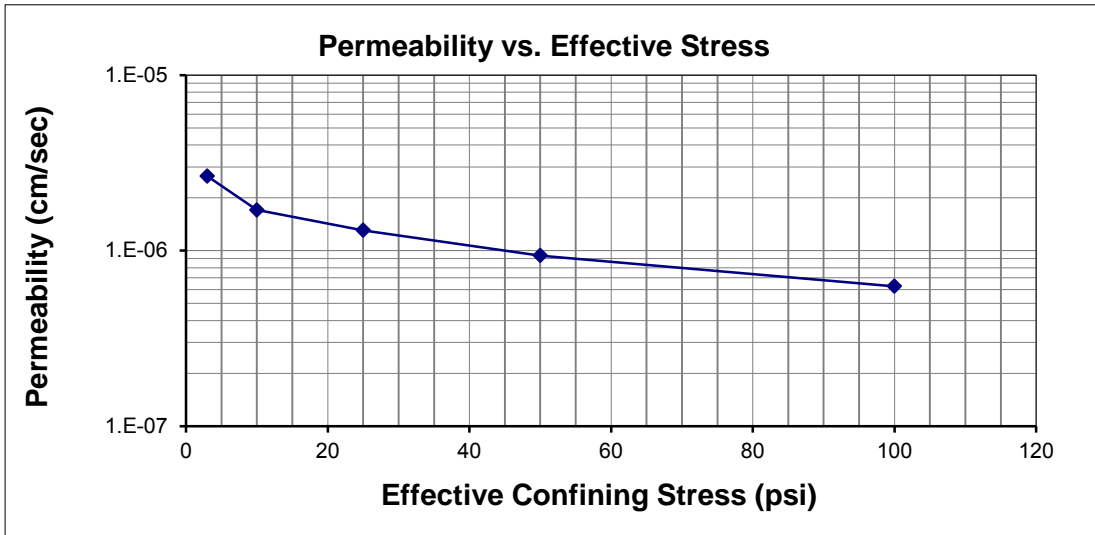
Golder Associates, Inc. Denver, Colorado			Title: SEDIMENTATION TESTING GRAPHICAL DATA		
Job Short Title: Copper Flat Tailings Design Study					
Sample No. Tailings Overflow Beach	System Single Drain	Reviewed: MB	Date: 08-Nov-12	Job Number: 103-92557	Figure: 2



	Initial	Final	
Length =	7.826	1.25	cm
Diameter =	7.11	7.11	cm
Wet Mass =	358.37	98.17	g
Area =	39.70	39.70	cm ²
Volume =	310.7	49.5	cm ³
Moisture Content =	378.6%	32.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	74.88	74.09	g
Unit Weight =	1.15	1.98	g/cm ³
Dry Unit Weight =	0.24	1.50	g/cm ³
Unit Weight =	72.00	123.62	lb/ft ³
Dry Unit Weight =	15.04	93.30	lb/ft ³
Percent Solids =	20.9%	75.5%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)
6.1	4.70E-06	1.24E-03	9.4	2.82E-06	2.1E-02	13.4	1.86E-06	3.2E-02	18.5	1.29E-06	5.1E-02	25.8	8.81E-07	7.0E-02
13.3	3.51E-06	9.28E-04	19.1	2.28E-06	1.7E-02	26.1	1.56E-06	2.7E-02	35.8	1.09E-06	4.4E-02	49.9	7.48E-07	5.9E-02
17.5	3.27E-06	8.65E-04	24.8	2.16E-06	1.6E-02	33.7	1.49E-06	2.5E-02	46.0	1.04E-06	4.2E-02	64.4	7.11E-07	5.6E-02
22.3	3.10E-06	8.19E-04	31.41	2.06E-06	1.5E-02	42.27	1.44E-06	2.4E-02	57.41	1.01E-06	4.0E-02	80.74	6.86E-07	5.4E-02
27.8	2.98E-06	7.88E-04	38.77	2.00E-06	1.5E-02	51.99	1.40E-06	2.4E-02	70.07	9.92E-07	4.0E-02	99.34	6.69E-07	5.3E-02
34.3	2.89E-06	7.63E-04	47.58	1.94E-06	1.4E-02	63.55	1.37E-06	2.3E-02	85.49	9.71E-07	3.9E-02	121.11	6.55E-07	5.2E-02
42.2	2.81E-06	7.43E-04	58.69	1.89E-06	1.4E-02	77.12	1.35E-06	2.3E-02	103.67	9.57E-07	3.8E-02	147.26	6.44E-07	5.1E-02
52.5	2.72E-06	7.19E-04	72.92	1.83E-06	1.4E-02	94.4	1.33E-06	2.3E-02	126.62	9.46E-07	3.8E-02	179.88	6.36E-07	5.0E-02
66.3	2.66E-06	7.03E-04	93.21	1.76E-06	1.3E-02	117.21	1.32E-06	2.2E-02	157	9.40E-07	3.8E-02	205.76	6.28E-07	5.0E-02
88.2	2.58E-06	6.82E-04	126.08	1.69E-06	1.2E-02	152.46	1.31E-06	2.2E-02	183.27	9.31E-07	3.7E-02	224.62	6.28E-07	5.0E-02
			145.49	1.67E-06	1.2E-02	175.79	1.30E-06	2.2E-02	203.01	9.40E-07	3.8E-02	248.31	6.24E-07	4.9E-02
Average (of final 3 values)	2.66E-06	7.01E-04	Average (of final 3 values)	1.71E-06	1.26E-02	Average (of final 3 values)	1.31E-06	2.23E-02	Average (of final 3 values)	9.37E-07	3.74E-02	Average (of final 3 values)	6.26E-07	4.95E-02

Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST SAMPLE DATA AND CALCULATIONS		
Job Short Title: Copper Flat Tailings Design Study				
Sample No. Tailings Overflow Flume Test Beach	Reviewed: CCS	Date: 12/3/2012	Job Number: 103-92557	Figure: 1



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
PERMEABILITY DATA

Job Short Title:
 Copper Flat Tailings Design Study

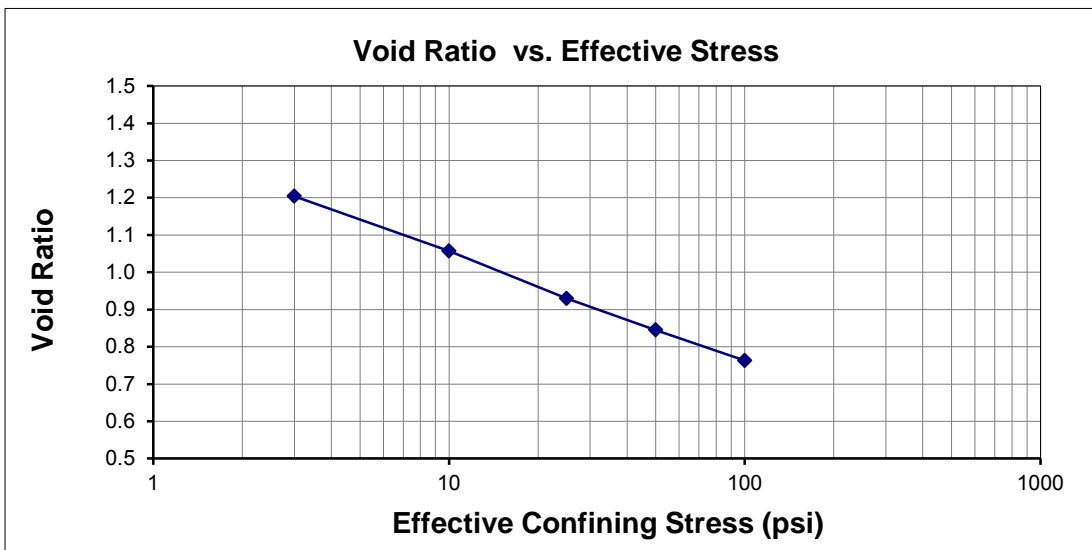
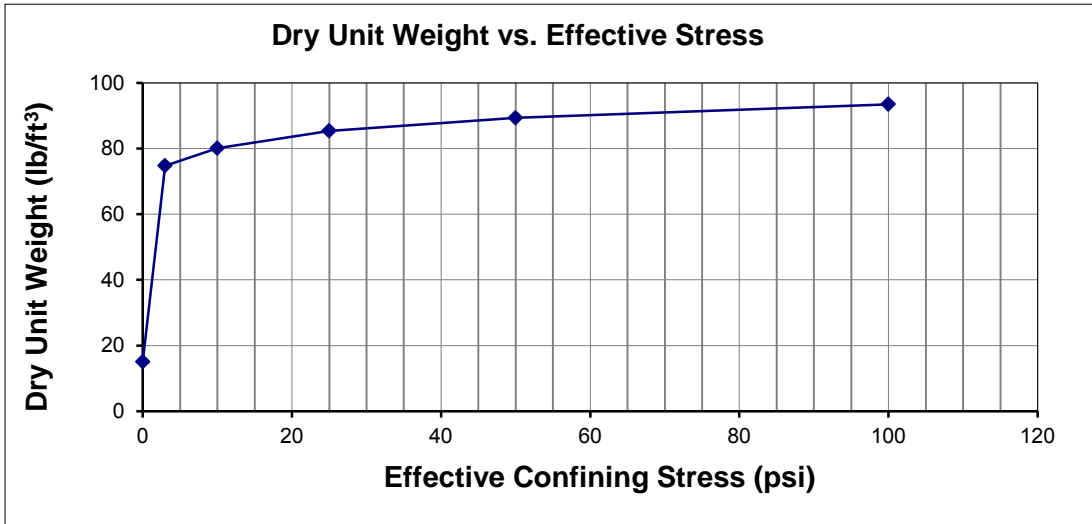
Sample No.
 Tailings Overflow Flume Test Beach

Reviewed:
 CCS

Date:
 12/3/2012

Job Number:
 103-92557

Figure:
 2



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
DENSITY DATA

Job Short Title:
 Copper Flat Tailings Design Study

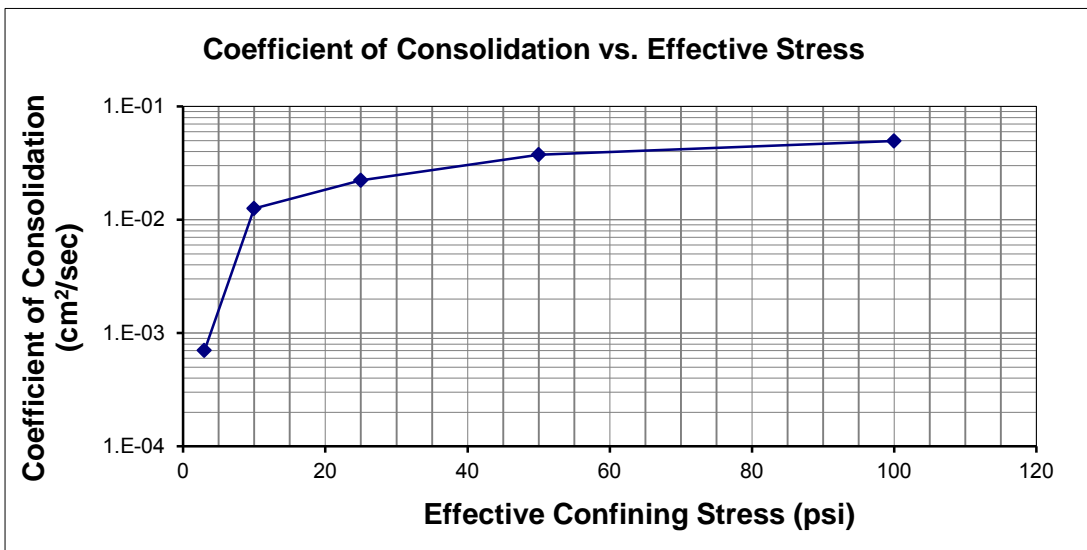
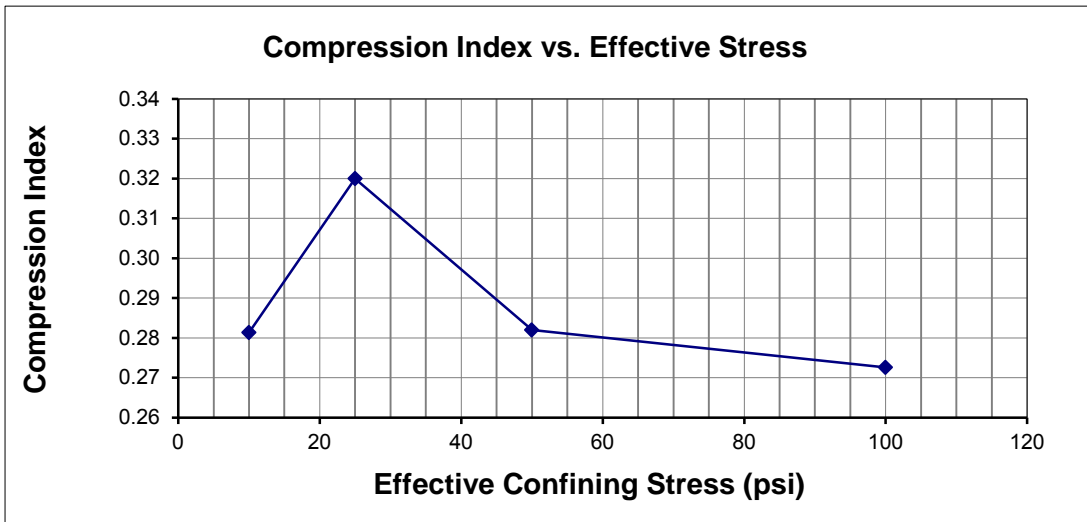
Sample No.
 Tailings Overflow Flume Test Beach

Reviewed:
 CCS

Date:
 12/3/2012

Job Number:
 103-92557

Figure:
 3



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
COMPRESSION DATA

Job Short Title:
 Copper Flat Tailings Design Study

Sample No.
 Tailings Overflow Flume Test Beach

Reviewed:
 CCS

Date:
 12/3/2012

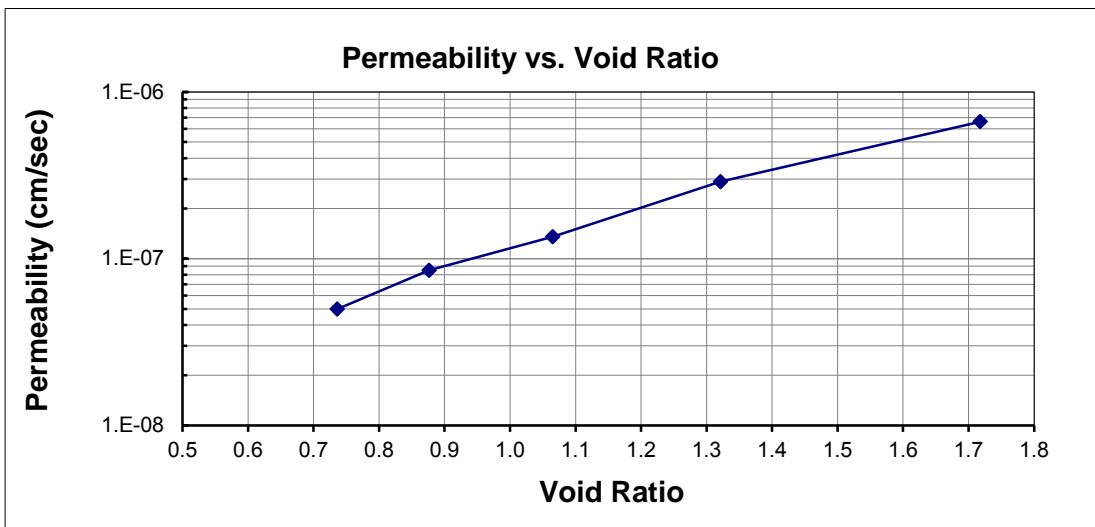
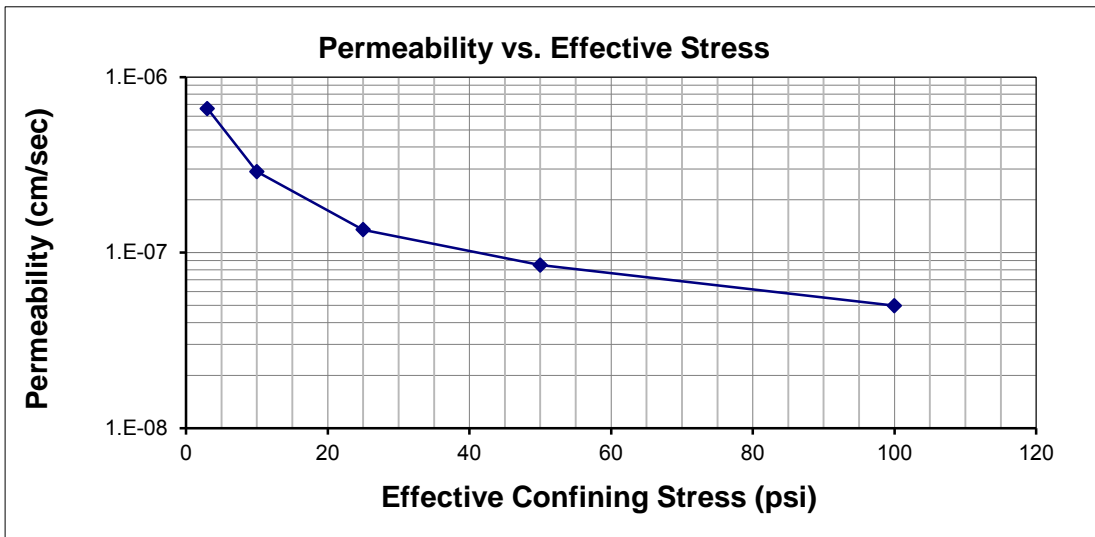
Job Number:
 103-92557

Figure:
 4

	Initial	Final	
Length =	8.172	0.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	351.12	53.21	g
Area =	39.64	39.64	cm ²
Volume =	323.9	25.5	cm ³
Moisture Content =	786.2%	37.1%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	39.62	38.81	g
Unit Weight =	1.08	2.08	g/cm ³
Dry Unit Weight =	0.12	1.52	g/cm ³
Unit Weight =	67.67	130.13	lb/ft ³
Dry Unit Weight =	7.64	94.92	lb/ft ³
Percent Solids =	11.3%	72.9%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)
22.46	8.20E-07	1.98E-04	50.77	3.10E-07	1.0E-03	98.24	1.42E-07	1.4E-03	144.92	8.78E-08	1.7E-03	228.9	5.14E-08	2.4E-03
41.52	7.28E-07	1.76E-04	86.68	2.98E-07	1.0E-03	166.02	1.38E-07	1.3E-03	243.05	8.59E-08	1.7E-03	384.0	5.03E-08	2.4E-03
52.62	7.06E-07	1.70E-04	107.84	2.94E-07	9.9E-04	205.40	1.37E-07	1.3E-03	301.11	8.52E-08	1.6E-03	474.8	5.00E-08	2.4E-03
65.16	6.89E-07	1.66E-04	131.71	2.91E-07	9.8E-04	250.52	1.36E-07	1.3E-03	364.17	8.52E-08	1.6E-03	576.64	4.98E-08	2.3E-03
79.43	6.78E-07	1.64E-04	158.8	2.90E-07	9.8E-04	299.74	1.36E-07	1.3E-03	436.95	8.51E-08	1.6E-03	690.24	4.98E-08	2.3E-03
96.23	6.68E-07	1.61E-04	190.02	2.89E-07	9.7E-04	359.30	1.36E-07	1.3E-03	524.36	8.46E-08	1.6E-03	824.08	4.98E-08	2.3E-03
115.84	6.63E-07	1.60E-04	226.55	2.90E-07	9.8E-04	430.18	1.36E-07	1.3E-03	625.14	8.49E-08	1.6E-03			
140.26	6.61E-07	1.60E-04				521.68	1.35E-07	1.3E-03	751.92	8.51E-08	1.6E-03			
172.37	6.63E-07	1.60E-04				590.52	1.35E-07	1.3E-03	849.86	8.51E-08	1.6E-03			
						638.40	1.36E-07	1.3E-03						
						707.55	1.35E-07	1.3E-03						
Average (of final 3 values)	6.62E-07	1.60E-04	Average (of final 3 values)	2.89E-07	9.77E-04	Average (of final 3 values)	1.35E-07	1.29E-03	Average (of final 3 values)	8.50E-08	1.64E-03	Average (of final 3 values)	4.98E-08	2.34E-03

Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST SAMPLE DATA AND CALCULATIONS			
Job Short Title: Copper Flat Tailings Design Study					
Sample No. Tailings Overflow Flume Test Slime	Reviewed: CCS	Date: 12/17/2012	Job Number: 103-92557	Figure: 1	



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
PERMEABILITY DATA

Job Short Title:
 Copper Flat Tailings Design Study

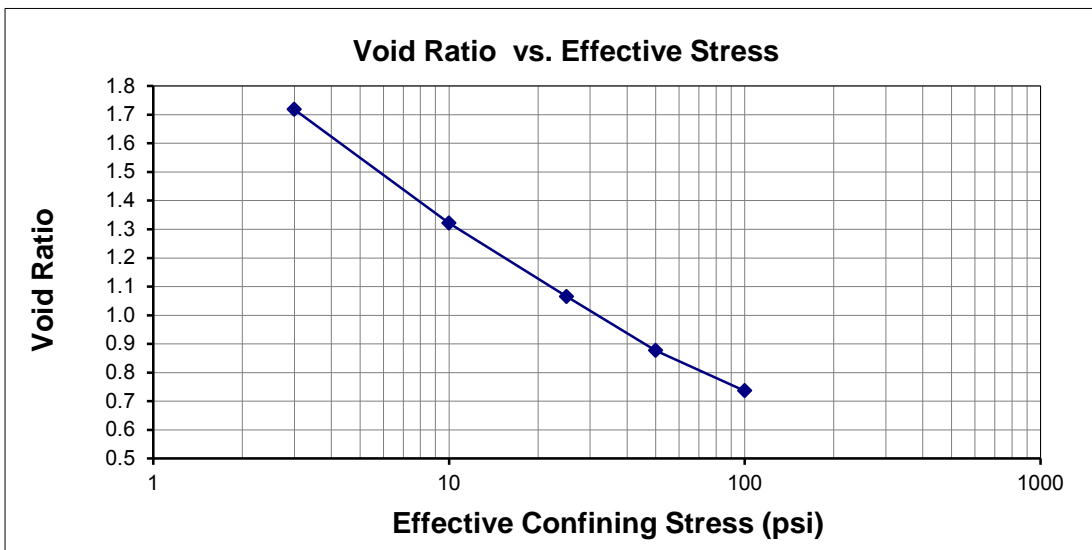
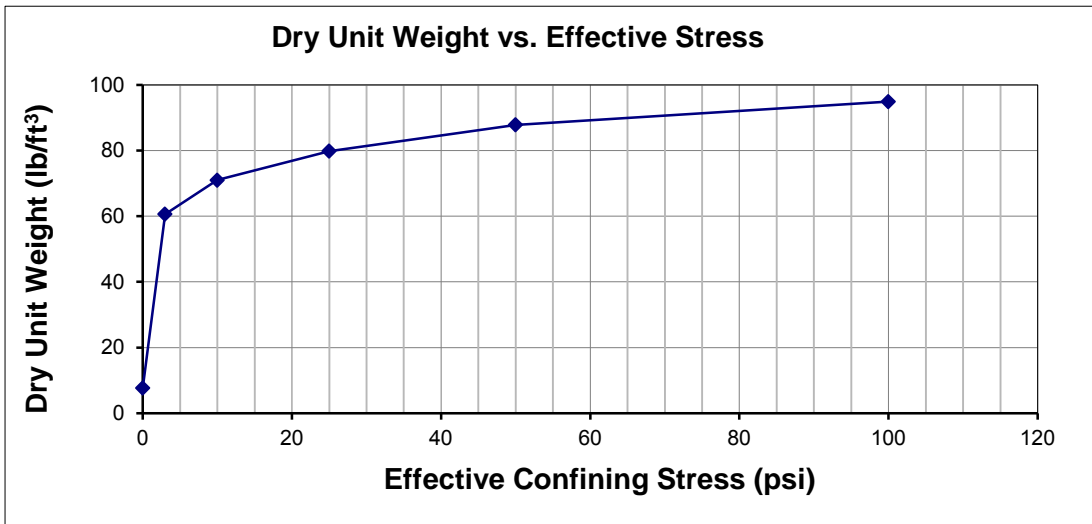
Sample No.
 Tailings Overflow Flume Test Slime

Reviewed:
 CCS

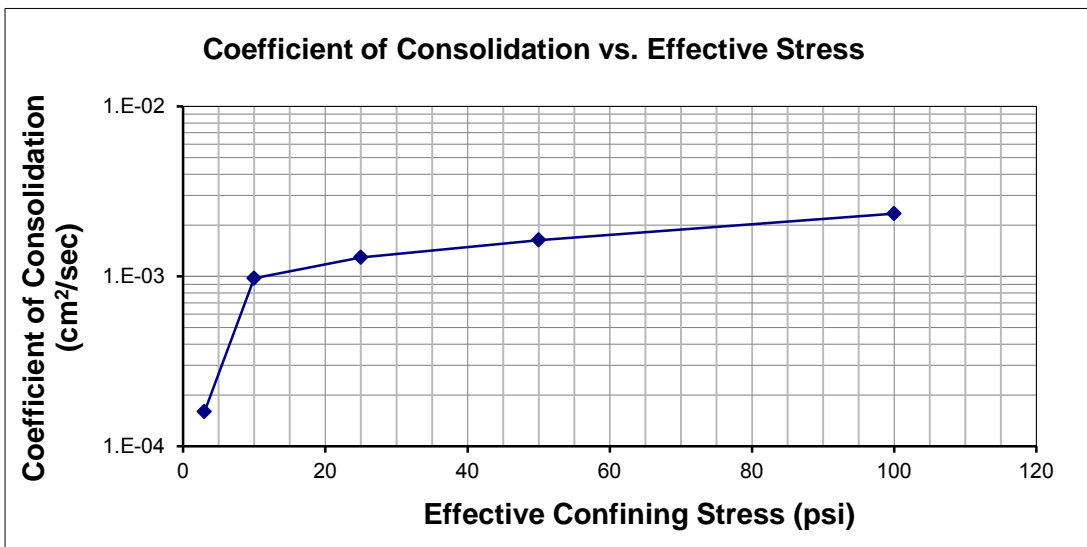
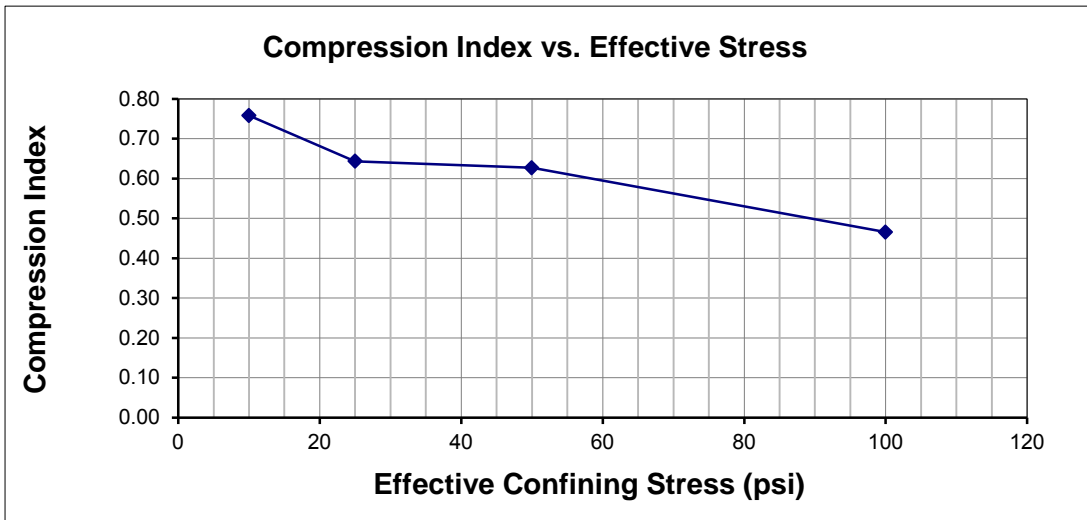
Date:
 12/17/2012

Job Number:
 103-92557

Figure:
 2



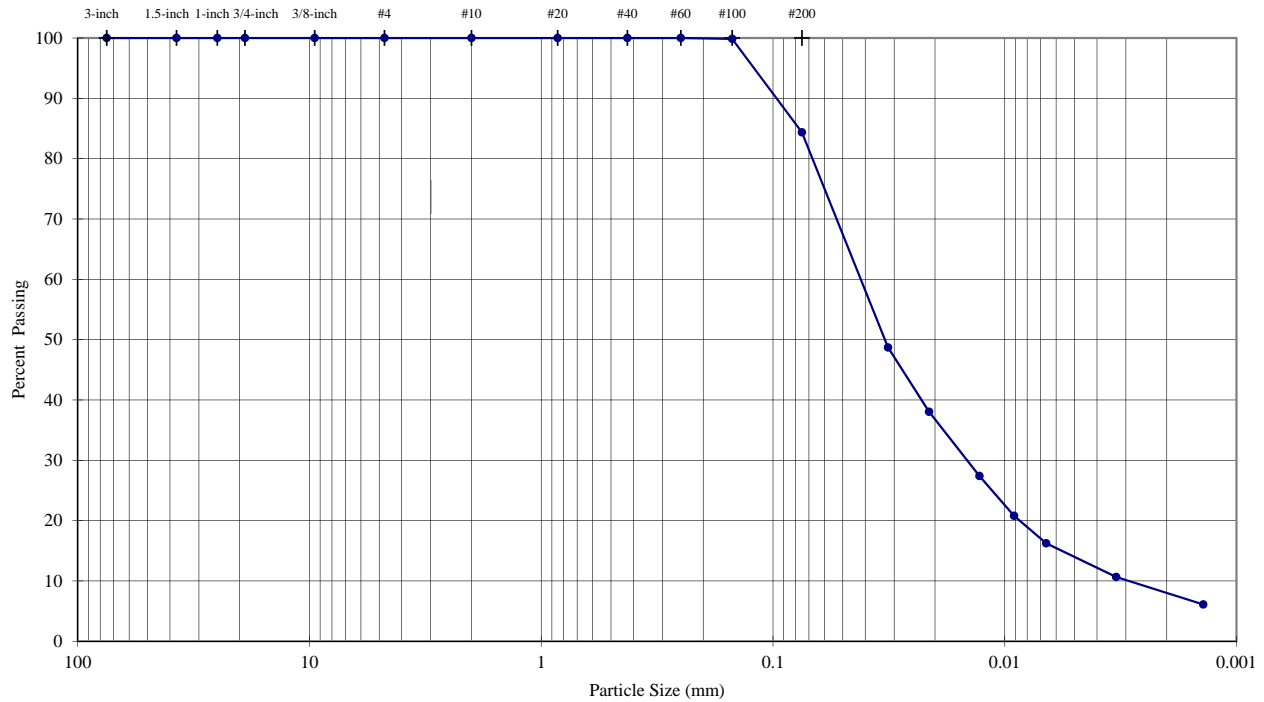
Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST RESULTS DENSITY DATA		
Job Short Title: Copper Flat Tailings Design Study				
Sample No. Tailings Overflow Flume Test Slime	Reviewed: CCS	Date: 12/17/2012	Job Number: 103-92557	Figure: 3



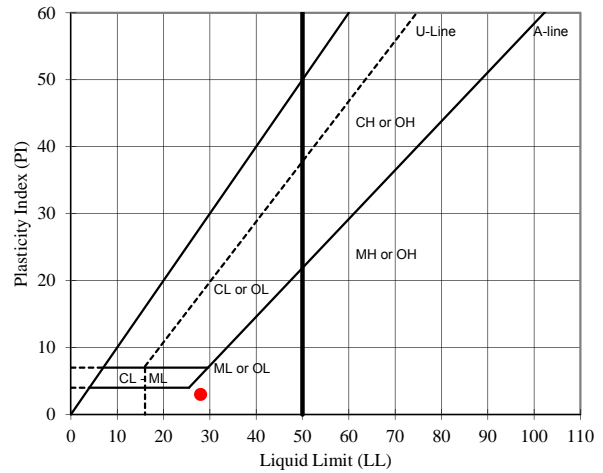
Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST RESULTS COMPRESSION DATA		
Job Short Title: Copper Flat Tailings Design Study				
Sample No. Tailings Overflow Flume Test Slime	Reviewed: CCS	Date: 12/17/2012	Job Number: 103-92557	Figure: 4

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Beach**
 TYPE: **Flume Test**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	0.00
3/8-inch	9.5	100.0		
#4	4.75	100.0	Coarse Sand	0.00
#10	2.0	100.0		
#20	0.85	100.0	Medium Sand	0.00
#40	0.425	100.0		
#60	0.25	100.0	Fine Sand	15.64
#100	0.15	99.9		
#200	0.075	84.4		
Hycrometer Analysis	0.032	48.7	Silt or Clay Fines	84.36
	0.021	38.0		
	0.013	27.4		
	0.009	20.8		
	0.007	16.2		
	0.003	10.7		
	0.001	6.1		



USCS Description (ASTM D 2487):
Dry, pale yellow silt

LL	PL	PI	SpG (assumed)
28	25	3	2.64

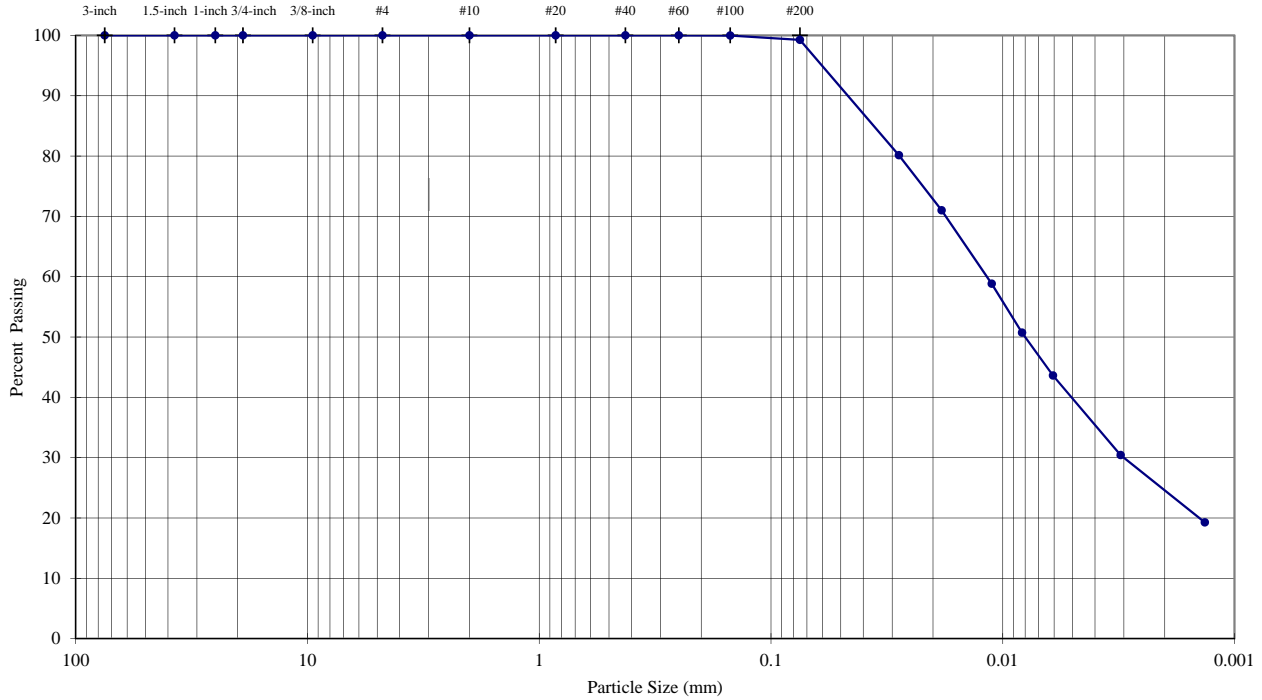
As-Received Moisture Content (%) #DIV/0! USCS Group Symbol ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute
 Sample prepared for Atterberg Limits testing by the dry method
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

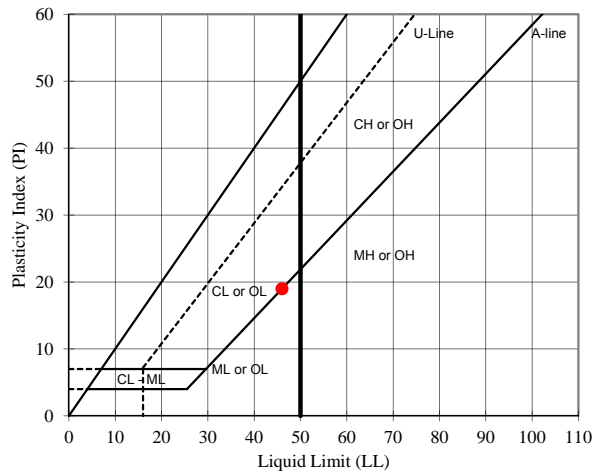
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Slime**
 TYPE: **Flume Test**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	0.00
3/8-inch	9.5	100.0		
#4	4.75	100.0	Coarse Sand	0.00
#10	2.0	100.0		
#20	0.85	100.0	Medium Sand	0.00
#40	0.425	100.0		
#60	0.25	100.0	Fine Sand	0.74
#100	0.15	100.0		
#200	0.075	99.3		
Hydrometer Analysis		0.028	Silt or Clay Fines	99.26
		0.018		
		0.011		
		0.008		
		0.006		
		0.003		
	0.001	19.3		



Visual Description (Golder Procedure):
 Dry, pale yellow silty clay

LL	PL	PI	SpG (assumed)
46	27	19	2.64

As-Received Moisture Content (%) USCS Group Symbol
-- CL-ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

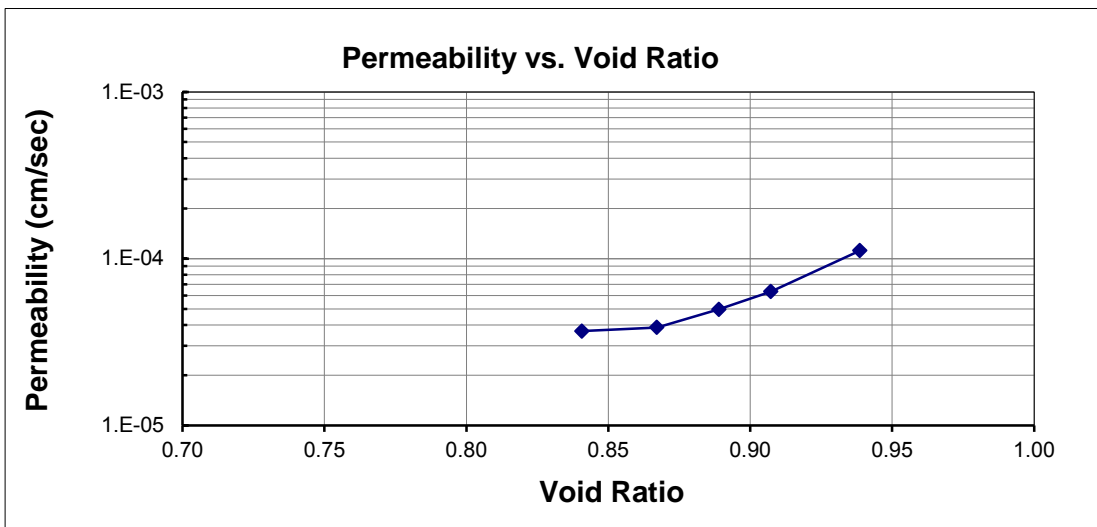
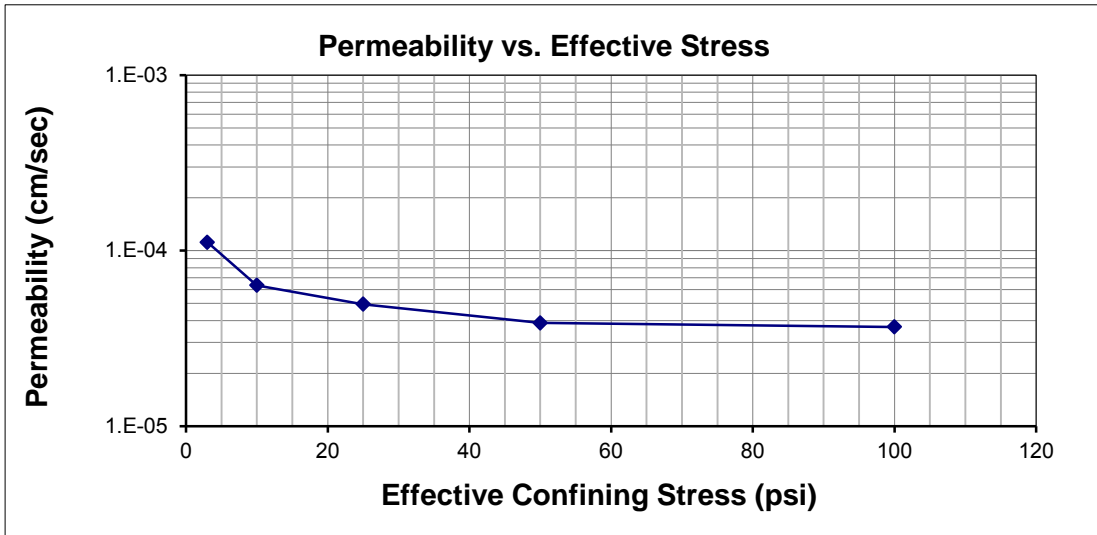
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

**APPENDIX B.4.2
WHOLE TAILINGS TEST REPORTS**

	Initial	Final	
Length =	7.852	5.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	515.99	424.51	g
Area =	39.64	39.64	cm ²
Volume =	311.2	223.6	cm ³
Moisture Content =	60.1%	32.4%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	322.29	320.63	g
Unit Weight =	1.66	1.90	g/cm ³
Dry Unit Weight =	1.04	1.43	g/cm ³
Unit Weight =	103.50	118.30	lb/ft ³
Dry Unit Weight =	64.65	89.35	lb/ft ³
Percent Solids =	62.5%	75.5%	

Piston Pressure:	8 psi	562.5 g/cm ²	Piston Pressure:	15 psi	1,054.6 g/cm ²	Piston Pressure:	30 psi	2,109.2 g/cm ²	Piston Pressure:	55 psi	3,866.9 g/cm ²	Piston Pressure:	105 psi	7,382.3 g/cm ²
Sample Pressure:	5 psi	351.5 g/cm ²	Sample Pressure:	5 psi	351.5 g/cm ²	Sample Pressure:	5 psi	351.5 g/cm ²	Sample Pressure:	5 psi	351.5 g/cm ²	Sample Pressure:	5 psi	351.5 g/cm ²
Consolidation pressure:	3 psi	210.9 g/cm ²	Consolidation Pressure:	10 psi	703.1 g/cm ²	Consolidation Pressure:	25 psi	1,757.7 g/cm ²	Consolidation Pressure:	50 psi	3,515.4 g/cm ²	Consolidation Pressure:	100 psi	7,030.8 g/cm ²
Before Consolidation			Before Consolidation			Before Consolidation			Before Consolidation			Before Consolidation		
Initial Sample Height:	7.85	cm	Initial Sample Height:	5.94	cm	Initial Sample Height:	5.84	cm	Initial Sample Height:	5.79	cm	Initial Sample Height:	5.72	cm
Initial Dry Unit Weight:	1.04	g/cm ³	Initial Dry Unit Weight:	1.36	g/cm ³	Initial Dry Unit Weight:	1.38	g/cm ³	Initial Dry Unit Weight:	1.40	g/cm ³	Initial Dry Unit Weight:	1.41	g/cm ³
Initial Void Ratio:	1.55		Initial Void Ratio:	0.94		Initial Void Ratio:	0.91		Initial Void Ratio:	0.89		Initial Void Ratio:	0.87	
After Consolidation			After Consolidation			After Consolidation			After Consolidation			After Consolidation		
Final Sample Height:	5.94	cm	Final Sample Height:	5.84	cm	Final Sample Height:	5.79	cm	Final Sample Height:	5.72	cm	Final Sample Height:	5.64	cm
Final Dry Unit Weight:	1.36	g/cm ³	Final Dry Unit Weight:	1.38	g/cm ³	Final Dry Unit Weight:	1.40	g/cm ³	Final Dry Unit Weight:	1.41	g/cm ³	Final Dry Unit Weight:	1.43	g/cm ³
Final Void Ratio:	0.94		Final Void Ratio:	0.91		Final Void Ratio:	0.89		Final Void Ratio:	0.87		Final Void Ratio:	0.84	
Calculations			Calculations			Calculations			Calculations			Calculations		
Coefficient of Compressibility, a _v	2.90E-03	cm ² /g	Coefficient of Compressibility, a _v	6.37E-05	cm ² /g	Coefficient of Compressibility, a _v	1.73E-05	cm ² /g	Coefficient of Compressibility, a _v	1.24E-05	cm ² /g	Coefficient of Compressibility, a _v	7.52E-06	cm ² /g
Coefficient of Volume Compressibility, m _v	1.14E-03	cm ² /g	Coefficient of Volume Compressibility, m _v	3.28E-05	cm ² /g	Coefficient of Volume Compressibility, m _v	9.09E-06	cm ² /g	Coefficient of Volume Compressibility, m _v	6.59E-06	cm ² /g	Coefficient of Volume Compressibility, m _v	4.03E-06	cm ² /g
Compression Index, C _c	-		Compression Index, C _c	0.06		Compression Index, C _c	0.05		Compression Index, C _c	0.07		Compression Index, C _c	0.09	
Δ Time	Permeability k	Coefficient of Consolidation, c_v	Δ Time	Permeability k	Coefficient of Consolidation, c_v	Δ Time	Permeability k	Coefficient of Consolidation, c_v	Δ Time	Permeability k	Coefficient of Consolidation, c_v	Δ Time	Permeability k	Coefficient of Consolidation, c_v
(sec)	(cm/sec)	(cm ² /sec)	(sec)	(cm/sec)	(cm ² /sec)	(sec)	(cm/sec)	(cm ² /sec)	(sec)	(cm/sec)	(cm ² /sec)	(sec)	(cm/sec)	(cm ² /sec)
3.85	1.12E-04	9.86E-02	6.70	6.33E-05	1.93	10.15	4.96E-05	5.46	10.77	3.86E-05	5.86	8.18	3.68E-05	9.14
4.60	1.12E-04	9.88E-02	8.03	6.33E-05	1.93	12.44	4.92E-05	5.41	12.80	3.89E-05	5.90	9.75	3.69E-05	9.16
5.68	1.11E-04	9.73E-02	9.69	6.38E-05	1.94	15.42	4.99E-05	5.50	15.65	3.86E-05	5.87	11.77	3.65E-05	9.07
Average (of final 3 values)	1.12E-04	9.83E-02	Average (of final 3 values)	6.35E-05	1.93E+00	Average (of final 3 values)	4.96E-05	5.46E+00	Average (of final 3 values)	3.87E-05	5.88E+00	Average (of final 3 values)	3.67E-05	9.12E+00

Golder Associates Inc.		Title:	
Denver, Colorado		SLURRY CONSOLIDATION TEST	
Job Short Title:		SAMPLE DATA AND CALCULATIONS	
Copper Flat Tailings Design Study			
Sample No.	Reviewed:	Date:	Job Number:
Whole Tailings Flume Test Beach	CCS	11/12/2012	103-92557
			Figure:
			1



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS

Job Short Title:
 Copper Flat Tailings Design Study

PERMEABILITY DATA

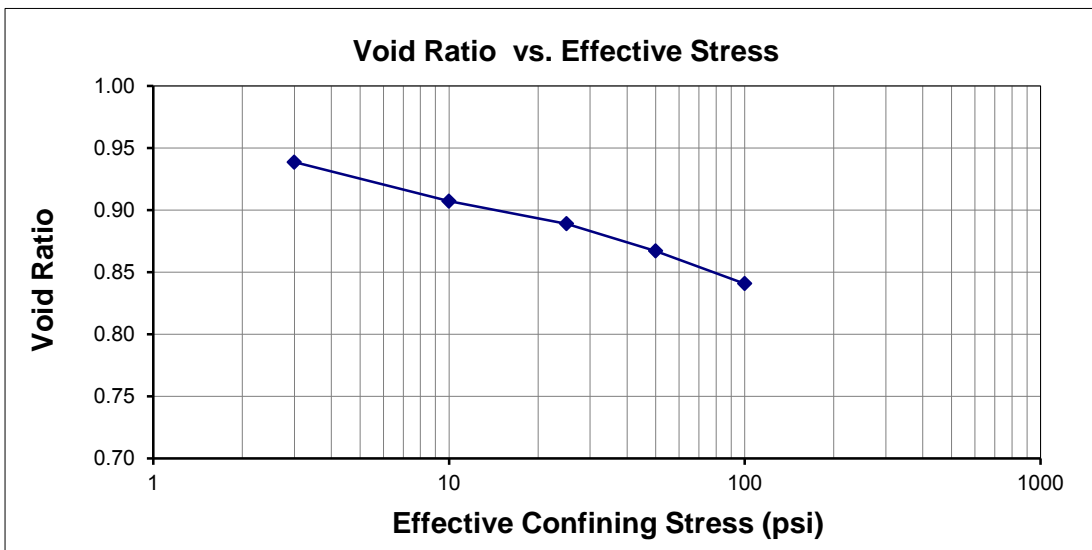
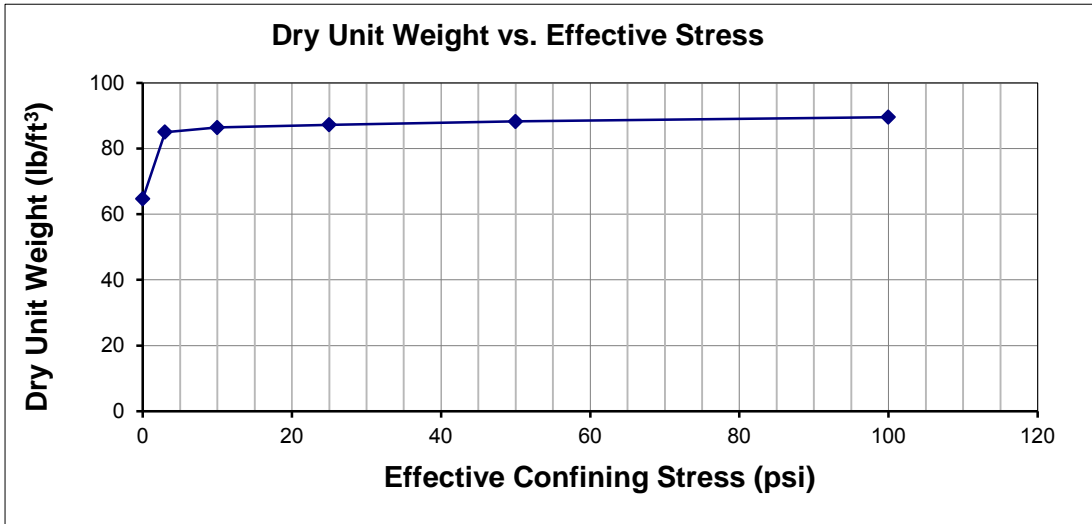
Sample No.
 Whole Tailings Flume Test Beach

Reviewed:
 CCS

Date:
 11/12/2012

Job Number:
 103-92557

Figure:
 2



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
DENSITY DATA

Job Short Title:
 Copper Flat Tailings Design Study

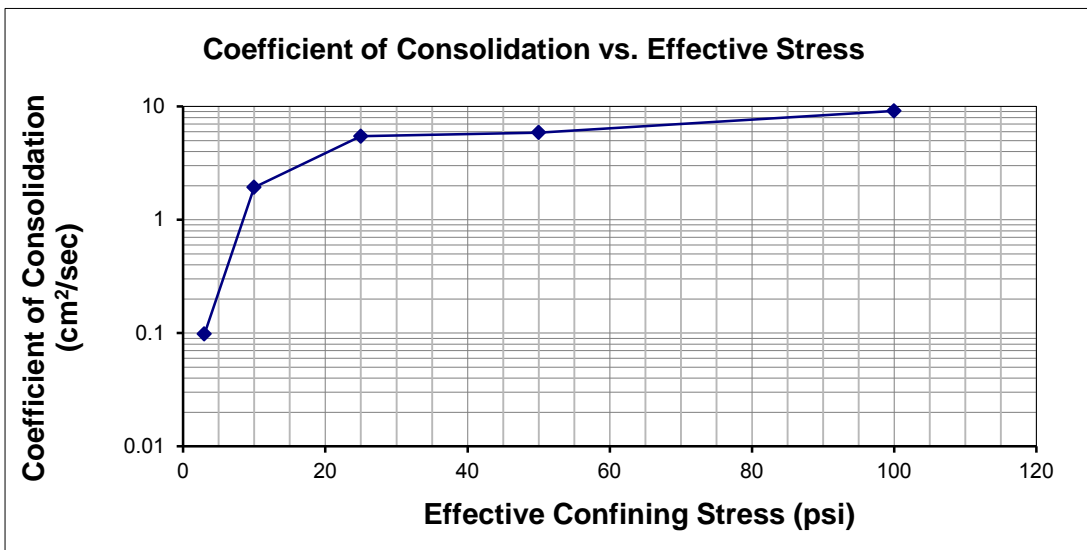
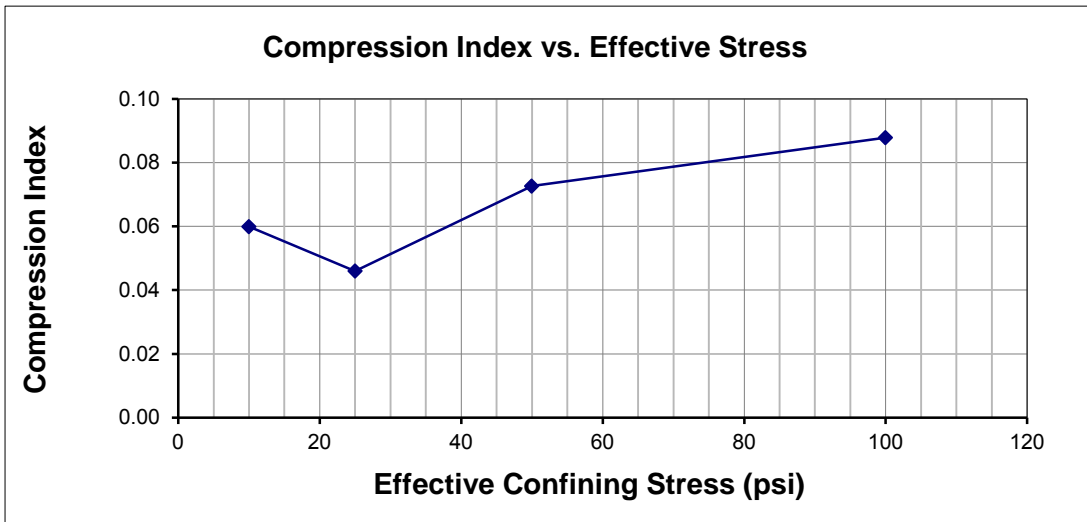
Sample No.
 Whole Tailings Flume Test Beach

Reviewed:
 CCS

Date:
 11/12/2012

Job Number:
 103-92557

Figure:
 3



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
COMPRESSION DATA

Job Short Title:
 Copper Flat Tailings Design Study

Sample No.
 Whole Tailings Flume Test Beach

Reviewed:
 CCS

Date:
 11/12/2012

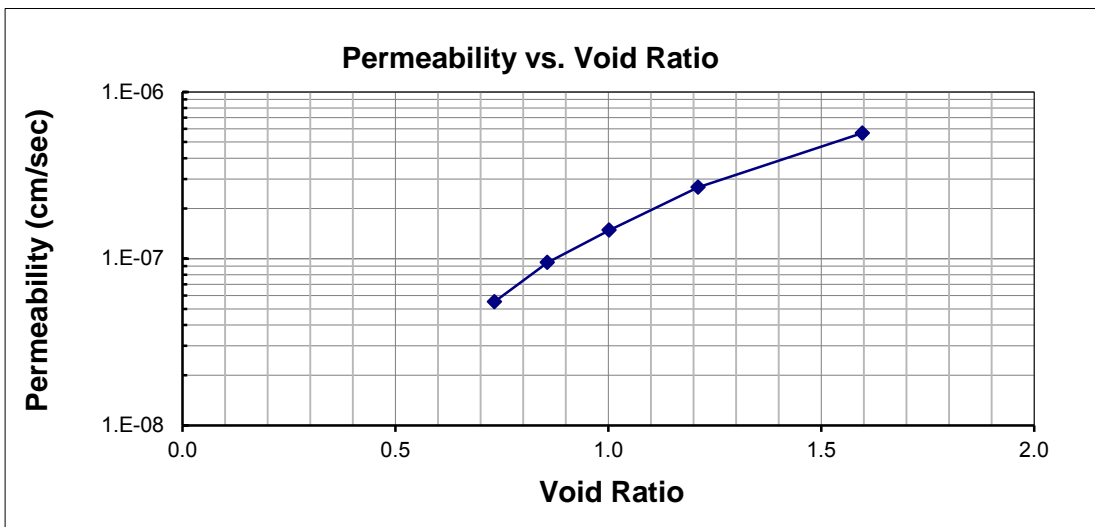
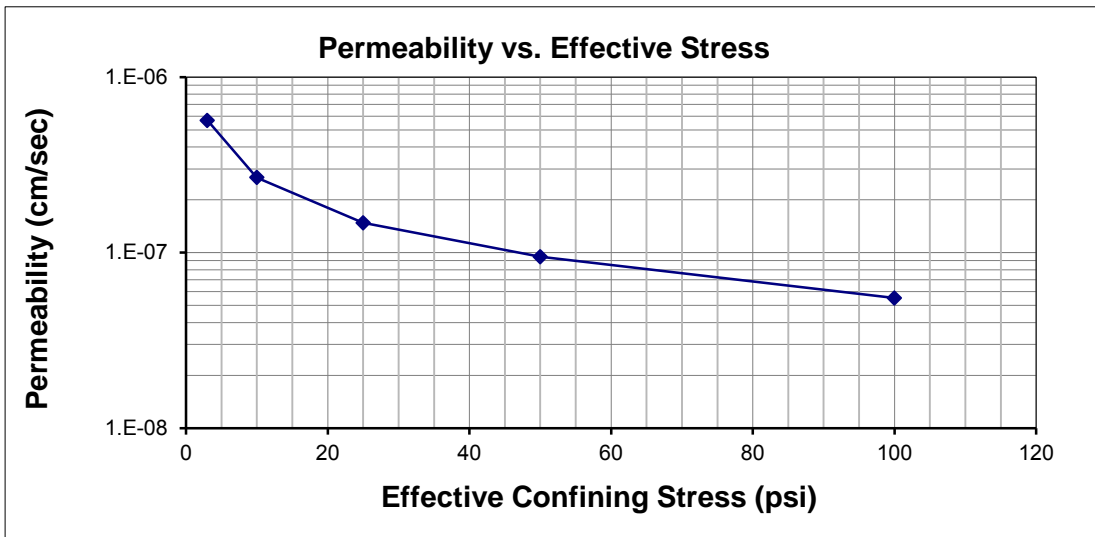
Job Number:
 103-92557

Figure:
 4

	Initial	Final	
Length =	7.285	0.740	cm
Diameter =	7.099	7.099	cm
Wet Mass =	338.65	60.46	g
Area =	39.58	39.58	cm ²
Volume =	288.3	29.29	cm ³
Moisture Content =	628.5%	35.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	46.49	44.62	g
Unit Weight =	1.174	2.064	g/cm ³
Dry Unit Weight =	0.1612	1.523	g/cm ³
Unit Weight =	73.32	128.6	lb/ft ³
Dry Unit Weight =	10.06	94.90	lb/ft ³
Percent Solids =	13.7%	73.8%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c _v (cm ² /sec)
204	5.66E-07	1.42E-04	367	2.67E-07	8.8E-04	604	1.47E-07	1.6E-03	533	9.48E-08	2.3E-03	1024	5.51E-08	2.9E-03
221	5.69E-07	1.43E-04	397	2.69E-07	8.9E-04	652	1.49E-07	1.7E-03	639	9.46E-08	2.3E-03	1233	5.52E-08	2.9E-03
245	5.65E-07	1.42E-04	440	2.67E-07	8.8E-04	718	1.48E-07	1.7E-03	770	9.47E-08	2.3E-03	1396	5.51E-08	2.9E-03
Average (of final 3 values)	5.66E-07	1.42E-04	Average (of final 3 values)	2.68E-07	8.86E-04	Average (of final 3 values)	1.48E-07	1.66E-03	Average (of final 3 values)	9.47E-08	2.30E-03	Average (of final 3 values)	5.51E-08	2.90E-03

Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST SAMPLE DATA AND CALCULATIONS			
Job Short Title: Copper Flat Tailings Design Study					
Sample No. Whole Tailings Flume Test Slime	Reviewed: CCS	Date: 10/30/2012	Job Number: 103-92557	Figure: 1	



Golder Associates Inc.
Denver, Colorado

Job Short Title:
 Copper Flat Tailings Design Study

Title:
SLURRY CONSOLIDATION TEST RESULTS
PERMEABILITY DATA

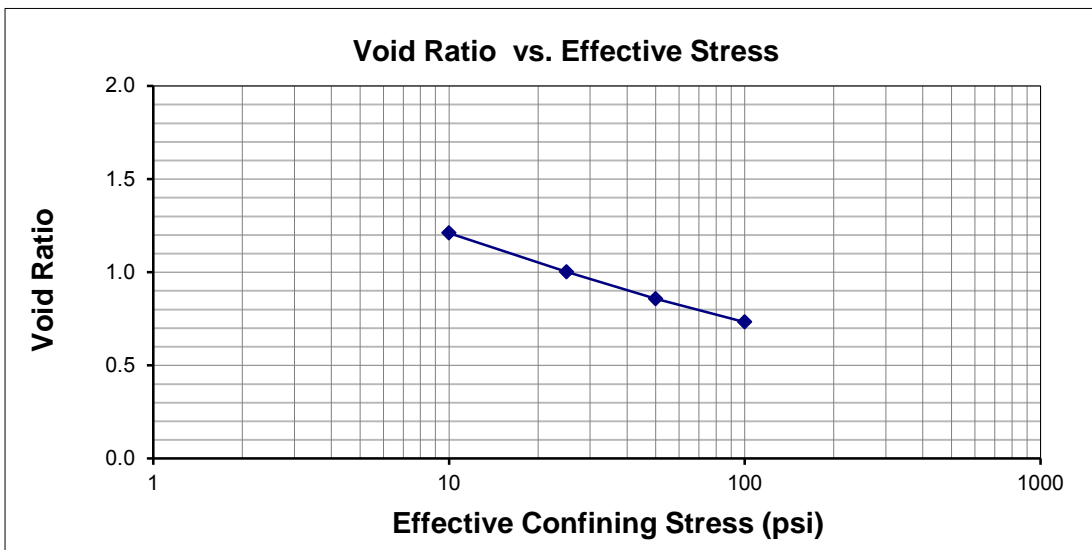
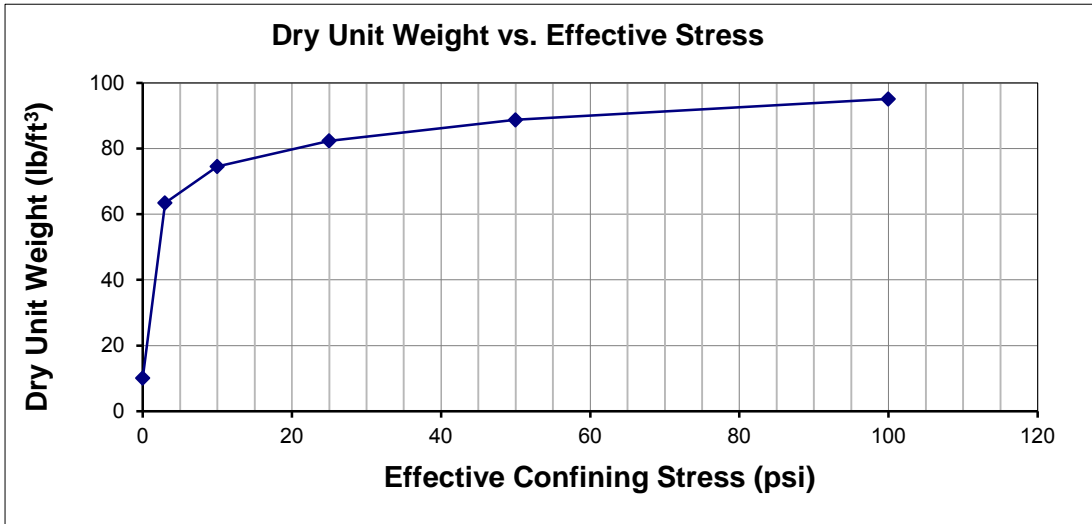
Sample No.
 Whole Tailings Flume Test Slime

Reviewed:
 CCS

Date:
 10/30/2012

Job Number:
 103-92557

Figure:
 2



Golder Associates Inc.
Denver, Colorado

Title:
SLURRY CONSOLIDATION TEST RESULTS
DENSITY DATA

Job Short Title:
 Copper Flat Tailings Design Study

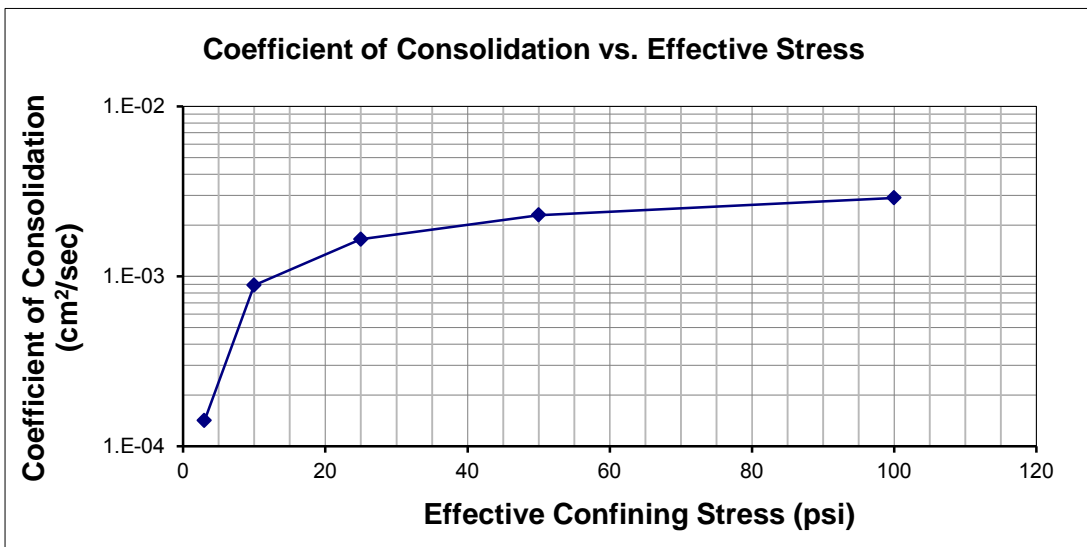
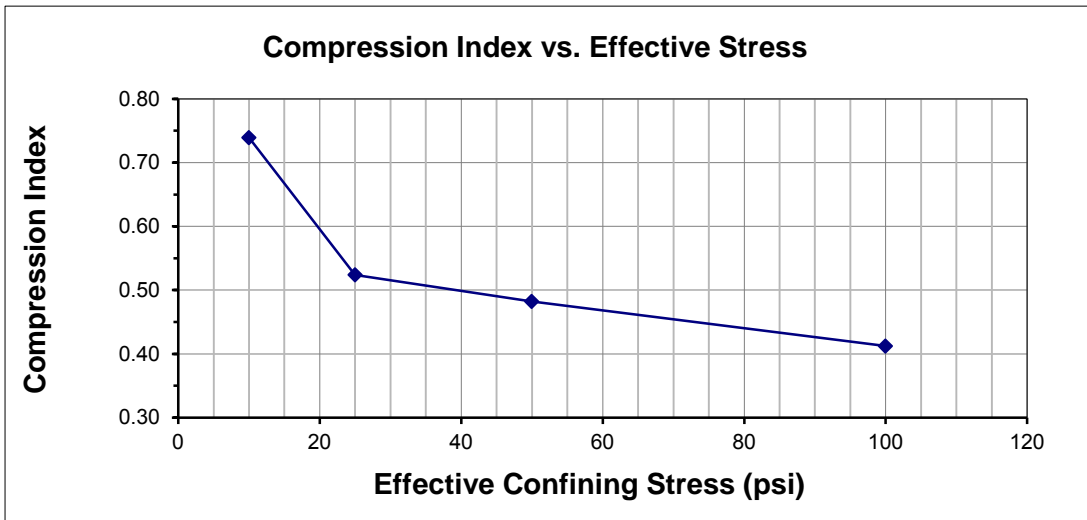
Sample No.
 Whole Tailings Flume Test Slime

Reviewed:
 CCS

Date:
 10/30/2012

Job Number:
 103-92557

Figure:
 3

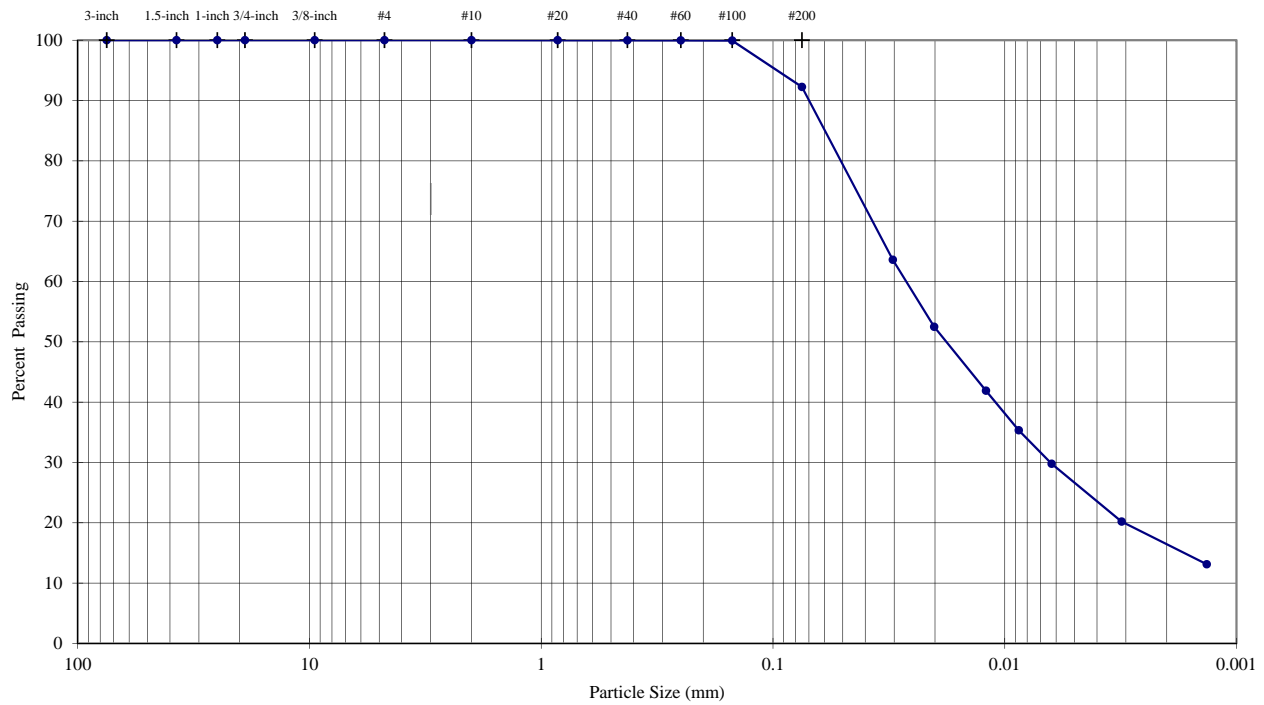


Golder Associates Inc. Denver, Colorado		Title: SLURRY CONSOLIDATION TEST RESULTS COMPRESSION DATA		
Job Short Title: Copper Flat Tailings Design Study				
Sample No. Whole Tailings Flume Test Slime	Reviewed: CCS	Date: 10/30/2012	Job Number: 103-92557	Figure: 4

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

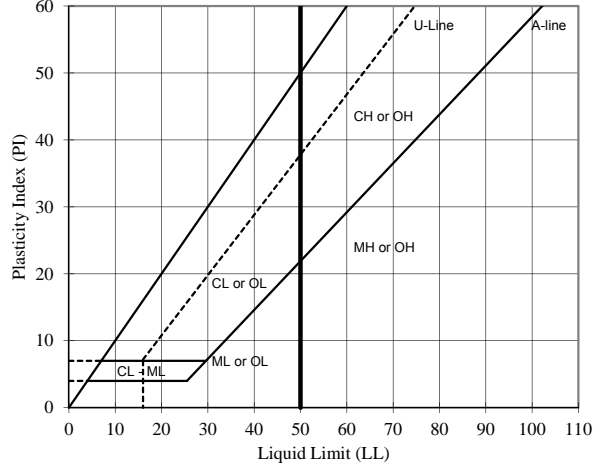
PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Whole Tailings Flume**
 TYPE: **Single Drain**

DEPTH (ft): **Beach**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.02
#40	0.425		
#60	0.25	Fine Sand	7.72
#100	0.15		
#200	0.075		
0.030	63.6	Silt or Clay Fines	92.26
0.020	52.5		
0.012	41.9		
0.009	35.3		
0.006	29.8		
0.003	20.2		
0.001	13.1		



Visual Description (Golder Procedure):
Wet, light yellowish brown SITLY SAND

LL	PL	PI	Spg (assumed)
--	--	--	2.64

As-Received Moisture Content (%) USCS Group Symbol
 -- --

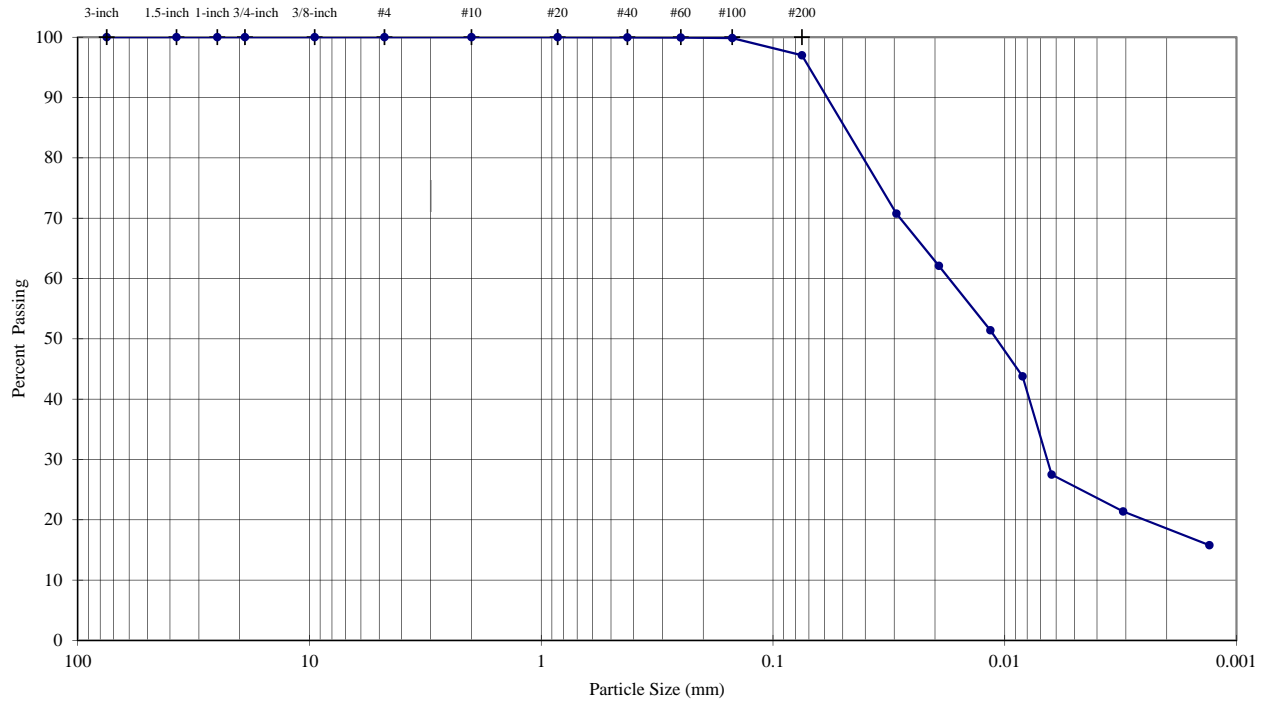
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH **RJM/SRS**
 DATE **11/8/2012**
 REVIEW **MB**

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

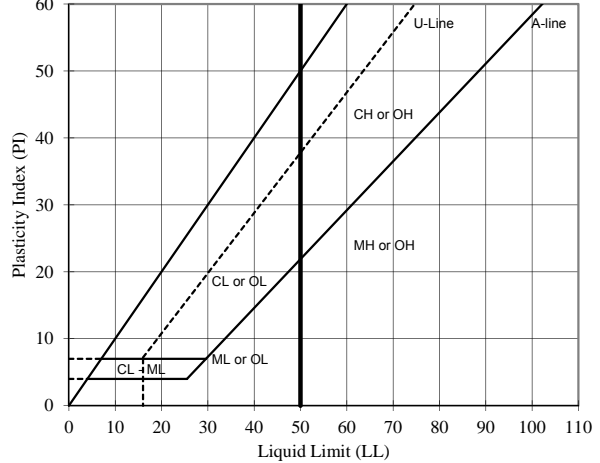
PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Whole Tailings**
 TYPE: **Flume**

DEPTH (ft): **Slime**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.04
#40	0.425		
#60	0.25	Fine Sand	2.96
#100	0.15		
#200	0.075		
	0.029	Silt or Clay Fines	97.00
	0.019		
	0.012		
	0.008		
	0.006		
	0.003		
	0.001		



Visual Description (Golder Procedure):
 Wet, light yellowish brown silty sand

LL	PL	PI	SpG (assumed)
--	--	--	2.64

As-Received Moisture Content (%) USCS Group Symbol
 -- --

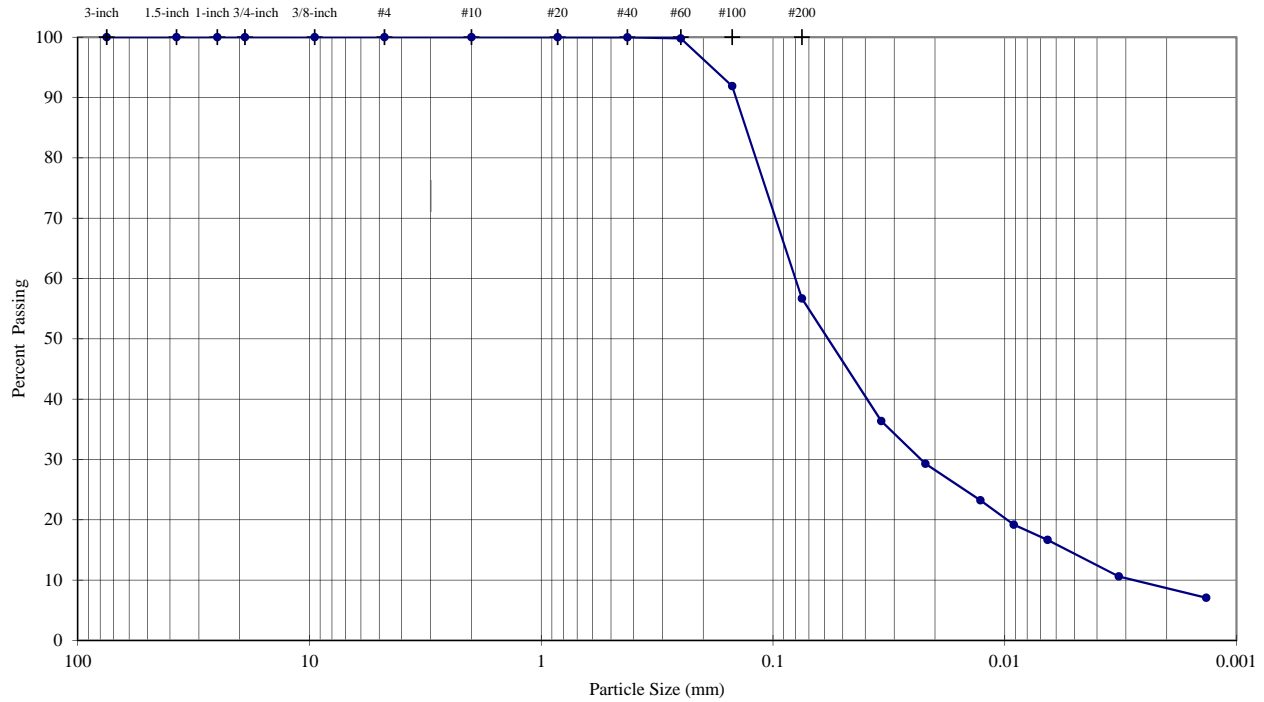
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH AM/SRS
 DATE 11/8/2012
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

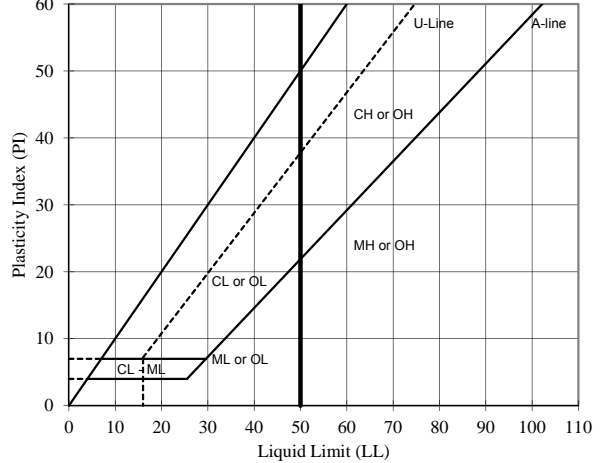
PROJECT NAME: **Copper Flat Tailings Design Study**
 SAMPLE ID: **Whole Tailings Flume**
 TYPE: **Single Drain**

DEPTH (ft): **Slime**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage		
3-inch	75.0	Coarse Gravel	0.00		
1.5-inch	37.5				
1-inch	25.0				
3/4-inch	19.0				
3/8-inch	9.5	Fine Gravel	0.00		
#4	4.75				
#10	2.0	Coarse Sand	0.00		
#20	0.85				
#40	0.425	Medium Sand	0.02		
#60	0.25				
#100	0.15	Fine Sand	43.29		
#200	0.075				
Hydrometer Analysis	0.034			Silt or Clay Fines	56.69
	0.022				
	0.013				
	0.009				
	0.007				
	0.003				
	0.001		7.1		



Visual Description (Golder Procedure):
Wet, light yellowish brown SITLY SAND

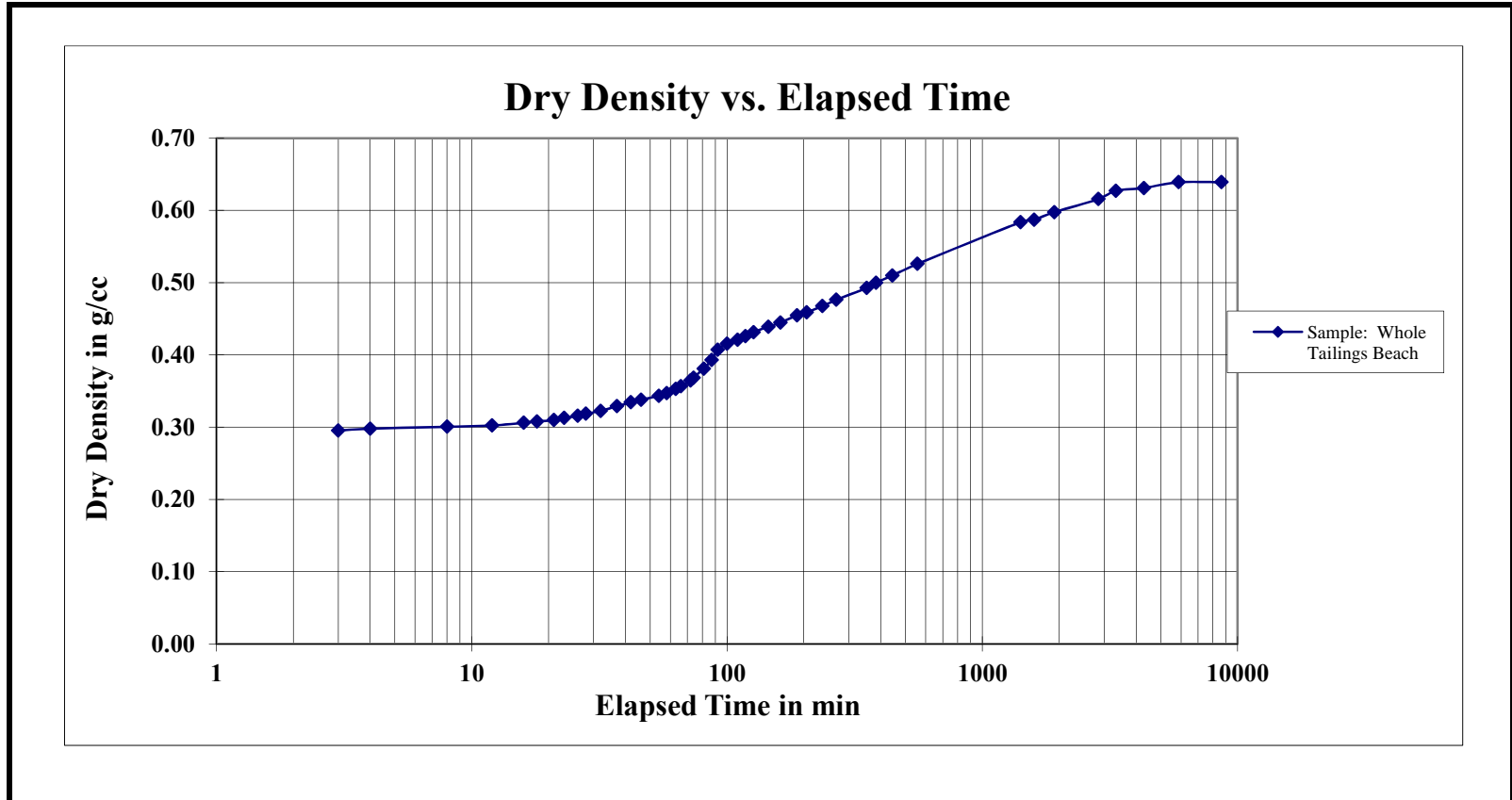
LL	PL	PI	SpG (assumed)
--	--	--	2.64

As-Received Moisture Content (%)
 --

USCS Group Symbol
 --

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH: **RJM/SRS**
 DATE: **11/8/2012**
 REVIEW: **MB**



Golder Associates, Inc. Denver, Colorado			Title: SEDIMENTATION TESTING GRAPHICAL DATA		
Job Short Title: Copper Flat Tailings Design Study					
Sample No. Whole Tailings Beach	System Single Drain	Reviewed: MB	Date: 23-Oct-12	Job Number: 103-92557	Figure: 2



APPENDIX C
FSCONSOL MODEL OUTPUT

Date:	November 11, 2013	Made by:	CDJ and DMW
Project No.:	133-92505	Checked by:	GM
Subject:	Cyclone Overflow Beach and Slimes Tailings Consolidation Analyses	Reviewed by:	GG
Project Short Title:	COPPER FLAT FEASIBILITY DESIGN		

1.0 OBJECTIVE

Estimate the Tailing Storage Facility (TSF) impoundment capacity by computing the average dry density of tailings at the end-of-filling using a one-dimensional (1D) large strain consolidation model to obtain the void ratio profile with depth. Use calculated 1D percent errors to compute an adjusted average dry density, which is then weighted based on the tailings cyclone overflow split between tailings “beach” versus tailings “slimes” within the TSF impoundment.

2.0 ASSUMPTIONS

- Total tailings production rate (overflow and underflow): 27,618 tons per day
- Total tailings capacity (overflow and underflow): 112 million tons
- Cyclone Availability: 100 percent (i.e. no whole tailings deposited in the TSF impoundment based on the recommendation to produce as much cyclone underflow tailings sand as possible)
- Percentage split of cyclone overflow tailings: 54.75 percent of total
- Tailings production rate (overflow): 15,121 tons per day (calculated based on total tailings production rate and percentage of cyclone overflow)
- Percentage split of slimes: 40 percent by mass of storage in TSF impoundment
- Initial Tailings Overflow Solids Content: 25.9 percent
- Tailings Overflow filling rate
 - Years 0-1:13.3 million kilograms per day
 - Years 1-5:15.4 million kilograms per day
 - Years 5-11:14.2 million kilograms per day
 - Years 11-11.12:13.2 million kilograms per day
- Impermeable Bottom Boundary condition



3.0 INPUTS

3.1 Geometry

Model geometry was based on the existing ground topography (THEMAC Resources, 2011) and Golder's optimized TSF design. Stage storage curve data used for TSF modeling is displayed in Table 1.

- Bottom Elevation: 5,196 feet above mean sea level (ft-msl)
- Top Dam Crest Elevation: 5,450 ft-msl
- Maximum depth of Impoundment: 254 feet
- Maximum Overflow Impoundment Area: 13.66 million square feet
- Impoundment volume used in the FSConsol analyses: 96.88 million cubic yards (MCY)

Table 1: Elevation–Area-Volume Relationship

Elevation (ft-msl)	Height (ft)	Area (acre)	Cumulative Volume (yd ³)
5,196	0	0.0	0
5,210	14	16.3	367,859
5,225	29	58.6	1,786,931
5,235	39	101.7	3,427,369
5,250	54	141.8	6,859,887
5,265	69	180.3	11,223,775
5,280	84	203.4	16,145,709
5,295	99	240.6	21,968,132
5,310	114	255.1	28,141,570
5,325	129	280.1	34,919,345
5,340	144	288.9	41,911,903
5,355	159	300.0	49,171,933
5,370	174	305.0	56,552,360
5,385	189	308.6	64,019,422
5,400	204	312.6	71,583,431
5,425	229	313.6	84,231,795
5,435	239	313.6	89,291,466
5,450	254	313.6	96,880,975

3.2 Material Properties

Material properties and numerical model inputs are based on laboratory tests performed by Golder Associates Inc. in Denver, CO. Slurry consolidation tests were performed to determine void ratio and permeability versus applied effective stresses. The specific gravity of the tailings material is 2.65 based on laboratory tests.

4.0 METHOD

Calculations were performed using the computer program FSConsol (GWP Software, 1999), which performs a one-dimensional, large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967).

4.1 Numerical Model

4.1.1 Equations

For modeling purposes, it is convenient to express constitutive relationships (permeability and compressibility, respectively) in a closed form. Non-linear relationships are proposed by Abu-Hejleh and Znidarcic (1994 and 1996), defined by Equations 4.1 and 4.2, which are used in consolidation and desiccation numerical models.

$$k = C e^D \quad \text{Equation 4.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 4.2}$$

When using FSConsol model, Equation 4.1 remains the same; however, Equation 4.2 is rewritten by the modified power law form to represent compressibility, shown in Equation 4.3.

$$e = A(\sigma')^B + M \quad \text{Equation 4.3}$$

In the above relationships, e is the void ratio, σ' is the effective stress, and k is hydraulic conductivity functionally dependent on void ratio.

4.1.2 Parameters

The five material parameters, A , B , C , D , and M (or Z), were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 through 4, which are on the following next two pages (pages 4 and 5). The fitted parameters calculated for those constitutive relationships are shown in Tables 2 and 3 for different systems of units.

Table 2: Permeability Input Parameters for Cyclone Overflow and Whole Tailings

Sample	C (centimeters per second)	D (dimensionless) ¹
Slimes	1.380×10^{-7}	3.353
Beach	1.523×10^{-6}	3.035

Table 3: Compressibility Input Parameters for Cyclone Overflow and Whole Tailings

Sample	A (1/kilopascals) ^B	B (dimensionless) ¹	M (dimensionless) ¹
Slimes	3.144	-0.1952	-0.1424
Beach	1.787	-0.2983	0.5224

Note:

¹ B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second (cgs) units

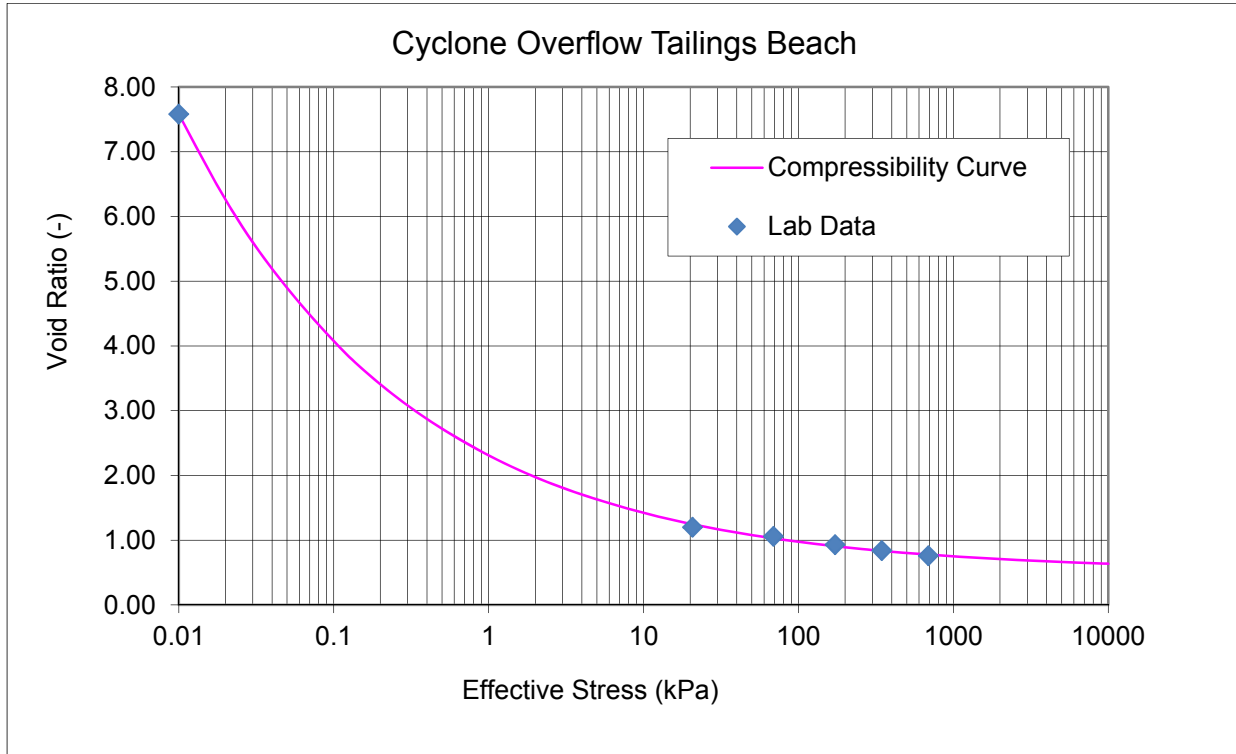


Figure 1: Cyclone Overflow Tailings Beach Compressibility Constitutive Relationship versus Lab Data

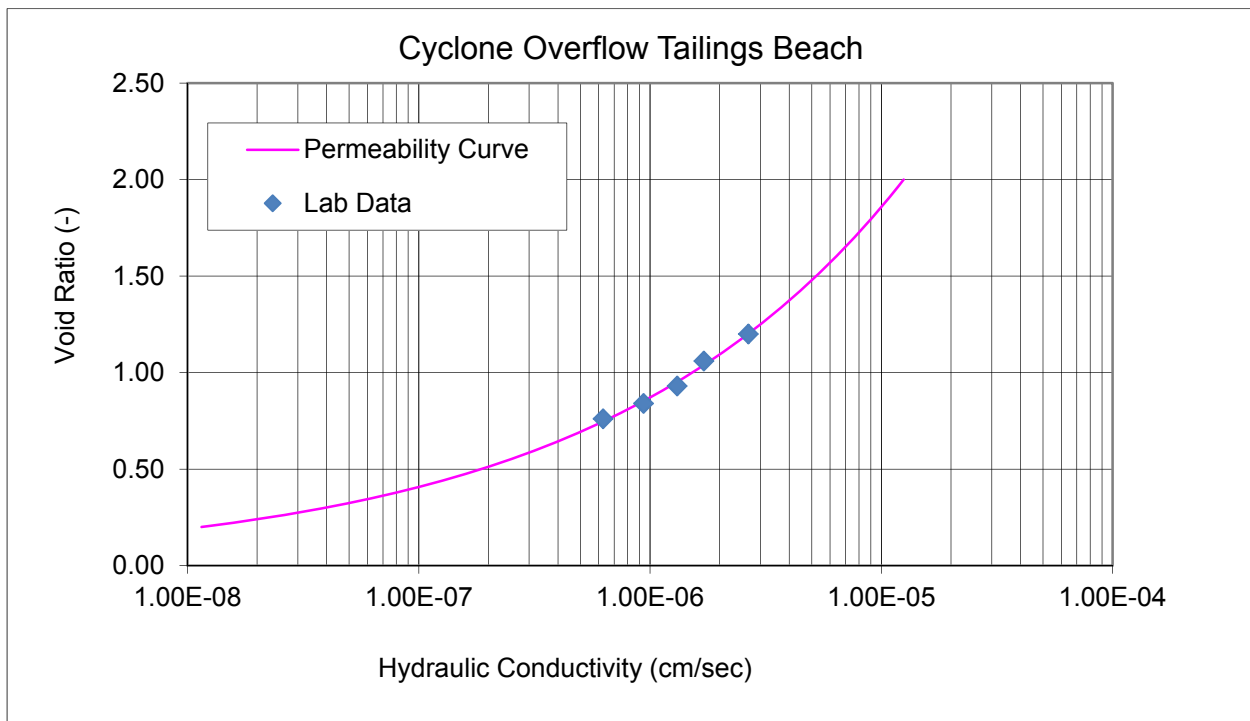


Figure 2: Cyclone Overflow Tailings Beach Permeability Constitutive Relationship versus Lab Data

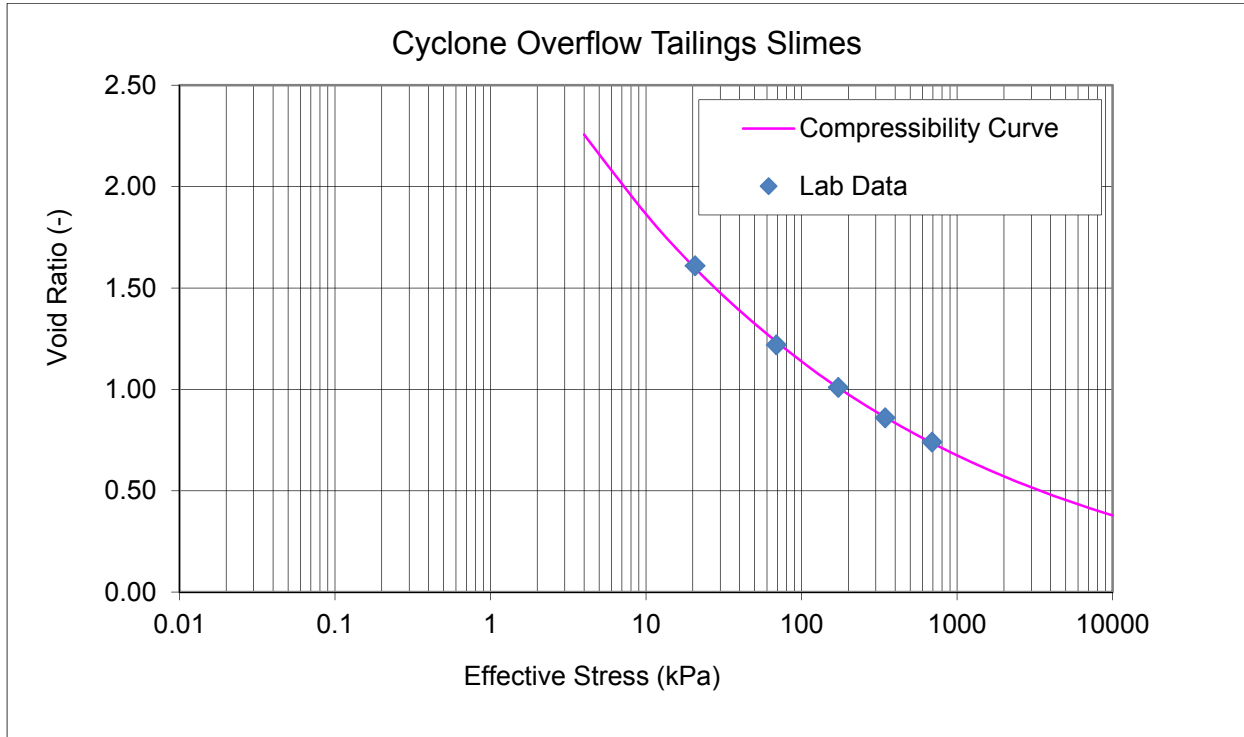


Figure 3: Cyclone Overflow Tailings Slimes Compressibility Constitutive Relationship versus Lab Data

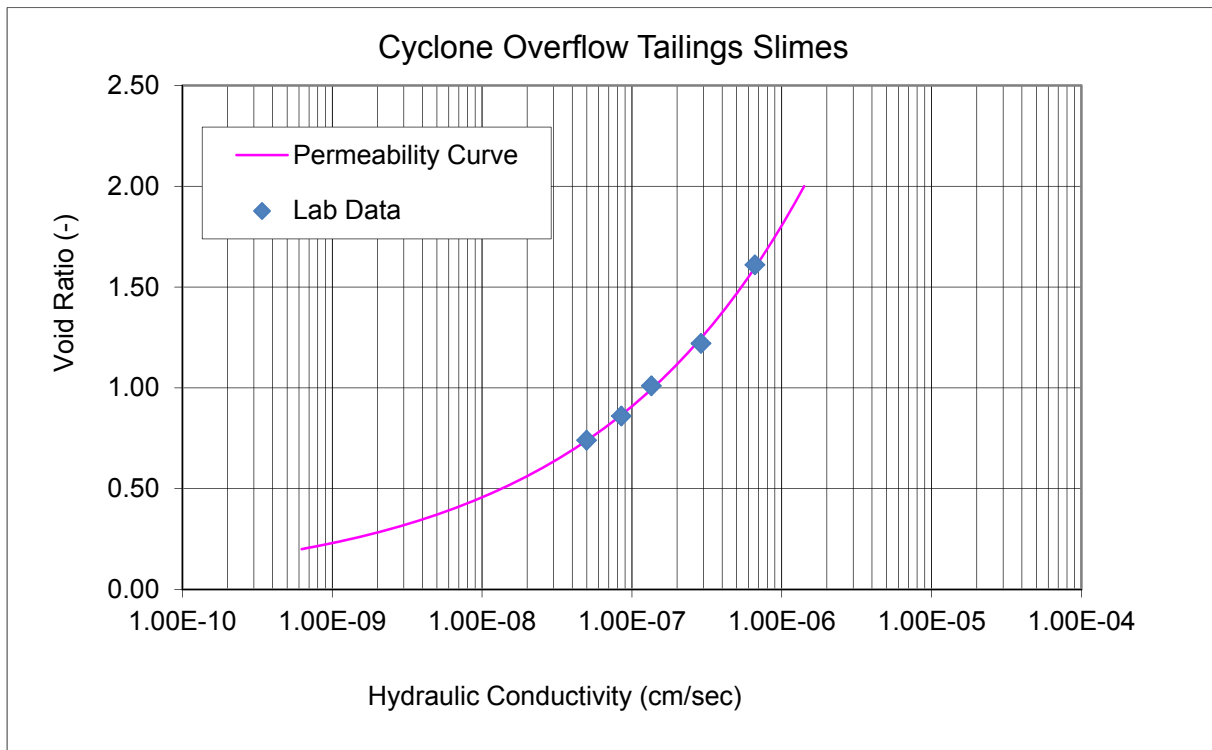


Figure 4: Cyclone Overflow Tailings Slimes Permeability Constitutive Relationship versus Lab Data

4.2 Procedure

The tailings cyclone overflow portion of the TSF was modeled by running a consolidation analysis on a single tailings column located in the deepest part of the impoundment. Using the parameters described above in Section 4.1.2 to define the compressibility and permeability relationships, separate analyses were performed for each material representing either the tailings beach or tailings slimes components of the cyclone overflow.

In each respective model run, the model analyzed each material component assuming it to represent 100 percent of the inflow into the model storage volume (i.e. the impoundment). In the model, the incremental area was calculated based on elevation and volume differences with Golder's TSF design volumes determined using AutoCAD Civil 3D (2013). The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities from the final void ratio profile of each tailings component to obtain an overall density that is weighted based on laboratory grain-size distribution results, which is detailed further in Section 4.2.1. Furthermore, the calculated densities based on the FSConsol analyses are adjusted for 1D percent errors as discussed in Section 4.2.2.

4.2.1 Overflow Beach/Slimes Split

Grain-size distribution tests were performed by Golder in the Denver, CO laboratory on tailings beach and slime samples from laboratory flume tests and on total overflow tailings before the flume tests. From the grain-size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the tailings beach and tailings slime that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the tailings beach and tailings slimes samples should equate to the percentage for the original cyclone overflow head sample. Table 4 depicts the results of the laboratory test and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes).

Table 4: Laboratory and Split Results

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids by Weight	% Sand of Total Overflow	% Fines of Total Overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head	9.63	90.37	100	9.63	90.37

4.2.2 1D Percent Error

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall geometry of the TSF (i.e. "bowl" shaped), and that mass and volume quantities are larger towards the top than at the bottom of the TSF due to an increase in surface area. The approximated average dry density of each tailings component is corrected based on the calculated average percent error between 1D versus three-dimensional (3D) by checking: height of solids, volume of solids, and mass of solids.

Based on Golder's experience, 1D analysis resulting in less than an average 15 percent error reasonably depicts expected consolidation in the field, and 3D impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error.

5.0 RESULTS

The average dry density generally increases during filling until the ultimate height of tailings is reached. At closure, the tailings density continues to increase as the consolidation continues. If 100 percent of the inflow into the TSF impoundment is represented by the cyclone overflow tailings slimes samples from the tail section of the flume (i.e., the finest fraction of the tailings), FSConsol predicts that the final average density of the slimes fraction over the modeled profile will be approximately 33.55 pcf. With 100 percent of the inflow representative of cyclone overflow tailings beach materials, FSConsol predicts a final average tailings density of approximately 76.65 pcf. Attachment 1 contains FSConsol outputs for each respective material properties used in the consolidation analysis.

Because a 1D model was used, percent error calculations were performed to account for 3D effects. The error is expected to be greater for cone-shaped impoundments than for flat-based (i.e. bowl shaped) impoundments, and greater with higher rates of rise than lower rates of rise. Based on calculated material properties, the TSF geometry, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1.1 to 13.7 percent with an average error ranging between 2.7 to 5.4 percent when considering calculated impoundment capacities. Attachment 2 contains the error calculations for each respective cyclone overflow tailings components.

After accounting for 3D characteristics of the impoundment geometry, the recommended dry density of each tailings component is 31.7 and 74.6 pcf, respectively. In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components.

6.0 CONCLUSIONS

Based on model results and error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow tailings slimes and cyclone overflow tailings beach materials at the end-of-filling. Capacity has been estimated with 40 percent of the total represented by tailings slime materials, which is based on the calculated beach to slime ratio split discussed previously in Section 4.2.1. Therefore without consideration of 3D and managed deposition effects, the calculated weighted density of tailings slimes and tailings beach based on each component's volume within the TSF impoundment is 48.4 pcf. At this density, the TSF will require approximately 93.8 MCY of storage, which can be achieved by building to an ultimate tailings surface elevation of 5450 ft-msl and an ultimate cyclone underflow tailings sand dam crest elevation of 5460 ft-msl.

A summary for the cyclone overflow tailings beach and slimes components is located in Attachment 3, which contains the percent errors, adjusted densities, corresponding volumes, and the weighted density based on the percentage of the overall volume occupied by that tailings materials.

3D and managed deposition effects could potentially increase the post deposition density to reduce storage volume requirements. On average, a density increase in 5 pcf results in approximately 10 MCY of additional TSF impoundment storage capacity and a decrease in ultimate crest elevation by approximately 20 feet.

Actual flows into the impoundment will be mixed, with cyclone overflow tailings beach inter-fingered with slimes layers as the decant pond migrates upstream away from the embankment and points of cyclone overflow discharge are relocated. The denser and more permeable beach layers will increase the rate of consolidation in the underlying slimes material, resulting in a higher rate of consolidation than that calculated in the model. In addition, the consolidation modeling does not account for managed deposition of tailings. Thin-lift, sub-aerial deposition techniques will result in increases of dry density and a decrease in required storage volume by enhancing evaporation of exposed tailings. Managed deposition will increase consolidation of both the cyclone overflow tailings beach and slimes. However due to the height of the proposed tailings embankment and the design out-slopes, an embankment raise of several feet will also be feasible near the end of the mining operation if additional capacity is needed.

7.0 REFERENCES

- Abu-Hejleh, A.N., and D. Znidarcic, 1994. *Estimation of the Consolidation Constitutive Relations*, Computer Methods and Advances in Geomechanics, Siriwardane and Zaman (eds), Balkema, Rotterdam, pp. 499-504.
- Abu-Hejleh, A.N., and D. Znidarcic, 1996. *Consolidation Characteristics of Phosphatic Clays*, Journal of Geotechnical Engineering", ASCE, New-York, Vol. 122, No. 4, pp. 295-301.
- AutoCAD Civil 3D Package, Version 2013.
- Gibson, R.E., England, G. L., and Hussey, M. J. L. (1967). The Theory of One-Dimensional Consolidation of Saturated Clays. *Geotechnique*, 17: 261-273.
- GWP Software, 1999. *FSConsol User's Manual*, GWP Software Inc.
- THEMAC Resources, 2011. *2-Foot Topography*. Developed. and provided by Cooper Aerial Survey Co., June 18, 2011.

Attachments: Attachment 1- FSConsol Model Output
Attachment 2 - FSConsol Error Checking
Attachment 3 - FSConsol Model Summary

**ATTACHMENT 1
FSCONSOL MODEL OUTPUT**

FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.
Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 12:58:34
input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-slimes.fs1

Overflow - slimes (Non-thickened)
Revised flow rates and pond areas 11/11/2013
CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond
Analysis Time = 16.115 years
Maximum Pond Height = 77.42 meters
Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 3.144; B = -0.1952 ; Mconst = -0.1424

Permeability Parameters (cm/second)

C = 1.38e-07; D = 3.353

BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	8.952323	27.665	99.338	0.662	4.84E+08	32.2474
73	11.974489	28.534	99.338	0.662	9.69E+08	23.1181
109.5	13.962894	29.275	99.29	0.71	1.45E+09	23.1181
146	15.951077	29.826	99.268	0.732	1.94E+09	23.1181
182.5	17.505217	30.356	99.233	0.767	2.42E+09	18.1828
219	18.909844	30.834	99.199	0.801	2.91E+09	18.1828
255.5	20.314418	31.243	99.176	0.824	3.39E+09	18.1828
292	21.599814	31.627	99.153	0.847	3.87E+09	16.1217
328.5	22.7607	31.998	99.128	0.872	4.36E+09	16.1217
365	23.92199	32.331	99.109	0.891	4.84E+09	18.7619
401.5	25.394898	32.553	99.113	0.887	5.41E+09	18.7619
438	26.573655	32.822	99.103	0.897	5.97E+09	15.86
474.5	27.703532	33.079	99.093	0.907	6.53E+09	15.86
511	28.833398	33.316	99.085	0.915	7.10E+09	15.86
547.5	29.963253	33.533	99.08	0.92	7.66E+09	15.86
584	31.006515	33.755	99.073	0.927	8.23E+09	14.9583
620.5	32.029764	33.966	99.066	0.934	8.79E+09	14.9583
657	33.053007	34.164	99.06	0.94	9.35E+09	14.9583
693.5	34.076244	34.349	99.056	0.944	9.92E+09	14.9583
730	35.045301	34.535	99.051	0.949	1.05E+10	13.6253
766.5	35.910971	34.735	99.042	0.958	1.10E+10	13.6253
803	36.776635	34.924	99.035	0.965	1.16E+10	13.6253
839.5	37.642296	35.105	99.028	0.972	1.22E+10	13.6253
876	38.507953	35.276	99.023	0.977	1.27E+10	13.6253
912.5	39.37049	35.441	99.018	0.982	1.33E+10	13.207
949	40.186694	35.608	99.012	0.988	1.39E+10	13.207
985.5	41.002895	35.768	99.007	0.993	1.44E+10	13.207
1022	41.819093	35.922	99.002	0.998	1.50E+10	13.207
1058.5	42.635291	36.069	98.998	1.002	1.56E+10	13.207
1095	43.4515	36.211	98.995	1.005	1.61E+10	13.207
1131.5	44.241255	36.353	98.991	1.009	1.67E+10	12.7196
1168	45.000136	36.496	98.987	1.013	1.72E+10	12.7196
1204.5	45.759528	36.633	98.982	1.018	1.78E+10	12.7196
1241	46.519832	36.766	98.979	1.021	1.84E+10	12.7196
1277.5	47.281436	36.893	98.975	1.025	1.89E+10	12.7196
1314	48.04463	37.015	98.972	1.028	1.95E+10	12.7196
1350.5	48.798638	37.134	98.969	1.031	2.01E+10	12.5125
1387	49.541361	37.25	98.966	1.034	2.06E+10	12.5125
1423.5	50.286082	37.362	98.963	1.037	2.12E+10	12.5125
1460	51.032752	37.47	98.961	1.039	2.18E+10	12.5125
1496.5	51.7813	37.573	98.959	1.041	2.23E+10	12.5125

1533	52.53164	37.672	98.957	1.043	2.29E+10	12.5125
1569.5	53.278167	37.768	98.956	1.044	2.34E+10	12.3672
1606	54.015293	37.863	98.954	1.046	2.40E+10	12.3672
1642.5	54.754054	37.954	98.953	1.047	2.46E+10	12.3672
1679	55.494353	38.041	98.952	1.048	2.51E+10	12.3672
1715.5	56.236103	38.126	98.951	1.049	2.57E+10	12.3672
1752	56.979224	38.207	98.951	1.049	2.63E+10	12.3672
1788.5	57.720815	38.286	98.951	1.049	2.68E+10	12.2088
1825	58.448575	38.365	98.95	1.05	2.74E+10	11.2472
1861.5	59.070262	38.461	98.946	1.054	2.79E+10	11.2472
1898	59.694243	38.553	98.943	1.057	2.84E+10	11.2472
1934.5	60.319981	38.642	98.94	1.06	2.89E+10	11.2472
1971	60.947381	38.728	98.937	1.063	2.95E+10	11.2472
2007.5	61.576356	38.812	98.934	1.066	3.00E+10	11.2472
2044	62.206642	38.893	98.932	1.068	3.05E+10	11.21
2080.5	62.834453	38.972	98.929	1.071	3.10E+10	11.21
2117	63.463654	39.049	98.927	1.073	3.15E+10	11.21
2153.5	64.094161	39.124	98.926	1.074	3.21E+10	11.21
2190	64.725911	39.197	98.924	1.076	3.26E+10	11.21
2226.5	65.358846	39.268	98.923	1.077	3.31E+10	11.21
2263	65.992908	39.337	98.921	1.079	3.36E+10	11.21
2299.5	66.628047	39.404	98.92	1.08	3.41E+10	11.21
2336	67.264213	39.469	98.919	1.081	3.47E+10	11.21
2372.5	67.90136	39.532	98.919	1.081	3.52E+10	11.21
2409	68.539445	39.594	98.918	1.082	3.57E+10	11.21
2445.5	69.178426	39.654	98.918	1.082	3.62E+10	11.21
2482	69.818263	39.713	98.917	1.083	3.67E+10	11.2094
2518.5	70.458862	39.77	98.917	1.083	3.73E+10	11.2094
2555	71.100248	39.826	98.917	1.083	3.78E+10	11.2094
2591.5	71.742389	39.88	98.917	1.083	3.83E+10	11.2094
2628	72.385254	39.933	98.917	1.083	3.88E+10	11.2094
2664.5	73.028814	39.985	98.917	1.083	3.93E+10	11.2094
2701	73.67304	40.036	98.918	1.082	3.99E+10	11.2094
2737.5	74.317906	40.085	98.918	1.082	4.04E+10	11.2094
2774	74.963389	40.134	98.919	1.081	4.09E+10	11.2094
2810.5	75.609464	40.181	98.919	1.081	4.14E+10	11.2094
2847	76.256109	40.227	98.92	1.08	4.19E+10	11.2094
2883.5	76.903302	40.272	98.921	1.079	4.24E+10	11.2094

Observation Point Data

Number of Observation Points: 50
Number of Observation Point Readings: 80
Observation Point Time Step: 36.50 days

EXCESS PORE PRESSURE (kPa)

Time Height from Bottom (metres)

Table with 40 columns representing observation points and 80 rows representing time steps. The table contains numerical data for excess pore pressure at various heights from the bottom over time.

NORMALIZED EXCESS PORE PRESSURE (%)

Time Height from Bottom (metres)

Table with 40 columns representing observation points and 80 rows representing time steps. The table contains numerical data for normalized excess pore pressure at various heights from the bottom over time.

Table with 100 columns of numerical data, likely representing spatial coordinates or sensor readings across a grid.

VOID RATIO e

Table with 100 columns and 100 rows, showing VOID RATIO e values. The first two columns are 'Time' and 'Height from Bottom (metres)'. The rest of the table contains numerical data for each time-height combination.

Table with 27 columns of numerical data representing various parameters across different categories.

EFFECTIVE STRESS (kPa)

Large table with 28 columns (Time, Height from Bottom, and 26 days) showing effective stress values for each day across different heights.

PERMEABILITY (cm/second)

Table with 28 columns (Time, Height from Bottom, and 26 days) showing permeability values for each day across different heights.

FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.
 Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 14:40:56
 input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-beach.fs1

Overflow - beach (Non-thickened)
 Revised flow rates and pond areas 11/11/2013
 CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond
 Analysis Time = 16.115 years
 Maximum Pond Height = 77.42 meters
 Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 1.787; B = -0.2983; Mconst = 0.5224

Permeability Parameters (cm/second)

C = 1.523e-06; D = 3.035

BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	6.098115	39.528	96.536	3.464	4.84E+08	55.9199
73	8.737703	43.656	95.973	4.027	9.69E+08	55.9199
109.5	9.507187	47.682	94.382	5.618	1.45E+09	32.2474
146	10.476951	50.081	93.328	6.672	1.94E+09	32.2474
182.5	11.558634	51.655	92.671	7.329	2.42E+09	32.2474
219	12.236617	53.232	91.655	8.345	2.91E+09	23.1181
255.5	12.810342	54.565	90.672	9.328	3.39E+09	23.1181
292	13.436283	55.614	89.868	10.132	3.87E+09	23.1181
328.5	14.096505	56.462	89.212	10.788	4.36E+09	23.1181
365	14.780841	57.164	88.681	11.319	4.84E+09	26.904
401.5	15.714982	57.566	88.64	11.36	5.41E+09	26.904
438	16.566093	58.003	88.43	11.57	5.97E+09	21.1605
474.5	17.159336	58.556	87.926	12.074	6.53E+09	21.1605
511	17.773119	59.023	87.525	12.475	7.10E+09	21.1605
547.5	18.397756	59.433	87.186	12.814	7.66E+09	21.1605
584	19.030285	59.798	86.897	13.103	8.23E+09	21.1605
620.5	19.668975	60.125	86.649	13.351	8.79E+09	21.1605
657	20.312613	60.421	86.438	13.562	9.35E+09	21.1605
693.5	20.960293	60.689	86.258	13.742	9.92E+09	21.1605
730	21.488683	61.011	85.902	14.098	1.05E+10	18.7619
766.5	22.017166	61.298	85.604	14.396	1.10E+10	18.7619
803	22.551957	61.559	85.34	14.66	1.16E+10	18.7619
839.5	23.091308	61.8	85.105	14.895	1.22E+10	18.7619
876	23.634322	62.023	84.893	15.107	1.27E+10	18.7619
912.5	24.180393	62.229	84.703	15.297	1.33E+10	18.7619
949	24.729068	62.422	84.532	15.468	1.39E+10	18.7619
985.5	25.279985	62.603	84.378	15.622	1.44E+10	18.7619
1022	25.762592	62.813	84.108	15.892	1.50E+10	15.86
1058.5	26.168928	63.039	83.774	16.226	1.56E+10	15.86
1095	26.583726	63.244	83.478	16.522	1.61E+10	15.86
1131.5	27.003419	63.436	83.204	16.796	1.67E+10	15.86
1168	27.426795	63.616	82.95	17.05	1.72E+10	15.86
1204.5	27.853193	63.785	82.711	17.289	1.78E+10	15.86
1241	28.282173	63.946	82.487	17.513	1.84E+10	15.86
1277.5	28.713408	64.098	82.277	17.723	1.89E+10	15.86
1314	29.146638	64.242	82.08	17.92	1.95E+10	15.86
1350.5	29.581652	64.38	81.894	18.106	2.01E+10	15.86
1387	30.018268	64.511	81.72	18.28	2.06E+10	15.86
1423.5	30.42497	64.651	81.498	18.502	2.12E+10	14.9583
1460	30.82099	64.787	81.28	18.72	2.18E+10	14.9583
1496.5	31.21996	64.915	81.079	18.921	2.23E+10	14.9583

1533	31.620997	65.037	80.889	19.111	2.29E+10	14.9583
1569.5	32.023724	65.154	80.709	19.291	2.34E+10	14.9583
1606	32.42791	65.266	80.539	19.461	2.40E+10	14.9583
1642.5	32.833389	65.373	80.378	19.622	2.46E+10	14.9583
1679	33.24003	65.477	80.225	19.775	2.51E+10	14.9583
1715.5	33.647726	65.576	80.079	19.921	2.57E+10	14.9583
1752	34.056384	65.671	79.941	20.059	2.63E+10	14.9583
1788.5	34.465926	65.764	79.81	20.19	2.68E+10	14.9583
1825	34.852455	65.864	79.636	20.364	2.74E+10	12.5521
1861.5	35.144936	65.994	79.325	20.675	2.79E+10	12.5521
1898	35.447468	66.11	79.059	20.941	2.84E+10	12.5521
1934.5	35.754018	66.22	78.81	21.19	2.89E+10	12.5521
1971	36.063325	66.325	78.572	21.428	2.95E+10	12.5521
2007.5	36.374798	66.426	78.345	21.655	3.00E+10	12.5521
2044	36.688086	66.523	78.126	21.874	3.05E+10	12.5521
2080.5	37.002952	66.616	77.914	22.086	3.10E+10	12.5521
2117	37.319222	66.706	77.71	22.29	3.15E+10	12.5521
2153.5	37.636758	66.793	77.513	22.487	3.21E+10	12.5521
2190	37.955451	66.877	77.322	22.678	3.26E+10	12.5521
2226.5	38.275209	66.958	77.137	22.863	3.31E+10	12.5521
2263	38.59595	67.038	76.958	23.042	3.36E+10	12.5521
2299.5	38.917606	67.114	76.785	23.215	3.41E+10	12.5521
2336	39.240116	67.189	76.617	23.383	3.47E+10	12.5521
2372.5	39.548675	67.267	76.425	23.575	3.52E+10	12.1668
2409	39.855497	67.342	76.241	23.759	3.57E+10	12.1668
2445.5	40.163729	67.415	76.064	23.936	3.62E+10	12.1668
2482	40.473023	67.485	75.893	24.107	3.67E+10	12.1668
2518.5	40.783223	67.553	75.728	24.272	3.73E+10	12.1668
2555	41.094232	67.62	75.567	24.433	3.78E+10	12.1668
2591.5	41.405978	67.685	75.411	24.589	3.83E+10	12.1668
2628	41.718406	67.748	75.26	24.74	3.88E+10	12.1668
2664.5	42.031468	67.809	75.113	24.887	3.93E+10	12.1668
2701	42.345125	67.869	74.971	25.029	3.99E+10	12.1668
2737.5	42.65934	67.927	74.832	25.168	4.04E+10	12.1668
2774	42.974082	67.984	74.698	25.302	4.09E+10	12.1668
2810.5	43.28932	68.04	74.567	25.433	4.14E+10	12.1668
2847	43.605029	68.094	74.44	25.56	4.19E+10	12.1668
2883.5	43.918571	68.149	74.309	25.691	4.24E+10	11.7178
2920	44.213906	68.207	74.151	25.849	4.30E+10	11.7178
2956.5	44.511467	68.263	74.005	25.995	4.35E+10	11.7178
2993	44.810075	68.317	73.864	26.136	4.40E+10	11.7178
3029.5	45.109458	68.369	73.728	26.272	4.45E+10	11.7178
3066	45.409486	68.42	73.596	26.404	4.50E+10	11.7178
3102.5	45.710079	68.47	73.468	26.532	4.56E+10	11.7178
3139	46.01118	68.519	73.344	26.656	4.61E+10	11.7178
3175.5	46.312747	68.567	73.222	26.778	4.66E+10	11.7178
3212	46.614745	68.614	73.105	26.895	4.71E+10	11.7178

3248.5	46.917146	68.659	72.99	27.01	4.76E+10	11.7178
3285	47.219924	68.704	72.878	27.122	4.82E+10	11.7178
3321.5	47.523058	68.748	72.769	27.231	4.87E+10	11.7178
3358	47.826528	68.791	72.663	27.337	4.92E+10	11.7178
3394.5	48.130317	68.833	72.559	27.441	4.97E+10	11.7178
3431	48.434408	68.875	72.458	27.542	5.02E+10	11.7178
3467.5	48.730334	68.918	72.344	27.656	5.08E+10	11.527
3504	49.026746	68.96	72.236	27.764	5.13E+10	11.527
3540.5	49.323742	69.001	72.131	27.869	5.18E+10	11.527
3577	49.621177	69.04	72.029	27.971	5.23E+10	11.527
3613.5	49.918983	69.08	71.93	28.07	5.28E+10	11.527
3650	50.21712	69.118	71.834	28.166	5.34E+10	11.527
3686.5	50.515559	69.156	71.74	28.26	5.39E+10	11.527
3723	50.814276	69.193	71.649	28.351	5.44E+10	11.527
3759.5	51.113254	69.229	71.56	28.44	5.49E+10	11.527
3796	51.412476	69.264	71.473	28.527	5.54E+10	11.527
3832.5	51.711193	69.3	71.388	28.612	5.60E+10	11.527
3869	52.011603	69.334	71.305	28.695	5.65E+10	11.527
3905.5	52.311483	69.368	71.224	28.776	5.70E+10	11.527
3942	52.611561	69.401	71.146	28.854	5.75E+10	11.527
3978.5	52.911828	69.434	71.069	28.931	5.80E+10	11.527
4015	53.20818	69.467	70.986	29.014	5.85E+10	10.5821
4051.5	53.463995	69.511	70.833	29.167	5.90E+10	10.5821
4088	53.301506	69.666	69.884	30.116	5.91E+10	0
4124.5	53.129642	69.794	69.049	30.951	5.91E+10	0
4161	52.978348	69.906	68.274	31.726	5.91E+10	0
4197.5	52.839417	70.01	67.527	32.473	5.91E+10	0
4234	52.709426	70.107	66.798	33.202	5.91E+10	0
4270.5	52.586503	70.199	66.079	33.921	5.91E+10	0
4307	52.469474	70.287	65.368	34.632	5.91E+10	0
4343.5	52.357536	70.372	64.661	35.339	5.91E+10	0
4380	52.250103	70.453	63.958	36.042	5.91E+10	0
4416.5	52.14673	70.531	63.257	36.743	5.91E+10	0
4453	52.047066	70.607	62.558	37.442	5.91E+10	0
4489.5	51.950825	70.68	61.86	38.14	5.91E+10	0
4526	51.857772	70.751	61.164	38.836	5.91E+10	0
4562.5	51.767705	70.82	60.468	39.532	5.91E+10	0
4599	51.680452	70.887	59.773	40.227	5.91E+10	0
4635.5	51.59586	70.952	59.079	40.921	5.91E+10	0
4672	51.513796	71.015	58.385	41.615	5.91E+10	0
4708.5	51.43414	71.076	57.693	42.307	5.91E+10	0
4745	51.356784	71.136	57.001	42.999	5.91E+10	0
4781.5	51.281628	71.194	56.31	43.69	5.91E+10	0
4818	51.208581	71.25	55.62	44.38	5.91E+10	0
4854.5	51.137561	71.305	54.932	45.068	5.91E+10	0
4891	51.068488	71.359	54.244	45.756	5.91E+10	0
4927.5	51.001291	71.411	53.559	46.441	5.91E+10	0

4964	50.9359	71.462	52.875	47.125	5.91E+10	0
5000.5	50.872254	71.512	52.193	47.807	5.91E+10	0
5037	50.810289	71.56	51.513	48.487	5.91E+10	0
5073.5	50.749951	71.607	50.836	49.164	5.91E+10	0
5110	50.691184	71.653	50.16	49.84	5.91E+10	0
5146.5	50.633936	71.698	49.488	50.512	5.91E+10	0
5183	50.578159	71.742	48.818	51.182	5.91E+10	0
5219.5	50.523806	71.784	48.151	51.849	5.91E+10	0
5256	50.470832	71.826	47.488	52.512	5.91E+10	0
5292.5	50.419194	71.867	46.827	53.173	5.91E+10	0
5329	50.368851	71.906	46.17	53.83	5.91E+10	0
5365.5	50.319763	71.945	45.517	54.483	5.91E+10	0
5402	50.271894	71.983	44.867	55.133	5.91E+10	0
5438.5	50.225206	72.02	44.221	55.779	5.91E+10	0
5475	50.179665	72.056	43.58	56.42	5.91E+10	0
5511.5	50.135237	72.091	42.942	57.058	5.91E+10	0
5548	50.09189	72.125	42.309	57.691	5.91E+10	0
5584.5	50.049592	72.159	41.68	58.32	5.91E+10	0
5621	50.008314	72.192	41.056	58.944	5.91E+10	0
5657.5	49.968027	72.224	40.437	59.563	5.91E+10	0
5694	49.928703	72.255	39.822	60.178	5.91E+10	0
5730.5	49.890315	72.286	39.213	60.787	5.91E+10	0
5767	49.852837	72.315	38.608	61.392	5.91E+10	0
5803.5	49.816243	72.345	38.008	61.992	5.91E+10	0
5840	49.780511	72.373	37.414	62.586	5.91E+10	0
5876.5	49.745615	72.401	36.825	63.175	5.91E+10	0

Observation Point Data

Number of Observation Points: 50
 Number of Observation Point Readings: 162
 Observation Point Time Step: 36.50 days

EXCESS PORE PRESSURE (kPa)		
Time	Height from Bottom (metres)	
days	0.001	1.54838
36.5	15.19856	11.46588
73	24.45406	21.45867
109.5	28.92925	26.48922
146	33.38402	31.3242
182.5	37.95893	36.17112
219	41.18531	39.60138
255.5	43.88642	42.46116
292	46.64616	45.34814
328.5	49.45535	48.26183
365	52.30825	51.20223
401.5	56.04124	55.00967
438	59.55284	58.58546
474.5	62.06778	61.15638
511	64.60934	63.74724
547.5	67.17514	66.35681
584	69.76312	68.98395
620.5	72.37153	71.62763
657	74.99884	74.28689
693.5	77.64374	76.96087
730	79.82888	79.17262
766.5	81.97079	81.33897
803	84.12712	83.51784
839.5	86.29706	85.70862
876	88.47983	87.91076
912.5	90.67477	90.12373
949	92.88126	92.34704
985.5	95.09872	94.58025
1022	97.0581	96.55442
1058.5	98.64905	98.15926
1095	100.2495	99.77283
1131.5	101.8591	101.3948
1168	103.4775	103.0249
1204.5	105.1043	104.6627
1241	106.7392	106.3081
1277.5	108.3818	107.9607
1314	110.032	109.6203
1350.5	111.6893	111.2868
1387	113.3537	112.9597
1423.5	114.8959	114.5102
1460	116.3724	116.1594
1496.5	117.8553	117.4855
1533	119.3443	118.9812
1569.5	120.8392	120.483
1606	122.3398	121.9904
1642.5	123.846	123.5007
1679	125.3577	125.0208
1715.5	126.8746	126.5436
1752	128.3967	128.0714
1788.5	129.9237	129.6039
1825	131.3621	131.0476
1861.5	132.3623	132.0529
1898	133.3671	133.0627
1934.5	134.3764	134.0768
1971	135.3901	135.0951
2007.5	136.4081	136.1176
2044	137.4303	137.1441
2080.5	138.4566	138.1746
2117	139.4869	139.209
2153.5	140.5211	140.2471
2190	141.5592	141.289
2226.5	142.601	142.3345
2263	143.6444	143.3836
2299.5	144.6955	144.4327
2336	145.7481	145.4922
2372.5	146.7993	146.4867
2409	147.7128	147.4634
2445.5	148.6896	148.4434
2482	149.6697	149.4266
2518.5	150.653	150.4128
2555	151.6395	151.4022
2591.5	152.6215	152.3899
2628	153.6215	153.3899
2664.5	154.6171	154.3881
2701	155.6155	155.3892
2737.5	156.6169	156.3931
2774	157.6211	157.3998
2810.5	158.6281	158.4093
2847	159.6378	159.4214
2883.5	160.6408	160.4267
2920	161.6557	161.344
2956.5	162.6734	162.2638
2993	163.6936	163.1862
3029.5	164.3163	164.1111
3066	165.2416	165.0385
3102.5	166.1693	165.9682
3139	167.0995	166.9004
3175.5	168.0321	167.835
3212	168.967	168.7719
3248.5	169.9044	169.7111
3285	170.844	170.6526
3321.5	171.7859	171.5963
3358	172.7301	172.5423
3394.5	173.6765	173.4905
3431	174.6252	174.4408
3467.5	175.5375	175.3548

**ATTACHMENT 2
FSCONSOL ERROR CHECKING**

1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

Parameters used for modeling						
H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

Check 1D Calc - Volume	
Compare volume based on filling time, production rate, & average density w/ stage-curve volume	
Time	2,883.50 days
Production	15,121.0 ton/day
Total mass	43,601,404 ton
Void Ratio	3.9303
Dry Density	33.540 pcf
Volume	2,599,975,881 ft^3
Cum Vol	2,615,786,325 ft^3
Error	-0.60%

Note: from FSConsol Time Step Data

Note: volume=Qm*/rho_dry

Note: from Rate of Rise Ht vs. Cap Table

EL 5,448.31

Material Properties	
Gs	2.65
A	5.6895 (1/psf)^B
B	-0.1952
M	-0.1424
e0	7.581
w	286.1%
s	25.9%
C	3.91E-04 ft/day
D	3.35

Production	
	15,121 t/day
	182,886 ft^3/day of dry solids
	1,569,366 ft^3/day of tailing
	2,883.50 days
	7.90 years

Check 1D Calc - height of solids				
Compare height of solids based on model inputs and based on the calculated void ratio profile				
Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	17.40	2.2121	4.49
2	29	36.04	0.6144	1.33
3	39	71.95	0.3543	1.48
4	54	157.93	0.2540	2.55
5	69	275.86	0.1998	2.75
6	84	407.95	0.1771	2.73
7	99	554.92	0.1497	2.56
8	114	718.78	0.1412	2.70
9	129	910.33	0.1286	2.87
10	144	1,115.33	0.1247	2.98
11	159	1,334.27	0.1201	3.06
12	174	1,557.63	0.1181	3.07
13	189	1,782.92	0.1168	3.07
14	204	2,042.42	0.1153	3.49
15	229	2,480.92	0.1149	5.87
16	239	2,654.21	0.1149	2.32
17	254	2,883.50	0.1149	3.07
Total				50.38
Hs_FSConsol				51.35
Error				1.93%

Note: based on e profile

1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

Check 1D calc - Mass

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = **2,883.50**

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	1.434	
2	5.080	1.533	2.044
3	10.160	1.645	1.962
4	14.000	1.740	1.426
5	15.240	1.771	0.450
6	20.320	1.911	1.788
7	25.400	2.064	1.700
8	29.000	2.180	1.153
9	30.480	2.227	0.462
10	35.560	2.397	1.534
11	39.000	2.513	0.996
12	40.639	2.569	0.463
13	45.719	2.740	1.390
14	50.799	2.908	1.328
15	54.000	3.010	0.809
16	55.879	3.069	0.465
17	60.959	3.225	1.225
18	66.039	3.373	1.182
19	69.000	3.455	0.671
20	71.119	3.514	0.472
21	76.199	3.649	1.109
22	81.279	3.778	1.078
23	84.000	3.844	0.566
24	86.359	3.901	0.484
25	91.439	4.020	1.024
26	96.519	4.134	1.001
27	99.000	4.187	0.481
28	101.599	4.243	0.498
29	106.678	4.349	0.959
30	111.758	4.452	0.941
31	114.000	4.496	0.409
32	116.838	4.552	0.514
33	121.918	4.648	0.907
34	126.998	4.742	0.892
35	129.000	4.778	0.348
36	132.078	4.834	0.530
37	137.158	4.923	0.864
38	142.238	5.010	0.851
39	144.000	5.039	0.292
40	147.318	5.095	0.547
41	152.398	5.178	0.828
42	157.478	5.260	0.817
43	159.000	5.284	0.243
44	162.558	5.340	0.564
45	167.637	5.418	0.796
46	172.717	5.495	0.787
47	174.000	5.514	0.197
48	177.797	5.570	0.580
49	182.877	5.644	0.769
50	187.957	5.717	0.760
51	189.000	5.732	0.155
52	193.037	5.789	0.597
53	198.117	5.859	0.744
54	203.197	5.929	0.737
55	204.000	5.939	0.116
56	208.277	5.997	0.614
57	213.357	6.064	0.723
58	218.437	6.131	0.716
59	223.517	6.196	0.709
60	228.597	6.261	0.703
61	229.000	6.266	0.056
62	233.677	6.324	0.641
63	238.757	6.387	0.691
64	239.000	6.390	0.033
65	243.836	6.449	0.652
66	248.916	6.510	0.679
67	254.000	7.581	0.632

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
Δ_Height (ft)	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
Height (ft)	14	29	39	54	69	84	99	114	129
Hs (ft)	5.43	5.09	2.99	3.99	3.54	3.22	2.99	2.81	2.66
e_avg	1.58	1.95	2.34	2.76	3.23	3.65	4.02	4.34	4.64
rho_dry (pcf)	64.17	56.12	49.47	43.99	39.06	35.55	32.96	30.95	29.33
Vol (ft^3)	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
Mass (ton)	318,662	1,075,213	1,095,477	2,038,244	2,300,832	2,362,077	2,590,583	2,579,371	2,683,614

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
Δ_Height (ft)	15.0	15.0	15.0	15.0	15.0	25.0	10.0	15.0
Height (ft)	144.0	159.0	174.0	189.0	204.0	229.0	239.0	254.0
Hs (ft)	2.54	2.43	2.34	2.26	2.19	3.52	1.36	1.96
e_avg	4.91	5.16	5.40	5.62	5.84	6.10	6.33	6.64
rho_dry (pcf)	27.98	26.83	25.84	24.97	24.19	23.28	22.57	21.64
Vol (ft^3)	188,799,066	196,020,810	199,271,529	201,610,674	204,228,243	341,505,828	136,611,117	204,916,743
Mass (ton)	2,641,449	2,630,075	2,574,663	2,516,815	2,470,154	3,975,111	1,541,374	2,217,043

Time	2,883.5	days
Production	15,121	ton/day
Total mass	43,601,404	ton
TDF - total mass	37,610,756	ton
Error (%)	13.74%	

Note: based on the time of filling and production rate
 Note: based on the integration of the void ratio profile

1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

Parameters used for modeling

H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

Check 1D calc - Volume

Compare volume based on filling time, production rate, & average density w/ stage-curve volume

Time	4,057.00	days
Production	15,121.0	ton/day
Total mass	61,345,897	ton
Void Ratio	1.1581	
Dry Density	76.623	pcf
Volume	1,601,244,549	ft^3
Cum Vol	1,547,514,922	ft^3
Error	3.47%	

Note: from FSConsol Time Step Data

Note: volume=Qm*t/rho_dry

Note: from Rate of Rise Ht vs. Cap Table

EL 5,371.53

Material Properties

Gs	2.65
A	4.4241 (1/psf)^B
B	-0.2983
M	0.5224
e0	7.581
w	286.1%
s	25.9%
C	4.32E-03 ft/day
D	3.04

Production

15,121	t/day
182,886	ft^3/day of dry solids
1,569,327	ft^3/day of tailing
4,057.00	days
11.12	years

Check 1D Calc - height of solids

Compare height of solids based on model inputs and based on the calculated void ratio profile

Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	25.54	2.2121	6.58
2	29	77.82	0.6144	3.74
3	39	200.19	0.3543	5.05
4	54	433.42	0.2540	6.90
5	69	698.40	0.1998	6.17
6	84	1,009.95	0.1771	6.43
7	99	1,401.09	0.1497	6.83
8	114	1,815.07	0.1412	6.81
9	129	2,345.37	0.1286	7.95
10	144	2,880.33	0.1247	7.77
11	159	3,434.57	0.1201	7.76
12	174	3,993.71	0.1181	7.70
13	189	4,057.00	0.1168	0.86
14	204	4,057.00	0.1153	0.00
15	229	4,057.00	0.1149	0.00
16	239	4,057.00	0.1149	0.00
17	254	4,057.00	0.1149	0.00
Total				80.56
Hs_FSConsol				81.47
Error				1.13%

Note: based on e profile

1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

Check 1D calc - Mass

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = **4,057.00**

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	0.885	
2	5.080	0.892	2.688
3	10.160	0.899	2.680
4	14.000	0.905	2.019
5	15.240	0.907	0.650
6	20.320	0.916	2.658
7	25.400	0.924	2.646
8	29.000	0.931	1.868
9	30.480	0.934	0.766
10	35.560	0.944	2.620
11	39.000	0.951	1.767
12	40.639	0.955	0.839
13	45.719	0.966	2.591
14	50.799	0.978	2.576
15	54.000	0.986	1.615
16	55.879	0.991	0.945
17	60.959	1.005	2.542
18	66.039	1.020	2.524
19	69.000	1.030	1.462
20	71.119	1.036	1.042
21	76.199	1.054	2.484
22	81.279	1.072	2.463
23	84.000	1.083	1.310
24	86.359	1.092	1.130
25	91.439	1.114	2.416
26	96.519	1.136	2.391
27	99.000	1.149	1.158
28	101.599	1.161	1.206
29	106.678	1.188	2.336
30	111.758	1.216	2.307
31	114.000	1.229	1.009
32	116.838	1.246	1.268
33	121.918	1.278	2.246
34	126.998	1.313	2.213
35	129.000	1.328	0.863
36	132.078	1.350	1.316
37	137.158	1.390	2.144
38	142.238	1.433	2.106
39	144.000	1.450	0.722
40	147.318	1.481	1.346
41	152.398	1.537	2.025
42	157.478	1.603	1.977
43	159.000	1.630	0.582
44	162.558	1.691	1.337
45	167.637	1.830	1.840
46	172.717	2.190	1.688
47	174.000	2.340	0.393
48	177.797	2.787	1.066
49	175.533	7.581	-0.366

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
Δ Height (ft)	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
Height (ft)	14	29	39	54	69	84	99	114	129
Hs (ft)	7.39	7.82	5.15	7.62	7.47	7.30	7.09	6.86	6.59
e_avg	0.90	0.92	0.94	0.97	1.01	1.06	1.11	1.19	1.28
rho_dry (pcf)	87.25	86.22	85.20	84.02	82.38	80.46	78.21	75.60	72.64
Vol (ft^3)	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
Mass (ton)	433,286	1,651,849	1,886,806	3,893,369	4,853,484	5,346,370	6,147,490	6,300,939	6,646,956

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
Δ Height (ft)	15.0	15.0	15.0	1.5				
Height (ft)	144.0	159.0	174.0	175.5				
Hs (ft)	6.29	5.93	5.26	0.70				
e_avg	1.39	1.53	1.85	1.19				
rho_dry (pcf)	69.32	65.36	57.96	75.44				
Vol (ft^3)	188,799,066	196,020,810	199,271,529	201,610,674				
Mass (ton)	6,543,555	6,406,050	5,775,192	7,605,173				

Time	4,057	days
Production	15,121	ton/day
Total mass	61,345,897	ton
TDF - total mass	63,490,519	ton
Error (%)	-3.50%	

Note: based on the time of filling and production rate
 Note: based on the integration of the void ratio profile

**ATTACHMENT 3
FSCONSOL MODEL SUMMARY**

Input Parameters

Total Mine Tailings	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Cyclone Availability	100.00%		Assumed Plant Operations
Sand Recovery	45.25%		Average Recovery from Cyclone Plant
Slimes Percentage	40.00%		Slimes vs. Beach (Overflow + Whole)
Overflow Percentage	54.75%		Tailings (Overflow + Whole)
Whole Percentage	0.00%		Tailings (Whole versus Total)
Production	27,618	t/day	Total Tailings (Underflow + Overflow)
	15,121	t/day	Tailings Overflow + Whole
	0	t/day	Tailings Whole (Beach + Slimes)
	15,121	t/day	Tailings Overflow (Beach + Slimes)

Overflow

Production	6,048	t/day	Tailings Overflow Slimes
Production	9,073	t/day	Tailings Overflow Beach
Production	0	t/day	Tailings Whole Slimes
Production	0	t/day	Tailings Whole Beach
Capacity	6.13E+07	tons	Tailings Overflow (Beach + Slimes)
Time	4,057	days	Tailings Overflow (Beach + Slimes)
	11.115	years	

FSConsol 1D Percent Errors

	Height	Volume	Mass	Average
OF-slimes	1.93%	0.60%	13.74%	5.42%
OF-beach	1.13%	3.47%	3.50%	2.70%
Whole-slimes	0.00%	0.00%	0.00%	0.00%
Whole-beach	0.00%	0.00%	0.00%	0.00%

FSConsol average densities (for deepest column in TSF)

	Unadjusted	Adjusted
OF-slimes	33.55 pcf	31.73 pcf
OF-beach	76.65 pcf	74.58 pcf
Whole-slimes	0.00 pcf	0.00 pcf
Whole-beach	0.00 pcf	0.00 pcf

Required Capacity

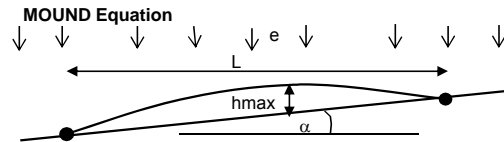
	5.07E+07	tons	Tailings Underflow Cyclone Sand
	2.45E+07	tons	Tailings Overflow Slimes
	3.68E+07	tons	Tailings Overflow Beach
	0.00E+00	tons	Tailings Whole Slimes
	0.00E+00	tons	Tailings Whole Beach
	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Check	0.0000	tons	OK

Required Volume and Weighted Density

	Unadjusted	Adjusted	Volume	Density
OF-slimes	1.46E+09 ft ³	1.55E+09 ft ³	61.04%	19.37 pcf
OF-beach	9.60E+08 ft ³	9.87E+08 ft ³	38.96%	29.05 pcf
Whole-slimes	0.00E+00 ft ³	0.00E+00 ft ³	0.00%	0.00 pcf
Whole-beach	0.00E+00 ft ³	0.00E+00 ft ³	0.00%	0.00 pcf
Total	8.97E+07 yd³	9.38E+07 yd³	100.00%	48.42 pcf

**APPENDIX D
UNDERDRAIN DESIGN CALCULATIONS**

**APPENDIX D.1
DRAIN PIPE SPACING CALCULATIONS**



$$h_{max} = \frac{L\sqrt{C}}{2} \left[\frac{\tan^2 \alpha}{C} + \left(1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing
C=Inflow Ratio (e/K)
e=Constant Recharge Rate
k=Drain Media Hydraulic Conductivity
tan α=Slope Between Pipes

h_{max} (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.41	45	5.07E-04	1.44E+00	1.00E-01	2.83E+02	5.07E-03	0.01

Assume sand applied over a 600 foot by 100 foot area.
Estimate an equivalent constant recharge rate (e) for the drainage through the sand in the active sand application area
Sand K will be higher but flow will be limited by water in underflow

Estimate Equivalent Constant Recharge Rate (e)

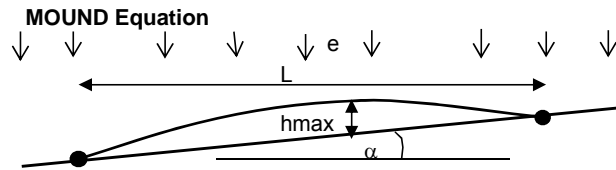
Tons milled	30,000 tpd
Cyclone recovery	46 percent
Q sand	13,800 tpd
Solids content	70 wt percent
Total underflow	19,714 tpd
Water in UF	5,914 tpd
Water in UF	1,042 gpm
Water retained	42.00%
Water lost to evap	15.00%
Seepage	448 gpm

Application area	600 L (feet)	Estimate
Application area	100 w (feet)	Estimate
Application area	60,000 ft ²	

Application rate 1.66392E-05 feet/sec
Application rate (e) 5.07E-04 cm/sec
Equivalent flow rate for active sand application area

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,¹ Yunmin Chen,² and Chuangbing Huang³
Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers
J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)



$$h_{max} = \frac{L\sqrt{C}}{2} \left[\frac{\tan^2 \alpha}{C} + \left(1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing
C=Inflow Ratio (e/K)
e=Constant Recharge Rate
k=Drain Media Hydraulic Conductivity
tan α=Slope Between Pipes

h_{max} (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.33	35	2.75E-07	7.80E-04	3.80E-05	1.08E-01	7.24E-03	0.01

K drain test result 3.8e-05 cm/sec, select (native)drain fill
Average of beach and slimes hydraulic conductivity applied (2.75x10⁷cm/sec).

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,¹ Yunmin Chen,² and Chuangbing Huang³
Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers
J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)

APPENDIX D.2
DRAIN PIPE DEFLECTION CALCULATIONS

Date: April 23, 2013 **Made by:** CDJ
Project No.: 103-92557 **Checked by:** GM
Subject: Underdrain Pipe Deflection Calculations **Reviewed by:** MJG

**Project
Short Title:** **COPPER FLAT FEASIBILITY DESIGN**

1.0 OBJECTIVE

Determine the amount of deflection that will be experienced by the piping system within the tailings storage facility (TSF) underdrains caused by the surcharge load of the ultimate tailings height.

2.0 ASSUMPTIONS

- Underdrain Fill has a cohesion of 0 pounds per square inch (psi) and a friction angle of 39 degrees based on Golder laboratory tests for tailings underflow cyclone sand material.
- Underdrain Fill has a stress versus strain relationship similar to Silty Sand at a relative density of 80 percent per Standard AASHTO compaction specifications.
- Linear relationship between an applied vertical load of 52.1 to 69.4 psi to extrapolate the appropriate strain for an applied vertical load of 200 psi, which is the total vertical stress component imposed by the ultimate tailings height.
- The maximum burial depth is 240 feet.
- All pipes are perforated corrugated polyethylene (PCPE) N-12 pipes with a flexural modulus of 22,000 psi for a 50-year life span.
- The optimum dry density of the tailings underflow cyclone sand in the TSF is 97 pounds per cubic foot (pcf) per laboratory soil density/moisture content compaction proctor tests. As-delivered moisture content is 30 percent for a wet density of 126 pcf. After initial draindown and evaporation, moisture content of approximately 17 percent (optimum water content) for a wet density of 113.5 pcf. Therefore, use the worse-case scenario of 126 pcf for the loading conditions applied to the underdrain.
- Underdrain pipes will be subject to surface loads associated with saturated tailings overflow. The average dry density of the tailings overflow thickened slimes is estimated to be 73 pcf with a wet density of 108 pcf. The loading condition imposed by the cyclone sand dam fill represents the worst-case scenario for pipe deflection analyses.
- Maximum deflection allowed is 15 percent.

3.0 CALCULATION OF PIPE DEFLECTIONS

Golder analyzed the pipe stresses and deformations based on the work of Burns and Richards (Burns and Richards, 1964) and Hoeg (1968) with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression. Calculations of deflections were completed using an Excel™ spreadsheet. Calculations are included in Attachment 1.

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix D.2 Drain Pipe Deflection Calculations

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Golder Associates: Operations in Africa, Asia, Australasia, Europe, North America and South America

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4.0 RESULTS

A summary of the maximum deflections of the underdrain collection system pipes within the Underdrain Fill for the TSF is shown below in Table 1.

Table 1: Summary of Maximum Pipe Deflections

Pipe Information	No Interface Slippage Vertical Deflection (%)	Full Slippage Vertical Deflection (%)
4-inch PCPE N-12	12.6	13.5
10-inch PCPE N-12	12.9	13.8
12-inch PCPE N-12	10.9	11.5

5.0 CONCLUSIONS

The calculated deflections of the 4-inch, 10-inch and 12-inch diameter PCPE N-12 pipes proposed for use in the impoundment and embankment underdrains at the Copper Flat TSF is estimated to be within the allowable limits under the anticipated worst-case loading and placement conditions. The maximum deflection is estimated to be 11 to 11.5 percent for 12-inch diameter pipe and 12.5 to 14 percent for 4-inch to 10-inch diameter pipes. The maximum allowable deflection of these pipes is 15 percent.

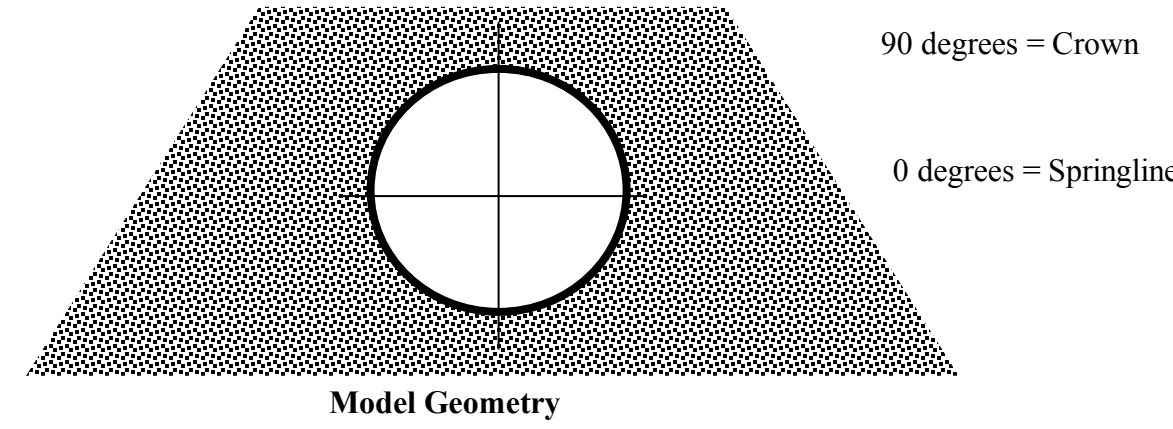
6.0 REFERENCES

- Burns, J.Q. and R.M. Richard, 1964. Attenuation of Stresses for Buried Conduits. Proceedings of the Symposium Soil-Structure Interaction, University of Arizona.
- Hoeg, K., 1968. Stresses Against Underground Structural Cylinders. J. Soil Mech. and Foundation Div. ASCE, Vol. 94, No. SM4.
- Lupo, J.F., 2001. Stability of HDPE Pipes Under High Heap Loads. SME Annual Meeting, Denver, Colorado. February 26-28, 2001.

**ATTACHMENT 1
PIPE DEFLECTION CALCULATION**

BURIED PLASTIC PIPE LOADING WORKSHEET V2.0

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat
4 IN N-12 PIPE

By: CDJ
Date: 04/23/2013
Note: Compression is positive, tension is negative

SOIL and PIPE Input Data

Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30

Pipe Diameter OD (in):	4.78
Pipe ID (in):	4.10
Weight of Pipe (lb/ft):	0.41
Pipe Corrugated (y/n):	y
Prescribed Constrained Modulus (psi):	na
Prescribed Constrained Modulus (psi):	na
Pipe Wall Thickness (in):	0.02
Pipe Area (in ² /in):	0.07
Flexural Modulus (psi), E _f =	22,000
Ring Compression Modulus (psi), E _{rc} =	22,000
C value (in)	0.29
Moment of Inertia (in ⁴ /in) non-corrugated:	11.8
Moment of Inertia (in ⁴ /in) corrugated (input from manufacturer data):	0.0014
Stiffness Coefficients	
Flexural Stiffness	13.7
Ring Compression Stiffness	647.1

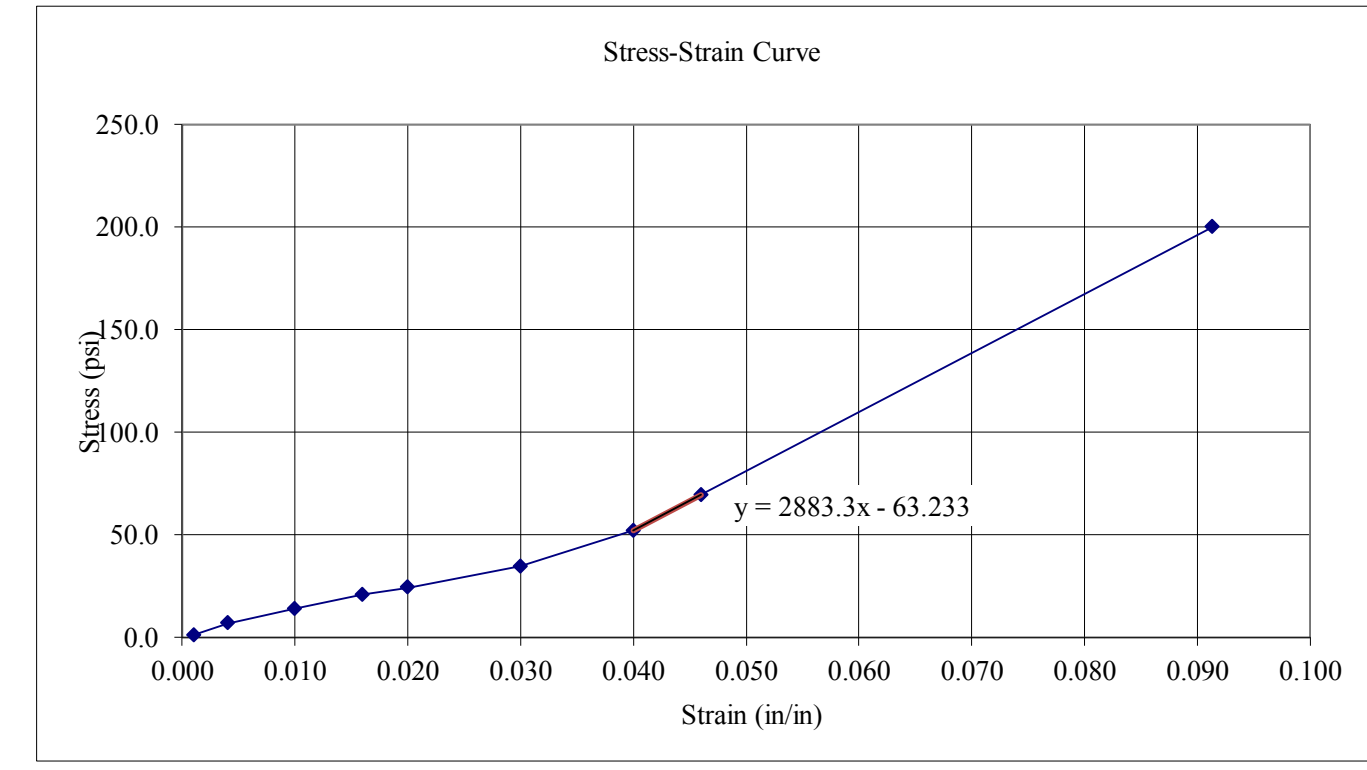
Lateral Pressure Parameters	
64.50	
0.23	
selected I:	0.0014
Ring Stiffness Factor:	2.3
Pipe Stiffness Less Than Soil	

Shell-Medium Parameters	
UF	4.72
VF	97.0
Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.	
Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and	
If both UF and VF are zero then a perfectly rigid embedded pipe.	

Pipe Mean Radius (in):	2.38
Depth of Burial (ft):	240.0
Applied Surface Stress (psf):	0.0
Soil Density (pcf):	120.0
Total Vertical Stress Component (psf):	28800.0
Total Vertical Stress Component (psi):	200.0

Free Field Stress Values

Critical Stress For Buckling Failure (psf): 194.7
Ovality: 0.3
Corrected Stress (psf): 58.4



Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

NO INTERFACE SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	119.6	308.1	0.0	-0.074	0.00E+00	280.7	7.7	4009.8	0.1823	0.0757	1593	1483	5602	5493
10	117.2	301.5	29.6	-0.054	5.92E-02	274.6	7.3	3923.3	0.1783	0.0741	1516	1412	5440	5335
20	110.3	282.5	55.6	0.005	1.11E-01	257.2	6.3	3674.5	0.1670	0.0694	1297	1208	4972	4882
30	99.7	253.4	75.0	0.095	1.50E-01	230.5	4.6	3293.2	0.1497	0.0622	961	895	4254	4188
40	86.8	217.7	85.2	0.205	1.70E-01	197.8	2.7	2825.5	0.1284	0.0533	549	511	3375	3337
50	73.0	179.7	85.2	0.322	1.70E-01	162.9	0.5	2327.8	0.1058	0.0440	110	103	2438	2431
60	60.0	144.0	75.0	0.433	1.50E-01	130.2	-1.5	1860.1	0.0846	0.0351	-302	-281	1558	1579
70	49.5	114.9	55.6	0.522	1.11E-01	103.5	-3.1	1478.9	0.0672	0.0279	-638	-594	841	885
80	42.6	95.9	29.6	0.581	5.92E-02	86.1	-4.1	1230.0	0.0559	0.0232	-857	-798	373	432
90	40.2	89.3	0.0	0.602	2.12E-17	80.1	-4.5	1143.6	0.0520	0.0216	-933	-869	210	275

Vertical Deflection (%):	12.58
Horizontal Deflection (%):	-3.11
Radial Soil Pressure at Crown (psi):	40.2
Circumferential Shortening (in):	1.95
Arc length of each sector (in) =	0.42

5788 149.93 258.17
Max. Compressive Stress (psi): 5602
Max. Tensile Stress (psi): No Tensile Stress

FULL SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.9	81.3	0.0	-0.116	0.00E+00	183.4	8.4	2620.0	0.1191	0.0495	1748	1628	4368	4247
10	76.1	88.4	121.8	-0.093	1.31E-01	183.2	8.0	2617.4	0.1190	0.0494	1663	1548	4280	4165
20	76.8	108.8	229.0	-0.027	2.46E-01	182.7	6.8	2609.8	0.1186	0.0493	1416	1319	4026	3928
30	77.9	140.0	308.5	0.074	3.31E-01	181.9	5.0	2598.3	0.1181	0.0491	1039	967	3637	3566
40	79.2	178.3	350.8	0.198	3.77E-01	180.9	2.8	2584.2	0.1175	0.0488	576	536	3160	3121
50	80.6	219.1	350.8	0.330	3.77E-01	179.8	0.4	2569.2	0.1168	0.0485	83	78	2653	2647
60	81.9	257.4	308.5	0.453	3.31E-01	178.9	-1.8	2555.0	0.1161	0.0482	-379	-353	2176	2202
70	83.0	288.6	229.0	0.554	2.46E-01	178.0	-3.7	2543.5	0.1156	0.0480	-757	-705	1787	1839
80	83.7	309.0	121.8	0.620	1.31E-01	177.5	-4.8	2536.0	0.1153	0.0479	-1003	-934	1533	1602
90	83.9	316.1	0.0	0.643	4.69E-17	177.3	-5.3	2533.4	0.1152	0.0478	-1089	-1013	1445	1520

Vertical Deflection (%):	13.46
Horizontal Deflection (%):	-4.87
Radial Soil Pressure at Crown (psi):	83.9
Circumferential Shortening (in):	1.95
Arc length of each sector (in) =	0.42

12084

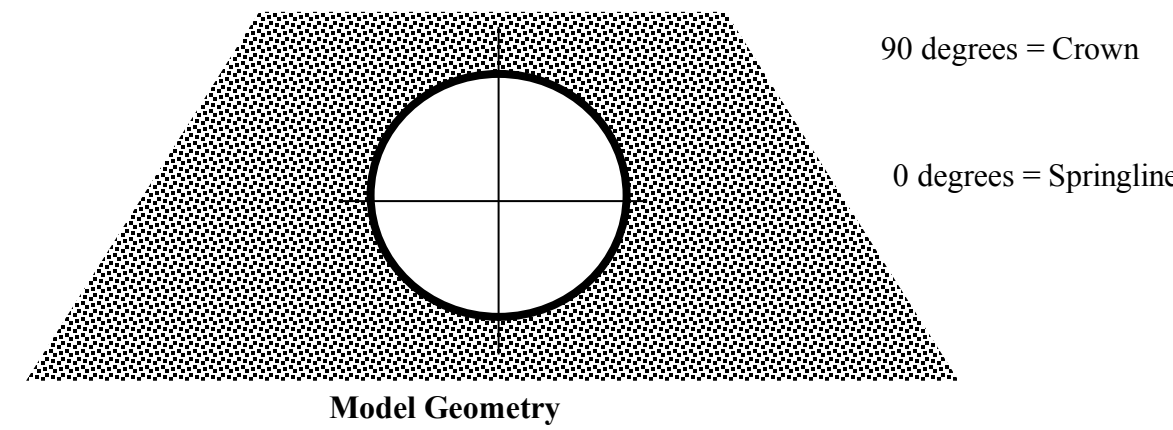
Max. Compressive Stress (psi): 4368
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi
Free Field Stress Times Pipe Radius: 19404 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
2.38	177.3	80.1	316.1	89.3	183.4	280.7	81.3	308.1
2.88	216.6	97.8	198.1	96.5	224.0	342.9	159.9	261.5
3.38	255.9	115.5	147.9	95.9	264.6	405.1	187.5	239.6
3.88	295.2	133.2	123.0	93.6	305.2	467.2	198.2	227.6
4.38	334.4	150.9	109.2	91.4	345.9	529.4	202.6	220.4
4.88	373.7	168.6	100.9	89.4	386.5	591.6	204.3	215.7
5.38	413.0	186.3	95.5	87.8	427.1	653.8	204.9	212.5
5.88	452.3	204.0	91.8	86.5	467.7	716.0	204.9	210.2
6.38	491.5	221.7	89.3	85.4	508.3	778.2	204.7	208.5
6.88	530.8	239.4	87.4	84.6	548.9	840.3	204.4	207.2
7.38	570.1	257.1	85.9	83.8	589.6	902.5	204.0	206.1
7.88	609.4	274.8	84.8	83.2	630.2	964.7	203.7	205.3
8.38	648.6	292.5	84.0	82.7	670.8	1026.9	203.4	204.7
8.88	687.9	310.2	83.3	82.3	711.4	1089.1	203.1	204.1
9.38	727.2	328.0	82.7	81.9	752.0	1151.3	202.9	203.7
9.88	766.5	345.7	82.2	81.6	792.6	1213.4	202.6	203.3
10.38	805.7	363.4	81.9	81.3	833.3	1275.6	202.4	203.0
10.88	845.0	381.1	81.5	81.1	873.9	1337.8	202.2	202.7
11.38	884.3	398.8	81.2	80.9	914.5	1400.0	202.1	202.4
11.88	923.6	416.5	81.0	80.7	955.1	1462.2	201.9	202.2
12.38	962.8	434.2	80.8	80.5	995.7	1524.4	201.8	202.1
12.88	1002.1	451.9	80.6	80.4	1036.3	1586.5	201.7	201.9
13.38	1041.4	469.6	80.4	80.3	1077.0	1648.7	201.6	201.7
13.88	1080.7	487.3	80.3	80.1	1117.6	1710.9	201.5	201.6
14.38	1119.9	505.0	80.2	80.0	1158.2	1773.1	201.4	201.5
Check Values:	648.6	292.5	155.8	92.0	670.8	1026.9	176.9	240.8
Soil Arching:	Positive Arch	Positive Arch	Negative Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

BURIED PLASTIC PIPE LOADING WORKSHEET V2.0

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat
10 IN N-12 PIPE

By: CDJ
Date: 04/23/2013
Note: Compression is positive, tension is negative

SOIL and PIPE Input Data

Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30

Pipe Diameter OD (in):	11.36
Pipe ID (in):	9.90
Weight of Pipe (lb/ft):	2.26
Pipe Corrugated (y/n):	y
Prescribed Constrained Modulus (psi):	na
Pipe Wall Thickness (in):	0.03
Pipe Area (in ² /in):	0.15
Flexural Modulus (psi), E _f =	22,000
Ring Compression Modulus (psi), E _{rc} =	22,000
C value (in)	0.34
Moment of Inertia (in ⁴ /in) non-corrugated:	346.0
Moment of Inertia (in ⁴ /in) corrugated (input from manufacturer data):	0.0110
Stiffness Coefficients	
Flexural Stiffness	8.0
Ring Compression Stiffness	562.9

Lateral Pressure Parameters	
64.50	0.23
selected I:	0.0110
Ring Stiffness Factor:	1.3
Pipe Stiffness Less Than Soil	

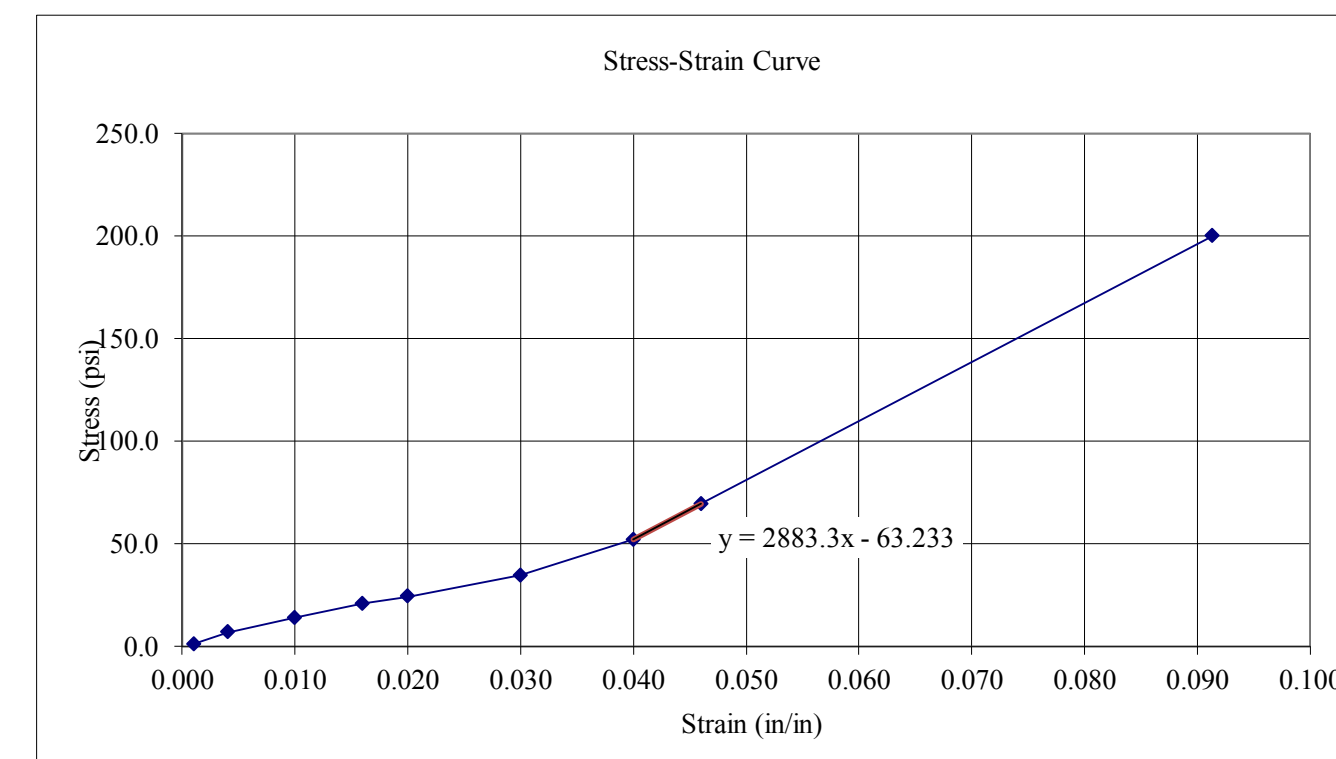
Shell-Medium Parameters	
UF	5.42
VF	166.7

Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and tangential loading.
If both UF and VF are zero then a perfectly rigid embedded pipe.

Pipe Mean Radius (in):	5.67
Depth of Burial (ft):	240.0
Applied Surface Stress (psf):	0.0
Soil Density (pcf):	120.0
Total Vertical Stress Component (psf):	28800.0
Total Vertical Stress Component (psi):	200.0

Free Field Stress Values

Critical Stress For Buckling Failure (psf): 642.5
Ovality: 0.3
Corrected Stress (psf): 192.8



Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

NO INTERFACE SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	118.2	313.4	0.0	-0.172	0.00E+00	652.7	25.6	4501.3	0.2046	0.2024	793	734	5294	5236
10	115.8	306.6	29.3	-0.123	1.41E-01	638.5	24.4	4403.7	0.2002	0.1980	755	699	5159	5103
20	108.7	287.1	55.1	0.019	2.65E-01	597.8	20.9	4122.8	0.1874	0.1854	646	598	4769	4721
30	97.9	257.2	74.2	0.237	3.57E-01	535.4	15.5	3692.3	0.1678	0.1660	479	444	4171	4136
40	84.6	220.6	84.3	0.504	4.06E-01	458.8	8.9	3164.3	0.1438	0.1423	274	254	3439	3418
50	70.4	181.6	84.3	0.788	4.06E-01	377.3	1.8	2602.3	0.1183	0.1170	57	52	2659	2655
60	57.1	144.9	74.2	1.055	3.57E-01	300.8	-4.8	2074.3	0.0943	0.0933	-148	-137	1926	1937
70	46.3	115.1	55.1	1.273	2.65E-01	238.4	-10.2	1643.8	0.0747	0.0739	-315	-292	1329	1352
80	39.2	95.6	29.3	1.415	1.41E-01	197.6	-13.7	1362.9	0.0619	0.0613	-424	-393	939	970
90	36.8	88.8	0.0	1.464	5.05E-17	183.5	-14.9	1265.3	0.0575	0.0569	-462	-428	804	837

Vertical Deflection (%):	12.89
Horizontal Deflection (%):	-3.03
Radial Soil Pressure at Crown (psi):	36.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

5295 149.93 258.17
Max. Compressive Stress (psi): 5294
Max. Tensile Stress (psi): No Tensile Stress

FULL SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.2	82.0	0.0	-0.271	0.00E+00	422.3	28.1	2912.3	0.1324	0.1309	869	805	3781	3717
10	75.3	89.2	123.0	-0.216	3.15E-01	422.0	26.7	2910.6	0.1323	0.1309	826	766	3737	3676
20	75.7	109.9	231.1	-0.057	5.93E-01	421.3	22.8	2905.5	0.1321	0.1306	704	652	3610	3558
30	76.3	141.5	311.4	0.187	7.98E-01	420.2	16.7	2897.8	0.1317	0.1303	517	479	3415	3377
40	77.1	180.4	354.1	0.487	9.08E-01	418.8	9.3	2888.3	0.1313	0.1299	288	266	3176	3155
50	77.9	221.8	354.1	0.805	9.08E-01	417.3	1.4	2878.3	0.1308	0.1294	43	40	2922	2918
60	78.7	260.6	311.4	1.105	7.98E-01	416.0	-6.0	2868.8	0.1304	0.1290	-186	-173	2683	2696
70	79.3	292.3	231.1	1.349	5.93E-01	414.9	-12.1	2861.0	0.1300	0.1286	-373	-346	2488	2515
80	79.7	313.0	123.0	1.508	3.15E-01	414.1	-16.0	2856.0	0.1298	0.1284	-495	-459	2361	2397
90	79.8	320.2	0.0	1.564	1.13E-16	413.9	-17.4	2854.3	0.1297	0.1283	-538	-498	2316	2356

Vertical Deflection (%):	13.76
Horizontal Deflection (%):	-4.79
Radial Soil Pressure at Crown (psi):	79.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

11497

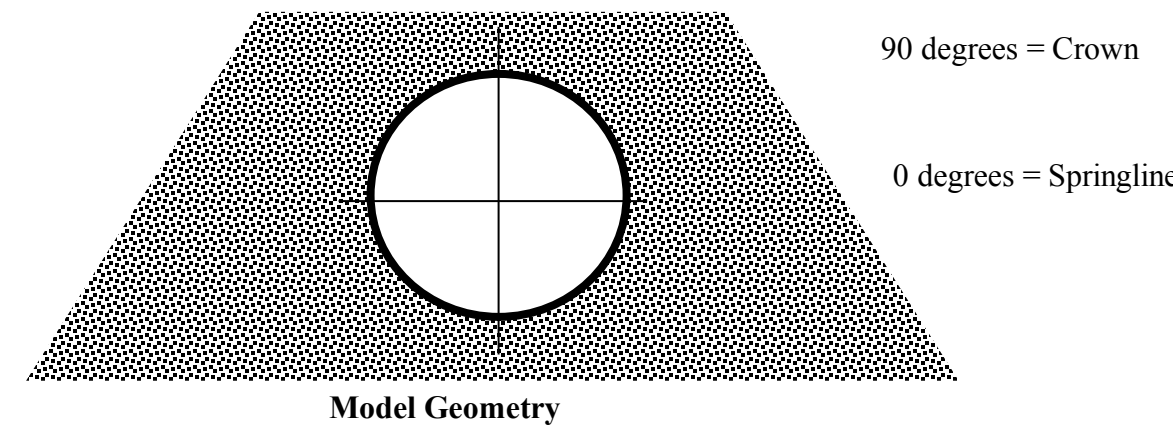
Max. Compressive Stress (psi): 3781
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi
Free Field Stress Times Pipe Radius: 33347 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
5.67	413.9	183.5	320.2	88.8	422.3	652.7	82.0	313.4
6.17	452.2	200.5	256.3	94.2	461.4	713.2	125.7	287.8
6.67	490.6	217.5	213.4	96.5	500.6	773.7	153.0	270.0
7.17	529.0	234.4	183.6	97.1	539.7	834.2	170.5	257.0
7.67	567.3	251.4	162.2	96.9	578.9	894.7	182.0	247.3
8.17	605.7	268.4	146.5	96.2	618.0	955.2	189.7	240.0
8.67	644.0	285.4	134.6	95.3	657.1	1015.8	194.9	234.2
9.17	682.4	302.4	125.5	94.3	696.3	1076.3	198.4	229.6
9.67	720.8	319.4	118.4	93.3	735.4	1136.8	200.8	225.9
10.17	759.1	336.4	112.7	92.4	774.6	1197.3	202.5	222.8
10.67	797.5	353.4	108.2	91.5	813.7	1257.8	203.6	220.3
11.17	835.9	370.4	104.4	90.6	852.9	1318.3	204.3	218.2
11.67	874.2	387.4	101.4	89.8	892.0	1378.8	204.8	216.4
12.17	912.6	404.4	98.8	89.1	931.1	1439.3	205.1	214.8
12.67	950.9	421.4	96.7	88.4	970.3	1499.8	205.2	213.5
13.17	989.3	438.4	94.8	87.8	1009.4	1560.4	205.3	212.3
13.67	1027.7	455.4	93.3	87.2	1048.6	1620.9	205.3	211.3
14.17	1066.0	472.4	91.9	86.7	1087.7	1681.4	205.2	210.4
14.67	1104.4	489.3	90.8	86.2	1126.8	1741.9	205.1	209.6
15.17	1142.7	506.3	89.7	85.8	1166.0	1802.4	205.0	208.9
15.67	1181.1	523.3	88.8	85.4	1205.1	1862.9	204.8	208.3
16.17	1219.5	540.3	88.0	85.0	1244.3	1923.4	204.7	207.8
16.67	1257.8	557.3	87.3	84.6	1283.4	1983.9	204.6	207.3
17.17	1296.2	574.3	86.7	84.3	1322.6	2044.4	204.4	206.8
17.67	1334.6	591.3	86.2	84.0	1361.7	2105.0	204.3	206.4
Check Values:	874.2	387.4	202.4	95.0	892.0	1378.8	156.8	264.2
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

BURIED PLASTIC PIPE LOADING WORKSHEET V2.0

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat
12 IN N-12 PIPE

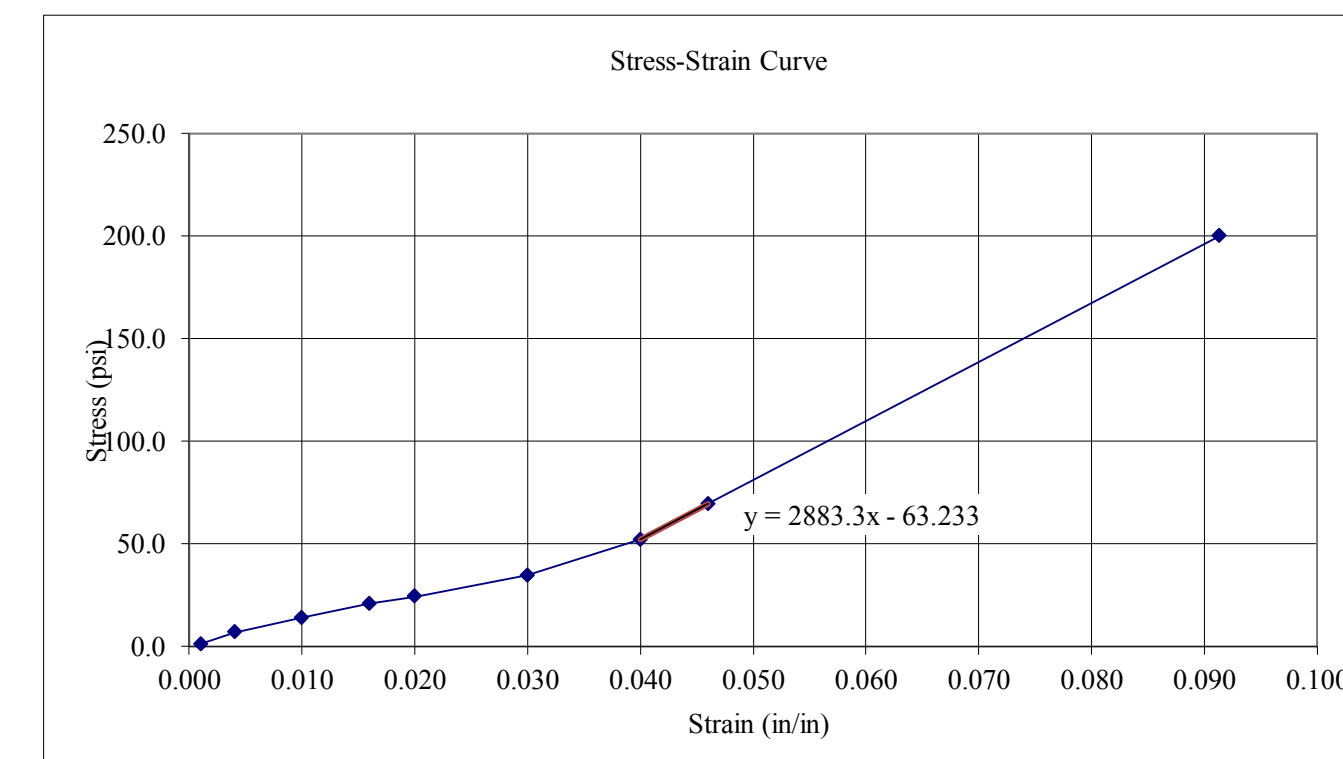
By: CDJ
Date: 04/23/2013
Note: Compression is positive, tension is negative

SOIL and PIPE Input Data

Lateral Pressure Parameters						
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30
Pipe Diameter OD (in):	14.45					
Pipe ID (in):	12.15					
Weight of Pipe (lb/ft):	3.19					
Pipe Corrugated (y/n):	y		64.50			
Prescribed Constrained Modulus (psi):	n		0.23			
Prescribed Constrained Modulus (psi):	na					
Pipe Wall Thickness (in):	0.04					
Pipe Area (in ² /in):	0.19					
Flexural Modulus (psi), E _f =	22,000					
Ring Compression Modulus (psi), E _{rc} =	22,000					
C value (in)	0.53					
Moment of Inertia (in ⁴ /in) non-corrugated:	1070.4					
Moment of Inertia (in ⁴ /in) corrugated (input from manufacturer data):	0.0410	selected I:	0.0410			
Stiffness Coefficients						
Flexural Stiffness	14.5					
Ring Compression Stiffness	573.8					
Ring Stiffness Factor: 2.4 Pipe Stiffness Less Than Soil						

Shell-Medium Parameters		
UF	5.32	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	92.0	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	7.21	
Depth of Burial (ft):	240.0	
Applied Surface Stress (psf):	0.0	
Soil Density (pcf):	120.0	
Total Vertical Stress Component (psf):	28800.0	Free Field Stress Values
Total Vertical Stress Component (psi):	200.0	



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

Critical Stress For Buckling Failure (psf): 1883.1
Ovality: 0.3
Corrected Stress (psf): 564.9

NO INTERFACE SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	97.9	343.0	0.0	-0.122	0.00E+00	693.6	63.5	3689.5	0.1677	0.2110	820	766	4510	4456
10	95.8	335.2	25.4	-0.071	1.40E-01	678.0	60.5	3606.5	0.1639	0.2062	782	730	4389	4337
20	90.0	312.9	47.7	0.075	2.64E-01	633.1	51.9	3367.6	0.1531	0.1926	671	627	4039	3994
30	80.9	278.8	64.2	0.301	3.56E-01	564.3	38.8	3001.4	0.1364	0.1716	502	468	3503	3470
40	69.9	236.9	73.0	0.577	4.04E-01	479.8	22.7	2552.3	0.1160	0.1459	293	274	2846	2826
50	58.1	192.3	73.0	0.871	4.04E-01	390.0	5.6	2074.3	0.0943	0.1186	72	67	2146	2141
60	47.1	150.4	64.2	1.147	3.56E-01	305.5	-10.5	1625.2	0.0739	0.0929	-136	-127	1489	1498
70	38.1	116.2	47.7	1.372	2.64E-01	236.7	-23.7	1259.0	0.0572	0.0720	-306	-286	953	973
80	32.2	93.9	25.4	1.519	1.40E-01	191.8	-32.2	1020.1	0.0464	0.0583	-417	-389	603	631
90	30.1	86.2	0.0	1.570	5.03E-17	176.2	-35.2	937.1	0.0426	0.0536	-455	-425	482	512

Vertical Deflection (%):	10.86				
Horizontal Deflection (%):	-1.70				
Radial Soil Pressure at Crown (psi):	30.1	4340	149.93	Max. Compressive Stress (psi):	4510
Circumferential Shortening (in):	5.29		258.17	Max. Tensile Stress (psi):	No Tensile Stress
Arc length of each sector (in) =	1.26				

FULL SLIPPAGE

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	60.5	96.7	0.0	-0.214	0.00E+00	442.9	68.7	2356.0	0.1071	0.1347	889	830	3245	3186
10	60.7	103.8	122.2	-0.157	3.23E-01	442.4	65.4	2353.4	0.1070	0.1346	846	790	3199	3144
20	61.3	124.3	229.6	0.006	6.07E-01	441.1	56.0	2346.0	0.1066	0.1341	723	676	3069	3022
30	62.2	155.6	309.3	0.255	8.18E-01	438.9	41.4	2334.7	0.1061	0.1335	536	500	2870	2835
40	63.4	194.1	351.7	0.561	9.30E-01	436.3	23.6	2320.7	0.1055	0.1327	305	285	2626	2606
50	64.6	235.0	351.7	0.887	9.30E-01	433.5	4.6	2305.9	0.1048	0.1318	60	56	2366	2362
60	65.8	273.5	309.3	1.192	8.18E-01	430.9	-13.2	2291.9	0.1042	0.1311	-170	-159	2122	2133
70	66.7	304.9	229.6	1.442	6.07E-01	428.8	-27.7	2280.6	0.1037	0.1304	-358	-335	1922	1946
80	67.3	325.3	122.2	1.605	3.23E-01	427.4	-37.2	2273.2	0.1033	0.1300	-481	-449	1792	1824
90	67.6	332.4	0.0	1.661	1.16E-16	426.9	-40.5	2270.6	0.1032	0.1298	-523	-489	1747	1782

Vertical Deflection (%):	11.50
Horizontal Deflection (%):	-2.96
Radial Soil Pressure at Crown (psi):	67.6
Circumferential Shortening (in):	5.29
Arc length of each sector (in) =	1.26


9728

Max. Compressive Stress (psi): 3245
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi
Free Field Stress Times Pipe Radius: 18401 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
7.21	426.9	176.2	332.4	86.2	442.9	693.6	96.7	343.0
7.71	458.3	189.1	278.0	93.0	475.5	744.7	131.0	316.0
8.21	489.7	202.0	238.5	96.8	508.1	795.8	154.3	295.9
8.71	521.1	215.0	209.0	98.7	540.7	846.9	170.3	280.6
9.21	552.5	227.9	186.6	99.4	573.3	897.9	181.4	268.6
9.71	583.9	240.8	169.3	99.5	605.9	949.0	189.3	259.1
10.21	615.3	253.8	155.7	99.1	638.5	1000.1	194.9	251.5
10.71	646.8	266.7	144.8	98.5	671.1	1051.2	198.9	245.2
11.21	678.2	279.6	136.0	97.7	703.7	1102.2	201.8	240.1
11.71	709.6	292.6	128.8	96.9	736.3	1153.3	203.8	235.8
12.21	741.0	305.5	122.9	96.0	768.9	1204.4	205.3	232.1
12.71	772.4	318.4	117.9	95.2	801.5	1255.5	206.3	229.0
13.21	803.8	331.4	113.7	94.3	834.1	1306.5	207.0	226.3
13.71	835.2	344.3	110.1	93.5	866.7	1357.6	207.4	224.0
14.21	866.6	357.2	107.1	92.8	899.3	1408.7	207.7	222.0
14.71	898.1	370.2	104.5	92.0	931.9	1459.7	207.8	220.2
15.21	929.5	383.1	102.2	91.4	964.4	1510.8	207.9	218.7
15.71	960.9	396.0	100.2	90.7	997.0	1561.9	207.8	217.3
16.21	992.3	409.0	98.4	90.1	1029.6	1613.0	207.7	216.1
16.71	1023.7	421.9	96.9	89.5	1062.2	1664.0	207.6	215.0
17.21	1055.1	434.8	95.5	89.0	1094.8	1715.1	207.5	214.0
17.71	1086.5	447.8	94.3	88.5	1127.4	1766.2	207.3	213.1
18.21	1118.0	460.7	93.2	88.0	1160.0	1817.3	207.1	212.3
18.71	1149.4	473.6	92.2	87.6	1192.6	1868.3	206.9	211.5
19.21	1180.8	486.6	91.3	87.2	1225.2	1919.4	206.7	210.9
Check Values:	803.8	331.4	224.2	96.1	834.1	1306.5	159.7	287.8
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

**APPENDIX E
TSF UNDERDRAIN COLLECTION POND
INFLOW ESTIMATION**

		Tailings Storage Facility - Underdrain Collection Pond Inflow Estimate				
		Client	New Mexico Copper Corp	By	GM/DW	Date
			Checked	GM	Date	15-Jul-13
Project No.	103-92557 Phase 011	Reviewed	MJG	Date		

Requirements	Criteria	
1 Collect downstream embankment face and toe area runoff	100-yr 24 hour storm	3.73 inches
2 Collect Underflow drainage	Upset period	24 hours
3 Collect impoundment seepage	Upset period	24 hours
4 Provide for contact of free water pond with exposed underdrain	Upset period	24 hours

Hydraulic Conductivity-fully consolidated (from slurry consol tests)		
Product	k (cm/sec)	K (ft/sec)
Cyclone overflow beach (cob)	5.00E-07	1.64E-08
Cyclone overflow slimes (cos)	5.00E-08	1.64E-09

The above assume fully consolidated material against drain

1) Downstream Dam Face Runoff

Worst case prior to Final Build-out	Runoff Volume	12.1 acre-feet
	See HEC model Output	3,942,528 Gallons

2) Underflow Drainage

Mining Rate		30000	tpd
Underflow percentage	(Cyclone Simulation)	45.5	percent
Underflow solids content	(Cyclone Simulation)	70	percent by weight
Slurry Volume	(Cyclone Simulation)	568	tons per hour max
Water volume	(Cyclone Simulation)	260	tons per hour max

Delivered water (wt water/wt solids)	1042	gpm
Available for Drainage and Evaporation	58.00%	estimate
Entrainment	42.00%	estimate
Evaporative loss	15.00%	estimate
Draindown	43.00%	
Draindown (gpm)	448	gpm

3) Impoundment Interior Seepage

Assume Areal Split	Beach	60%	Slimes	40%
--------------------	-------	-----	--------	-----

	Total Area (ft ³)	Beach Area	Slimes Area
Final Build-out	14,012,401	8,407,441	5,604,960

Assume unit hydraulic gradient Hydraulic conductivity based on results of slurry consolidation testing						
Beach Seepage	K floor (COB)			Slimes Seepage	K Floor (COS)	
	ft ³ /sec	gpm	Upset (gal)		ft ³ /sec	gpm
	0.1379	62	89,132	0.0092	4.13	5,942

4) Seepage Through Exposed Drain

Area exposed	20	Acre	871,200	ft ³	Assumption/estimate
K drain surface layer	3.8000E-05	cm/sec	1.2467E-06	ft/sec	K drain based on permeability test result
Assume max depth 5 feet, average depth 2.5 feet		i=	2.5		
	Q	cfs	gpm	Upset (gal)	
		2.7154	1,219	1,754,857	

Summary	Pond Capacity Requirements	Duration	Storage (gallons)	
Dam exterior Runoff		Event	3,942,528 (HEC runoff estimate)	
Beach Seepage		Upset	89,132	
Slimes Seepage		Upset	5,942	
Exposed impoundment Underdrain Seepage		Upset	1,754,857	Can be controlled
Dam underdrainage		Upset	645,206	
Total			6,437,666	
			gpm	
Normal flows		Dam drain	448	gpm
		Beach/slimes	66	gpm
		Free Water Pond seepage	1,219	gpm
		Total	1,733	gpm

APPENDIX F
HYDROLOGIC CALCULATIONS

APPENDIX F.1
IMPOUNDMENT DIVERSION DITCH CALCULATIONS

Copper Flat Diversions, Copper Flat New Mexico
Hydrologic and Hydraulic Calculations / Calculation of Time Concentration (t_c)
AMC II Moisture Conditions

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile			S	L	H1	H2	Y	Lag	t _c	t _c	Lag	Area
	Area		Area		Area	Wt.											
	(ac)	CN	(ac)	CN	(ac)	CN	CN										
Northeast- PH1	66.41	85	0.00	92	0.00	50	85.0	1.76	5,318	5,537	5,317	4.13	0.50	0.84	50.5	30	0.10376
Southwest- PH1	47.92	85	0.00	92	0.00	50	85.0	1.76	5,835	5,452	5,307	2.48	0.70	1.17	70.2	42	0.07488
SW-Periphery- PH3	26.09	85	0.00	92	0.00	50	85.0	1.76	4,052	5,452	5,346	2.60	0.51	0.85	51.1	31	0.04076

Curve Number Estimation:

Undisturbed Native Ground
 Arid and semiarid rangelands
 Cover type =
 Antecedent condition =
 Hydrologic condition =
 Hydrologic soil group =
 Curve number =

Compacted Cover
 Arid and semiarid rangelands
 Cover type =
 Antecedent condition =
 Hydrologic condition =
 Hydrologic soil group =
 Curve number =

Fill
 OutSlope
 Cover type =
 Antecedent condition =
 Hydrologic condition =
 Hydrologic soil group =
 Curve number =

TSF
 Sand
 Cover type =
 Antecedent condition =
 Hydrologic condition =
 Hydrologic soil group =
 Curve number =

Notes:

ac = acres
 CN = curve number
 Wt. CN = weighted curve number
 S = soil and cover parameter
 L = length of longest flow path (feet)
 H1 = elevation at top of longest flow path (feet)
 H2 = elevation at bottom of longest flow path (feet)
 Y = H1-H2/L x 100 = slope (%)
 Lag = Lag time (hours)
 t_c = time of concentraton (hr or min)

Hydrologic Condition:
 Poor: <30% ground cover
 Fair: 30 to 70% ground cover
 Good: >70% ground cover

Copper Flat Diversions - Hydrologic and Hydraulic Results - PMP Storm Evaluation

Global Summary Results for Run "PMP Evaluation"

Project: 103-92557 Phase 1 Diversions Simulation Run: PMP Evaluation
 Start of Run: 23Apr2013, 00:00 Basin Model: Copper Flat
 End of Run: 28Apr2013, 00:01 Meteorologic Model: PMP
 Compute Time: 26Apr2013, 13:07:57 Control Specifications: 5-Day

Show Elements: All Elements Volume Units: IN AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Northeast-1	0.10376	524.666	25Apr2013, 00:35	143.715
D-1	0.10376	524.666	25Apr2013, 00:35	143.715
Southwest-1	0.07488	339.047	25Apr2013, 00:47	103.714
D-2	0.07488	339.047	25Apr2013, 00:47	103.714
Southwest Periphery	0.04076	204.294	25Apr2013, 00:36	56.456
D-3	0.04076	204.294	25Apr2013, 00:36	56.456

Subbasin Area [Copper Flat]

Subbasin	Area (MI2)
Northeast-1	0.10376
Southwest-1	0.07488
Southwest Periphery	0.04076

Curve Number Loss [Copper Flat]

Subbasin	Initial Abstraction (IN)	Curve Number
Northeast-1		85
Southwest-1		85
Southwest Periphery		85

SCS Transform [Copper Flat]

Subbasin	Lag Time (MIN)
Northeast-1	30
Southwest-1	42
Southwest Periphery	31

Precipitation Parameters

Met Name: PMP
 Probability: 0.2 Percent
 Input Type: Partial Duration
 Output Type: Annual Duration
 Intensity Duration: 15 Minutes
 Storm Duration: 4 Days
 Intensity Position: 50 Percent
 Storm Area (MI2): 0.7903

5 Minutes (IN):
 *15 Minutes (IN): 1.9500
 *1 Hour (IN): 8.0000
 *2 Hours (IN): 10.750
 *3 Hours (IN): 12.500
 *6 Hours (IN): 15.000
 *12 Hours (IN): 18.500
 *1 day (IN): 22.000
 *2 Days (IN): 24.500
 *4 Days (IN): 28.000
 7 Days (IN):
 10 Days (IN):

WARNING 20045: Control specifications time intervals less than duration or maximum intensity. Precipitation data will be interpolated.
 NOTE 40040: The basin model contains 3 outlets: D-1, D-2, D-3
 NOTE 40049: Found no parameter problems in basin model "Copper Flat".
 NOTE 10185: Finished computing simulation run "PMP Evaluation" at time 26Apr2013, 13:07:57.

103-92557 Copper Flat Stormwater Diversions (PMP) Report

Label	Solve For	Friction Method	Roughness Coefficient	Channel Slope (ft/ft)
D-1 North PMP	Normal Depth	Manning Formula	0.040	0.00500
D-2 West PMP	Normal Depth	Manning Formula	0.045	0.00500
D-3 SW Periphery PMP	Normal Depth	Manning Formula	0.045	0.00500

Normal Depth (ft)	Left Side Slope (ft/ft (H:V))	Right Side Slope (ft/ft (H:V))	Bottom Width (ft)	Discharge (ft ³ /s)
4.52	3.50	2.00	10.00	525.00
4.15	2.00	2.00	10.00	340.00
3.20	2.00	2.00	10.00	205.00

Flow Area (ft ²)	Wetted Perimeter (ft)	Hydraulic Radius (ft)	Top Width (ft)	Critical Depth (ft)
101.29	36.54	2.77	34.85	3.27
75.89	28.55	2.66	26.59	2.73
52.54	24.32	2.16	22.81	2.04

Critical Slope (ft/ft)	Velocity (ft/s)	Velocity Head (ft)	Specific Energy (ft)	Froude Number
0.01893	5.18	0.42	4.93	0.54
0.02529	4.48	0.31	4.46	0.47
0.02713	3.90	0.24	3.44	0.45

Flow Type	Notes	Messages
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Subcritical

Subcritical

Subcritical

**APPENDIX F.2
DAM AND TSF UNDERDRAIN COLLECTION POND
AREA HYDROLOGIC CALCULATIONS**

**Copper Flat Dam, Copper Flat, New Mexico
Seepage and Runoff / Calculation of Time Runoff (t_c)**

Hydrologic and Hydraulic Calculations

AMC II Moisture Conditions

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile		Rational method Inputs														
	Area		Area		Area	Wt.	L	H1	H2	Y	Lag	t _c	t _c	Lag	Area	Area	C	Intensity	Q		
	(ac)	CN	(ac)	CN	(ac)	CN	CN	S	(ft)	(ft)	(ft)	(%)	(hr)	(hr)	(min)	(min)	(mi ²)	(acre)		(in/hr)	(cfs)
Phase 4	71.09	85	0.00	92	91.82	50	65.3	5.32	8,640	5,385	5,170	2.49	1.71	2.85	171.0	103	0.25454	162.91	0.38	0.907	71.1

Curve Number Estimation:

Undisturbed Native Ground

Arid and semiarid rangelands

Cover type = Desert Shrub / Pinyon-Juniper

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = C

Curve number = **85**

C = **0.43**

Compacted Cover

Arid and semiarid rangelands

Cover type = Fill

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = D

Curve number = **92**

C = **0.46**

TSF

OutSlope

Cover type = Sand

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = A

Curve number = **50**

C = **0.35**

Notes:

ac = acres

CN = curve number

Wt. CN = weighted curve number

S = soil and cover parameter

L = length of longest flow path (feet)

H1 = elevation at top of longest flow path (feet)

H2 = elevation at bottom of longest flow path (feet)

Y = H1-H2/L x 100 = slope (%)

Lag = Lag time (hours)

t_c = time of concentration (hr or min)

C = Rational Method Coefficient

C_f = Correction factor for 100-year storm event (1.25)

I = rainfall intensity (inches/hour)

A = Area (acres)

Hydrologic Condition:

Poor: <30% ground cover

Fair: 30 to 70% ground cover

Good: >70% ground cover

Rational Method

Q=CiA

Q=C_fCiA

i= 0.907

Includes correction factor for 100-yr storm duration.

100-year, 3 hour storm intensity

**Copper Flat Dam Toe / Seepage and Runoff
Hydrologic and Hydraulic Results / 100-yr, 24-h Storm Volume**

Global Summary Results for Run "100-yr, 24-hr"

Project: 103-92557 Copper Flat Simulation Run: 100-yr, 24-hr
 Start of Run: 15Mar2013, 00:00 Basin Model: Basin 1
 End of Run: 20Mar2013, 12:01 Meteorologic Model: 100 yr-24hr
 Compute Time: 26Apr2013, 16:33:13 Control Specifications: 5 day

Show Elements: All Elements Volume Units: IN AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Copper Flat Outslope...	0.25454	31.790	15Mar2013, 13:53	12.101

Subbasin Area [Basin 1]

Show Elements: All Elements Sorting: Hydrologic

Subbasin	Area (MI2)
Copper Flat Outslo...	0.25454

Curve Number Loss [Basin 1]

Show Elements: All Elements Sorting: Hydrologic

Subbasin	Initial Abstraction (IN)	Curve Number	Impervious (%)
Copper Flat Outslo...		65.3	0.0

SCS Transform[Basin 1]

Show Elements: All Elements Sorting: Hydrologic

Subbasin	Lag Time (MIN)
Copper Flat Outslo...	103

Precipitation Summary

Met Name: 100 yr-24hr
 Method: Type 2
 *Depth (IN) 3.73

NOTE 10180: Opened meteorologic model "100 yr-24hr" at time 26Apr2013, 16:33:10.
 NOTE 10184: Began computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.
 NOTE 20364: Found no parameter problems in meteorologic model "100 yr-24hr".
 NOTE 40049: Found no parameter problems in basin model "Basin 1".
 NOTE 10185: Finished computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.

Worksheet for Dam Toe Seepage & Runoff Collection Trench

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.012	
Channel Slope	0.02500	ft/ft
Left Side Slope	2.00	ft/ft (H:V)
Right Side Slope	2.00	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	71.10	ft ³ /s

Results

Normal Depth	0.54	ft
Flow Area	5.94	ft ²
Wetted Perimeter	12.40	ft
Hydraulic Radius	0.48	ft
Top Width	12.14	ft
Critical Depth	1.08	ft
Critical Slope	0.00226	ft/ft
Velocity	11.98	ft/s
Velocity Head	2.23	ft
Specific Energy	2.77	ft
Froude Number	3.02	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.54	ft
Critical Depth	1.08	ft
Channel Slope	0.02500	ft/ft

Worksheet for Dam Toe Seepage & Runoff Collection Trench

GVF Output Data

Critical Slope 0.00226 ft/ft

**APPENDIX G
WATER BALANCE CALCULATIONS**

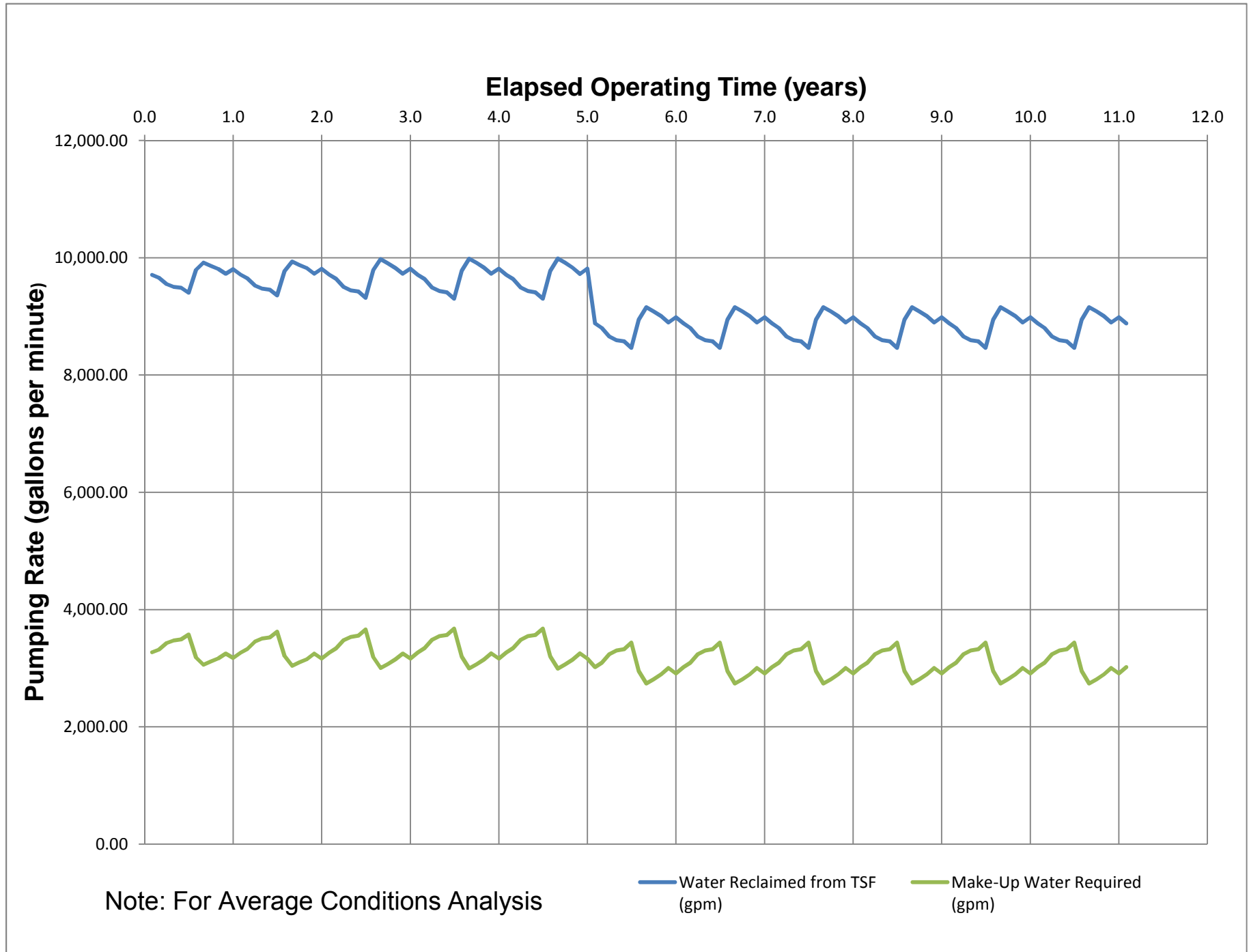


Table 1: Water Balance Calculations

Date:	12/16/2013	Made by:	4/29/2013 RL/GM
Project No.:	103-92557	Checked by:	7/15/2013 GM
Subject:	Copper Flat Climate Summary	Reviewed	7/15/2013 MG
Project Short Title:	Copper Flat Feasibility Study, New Mexico	Page:	1 of 3

Elevation at the site is approximately 5180-5270 feet.

Average Precipitation (mm)

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,270.00	4.00
Caballo Dam (1936-2010)	0.42	0.38	0.27	0.24	0.37	0.70	1.80	2.03	1.55	0.89	0.44	0.56	9.65	4,190.00	12.30
Copper Flat Site Estimate	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,200.00	

Average Days with Rain

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	3	3	2	2	3	3	9	9	6	4	2	3	49	5,270.00	4.00
Caballo Dam (1936-2010)	3	2	2	1	2	3	8	9	6	4	2	3	45	4,190.00	12.30
Copper Flat Site Estimate	3	3	2	2	3	3	9	9	6	4	2	3	49	5,200.00	

Local Normal Evaporation (mm)

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
														4,190.00	
Copper Flat Site Estimate	4.22	5.57	8.55	9.52	11.15	14.25	10.34	5.94	6.18	3.96	3.68	2.74	86.10	5,385.00	

Table 2: Water Balance Calculations Year 1-5 30,000 tpd mine plan

Date:		12/16/2013		Made by:		4/29/2013 RL	
Project No.:		103-92557		Checked by:		11/15/2013 GM	
Subject:		Tailings Properties and Flow rates		Reviewed:		11/15/2013 BNS	
Project Short Title:		Copper Flat Feasibility Study, New Mexico		Page:		2 of 3	
Year 1-5		Design Discharge Rate		1,333 tph			
		Cyclone Recovery		608.47 tph		Krebs	
Tailings Discharge		Overflow		725 tph			
		Total		1333 tph		Net (post concentrate recovery)	
						1316.0 tph	
Cyclone feed solids		29.10%				Factor for average TSF inputs	
						99%	
Cyclone Discharge						Result (entrainment only)	
Year 1-5 delivery		31,992 tpd		1,333.0 tph		Average Reclaim	
SG		2.64				9,749 gpm	
						Average Make-up	
						3,229 gpm	
Cyclone Feed				Underflow Tailings		Cyclone Overflow (Slimes)	
Total (wt)		109,938 tpd		20,862 tpd		35,633 tpd	
Water (wt)		77,946 tpd		6,258.55 tpd		28,677 tpd	
Solids (wt)		31,992 tpd		14,603 tpd		6,955 tpd	
		14,996 gpm		1,963 gpm		5,213 gpm	
		12,978 gpm		1,042 gpm		4,775 gpm	
		2,018 gpm		921 gpm		439 gpm	
Post Deposition				Post Deposition		Post Deposition	
Dry Unit Weight		95.00 pcf		31.70 pcf		74.60 pcf	
Vsolids		0.58 ft^3		0.19 ft^3		0.45 ft^3	
Vvoids		0.42 ft^3		0.81 ft^3		0.55 ft^3	
Void ratio		0.73		4.20		1.21	
Saturated Moisture (wt%)		28%		159%		46%	
Assumed Moisture (wt%)		18%		159%		46%	
Vwater		0.27 ft^3		0.81 ft^3		0.55 ft^3	
Total weight		112.10 lbs		82.09 lbs		108.74 lbs	
Entrained Water		84,250 ft^3/day		354,388 ft^3/day		153,045 ft^3/day	
Drainage and Evaporation		604.42 gpm		1,841 gpm		795 gpm	
Evaporation		156.31 gpm		2,934 gpm		6,367 gpm	
Drainage		448.11 gpm		Free Water		Free Water	
Year 6-11		Design Discharge Rate		1,222 tph			
		Cyclone Recovery		552.15 tph		Krebs	
Tailings Discharge		Overflow		670 tph			
		Total		1222 tph		Net (post concentrate recovery)	
						1206.0 tph	
Cyclone feed solids		29.10%				Factor for average TSF inputs	
						99%	
Cyclone Discharge						Result (entrainment only)	
Year 6-11 delivery		29,328 tpd		1,222.0 tph		Average Reclaim	
SG		2.64				8,922 gpm	
						Average Make-up	
						2,975 gpm	
Cyclone Feed				Underflow Tailings		Cyclone Overflow (Slimes)	
Total (wt)		100,784 tpd		18,931 tpd		32,742 tpd	
Water (wt)		71,456 tpd		5,679.26 tpd		26,312 tpd	
Solids (wt)		29,328 tpd		13,252 tpd		6,431 tpd	
		13,747 gpm		1,781 gpm		4,786 gpm	
		11,897 gpm		946 gpm		4,381 gpm	
		1,850 gpm		836 gpm		406 gpm	
Post Deposition				Post Deposition		Post Deposition	
Dry Unit Weight		95.00 pcf		31.70 pcf		74.60 pcf	
Vsolids		0.58 ft^3		0.19 ft^3		0.45 ft^3	
Vvoids		0.42 ft^3		0.81 ft^3		0.55 ft^3	
Void ratio		0.73		4.20		1.21	
Saturated Moisture (wt%)		28%		159%		46%	
Assumed Moisture (wt%)		18%		159%		46%	
Vwater		0.27 ft^3		0.81 ft^3		0.55 ft^3	
Total weight		112.10 lbs		82.09 lbs		108.74 lbs	
Entrained Water		76,452 ft^3/day		327,642 ft^3/day		141,495 ft^3/day	
Drainage and Evaporation		397 gpm		1,702 gpm		735 gpm	
Evaporation		548 gpm		2,679 gpm		5,836 gpm	
Drainage		406.63 gpm		Free Water		Free Water	

Table 3: Water Balance Calculations Rev 1.

Date: 12/16/2013 Project No.: 103-92557 Subject: Average Year Rainfall Conditions Project Short Title: Copper Flat Feasibility Study, New Mexico	Made by: 4/29/2013 RL Checked: 11/15/2013 GM Reviewed: 11/15/2013 BNS Page: 3 of 3
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Precipitation and Evaporation Data

Month	Days	Precip. (in)	Pan evap. (in)	Storms/Month	Precip/Storm	S	Runoff/storm (in)	Monthly runoff (in)
January	31	0.58	4.22	1	0.58	0.87	0.13	0.13
February	28	0.56	5.57	1	0.56	0.87	0.12	0.12
March	31	0.38	8.55	1	0.38	0.87	0.04	0.04
April	30	0.33	9.52	1	0.33	0.87	0.02	0.02
May	31	0.53	11.15	1	0.53	0.87	0.10	0.10
June	30	0.72	14.25	1	0.72	0.87	0.21	0.21
July	31	2.33	10.34	1	2.33	0.87	1.54	1.54
August	31	2.46	5.94	3	0.82	0.87	0.28	0.83
September	30	2.11	6.18	3	0.70	0.87	0.20	0.60
October	31	1.18	3.96	3	0.39	0.87	0.04	0.13
November	30	0.55	3.68	1	0.55	0.87	0.11	0.11
December	31	0.81	2.74	1	0.81	0.87	0.27	0.27

	Average Make-up (gpm)
	Result 3,169
	Entrainment 2924
	Precip/runon 2098
	Evap 4931
	Water in 12438

Notes:
 Tailings direct precip. reports 100% to water balance
 Impoundment refers to the interior, cyclone overflow and whole tailings storage area
 This model does not consider whole tailings discharge which accounts for approximately 7.5% of inflow, except as noted below.
 Average post deposition densities reflect periodic whole tailings discharge

Variables used in Water Balance

Water in Delivered Tailings Year 1-5	12,978	gpm
Water in Delivered Tailings Year 6-11	11,897	gpm
Beach Loss evap factor	50%	of Pan Evap
Surface area subject to wetting	40%	
Net discharge rate factor (post concentrate recovery)	99.0%	
Entrained water (Sand) Year 1-5	433	gpm
Entrained water (slimes) Year 1-5	1,822	gpm
Entrained water (beach) Year 1-5	787	gpm
Entrained water (Sand) Year 6-11	393	gpm
Entrained water (slimes) Year 6-11	1,685	gpm
Entrained water (beach) Year 6-11	728	gpm
TSF Free Water area	40%	of Tailings Area
Maximum Pond Area (Acre)	40	1,742,400 ft ²
TSF Free Water Pan Evap. coefficient	75%	
Reclaim Pump capacity	12,978	gpm
Minimum Water Storage	500,000	gal
CN-Native ground areas	92	
Dam Evap (underflow water) Year 1-5	155	gpm
Dam Evap (underflow water) Year 6-11	140	gpm

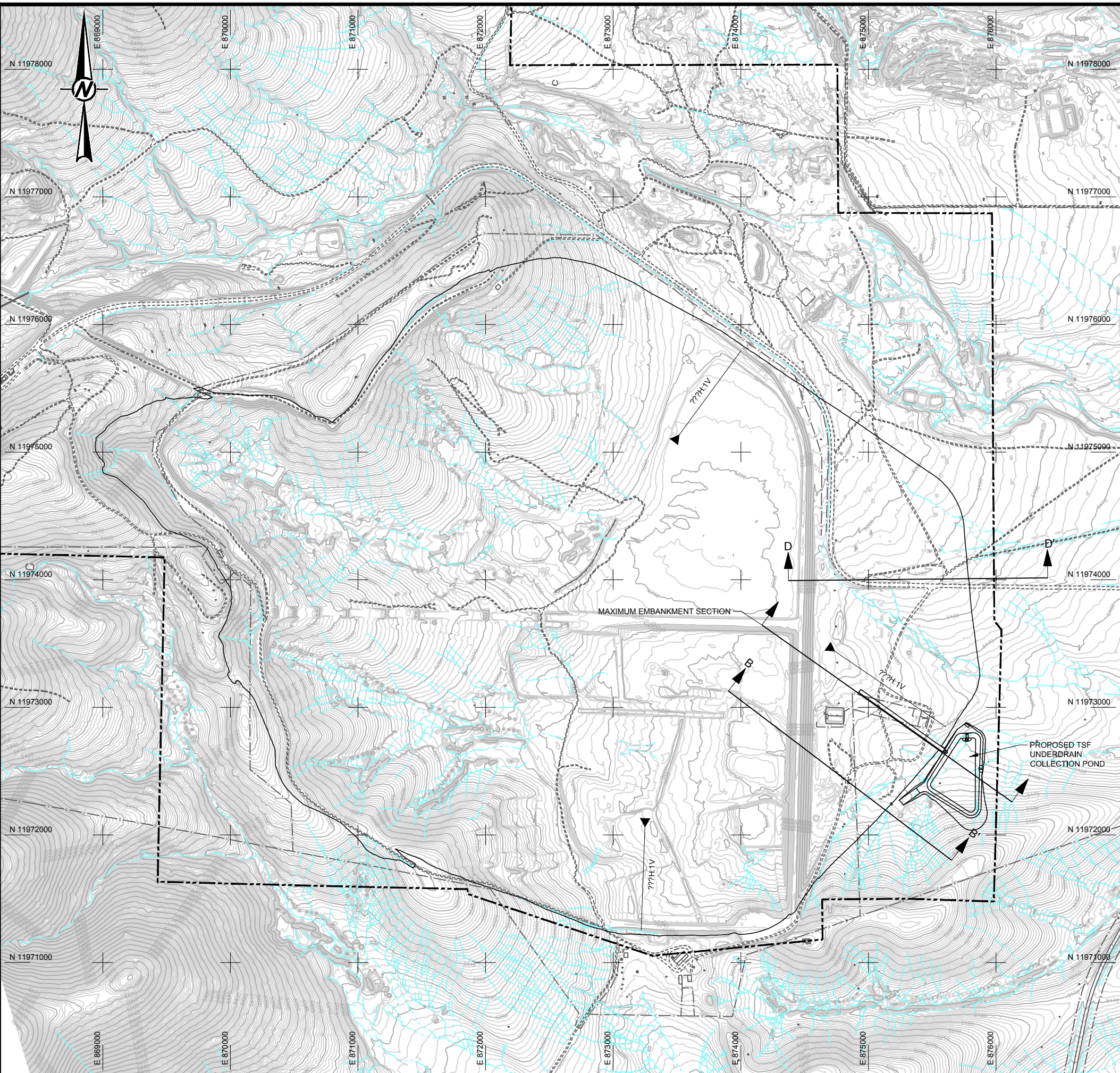
elev	Phase	Time	Total Imp. Area	Lined Area (ft ²)	Undiverted Area (ft ²)
5250	1	0.8		8,482,454	3,369,410
5280	2	1.8		10,415,749	1,317,126
5310	3	3.0	13,385,899	12,231,496	1,154,403
5340	4	4.5	13,385,899	12,964,378	421,521
5380	5	6.7	13,385,899	13,385,899	0
5435	6	9.74	13,385,899	13,385,899	
Results	Average Reclaim		9,215 gpm		
	Average Make-up		3,169 gpm		

Year	Month	Elapsed Years	Total Impoundment Area (ft ²)	Pond Area (ft ²)	Beach Area (ft ²)	Water Inflows				Water Losses				Monthly Balance at TSF (gal)	Water Reclaimed from TSF (gal)	End of Month Water Storage (gal)	Water Reclaimed from TSF (gpm)	Make-Up Water Required (gpm)
						Tailings Water (gal)	Direct Precip. (gal)	Runon from undiverted Area (gal)	Total Entrained Water (UF) (gal)	Free Water Pond Evap (gal)	Impoundment Beach Evap. (gal)	Embankment Evap. (gal)						
10	Jan-24	9.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
	Feb-24	9.16	13,385,899	1,742,400	11,643,499	479,701,263	4,672,895	-	113,125,473	4,537,481	6,064,297	5,661,782	355,485,125	354,985,125	500,000	8,804	3,093	
	Mar-24	9.25	13,385,899	1,742,400	11,643,499	531,097,826	3,170,893	-	125,246,059	6,965,074	9,308,750	6,268,402	386,980,435	386,480,435	500,000	8,658	3,240	
	Apr-24	9.33	13,385,899	1,742,400	11,643,499	513,965,638	2,753,670	-	121,205,864	7,755,264	10,364,830	6,066,195	371,827,156	371,327,156	500,000	8,596	3,302	
	May-24	9.41	13,385,899	1,742,400	11,643,499	531,097,826	4,422,562	-	125,246,059	9,083,108	12,139,481	6,268,402	383,283,338	382,783,338	500,000	8,575	3,322	
	Jun-24	9.50	13,385,899	1,742,400	11,643,499	513,965,638	6,008,008	-	121,205,864	11,608,456	15,514,583	6,066,195	366,078,548	365,578,548	500,000	8,462	3,435	
	Jul-24	9.58	13,385,899	1,742,400	11,643,499	531,097,826	19,442,582	-	125,246,059	8,423,259	11,257,599	6,268,402	399,845,090	399,345,090	500,000	8,946	2,951	
	Aug-24	9.67	13,385,899	1,742,400	11,643,499	531,097,826	20,527,362	-	125,246,059	4,838,893	6,467,132	6,268,402	409,304,702	408,804,702	500,000	9,158	2,740	
	Sep-24	9.75	13,385,899	1,742,400	11,643,499	513,965,638	17,606,802	-	121,205,864	5,034,404	6,728,430	6,066,195	393,037,547	392,537,547	500,000	9,087	2,811	
	Oct-24	9.83	13,385,899	1,742,400	11,643,499	531,097,826	9,846,458	-	125,246,059	3,225,929	4,311,421	6,268,402	402,392,473	401,892,473	500,000	9,003	2,894	
	Nov-24	9.92	13,385,899	1,742,400	11,643,499	513,965,638	4,589,451	-	121,205,864	2,997,833	4,006,573	6,066,195	384,778,624	384,278,624	500,000	8,895	3,002	
	Dec-24	10.00	13,385,899	1,742,400	11,643,499	531,097,826	6,759,009	-	125,246,059	2,232,082	2,983,155	6,268,402	401,627,138	401,127,138	500,000	8,986	2,912	
11	Jan-25	10.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
	Feb-25	10.16	13,385,899	1,742,400	11,643,499	479,701,263	4,672,895	-	113,125,473	4,537,481	6,064,297	5,661,782	355,485,125	354,985,125	500,000	8,804	3,093	
	Mar-25	10.25	13,385,899	1,742,400	11,643,499	531,097,826	3,170,893	-	125,246,059	6,965,074	9,308,750	6,268,402	386,980,435	386,480,435	500,000	8,658	3,240	
	Apr-25	10.33	13,385,899	1,742,400	11,643,499	513,965,638	2,753,670	-	121,205,864	7,755,264	10,364,830	6,066,195	371,827,156	371,327,156	500,000	8,596	3,302	
	May-25	10.41	13,385,899	1,742,400	11,643,499	531,097,826	4,422,562	-	125,246,059	9,083,108	12,139,481	6,268,402	383,283,338	382,783,338	500,000	8,575	3,322	
	Jun-25	10.50	13,385,899	1,742,400	11,643,499	513,965,638	6,008,008	-	121,205,864	11,608,456	15,514,583	6,066,195	366,078,548	365,578,548	500,000	8,462	3,435	
	Jul-25	10.58	13,385,899	1,742,400	11,643,499	531,097,826	19,442,582	-	125,246,059	8,423,259	11,257,599	6,268,402	399,845,090	399,345,090	500,000	8,946	2,951	
	Aug-25	10.67	13,385,899	1,742,400	11,643,499	531,097,826	20,527,362	-	125,246,059	4,838,893	6,467,132	6,268,402	409,304,702	408,804,702	500,000	9,158	2,740	
	Sep-25	10.75	13,385,899	1,742,400	11,643,499	513,965,638	17,606,802	-	121,205,864	5,034,404	6,728,430	6,066,195	393,037,547	392,537,547	500,000	9,087	2,811	
	Oct-25	10.83	13,385,899	1,742,400	11,643,499	531,097,826	9,846,458	-	125,246,059	3,225,929	4,311,421	6,268,402	402,392,473	401,892,473	500,000	9,003	2,894	
	Nov-25	10.92	13,385,899	1,742,400	11,643,499	513,965,638	4,589,451	-	121,205,864	2,997,833	4,006,573	6,066,195	384,778,624	384,278,624	500,000	8,895	3,002	
	Dec-25	11.00	13,385,899	1,742,400	11,643,499	531,097,826	6,759,009	-	125,246,059	2,232,082	2,983,155	6,268,402	401,627,138	401,127,138	500,000	8,986	2,912	
12	Jan-26	11.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
						542,619,654	8,173,890		127,890	127,590,729	5,826,860	7,227,106	6,434,351	404,240,459	403,740,459	500,000	9,215	3,169
									Average Make-up water								3,169	gpm
									Maximum Make-up water								3,676	gpm

Average (gpm)	12,368	186	3	2,909	133	165	147	9,215
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**APPENDIX H
STABILITY ANALYSIS SUPPORTING DATA
AND COMPUTER OUTPUT**

P:\ABO Projects\2015 Projects\1531453 THE MAC DP Permit Support\Supporting Documentation\Vol 1\Facility Design\10392557\202_10222015.dwg | Layout: H.1 STABILITY SECTION LOCATION PLAN | Modified: C:\MONTVOYA_10222015_845.AVI | Printed: C:\MONTVOYA_10222015

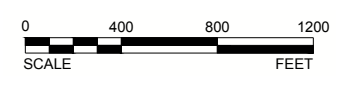


LEGEND

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE STABILITY SECTION LOCATION

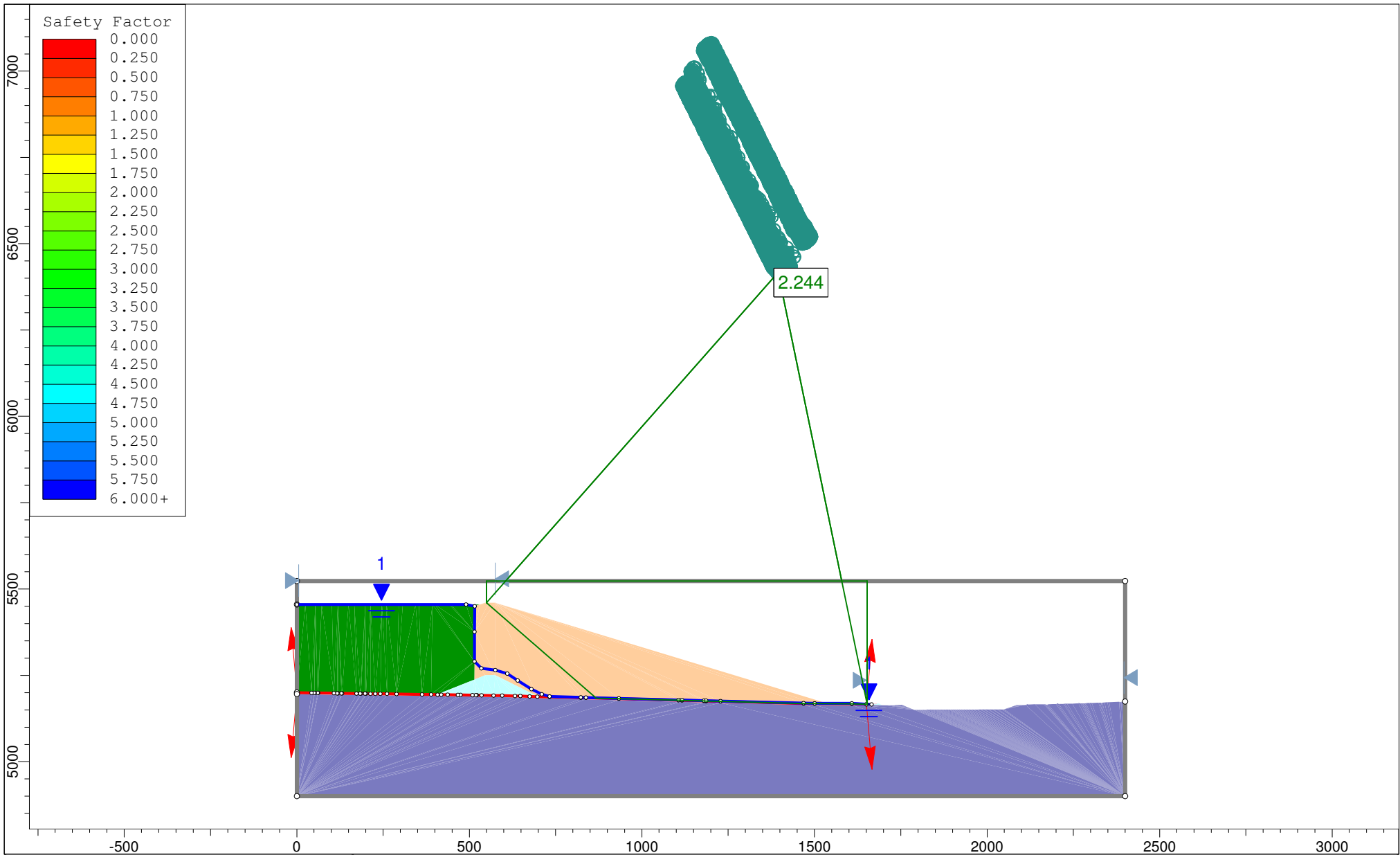
NOTES


- TAILINGS STORAGE FACILITY SHOWN AT FINAL BUILDOUT.



PROJECT DRAFT COPPER FLAT PROJECT 30 TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO			
STABILITY SECTION LOCATION PLAN			
		PROJECT No. 103-92557 DESIGN DMW 2013-04-30 CADD JHR 2013-04-30 CHECK GM 2013-05-07 REVIEW DAK 2013-05-07	FILE No. 10392557K202_10222015 SCALE AS SHOWN H.1

**APPENDIX H.1
MAXIMUM EMBANKMENT SECTION**



	Project			Copper Flat		
	Analysis Description					Section A Stability: Downstream, Static, Block Failure, Global
	Drawn By	GS	Scale	1:4620	Company	Golder Associates Inc.
	Date	11/4/2013, 3:02:21 PM		File Name	1a - SectionA 5460R DS_S_B_G.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 1a - SectionA 5460R DS_S_B_G.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Static, Block Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:
 103-92557
 Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

 Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None







Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 2.244240
 Axis Location: 1394.865, 6416.541
 Left Slip Surface Endpoint: 549.279, 5460.000
 Right Slip Surface Endpoint: 1652.746, 5166.147
 Left Slope Intercept: 549.279 5522.990
 Right Slope Intercept: 1652.746 5522.990
 Resisting Moment=8.08765e+009 lb-ft

Driving Moment=3.60374e+009 lb-ft
 Resisting Horizontal Force=5.71009e+006 lb
 Driving Horizontal Force=2.54433e+006 lb

Global Minimum Coordinates

Method: spencer

X	Y
549.279	5460
868.036	5183.72
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1608.27	5167.61
1623.49	5166.77
1652.75	5166.15
1652.75	5522.99

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 757
 Number of Invalid Surfaces: 4064

Error Codes:

- Error Code -108 reported for 7 surfaces
- Error Code -111 reported for 2282 surfaces
- Error Code -112 reported for 19 surfaces
- Error Code -116 reported for 1756 surfaces

Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1+\tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	52.9277	124924	Cyclone Underflow	0	39	591.131	1326.64	1638.28	0	1638.28
2	52.9277	315093	Cyclone Underflow	0	39	1658.67	3722.45	4596.85	0	4596.85
3	52.9277	493540	Cyclone Underflow	0	39	2613.17	5864.58	7242.16	0	7242.16
4	52.9277	671986	Cyclone Underflow	0	39	3567.68	8006.72	9887.48	0	9887.48
5	52.9277	850433	Cyclone Underflow	0	39	4522.2	10148.9	12532.8	0	12532.8
6	52.9277	1.02888e+006	Cyclone Underflow	0	39	5476.69	12291	15178.1	0	15178.1
7	1.19075	25204	Liner Interface Zone	0	26.5	4101.89	9205.63	18494.8	31.1774	18463.6
8	65.4393	1.32103e+006	Liner Interface Zone	0	26.5	4296.98	9643.45	19404.1	62.3547	19341.7
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	3873.77	8693.66	17499.2	62.3583	17436.9
10	57.4662	946268	Liner Interface Zone	0	26.5	3476.66	7802.45	15711.7	62.3583	15649.3
11	57.4662	842833	Liner Interface Zone	0	26.5	3079.55	6911.24	13924.2	62.3583	13861.8
12	10.1889	138642	Liner Interface Zone	0	26.5	2845.74	6386.52	12871.8	62.3576	12809.4
13	63.358	789325	Liner Interface Zone	0	26.5	2592.07	5817.23	11729.9	62.3559	11667.6
14	6.66227	75711.8	Liner Interface Zone	0	26.5	2350.93	5276.05	10644.5	62.3581	10582.1
15	41.4898	440289	Liner Interface Zone	0	26.5	2184.77	4903.15	9896.55	62.3559	9834.2
16	48.1756	443630	Liner Interface Zone	0	26.5	1875.42	4208.9	8504.11	62.3594	8441.75
17	48.1756	370848	Liner Interface Zone	0	26.5	1542.11	3460.86	7003.76	62.3594	6941.4
18	48.1756	298067	Liner Interface Zone	0	26.5	1208.79	2712.81	5503.4	62.3594	5441.04
19	48.1756	225285	Liner Interface Zone	0	26.5	875.468	1964.76	4003.07	62.3594	3940.71
20	48.1756	152503	Liner Interface Zone	0	26.5	542.148	1216.71	2502.71	62.3594	2440.36
21	31.9454	59717.2	Liner Interface Zone	0	26.5	256.986	576.739	1219.16	62.3993	1156.76

22	53.9361	28069.4	Liner Interface Zone	0	26.5	6.26742	14.0656	90.6102	62.3991	28.2111
23	53.9361	6472.33	Liner Interface Zone	0	26.5	10.742	24.1077	110.752	62.3991	48.3526
24	15.217	1826.04	Liner Interface Zone	0	26.5	0	0	0.432806	62.2097	-61.7769
25	29.2604	1755.63	Liner Interface Zone	0	26.5	2857.95	6413.92	12895.4	31.1049	12864.3

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	549.279	5460	1.98387e-022	0	0
2	602.206	5414.12	-113840	11116	-5.57701
3	655.134	5368.25	-69899.2	6825.37	-5.57702
4	708.062	5322.37	28843.8	-2816.47	-5.577
5	760.99	5276.5	182389	-17809.5	-5.57701
6	813.917	5230.62	390735	-38153.7	-5.57702
7	866.845	5184.75	653884	-63849.1	-5.57702
8	868.036	5183.72	664679	-64903.2	-5.57702
9	933.475	5181.95	217883	-21275.3	-5.577
10	990.941	5180.47	-174451	17034.4	-5.57701
11	1048.41	5178.98	-565551	55223.8	-5.57702
12	1105.87	5177.5	-955418	93292.6	-5.57701
13	1116.06	5177.23	-1.02438e+006	100027	-5.57705
14	1179.42	5175.55	-1.45202e+006	141784	-5.57703
15	1186.08	5175.37	-1.49696e+006	146172	-5.57702
16	1227.57	5174.27	-1.77617e+006	173435	-5.57699
17	1275.75	5173.04	-2.10008e+006	205064	-5.57701
18	1323.92	5171.81	-2.42308e+006	236604	-5.57702
19	1372.1	5170.58	-2.74518e+006	268055	-5.57701
20	1420.28	5169.35	-3.06636e+006	299418	-5.57703
21	1468.45	5168.13	-3.38664e+006	330691	-5.57701
22	1500.4	5168.02	-3.59915e+006	351442	-5.57701
23	1554.33	5167.81	-3.83666e+006	374634	-5.57702
24	1608.27	5167.61	-3.84174e+006	375130	-5.57702
25	1623.49	5166.77	-3.86037e+006	376950	-5.57703
26	1652.75	5166.15	6.36684e-021	0	0

List Of Coordinates

Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63

632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

External Boundary

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5

0	5195.5
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Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23

1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54

1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6

2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary



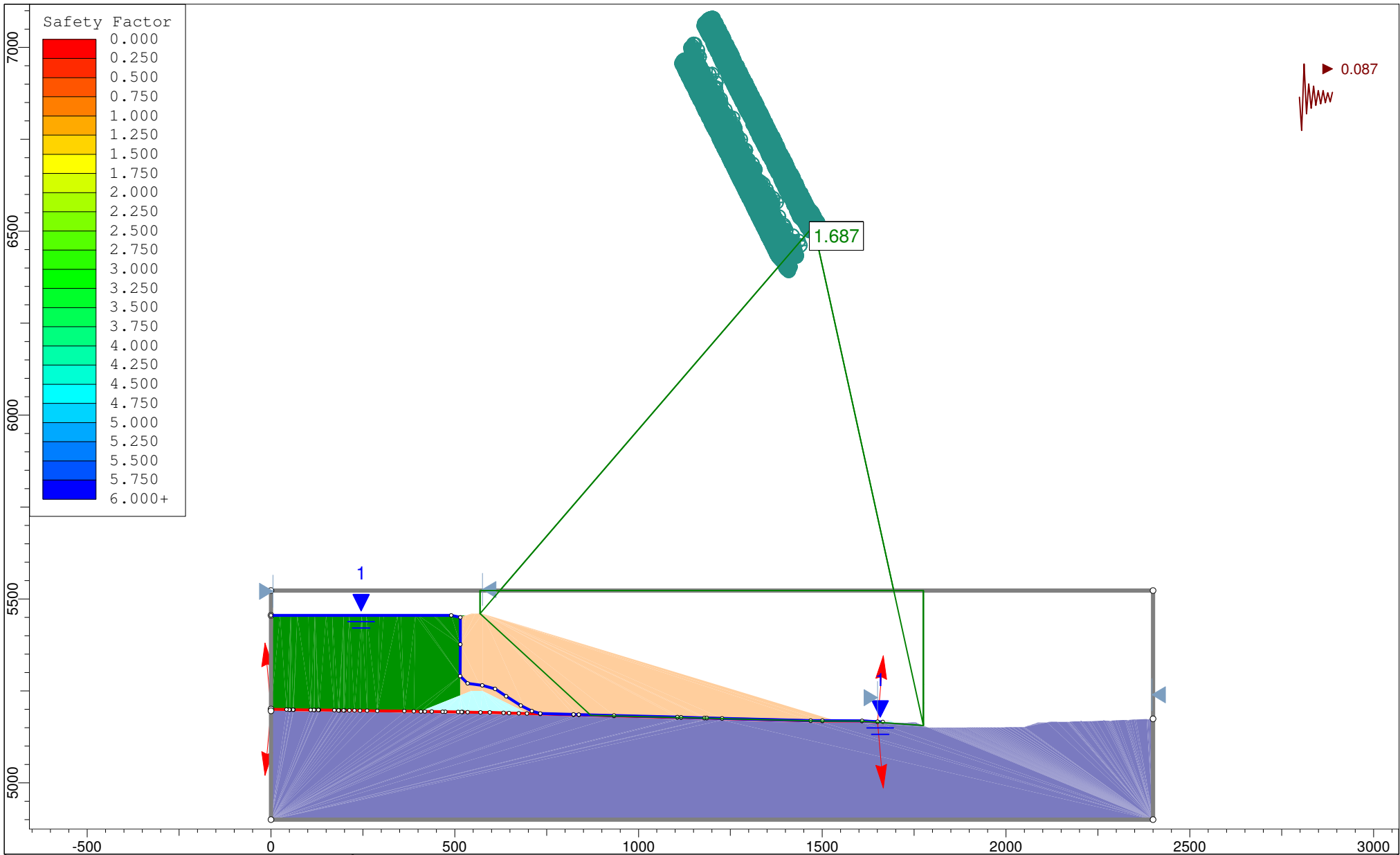
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83


Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



	Project			Copper Flat		
	Analysis Description			Section A Stability: Downstream, Pseudo Static, Block Failure, Global		
	Drawn By	GS	Scale	1:4338	Company	Golder Associates Inc.
	Date	11/4/2013, 3:02:21 PM		File Name	2a - SectionA 5460R DS_PS_B_G.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 2a - SectionA 5460R_DS_PS_B_G.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:
 103-92557
 Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

 Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3







Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Loading

Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 1.686520

Axis Location: 1476.350, 6514.282
 Left Slip Surface Endpoint: 568.826, 5460.000
 Right Slip Surface Endpoint: 1775.261, 5155.693
 Left Slope Intercept: 568.826 5522.990
 Right Slope Intercept: 1775.261 5522.990
 Resisting Moment=8.24296e+009 lb-ft
 Driving Moment=4.88754e+009 lb-ft
 Resisting Horizontal Force=5.38918e+006 lb
 Driving Horizontal Force=3.19543e+006 lb

Global Minimum Coordinates

Method: spencer

X	Y
568.826	5460
870.106	5183.66
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1601.92	5167.63
1775.26	5155.69
1775.26	5522.99

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 518
 Number of Invalid Surfaces: 4303

Error Codes:

- Error Code -108 reported for 22 surfaces
- Error Code -111 reported for 2509 surfaces
- Error Code -112 reported for 16 surfaces
- Error Code -116 reported for 1756 surfaces

Error Codes

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

- 111 = safety factor equation did not converge
- 112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1+\tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	50.0261	97432.4	Cyclone Underflow	0	39	554.036	934.392	1153.88	0	1153.88
2	50.0261	271871	Cyclone Underflow	0	39	1829.51	3085.51	3810.28	0	3810.28
3	50.0261	445560	Cyclone Underflow	0	39	3022.48	5097.47	6294.86	0	6294.86
4	50.0261	619248	Cyclone Underflow	0	39	4215.44	7109.43	8779.42	0	8779.42
5	50.0261	792937	Cyclone Underflow	0	39	5408.41	9121.4	11264	0	11264
6	50.0261	966626	Cyclone Underflow	0	39	6601.4	11133.4	13748.6	0	13748.6
7	1.12325	23701.6	Liner Interface Zone	0	26.5	5119.91	8634.83	17350	31.1774	17318.8
8	63.369	1.27719e+006	Liner Interface Zone	0	26.5	5707.87	9626.44	19369.9	62.3547	19307.6
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	5154.91	8693.86	17499.6	62.3583	17437.2
10	57.4662	946268	Liner Interface Zone	0	26.5	4626.75	7803.1	15713	62.3583	15650.6
11	57.4662	842833	Liner Interface Zone	0	26.5	4098.59	6912.35	13926.4	62.3583	13864
12	10.1889	138642	Liner Interface Zone	0	26.5	3787.56	6387.79	12874.2	62.3576	12811.9
13	63.358	789325	Liner Interface Zone	0	26.5	3450.05	5818.58	11732.6	62.3559	11670.3
14	6.66227	75711.8	Liner Interface Zone	0	26.5	3129.49	5277.95	10648.3	62.3581	10585.9
15	41.4898	440289	Liner Interface Zone	0	26.5	2908.36	4905.01	9900.29	62.3559	9837.93
16	60.2195	543166	Liner Interface Zone	0	26.5	2441.73	4118.02	8321.82	62.3594	8259.46
17	60.2195	429444	Liner Interface Zone	0	26.5	1887.56	3183.41	6447.29	62.3594	6384.93
18	60.2195	315723	Liner Interface Zone	0	26.5	1333.4	2248.8	4572.76	62.3594	4510.4
			Liner Interface							

		Zone								
20	31.9454	59717.2	Liner Interface Zone	0	26.5	345.09	582.001	1229.71	62.3993	1167.31
21	50.7606	27688.4	Liner Interface Zone	0	26.5	9.96282	16.8025	96.0997	62.3991	33.7006
22	50.7606	6091.28	Liner Interface Zone	0	26.5	14.4972	24.4499	111.438	62.3991	49.0388
23	62.898	13110.9	Liner Interface Zone	0	26.5	0	0	102.782	109.089	-6.30725
24	55.2228	27974.9	Foundation Materials	150	29	213.592	360.228	379.262	0	379.262
25	55.2228	30469.2	Foundation Materials	150	29	2490.45	4200.2	7306.76	0	7306.76

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	568.826	5460	1.98387e-022	0	0
2	618.852	5414.12	-147187	14297.2	-5.54809
3	668.878	5368.23	-119420	11600	-5.54809
4	718.904	5322.35	-36557.4	3551.05	-5.54809
5	768.93	5276.46	101402	-9849.77	-5.54807
6	818.957	5230.58	294457	-28602.4	-5.54808
7	868.983	5184.69	542608	-52706.9	-5.54809
8	870.106	5183.66	553563	-53771	-5.54809
9	933.475	5181.95	142080	-13801.1	-5.54808
10	990.941	5180.47	-232631	22596.9	-5.54809
11	1048.41	5178.98	-607567	59016.7	-5.54808
12	1105.87	5177.5	-982726	95458.3	-5.54809
13	1116.06	5177.23	-1.04924e+006	101919	-5.54808
14	1179.42	5175.55	-1.46262e+006	142073	-5.54807
15	1186.08	5175.37	-1.50616e+006	146303	-5.5481
16	1227.57	5174.27	-1.77711e+006	172622	-5.5481
17	1287.79	5172.73	-2.17125e+006	210907	-5.54809
18	1348.01	5171.2	-2.56556e+006	249209	-5.54809
19	1408.23	5169.66	-2.96006e+006	287529	-5.54809
20	1468.45	5168.13	-3.35475e+006	325867	-5.54808
21	1500.4	5168.02	-3.56488e+006	346279	-5.54809
22	1551.16	5167.82	-3.79988e+006	369105	-5.54807
23	1601.92	5167.63	-3.80432e+006	369537	-5.54809
24	1664.82	5163.3	-3.87274e+006	376183	-5.54808
25	1720.04	5159.5	-3.93678e+006	382404	-5.54809
26	1775.26	5155.69	6.74534e-021	0	0

List Of Coordinates

Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92

732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

External Boundary

X	Y
0	4900
2400	4900
2400	5173.98

2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92

837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5

1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61

2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83

2396.71	5174
2400	5173.98

Material Boundary

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199

117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8

1675.27	5168.86
1680.63	5168.86

Material Boundary

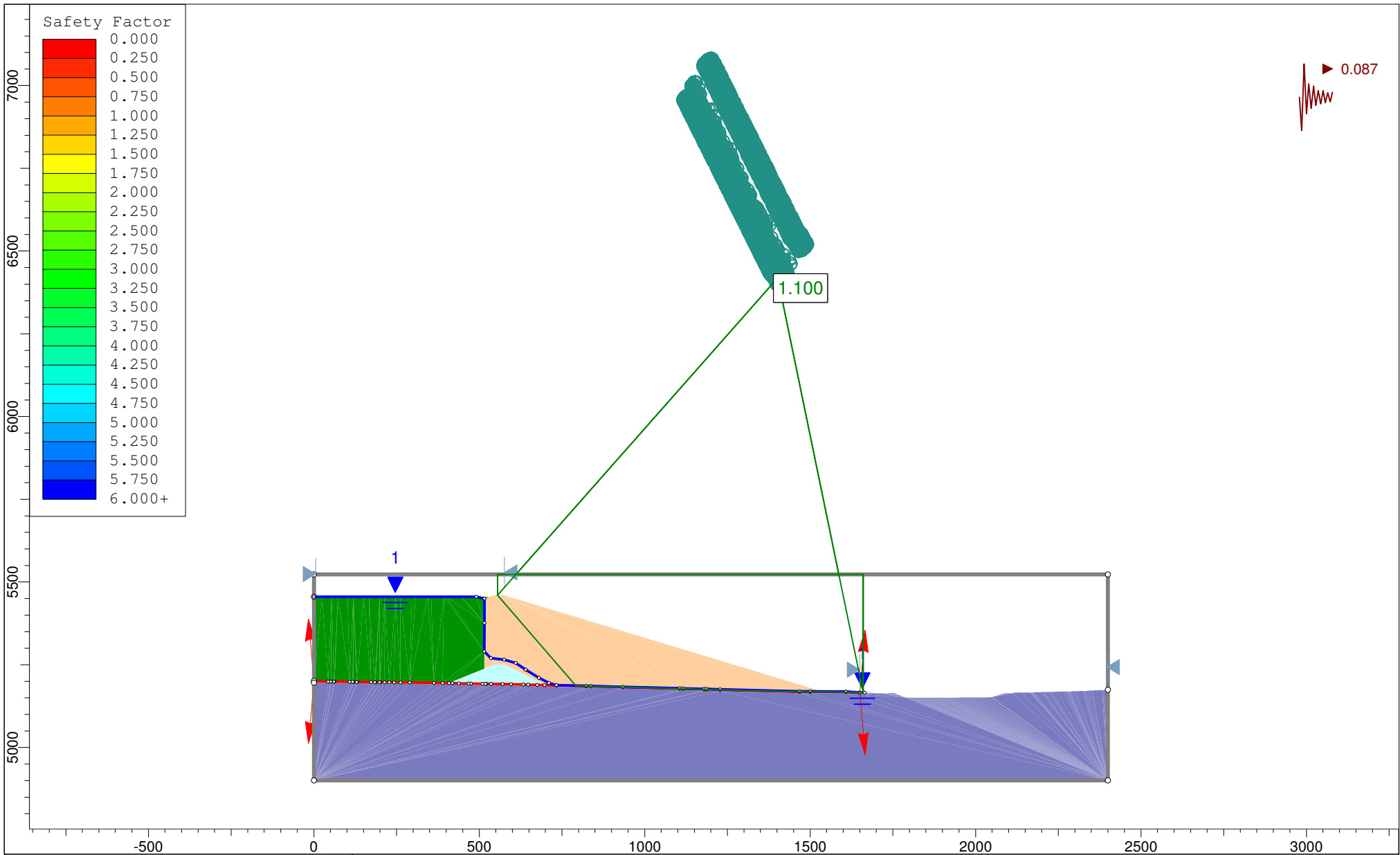
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83


Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



	Project			Copper Flat														
	Analysis Description						Section A Stability: Downstream, Pseudo Static, Block Failure, Global											
	Drawn By			GS			Scale			1:4820			Company			Golder Associates Inc.		
	Date			11/4/2013, 3:02:21 PM			File Name			3a - SectionA 5460R DS_PS_B_G_min_phi.slim								

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 3a - SectionA 5460R_DS_PS_B_G_min_phi.slim
Last saved with Slide version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:

103-92557
Material Property Edits 12/2013
Liner interface min phi=13.6deg

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer

Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3







Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Loading

Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

List Of Coordinates

Piezoline

X	Y

0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03

595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

External Boundary

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary



X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary



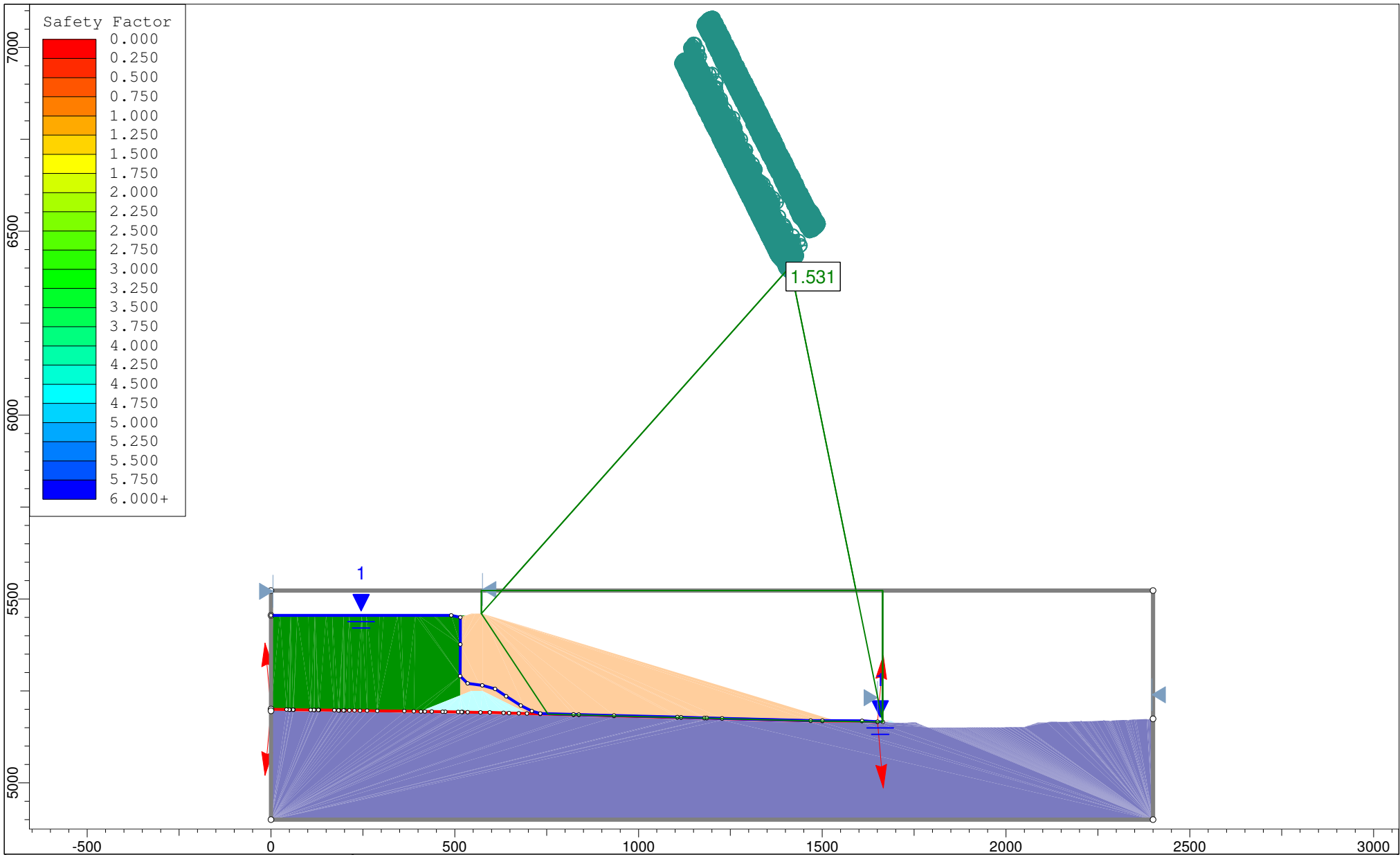
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Block Failure, Global	
Drawn By	GS	Scale	1:4338
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	3b - SectionA 5460R DS_S_B_G_min_phi.slim

Slide Analysis Information

Copper Flat

Project Summary

File Name: 3b - SectionA 5460R DS_S_B_G_min_phi.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Static, Block Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:

103-92557
Material Property Edits 12/2013
Liner Interface phi =13.6deg

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer

Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check malpha < 0.2: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None







Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 1.530610
 Axis Location: 1412.961, 6404.397
 Left Slip Surface Endpoint: 572.648, 5460.000
 Right Slip Surface Endpoint: 1664.291, 5165.509
 Left Slope Intercept: 572.648 5522.990
 Right Slope Intercept: 1664.291 5522.990
 Resisting Moment=5.18841e+009 lb-ft

Driving Moment=3.38976e+009 lb-ft
 Resisting Horizontal Force=3.37882e+006 lb
 Driving Horizontal Force=2.20749e+006 lb

Global Minimum Coordinates

Method: spencer

X	Y
572.648	5460
753.658	5186.79
823.107	5184.92
837.37	5184.54
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1456.16	5168.44
1664.29	5165.51
1664.29	5522.99

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 1279
 Number of Invalid Surfaces: 3542

Error Codes:

- Error Code -108 reported for 11 surfaces
- Error Code -111 reported for 1763 surfaces
- Error Code -112 reported for 12 surfaces
- Error Code -116 reported for 1756 surfaces

Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha = $\cos(\alpha)(1+\tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	45.0838	142718	Cyclone Underflow	0	39	872.365	1335.25	1648.9	0	1648.9
2	45.0838	419922	Cyclone Underflow	0	39	2833.01	4336.24	5354.81	0	5354.81
3	45.0838	696992	Cyclone Underflow	0	39	4722.56	7228.4	8926.34	0	8926.34
4	45.0838	974063	Cyclone Underflow	0	39	6612.14	10120.6	12497.8	0	12497.8
5	0.674563	16680.5	Liner Interface Zone	0	13.6	3447.02	5276.04	21839.8	31.1774	21808.6
6	69.4483	1.64551e+006	Liner Interface Zone	0	13.6	3646.43	5581.26	23132.6	62.3548	23070.2
7	14.2633	319322	Liner Interface Zone	0	13.6	3440.05	5265.38	21826.8	62.3568	21764.5
8	48.0526	1.02905e+006	Liner Interface Zone	0	13.6	3286.15	5029.81	20853.1	62.3547	20790.8
9	48.0526	957013	Liner Interface Zone	0	13.6	3049.14	4667.05	19353.6	62.3547	19291.3
10	57.4662	1.0497e+006	Liner Interface Zone	0	13.6	2788.59	4268.24	17705.2	62.3583	17642.8
11	57.4662	946268	Liner Interface Zone	0	13.6	2504.03	3832.69	15904.8	62.3583	15842.4
12	57.4662	842833	Liner Interface Zone	0	13.6	2219.46	3397.13	14104.4	62.3583	14042.1
13	10.1889	138642	Liner Interface Zone	0	13.6	2051.94	3140.72	13044.6	62.3576	12982.2
14	63.358	789325	Liner Interface Zone	0	13.6	1870.22	2862.57	11894.8	62.3559	11832.4
15	6.66227	75711.8	Liner Interface Zone	0	13.6	1697.36	2597.99	10801.2	62.3581	10738.8
16	41.4898	440289	Liner Interface Zone	0	13.6	1578.34	2415.83	10048.2	62.3559	9985.82
17	45.7181	422762	Liner Interface Zone	0	13.6	1362.68	2085.73	8683.74	62.3594	8621.38
18	45.7181	357216	Liner Interface Zone	0	13.6	1136.02	1738.8	7249.7	62.3594	7187.34
19	45.7181	291670	Liner Interface Zone	0	13.6	909.35	1391.86	5815.63	62.3594	5753.27
20	45.7181	226124	Liner Interface Zone	0	13.6	682.689	1044.93	4381.59	62.3594	4319.23
21	45.7181	160579	Liner Interface Zone	0	13.6	456.025	697.997	2947.52	62.3594	2885.17
			Liner Interface							

		Zone								
23	52.032	22869.3	Liner Interface Zone	0	13.6	5.72276	8.75932	132.838	96.6312	36.2067
24	52.032	12959.2	Liner Interface Zone	0	13.6	16.2967	24.9439	233.108	130.003	103.105
25	52.032	6697.29	Liner Interface Zone	0	13.6	1011.39	1548.04	6465.57	66.7275	6398.84

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	572.648	5460	1.98387e-022	0	0
2	617.732	5391.95	-106388	9073.38	-4.87472
3	662.816	5323.9	59899.5	-5108.59	-4.87473
4	707.9	5255.86	372435	-31763.5	-4.87473
5	752.984	5187.81	831219	-70891.4	-4.87474
6	753.658	5186.79	849640	-72462.5	-4.87474
7	823.107	5184.92	472351	-40285	-4.87474
8	837.37	5184.54	393696	-33576.8	-4.87474
9	885.422	5183.25	126906	-10823.3	-4.87473
10	933.475	5181.95	-143674	12253.4	-4.87474
11	990.941	5180.47	-473336	40368.9	-4.87473
12	1048.41	5178.98	-808251	68932.5	-4.87473
13	1105.87	5177.5	-1.14842e+006	97944.2	-4.87473
14	1116.06	5177.23	-1.20925e+006	103132	-4.87473
15	1179.42	5175.55	-1.59087e+006	135679	-4.87473
16	1186.08	5175.37	-1.63143e+006	139138	-4.87473
17	1227.57	5174.27	-1.88531e+006	160790	-4.87472
18	1273.29	5173.1	-2.16872e+006	184961	-4.87472
19	1319.01	5171.94	-2.45542e+006	209413	-4.87473
20	1364.73	5170.77	-2.74542e+006	234146	-4.87473
21	1410.44	5169.6	-3.03871e+006	259160	-4.87474
22	1456.16	5168.44	-3.3353e+006	284454	-4.87472
23	1508.2	5167.71	-3.67744e+006	313635	-4.87474
24	1560.23	5166.97	-3.86453e+006	329590	-4.87473
25	1612.26	5166.24	-3.87412e+006	330409	-4.87474
26	1664.29	5165.51	6.38965e-021	0	0

List Of Coordinates

Piezoline

X	Y
---	---

0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03

595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

External Boundary

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary



X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary



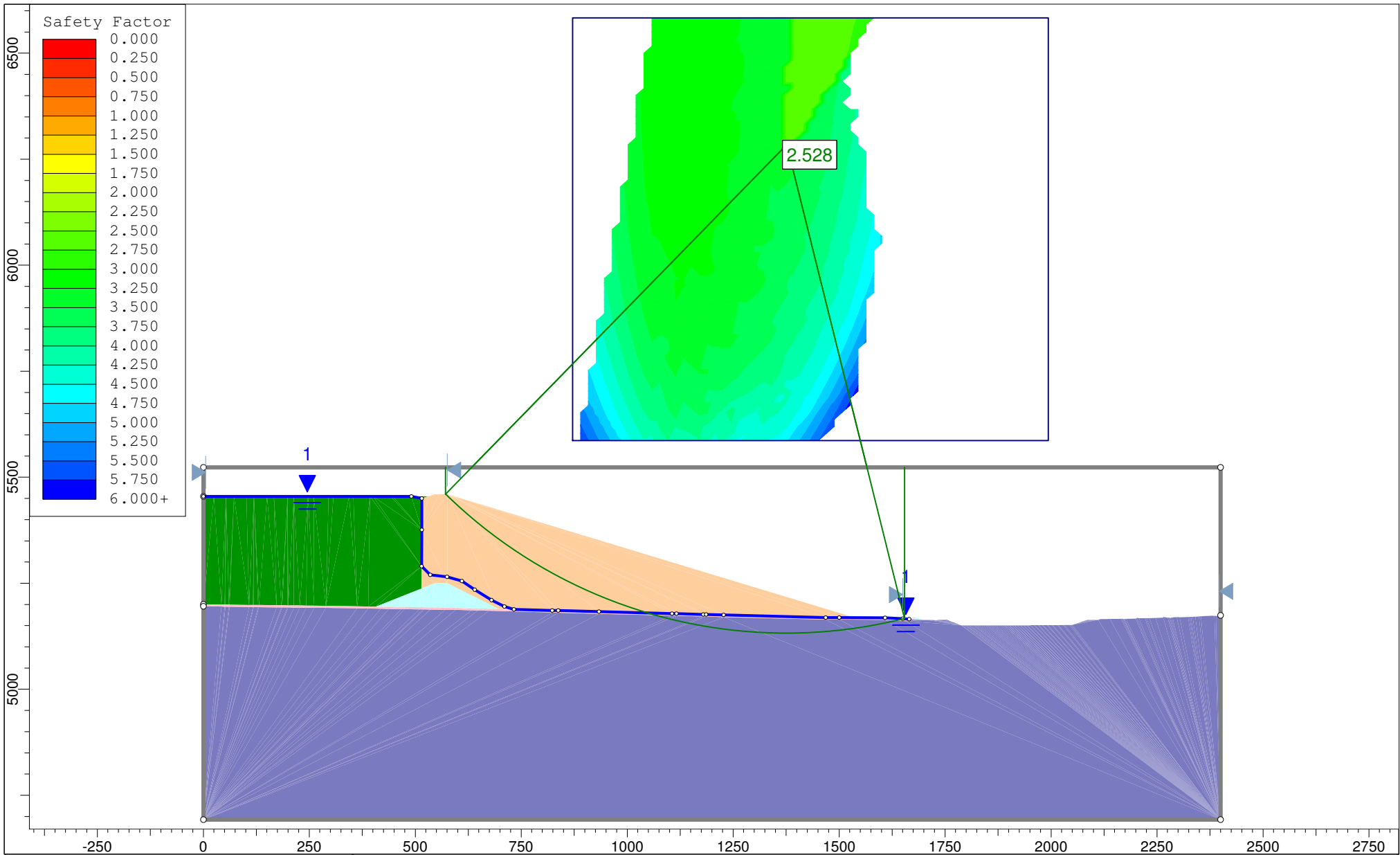
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project				Copper Flat			
Analysis Description				Section A Stability: Downstream, Static, Circular Failure, Global			
Drawn By		GS		Scale		1:3762	
Company				Golder Associates Inc.			
Date		11/4/2013, 3:02:21 PM		File Name		4a - SectionA 5460R DS_S_C_G.slim	

Slide Analysis Information

Copper Flat

Project Summary

File Name: 4a - SectionA 5460R DS_S_C_G.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Static, Circular Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:
 103-92557
 Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

 Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None







Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
 Search Method: Grid Search
 Radius Increment: 10
 Composite Surfaces: Disabled
 Reverse Curvature: Create Tension Crack
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 2.527760
 Center: 1376.188, 6284.432
 Radius: 1152.416
 Left Slip Surface Endpoint: 570.971, 5460.000
 Right Slip Surface Endpoint: 1654.244, 5166.064
 Left Slope Intercept: 570.971 5522.990
 Right Slope Intercept: 1654.244 5522.990
 Resisting Moment=8.33108e+009 lb-ft
 Driving Moment=3.29583e+009 lb-ft
 Resisting Horizontal Force=6.7452e+006 lb

Driving Horizontal Force=2.66845e+006 lb

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 11730

Number of Invalid Surfaces: 29201

Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4317 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3813 surfaces
- Error Code -1000 reported for 17897 surfaces

Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.216	953.512	1177.49	0	1177.49
2	47.6079	233141	Cyclone Underflow	0	39	1249.89	3159.43	3901.57	0	3901.57
3	47.6079	356305	Cyclone Underflow	0	39	1954.83	4941.33	6102.03	0	6102.03
4	47.6079	458937	Cyclone Underflow	0	39	2561.02	6473.65	7994.29	0	7994.29
5	47.6079	543134	Cyclone Underflow	0	39	3074.2	7770.84	9596.18	0	9596.18

6	47.6079	610541	Cyclone Underflow	0	39	3499.06	8844.78	10922.4	0	10922.4
7	47.6079	662469	Cyclone Underflow	0	39	3839.45	9705.2	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.49	10360	12793.5	0	12793.5
9	47.6079	723916	Cyclone Underflow	0	39	4278.69	10815.5	13356	0	13356
10	47.6079	734997	Cyclone Underflow	0	39	4381.94	11076.5	13678.4	0	13678.4
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.41	7119.2	14434.9	155.948	14278.9
12	43.9418	677489	Foundation Materials	150	29	3185.32	8051.73	14255.1	0	14255.1
13	43.9418	666065	Foundation Materials	150	29	3142.02	7942.28	14057.7	0	14057.7
14	43.9418	645159	Foundation Materials	150	29	3052.52	7716.04	13649.5	0	13649.5
15	43.9418	614993	Foundation Materials	150	29	2917.32	7374.28	13032.9	0	13032.9
16	43.9418	575744	Foundation Materials	150	29	2736.7	6917.73	12209.3	0	12209.3
17	43.9418	527526	Foundation Materials	150	29	2510.7	6346.44	11178.7	0	11178.7
18	43.9418	470419	Foundation Materials	150	29	2239.12	5659.95	9940.21	0	9940.21
19	43.9418	404462	Foundation Materials	150	29	1921.59	4857.32	8492.24	0	8492.24
20	43.9418	329659	Foundation Materials	150	29	1557.52	3937.03	6831.97	0	6831.97
21	43.9418	246078	Foundation Materials	150	29	1146.59	2898.3	4958.07	0	4958.07
22	43.9418	156715	Foundation Materials	150	29	744.157	1881.05	3122.9	0	3122.9
23	43.9418	104701	Foundation Materials	150	29	581.701	1470.4	2382.07	0	2382.07
24	43.9418	56711	Foundation Materials	150	29	328.326	829.93	1226.63	0	1226.63
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4370.71	11048.1	22314.9	155.762	22159.1

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146707	13731.9	-5.34735

3	666.187	5376.71	-127700	11952.8	-5.34734
4	713.795	5341.41	-93307	8733.58	-5.34733
5	761.402	5309.7	-62718.8	5870.51	-5.34733
6	809.01	5281.25	-50008.5	4680.82	-5.34733
7	856.618	5255.79	-65397.2	6121.21	-5.34733
8	904.226	5233.09	-116109	10867.9	-5.34735
9	951.834	5212.99	-206964	19371.9	-5.34732
10	999.442	5195.34	-340792	31898.3	-5.34733
11	1047.05	5180.02	-518731	48553.5	-5.34734
12	1066.08	5174.52	-568159	53179.9	-5.34733
13	1110.02	5163.18	-727759	68118.6	-5.34733
14	1153.96	5153.65	-924346	86519.3	-5.34734
15	1197.9	5145.89	-1.15617e+006	108218	-5.34733
16	1241.85	5139.87	-1.42056e+006	132965	-5.34733
17	1285.79	5135.57	-1.71391e+006	160423	-5.34734
18	1329.73	5132.95	-2.03175e+006	190173	-5.34734
19	1373.67	5132.02	-2.36865e+006	221707	-5.34734
20	1417.61	5132.76	-2.71825e+006	254430	-5.34734
21	1461.56	5135.18	-3.07316e+006	287649	-5.34733
22	1505.5	5139.29	-3.42492e+006	320574	-5.34733
23	1549.44	5145.11	-3.67939e+006	344392	-5.34732
24	1593.38	5152.67	-3.72663e+006	348815	-5.34734
25	1637.32	5161.99	-3.78937e+006	354686	-5.34732
26	1654.24	5166.06	6.3698e-021	0	0

List Of Coordinates

Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5

1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

External Boundary

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27

1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455

530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary

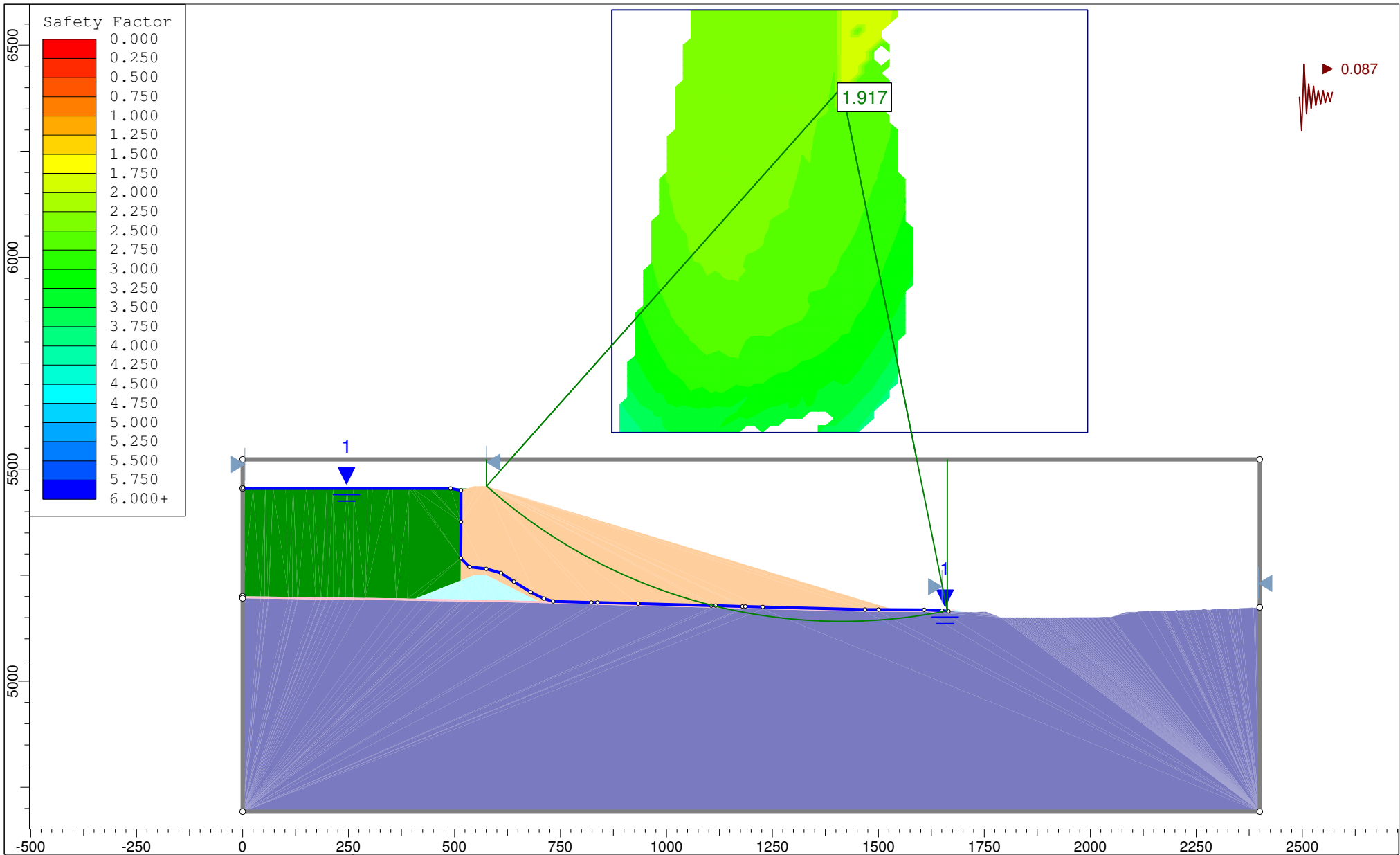
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Pseudo Static, Circular Failure, Global	
Drawn By	GS	Scale	1:3762
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	5a - SectionA 5460R DS_PS_C_G.slim

Slide Analysis Information

Copper Flat

Project Summary

File Name: 5a - SectionA 5460R_DS_PS_C_G.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Pseudo Static, Circular Failure, Global
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:
 103-92557
 Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

 Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3







Surface Options

Surface Type: Circular
 Search Method: Grid Search
 Radius Increment: 10
 Composite Surfaces: Disabled
 Reverse Curvature: Create Tension Crack
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Loading

Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 1.916600
 Center: 1413.608, 6400.750
 Radius: 1259.972
 Left Slip Surface Endpoint: 575.445, 5460.000

Right Slip Surface Endpoint: 1662.506, 5165.607
 Left Slope Intercept: 575.445 5522.990
 Right Slope Intercept: 1662.506 5522.990
 Resisting Moment=7.97722e+009 lb-ft
 Driving Moment=4.16218e+009 lb-ft
 Resisting Horizontal Force=5.93993e+006 lb
 Driving Horizontal Force=3.0992e+006 lb

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 10557
 Number of Invalid Surfaces: 30374

Error Codes:

Error Code -101 reported for 408 surfaces
 Error Code -103 reported for 2326 surfaces
 Error Code -108 reported for 4489 surfaces
 Error Code -110 reported for 440 surfaces
 Error Code -111 reported for 4814 surfaces
 Error Code -1000 reported for 17897 surfaces

Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.9166

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	44.3652	60862.7	Cyclone Underflow	0	39	321.259	615.725	760.358	0	760.358
2	44.3652	174209	Cyclone Underflow	0	39	1244.73	2385.64	2946.02	0	2946.02
			Cyclone							

			Underflow								
4	44.3652	354600	Cyclone Underflow	0	39	2655.19	5088.94	6284.31	0	6284.31	
5	44.3652	424335	Cyclone Underflow	0	39	3230.76	6192.08	7646.58	0	7646.58	
6	44.3652	481812	Cyclone Underflow	0	39	3723.19	7135.87	8812.07	0	8812.07	
7	44.3652	527840	Cyclone Underflow	0	39	4134.48	7924.15	9785.52	0	9785.52	
8	44.3652	563095	Cyclone Underflow	0	39	4466.28	8560.07	10570.8	0	10570.8	
9	44.3652	588143	Cyclone Underflow	0	39	4719.87	9046.11	11171	0	11171	
10	44.3652	603459	Cyclone Underflow	0	39	4896.24	9384.13	11588.4	0	11588.4	
11	44.3652	609444	Cyclone Underflow	0	39	4996.03	9575.4	11824.6	0	11824.6	
12	44.3652	606436	Cyclone Underflow	0	39	5019.62	9620.6	11880.4	0	11880.4	
13	23.4132	316539	Liner Interface Zone	0	26.5	3196.81	6127.01	12445	156.077	12288.9	
14	46.4639	615182	Foundation Materials	150	29	3573.33	6848.64	12084.7	0	12084.7	
15	46.4639	590270	Foundation Materials	150	29	3445.2	6603.07	11641.7	0	11641.7	
16	46.4639	555377	Foundation Materials	150	29	3255.27	6239.06	10985	0	10985	
17	46.4639	510675	Foundation Materials	150	29	3003.43	5756.37	10114.2	0	10114.2	
18	46.4639	456280	Foundation Materials	150	29	2689.22	5154.15	9027.73	0	9027.73	
19	46.4639	392275	Foundation Materials	150	29	2311.95	4431.08	7723.26	0	7723.26	
20	46.4639	318701	Foundation Materials	150	29	1870.65	3585.29	6197.43	0	6197.43	
21	46.4639	235654	Foundation Materials	150	29	1364.61	2615.41	4447.71	0	4447.71	
22	46.4639	146214	Foundation Materials	150	29	861.912	1651.94	2709.57	0	2709.57	
23	46.4639	98503.8	Foundation Materials	150	29	693.875	1329.88	2128.55	0	2128.55	
24	46.4639	56028.4	Foundation Materials	150	29	408.79	783.486	1142.84	0	1142.84	
25	20.1633	6058.23	Liner Interface Zone	0	26.5	4794.2	9188.57	18585.2	155.762	18429.4	

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.9166



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	575.445	5460	1.98387e-022	0	0
2	619.81	5422.28	-162525	15041.2	-5.28749
3	664.175	5387.89	-170970	15822.8	-5.2875
4	708.54	5356.52	-168647	15607.8	-5.2875
5	752.906	5327.9	-167853	15534.3	-5.28749
6	797.271	5301.82	-178146	16486.9	-5.2875
7	841.636	5278.09	-206894	19147.4	-5.28749
8	886.001	5256.57	-259659	24030.7	-5.2875
9	930.366	5237.13	-340500	31512.3	-5.2875
10	974.731	5219.68	-452179	41847.8	-5.28749
11	1019.1	5204.13	-596323	55187.9	-5.28749
12	1063.46	5190.41	-773548	71589.6	-5.2875
13	1107.83	5178.45	-983538	91023.5	-5.28749
14	1131.24	5172.83	-1.06166e+006	98253.7	-5.2875
15	1177.7	5163.06	-1.26537e+006	117106	-5.28748
16	1224.17	5155.1	-1.50305e+006	139103	-5.2875
17	1270.63	5148.92	-1.77205e+006	163998	-5.28749
18	1317.1	5144.48	-2.06868e+006	191450	-5.28749
19	1363.56	5141.77	-2.38827e+006	221027	-5.28749
20	1410.02	5140.78	-2.7251e+006	252200	-5.28749
21	1456.49	5141.51	-3.07236e+006	284338	-5.2875
22	1502.95	5143.95	-3.42212e+006	316707	-5.28749
23	1549.42	5148.12	-3.68084e+006	340651	-5.2875
24	1595.88	5154.03	-3.72095e+006	344363	-5.28749
25	1642.34	5161.71	-3.78664e+006	350442	-5.28749
26	1662.51	5165.61	6.38612e-021	0	0

List Of Coordinates

Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195

732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

External Boundary

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81

213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164

1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85

1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42

2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455

441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01

570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

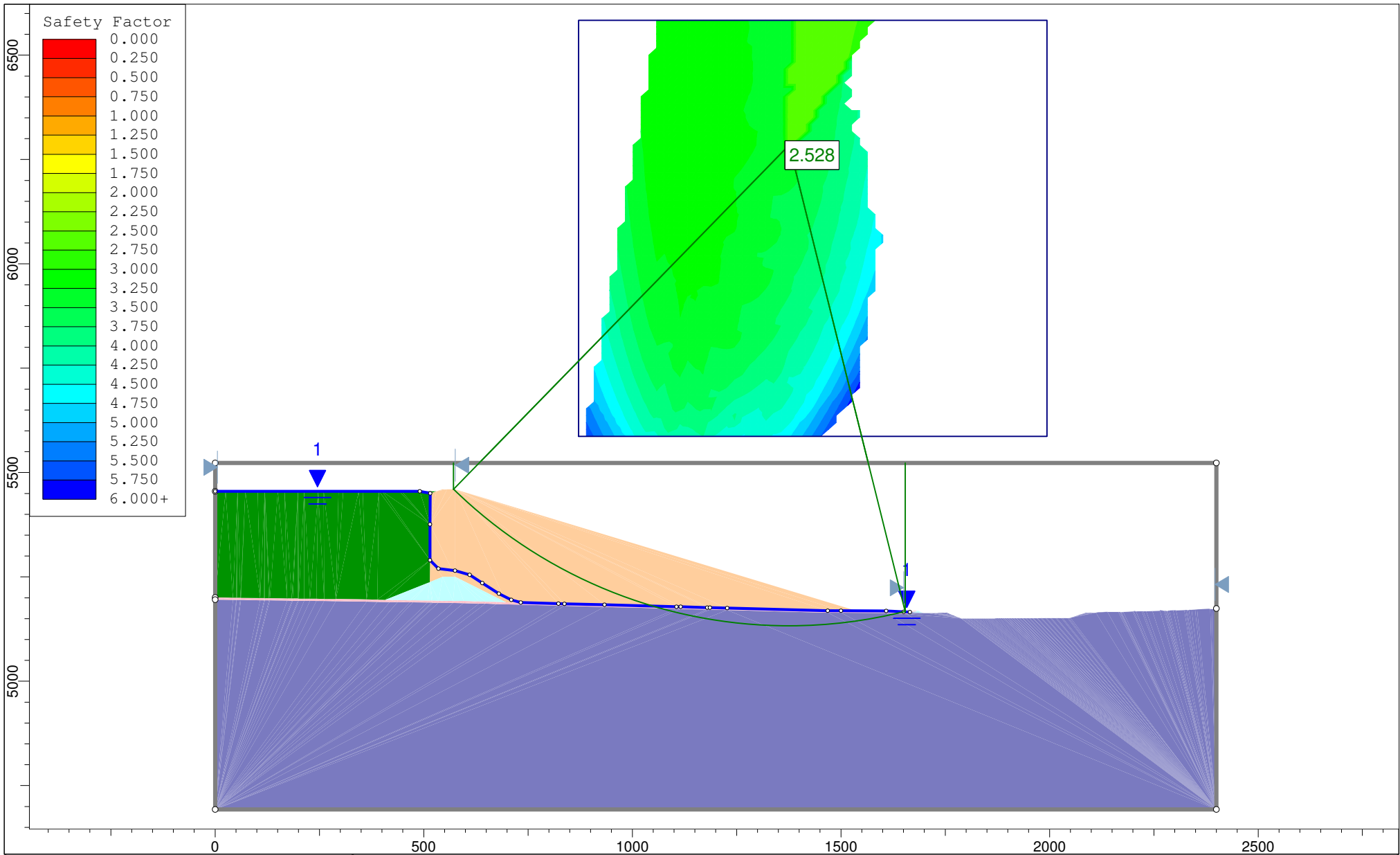
Material Boundary

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63

545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength	
Drawn By	GS	Scale	1:3822
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	6a - SectionA 5460R DS_S_C_G_postliq.slim

Slide Analysis Information

Copper Flat

Project Summary

File Name: 6a - SectionA 5460R DS_S_C_G_postliq.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 3:02:21 PM
Comments:
 103-92557
 Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer

Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check $m_{\alpha} < 0.2$: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft³
Advanced Groundwater Method: None







Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
 Search Method: Grid Search
 Radius Increment: 10
 Composite Surfaces: Disabled
 Reverse Curvature: Create Tension Crack
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.05
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

Global Minimums

Method: spencer

FS: 2.527620
 Center: 1376.188, 6284.432
 Radius: 1152.416
 Left Slip Surface Endpoint: 570.971, 5460.000
 Right Slip Surface Endpoint: 1654.244, 5166.064
 Left Slope Intercept: 570.971 5522.990
 Right Slope Intercept: 1654.244 5522.990
 Resisting Moment=8.3306e+009 lb-ft
 Driving Moment=3.29583e+009 lb-ft
 Resisting Horizontal Force=6.74519e+006 lb

Driving Horizontal Force=2.6686e+006 lb

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 11802

Number of Invalid Surfaces: 29129

Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4262 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3796 surfaces
- Error Code -1000 reported for 17897 surfaces

Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.344	953.782	1177.82	0	1177.82
2	47.6079	233141	Cyclone Underflow	0	39	1249.93	3159.35	3901.47	0	3901.47
3	47.6079	356305	Cyclone Underflow	0	39	1954.88	4941.2	6101.88	0	6101.88
4	47.6079	458937	Cyclone Underflow	0	39	2561.11	6473.51	7994.12	0	7994.12
5	47.6079	543134	Cyclone Underflow	0	39	3074.33	7770.73	9596.06	0	9596.06

6	47.6079	610541	Cyclone Underflow	0	39	3499.23	8844.72	10922.3	0	10922.3
7	47.6079	662469	Cyclone Underflow	0	39	3839.67	9705.22	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.76	10360.1	12793.7	0	12793.7
9	47.6079	723916	Cyclone Underflow	0	39	4279.01	10815.7	13356.3	0	13356.3
10	47.6079	734997	Cyclone Underflow	0	39	4382.34	11076.9	13678.8	0	13678.8
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.62	7119.35	14435.1	155.948	14279.2
12	43.9418	677489	Foundation Materials	150	29	3185.59	8051.96	14255.5	0	14255.5
13	43.9418	666065	Foundation Materials	150	29	3142.32	7942.59	14058.2	0	14058.2
14	43.9418	645159	Foundation Materials	150	29	3052.84	7716.42	13650.2	0	13650.2
15	43.9418	614993	Foundation Materials	150	29	2917.65	7374.71	13033.7	0	13033.7
16	43.9418	575744	Foundation Materials	150	29	2737.05	6918.22	12210.2	0	12210.2
17	43.9418	527526	Foundation Materials	150	29	2511.05	6346.98	11179.6	0	11179.6
18	43.9418	470419	Foundation Materials	150	29	2239.47	5660.53	9941.26	0	9941.26
19	43.9418	404462	Foundation Materials	150	29	1921.94	4857.93	8493.33	0	8493.33
20	43.9418	329659	Foundation Materials	150	29	1557.85	3937.65	6833.1	0	6833.1
21	43.9418	246078	Foundation Materials	150	29	1146.9	2898.93	4959.21	0	4959.21
22	43.9418	156715	Foundation Materials	150	29	744.38	1881.51	3123.73	0	3123.73
23	43.9418	104701	Foundation Materials	150	29	581.769	1470.49	2382.23	0	2382.23
24	43.9418	56711	Foundation Materials	150	29	328.39	830.044	1226.83	0	1226.83
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4364.66	11032.2	22282.9	155.762	22127.2

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146699	13710.3	-5.33928

3	666.187	5376.71	-127697	11934.4	-5.33928
4	713.795	5341.41	-93312.8	8720.9	-5.33928
5	761.402	5309.7	-62734.3	5863.08	-5.33929
6	809.01	5281.25	-50034.2	4676.14	-5.33929
7	856.618	5255.79	-65433.1	6115.3	-5.33929
8	904.226	5233.09	-116155	10855.7	-5.33928
9	951.834	5212.99	-207020	19347.9	-5.3393
10	999.442	5195.34	-340860	31856.3	-5.33927
11	1047.05	5180.02	-518810	48487.4	-5.33929
12	1066.08	5174.52	-568241	53107.1	-5.33928
13	1110.02	5163.18	-727848	68023.8	-5.33929
14	1153.96	5153.65	-924444	86397.4	-5.33928
15	1197.9	5145.89	-1.15628e+006	108064	-5.33926
16	1241.85	5139.87	-1.42067e+006	132774	-5.33928
17	1285.79	5135.57	-1.71404e+006	160192	-5.33928
18	1329.73	5132.95	-2.03189e+006	189898	-5.33929
19	1373.67	5132.02	-2.36881e+006	221386	-5.33928
20	1417.61	5132.76	-2.71843e+006	254061	-5.33928
21	1461.56	5135.18	-3.07335e+006	287232	-5.33929
22	1505.5	5139.29	-3.42512e+006	320108	-5.33929
23	1549.44	5145.11	-3.67961e+006	343892	-5.33929
24	1593.38	5152.67	-3.72686e+006	348308	-5.33929
25	1637.32	5161.99	-3.7896e+006	354171	-5.33928
26	1654.24	5166.06	6.3698e-021	0	0

List Of Coordinates

Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5

1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

External Boundary

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27

1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

Material Boundary

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455

530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

Material Boundary

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

Material Boundary

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

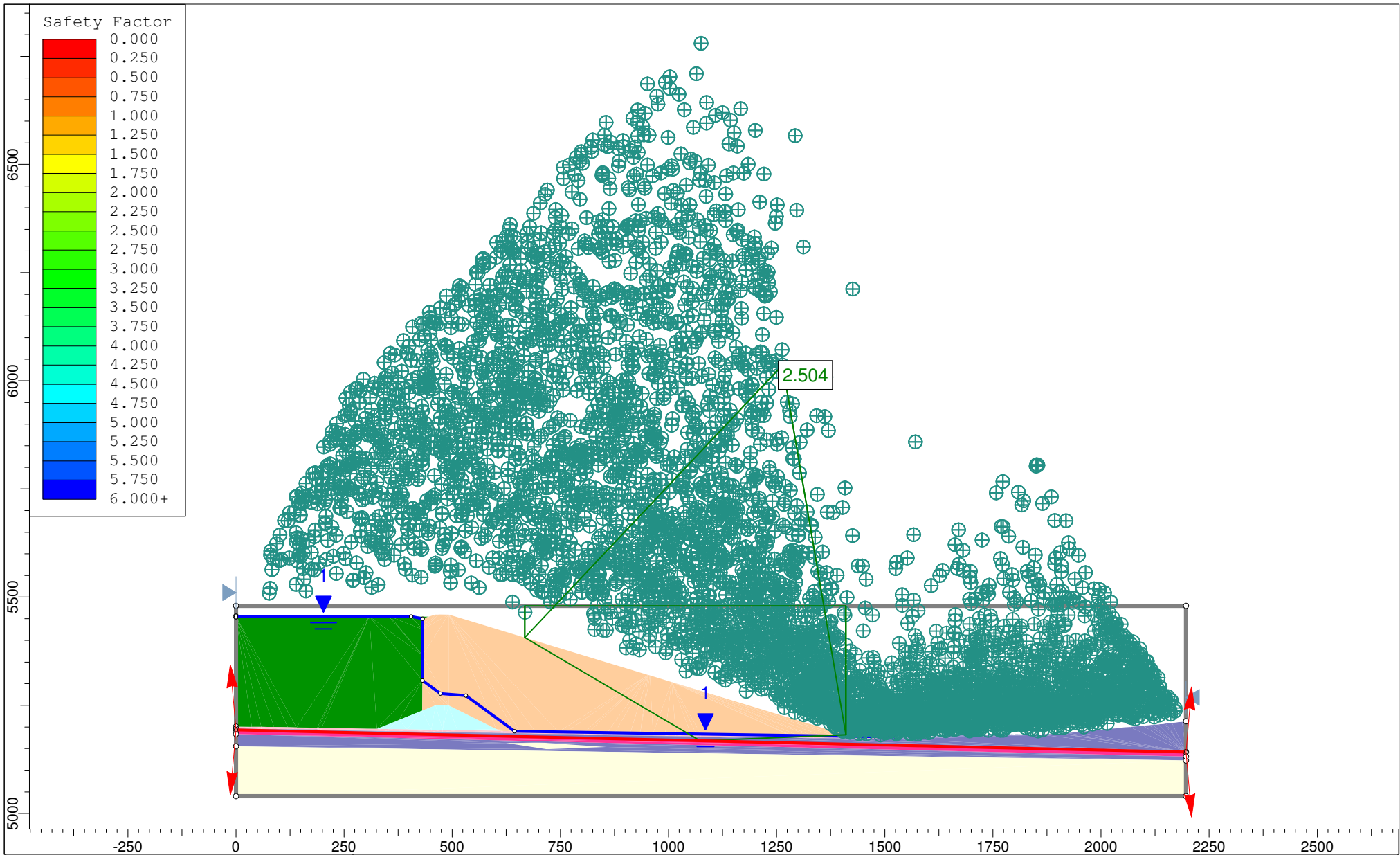
Material Boundary


X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

Material Boundary

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32

**APPENDIX H.2
STABILITY SECTION B-B'**



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Static, Block Failure		
	Drawn By	GS	Scale	1:3686	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	1 - Section B 5460R - DS_S_B.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 1 - Section B 5460R - DS_S_B.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section B-B' Stability: Downstream, Static, Block Failure
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 11:49:04 AM
Comments:
103-92557
Material Property Edits 12/13

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer

Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check malpha < 0.2: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft3
Advanced Groundwater Method: None









Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765

2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5

22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7

42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6

62656.7	15211.8
---------	---------

Global Minimums

Method: spencer

FS: 2.504180
 Axis Location: 1263.734, 6036.472
 Left Slip Surface Endpoint: 667.648, 5406.642
 Right Slip Surface Endpoint: 1409.947, 5181.705
 Left Slope Intercept: 667.648 5480.000
 Right Slope Intercept: 1409.947 5480.000
 Resisting Moment=2.81387e+009 lb-ft
 Driving Moment=1.12367e+009 lb-ft
 Resisting Horizontal Force=3.03106e+006 lb
 Driving Horizontal Force=1.2104e+006 lb

Global Minimum Coordinates

Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 584
 Number of Invalid Surfaces: 4416

Error Codes:

Error Code -106 reported for 2 surfaces
 Error Code -107 reported for 1331 surfaces
 Error Code -108 reported for 2123 surfaces
 Error Code -110 reported for 70 surfaces
 Error Code -111 reported for 595 surfaces
 Error Code -112 reported for 295 surfaces

Error Codes

The following errors were encountered during the computation:

- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the

driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation.

This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.50418

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	25.4457	63.7207	78.6884	0	78.6884
2	31.651	48443.9	Cyclone Underflow	0	39	393.5	985.395	1216.86	0	1216.86
3	31.651	80739.8	Cyclone Underflow	0	39	671.529	1681.63	2076.63	0	2076.63
4	31.651	113036	Cyclone Underflow	0	39	949.556	2377.86	2936.4	0	2936.4
5	31.651	145332	Cyclone Underflow	0	39	1227.58	3074.09	3796.18	0	3796.18
6	31.651	177628	Cyclone Underflow	0	39	1505.61	3770.32	4655.95	0	4655.95
7	31.651	209923	Cyclone Underflow	0	39	1783.64	4466.55	5515.73	0	5515.73
8	31.651	242219	Cyclone Underflow	0	39	2061.66	5162.78	6375.5	0	6375.5
9	31.651	274515	Cyclone Underflow	0	39	2339.69	5859.01	7235.27	0	7235.27
10	31.651	306847	Cyclone Underflow	0	39	2618.13	6556.28	8096.33	0	8096.33
11	31.651	339198	Cyclone Underflow	0	39	2896.61	7253.64	8957.49	0	8957.49
12	31.651	371504	Cyclone Underflow	0	39	3174.66	7949.91	9817.32	0	9817.32
13	8.75148	108572	Liner Interface Zone	0	26.5	2185.06	5471.78	11170.6	195.919	10974.6
14	16.9638	218358	Foundation Materials	150	29	2579.64	6459.89	11383.3	0	11383.3
15	30.2226	380408	Foundation Materials	150	29	2774.58	6948.05	12264	0	12264
16	30.2226	344600	Foundation Materials	150	29	2511.83	6290.08	11077	0	11077
17	30.2226	308792	Foundation Materials	150	29	2249.09	5632.12	9890.02	0	9890.02
18	30.2226	272984	Foundation Materials	150	29	1986.34	4974.16	8703	0	8703
19	30.2226	237176	Foundation Materials	150	29	1723.6	4316.2	7516.03	0	7516.03
20	30.2226	201368	Foundation Materials	150	29	1460.85	3658.23	6329.01	0	6329.01
21	31.0757	169713	Liner Interface Zone	0	26.5	967.095	2421.78	5130.02	272.678	4857.34
22	31.0757	131843	Liner Interface Zone	0	26.5	745.37	1866.54	3908.11	164.386	3743.72
23	31.0757	93991.2	Liner Interface Zone	0	26.5	523.772	1311.62	2686.8	56.0937	2630.7
24	31.1043	56349.3	Cyclone Underflow	0	39	474.407	1188	1467.06	0	1467.06
25	31.1043	18783.1	Cyclone Underflow	0	39	1844.88	4619.92	5705.13	0	5705.13

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.50418

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214017	13186.2	-3.5257
3	730.95	5369.4	-256320	15792.6	-3.5257
4	762.601	5350.78	-297148	18308.2	-3.52571
5	794.252	5332.16	-336500	20732.8	-3.52571
6	825.903	5313.53	-374377	23066.5	-3.52571
7	857.554	5294.91	-410779	25309.3	-3.5257
8	889.205	5276.29	-445705	27461.2	-3.52571
9	920.856	5257.67	-479156	29522.2	-3.5257
10	952.507	5239.05	-511131	31492.3	-3.52571
11	984.158	5220.43	-541429	33359	-3.5257
12	1015.81	5201.81	-570296	35137.6	-3.5257
13	1047.46	5183.19	-597825	36833.8	-3.52571
14	1056.21	5178.04	-590828	36402.6	-3.5257
15	1073.18	5168.06	-583080	35925.3	-3.52571
16	1103.4	5169.28	-796681	49085.8	-3.5257
17	1133.62	5170.51	-1.00613e+006	61990.4	-3.52569
18	1163.84	5171.73	-1.21142e+006	74639.1	-3.5257
19	1194.07	5172.96	-1.41256e+006	87031.9	-3.5257
20	1224.29	5174.18	-1.60955e+006	99168.8	-3.52569
21	1254.51	5175.41	-1.80238e+006	111050	-3.52571
22	1285.59	5176.66	-1.98927e+006	122565	-3.52571
23	1316.66	5177.92	-2.17327e+006	133902	-3.52572
24	1347.74	5179.18	-2.35439e+006	145061	-3.52571
25	1378.84	5180.44	-2.53815e+006	156383	-3.52571
26	1409.95	5181.7	4.449e-021	0	0

List Of Coordinates

Piezoline

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

Block Search Lines

X	Y

0	5193.11
2196.33	5141.83

External Boundary

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

Material Boundary

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28

814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

Material Boundary

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18

745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

Material Boundary

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

Material Boundary

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

Material Boundary

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

Material Boundary

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77

1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13

1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

Material Boundary

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

Material Boundary

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250

491.57	5250
601.751	5194.94
610.156	5190.74

Material Boundary

X	Y
0	5193.11
2196.33	5141.83

Material Boundary

X	Y
0	5183.11
2196.33	5131.83

Material Boundary

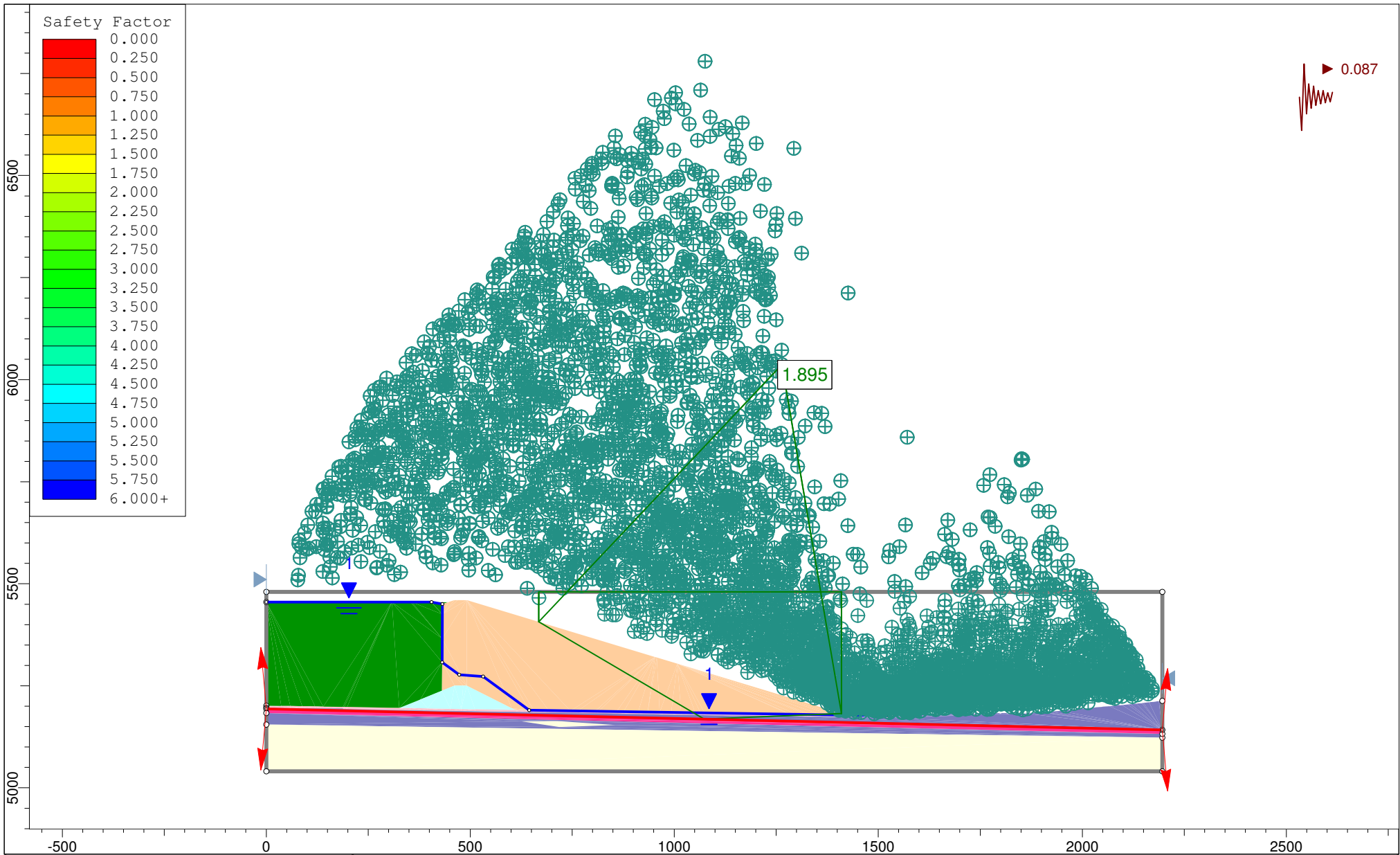
X	Y
0	5154.97
2196.33	5122.6


Material Boundary

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

Material Boundary

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat		
	Analysis Description					Section B-B' Stability: Downstream, Pseudo Static, Block Failure
	Drawn By	GS	Scale	1:3908	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	2 - Section B 5460R - DS_PS_B.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 2 - Section B 5460R - DS_PS_B.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Block Failure
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 11:49:04 AM
Comments:
103-92557
Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check malpha < 0.2: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft3
Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3









Surface Options

Surface Type: Non-Circular Block Search
 Number of Surfaces: 5000
 Pseudo-Random Surfaces: Enabled
 Convex Surfaces Only: Disabled
 Left Projection Angle (Start Angle): 95
 Left Projection Angle (End Angle): 265
 Right Projection Angle (Start Angle): 85
 Right Projection Angle (End Angle): -85
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Loading

Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft ³]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186

835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2

20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7

40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

Global Minimums

Method: spencer

FS: 1.895340
 Axis Location: 1263.734, 6036.472
 Left Slip Surface Endpoint: 667.648, 5406.642
 Right Slip Surface Endpoint: 1409.947, 5181.705
 Left Slope Intercept: 667.648 5480.000
 Right Slope Intercept: 1409.947 5480.000
 Resisting Moment=2.74873e+009 lb-ft
 Driving Moment=1.45026e+009 lb-ft
 Resisting Horizontal Force=2.96703e+006 lb
 Driving Horizontal Force=1.56543e+006 lb

Global Minimum Coordinates

Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 562
 Number of Invalid Surfaces: 4438

Error Codes:

Error Code -106 reported for 2 surfaces
 Error Code -107 reported for 578 surfaces
 Error Code -108 reported for 2726 surfaces
 Error Code -110 reported for 101 surfaces
 Error Code -111 reported for 671 surfaces
 Error Code -112 reported for 360 surfaces

Error Codes

The following errors were encountered during the computation:

- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha = $\cos(\alpha)(1+\tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	0	0	-57.4289	0	-57.4289
2	31.651	48443.9	Cyclone Underflow	0	39	484.264	917.844	1133.44	0	1133.44
3	31.651	80739.8	Cyclone Underflow	0	39	833.882	1580.49	1951.75	0	1951.75
4	31.651	113036	Cyclone Underflow	0	39	1183.5	2243.14	2770.05	0	2770.05
5	31.651	145332	Cyclone Underflow	0	39	1533.12	2905.79	3588.35	0	3588.35
6	31.651	177628	Cyclone Underflow	0	39	1882.74	3568.44	4406.65	0	4406.65
7	31.651	209923	Cyclone Underflow	0	39	2232.36	4231.08	5224.95	0	5224.95
8	31.651	242219	Cyclone Underflow	0	39	2581.98	4893.73	6043.25	0	6043.25
9	31.651	274515	Cyclone Underflow	0	39	2931.6	5556.38	6861.55	0	6861.55
10	31.651	306847	Cyclone Underflow	0	39	3281.79	6220.1	7681.19	0	7681.19
11	31.651	339198	Cyclone Underflow	0	39	3631.96	6883.8	8500.79	0	8500.79
12	31.651	371504	Cyclone Underflow	0	39	3981.57	7546.43	9319.07	0	9319.07
13	8.75148	108572	Liner Interface Zone	0	26.5	2802.54	5311.77	10849.7	195.919	10653.8
14	16.9638	218358	Foundation Materials	150	29	3293.35	6242.01	10990.3	0	10990.3
15	30.2226	380408	Foundation Materials	150	29	3636.26	6891.94	12162.8	0	12162.8
16	30.2226	344600	Foundation Materials	150	29	3288.53	6232.88	10973.8	0	10973.8
17	30.2226	308792	Foundation Materials	150	29	2940.8	5573.81	9784.82	0	9784.82
18	30.2226	272984	Foundation Materials	150	29	2593.07	4914.75	8595.82	0	8595.82
19	30.2226	237176	Foundation Materials	150	29	2245.34	4255.68	7406.86	0	7406.86
20	30.2226	201368	Foundation Materials	150	29	1897.61	3596.62	6217.86	0	6217.86
21	31.0757	169713	Liner Interface Zone	0	26.5	1250.03	2369.24	5024.65	272.678	4751.98
22	31.0757	131843	Liner Interface Zone	0	26.5	956.103	1812.14	3798.98	164.386	3634.59
23	31.0757	93991.2	Liner Interface Zone	0	26.5	662.33	1255.34	2573.92	56.0937	2517.83
24	31.1043	56349.3	Cyclone Underflow	0	39	574.915	1089.66	1345.62	0	1345.62
25	31.1043	18783.1	Cyclone Underflow	0	39	3181.77	6030.54	7447.09	0	7447.09

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214342	17977.3	-4.79429
3	730.95	5369.4	-256846	21542.3	-4.79432
4	762.601	5350.78	-298096	25002	-4.79431
5	794.252	5332.16	-338091	28356.5	-4.79431
6	825.903	5313.53	-376832	31605.8	-4.79431
7	857.554	5294.91	-414318	34749.8	-4.79431
8	889.205	5276.29	-450549	37788.6	-4.79431
9	920.856	5257.67	-485526	40722.2	-4.79431
10	952.507	5239.05	-519248	43550.5	-4.79431
11	984.158	5220.43	-551514	46256.8	-4.79431
12	1015.81	5201.81	-582571	48861.6	-4.79431
13	1047.46	5183.19	-612510	51372.6	-4.79431
14	1056.21	5178.04	-603106	50583.9	-4.79431
15	1073.18	5168.06	-592352	49682	-4.79432
16	1103.4	5169.28	-798697	66988.6	-4.79431
17	1133.62	5170.51	-1.00144e+006	83993.1	-4.79431
18	1163.84	5171.73	-1.20058e+006	100696	-4.79434
19	1194.07	5172.96	-1.39612e+006	117096	-4.79432
20	1224.29	5174.18	-1.58806e+006	133194	-4.7943
21	1254.51	5175.41	-1.7764e+006	148990	-4.79428
22	1285.59	5176.66	-1.95716e+006	164151	-4.79429
23	1316.66	5177.92	-2.13608e+006	179158	-4.79431
24	1347.74	5179.18	-2.31317e+006	194011	-4.79431
25	1378.84	5180.44	-2.49498e+006	209260	-4.79431
26	1409.95	5181.7	4.449e-021	0	0

List Of Coordinates

Piezoline

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

Block Search Lines

X	Y
0	5193.11
2196.33	5141.83

External Boundary

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

Material Boundary

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8

692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

Material Boundary

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69

617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

Material Boundary

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

Material Boundary

X	Y
1529.3	5171.41
1531.63	5172.17

1540.96	5175.24
1544.53	5175.72

Material Boundary

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

Material Boundary

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73

1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89

1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

Material Boundary

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

Material Boundary

X	Y

325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

Material Boundary

X	Y
0	5193.11
2196.33	5141.83

Material Boundary

X	Y
0	5183.11
2196.33	5131.83

Material Boundary

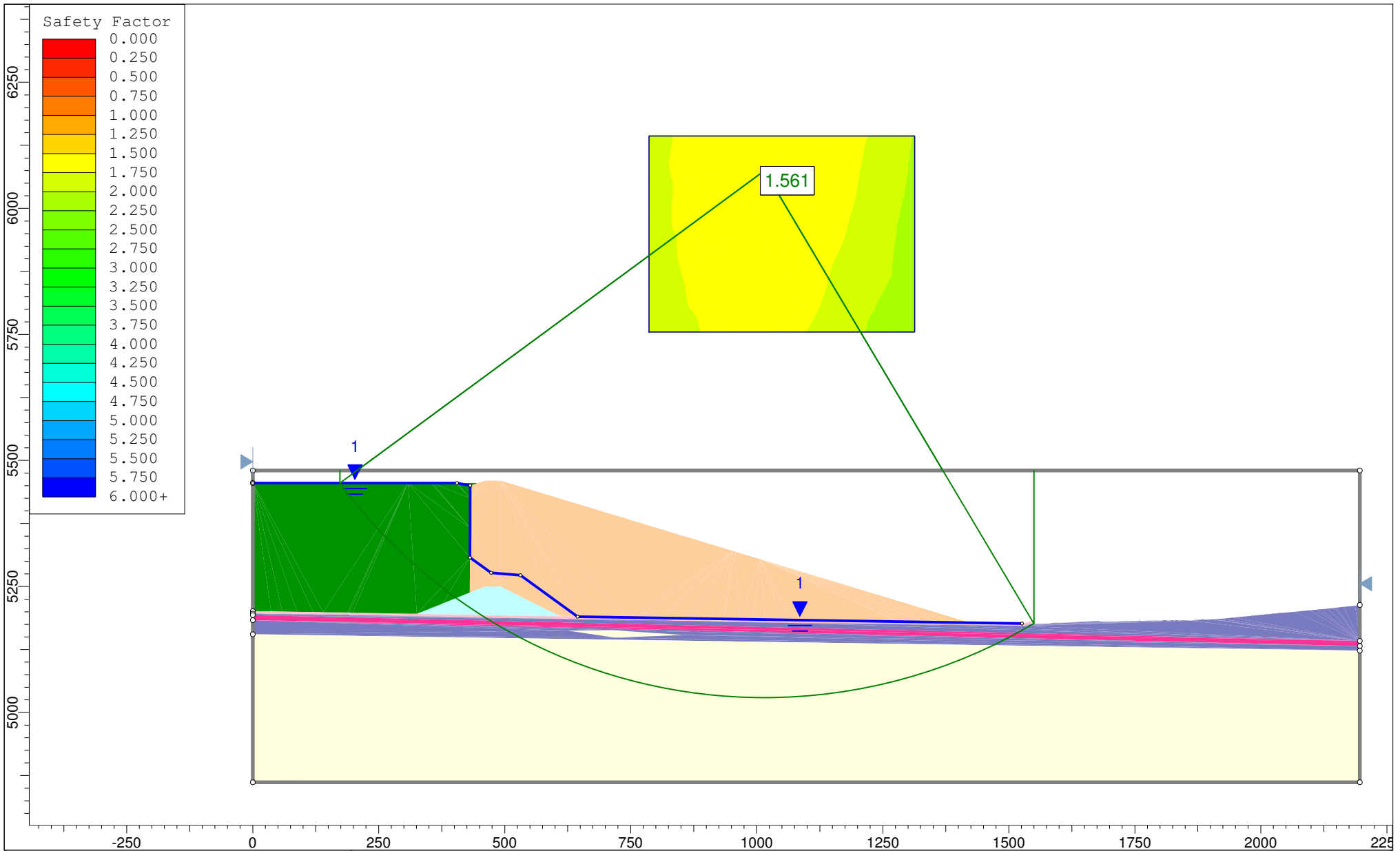
X	Y
0	5154.97
2196.33	5122.6

Material Boundary

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

Material Boundary

X	Y
1544.4	5180.22
1544.53	5175.72



SLIDEINTERPRET 6.008

Project				Copper Flat			
Analysis Description				Section B-B' Stability: Downstream, Static, Circular Failure			
Drawn By		GS		Scale		1:3149	
Company				Golder Associates Inc.			
Date		11/4/2013, 11:49:04 AM		File Name		3 - Section B 5460R - DS_S_C.slim	

Slide Analysis Information

Copper Flat

Project Summary

File Name: 3 - Section B 5460R - DS_S_C.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section B-B' Stability: Downstream, Static, Circular Failure
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 11:49:04 AM
Comments:
103-92557
Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer

Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check malpha < 0.2: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft3
Advanced Groundwater Method: None









Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
 Search Method: Grid Search
 Radius Increment: 10
 Composite Surfaces: Disabled
 Reverse Curvature: Create Tension Crack
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit								
Weight [lbs/ft3]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3

3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8

43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

Global Minimums

Method: spencer

FS: 1.560970
 Center: 1015.357, 6074.806
 Radius: 1045.828
 Left Slip Surface Endpoint: 173.020, 5454.947
 Right Slip Surface Endpoint: 1550.009, 5175.971
 Left Slope Intercept: 173.020 5480.000
 Right Slope Intercept: 1550.009 5480.000
 Resisting Moment=1.06294e+010 lb-ft
 Driving Moment=6.80947e+009 lb-ft
 Resisting Horizontal Force=9.56258e+006 lb
 Driving Horizontal Force=6.12606e+006 lb

Valid / Invalid Surfaces

Method: spencer

Number of Valid Surfaces: 2506
 Number of Invalid Surfaces: 2246

Error Codes:

Error Code -103 reported for 588 surfaces
 Error Code -108 reported for 568 surfaces
 Error Code -110 reported for 58 surfaces
 Error Code -111 reported for 1032 surfaces

Error Codes

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	76.7812	384179	Cyclone Overflow	421.855	0	270.252	421.855	3388.77	2894.28	494.494
2	76.7812	1.07712e+006	Cyclone Overflow	1183.95	0	758.471	1183.95	10074.5	8108.62	1965.9
3	76.7812	1.63914e+006	Cyclone Overflow	1802.07	0	1154.46	1802.07	16242.9	12337.8	3905.11
4	49.0448	1.33572e+006	Structural Fill Liner Interface	0	29	7124.22	11120.7	20062.4	0	20062.4

			Zone							
6	9.92233	312599	Foundation Materials	150	29	8606.51	13434.5	23965.9	0	23965.9
7	17.4329	567165	Soft Clay	851.163	13.3468	4488.85	7006.95	25946.3	0	25946.3
8	44.5694	1.5136e+006	Foundation Materials	150	29	9663.09	15083.8	26941.3	0	26941.3
9	67.0136	2.36955e+006	Clay	958.267	13.1388	5094.31	7952.07	29962.1	0	29962.1
10	67.0136	2.45379e+006	Clay	1025.15	13.0217	5445.36	8500.05	32321.5	0	32321.5
11	67.0136	2.50548e+006	Clay	1089.15	12.9174	5739.42	8959.07	34313.8	0	34313.8
12	67.0136	2.50972e+006	Clay	1124.79	12.8623	5943.34	9277.38	35704	0	35704
13	67.0136	2.47166e+006	Clay	1148.83	12.8262	6062.3	9463.07	36518	0	36518
14	67.0136	2.39734e+006	Clay	1148.83	12.8262	6103.92	9528.04	36803.3	0	36803.3
15	67.0136	2.28693e+006	Clay	1148.83	12.8262	6063.17	9464.42	36524	0	36524
16	67.0136	2.13981e+006	Clay	1124.79	12.8623	5933.03	9261.28	35633.4	0	35633.4
17	67.0136	1.95593e+006	Clay	1072.75	12.9435	5704.6	8904.71	34077.2	0	34077.2
18	67.0136	1.7351e+006	Clay	1017.45	13.0347	5367	8377.72	31793.1	0	31793.1
19	67.0136	1.47639e+006	Clay	923.265	13.2038	4903.36	7653.99	28688	0	28688
20	67.0136	1.1784e+006	Clay	802.785	13.4506	4288.2	6693.74	24631.1	0	24631.1
21	67.0136	839198	Clay	656.503	13.8133	3481.23	5434.09	19431.3	0	19431.3
22	67.0136	496697	Clay	415.25	14.6733	2204.49	3441.14	11555.9	0	11555.9
23	29.3922	131593	Foundation Materials	150	29	3334.74	5205.43	9120.25	0	9120.25
24	17.8998	52163.6	Soft Clay	192.084	16.2309	1043.45	1628.79	4935.28	0	4935.28
25	32.0382	41314	Foundation Materials	150	29	0	0	-38306.9	0	-38306.9

Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	173.02	5454.95	3.13838e-023	0	0
2	249.802	5362.29	273636	97200.6	19.556
3	326.583	5287.82	965550	342981	19.556
4	403.364	5226.74	1.86896e+006	663889	19.556
5	452.409	5193.42	2.19051e+006	778111	19.556
6	460.564	5188.26	2.26451e+006	804395	19.556
7	470.486	5182.13	2.32639e+006	826376	19.556
8	487.919	5171.72	2.51812e+006	894484	19.556
9	532.488	5147.12	2.72923e+006	969472	19.556
10	599.502	5115.21	3.28961e+006	1.16853e+006	19.556
11	666.515	5088.87	3.69589e+006	1.31285e+006	19.556
12	733.529	5067.67	3.93299e+006	1.39707e+006	19.556
13	800.543	5051.28	3.98821e+006	1.41669e+006	19.556
14	867.556	5039.47	3.85556e+006	1.36957e+006	19.556
15	934.57	5032.1	3.53467e+006	1.25558e+006	19.556
16	1001.58	5029.07	3.03059e+006	1.07652e+006	19.556
17	1068.6	5030.33	2.35334e+006	835950	19.556

18	1135.61	5035.91	1.52056e+006	540130	19.5559
19	1202.62	5045.88	558004	198213	19.5559
20	1269.64	5060.36	-497755	-176812	19.556
21	1336.65	5079.55	-1.59529e+006	-566675	19.5559
22	1403.67	5103.74	-2.6616e+006	-945449	19.556
23	1470.68	5133.3	-3.26006e+006	-1.15803e+006	19.5559
24	1500.07	5148.09	-3.50629e+006	-1.2455e+006	19.556
25	1517.97	5157.67	-3.58004e+006	-1.2717e+006	19.556
26	1550.01	5175.97	4.62169e-021	0	0

List Of Coordinates

Piezoline

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

External Boundary

X	Y
2196.33	4861.27
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	4861.27

Material Boundary

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04

87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

Material Boundary

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

Material Boundary

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

Material Boundary

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

Material Boundary

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

Material Boundary

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178

1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32

1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

Material Boundary

X	Y
430.6	5237.63
430.947	5307.07

431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

Material Boundary

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

Material Boundary

X	Y
0	5193.11
2196.33	5141.83

Material Boundary

X	Y
0	5183.11
2196.33	5131.83

Material Boundary

X	Y
0	5154.97
2196.33	5122.6

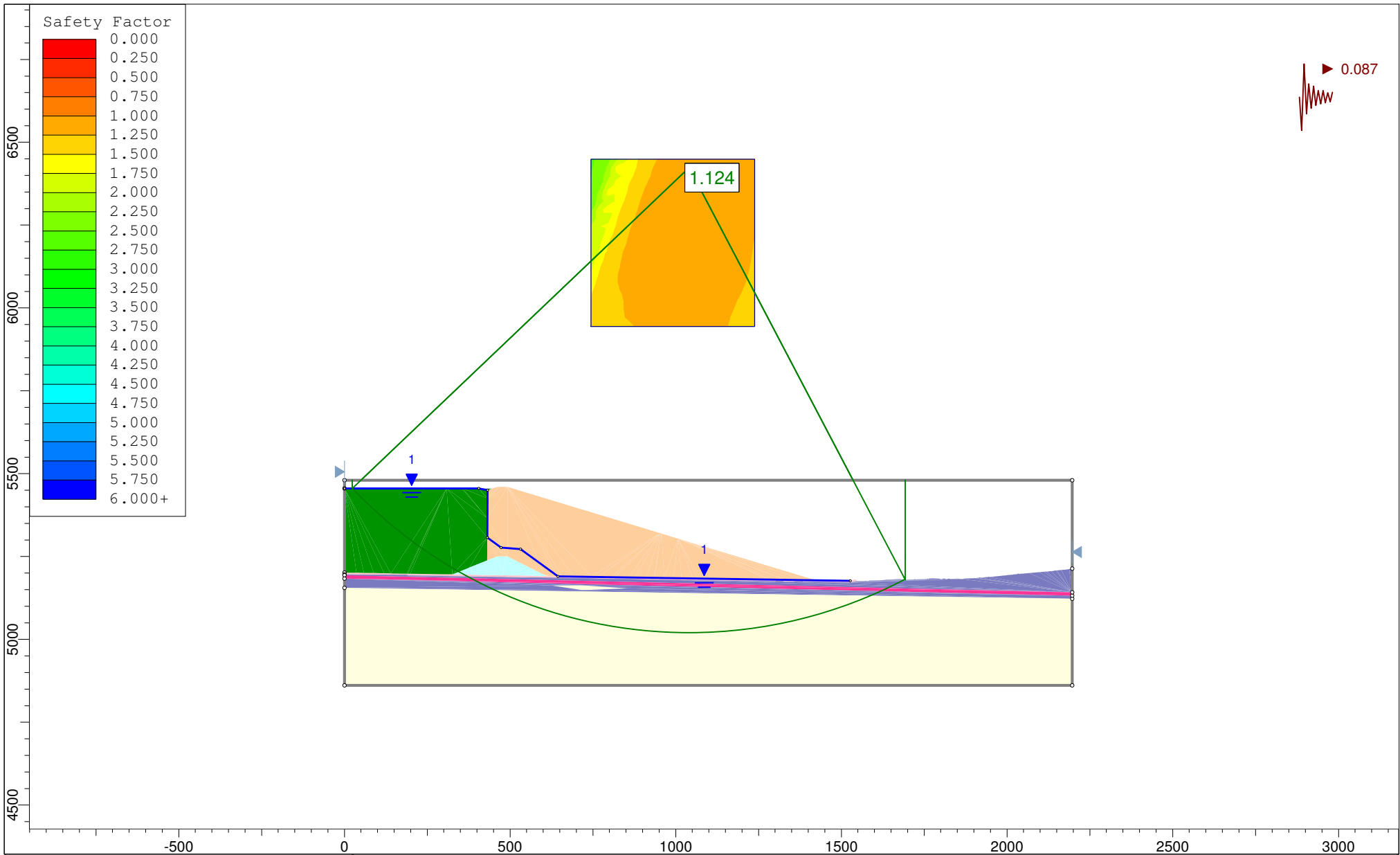
Material Boundary


X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54

712.872 5163.35

Material Boundary

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Pseudo Static, Circular Failure		
	Drawn By	GS	Scale	1:4812	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	4 - Section B 5460R - DS_PS_C.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

File Name: 4 - Section B 5460R - DS_PS_C.slim
Slide Modeler Version: 6.008
Project Title: Copper Flat
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Circular Failure
Author: GS
Company: Golder Associates Inc.
Date Created: 11/4/2013, 11:49:04 AM
Comments:
103-92557
Material Property Edits 12/2013

General Settings

Units of Measurement: Imperial Units
Time Units: days
Permeability Units: feet/second
Failure Direction: Left to Right
Data Output: Standard
Maximum Material Properties: 20
Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Spencer
Number of slices: 25
Tolerance: 0.005
Maximum number of iterations: 50
Check malpha < 0.2: Yes
Initial trial value of FS: 1
Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
Pore Fluid Unit Weight: 62.4 lbs/ft3
Advanced Groundwater Method: None

Random Numbers







0
10
20
30
40
50
60
70
80
90
100

Sample	Element	Concentration	Unit
1	Al	0.0000	g/g
1	As	0.0000	g/g
1	B	0.0000	g/g
1	Ca	0.0000	g/g
1	Co	0.0000	g/g
1	Cu	0.0000	g/g
1	Fe	0.0000	g/g
1	Li	0.0000	g/g
1	Mn	0.0000	g/g
1	Ni	0.0000	g/g
1	P	0.0000	g/g
1	Pb	0.0000	g/g
1	S	0.0000	g/g
1	Se	0.0000	g/g
1	Si	0.0000	g/g
1	Te	0.0000	g/g
1	Ti	0.0000	g/g
1	V	0.0000	g/g
1	Zn	0.0000	g/g

Sample	Element	Concentration	Unit
1	Al	0.0000	g/g
1	As	0.0000	g/g
1	B	0.0000	g/g
1	Ca	0.0000	g/g
1	Co	0.0000	g/g
1	Cu	0.0000	g/g
1	Fe	0.0000	g/g
1	Li	0.0000	g/g
1	Mn	0.0000	g/g
1	Ni	0.0000	g/g
1	P	0.0000	g/g
1	Pb	0.0000	g/g
1	S	0.0000	g/g
1	Se	0.0000	g/g
1	Si	0.0000	g/g
1	Te	0.0000	g/g
1	Ti	0.0000	g/g
1	V	0.0000	g/g
1	Zn	0.0000	g/g

Table with 2 columns: Element, Concentration (ppm)

Element	Concentration (ppm)
Al	0.00
As	0.00
B	0.00
Be	0.00
Ba	0.00
Bi	0.00
Bk	0.00
Br	0.00
Bs	0.00
C	0.00
Ca	0.00
Ce	0.00
Cf	0.00
Cl	0.00
Co	0.00
Cs	0.00
Cu	0.00
D	0.00
Dy	0.00
E	0.00
Er	0.00
F	0.00
Fe	0.00
Ga	0.00
Ge	0.00
Gd	0.00
H	0.00
Hf	0.00
Hg	0.00
Ho	0.00
I	0.00
In	0.00
Ir	0.00
Is	0.00
K	0.00
Kr	0.00
Ky	0.00
La	0.00
Li	0.00
Lr	0.00
M	0.00
Mn	0.00
Mo	0.00
N	0.00
Nb	0.00
Nd	0.00
Ne	0.00
Ni	0.00
Nm	0.00
No	0.00
O	0.00
Os	0.00
P	0.00
Pb	0.00
Pd	0.00
Pf	0.00
Pg	0.00
Pt	0.00
Pr	0.00
R	0.00
Rb	0.00
Rf	0.00
Rh	0.00
Rn	0.00
S	0.00
Sa	0.00
Sb	0.00
Sc	0.00
Se	0.00
Si	0.00
Sm	0.00
Sr	0.00
Ta	0.00
Tb	0.00
Tc	0.00
Td	0.00
Tf	0.00
Tg	0.00
Te	0.00
Th	0.00
Ti	0.00
Tl	0.00
Tm	0.00
Tn	0.00
U	0.00
V	0.00
Va	0.00
Vm	0.00
Vn	0.00
Vu	0.00
W	0.00
Y	0.00
Yb	0.00
Yr	0.00
Z	0.00
Zn	0.00
Zr	0.00

TOC science
SLIDEINTERPRET 6.008
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TOC science
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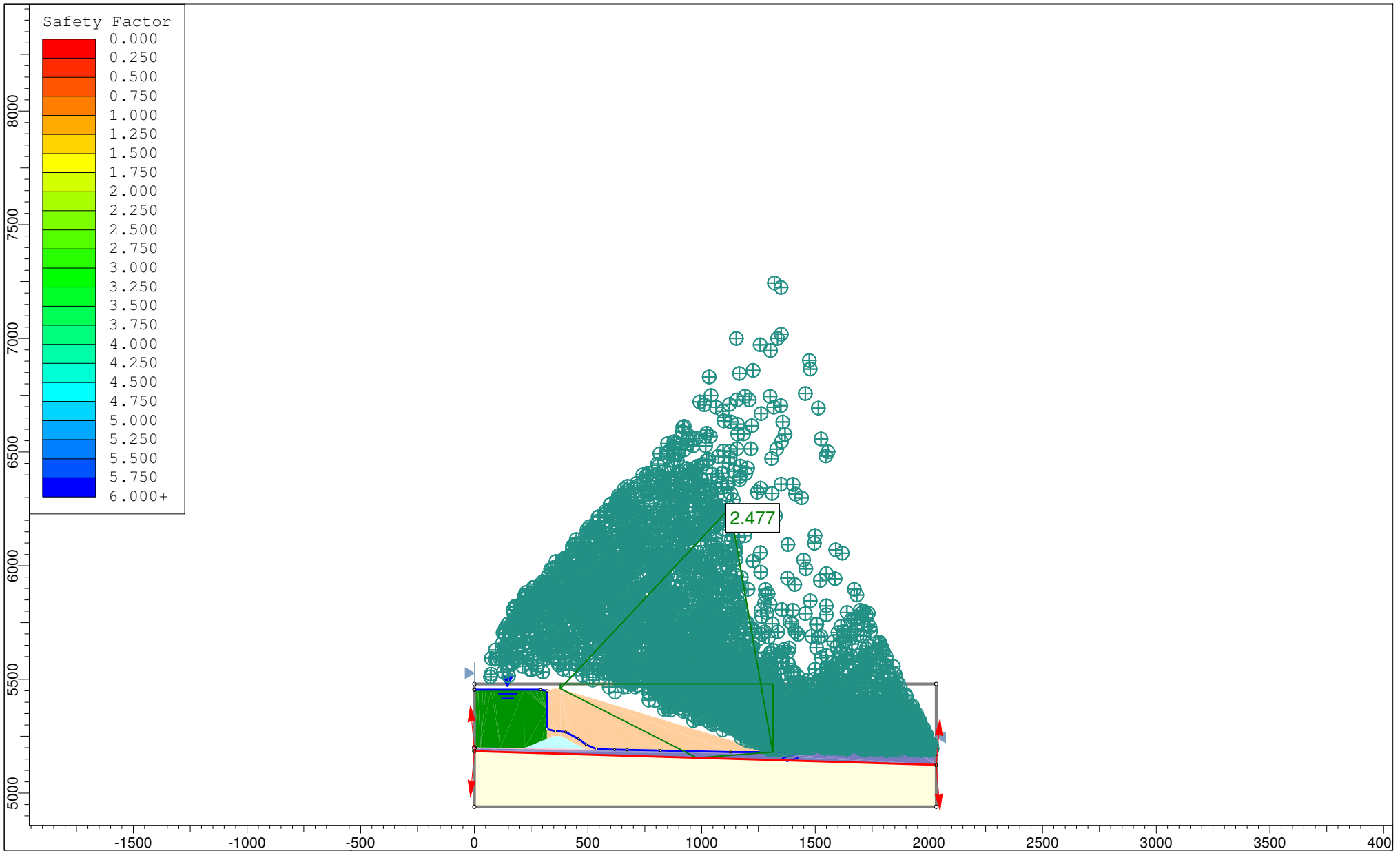
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11/4/2013 11:49:04 AM
4 - Section B 5460R - DS_PS_C.slim

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- Slide 1
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**APPENDIX H.3
STABILITY SECTION D-D'**



SLIDEINTERPRET 6.008

<i>Project</i>				Copper Flat	
<i>Analysis Description</i>				Section D-D' Stability: Downstream, Static, Block Failure	
<i>Drawn By</i>	GS	<i>Scale</i>	1:6984	<i>Company</i>	Golder Associates
<i>Date</i>	11/4/2013, 1:11:49 PM			<i>File Name</i>	1 - Section D 5460R - DS_S_B.slim

Slide Analysis Information

Copper Flat

Project Summary

- File Name: 1 - Section D 5460R - DS_S_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
 - 103-92557
 - Material Property Edits 12/2013

General Settings

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft³
- Advanced Groundwater Method: None








Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft ³]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma							0.2

Ratio							
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9

12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6

26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7

40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4

58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1388.41	5182.87
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Block Search Lines

X	Y
0	5184.09
2031.79	5123.76

External Boundary

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

List Of Coordinates

Piezoline

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14

Material Boundary

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04

1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01

1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

Material Boundary

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

Material Boundary

Material Boundary

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24

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1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

Material Boundary

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

Material Boundary

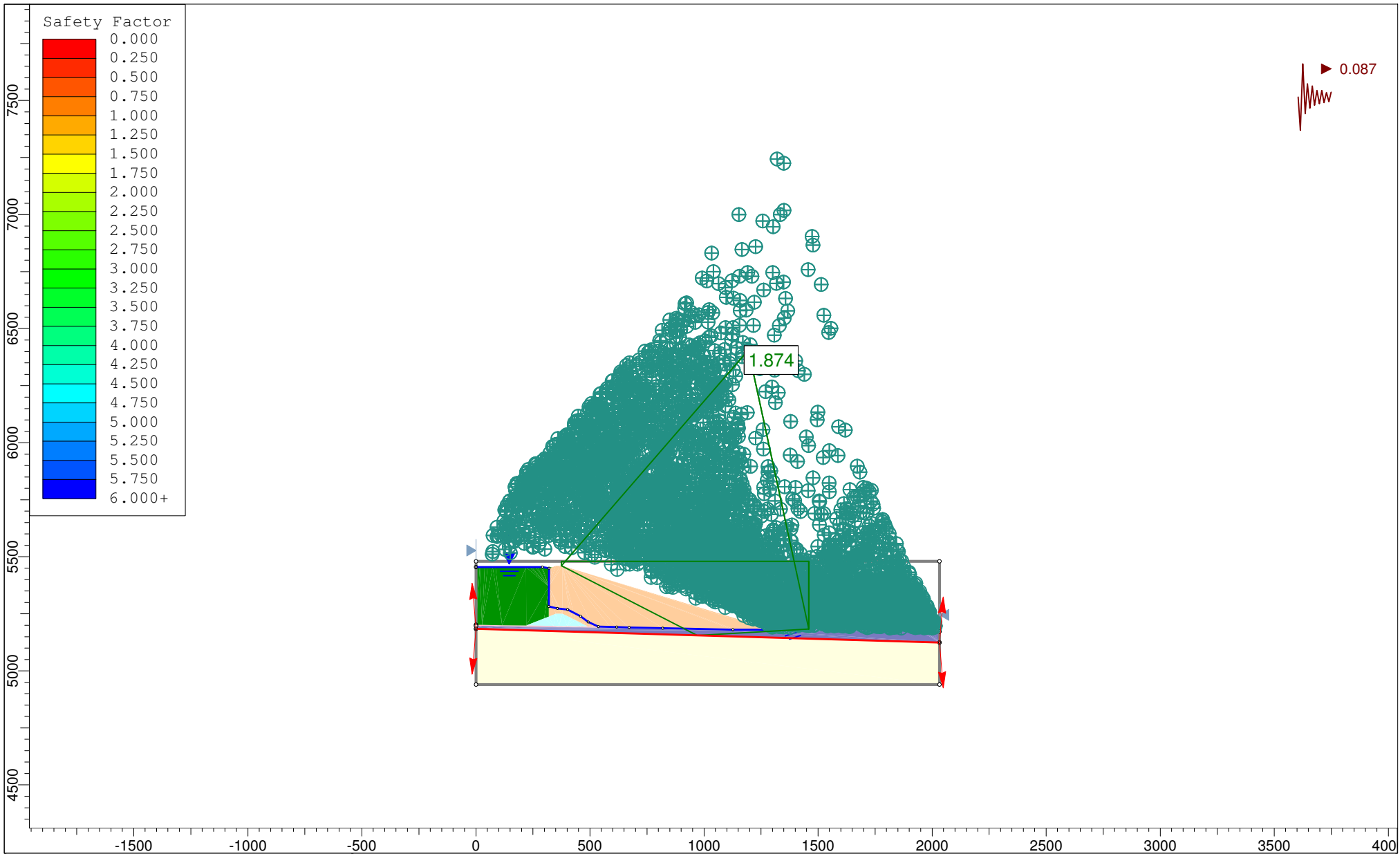
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318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


Material Boundary

X	Y
0	5184.09
2031.79	5123.76

Material Boundary

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455



	Project			Copper Flat		
	Analysis Description					Section D-D' Stability: Downstream, Pseudo Static, Block Failure
	Drawn By	GS	Scale	1:6984	Company	Golder Associates
	Date	11/4/2013, 1:11:49 PM		File Name	2 - Section D 5460R - DS_PS_B.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

- File Name: 2 - Section D 5460R - DS_PS_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
 - 103-92557
 - Material properties edits 12/2013

General Settings

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft³
- Advanced Groundwater Method: None

Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

Loading

- Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight	1e-025	113	120	120	120	127	108

[lbs/ft3]							
Cohesion [psf]	0	0	150	0			
Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5

9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
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18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2

21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
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32999.4	8656.6
33417.1	8752.9
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39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8

50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

Block Search Lines

X	Y
0	5184.09
2031.79	5123.76

External Boundary

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58

List Of Coordinates

Piezoline

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450

-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

Material Boundary

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04

1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

Material Boundary

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

Material Boundary

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

Material Boundary

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

491.186	5194.33
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Material Boundary

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

Material Boundary

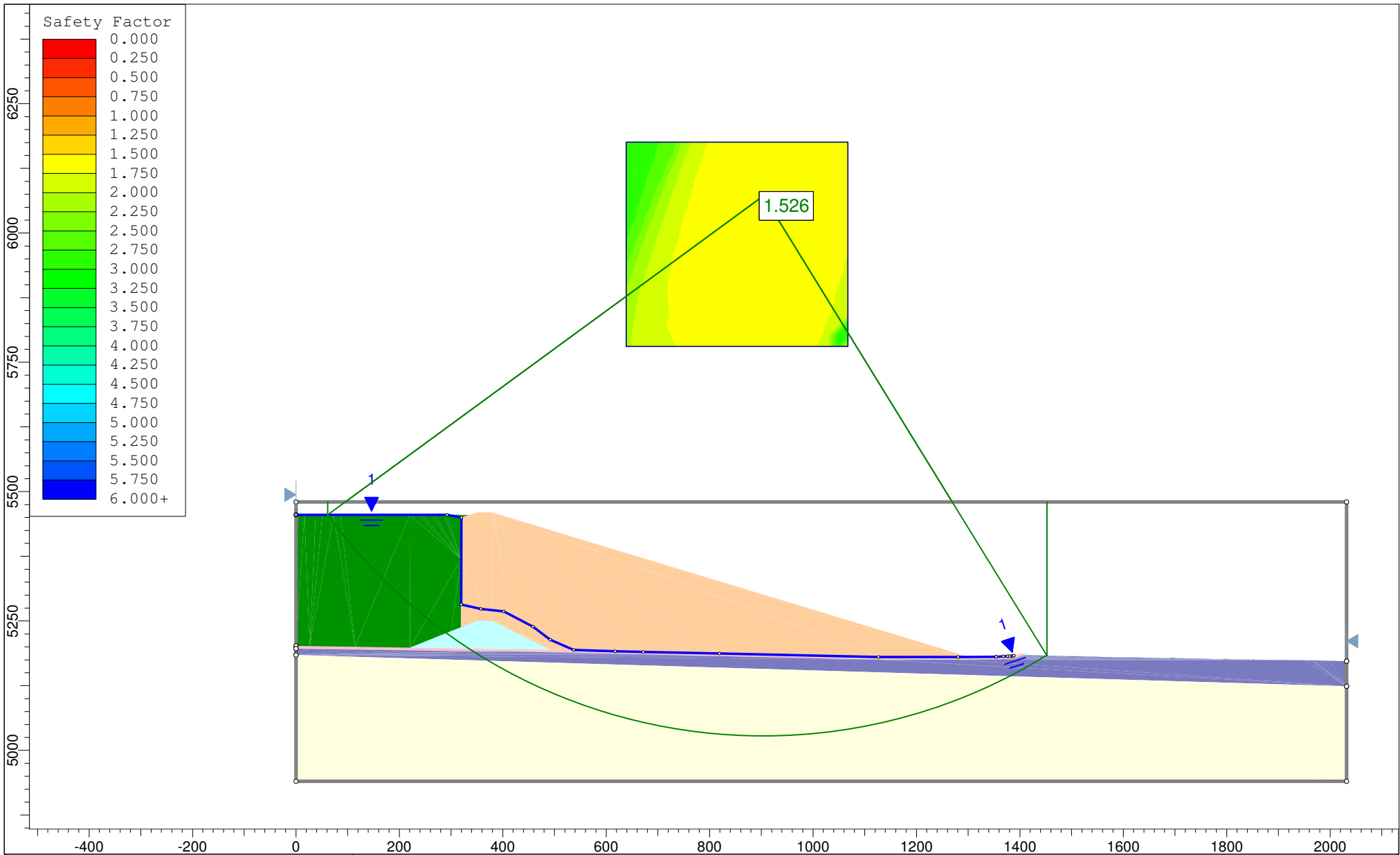
X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

Material Boundary

X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52

Material Boundary

X	Y
0	5184.09
2031.79	5123.76



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section D-D' Stability: Downstream, Static, Circular Failure	
Drawn By	GS	Scale	1:3083
Date	11/4/2013, 1:11:49 PM	Company	Golder Associates
		File Name	3 - Section D 5460R - DS_S_C.slim

Slide Analysis Information

Copper Flat

Project Summary

- File Name: 3 - Section D 5460R - DS_S_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
 - 103-92557
 - Material Property Edits 12/2013

General Settings

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None








Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1

Hu Value	Automatically Calculated	Automatically Calculated	Automatically Calculated
Ru Value	0	0	0

Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4

13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3

28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5

43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

2031.79	5123.76
---------	---------

List Of Coordinates

Piezoline

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

External Boundary

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

Material Boundary

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01

Block Search Lines

X	Y
0	5184.09

1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92

1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

Material Boundary

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

Material Boundary

Material Boundary

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14

1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

Material Boundary

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

Material Boundary

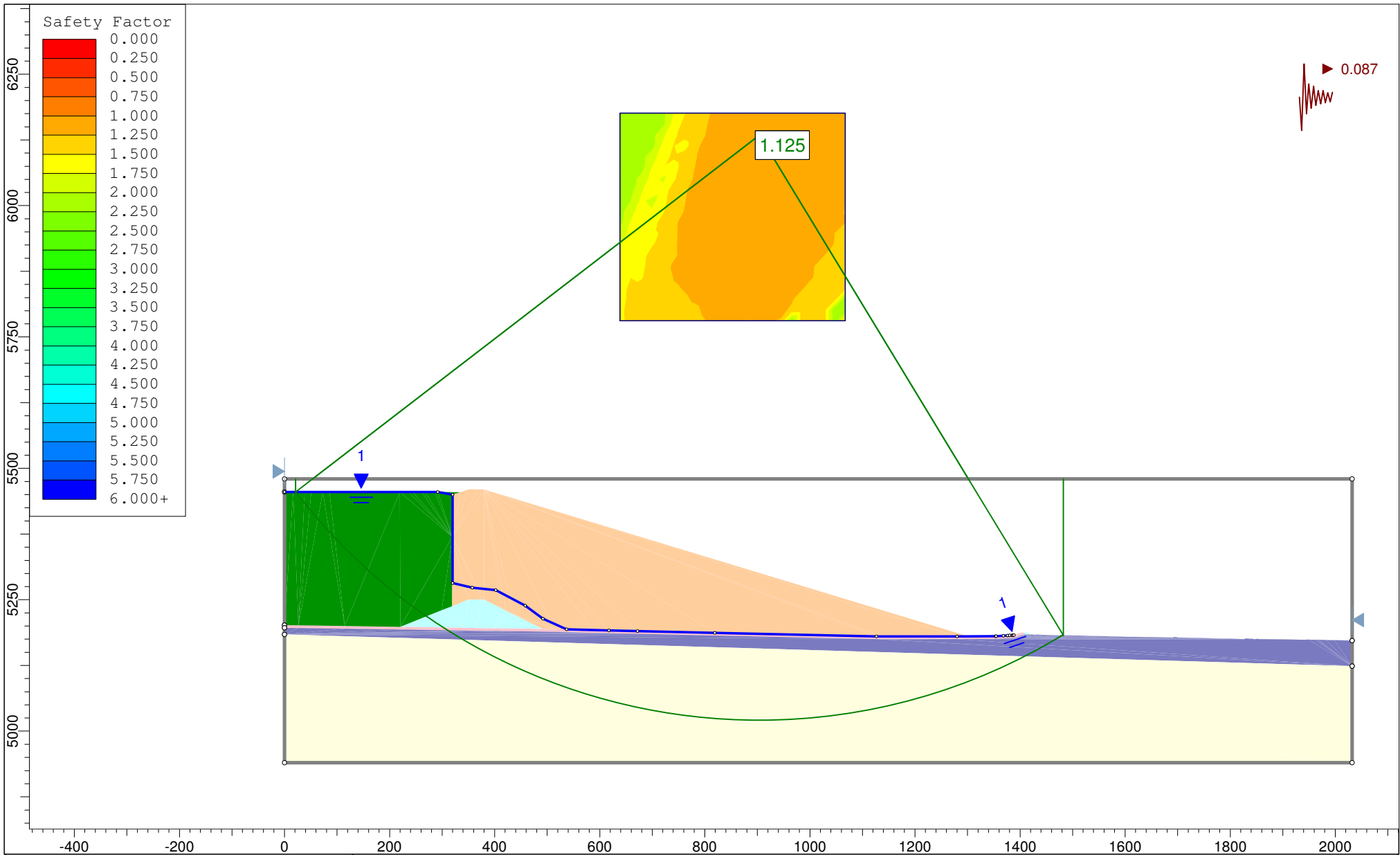
X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


Material Boundary

X	Y
0	5184.09
2031.79	5123.76

Material Boundary

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455



	Project			Copper Flat		
	Analysis Description					Section D-D' Stability: Downstream, Pseudo Static, Circular Failure
	Drawn By	GS	Scale	1:3035	Company	Golder Associates
	Date	11/4/2013, 1:11:49 PM		File Name	4 - Section D 5460R - DS_PS_C.slim	

SLIDEINTERPRET 6.008

Slide Analysis Information

Copper Flat

Project Summary

- File Name: 4 - Section D 5460R - DS_PS_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
 - 103-92557
 - Material Property Edits 12/2013

General Settings

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None

Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

Loading

- Seismic Load Coefficient (Horizontal): 0.087

Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		

Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio				0.2			
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1

10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8

36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6

54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

Block Search Lines

X	Y
0	5184.09
2031.79	5123.76

External Boundary

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

List Of Coordinates

Piezoline

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92

Material Boundary

X	Y
---	---

1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26

1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

Material Boundary

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

Material Boundary

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69

Material Boundary

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23

458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

Material Boundary

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

Material Boundary

X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33

Material Boundary

X	Y
0	5184.09
2031.79	5123.76

Material Boundary

X	Y
-8.88178e-016	5455
3.52609	5455

APPENDIX I
FOUNDATION SETTLEMENT POTENTIAL EVALUATION

APPENDIX I.1
SETTLEMENT POTENTIAL, COPPER FLAT TAILINGS EMBANKMENT FOUNDATION

Date: April 23, 2013
Project No.: 103-92557
Subject: Settlement Potential, Copper Flat
Tailings Embankment Foundation
Project Short Title: **COPPER FLAT FEASIBILITY STUDY TAILINGS DISPOSAL FACILITY DESIGN**

Made by: DP
Checked by: GM
Reviewed by: MJG

1.0 OBJECTIVE

Evaluate the consolidation characteristics of the soil strata, and estimate the post-construction subgrade settlement under the containment embankments for the Copper Flat Tailings Impoundment.

2.0 BACKGROUND

Golder is completing the feasibility level designing of the Copper Flat tailings impoundment embankment which will impound slurried tailings to an estimated maximum depth of approximately 230 feet at the impoundment embankment. An earthen starter dam will be constructed to a height of approximately 50 feet and the remainder of the dam will be constructed with sand recovered from the tailings in a cyclone plant. A geotechnical investigation was performed in the vicinity of the proposed embankment, which included standard penetration testing and sample collection from the surface to a depth of 53 feet. Drilling indicated that the tailings embankment area consists primarily of alluvial deposits that include silt, sand and gravel, underlain by clay.

For the settlement analysis, representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Additionally, select samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing. The results are summarized in the following sections.

Drill hole logs were input onto a site plan and cross-sections through the embankment were developed. Two cross-sections (B-B' and D-D') were further developed for settlement potential analyses with the addition of information from the site investigation conducted in 1980 for the former Quintana mining operation (*Tailings Dam and Disposal Area, Quintana Minerals Corporation, Copper Flats Project, Golddust, New Mexico, Sergeant Hauskins and Beckwith, 1980*). Generalized soil and rock designations were assigned to the strata.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because the results for the post-construction condition demonstrate that the amount of predicted settlement (approximately 2.1 feet under the heaviest loaded portion of embankment) is tolerable, and, will be adequately mitigated by fill placement during ongoing embankment construction.

Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc. for the analysis of settlement and consolidation under foundations and embankments.

The results of the consolidation tests performed on the samples obtained during geotechnical investigations are included in Attachment 1. Tables summarizing the laboratory consolidation test results and the computer modeling input parameters are included in Attachment 2. The SETTLE3D computer model output files are included in Attachment 3. Figures depicting the two embankment sections

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix I Foundation Settlement Potential Evaluation

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analyzed for foundation settlement, as well as a graphical representation of the analysis results, are included in Attachment 4.

3.0 LABORATORY ANALYSIS

Laboratory one-dimensional consolidation analyses were performed on representative samples obtained during the geotechnical investigation. The results of the geotechnical laboratory analyses are summarized in the Tables included in Attachment 1. Consolidation tests were generally conducted on the fine grain fraction of the soil- strata, on remolded drill hole samples.

As shown, the soil strata generally have compression index values (C_c) ranging from 0.035 to 0.09, and estimated initial void ratio values ranging from 0.71 to 0.75. The compression index results were obtained from review of the laboratory graphs of void ratio versus stress, and were obtained for loads ranging from 16 to 32 kilopounds per square foot (ksf).

4.0 COMPUTER MODELING

Computer modeling was performed using SETTLE3D v. 2.0. The results of the modeling are included in Attachment 3.

The following parameters were incorporated into the analyses:

- Settlement Index (C_c) values obtained from laboratory testing of soil fraction of field samples.
- Initial void ratio (e_0) values estimated from review of the consolidation test graphs.
- Unit weights estimated from the consolidation test results, and based on previous experience with similar soils.
- An overconsolidation ratio (OCR) of 1.0 assumed.
- A Poisson's Ratio of 0.3 was assumed, based on a published range of values from 0.2 to 0.4 for sandy clays.
- The model was initialized for primary consolidation, with non-linear settlement characteristics.

A conservative value of 4,000 ksf was assigned for the constrained modulus of the coarse sand and gravel, caliche and weathered basalt layers. The constrained modulus was assigned to represent inter-particle consolidation in the gravels and sands. Settlement in these layers was demonstrated to be minimal, even with the conservative modulus value used in the model. (Reference: US Army Corps of Engineers, EM 1110-1-1904, 1990).

Modeling was performed on an individual point basis, at a minimum of 5 analysis points on each embankment cross-section. The model was initialized for 1-dimensional Boussinesq Methodology.

5.0 RESULTS

5.1 Settlement Beneath Embankment

The results of the consolidation analyses are summarized on Figures 5 and 7 in Attachment 4. As shown on the figures, calculated settlement beneath the impoundment embankment ranges from approximately 2.1 to 0.2 feet, with settlement decreasing as the weight of post-construction loading decreases towards the outer toe of the embankment. The results of the consolidation testing indicate the various soils at the site exhibit generally similar consolidation characteristics. Based on the consolidation testing and computer modeling, settlement beneath the embankment is projected to be generally uniform.

The consolidation analyses results are considered conservative, as the laboratory consolidation testing was performed on the fine-grained fraction of soil samples. The effects of inter-particle bearing contact within the gravel-rich strata, which will reduce the estimated settlement, have not been considered in the analyses. Based on the field Standard Penetration Testing (SPT) results, which showed the strata to

generally be very dense to hard, actual post-construction consolidation settlement of less than 1 foot is anticipated.

As the embankment will be constructed continuously, it is anticipated that observed settlement will be accommodated by managing dam fill placement during embankment construction.

5.2 Settlement Beneath Outlet Piping

Section F-F' was developed along the proposed outlet pipe alignment, as shown on Figure 9. The pipe will be comprised of 14-inch diameter Schedule 80 carbon steel, encased in a minimum 42-inch by 42-inch concrete-filled trench.

From review of Section F-F', it appears the inlet of the outlet pipe will be founded within clay overlaying a basalt rock layer. Settlement of the outlet pipe foundation may be reduced at the inlet side by the basalt, which appears to transition to soil-gravel strata at some undefined location along the pipe downstream of the inlet. Stresses induced by the limited differential settlement are expected to be negligible, and limited by the concrete backfill and steel pipe strength. The pipe currently is proposed with a vertical drop of approximately 31 feet along the 1290-foot length of pipe. Post consolidation pipe slope (assuming a worst-case settlement of 2.1 feet at the pipe inlet) would average 2.3 percent along the length of the pipe. As described above, field standard penetration tests suggest actual post-construction settlement will be less than that calculated on the basis of consolidation test results.

Attachments

- Attachment 1 – Laboratory Test Summary Tables and Consolidation Test Results
- Attachment 2 – Consolidation Testing Summary and SETTLE 3D Model Input Tables
- Attachment 3 – SETTLE 3D Output Files
- Attachment 4 – Embankment Analysis Cross-sections with Graphical Output Results
(Sections B-B' and D-D' only)

ATTACHMENT 1
LABORATORY TEST SUMMARY TABLES AND CONSOLIDATION TEST RESULTS

	Initial		Final
Height =	0.994 in		0.960 in
Diameter =	2.499 in		2.499 in
Area =	4.905 in ²		4.905 in ²
Volume =	4.875 in ³		4.709 in ³
Water Content =	14.4%		4.8%
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)
Height of Solids =	0.5641 in		0.5641 in
Void Ratio =	0.762		0.702
Degree of Saturation =	51.1%		18.5%
Wet Mass =	0.308 lb		0.282 lb
Dry Mass =	0.269 lb		0.269 lb
Wet Unit Weight =	109.3 pcf		103.6 pcf
Dry Unit Weight =	95.5 pcf		98.9 pcf

Notes

USCS description (ASTM D2487):
 Atterberg Limits (ASTM D4318):
 Percent Finer (ASTM D422):
 Specimen Type:
 Remold Targets:
 Water Content of Trimmings (ASTM D2216):
 Trimming Procedure:
 Inundation:
 Test Method:
 Apparatus:
 Final Water Content Specimen:
 Final Differential Height:
 Estimated Preconsolidation Stress:

Lean clay with sand, yellowish red, moist
 LL = 47 PL = 20 PI = 27
 3/4 in. = 100% No. 4 = 99% No. 200 = 81%
 Intact Reconstituted
 95.0 pcf (dry) at 15.0% moisture
 14.6%
 Specimen remolded in ring
 Not inundated Inundated
 A B
 Frame No. 1 (Wykeham Farrance 24251)
 Entire Partial
 -0.0158 in
 Not Computed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

Golder Associates Inc.
Denver, Colorado

Job Short Title:

Copper Flat Tailings Design Study

Title:

ASTM D2435
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT
 SPECIMEN AND SUMMARY DATA

Sample:

BH-10 @ 19-33 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/11/2013

Job Number:

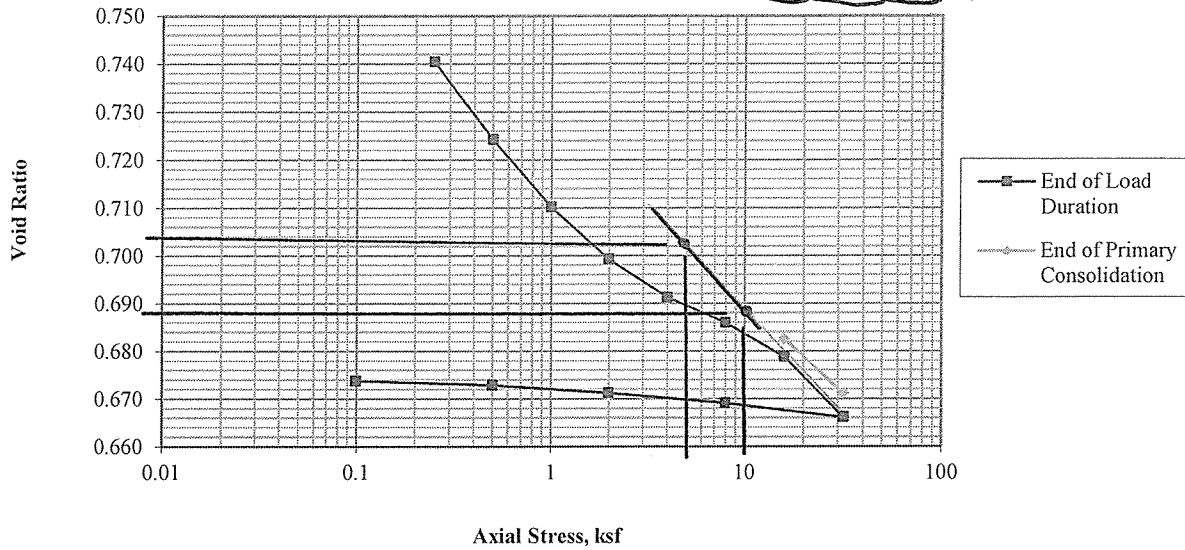
103-92557.006

Figure:

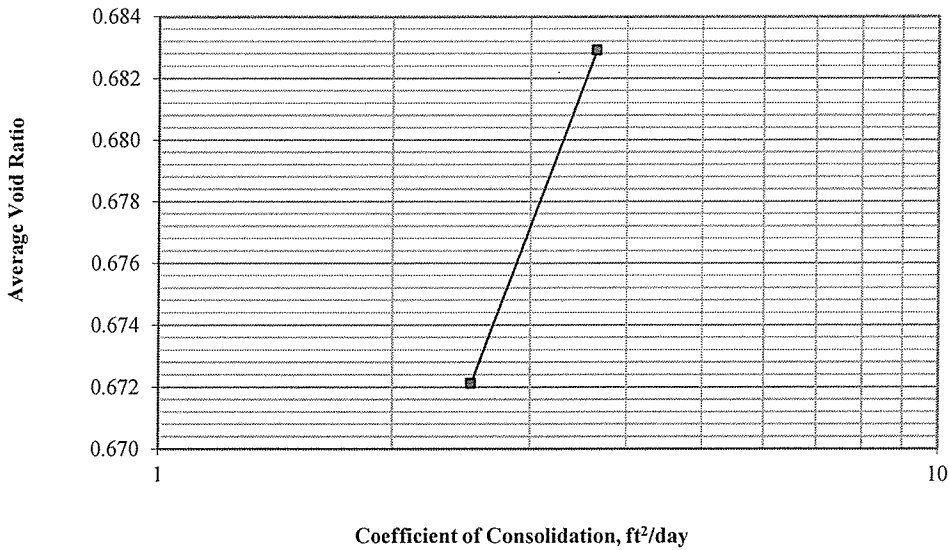
1

Void Ratio vs. Axial Stress

$$C_c = \frac{.702 - .688}{\log\left(\frac{10}{5}\right)} = 0.05$$

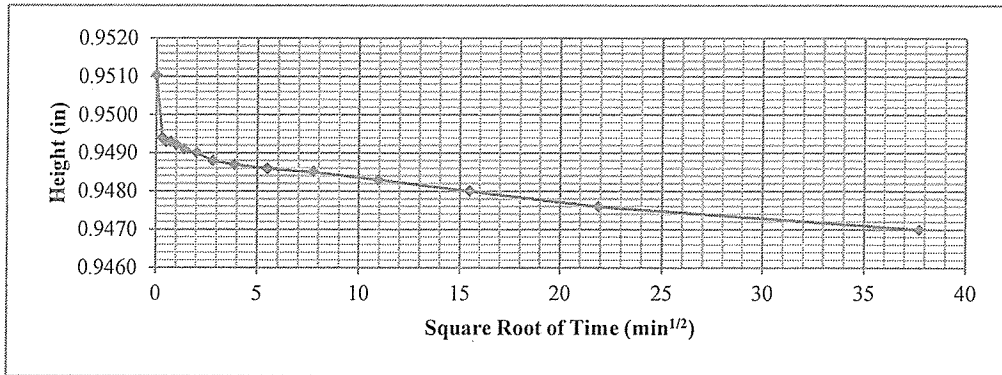


Average Void Ratio vs. Coefficient of Consolidation

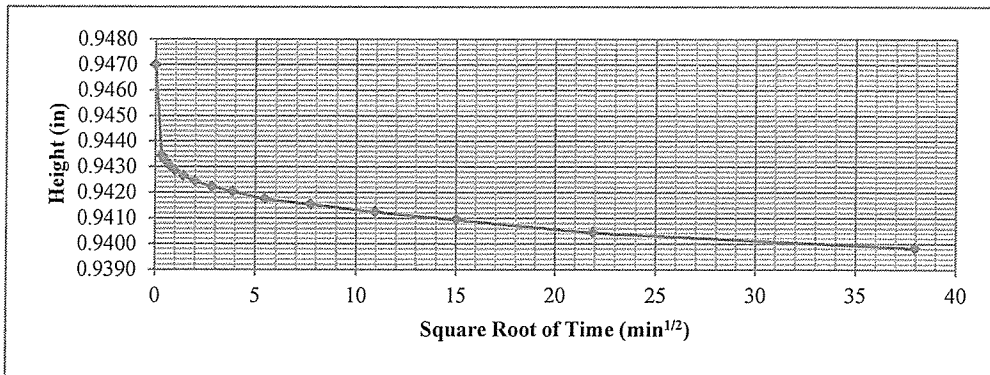


Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-10 @ 19-33 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2	

16.00 ksf



32.00 ksf

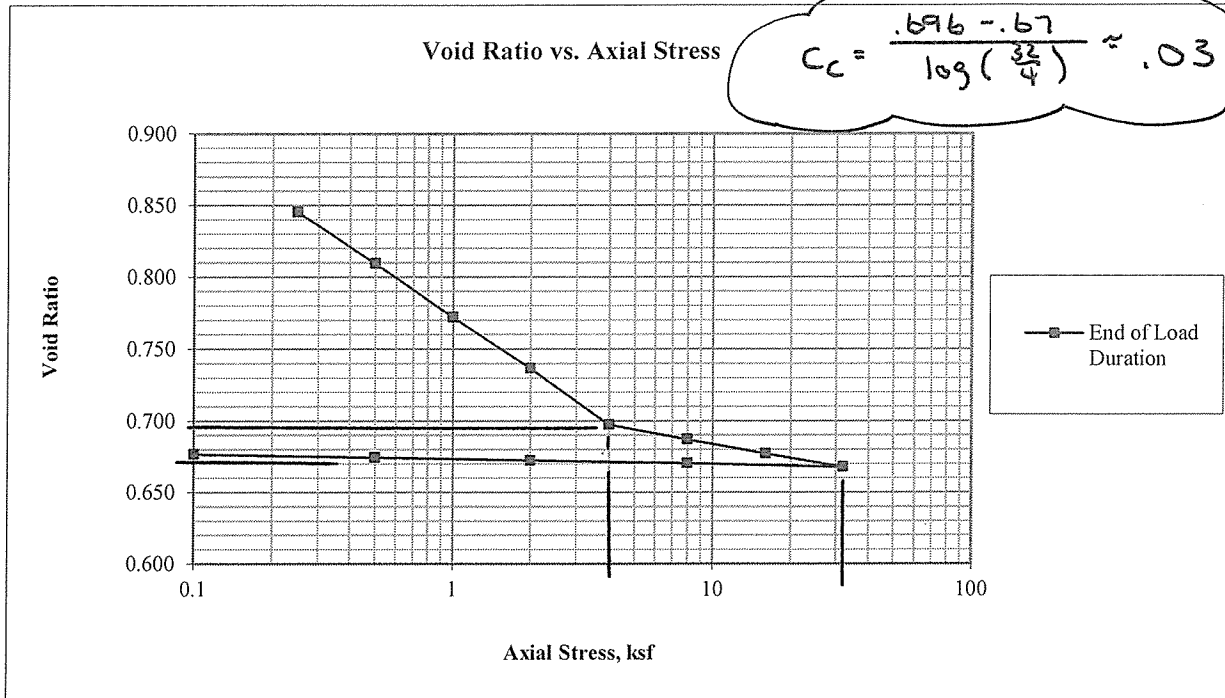


<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: BH-10 @ 19-33 ft</p>	<p>Technician: RJM</p>	<p>Reviewed: CCS</p>	<p>Start Date: 3/11/2013</p>	<p>Job Number: 103-92557.006</p>	<p>Figure: 3</p>

	Initial		Final	Notes			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in ²		4.893 in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in ³		4.428 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-12 @ 33.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-12 @ 33.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

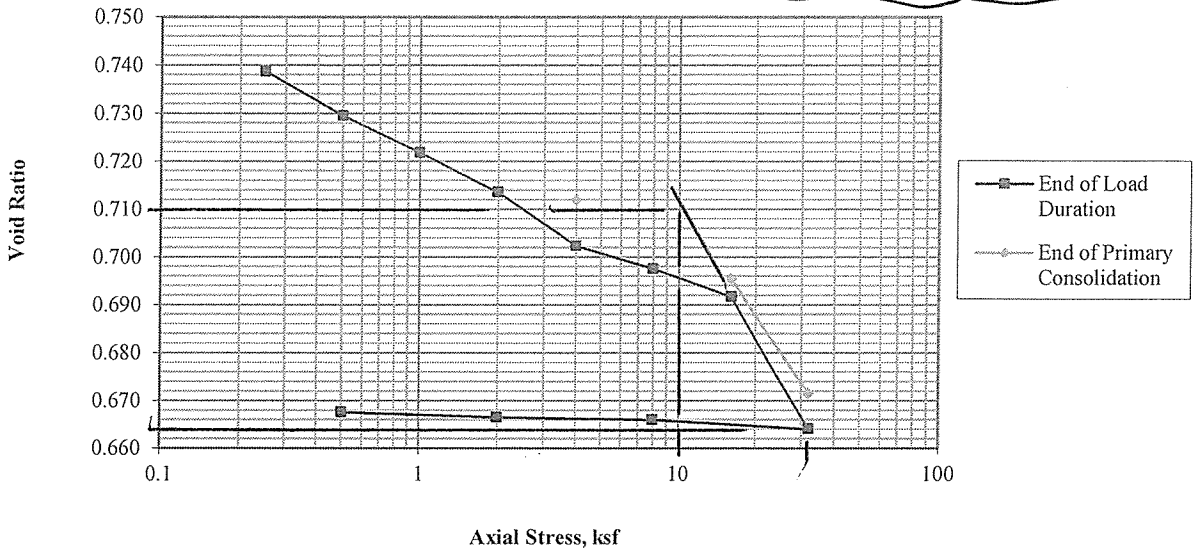
	Initial		Final	Notes			
Height =	0.993	in	0.887	in	USCS description (ASTM D2487):	Clayey sand with gravel, yellowish brown, dry	
Diameter =	2.500	in	2.500	in	Atterberg Limits (ASTM D4318):	LL = 23	PL = 14
Area =	4.909	in ²	4.909	in ²	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 83%
Volume =	4.874	in ³	4.354	in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted
Water Content =	14.1%		1.7%		Remold Targets:	95.0	pcf (dry) at 15.0% moisture
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%	
Height of Solids =	0.5636	in	0.5636	in	Trimming Procedure:	Specimen remolded in ring	
Void Ratio =	0.762		0.574		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated
Degree of Saturation =	49.8%		8.0%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B
Wet Mass =	0.307	lb	0.274	lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial
Wet Unit Weight =	108.9	pcf	108.7	pcf	Final Differential Height:	0.0528 in	
Dry Unit Weight =	95.5	pcf	106.9	pcf	Estimated Preconsolidation Stress:	16.4 ksf	
							PI = 9
							No. 200 = 36%

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

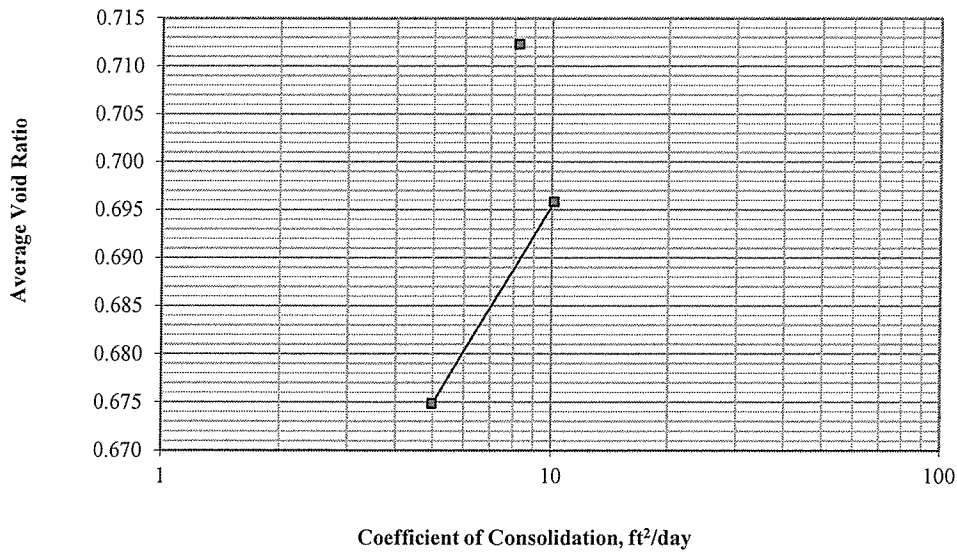
Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: I	

Void Ratio vs. Axial Stress

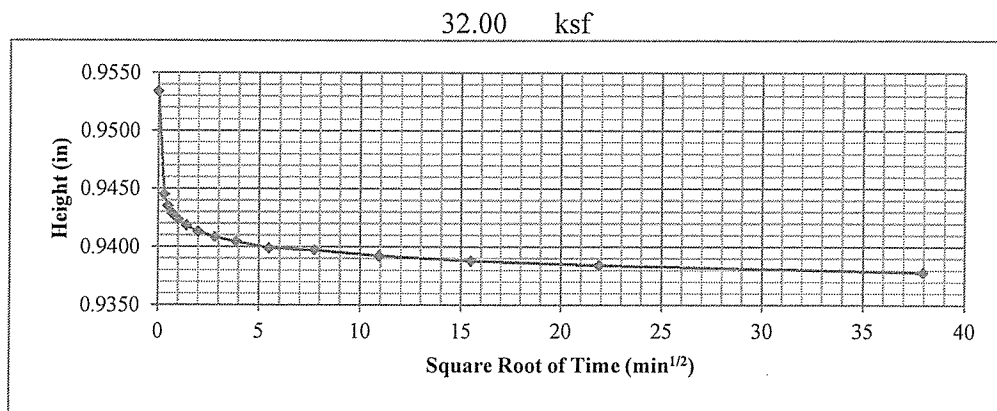
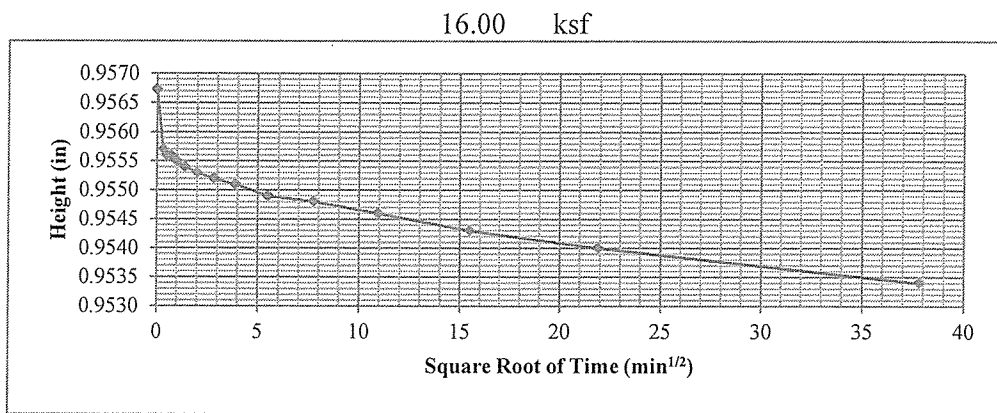
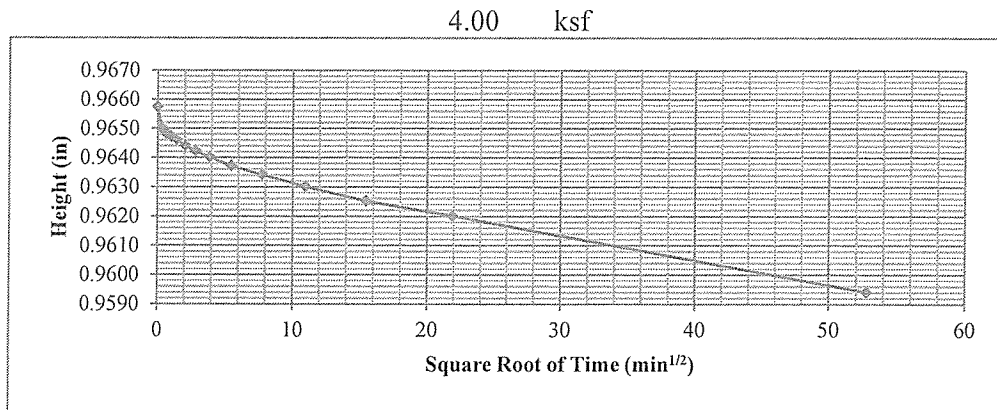
$$C_c = \frac{.71 - .664}{\log\left(\frac{32}{10}\right)} \approx .09$$



Average Void Ratio vs. Coefficient of Consolidation



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
	Job Short Title: Copper Flat Tailings Design Study				
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 3

	Initial		Final	Notes			
Height =	0.997 in		0.960 in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 14	PI = 11
Area =	4.901 in ²		4.901 in ²	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 52%
Volume =	4.886 in ³		4.705 in ³	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.5%		2.7%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%		
Height of Solids =	0.5638 in		0.5638 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.768		0.703	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		10.4%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308 lb		0.276 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.0 pcf		101.5 pcf	Final Differential Height:	0.0045 in		
Dry Unit Weight =	95.1 pcf		98.8 pcf	Estimated Preconsolidation Stress:	14.7 ksf		

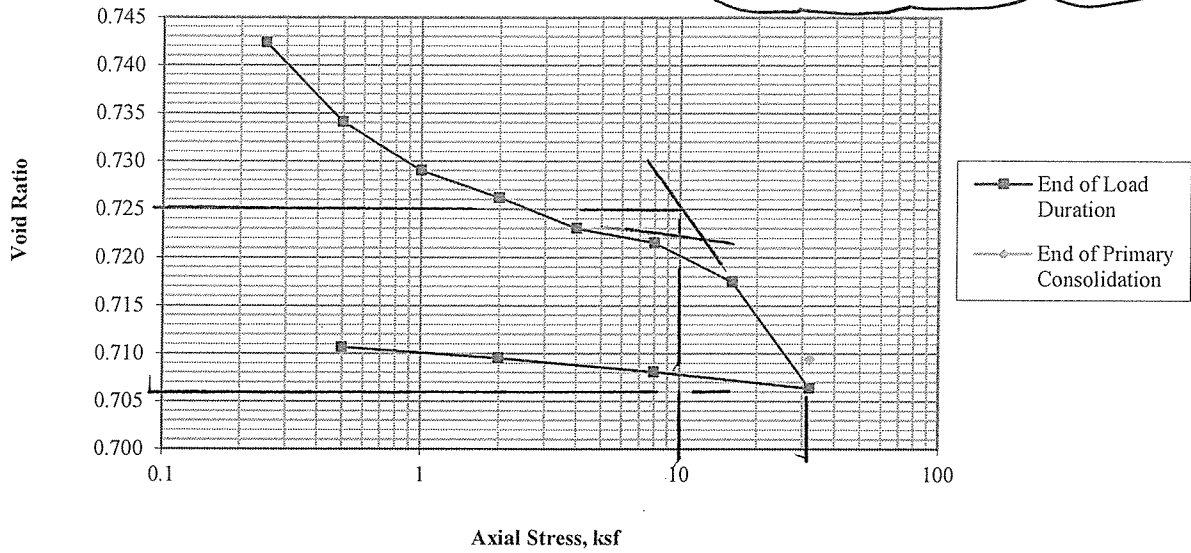
CL

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

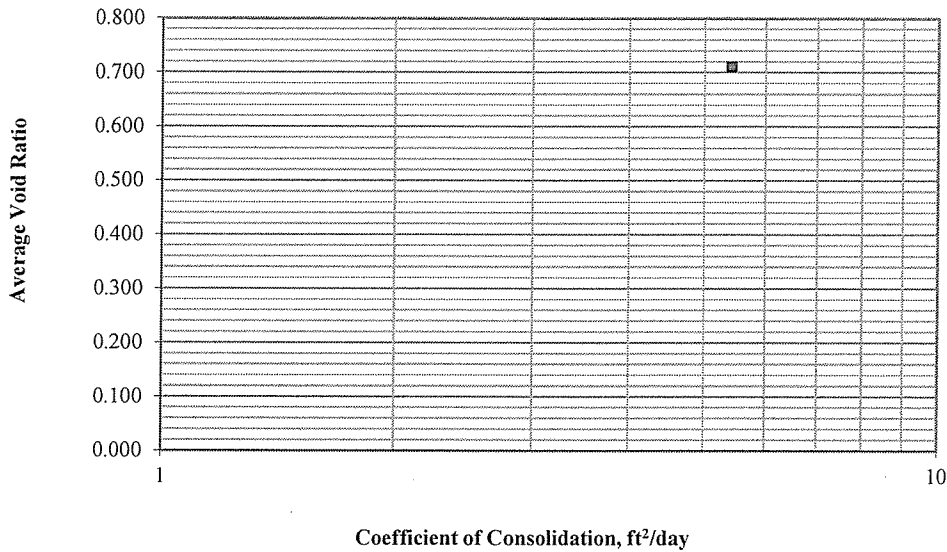
Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study		Technician: RJM		Reviewed: CCS	Start Date: 3/25/2013
Sample: BH-18 @ 23-33.5 ft		Job Number: 103-92557.006		Figure: 1	

Void Ratio vs. Axial Stress

$$C_c = \frac{.725 - .706}{\log\left(\frac{32}{10}\right)} \approx .04$$

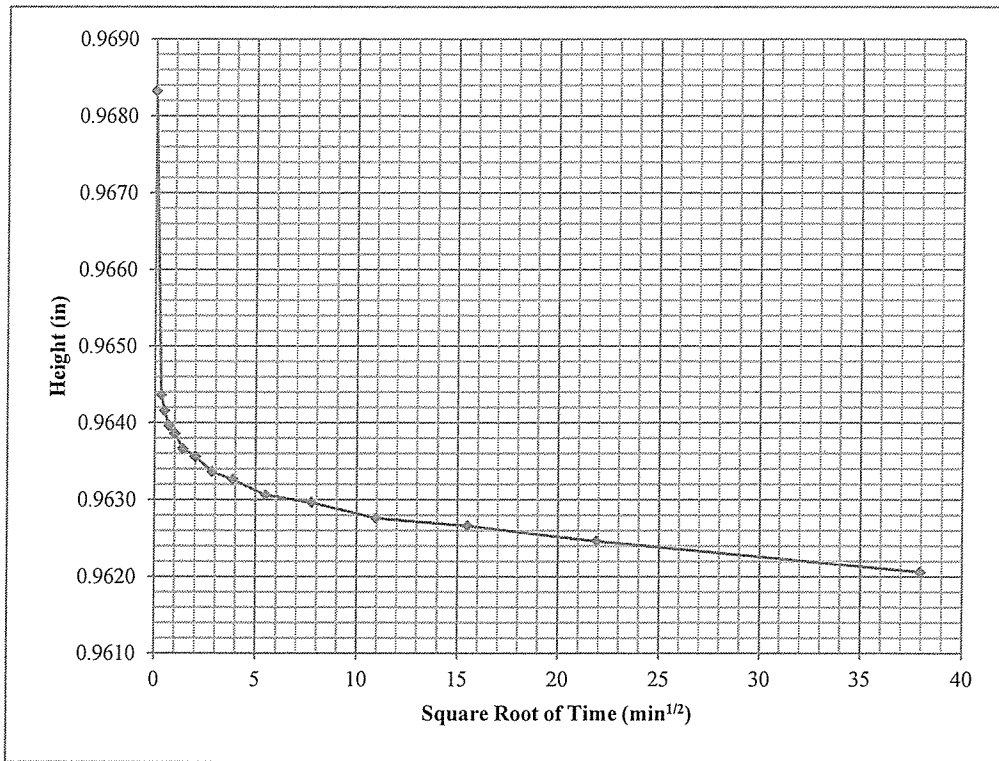


Average Void Ratio vs. Coefficient of Consolidation



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	

32.00 ksf



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 3	

	Initial		Final	
Height =	0.994	in	0.924	in
Diameter =	2.498	in	2.498	in
Area =	4.901	in ²	4.901	in ²
Volume =	4.871	in ³	4.528	in ³
Water Content =	28.9%		9.0%	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)
Height of Solids =	0.5085	in	0.5085	in
Void Ratio =	0.955		0.817	
Degree of Saturation =	81.7%		29.7%	
Wet Mass =	0.313	lb	0.264	lb
Dry Mass =	0.243	lb	0.243	lb
Wet Unit Weight =	110.9	pcf	100.9	pcf
Dry Unit Weight =	86.1	pcf	92.6	pcf

Notes

USCS description (ASTM D2487):
 Atterberg Limits (ASTM D4318):
 Percent Finer (ASTM D422):
 Specimen Type:
 Remold Targets:
 Water Content of Trimings (ASTM D2216):
 Trimming Procedure:
 Inundation:
 Test Method:
 Apparatus:
 Final Water Content Specimen:
 Final Differential Height:
 Estimated Preconsolidation Stress:

Fat clay with sand, dark red, wet

LL = 62
 3/4 in. = 100%

PL = 20
 No. 4 = 100%

PI = 42
 No. 200 = 82%

Intact Reconstituted
 86.0 pcf (dry) at 29.0% moisture

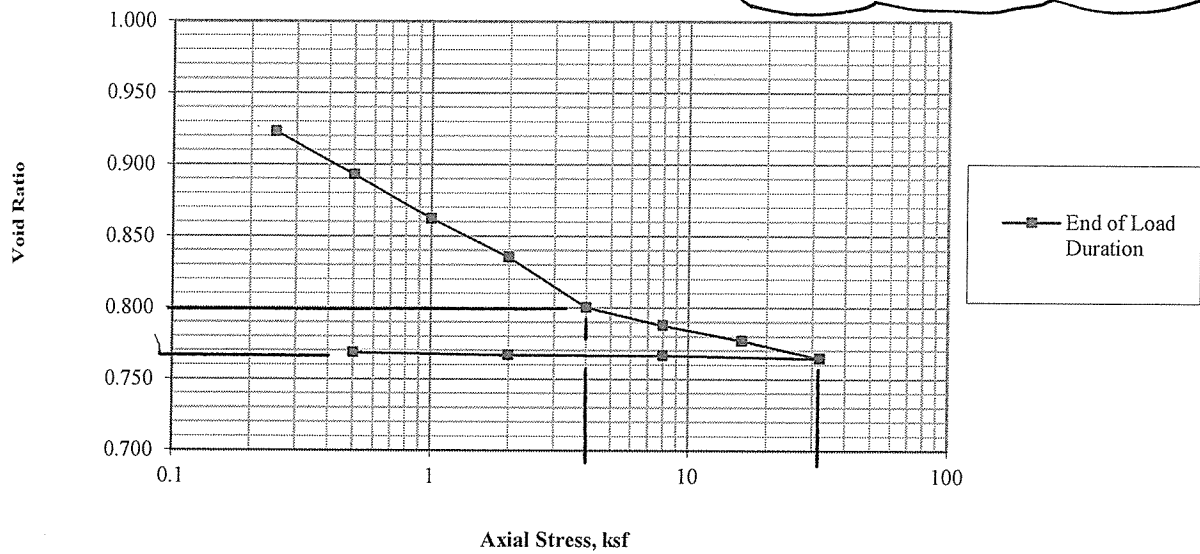
Specimen remolded in ring
 Not inundated Inundated
 A B
 Frame No. 5 (ELE C-320A)
 Entire Partial
 -0.0246 in
 -- ksf
 -- indicates test was not performed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

Golder Associates Inc. Denver, Colorado		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Sample: BH-18 @ 43.5-48.5 ft		Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006
					Figure: 1

Void Ratio vs. Axial Stress

$$C_c \approx \frac{.80 - .768}{\log\left(\frac{32}{4}\right)} \approx .04$$



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 43.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	

	Initial		Final	
Height =	1.000	in	0.981	in
Diameter =	2.497	in	2.497	in
Area =	4.897	in ²	4.897	in ²
Volume =	4.897	in ³	4.804	in ³
Water Content =	9.7%		1.1%	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)
Height of Solids =	0.5363	in	0.5363	in
Void Ratio =	0.865		0.829	
Degree of Saturation =	30.3%		3.6%	
Wet Mass =	0.281	lb	0.259	lb
Dry Mass =	0.256	lb	0.256	lb
Wet Unit Weight =	99.0	pcf	93.0	pcf
Dry Unit Weight =	90.2	pcf	92.0	pcf

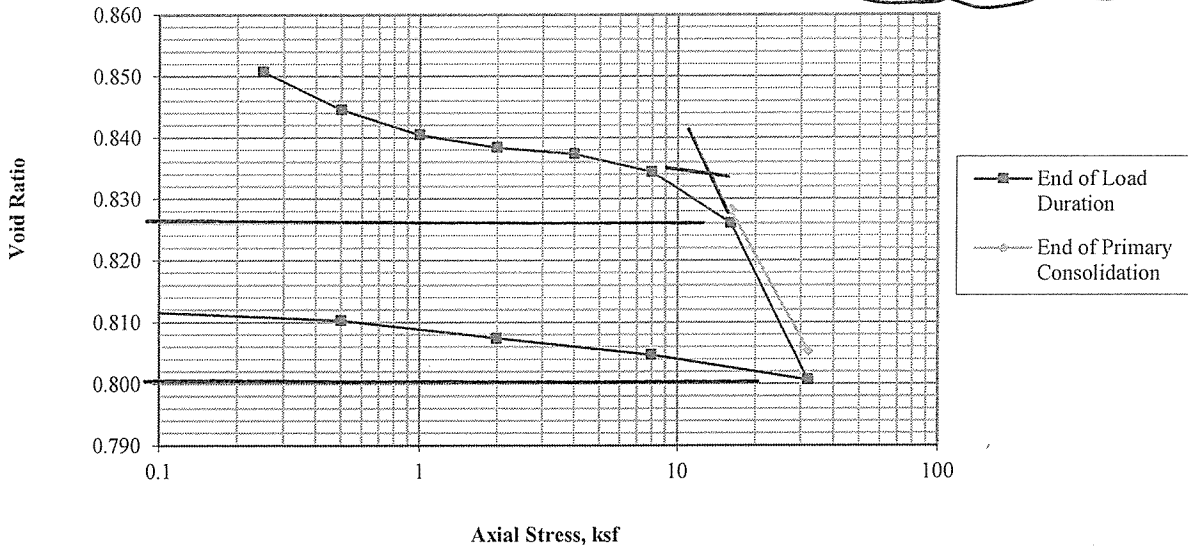
Notes
USCS description (ASTM D2487): Sandy silty clay, brownish yellow, moist
Atterberg Limits (ASTM D4318): LL = 25 PL = 21 PI = 4
Percent Finer (ASTM D422): 3/4 in. = 99% No. 4 = 99% No. 200 = 52%
Specimen Type: Intact Reconstituted
Remold Targets: 90.0 pcf (dry) at 10.0% moisture
Water Content of Trimmings (ASTM D2216): 9.4%
Trimming Procedure: Specimen remolded in ring
Inundation: Not inundated Inundated
Test Method: A B
Apparatus: Frame No. 6 (ELE C-320A)
Final Water Content Specimen: Entire Partial
Final Differential Height: -0.0094 in
Estimated Preconsolidation Stress: 13.0 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

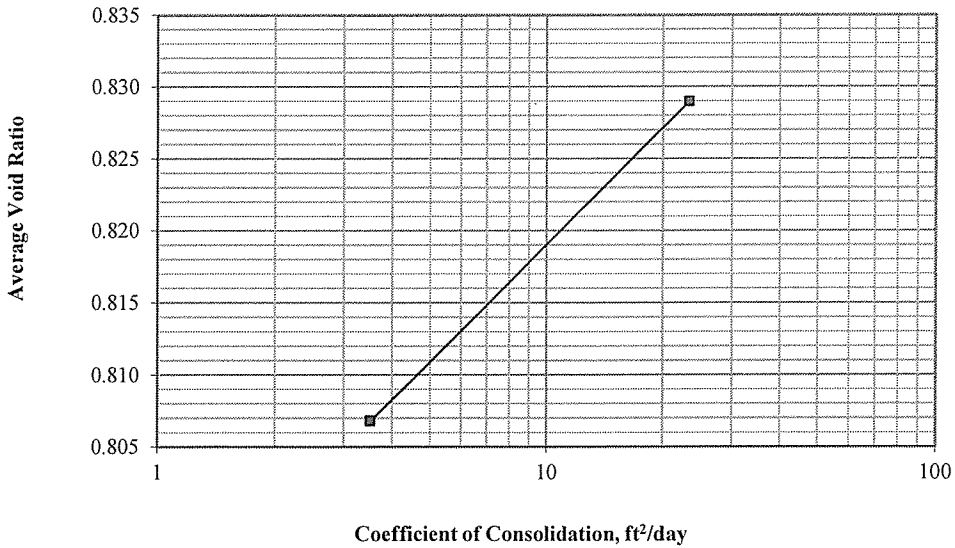
Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 0-8.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

Void Ratio vs. Axial Stress

$$C_c = \frac{.826 - .80}{\log\left(\frac{32}{16}\right)} = 0.09$$

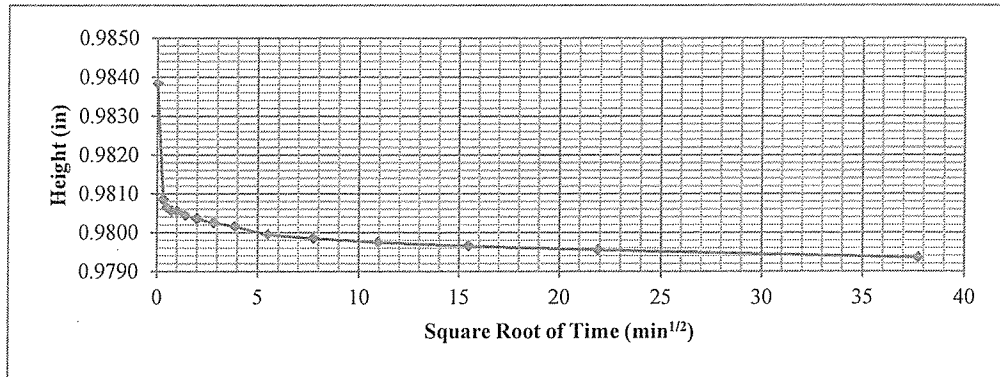


Average Void Ratio vs. Coefficient of Consolidation

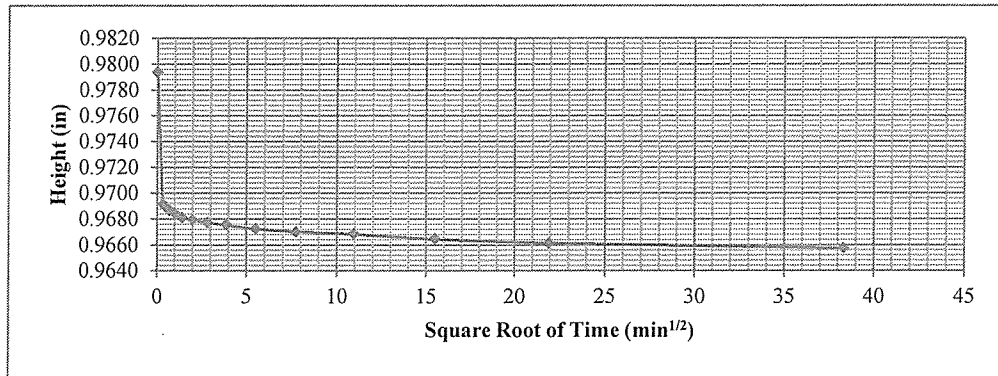


Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
	Job Short Title: Copper Flat Tailings Design Study				
Sample: BH-22 @ 0-8.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2

16.00 ksf



32.00 ksf

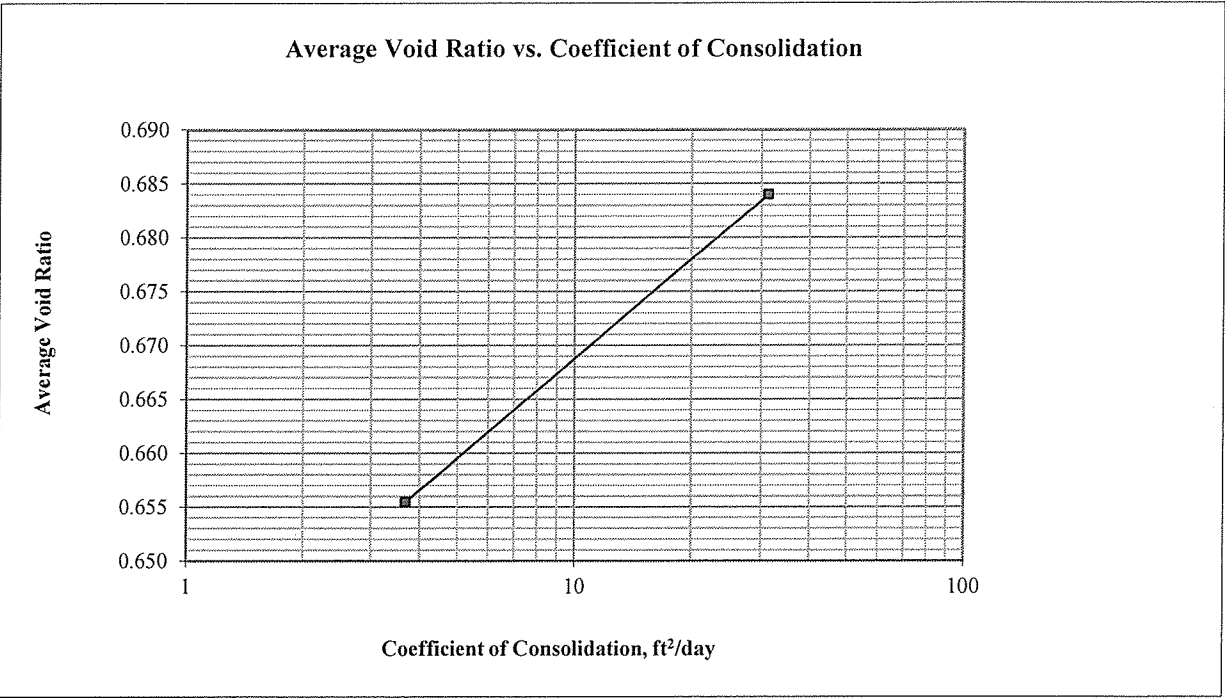
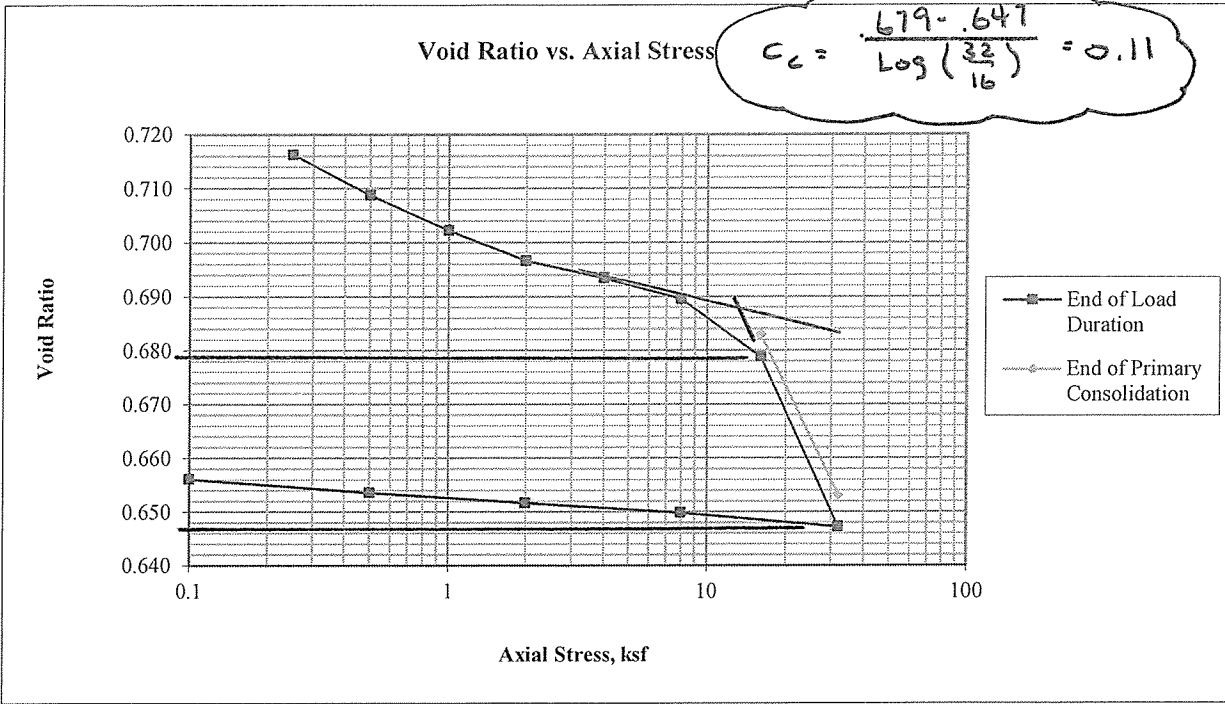


<p>Golder Associates Inc. Denver, Colorado</p>		<p>Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</p>			
<p>Job Short Title: Copper Flat Tailings Design Study</p>					
<p>Sample: BH-22 @ 0-8.5 ft</p>	<p>Technician: RJM</p>	<p>Reviewed: CCS</p>	<p>Start Date: 3/11/2013</p>	<p>Job Number: 103-92557.006</p>	<p>Figure: 3</p>

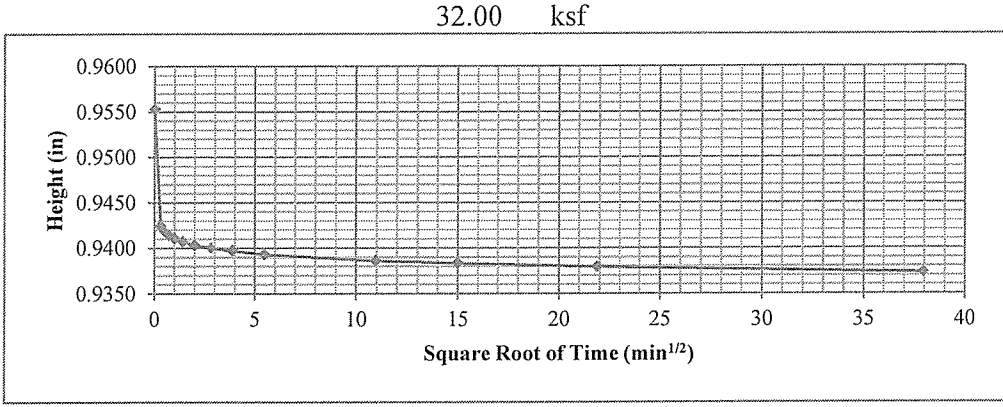
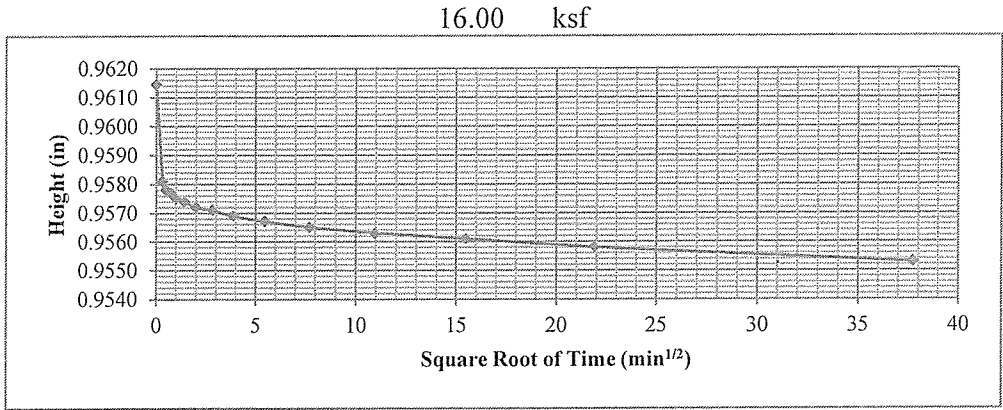
	Initial		Final	Notes	
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36 PL = 18 PI = 18
Area =	4.901 in ²		4.901 in ²	Percent Finer (ASTM D422):	3/4 in. = 98% No. 4 = 82% No. 200 = 37%
Volume =	4.886 in ³		4.528 in ³	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at 15.0% moisture
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated <input type="checkbox"/> Inundated
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5 (ELE C-320A)
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3

	Initial	Final
Height =	1.000 in	0.973 in
Diameter =	2.498 in	2.498 in
Area =	4.901 in ²	4.901 in ²
Volume =	4.901 in ³	4.769 in ³
Water Content =	9.8%	4.2%
Specific Gravity =	2.70 (Assumed)	2.70 (Assumed)
Height of Solids =	0.5558 in	0.5558 in
Void Ratio =	0.799	0.751
Degree of Saturation =	33.0%	15.1%
Wet Mass =	0.291 lb	0.276 lb
Dry Mass =	0.265 lb	0.265 lb
Wet Unit Weight =	102.7 pcf	100.1 pcf
Dry Unit Weight =	93.5 pcf	96.1 pcf

Notes

USCS description (ASTM D2487):
 Atterberg Limits (ASTM D4318):
 Percent Finer (ASTM D422):
 Specimen Type:
 Remold Targets:
 Water Content of Trimmings (ASTM D2216):
 Trimming Procedure:
 Inundation:
 Test Method:
 Apparatus:
 Final Water Content Specimen:
 Final Differential Height:
 Estimated Preconsolidation Stress:

Clayey sand, reddish brown, moist

LL = 37 PL = 17 PI = 20

3/4 in. = 100% No. 4 = 95% No. 200 = 39%

Intact Reconstituted

93.0 pcf (dry) at 10.0% moisture

9.9%

Specimen remolded in ring

Not inundated Inundated

A B

Frame No. 6 (ELE C-320A)

Entire Partial

-0.0136 in

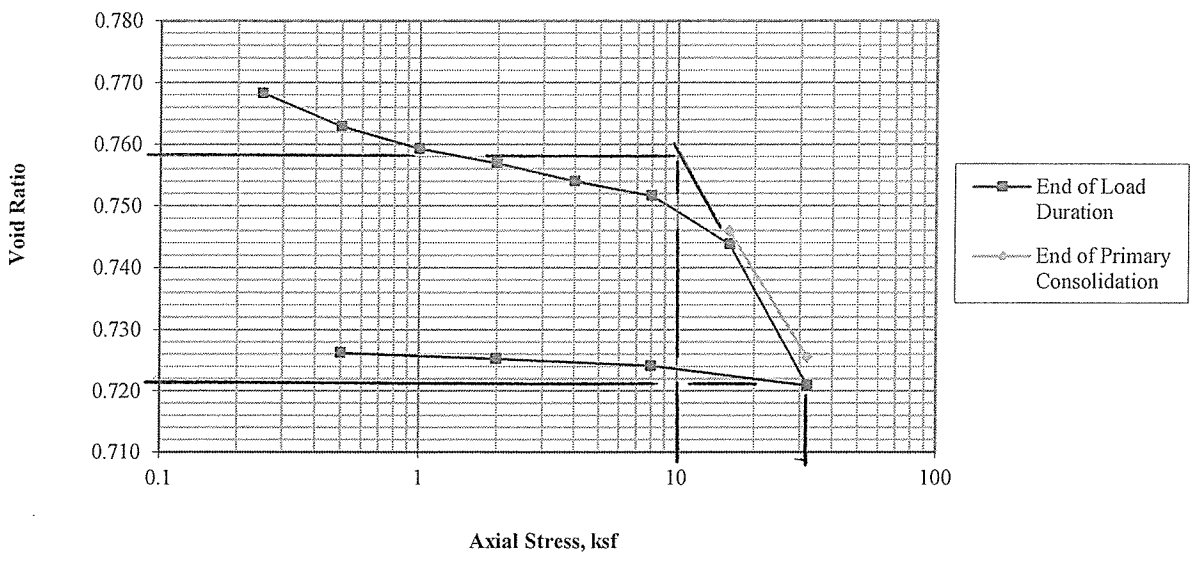
13.8 ksf

Seating	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft ² /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

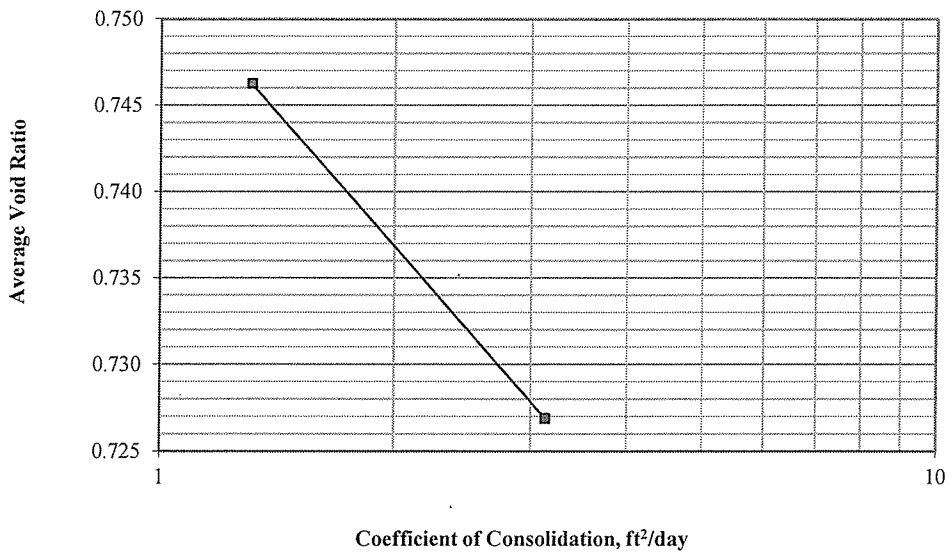
Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1

Void Ratio vs. Axial Stress

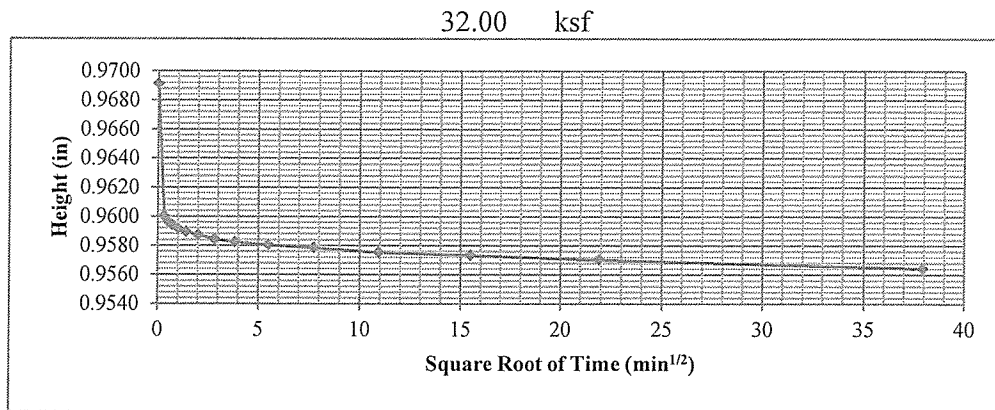
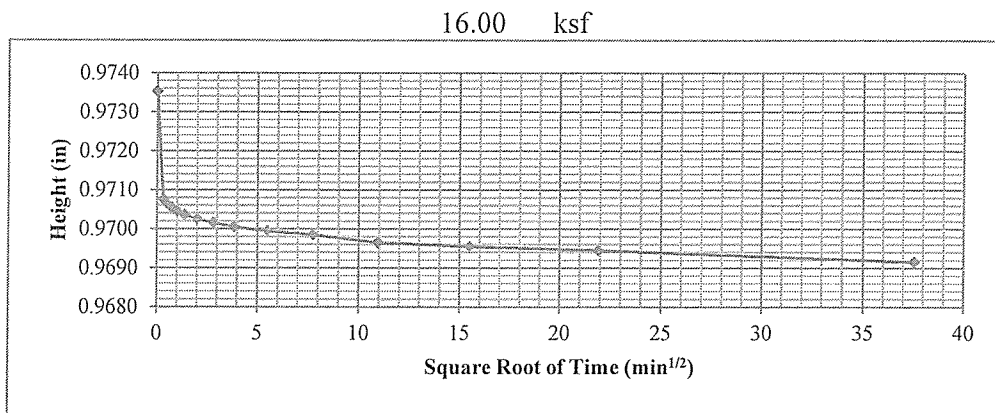
$$C_c = \frac{.758 - .721}{\log\left(\frac{32}{16}\right)} \approx .07$$



Average Void Ratio vs. Coefficient of Consolidation



Golder Associates Inc. Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	



Golder Associates Inc. Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 3

ATTACHMENT 2
CONSOLIDATION TESTING SUMMARY AND SETTLE 3D MODEL INPUT TABLES

Table 1: Summary Results Of Consolidation Tests -- Copper Flat, Sierra County, New Mexico

Boring No.	Sample Depth	In-situ Overburden Pressure at Midpoint of Tested Stratum (kips, Note 1)	Material Classification	Void Ratio, min	Void Ratio, max	Void Ratio, mean	Estimated Initial Void Ratio (from consolidation curve)	Cc for Analysis (Note 2)	Estimated Moist Density (final) (pcf) (Note 3)
BH-10	19-33	2.86	CL	0.666	0.74	0.703	0.69	0.05	104
BH-12	33.5-48.5	4.51	CH	0.67	0.845	0.7575	0.69	0.3	104
BH-16	29-34	3.465	SC	0.664	0.738	0.701	0.704	0.09	110
BH-18	23-33.5	3.1075	CL	0.706	0.742	0.724	0.724	0.04	103
BH-18	43.5-48.5	5.06	CH	0.767	0.925	0.846	0.79	0.04	102
BH-22	0-8.5	0.4675	ML	0.8	0.85	0.825	0.844	0.09	98
BH-22	28-30	3.19	SC	0.646	0.716	0.681	0.695	0.11	108
BH-25	22-34	3.08	SC	0.727	0.746	0.7365	0.756	0.07	101

Notes

1. Based on an assumed unit weight of 110 pcf.
2. Generally based on portion of curve developed for loading between 16 and 32 kips.
3. Moist Density estimated for water content ranging from 3 to 6 percent.

Table 2: Stratum Parameters For Settlement Analysis -- Copper Flat, Sierra County, New Mexico

Stratum Designation	Soil Types	Assumed Initial Void Ratio	Assumed Cc Value	Referenced Consolidation Tests	Assumed Unit Weight (pcf)	Young's Modulus (ksf)
1	ML, CL-ML, SM	0.74	0.09	BH-16, BH-22 (2), BH-25	106	NA
2	CL	0.71	0.045	BH-10, BH-18	104	NA
3	CH, MH	0.75	0.035	BH-12, BH-18	103	NA
4 (Note 1)	Gravel, Sand	NA	NA	Ref. 1	110	4000
5 (Note 2)	Basalt, Caliche	NA	NA	Ref. 1	125	4000
6 (Note 3)	Embankment Fill	NA	NA	NA	97	NA
7 (Note 4)	Tailings	NA	NA	NA	97	NA

Notes

1. Settlement within embankment fill not calculated for foundation settlement evaluation.
2. Settlement within tailings not analyzed for foundation settlement evaluation.

References

1. US Army Corps of Engineer, EM-1110-1904, 1990.

Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

Notes

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.

Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

Notes

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.

**ATTACHMENT 3
SETTLE 3D OUTPUT FILES**

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section B-B', Line 1.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.49066 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.97303
Consolidation Settlement [ft]	0	1.97303
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	22.7
Total Stress [ksf]	0	31.6782
Total Strain	-0	0.10667
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.020072	31.6754
Over-consolidation Ratio	1	1
Void Ratio	0.527594	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 22.7 ksf
 Depth: 0 ft

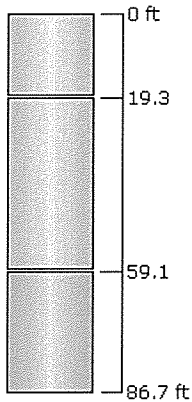
Installation Stage: Stage 1

Coordinates



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	19.3	0
2	Soil Stratum 2	39.8	19.3
3	Soil Stratum 3	27.6	59.1



Soil Properties

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft ³]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

Field Point Grid

Number of points: 289

Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section B-B', Line 2.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.646645 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98619
Consolidation Settlement [ft]	0	1.98619
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.9
Total Stress [ksf]	0	27.3058
Total Strain	-0	0.0869309
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.006916	27.303
Over-consolidation Ratio	1	1
Void Ratio	0.561348	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 13.9 ksf
 Depth: 0 ft

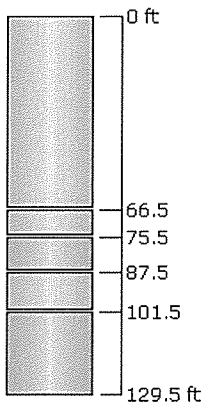
Installation Stage: Stage 1

Coordinates

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	66.5	0
2	Soil Stratum 2	9	66.5
3	Soil Stratum 3	12	75.5
4	Soil Stratum 2	14	87.5
5	Soil Stratum 3	28	101.5



Soil Properties

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft ³]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 59

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section B-B', Line 3.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.561914 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.4179
Consolidation Settlement [ft]	0	1.4179
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.3
Total Stress [ksf]	0	21.0132
Total Strain	-0	0.103067
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.01612	21.0103
Over-consolidation Ratio	1	1
Void Ratio	0.533756	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 13.3 ksf
 Depth: 0 ft

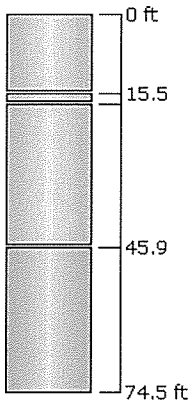
Installation Stage: Stage 1

Coordinates



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	15.5	0
2	Soil Stratum 3	2.1	15.5
3	Soil Stratum 2	28.3	17.6
4	Soil Stratum 3	28.6	45.9



Soil Properties

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft ³]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section B-B', Line 4.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.491452 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.00705
Consolidation Settlement [ft]	0	1.00705
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	7.3
Total Stress [ksf]	0	14.379
Total Strain	-0	0.0723724
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.065	14.3761
Over-consolidation Ratio	1	1
Void Ratio	0.586243	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 7.3 ksf
 Depth: 0 ft

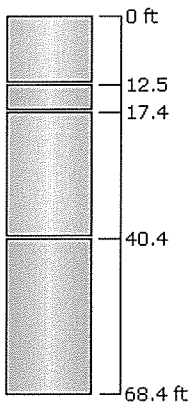
Installation Stage: Stage 1

Coordinates



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	12.5	0
2	Soil Stratum 3	4.9	12.5
3	Soil Stratum 2	23	17.4
4	Soil Stratum 3	28	40.4



Soil Properties

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft ³]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 53

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section B-B', Line 5.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.562388 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.508677
Consolidation Settlement [ft]	0	0.508677
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	1
Total Stress [ksf]	0	8.18236
Total Strain	-0	0.139728
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019928	8.17977
Over-consolidation Ratio	1	1
Void Ratio	0.496874	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 1 ksf
 Depth: 0 ft

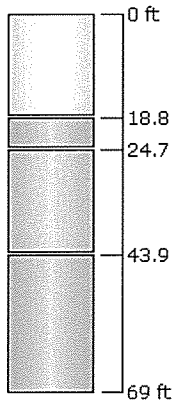
Installation Stage: Stage 1

Coordinates




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.8	0
2	Soil Stratum 3	5.9	18.8
3	Soil Stratum 2	19.2	24.7
4	Soil Stratum 3	25.1	43.9



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft ³]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section D-D', Line 1.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.499416 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	2.11072
Consolidation Settlement [ft]	0	2.11072
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	23.4
Total Stress [ksf]	0	28.7145
Total Strain	-0	0.211235
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019292	28.7127
Over-consolidation Ratio	1	1
Void Ratio	0.372451	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 23.4 ksf
 Depth: 0 ft

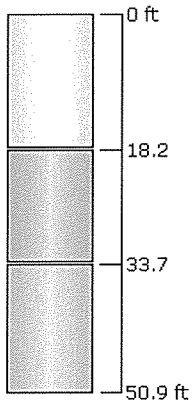
Installation Stage: Stage 1

Coordinates




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.2	0
2	Soil Stratum 3	15.5	18.2
3	Soil Stratum 2	17.2	33.7



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft ³]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

Field Point Grid

Number of points: 289

Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section D-D', Line 2.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.498987 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98358
Consolidation Settlement [ft]	0	1.98358
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	18.5
Total Stress [ksf]	0	24.1716
Total Strain	-0	0.206331
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.018974	24.1698
Over-consolidation Ratio	1	1
Void Ratio	0.380985	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 18.5 ksf
 Depth: 0 ft

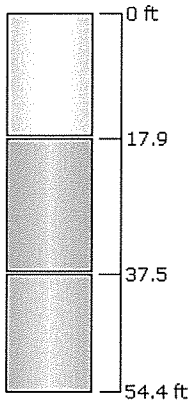
Installation Stage: Stage 1

Coordinates




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	17.9	0
2	Soil Stratum 3	19.6	17.9
3	Soil Stratum 2	16.9	37.5



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft ³]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

Field Point Grid

Number of points: 289

Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section D-D', Line 3.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.576466 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.40443
Consolidation Settlement [ft]	0	1.35522
Immediate Settlement [ft]	0	0.0492125
Loading Stress [ksf]	0	12.7
Total Stress [ksf]	0	18.5709
Total Strain	-0	0.203888
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.014522	18.5692
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 12.7 ksf
 Depth: 0 ft

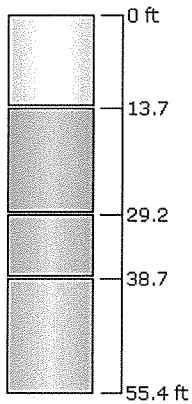
Installation Stage: Stage 1

Coordinates

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	13.7	0
2	Soil Stratum 4	15.5	13.7
3	Soil Stratum 3	9.5	29.2
4	Soil Stratum 2	16.7	38.7



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4
Color				
Unit Weight [kips/ft ³]	0.106	0.103	0.104	0.11
Immediate Settlement	Disabled	Disabled	Disabled	Enabled
Es [ksf]				4000
Esur [ksf]				4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear	
Cc	0.09	0.035	0.045	
Cr	0.1	0.1	0.1	
e0	0.74	0.75	0.71	
OCR	1	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 65

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section D-D', Line 4.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.636294 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.948079
Consolidation Settlement [ft]	0	0.914599
Immediate Settlement [ft]	0	0.03348
Loading Stress [ksf]	0	6.2
Total Stress [ksf]	0	12.7328
Total Strain	-0	0.141231
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.05777	12.7313
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 6.2 ksf
 Depth: 0 ft

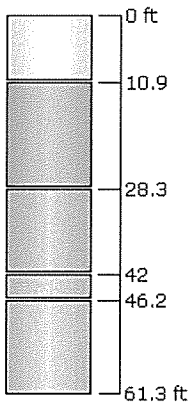
Installation Stage: Stage 1

Coordinates






X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	10.9	0
2	Soil Stratum 4	17.4	10.9
3	Soil Stratum 3	13.7	28.3
4	Soil Stratum 5	4.2	42
5	Soil Stratum 2	15.1	46.2



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft ³]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	4000
Esur [ksf]				4000	4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 71

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

Settle3D Analysis Information

Copper Flat Embankment Post-Construction Settlement

Project Settings

Document Name: Section D-D', Line 5.s3z
 Project Title: Copper Flat Embankment Post-Construction Settlement
 Analysis: 1-D Boussinesq
 Author: David Poe, P.E.
 Company: Golder Associates
 Date Created: 4/25/2013, 2:08:28 PM
 Stress Computation Method: Boussinesq
 Use average properties to calculate layered stresses

Stage Settings

Stage #	Name
1	Stage 1

Results

Time taken to compute: 0.583683 seconds

Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.175852
Consolidation Settlement [ft]	0	0.173272
Immediate Settlement [ft]	0	0.00257999
Loading Stress [ksf]	0	0.4
Total Stress [ksf]	0	7.63261
Total Strain	-0	0.0875971
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.04134	7.61899
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

Loads

1. Rectangular Load

Length: 2000 ft
 Width: 2000 ft
 Rotation angle: 0 degrees
 Load Type: Flexible
 Area of Load: 4e+006 ft²
 Load: 0.4 ksf
 Depth: 0 ft

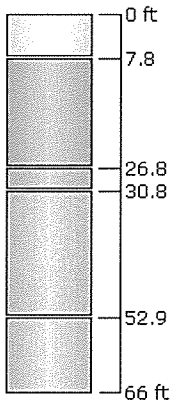
Installation Stage: Stage 1

Coordinates

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

Soil Layers

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	7.8	0
2	Soil Stratum 4	19	7.8
3	Soil Stratum 3	4	26.8
4	Soil Stratum 5	22.1	30.8
5	Soil Stratum 2	13.1	52.9



Soil Properties

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft ³]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	13000
Esur [ksf]				4000	13000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

Query Points

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 63

Field Point Grid

Number of points: 289
Expansion Factor: 2

Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

ATTACHMENT 4
EMBANKMENT ANALYSIS CROSS-SECTIONS WITH GRAPHICAL OUTPUT RESULTS
(SECTIONS B-B' AND D-D' ONLY)

SECTION B-B' PROFILE

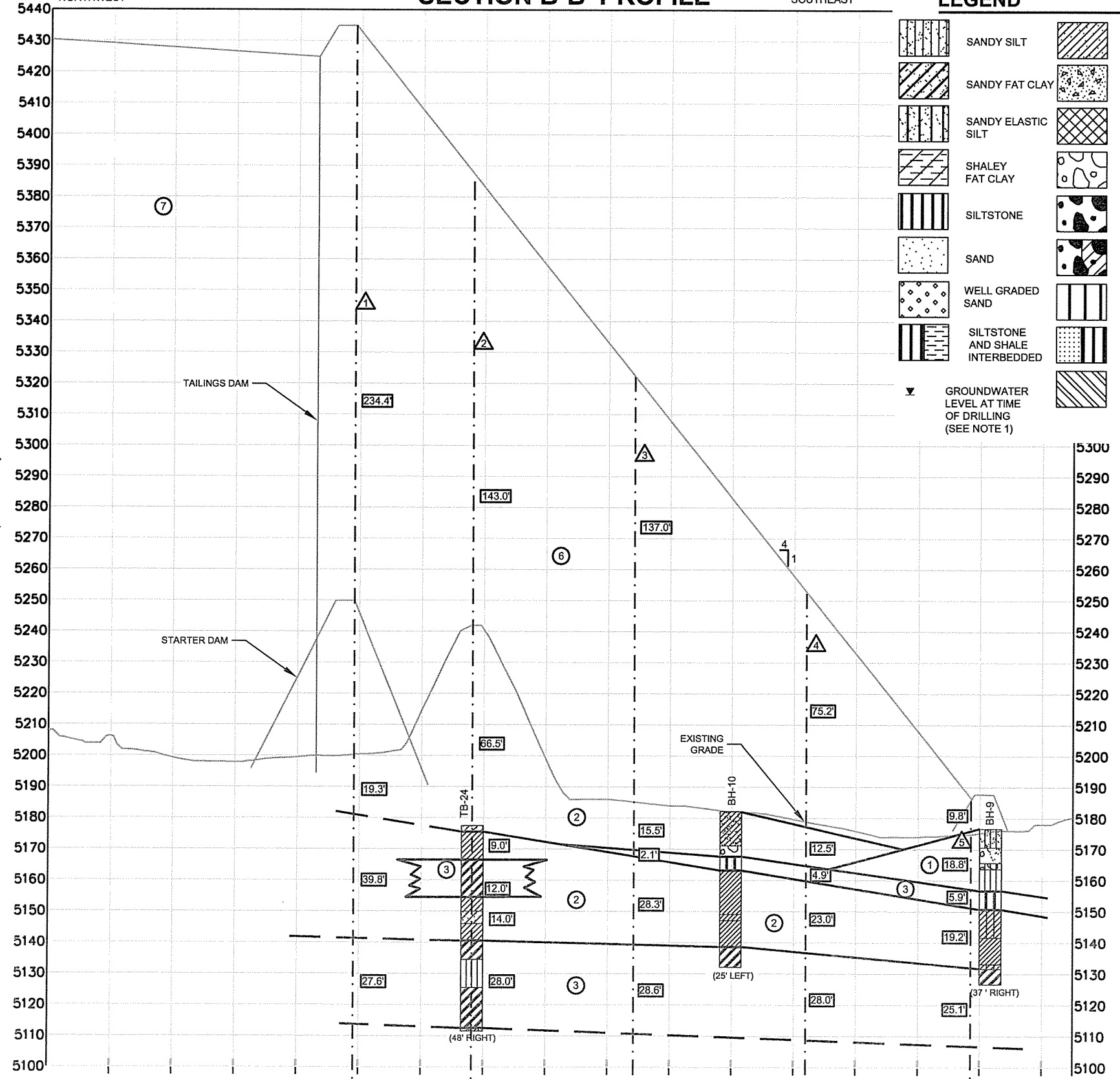
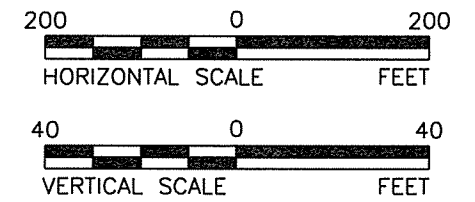
B' NORTHWEST
B' SOUTHEAST

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		USCS CL-CH		POORLY GRADED SAND WITH SILT

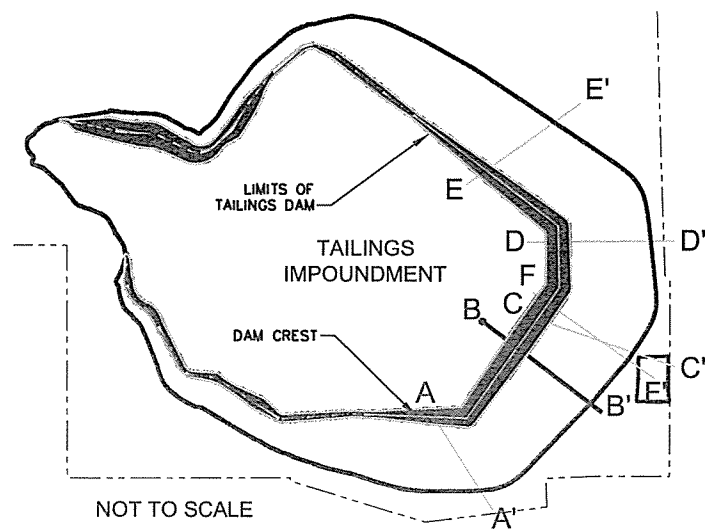
NOTES

- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.



STRATUM DESCRIPTORS

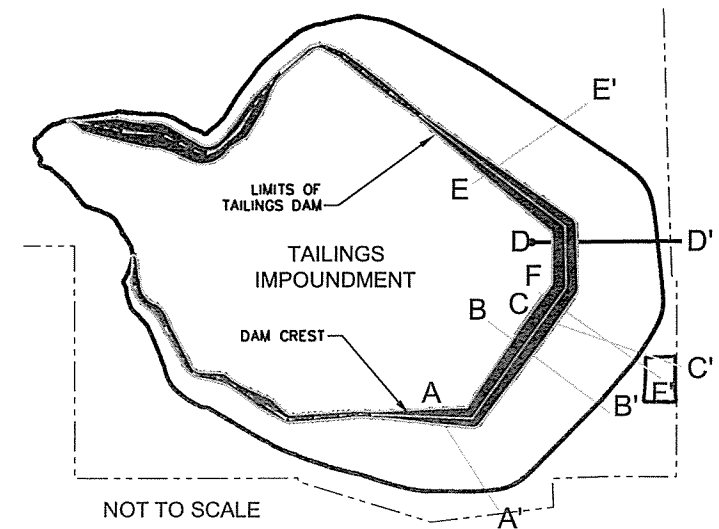
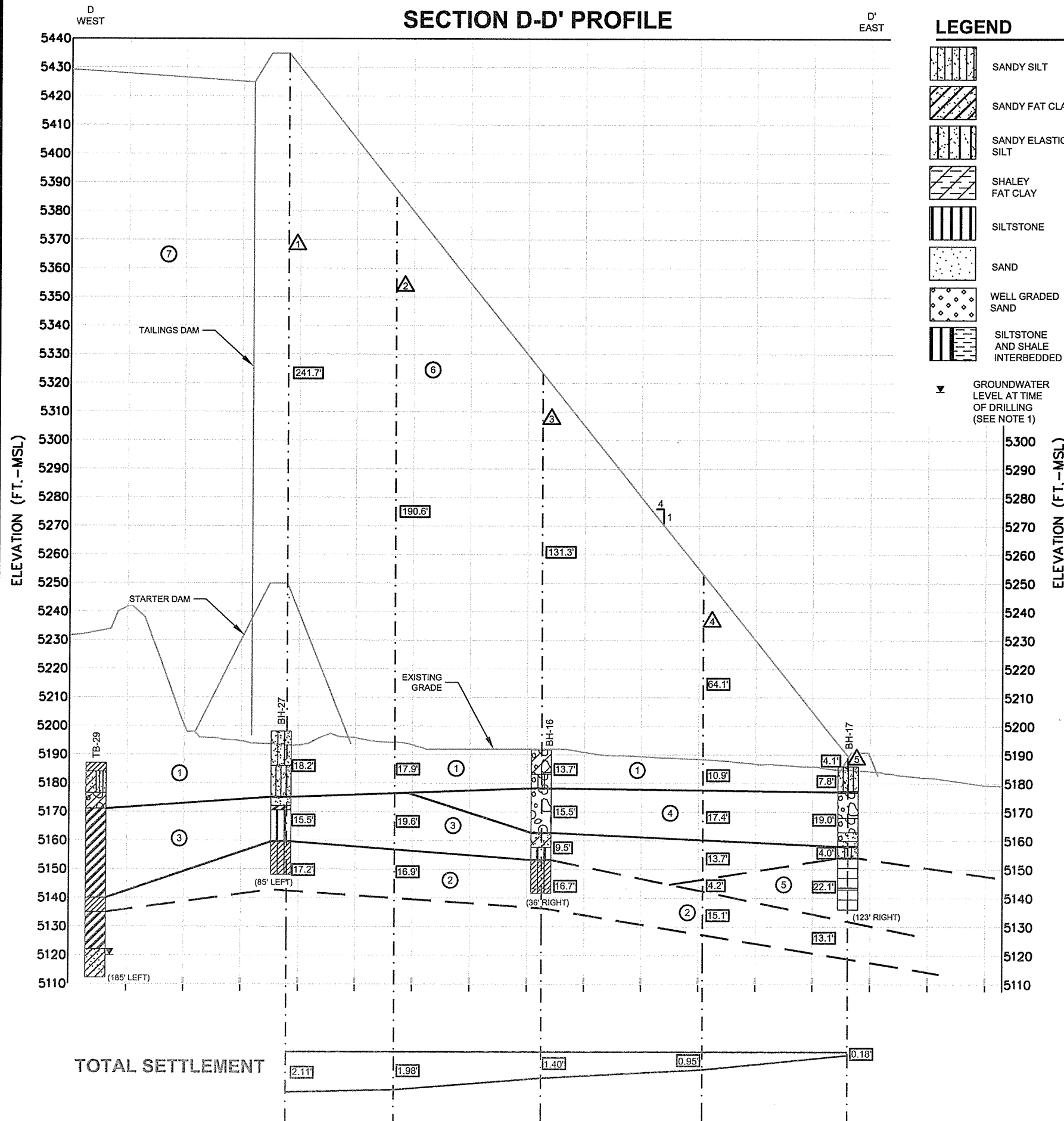
①	ML, CL-ML, SM
②	CL
③	CH, MH
④	GRAVEL, SAND
⑤	BASALT, CALICHE
⑥	EMBANKMENT FILL
⑦	TAILINGS (CONSOLIDATED)
△	ANALYSIS SECTION
12.0'	STRATUM/FILL THICKNESS



DRAFT

PROJECT	COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO				
TITLE	GEOLOGIC CROSS SECTION B-B'				
	PROJECT No.	10392557	FILE No.	10392557A003	
	DESIGN	CMT	2/21/13	SCALE	AS SHOWN
	CADD	CMT	2/21/13	FIGURE	
	CHECK	DEP	2/21/13		
	REVIEW	GM	2/21/13		

SECTION D-D' PROFILE



DRAFT

PROJECT				COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO			
TITLE				GEOLOGIC CROSS SECTION D-D'			
PROJECT No.		10392557		FILE No.		10392557A005	
DESIGN	CMT	2/21/13	SCALE	AS SHOWN			
CADD	CMT	2/21/13	FIGURE	7			
CHECK	DEP	2/21/13					
REVIEW	GM	2/21/13					



APPENDIX I.2
SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS

SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS

Made By: JL
 Checked by: GM
 Reviewed by: MP

1.0 OBJECTIVE

Estimate the tensile strain caused by differential settlement of the in-situ subsurface materials inferred below the proposed Copper Flat tailing facility.

2.0 METHODOLOGY

The proposed geomembrane liner system may experience tensile strain because of differential settlement caused from the loading (tailings and embankment) of the subsurface soils.

2.1 Settlement Analysis

Settlement was calculated using the finite element software SigmaW from the 2012 GeoStudio package. Cross sections A and B (both shown in plan view on Figure 1 and in profile view in Figure 2) showing the proposed tailing facility and tailings embankment layout/dimensions, inferred subsurface soils and boundaries were imported into the software for analyses. Geotechnical properties for each subsurface material layer were selected from previous reports (Refs. 1 and 2) and from experience with similar soils. The geotechnical properties were incorporated into the software and used for the settlement analyses. Table 1 below provides a list of the geotechnical subsurface material layers and properties.

Table 1: Geotechnical Subsurface Material Layers and Properties

Material No.	Material Name	Geotechnical Properties		
		Unit Weight (lb/ft ³)	Poisson's Ratio (-)	Effective Modulus (lb/ft ²)
0	Tailings/Embankment	97	0.49	10,000,000
1	Well-Graded Gravel	110	0.30	4,000,000
2	Well-Graded Sand with Silt and Gravel	110	0.30	4,000,000
3	Conglomerate	130	0.30	5,000,000
4	Basalt	160	0.30	100,000,000
5	Lean Clay, Fat Clay, Silty Clay	104	0.30	790,600
6	Silt	106	0.30	671,400
7	Caliche	125	0.30	100,000,000
8	Bedrock	175	0.30	100,000,000

2.1 Tensile Strain from Differential Settlement

Settlement results from the SigmaW runs were used to calculate the induced strain in the geomembrane liner system along Cross Section A and B shown in Figure 1 and Figure 2.

The tensile strain of a base liner system caused by differential settlement can be estimated by the following equation:

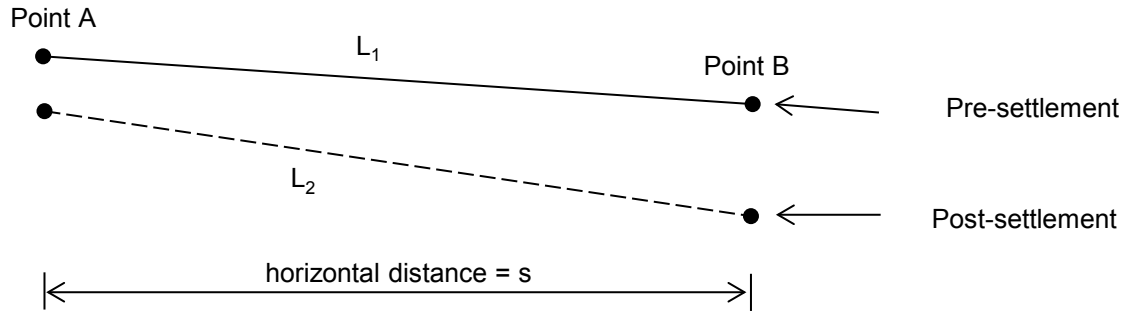


Illustration: Liner Differential Settlement

$$\epsilon = \frac{L_2 - L_1}{L_1}$$

$$L = \sqrt{(Elev.A - Elev.B)^2 + s^2}$$

- Where:
- ϵ = Tensile strain in liner system between Points A and B
 - L_1 = Distance between Points A and B, pre-settlement
 - L_2 = Distance between Points A and B, post-settlement
 - s = Horizontal distance between Points A and B

3.0 CALCULATIONS AND RESULTS

3.1 Tensile Strain from Differential Settlement

The settlement results for Cross Section A and Cross Section B are illustrated below. Points for liner strain evaluation were selected at locations where peaks or valleys were observed in the results. The liner strain evaluations due to differential settlement of the subsurface materials are summarized in Table 2 and Table 3.

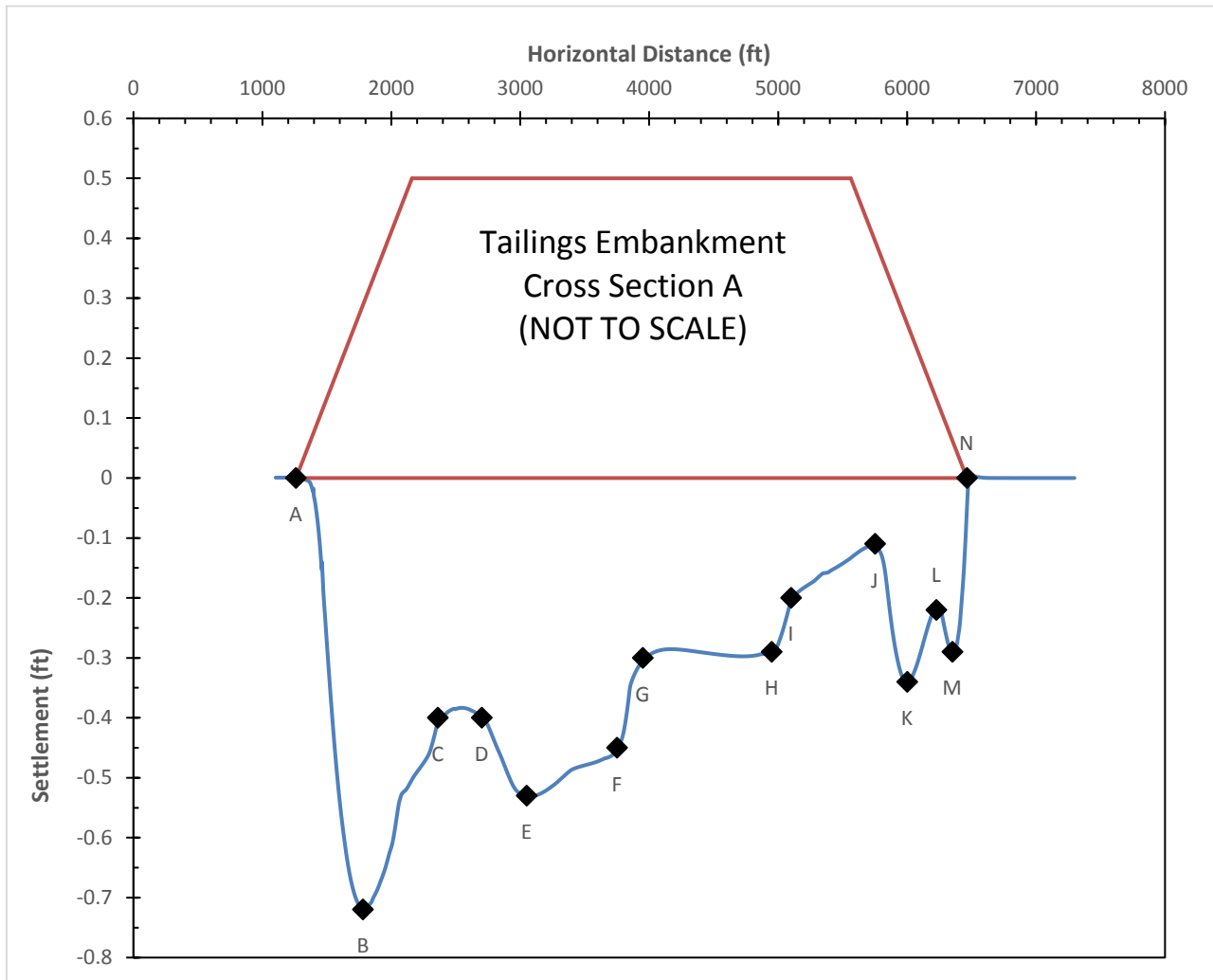


Illustration: Settlement Profile - Cross Section A
(refer to Figure 2 for location along horizontal distance)

Table 2: Liner Integrity Analysis Results - Cross Section A

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L ₁), feet	Post-settlement Dist. (L ₂), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5383.4	5383.4	0.00	520.00	524.920	525.019	0.0188%
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
C	5275.5	5275.1	0.40	350.00	350.352	350.358	0.0017%
D	5267.5	5267.1	0.40	350.00	350.352	350.358	0.0017%
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
G	5230.2	5229.9	0.30	150.00	155.850	155.826	Under Compression
H	5239.6	5239.3	0.29	150.00	155.850	155.826	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
N	5168.5	5168.5	0.00				
Maximum Tensile Strain due to Differential Settlement =							0.0188%

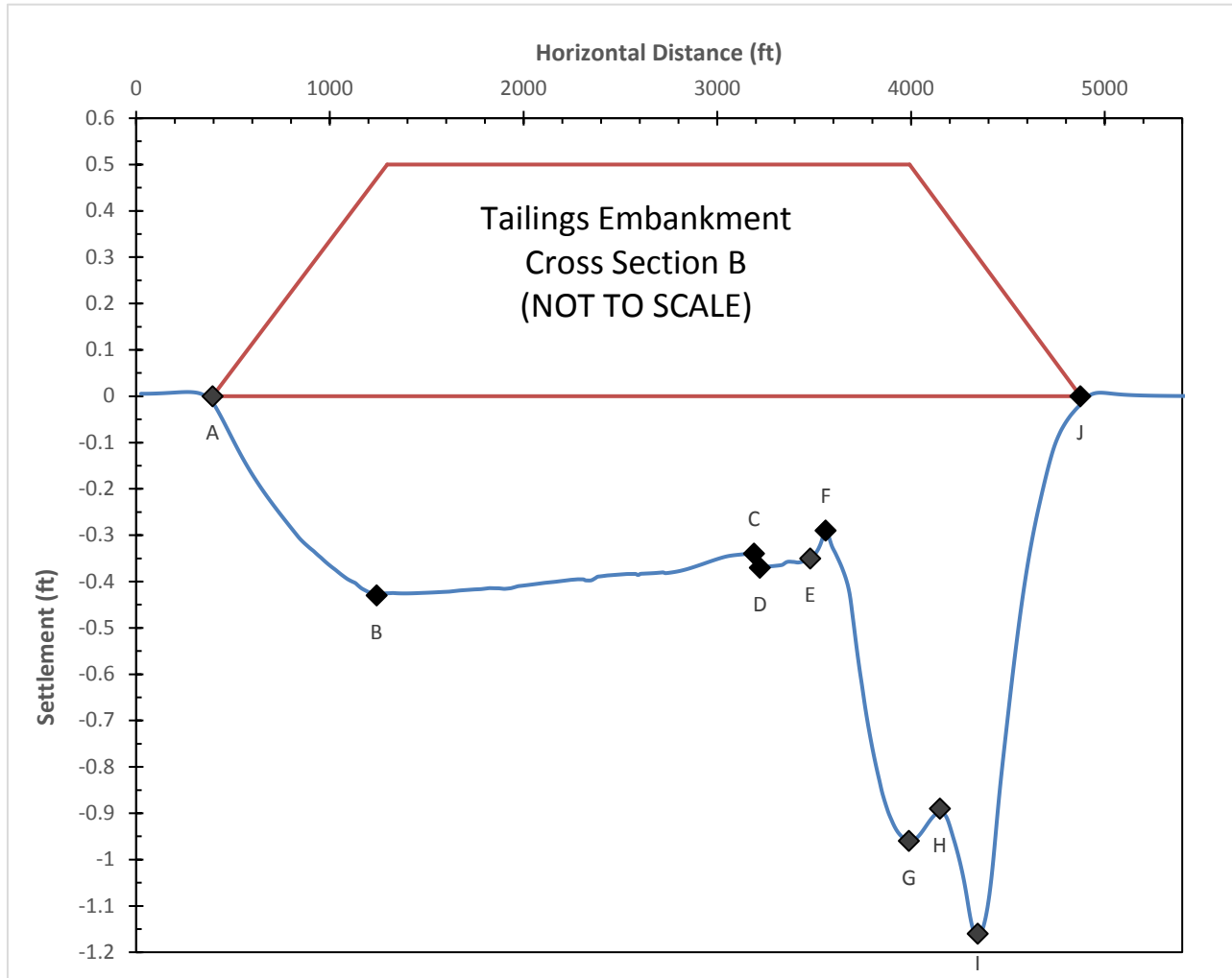


Illustration: Settlement Profile - Cross Section B
(refer to Figure 2 for location along horizontal distance)

Table 3: Liner Integrity Analysis Results - Cross Section B

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L ₁), feet	Post-settlement Dist. (L ₂), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5280.2	5280.2	0.00	847.00	848.707	848.734	0.0032%
B	5226.4	5226.0	0.43				
C	5213.7	5213.4	0.34	30.00	34.000	33.986	Under Compression
D	5229.7	5229.3	0.37				
E	5239.3	5239.0	0.35	79.00	83.716	83.696	Under Compression
F	5211.6	5211.3	0.29				
F	52116.0	52115.7	0.29	430.00	46921.770	46922.440	0.0014%
G	5196.2	5195.2	0.96				
G	5196.2	5195.2	0.96	160.00	160.004	160.004	0.0003%
H	5197.3	5196.4	0.89				
H	5197.3	5196.4	0.89	195.00	195.004	195.003	Under Compression
I	5198.6	5197.4	1.16				
I	5198.6	5197.4	1.16	530.00	530.000	530.003	0.0005%
J	5199.3	5199.3	0.00				
Maximum Tensile Strain due to Differential Settlement =							0.0032%

4.0 DISCUSSION AND CONCLUSIONS

It is understood that the liner system will consist of HDPE 80 mil geomembrane liner between a liner bedding fill layer and tailings. The minimum allowable tensile strain for geomembrane is 10% (Refs. 3). Based on the analysis performed herein and available information at the time of this calculation, the estimated tensile strain along Cross Section A and Cross Section B are less than the allowable tensile strain. The allowable strain is presented in Table 4.

Table 4: Summary of Allowable Liner Strains

Cross Section	Max. Tensile Strain from Differential Settlement	Liner Component	Allowable Tensile Strain	Tensile Strain less than Allowable?
A	0.0188%	Geomembrane	10%	Yes
B	0.0032%			Yes

The potential strain of the geomembrane liner system was analyzed for overall differential settlement along two cross sections (Cross Section A and B) within the proposed Copper Flat tailing facility. Based on the available information, experience with similar subsurface materials and conservative assumptions, the maximum liner strain is estimated to be 0.02%, from differential settlement which is less than the allowable strain for geomembrane liners.

5.0 REFERENCES

1. - Golder 2013, Feasibility Study Copper Flat Project, Sierra County, New Mexico, Volume 1 - Tailings Storage Facility, report dated July 2013, Golder Project No. 103-92557.011
2. - SHB (Sergent, Hauskins and Beckwith), 1980. Tailings Dam and Disposal Area - Quintana Minerals Corporation - Copper Flats Project - Golddust, New Mexico. October 14, 1980
3. - Robert M. Koerner (2005) Designing with Geosynthetics, Fifth Edition, Pearson/Prentice Hall.

GeoStudio File Names

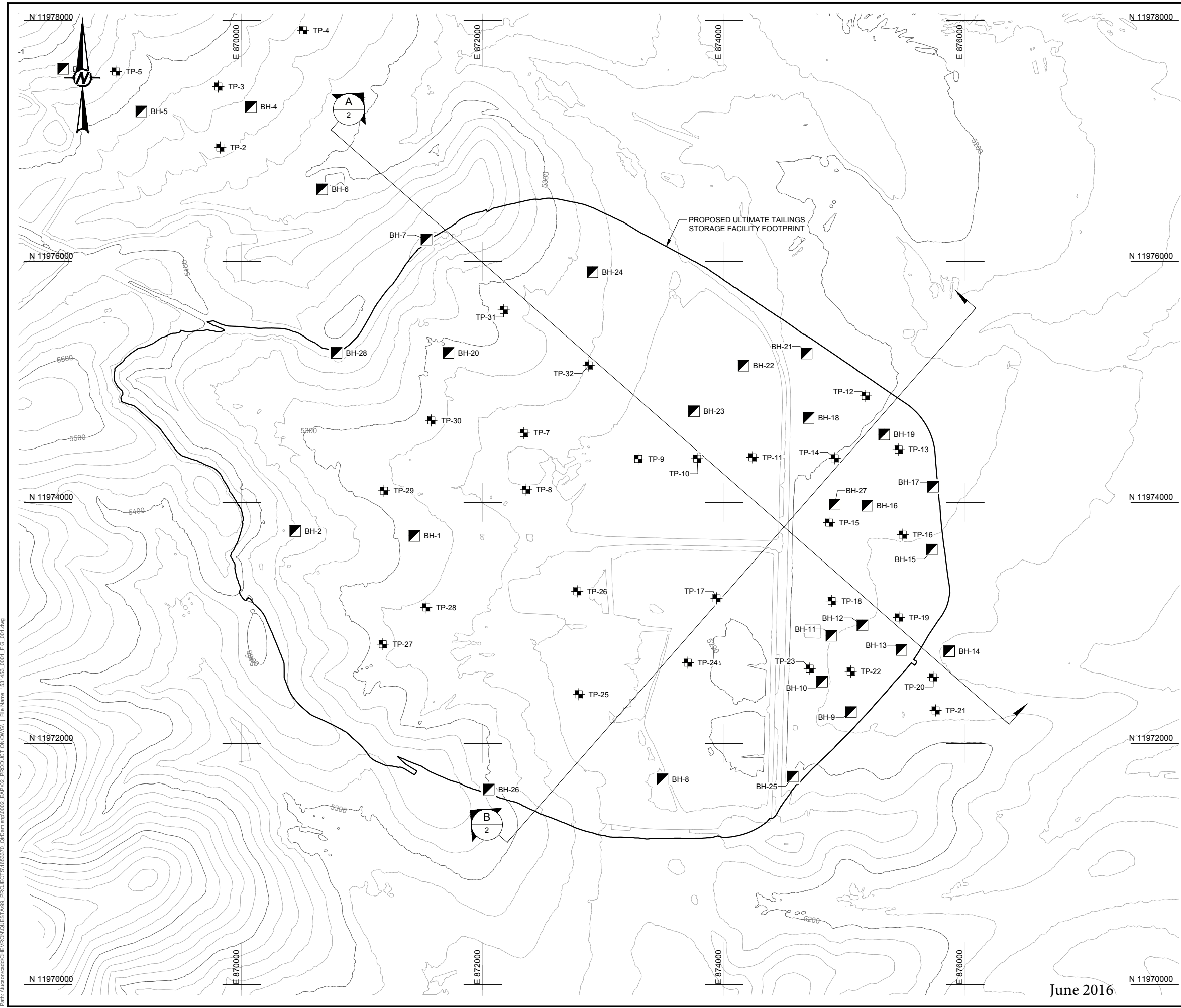
Full Cross Section A.gsz
Full Cross Section B.gsz

Description

Cross Section A Settlement Analysis
Cross Section B Settlement Analysis

Attachments

Figure 1: Cross Section Location Plan
Figure 2: Geologic Cross Sections



LEGEND

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- BH-11 HOLLOW STEM AUGER (HSA) BOREHOLE
- TP-8 TEST PIT
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



CLIENT
THEMAC RESOURCES | NEW MEXICO COPPER CORPORATION
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PROJECT
COPPER FLAT

SIERRA COUNTY, NEW MEXICO

TITLE
CROSS-SECTION LOCATION PLAN

CONSULTANT	YYYY-MM-DD	2016-05-11
	DESIGNED	JS
	PREPARED	JHR
	REVIEWED	JL
	APPROVED	GM

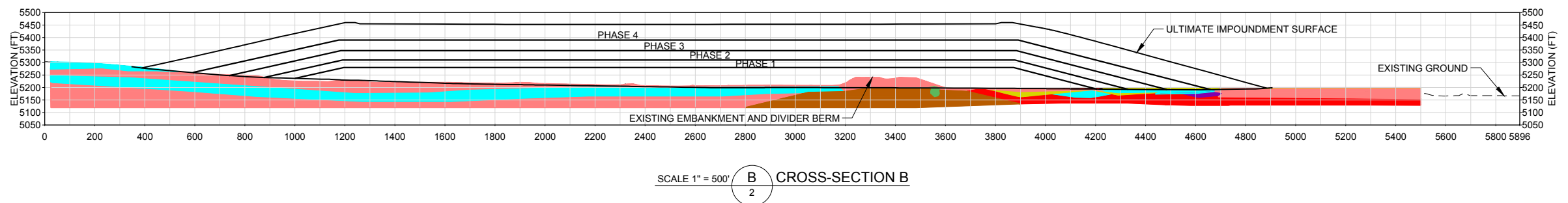
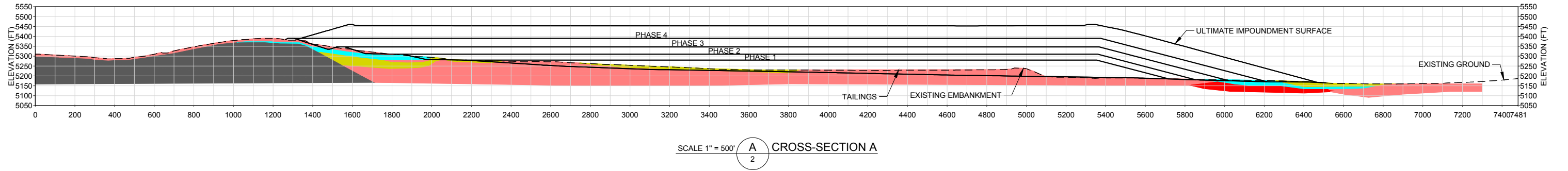
PROJECT NO. 1531453 CONTROL 0001 REV. A FIGURE 1

June 2016

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LEGEND

	WELL-GRADED GRAVEL
	WELL-GRADED SAND WITH SILT AND GRAVEL
	CONGLOMERATE
	BASALT
	LEAN CLAY, FAT CLAY, SILTY CLAY
	SILT
	CALICHE
	BEDROCK

CLIENT	
THEMAC RESOURCES	NEW MEXICO COPPER CORPORATION
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PROJECT COPPER FLAT	
SIERRA COUNTY, NEW MEXICO	
TITLE GEOLOGIC CROSS-SECTIONS	
CONSULTANT	YYYY-MM-DD 2016-05-11
	DESIGNED JS
	PREPARED JHR
	REVIEWED JL
	APPROVED GM



June 2016

PROJECT NO. 1531453	CONTROL 0001	REV. A	FIGURE 2
------------------------	-----------------	-----------	-------------

1 in. IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B

**APPENDIX J
DRAWINGS**



**NEW MEXICO
COPPER
CORPORATION**

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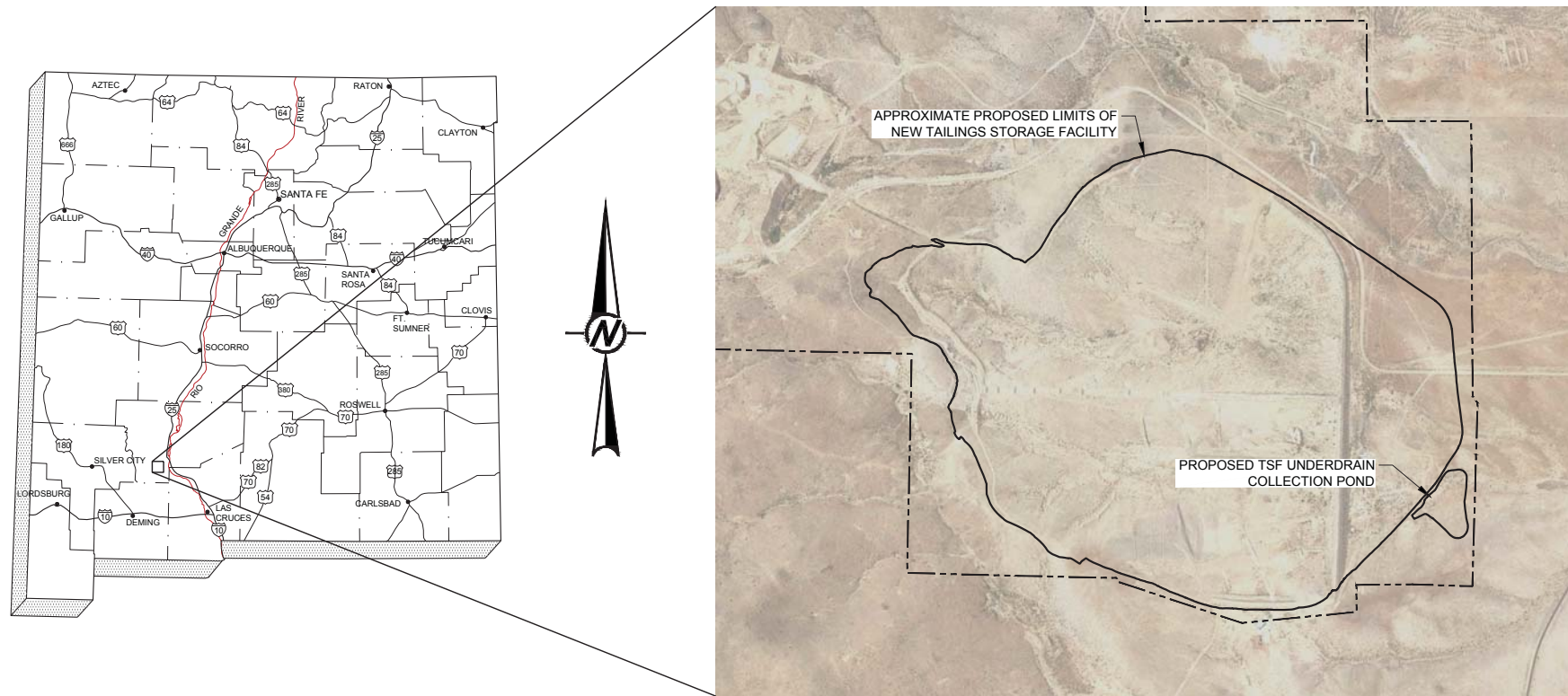
COPPER FLAT PROJECT

30K TPD TAILINGS STORAGE FACILITY

FEASIBILITY DESIGN

SIERRA COUNTY, NEW MEXICO

NOVEMBER 2015



STATE OF NEW MEXICO
NOT TO SCALE

LOCATION MAP
NOT TO SCALE

DRAWING LIST

No.	DRAWING TITLE
1	TITLE SHEET AND LOCATION MAP
2	EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN
3	GEOTECHNICAL TEST AND GEOLOGIC CROSS-SECTION LOCATIONS
4	GEOLOGIC CROSS-SECTION A-A'
5	GEOLOGIC CROSS-SECTION B-B'
6	GEOLOGIC CROSS-SECTION C-C'
7	GEOLOGIC CROSS-SECTION D-D'
8	GEOLOGIC CROSS-SECTION E-E'
9	GEOLOGIC CROSS-SECTION F-F'
10	TAILINGS STORAGE FACILITY GRADING PLAN
11	TAILINGS STORAGE FACILITY PHASE 1 PLAN
12	TAILINGS STORAGE FACILITY AT FINAL BUILDOUT
13	TAILINGS STORAGE FACILITY CROSS-SECTIONS
14	TAILINGS STORAGE FACILITY DETAILS
15	DAM AND IMPOUNDMENT UNDERDRAIN PLAN
16	DAM UNDERDRAIN DETAILS (1 OF 2)
17	DAM UNDERDRAIN DETAILS (2 OF 2)
18	TSF UNDERDRAIN COLLECTION POND PLAN
19	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)
20	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(2 OF 2)
21	CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN
22	SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS
23	GENERAL PROCESS FLOW DIAGRAM
24	TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN
25	TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)
26	TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2)
27	WATER RECLAIM SYSTEM PIPING PLAN
28	WATER RECLAIM SYSTEM DETAILS
29	TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS

GENERAL REFERENCES

- 2-FOOT TOPOGRAPHY DEVELOPED BY COOPER AERIAL SURVEY CO. BASED ON JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMATIC RESOURCES.
- COORDINATE SYSTEM IS IN UTM ZONE 13 NAD83 U.S. FOOT.

NOTES

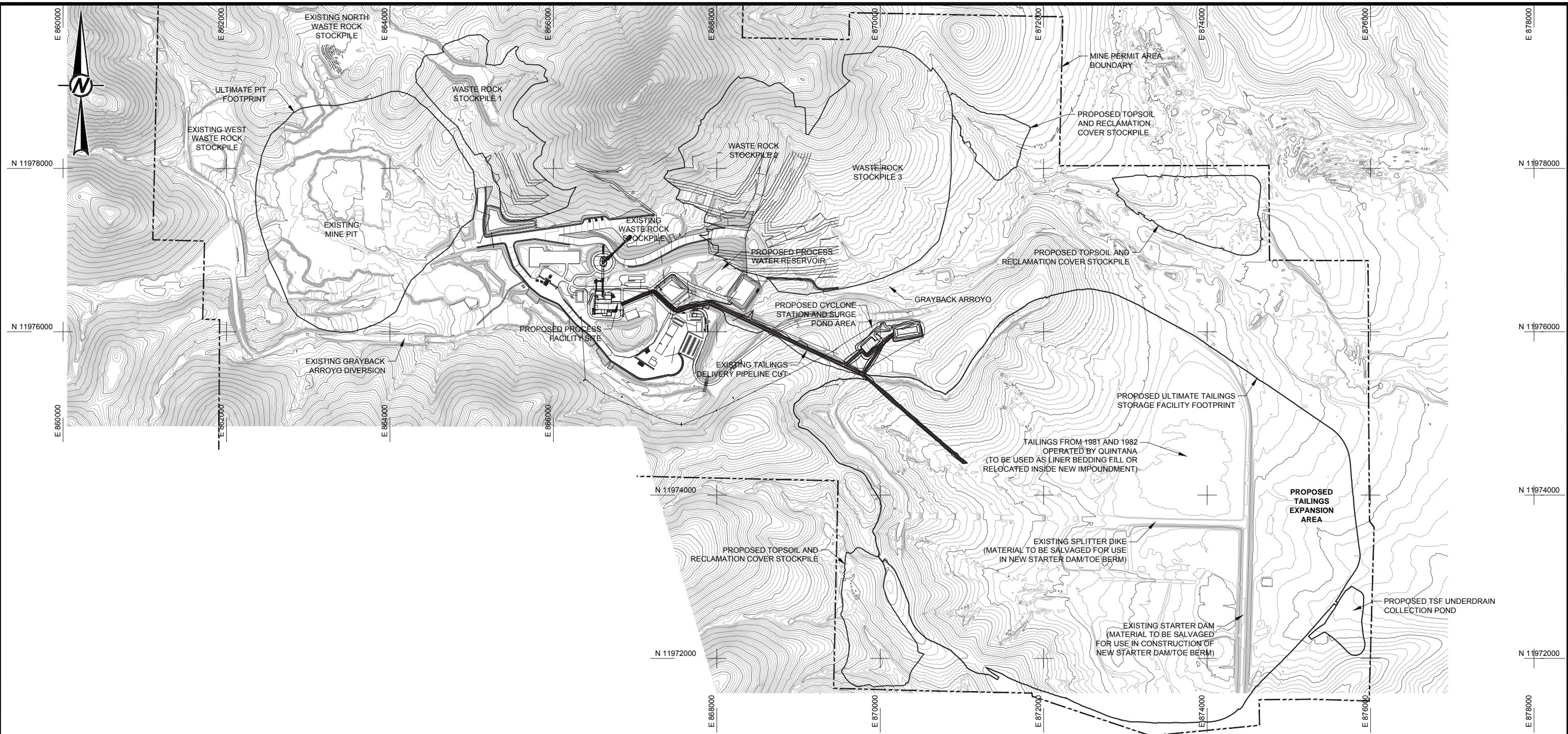
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2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

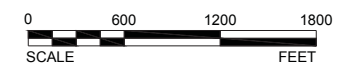
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DESIGN	DW 2013-04-08	SCALE	NOT TO SCALE
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CHECK	GM 2013-07-16		
REVIEW	DAK 2013-07-17		

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- NOTES**
- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
 - TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
 FOR AGENCY REVIEW

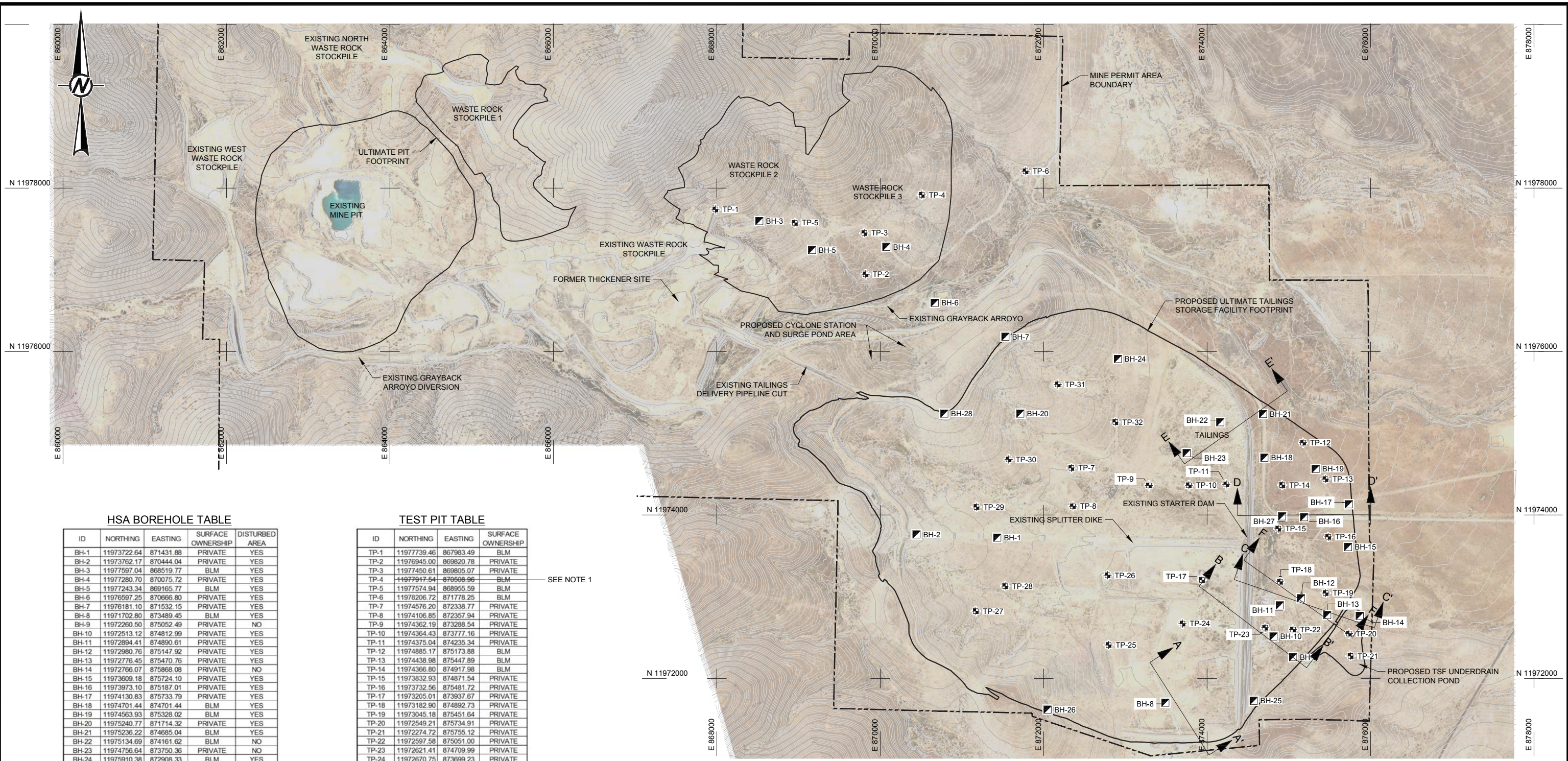
PROJECT COPPER FLAT PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
 RESOURCES
 Environmentally Responsible. Community Minded. Local Opportunities.
30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN
 SIERRA COUNTY, NEW MEXICO

EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN

PROJECT No.	103-92557	FILE No.	10392557K002
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	2
REVIEW	DAK	2013-07-17	



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HSA BOREHOLE TABLE

ID	NORTHING	EASTING	SURFACE OWNERSHIP	DISTURBED AREA
BH-1	11973722.64	871431.88	PRIVATE	YES
BH-2	11973762.17	870444.04	PRIVATE	YES
BH-3	11977597.04	868519.77	BLM	YES
BH-4	11977280.70	870075.72	PRIVATE	YES
BH-5	11977243.34	869165.77	BLM	YES
BH-6	11976597.25	870666.80	PRIVATE	YES
BH-7	11976181.10	871532.15	PRIVATE	YES
BH-8	11971702.80	873489.45	BLM	YES
BH-9	11972260.50	875052.49	PRIVATE	NO
BH-10	11972513.12	874812.99	PRIVATE	YES
BH-11	11972894.41	874890.61	PRIVATE	YES
BH-12	11972980.76	875147.92	PRIVATE	YES
BH-13	11972776.45	875470.76	PRIVATE	YES
BH-14	11972766.07	875868.08	PRIVATE	YES
BH-15	11973609.18	875724.10	PRIVATE	NO
BH-16	11973973.10	875187.01	PRIVATE	YES
BH-17	11974130.83	875733.79	PRIVATE	YES
BH-18	11974701.44	874701.44	BLM	YES
BH-19	11974563.93	875328.02	BLM	YES
BH-20	11975240.77	871714.32	PRIVATE	YES
BH-21	11975236.22	874685.04	BLM	YES
BH-22	11975134.69	874161.62	BLM	NO
BH-23	11974756.64	873750.36	PRIVATE	NO
BH-24	11975910.38	872908.33	BLM	YES
BH-25	11971725.72	874570.21	BLM	YES
BH-26	11971618.08	872048.45	BLM	YES
BH-27	11973982.94	874917.98	PRIVATE	YES
BH-28	11975241.04	870785.44	PRIVATE	YES

TEST PIT TABLE

ID	NORTHING	EASTING	SURFACE OWNERSHIP
TP-1	11977739.46	867983.49	BLM
TP-2	11976945.00	869820.78	PRIVATE
TP-3	11977450.61	869805.07	PRIVATE
TP-4	11977917.64	870568.96	BLM
TP-5	11977574.94	868955.59	BLM
TP-6	11978206.72	871778.25	BLM
TP-7	11974576.20	872338.77	PRIVATE
TP-8	11974106.85	872357.94	PRIVATE
TP-9	11974362.19	873288.54	PRIVATE
TP-10	11974364.43	873777.16	PRIVATE
TP-11	11974375.04	874235.34	PRIVATE
TP-12	11974885.17	875173.88	BLM
TP-13	11974438.98	875447.89	BLM
TP-14	11974366.80	874917.98	BLM
TP-15	11973832.93	874871.54	PRIVATE
TP-16	11973732.56	875481.72	PRIVATE
TP-17	11973205.01	873937.67	PRIVATE
TP-18	11973182.90	874892.73	PRIVATE
TP-19	11973045.18	875451.64	PRIVATE
TP-20	11972549.21	875734.91	PRIVATE
TP-21	11972274.72	875755.12	PRIVATE
TP-22	11972597.58	875051.00	PRIVATE
TP-23	11972621.41	874709.99	PRIVATE
TP-24	11972670.75	873699.23	PRIVATE
TP-25	11972408.78	872794.31	PRIVATE
TP-26	11973262.64	872782.60	PRIVATE
TP-27	11972823.18	871169.18	PRIVATE
TP-28	11973129.48	871528.89	PRIVATE
TP-29	11974098.42	871178.07	PRIVATE
TP-30	11974680.06	871571.56	PRIVATE
TP-31	11975597.72	872172.62	BLM
TP-32	11975136.61	872876.80	BLM

SEE NOTE 1

LEGEND

- MINE PERMIT AREA BOUNDARY
- BH-1 HOLLOW STEM AUGER (HSA) BOREHOLE
- ⊕ TP-29 TEST PIT

NOTES

- TP-4 TEST PIT NOT EXCAVATED DUE TO LACK OF ACCESS.

REFERENCES

- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

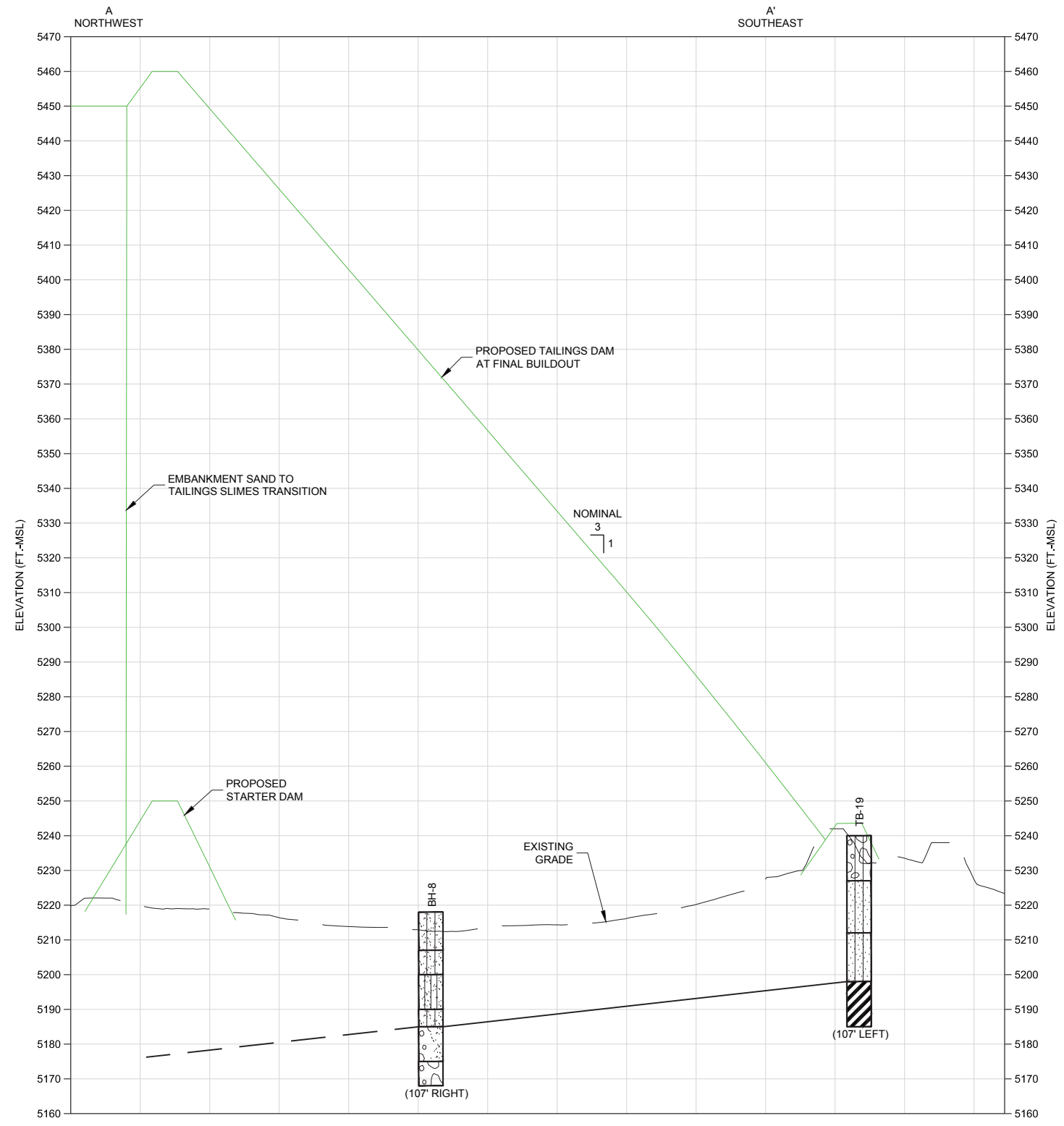
PROJECT No. 103-92557 FILE No. 10392557K003
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE	PROJECT No.	FILE No.	SCALE
GEOTECHNICAL TEST AND GEOLOGIC CROSS-SECTION LOCATIONS	DESIGN	DW	2013-04-08
	CADD	JHR	2013-07-10
	CHECK	GM	2013-07-16
	REVIEW	DAK	2013-07-17

3



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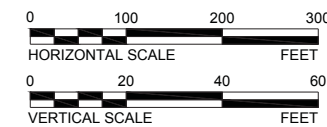


NOTES

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

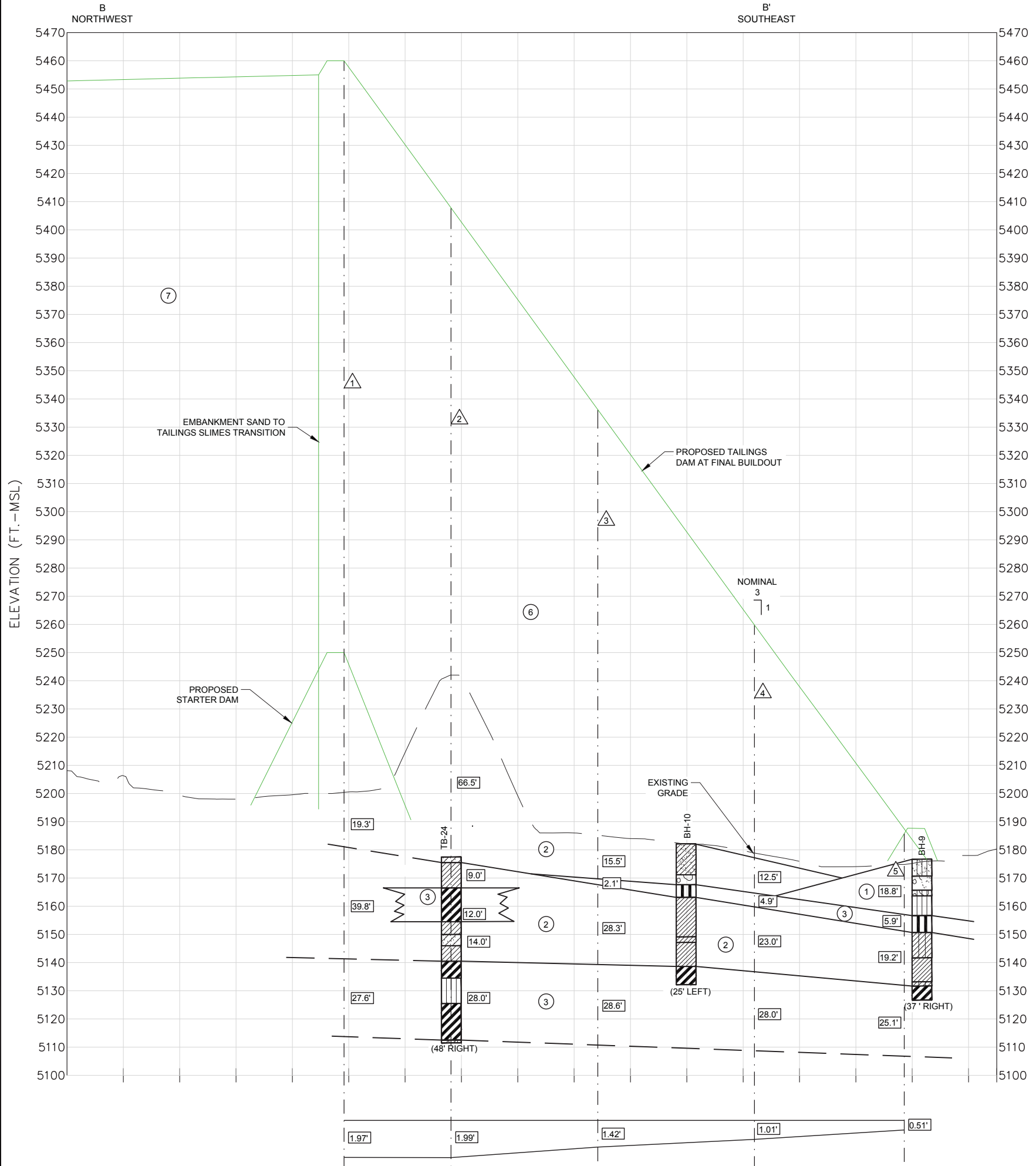
	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
2013-11-15		ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE PRELIMINARY FOR AGENCY REVIEW	PROJECT THEMAC RESOURCES NEW MEXICO COPPER CORPORATION Environmentally Responsible. Community Minded. Local Opportunities.				COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO			
	TITLE GEOLOGIC CROSS-SECTION A-A'							
PROJECT No. 103-92557		FILE No. 10392557K004		DESIGN DW 2013-04-08		SCALE AS SHOWN		
CADD JHR 2013-07-10		DRAWING		CHECK GM 2013-07-16		4		
REVIEW DAK 2013-07-17								

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STRATUM DESCRIPTORS

- ① ML, CL-ML, SM
- ② CL
- ③ CH, MH
- ④ GRAVEL, SAND
- ⑤ BASALT, CALICHE
- ⑥ EMBANKMENT FILL
- ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

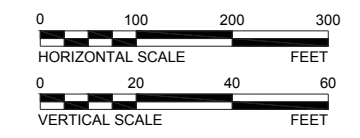
NOTES

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
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3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)
 SETTLEMENT SECTION LINE AND IDENTIFIER



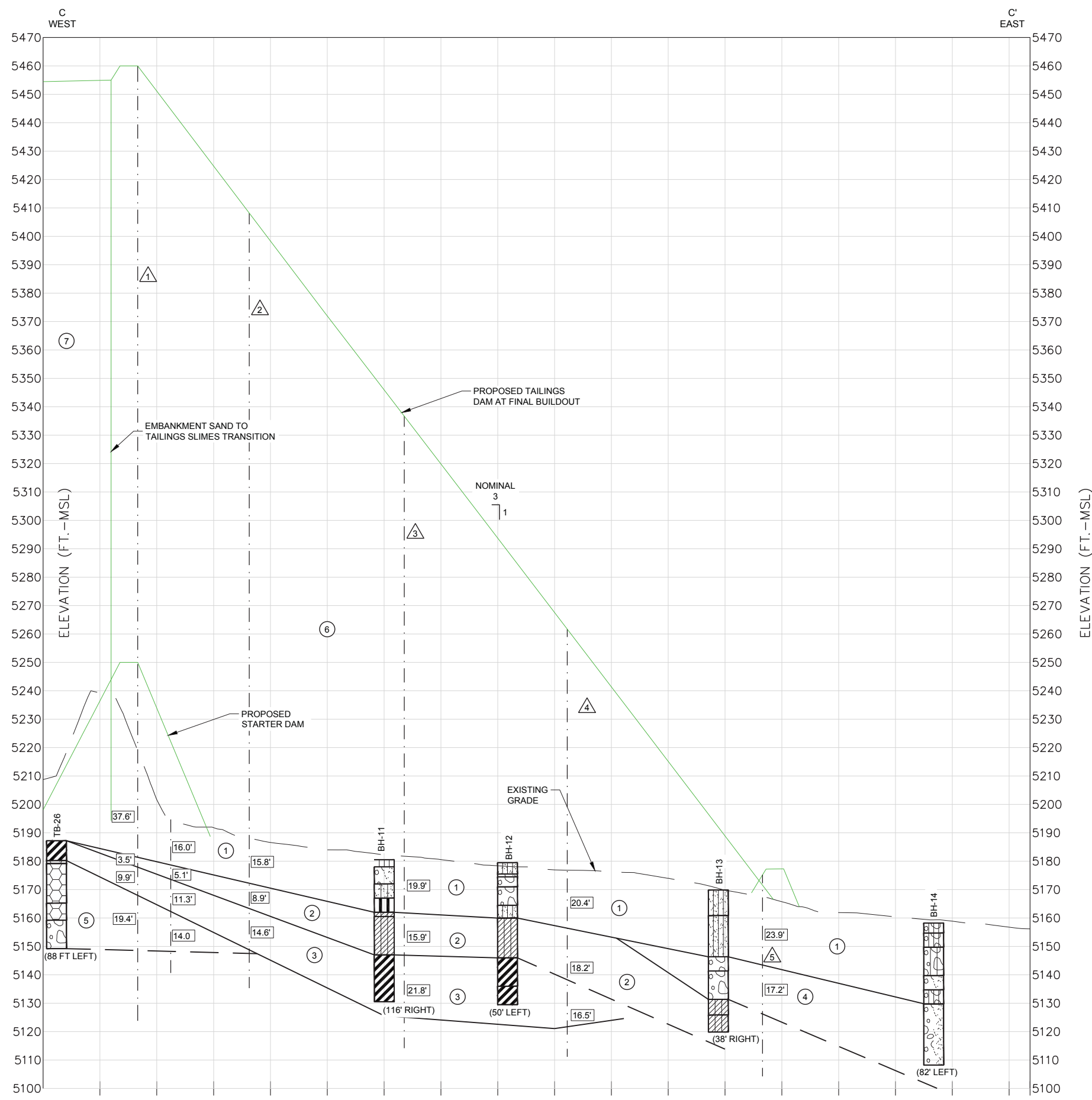
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PRELIMINARY
FOR AGENCY REVIEW

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
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2	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
3	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
4	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
5	2013-05-03	ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

PROJECT: **COPPER FLAT PROJECT**
THEMAC RESOURCES
 Environmentally Responsible. Community Minded. Local Opportunities.
 NEW MEXICO COPPER CORPORATION
 30K TPD TAILINGS STORAGE FACILITY
 FEASIBILITY DESIGN
 SIERRA COUNTY, NEW MEXICO

GEOLOGIC CROSS-SECTION B-B'					
PROJECT No.	103-92557	FILE No.	10392557K004		
DESIGN	DW	2013-04-08	SCALE	AS SHOWN	
CADD	JHR	2013-07-10	DRAWING	5	
CHECK	GM	2013-07-16			
REVIEW	DAK	2013-07-17			

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- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
 - ② CL
 - ③ CH, MH
 - ④ GRAVEL, SAND
 - ⑤ BASALT, CALICHE
 - ⑥ EMBANKMENT FILL
 - ⑦ TAILINGS (CONSOLIDATED)
 - 12.0' STRATUM THICKNESS

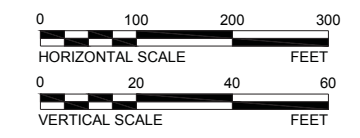
- NOTES**
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 2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
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 4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

▼ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

△ SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

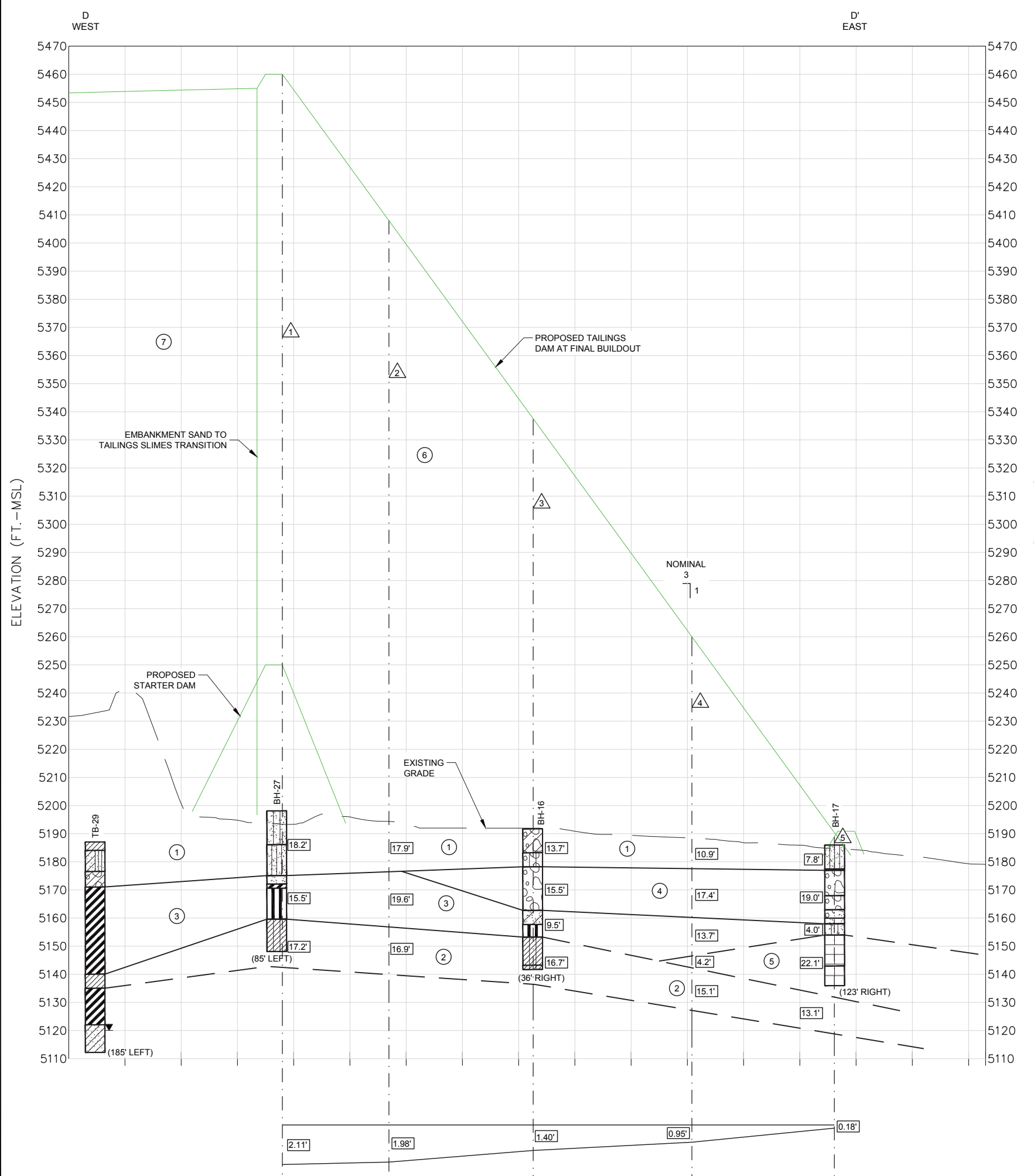
PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
GEOLOGIC CROSS-SECTION C-C'

PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	6

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- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
 - ② CL
 - ③ CH, MH
 - ④ GRAVEL, SAND
 - ⑤ BASALT, CALICHE
 - ⑥ EMBANKMENT FILL
 - ⑦ TAILINGS (CONSOLIDATED)
 - 12.0' STRATUM THICKNESS

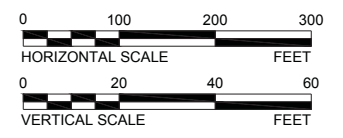
- NOTES**
1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
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 4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

▼ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

△ SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

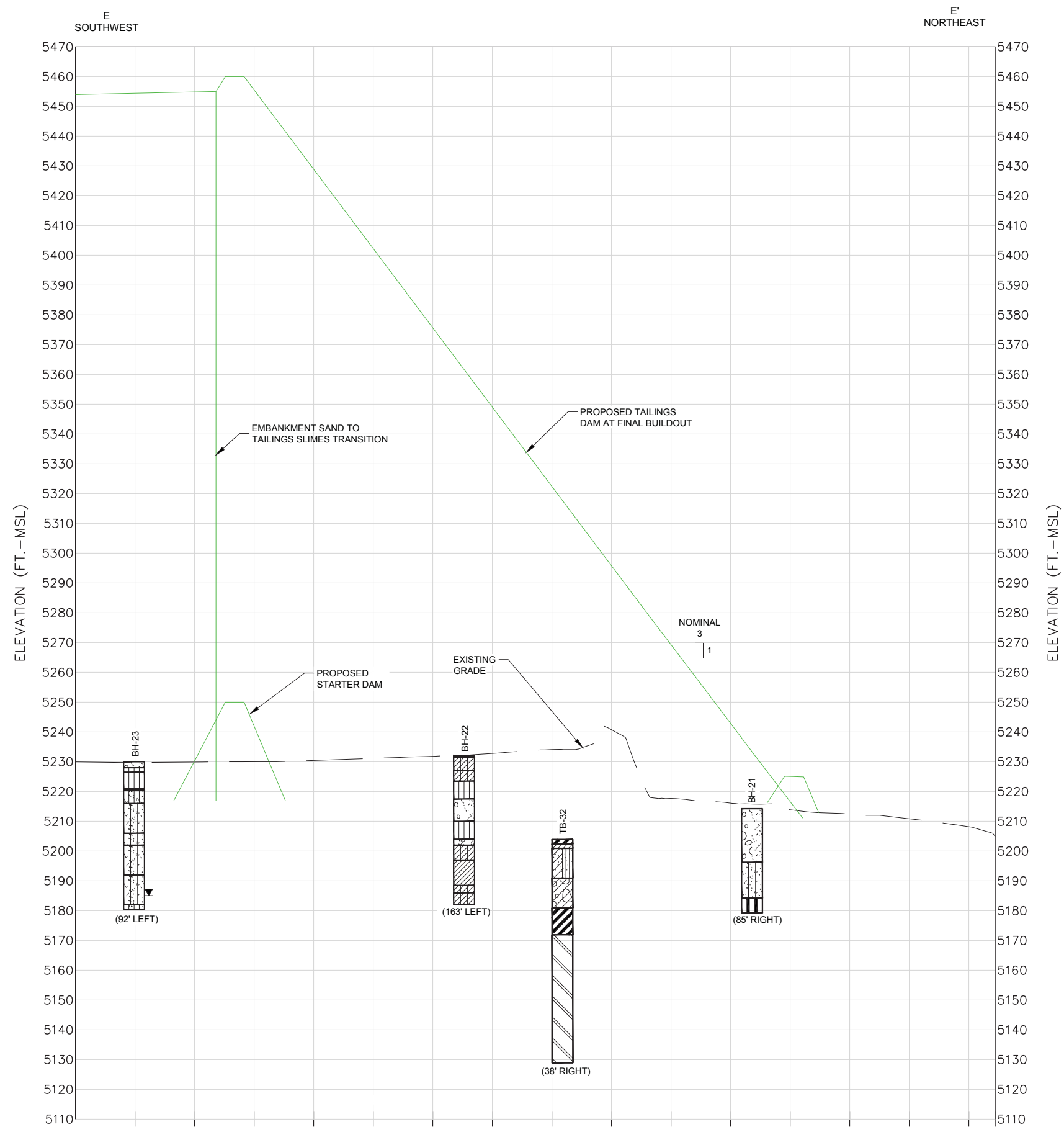
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30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
GEOLOGIC CROSS-SECTION D-D'

DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

7

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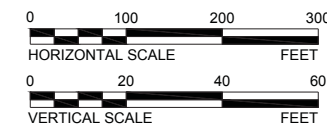


NOTES

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4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



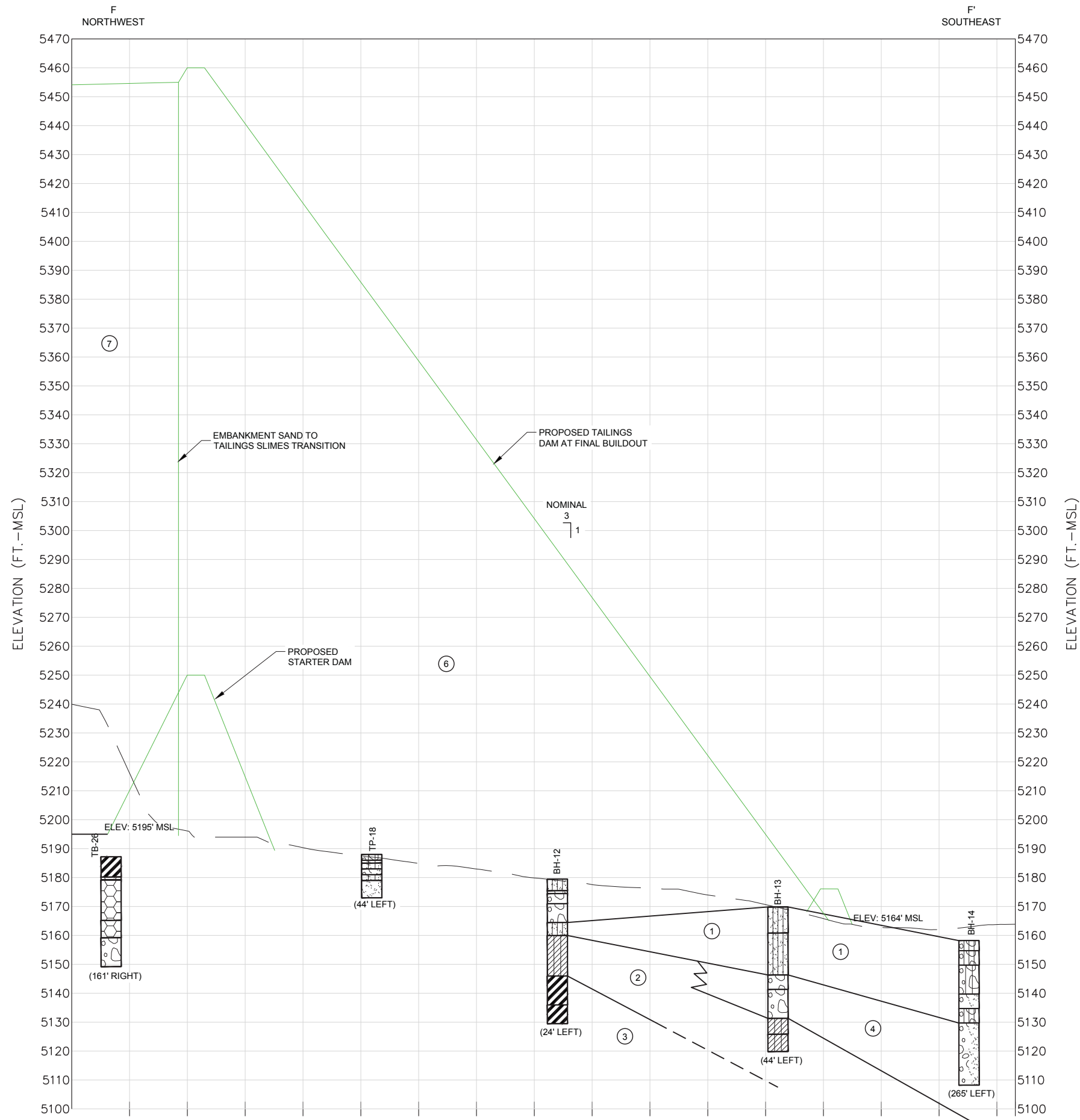
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2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT: **COPPER FLAT PROJECT**
THEMAC RESOURCES | NEW MEXICO COPPER CORPORATION
 Environmentally Responsible. Community Minded. Local Opportunities.
 30K TPD TAILINGS STORAGE FACILITY
 FEASIBILITY DESIGN
 SIERRA COUNTY, NEW MEXICO

GEOLOGIC CROSS-SECTION E-E'					
PROJECT No.	103-92557	FILE No.	10392557K004		
DESIGN	DW	2013-04-08	SCALE	AS SHOWN	
CADD	JHR	2013-07-10	DRAWING	8	
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REVIEW	DAK	2013-07-17			

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STRATUM DESCRIPTORS

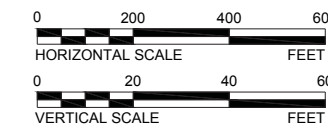
- ① ML, CL-ML, SM
 - ② CL
 - ③ CH, MH
 - ④ GRAVEL, SAND
 - ⑤ BASALT, CALICHE
 - ⑥ EMBANKMENT FILL
 - ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

NOTES

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4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

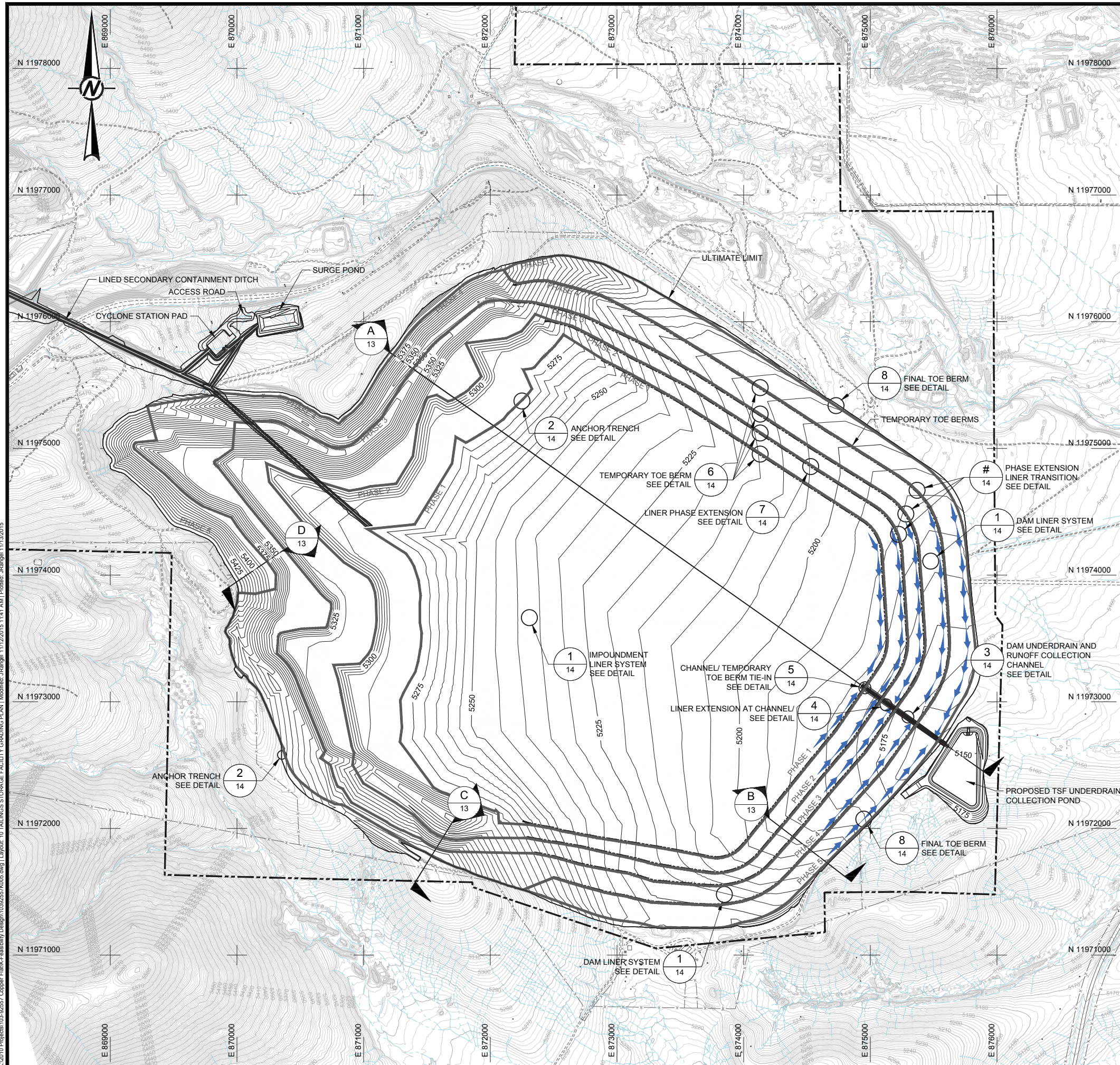
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE			
GEOLOGIC CROSS-SECTION F-F'			
PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	9
REVIEW	DAK	2013-07-17	



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LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- DRAINAGE
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

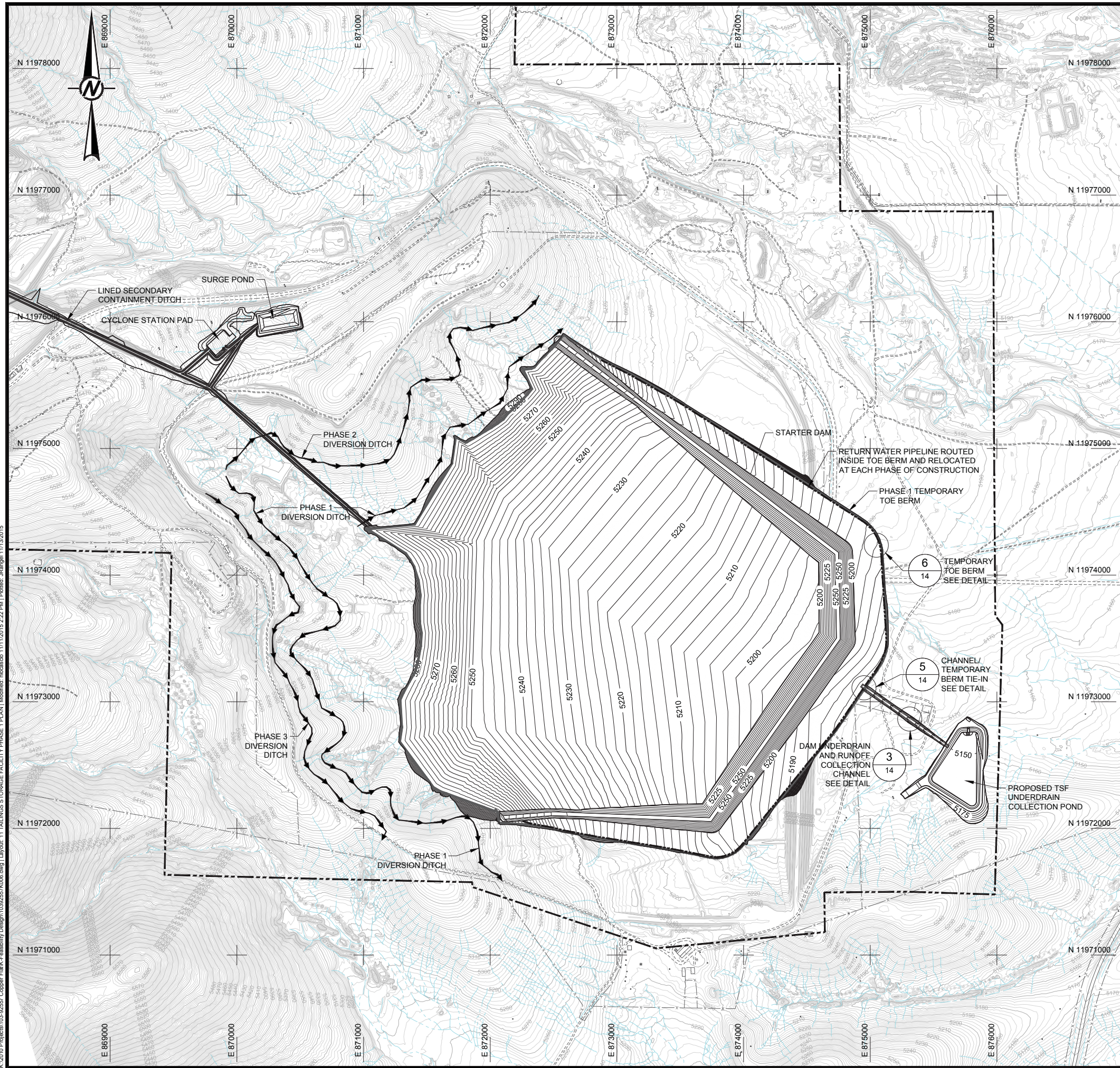
PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TAILINGS STORAGE FACILITY GRADING PLAN

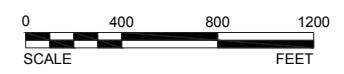
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CHECK	GM	2013-07-16	10
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LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
RESOURCES
Environmentally Responsible. Community Minded. Local Opportunities.

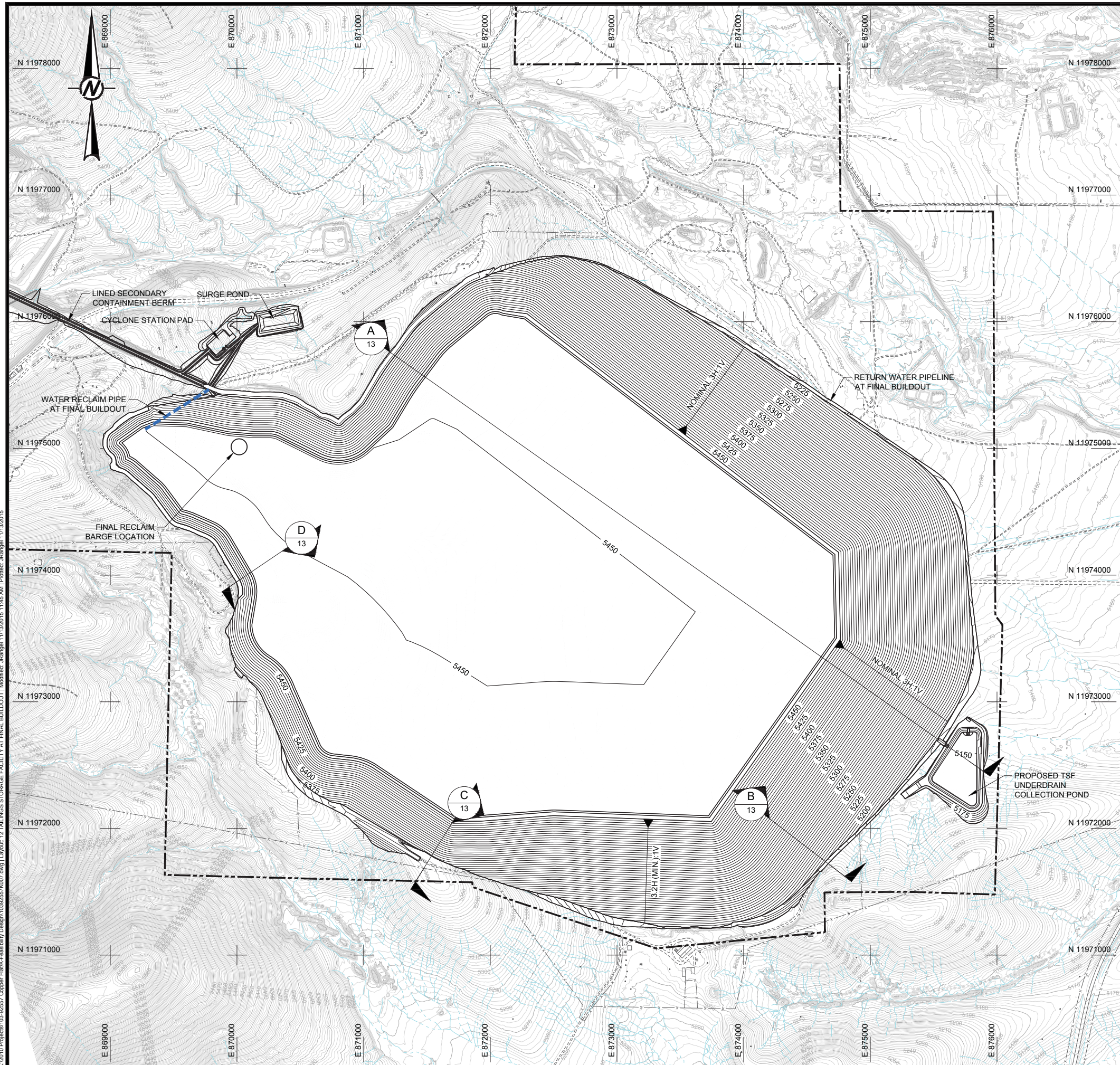
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
**TAILINGS STORAGE FACILITY PHASE 1
PLAN**

PROJECT No.	103-92557	FILE No.	10392557K006
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CHECK	GM 2013-07-16		
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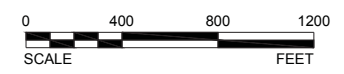


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LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1**
14
DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- A**
13
CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

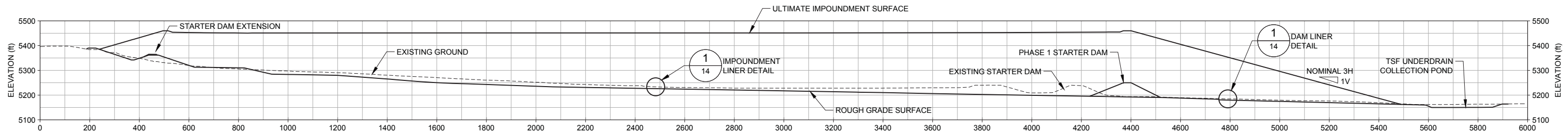
PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557 FILE No. 10392557K007
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

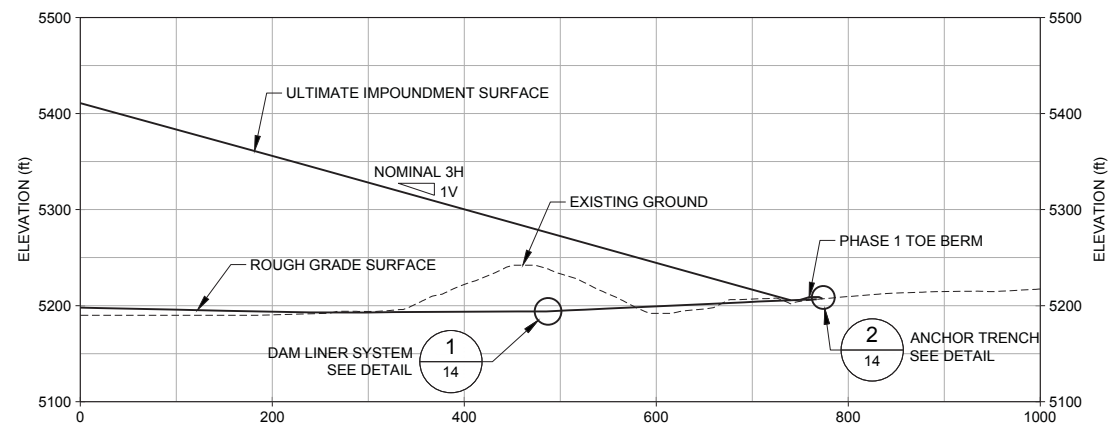
TITLE
TAILINGS STORAGE FACILITY AT FINAL BUILDOUT

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REVIEW	DAK	2013-07-17		

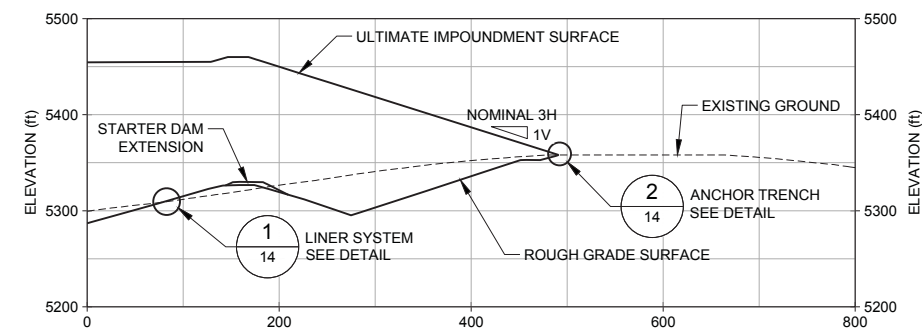
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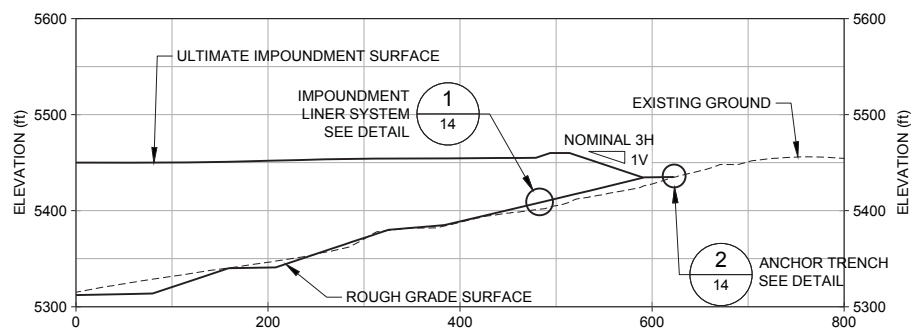
SCALE A **A** CROSS-SECTION A
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SCALE B **B** CROSS-SECTION B
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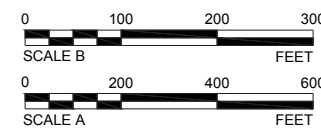


SCALE B **C** CROSS-SECTION C
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NOTE: STARTER DAM EXTENSION TO BE SHOWN ON DETAILED DESIGN DRAWINGS.

SCALE B **D** CROSS-SECTION D
13



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

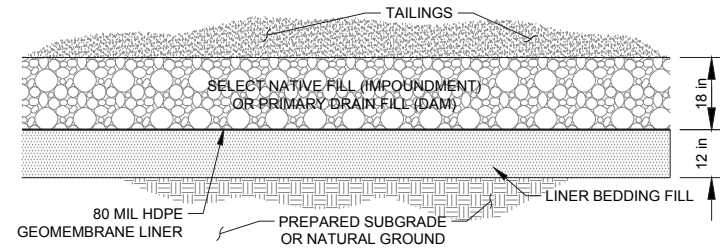
DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

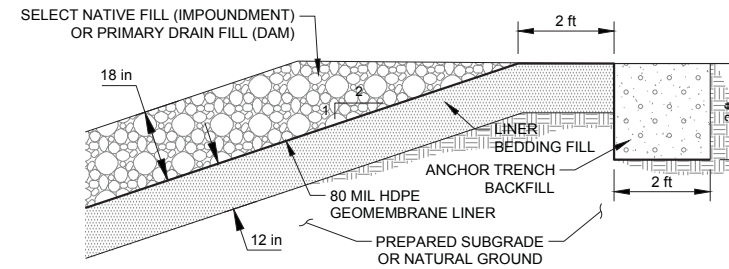
PROJECT No. 103-92557 FILE No. 10392557K008
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE			
TAILINGS STORAGE FACILITY CROSS-SECTIONS			
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	13
REVIEW	DAK	2013-07-17	

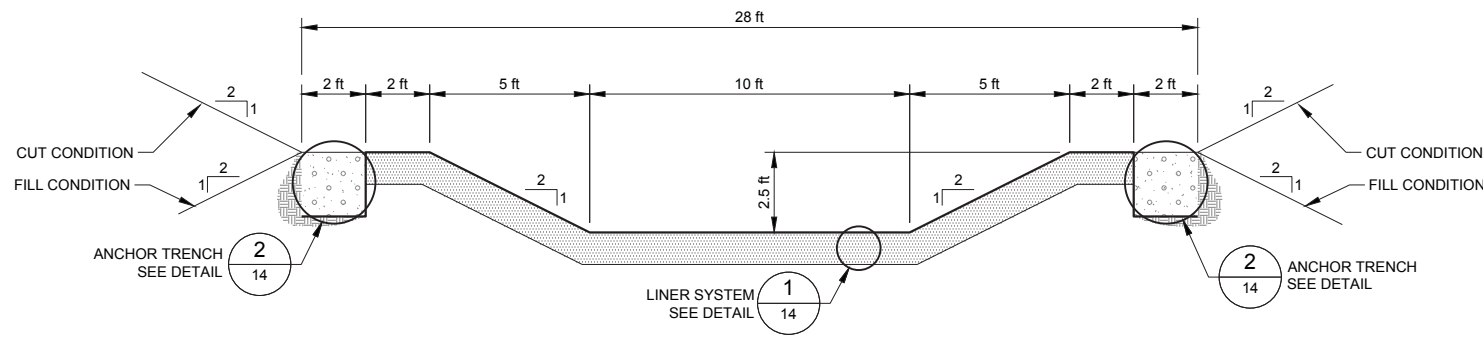




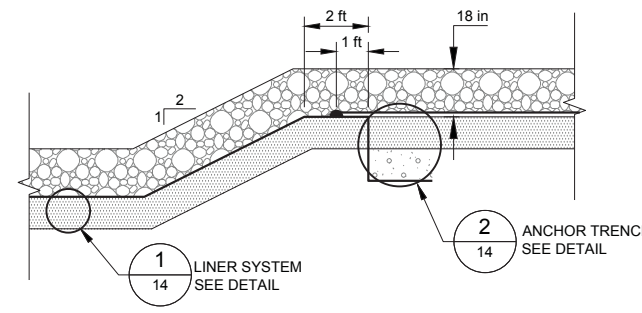
SCALE: N.T.S. 1 IMPOUNDMENT LINER SYSTEM DETAIL



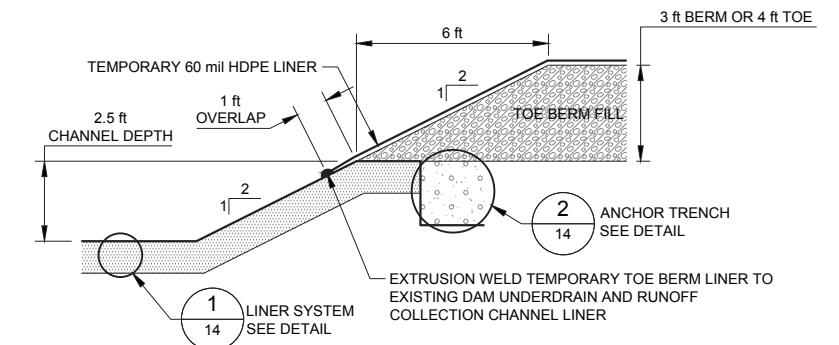
SCALE: N.T.S. 2 ANCHOR TRENCH DETAIL



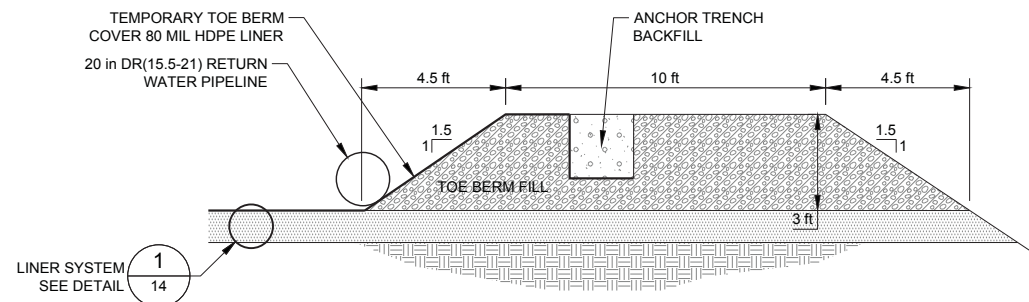
SCALE: N.T.S. 3 DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL DETAIL



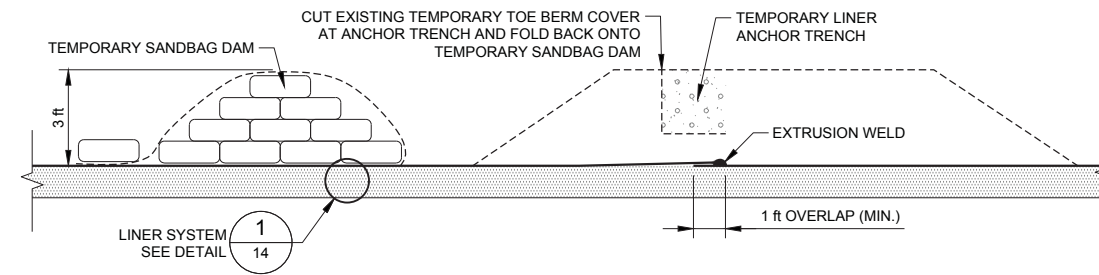
SCALE: N.T.S. 4 LINER EXTENSION WITH CHANNEL ANCHOR DETAIL



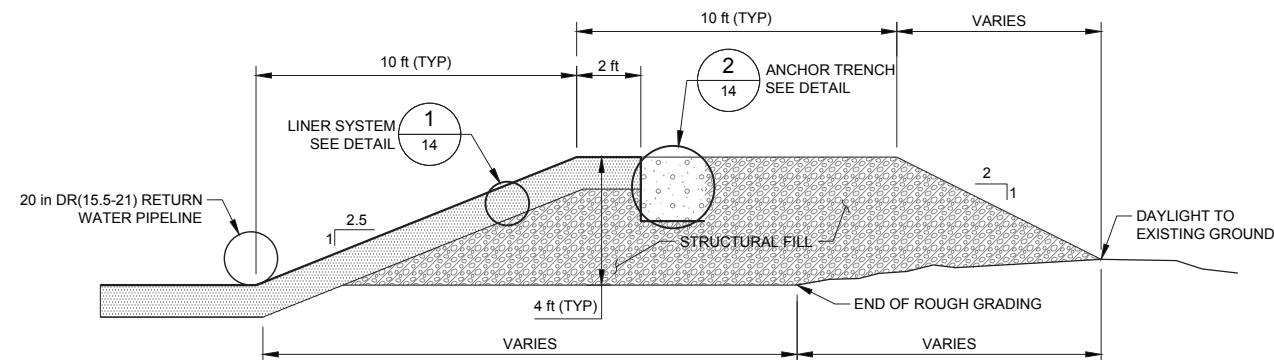
SCALE: N.T.S. 5 CHANNEL/TEMPORARY BERM TIE-IN DETAIL



SCALE N.T.S. 6 TEMPORARY TOE BERM DETAIL



SCALE N.T.S. 7 PHASE EXTENSION LINER TIE-IN AT TOE BERM DETAIL



SCALE: N.T.S. 8 FINAL TOE BERM DETAIL

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
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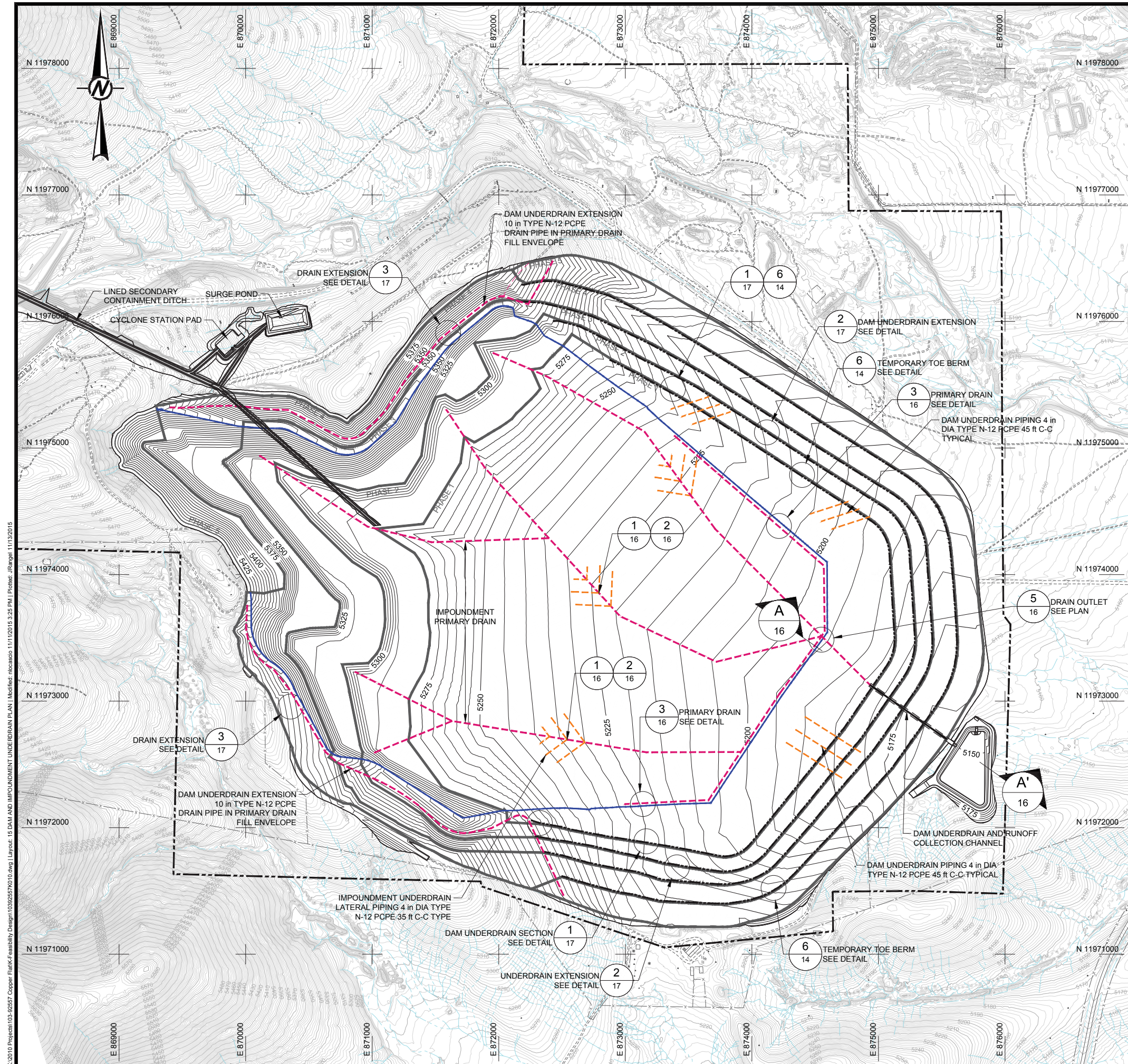
DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
RESOURCES
Environmentally Responsible. Community Minded. Local Opportunities.
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE					
TAILINGS STORAGE FACILITY DETAILS					
PROJECT No.	103-92557	FILE No.	10392557K009		
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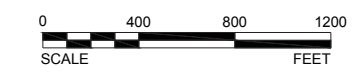
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LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 10 IN. DIA. N-12 PCPE PIPE
- 12 IN. DIA. N-12 PCPE PIPE
- 14 IN SCH80 STEEL
- 4 IN. DIA. N-12 PCPE PIPE
- LIMIT IMPOUNDMENT UNDERDRAIN SYSTEMS
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION

- NOTES**
- 1.) TYPICAL ORIENTATION FOR 4 IN UNDERDRAIN PIPING SHOWN. COVERAGE WILL BE AT 35 FT C-C SPACING OVER IMPOUNDMENT UNDERDRAIN AREA. COVERAGE WILL BE 45 FT C-C OVER DAM UNDERDRAIN AREA.
 - 2.) THE MAIN BODY OF THE EMBANKMENT WILL BE UNDERLAIN WITH A BLANKET DRAIN WITH INTERNAL 4 IN PCPE PIPE NETWORK. DAM UNDERDRAIN EXTENSION IN PHASE 2 THROUGH PHASE 5 WILL INCLUDE A 10 IN DIAMETER PCPE DRAIN CONSTRUCTED ON THE OUTER TOE OF THE STARTER DAM



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

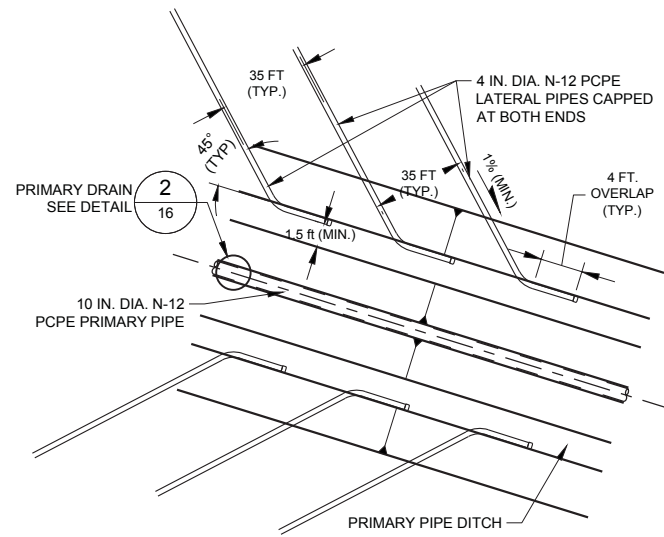
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
DAM AND IMPOUNDMENT UNDERDRAIN PLAN

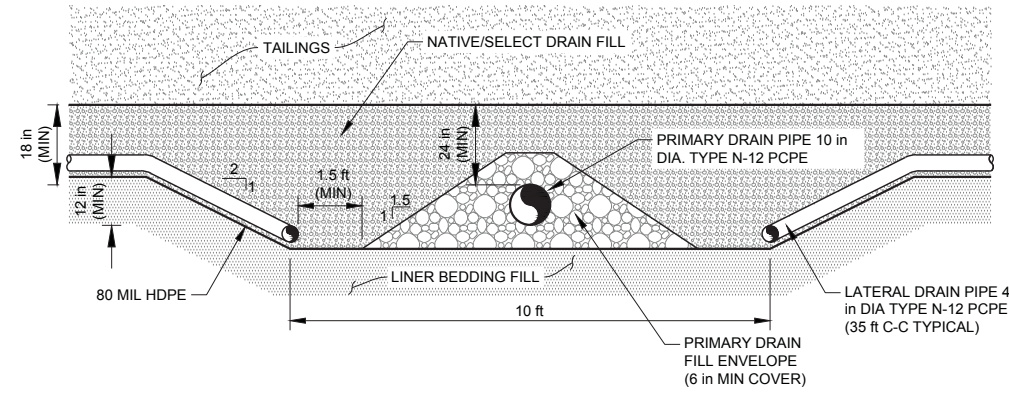
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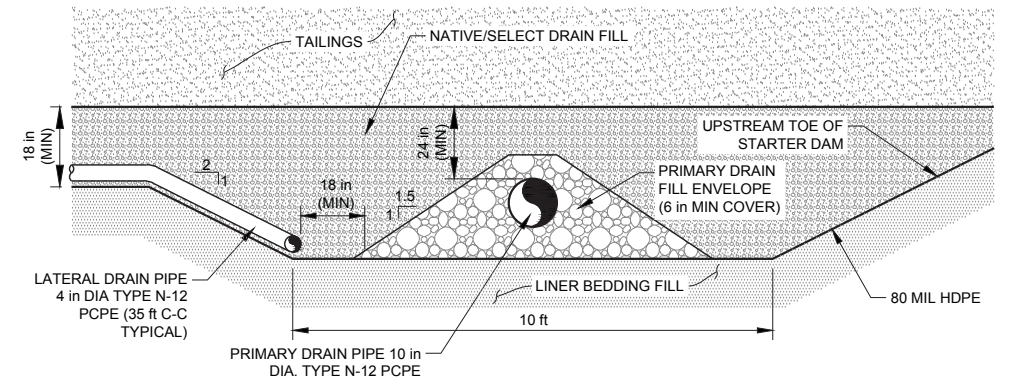
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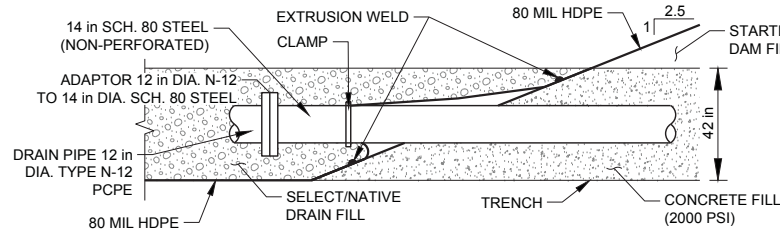
NTS 1 IMPOUNDMENT DRAIN PLACEMENT
16



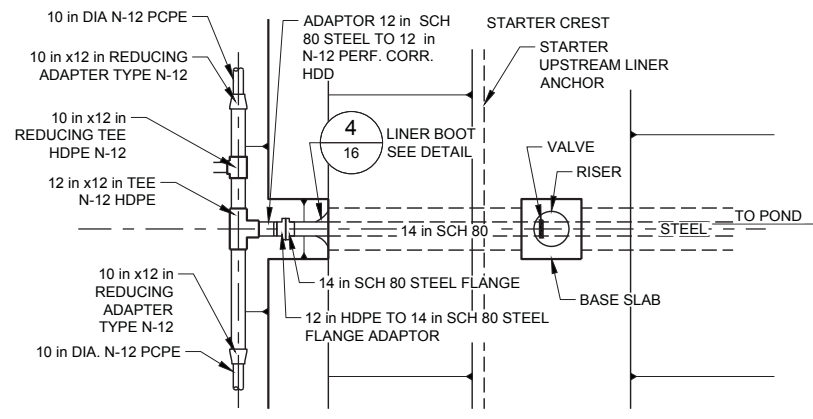
NTS 2 PRIMARY DRAIN PLACEMENT
16



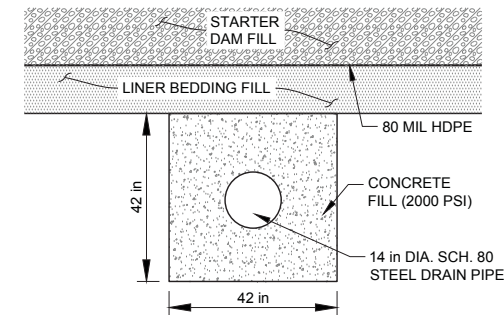
NTS 3 PRIMARY DRAIN PLACEMENT AT STARTER DAM TOE
16



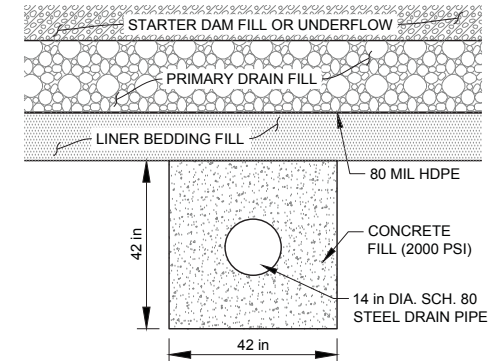
NTS 4 DETAIL DRAIN PIPE TO STARTER DAM ENTRY
16



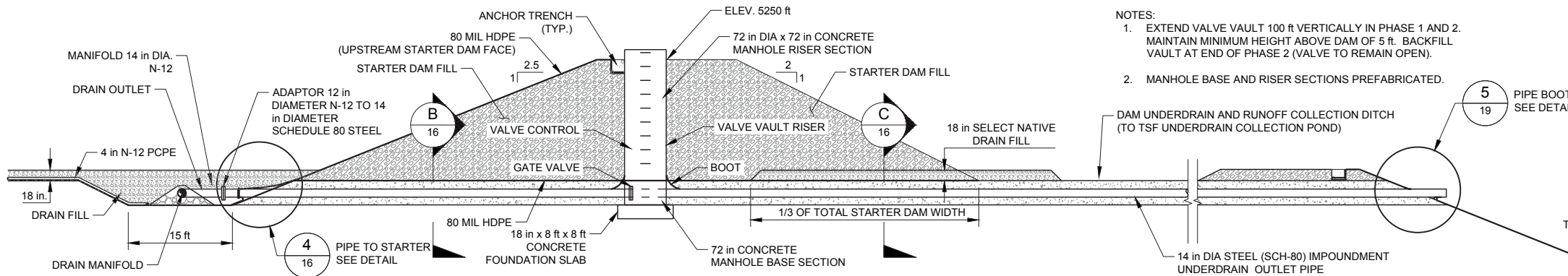
NTS 5 DRAIN OUTLET PLAN
16



NTS B SECTION B-B
16



NTS C SECTION C-C
16



NTS A SECTION A-A DRAIN OUTLET
16

- NOTES:
1. EXTEND VALVE VAULT 100 ft VERTICALLY IN PHASE 1 AND 2. MAINTAIN MINIMUM HEIGHT ABOVE DAM OF 5 ft. BACKFILL VAULT AT END OF PHASE 2 (VALVE TO REMAIN OPEN).
 2. MANHOLE BASE AND RISER SECTIONS PREFABRICATED.

5 PIPE BOOT SEE DETAIL
19

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

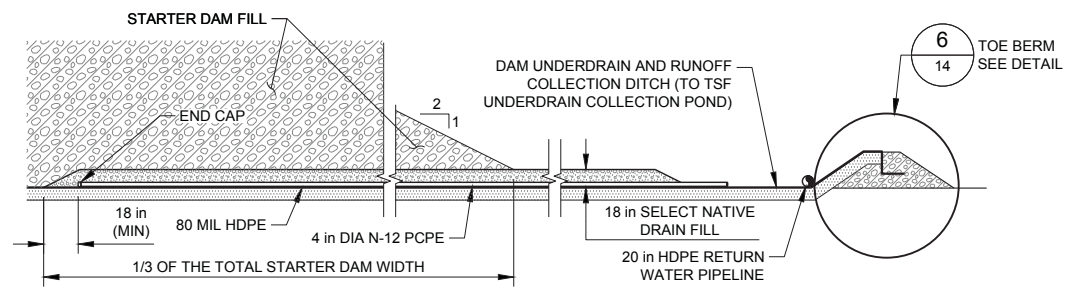
DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC NEW MEXICO COPPER CORPORATION
RESOURCES
Environmentally Responsible. Community Minded. Local Opportunities.
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

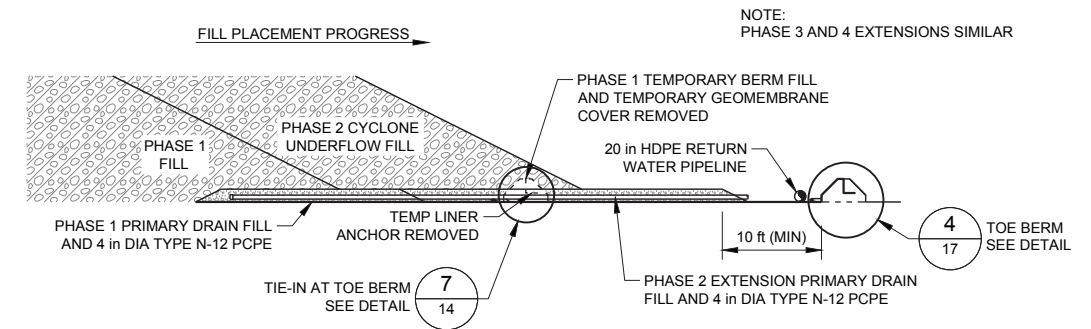
DAM UNDERDRAIN DETAILS (1 of 2)

PROJECT No.	103-92557	FILE No.	10392557K011
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CHECK	GM 2013-07-16		
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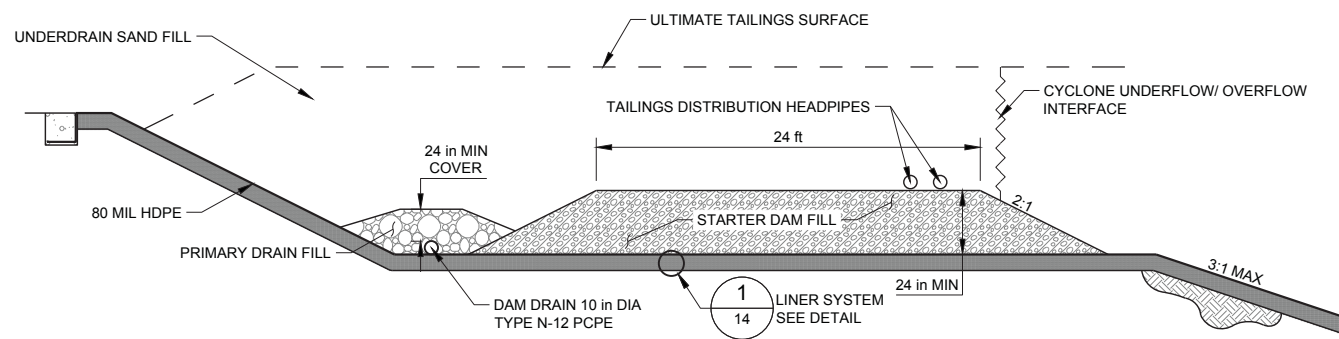




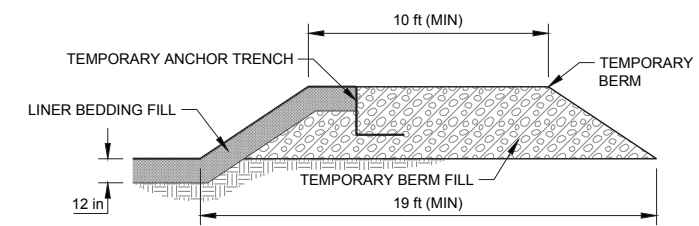
NTS **1** DAM UNDERDRAIN SECTION
17



NTS **2** PHASE 1 TO PHASE 2 TSF LINER AND DAM UNDERDRAIN EXTENSION
17



NTS **3** DAM UNDERDRAIN 10 IN PCPE DRAIN EXTENSION
17



NTS **4** TEMPORARY PERIMETER BERM
17

K:\2010 Projects\103-9257-Copper-Flat\Feasibility Design\1039257K011.dwg | Layout: 17 DAM UNDERDRAIN DETAILS (2 of 2) | Modified: mcaasico 11/11/2015 4:06 PM | Plotted: jRangel 11/13/2015

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

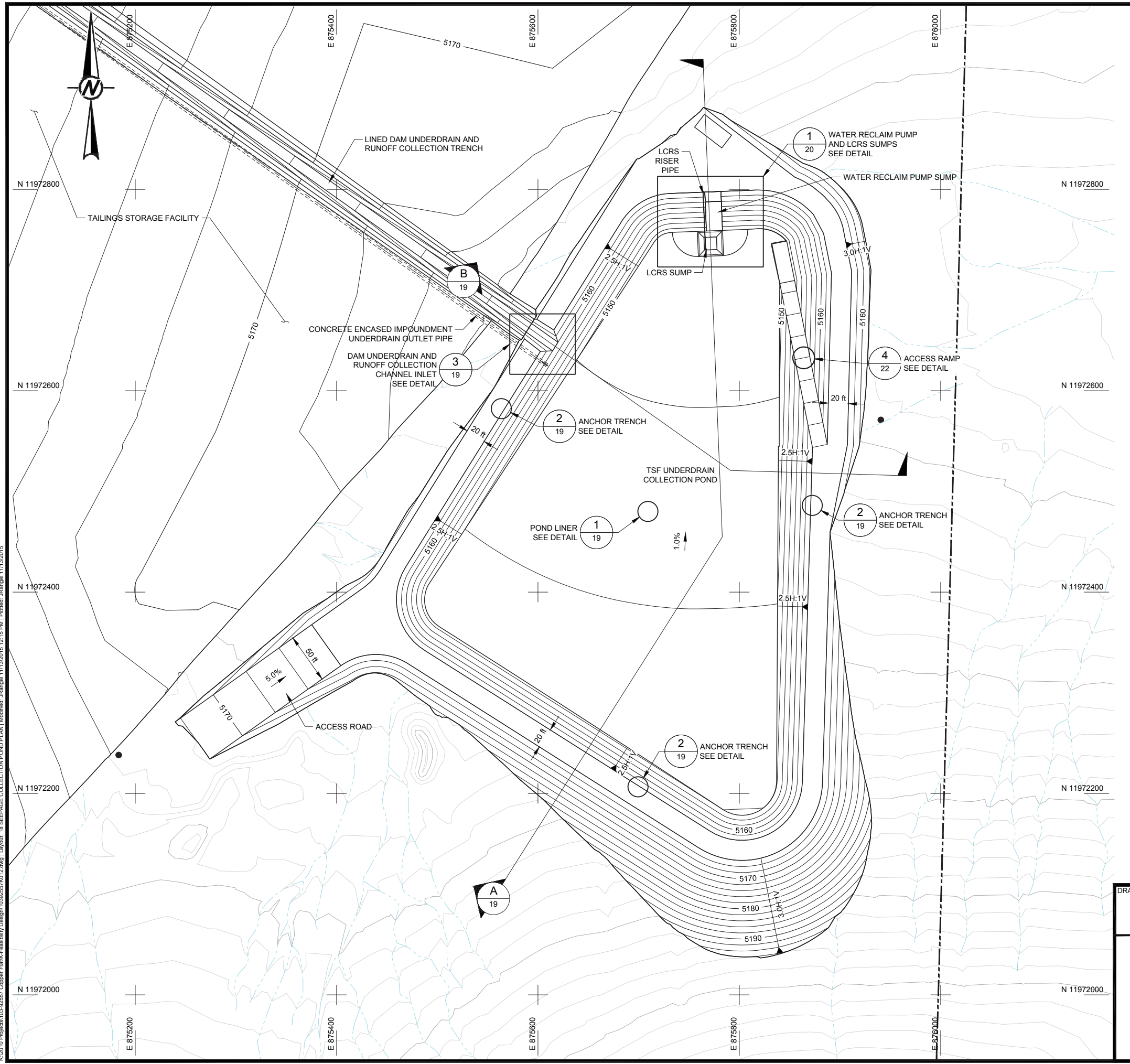
DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
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PROJECT No. 103-92557 FILE No. 10392557K011
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE DAM UNDERDRAIN DETAILS (2 of 2)					
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
	REVIEW	DAK	2013-07-17		
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K:\2010 Projects\103-9257-Copper Flat\Feasibility Design\1039257K012.dwg | Layout: 18 SEEPAGE COLLECTION POND PLAN | Modified: 11/13/2015 12:15 PM | Plotted: 11/13/2015



LEGEND

- 3600 --- EXISTING GROUND CONTOUR (ft -MSL)
- - - - - EXISTING ROADS
- - - - - EXISTING DRAINAGE
- x - x - x - EXISTING FENCELINE
- - - - - MINE PERMIT AREA BOUNDARY
- == 3600 == REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- ▲ SLOPE INDICATOR
- 3H:1V or 3H 1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1 14 DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- A 13 CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
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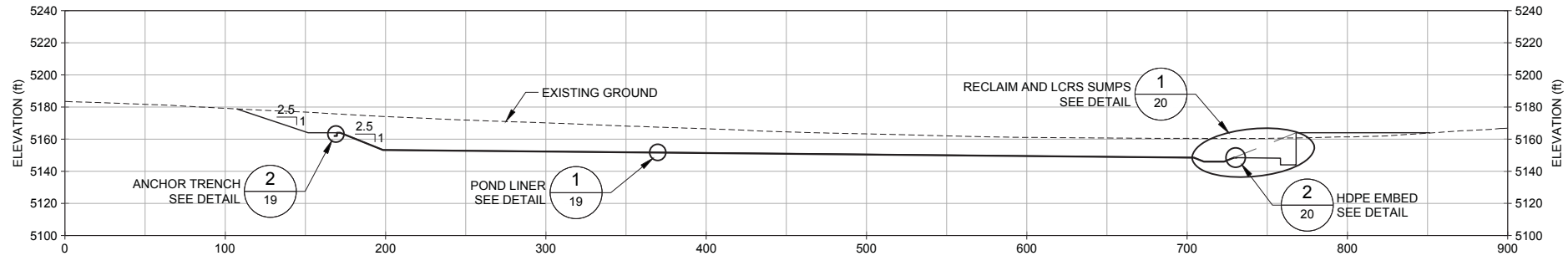
DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
Environmentally Responsible. Community Minded. Local Opportunities.

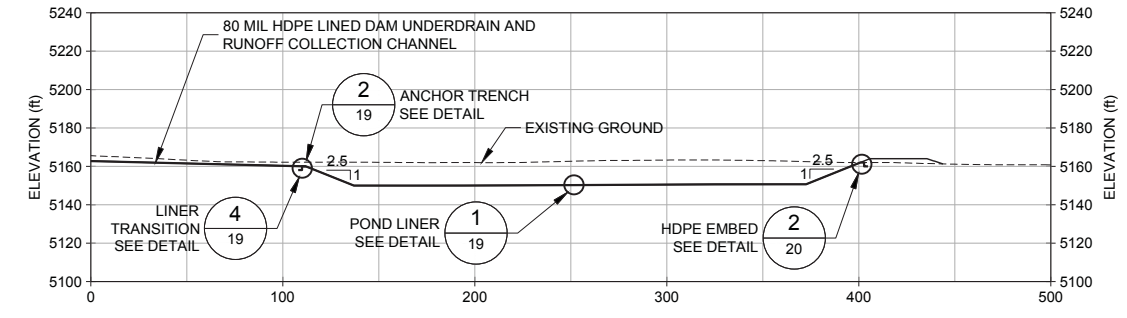
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TSF UNDERDRAIN COLLECTION POND PLAN

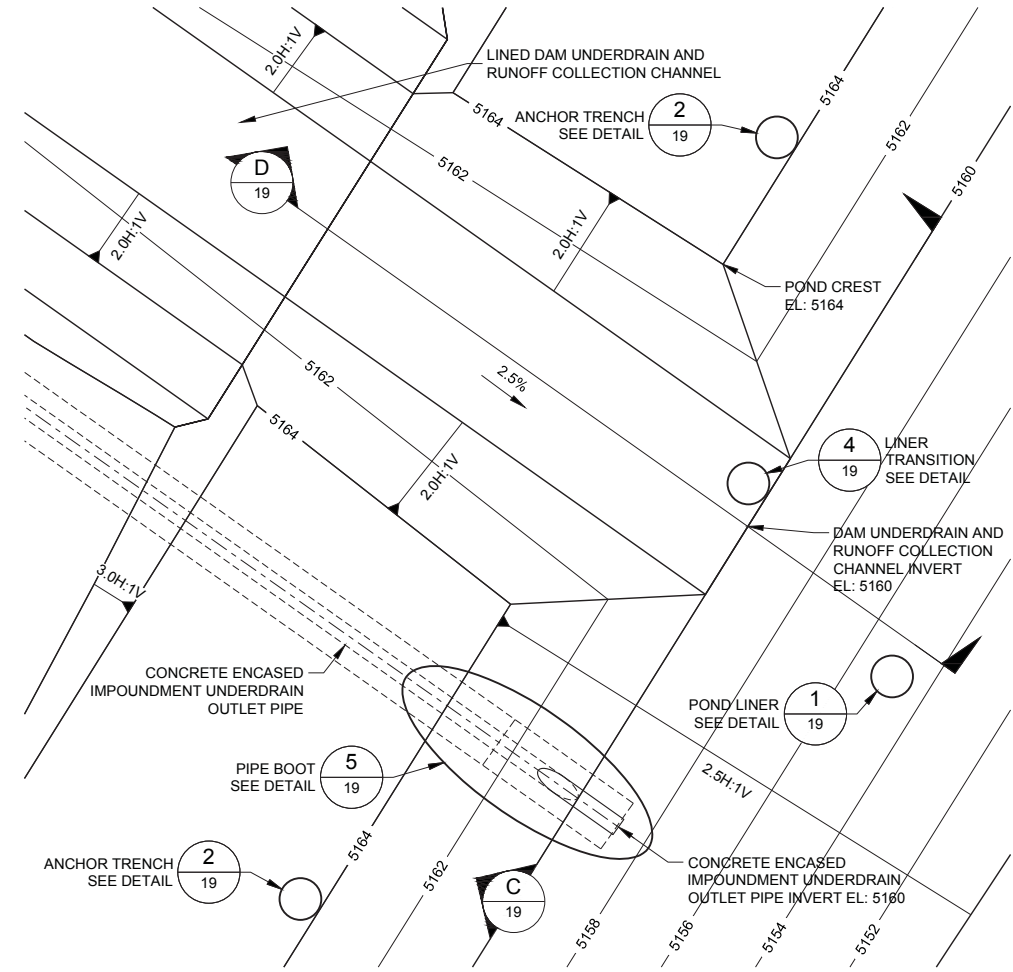
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DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	18
REVIEW	DAK	2013-07-17	



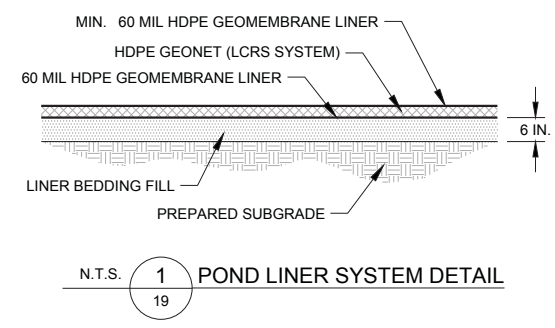
SCALE A **A** CROSS-SECTION A
19



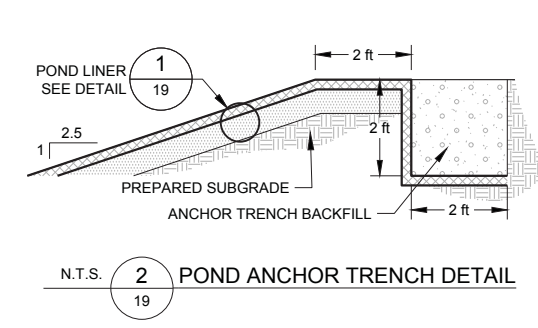
SCALE A **B** CROSS-SECTION B
19



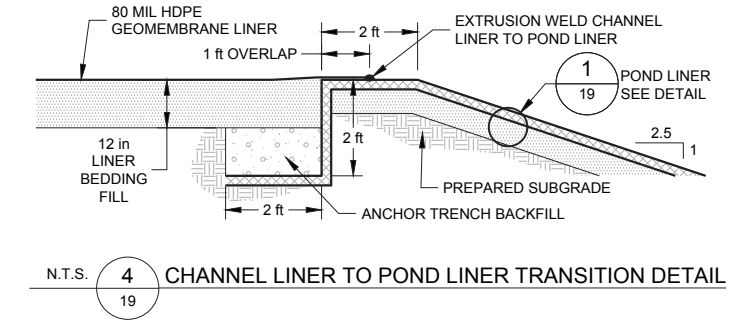
SCALE B **3** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET TO POND
19



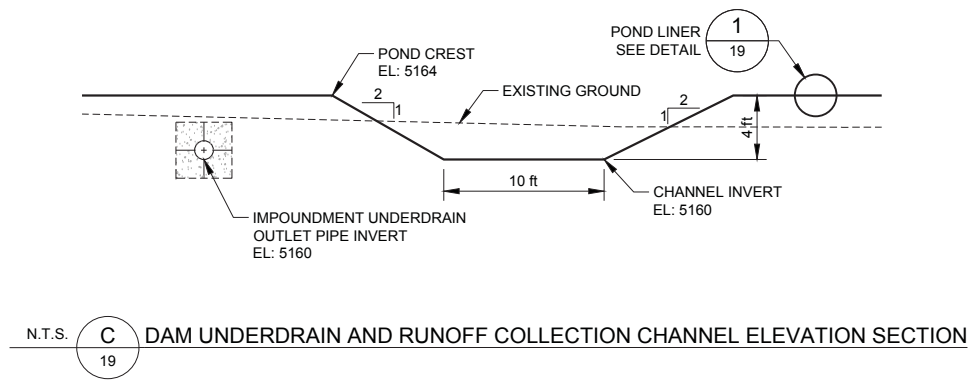
N.T.S. **1** POND LINER SYSTEM DETAIL
19



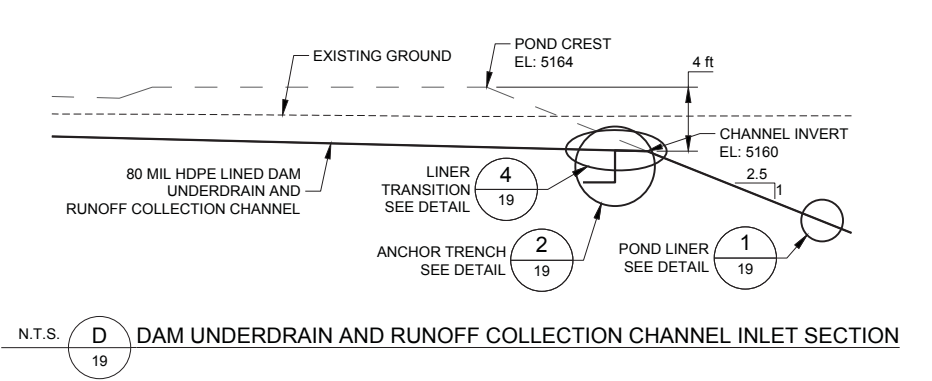
N.T.S. **2** POND ANCHOR TRENCH DETAIL
19



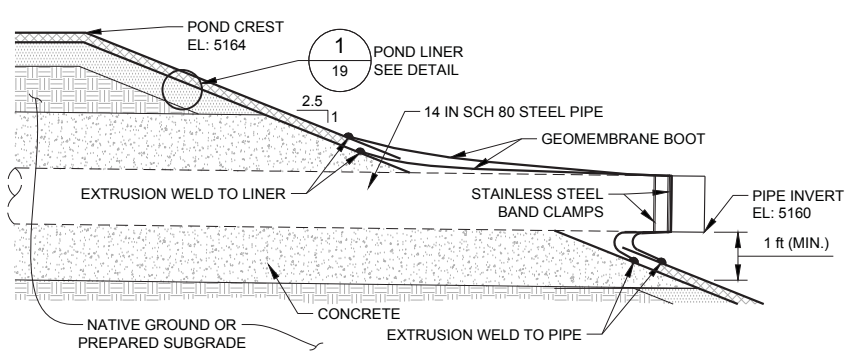
N.T.S. **4** CHANNEL LINER TO POND LINER TRANSITION DETAIL
19



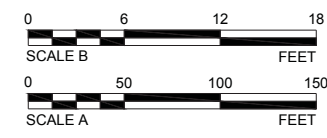
N.T.S. **C** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL ELEVATION SECTION
19



N.T.S. **D** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET SECTION
19



N.T.S. **5** IMPOUNDMENT UNDERDRAIN OUTLET PIPE BOOT DETAIL
19



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
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COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

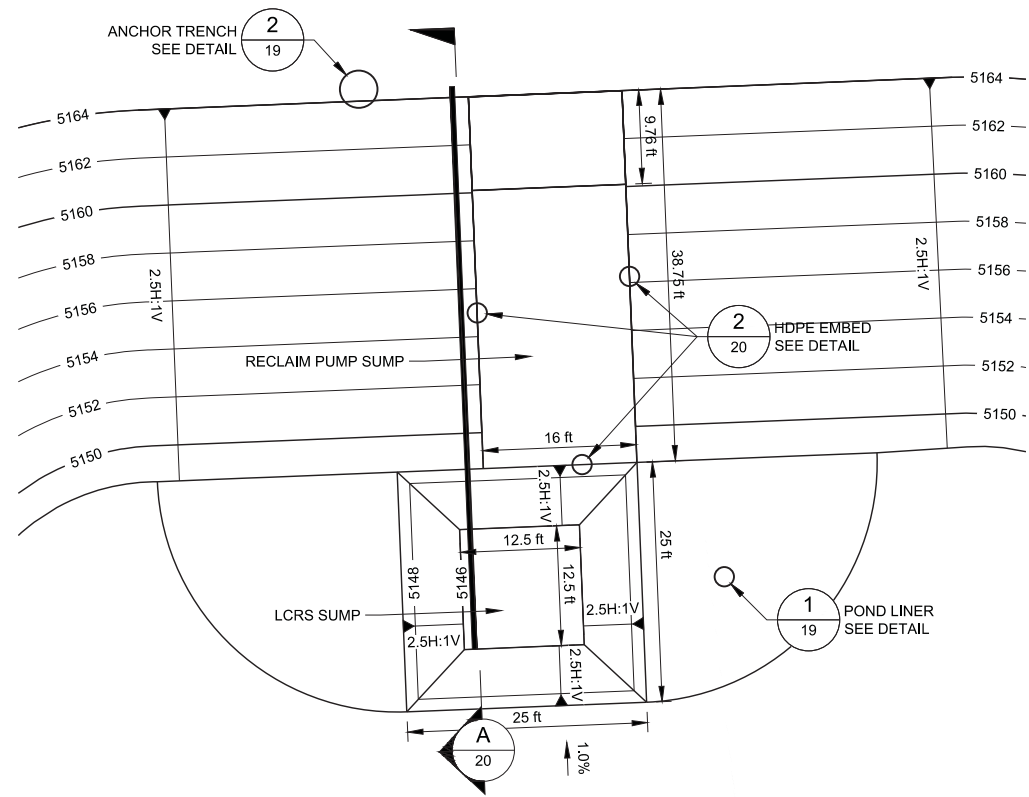
TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)

PROJECT No.	103-92557	FILE No.	10392557K013
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

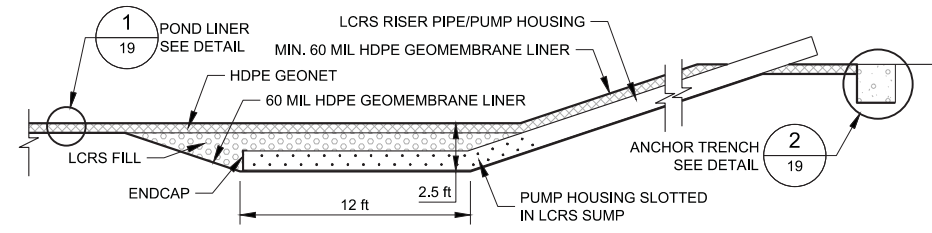


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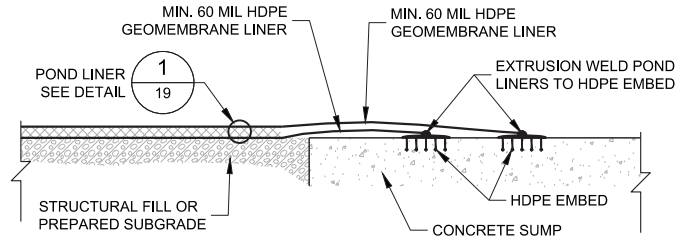
K:\2010 Projects\103-92557\Copper Flat\KF\Feasibility Design\10392557K013.dwg | Layout: 19 SEEPAGE COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2) | Modifier: nicolasa 11/11/2015 4:21 PM | Plotter: JRange1 11/13/2015



SCALE A **1** RECLAIM PUMP SUMP AND LCRS SUMP DETAIL
20



N.T.S. **A** LCRS SUMP SECTION
20



N.T.S. **2** CONCRETE DOUBLE EMBED DETAIL
20

K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K013.dwg | Layout: 20 SEEPAGE COLLECTION POND CROSS-SECTIONS AND DETAILS (2 OF 2) | Modified: nccasdo 11/11/2015 4:21 PM | Plotter: JRange1 11/13/2015

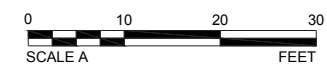
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2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK	
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

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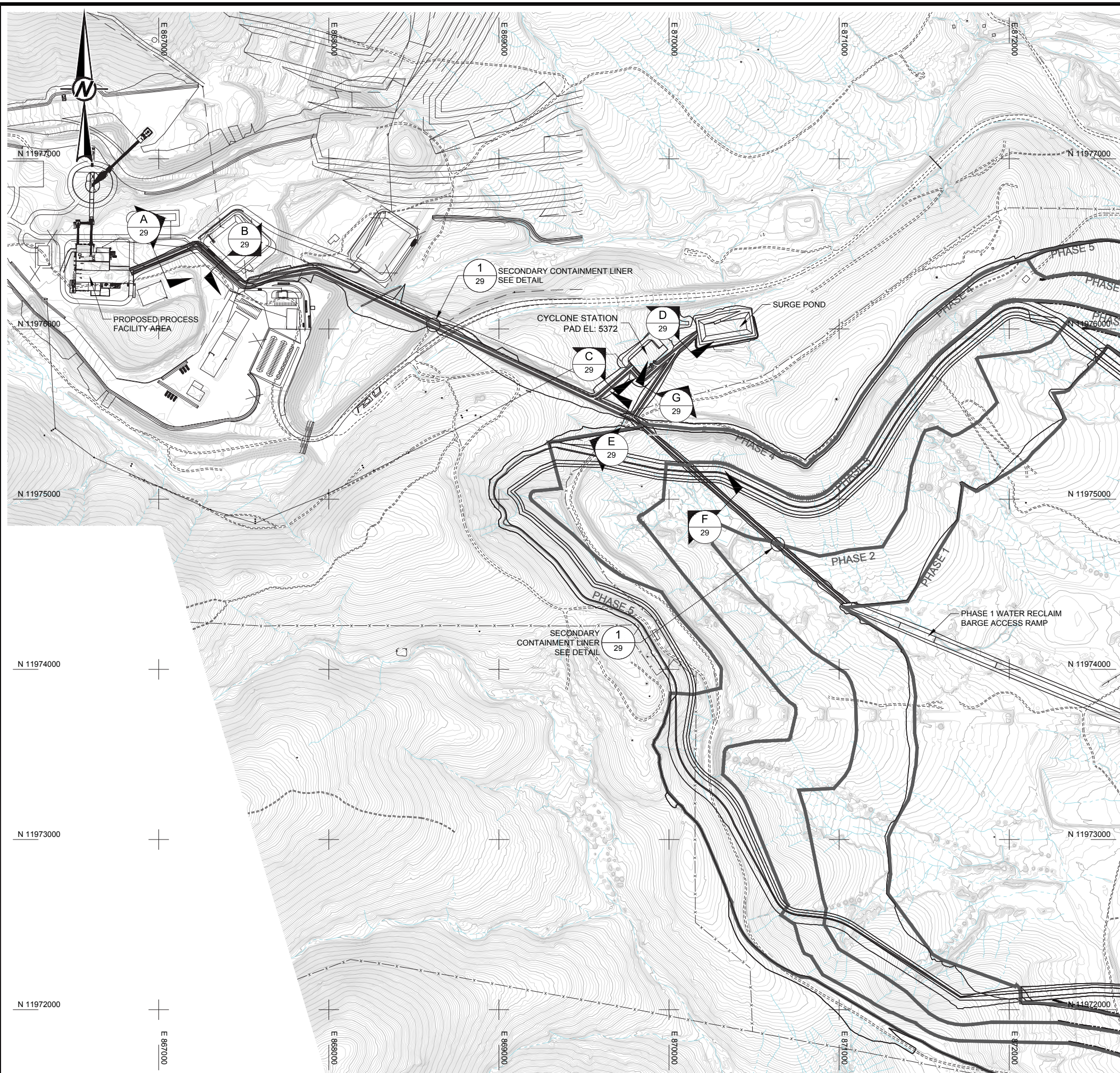
**30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO**

TITLE
**TSF UNDERDRAIN COLLECTION POND
CROSS-SECTION AND DETAILS(2 OF 2)**



PROJECT No.	103-92557	FILE No.	10392557K013	
DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		20

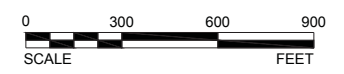




LEGEND

	EXISTING GROUND CONTOUR (ft -MSL)
	EXISTING ROADS
	EXISTING DRAINAGE
	EXISTING FENCELINE
	REGRADED CONTOURS (ft -MSL)
	GRADE BREAK
	SLOPE INDICATOR
	3 HORIZONTAL TO 1 VERTICAL SLOPE
	GRADE INDICATOR
	DETAIL CALLOUT DETAIL ID DRAWING SHEET LOCATION
	CROSS-SECTION CALLOUT SECTION ID DRAWING SHEET LOCATION

K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K014.dwg | Layout: 21 CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN | Modified: nbarcaco 11/11/2016 4:45 PM | Plotted: JRange 11/13/2015



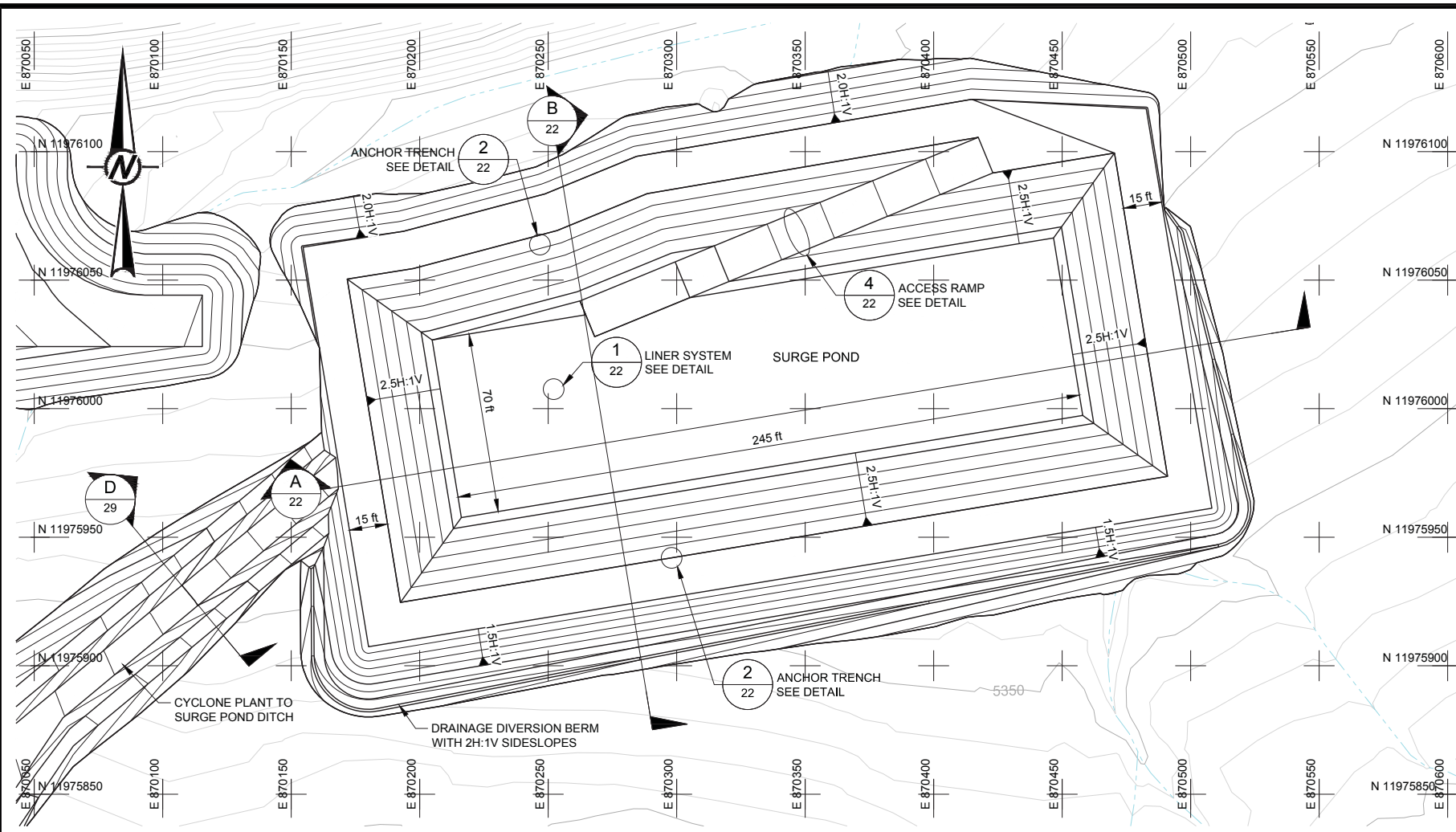
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

PROJECT: **COPPER FLAT PROJECT**
THEMAC RESOURCES | NEW MEXICO COPPER CORPORATION
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30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN
 SIERRA COUNTY, NEW MEXICO

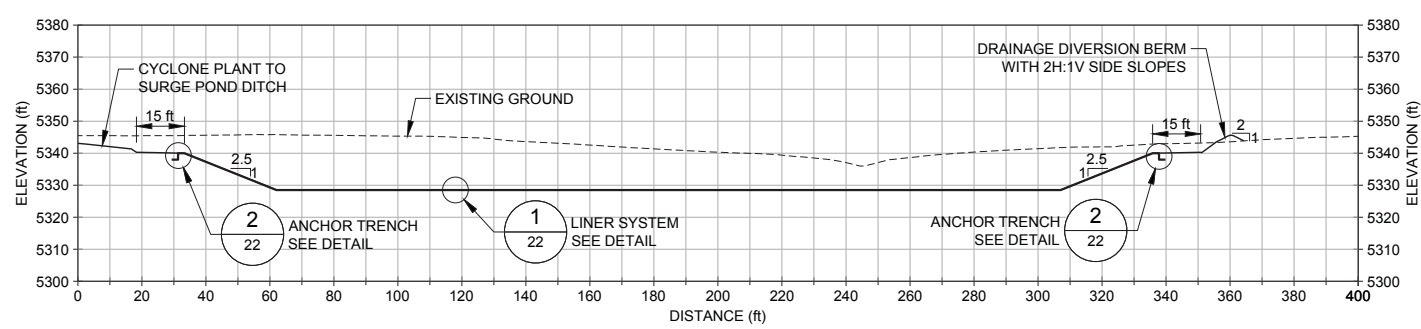
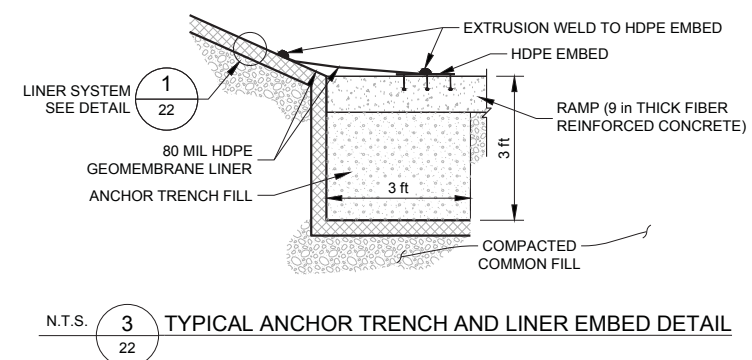
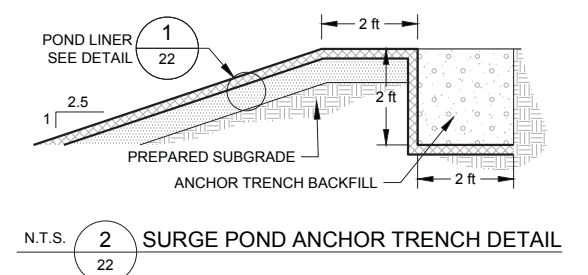
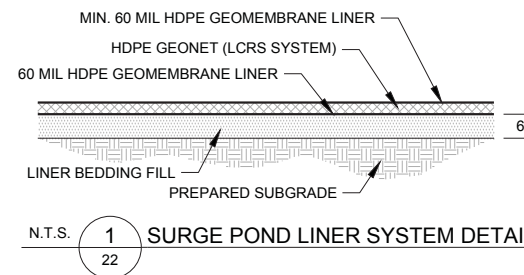
TITLE
CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN

	PROJECT No.	103-92557	FILE No.	10392557K014	
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
	REVIEW	DAK	2013-07-17		

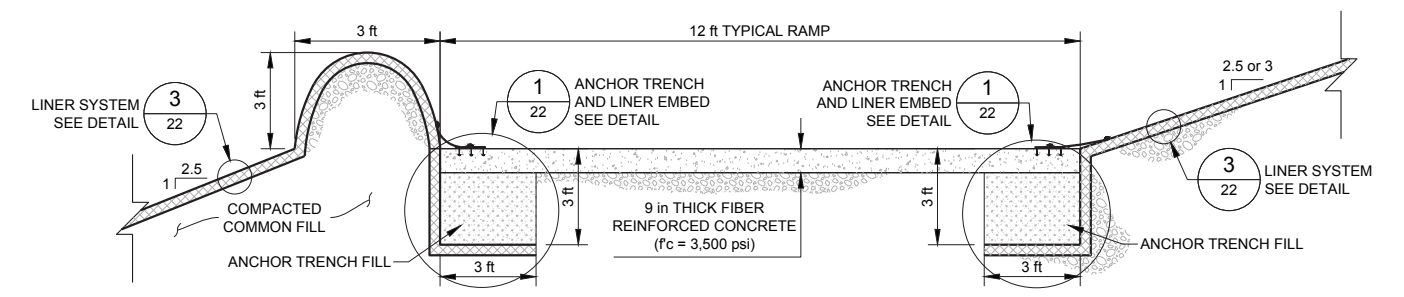


SURGE POND PLAN
SCALE A

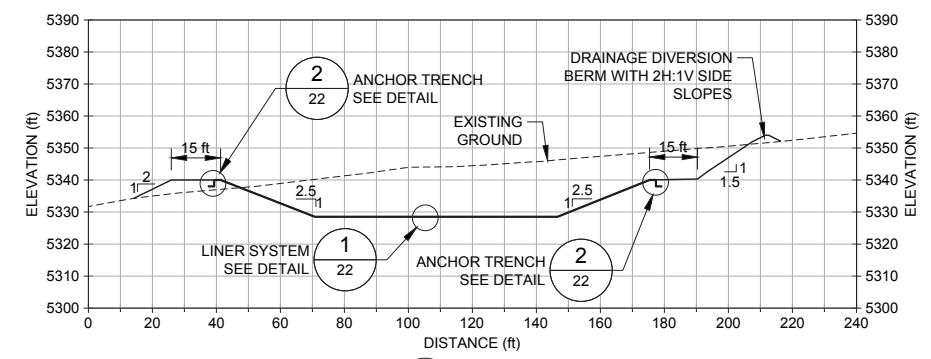
- LEGEND**
- 3600 EXISTING GROUND CONTOUR (ft -MSL)
 - 3600 REGRADED CONTOURS (ft -MSL)
 - GRADE BREAK
 - SLOPE INDICATOR
 - $3H:1V$ or $\frac{3H}{1V}$ 3 HORIZONTAL TO 1 VERTICAL SLOPE
 - 5% GRADE INDICATOR
 - 1
2-315
DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
 - A
2-313
CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION



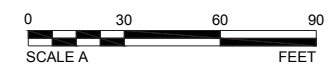
SCALE A **A**
22
CROSS-SECTION A



N.T.S. **4**
22
TYPICAL ACCESS RAMP DETAIL



SCALE A **B**
22
CROSS-SECTION B



2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK
REV	DATE	DES	CADD	CHK	RVW

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

THEMAC NEW MEXICO COPPER CORPORATION
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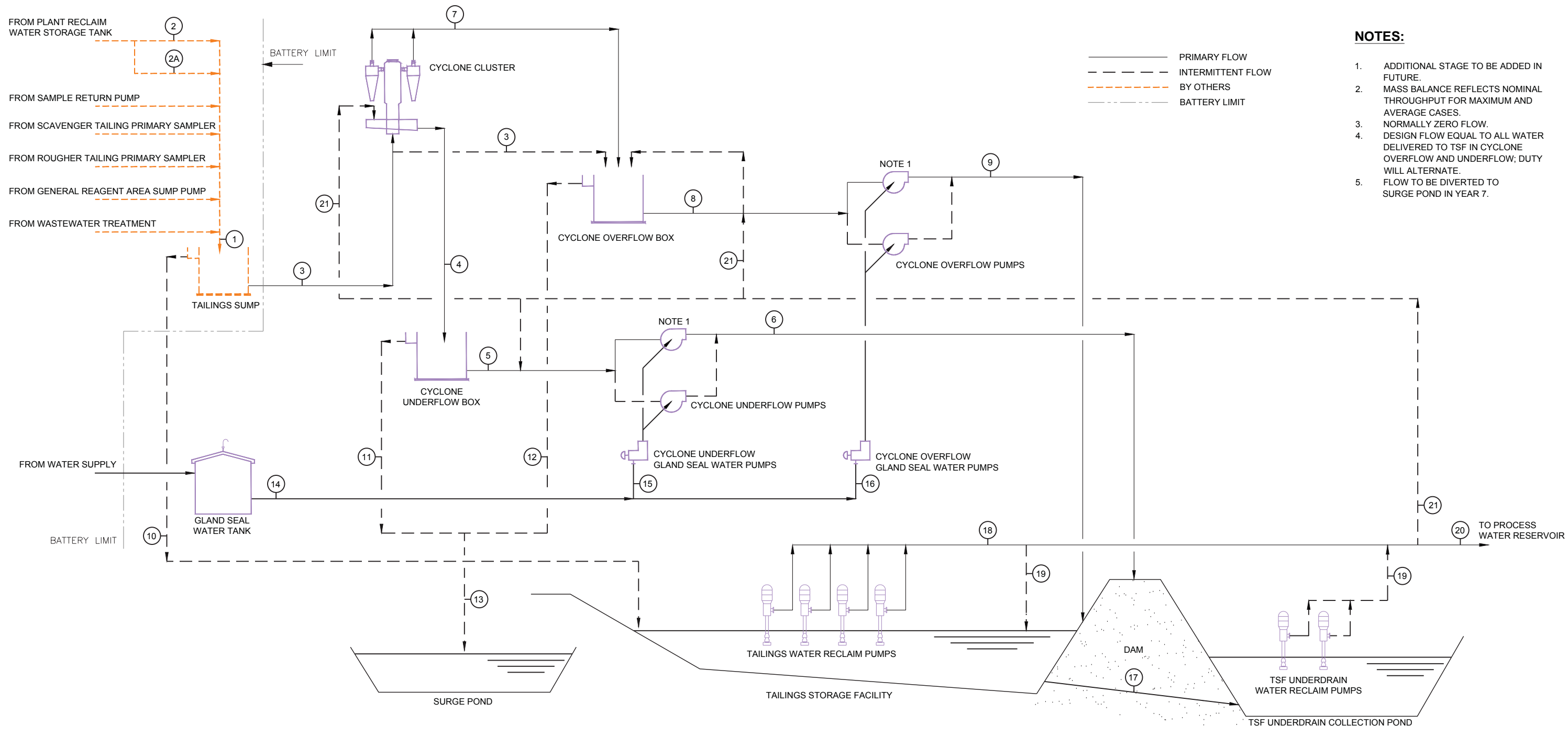
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS

PROJECT No.	103-92557	FILE No.	10392557K015	
DESIGN	DW	2013-05-03	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		



K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K015.dwg | Layout: 22 SURGE POND PLAN CROSS-SECTIONS AND DETAILS | Modified: tccasocio 11/12/2015 7:55 AM | Plotter: JRange | 11/13/2015



- NOTES:**
- ADDITIONAL STAGE TO BE ADDED IN FUTURE.
 - MASS BALANCE REFLECTS NOMINAL THROUGHPUT FOR MAXIMUM AND AVERAGE CASES.
 - NORMALLY ZERO FLOW.
 - DESIGN FLOW EQUAL TO ALL WATER DELIVERED TO TSF IN CYCLONE OVERFLOW AND UNDERFLOW; DUTY WILL ALTERNATE.
 - FLOW TO BE DIVERTED TO SURGE POND IN YEAR 7.

K:\2010 Projects\103-92557\Copper\Final\Facility Design\10392557K023.dwg | Layout: 23 GENERAL PROCESS FLOW DIAGRAM | Modified: nicascos 11/11/2015 6:09 PM | Plotted: J.Rangel 11/13/2015

Note 2

	Tailings Sump Feed	Tailings Sump Flush Water Note 3	Dilution Water Note 3	Tailings Sump Outlet/ Cyclone Inlet	Cyclone Underflow w	Cyclone Underflow Box Discharge	Cyclone Underflow Pump Discharge	Cyclone Overflow	Cyclone Overflow Box Discharge	Cyclone Overflow Pump Discharge	Tailings Sump Overflow Note 3, 5	Cyclone Underflow Box Overflow Note 3	Cyclone Overflow Box Overflow Note 3	Cyclone Area Surge Discharge Note 3	Gland Seal Water Supply Header	Cyclone Underflow Pump Gland Seal Water	Cyclone Overflow Pump Gland Seal Water	TSF Underflow	TSF Reclaim Water Note 4	TSF Underdrain Collection Pond Reclaim Water Note 4	Reclaim Water to Plant	Cyclone Area Flush Water from TSF Note 3
Stream	1	2	2A	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21
TPD (solids)	32000			32000	14550	14550	14550	17450	17450	17450	32000	14550	32000	32000								
Solids (t/h)	1333			1333	606	606	606	727	727	727	1333	606	1333	1333								
Solution (t/h)	3249	3755	0	3249	260	260	264	2989	2989	3001	3249	260	3249	3249	17	4	13	3267	3267	1001	3267	490
Slurry (t/h)	4582			4582	866	866	870	3716	3716	3728	4582	866	4582	4582								
Solids (%) Cw	29.1%			29.1%	70.0%	70.0%	69.7%	19.6%	19.6%	19.5%	29.1%	70.0%	29.1%	29.1%								
Solids (gpm)	2018			2018	918	918	918	1101	1101	1101	2018	918	2018	2018								
Solution (gpm)	12981	14999	0	12981	1038	1038	1054	11943	11943	11993	12981	1038	12981	12981	66	16	50	13047	13047	4000	13047	1956
Slurry (gpm)	14999			14999	1956	1956	1972	13044	13044	13094	14999	1956	14999	14999								
Solids (%) Cv	13.5%			13.5%	46.9%	46.9%	46.5%	8.4%	8.4%	8.4%	13.5%	46.9%	13.5%	13.5%								
Solids SG	2.64			2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64								
Solution SG	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Slurry SG	1.22			1.22	1.77	1.77	1.76	1.14	1.14	1.14	1.22	1.77	1.77	1.22								

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	MJG	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

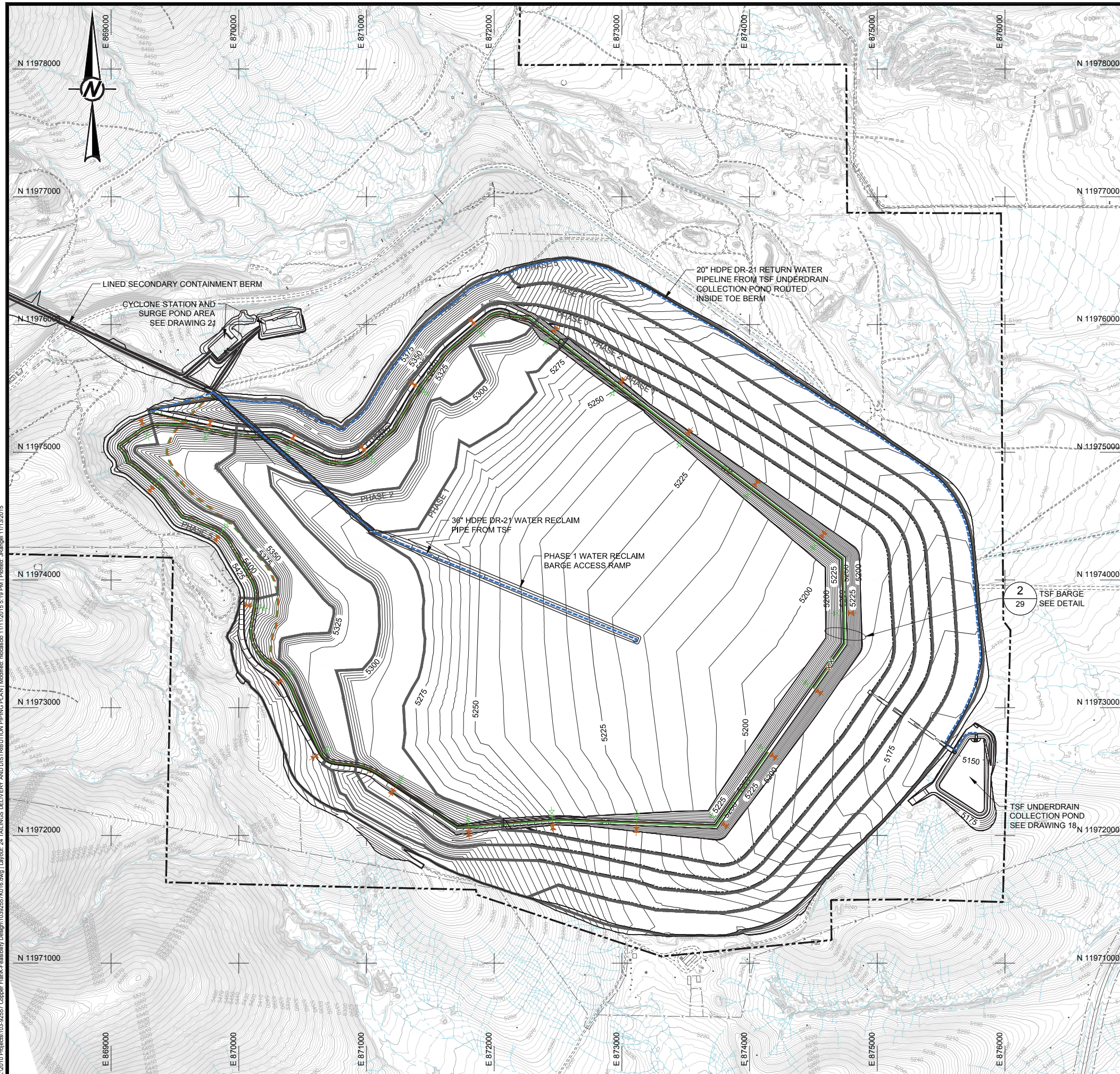
PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
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COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

GENERAL PROCESS FLOW DIAGRAM				
PROJECT No.	103-92557	FILE No.	10392557K023	
DESIGN	DMW	2013-04-08	SCALE	NTS
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		
			23	



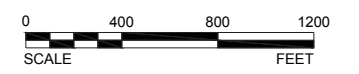
K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K016.dwg | Layout: 24 TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN | Modified: 11/11/2015 5:19 PM | Plotter: JRange1 11/13/2015



LEGEND

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- PHASE 1 PHASE BOUNDARY
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - PERMANENT ALIGNMENT
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - TEMPORARY ALIGNMENT
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - FINAL ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - PERMANENT ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - TEMPORARY ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - FINAL ALIGNMENT
- PINCH VALVES
- KNIFE GATES
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT
SECTION ID
DRAWING SHEET LOCATION

- NOTES**
- PHASE 1 TAILINGS DISTRIBUTION BEGINS AT EL: 5250 AND ENDS AT EL: 5280.
 - KNIFE GATE VALVES ON CYCLONE UNDERFLOW AND CYCLONE OVERFLOW DISTRIBUTION PIPES AT 2000 ft SPACING. (SHOWN SPACED FOR CLARITY)
 - SPIGOTS ON CYCLONE OVERFLOW PIPE AT 660 ft SPACING. (SHOWN SPACED FOR CLARITY)
 - SPIGOTS ON CYCLONE UNDERFLOW PIPE AT 330 ft SPACING. (SHOWN SPACED FOR CLARITY)



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

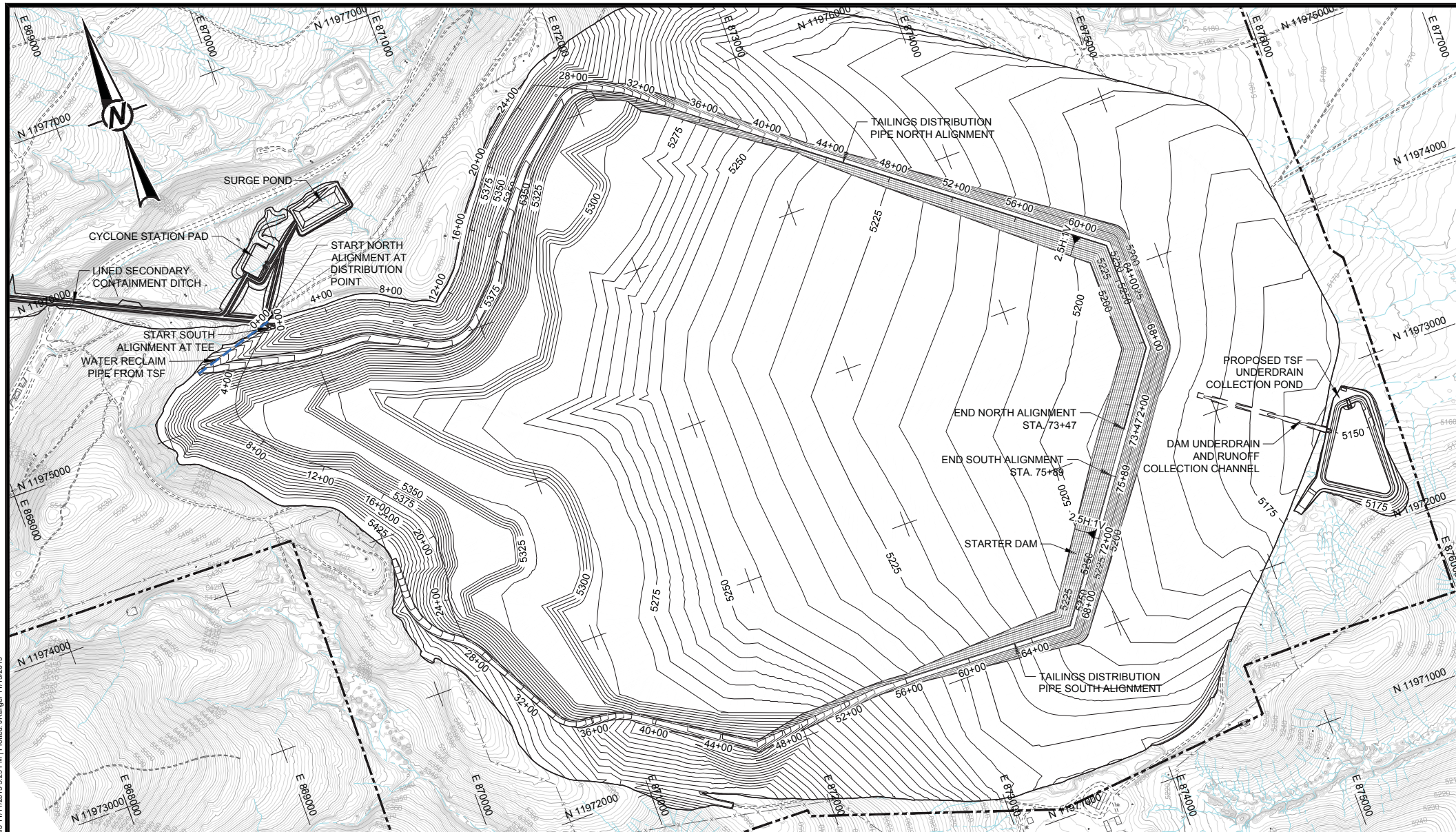
PROJECT
THEMAC RESOURCES
NEW MEXICO COPPER CORPORATION
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PROJECT COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN

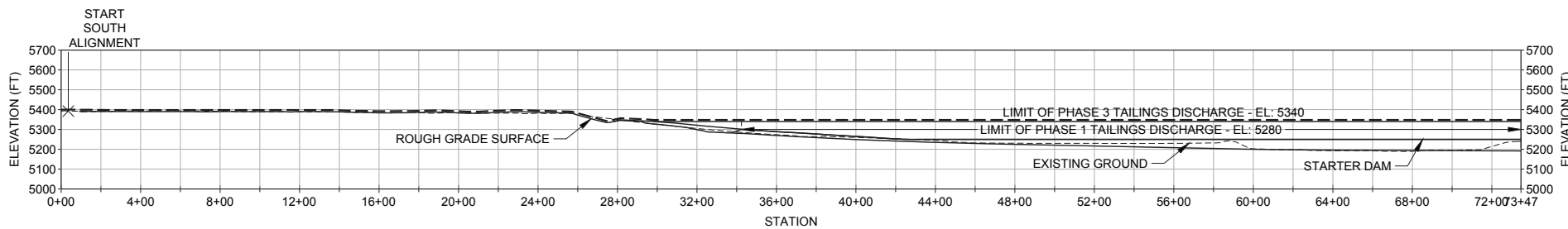
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DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
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REVIEW	DAK	2013-07-17	



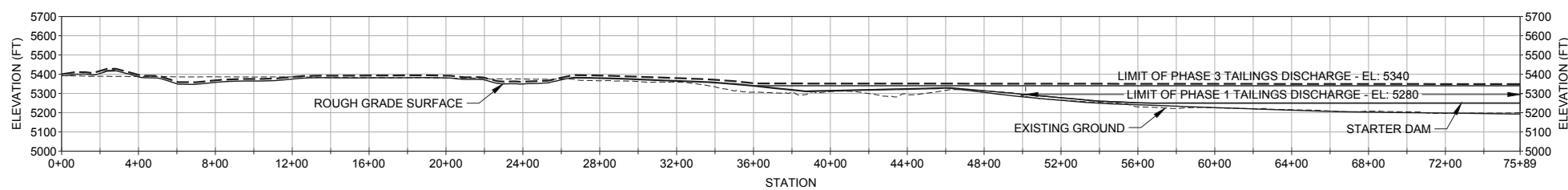


PHASES 1-3 TAILINGS DISTRIBUTION PLAN
SCALE A

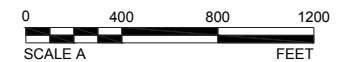
- LEGEND**
- EXISTING GROUND CONTOUR (ft -MSL)
 - EXISTING ROADS
 - EXISTING DRAINAGE
 - EXISTING FENCELINE
 - MINE PERMIT AREA BOUNDARY
 - REGRADED CONTOURS (ft -MSL)
 - GRADE BREAK
 - SLOPE INDICATOR
 - 3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
 - GRADE INDICATOR
 - TAILINGS DELIVERY AND DISTRIBUTION PIPE AT PHASE 3 LIMITS
 - WATER RECLAIM PIPE



PHASES 1-3 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE
SCALE A



PHASES 1-3 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE
SCALE A



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE
PRELIMINARY
FOR AGENCY REVIEW

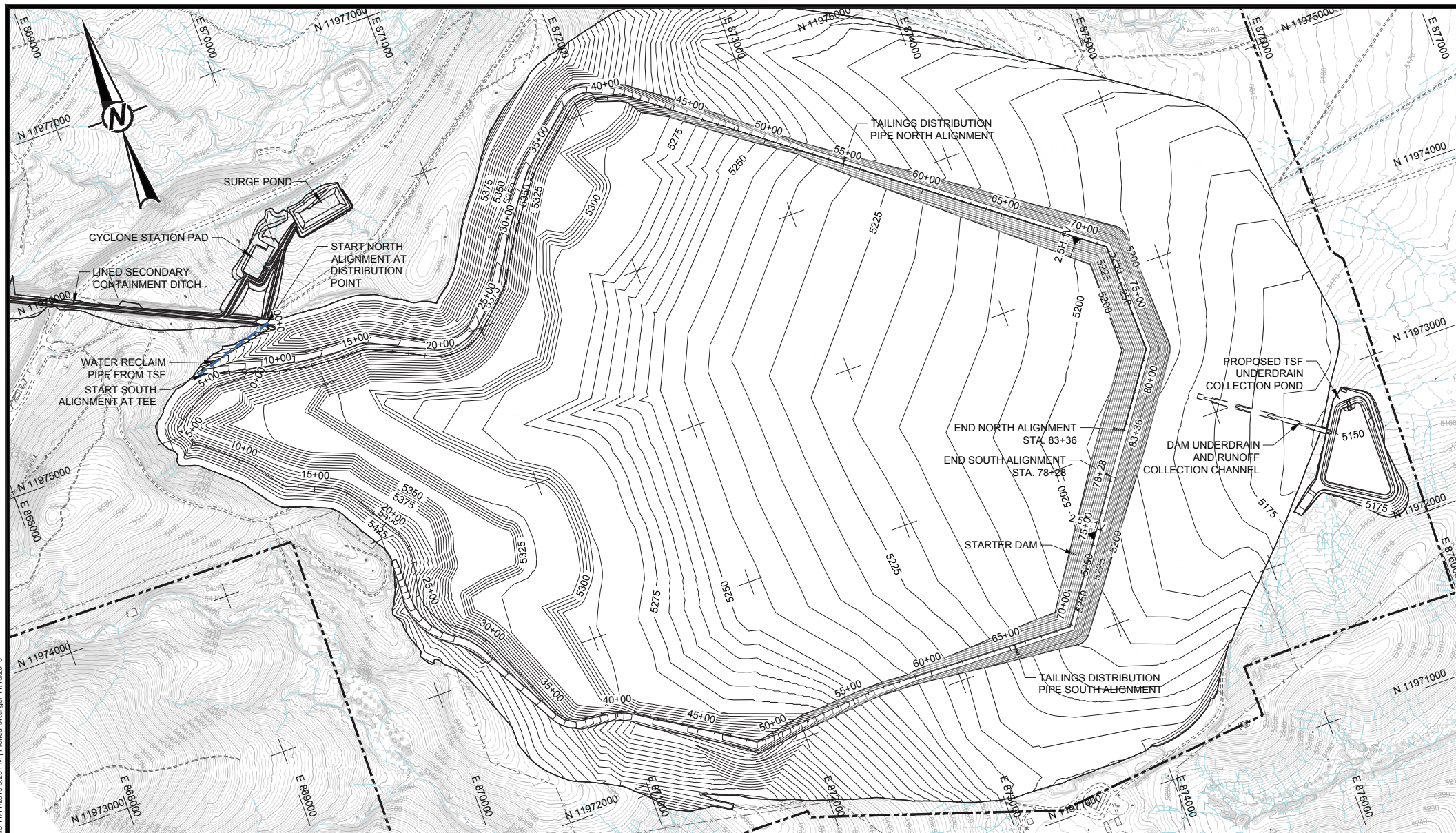
PROJECT
THEMAC RESOURCES
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COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)

PROJECT No.	103-92557	FILE No.	10392557K017
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REVIEW	DAK	2013-07-17	

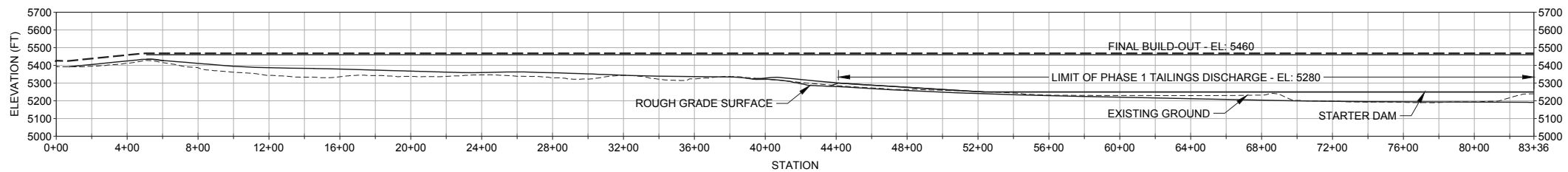
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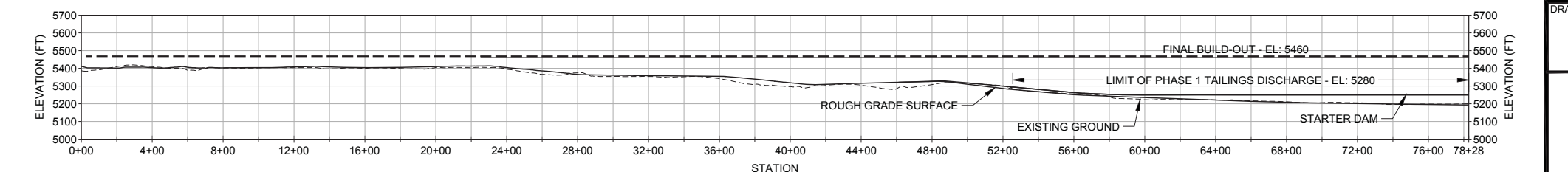
PHASES 4-5 TAILINGS DISTRIBUTION PLAN
SCALE A

LEGEND

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- TAILINGS DELIVERY AND DISTRIBUTION PIPE AT FINAL BUILD-OUT
- WATER RECLAIM PIPE



PHASES 4-5 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE
SCALE A



PHASES 4-5 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE
SCALE A



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
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2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

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TITLE

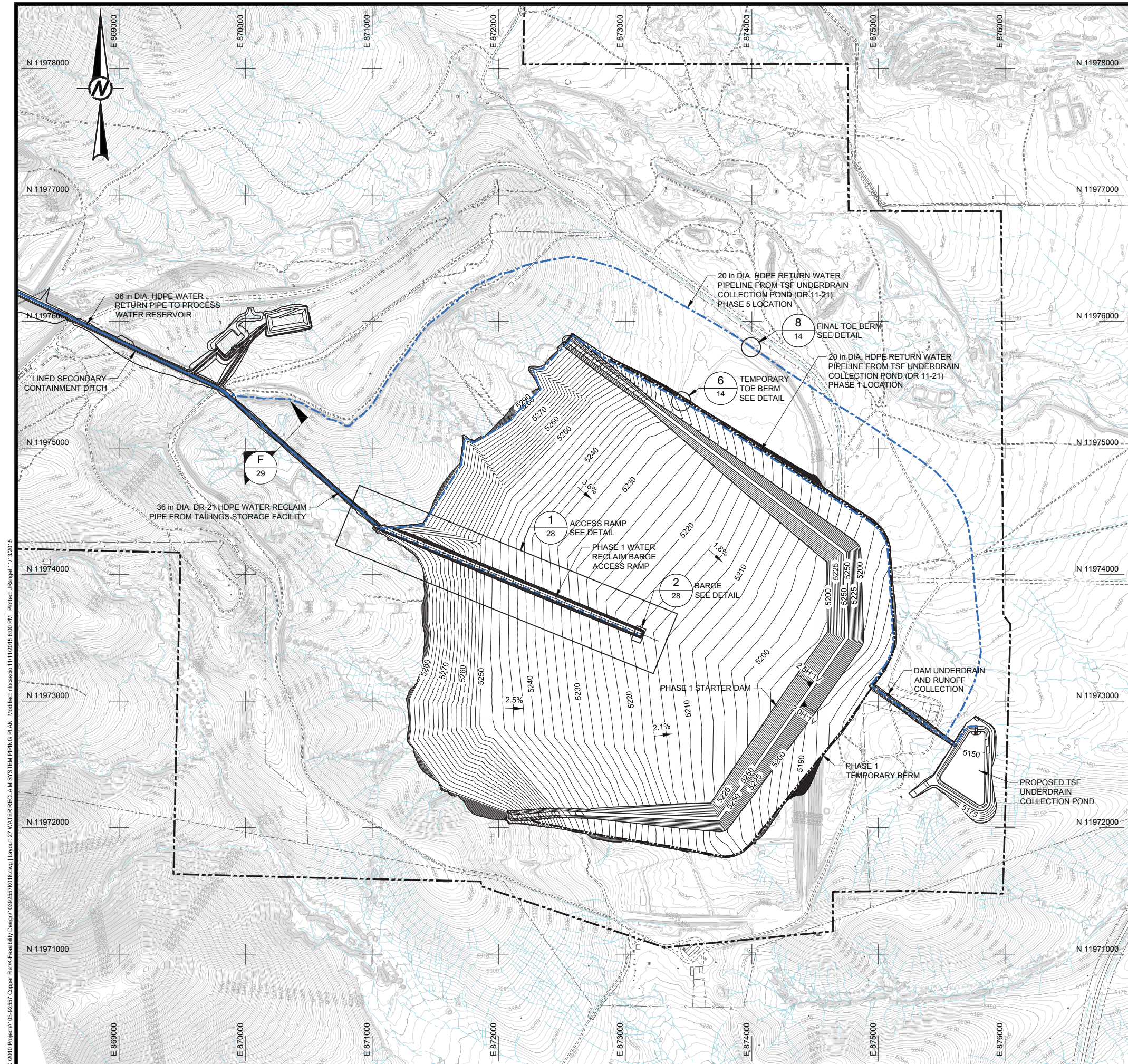
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REVIEW	DAK	2013-07-17	

26



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LEGEND

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- INTERMEDIATE TOE BERM CENTERLINE
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1 14 DETAIL CALLOUT
DETAIL ID
DRAWING SHEET LOCATION

- NOTES**
- SEE DRAWING 29 FOR RETURN WATER PIPE DITCH SECTIONS AND DETAILS.
 - RETURN WATER PIPELINE FROM TSF UNDERDRAIN AND RUNOFF COLLECTION POND WILL BE ROUTED INSIDE THE EMBANKMENT TOE BERM AND WILL REQUIRE RELOCATION AT EACH CONSTRUCTION PHASE.
 - RETURN WATER PIPELINE WILL BE RELOCATED TO THE TOE BERM AT EACH PHASE OF BUILDOUT. PHASE 1 AND PHASE 5 LOCATIONS ARE SHOWN ON DRAWING.

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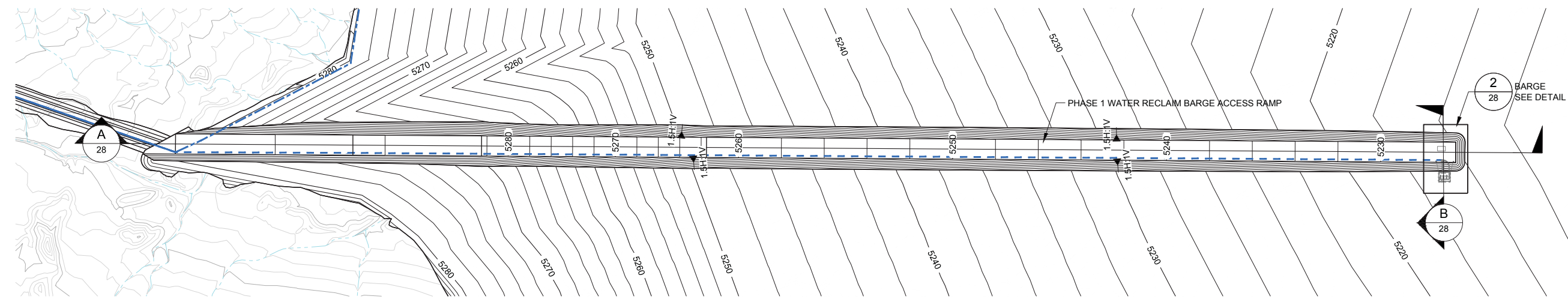
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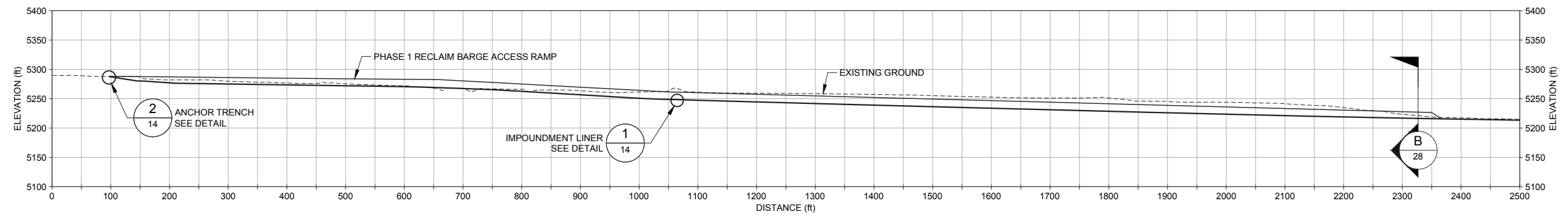
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
WATER RECLAIM SYSTEM PIPING PLAN

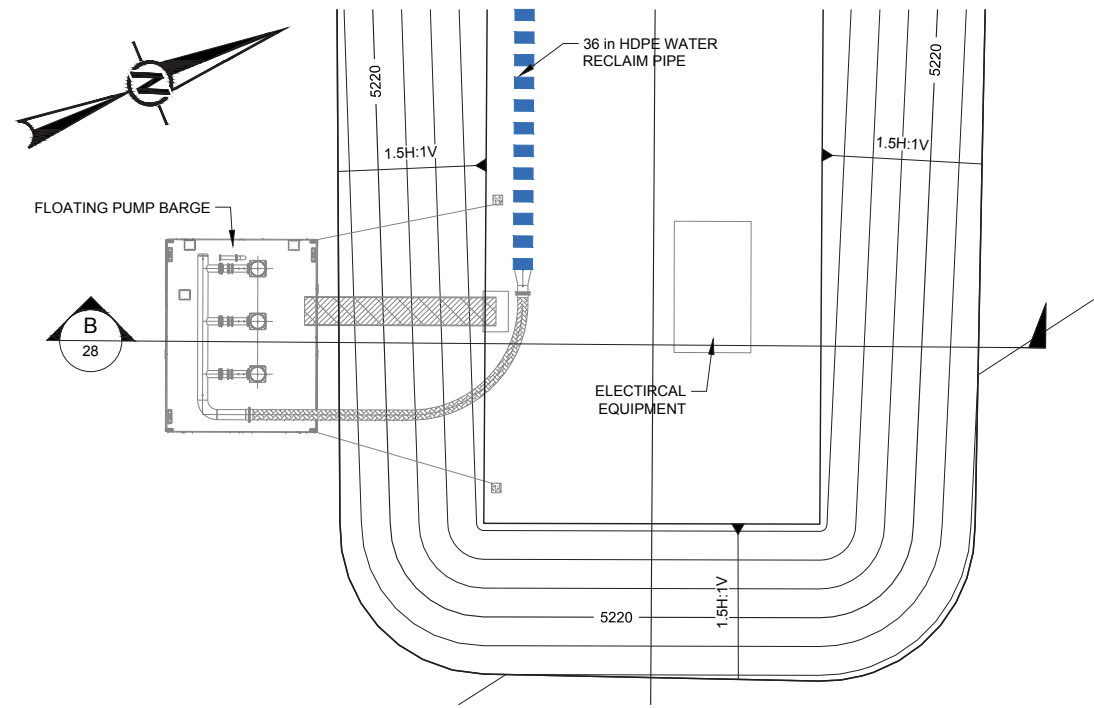
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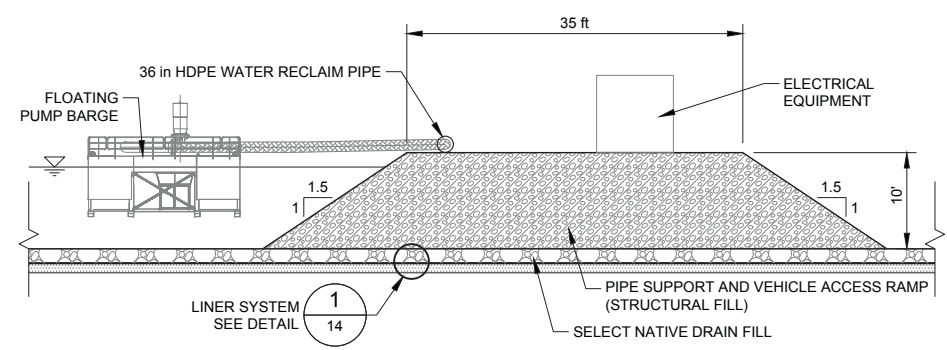
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28



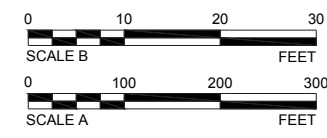
SCALE A **A** PIPE RAMP SECTION
28



SCALE B **2** RECLAIM BARGE DETAIL
28



N.T.S. **B** RECLAIM BARGE SECTION
28



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2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

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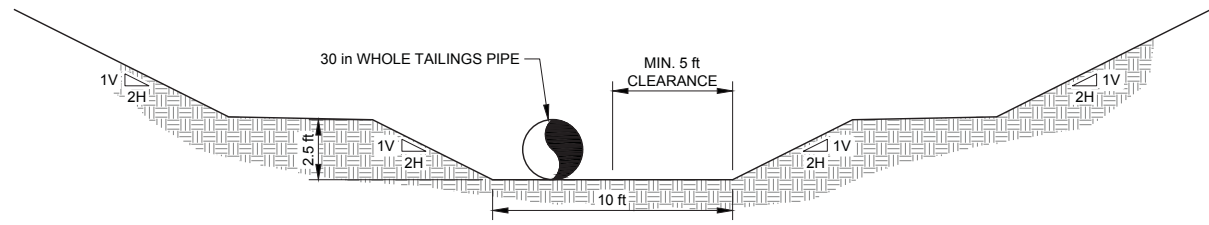
COPPER FLAT PROJECT
30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE			
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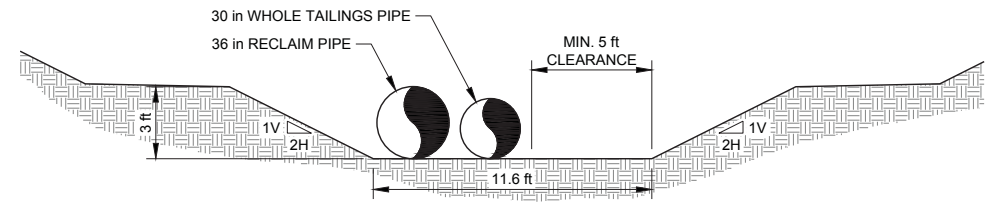


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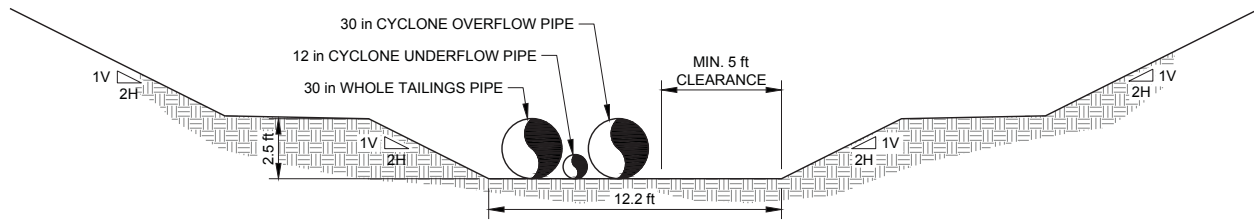
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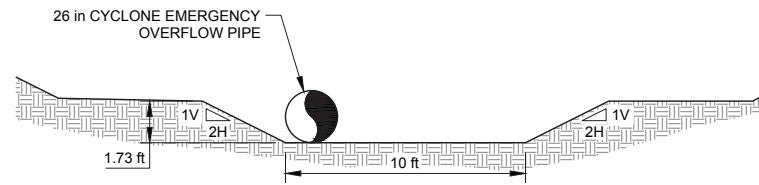
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29
SECONDARY CONTAINMENT MAIN DITCH ABOVE PROCESS WATER RESEVOIR



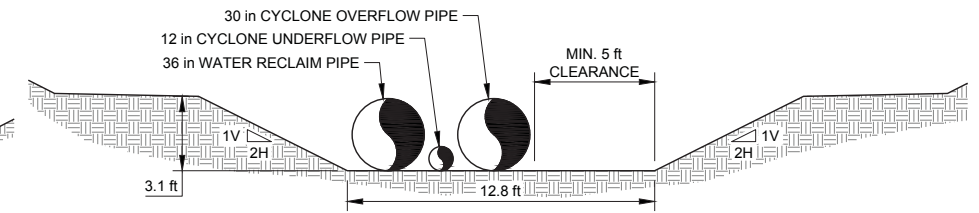
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MAIN DITCH BELOW PROCESS WATER RESEVOIR



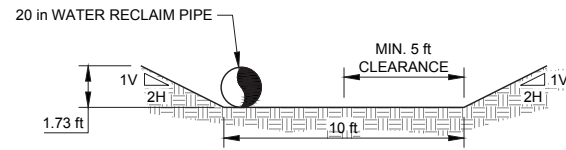
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MAIN DITCH TO CYCLONE PLANT



SCALE: N.T.S. **D**
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CYCLONE PLANT TO SURGE POND DITCH



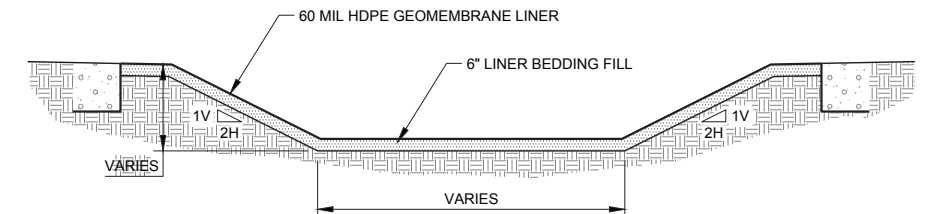
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MAIN DITCH BELOW CYCLONE PLANT



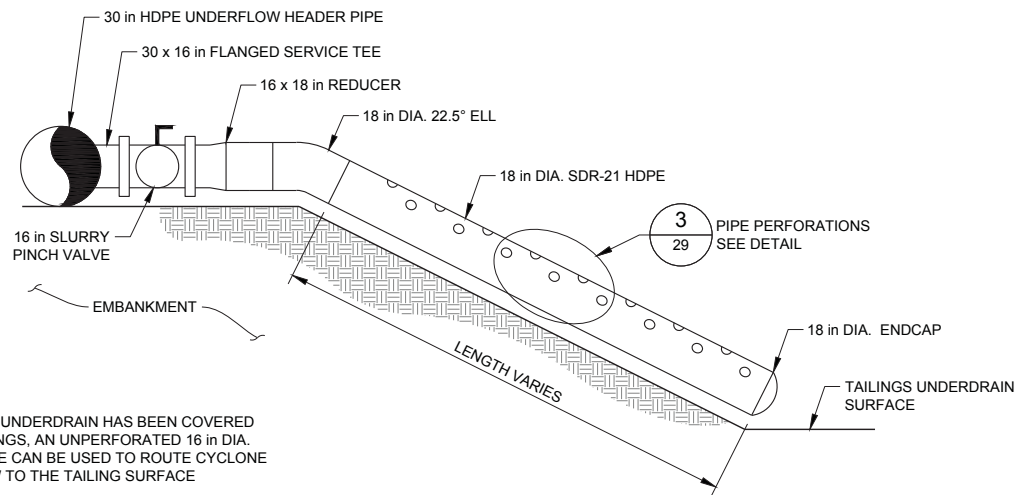
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IN TSF RECLAIM PIPE DITCH



SCALE: N.T.S. **G**
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POST YEAR 6 SURGE POND DITCH

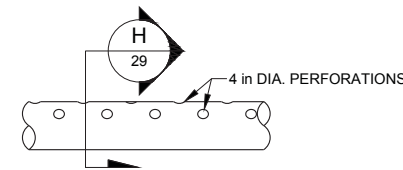


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TYPICAL SECONDARY CONTAINMENT LINER

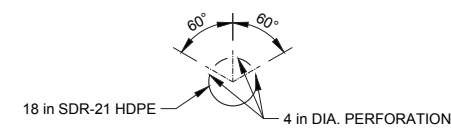


NOTE:
WHEN THE UNDERDRAIN HAS BEEN COVERED WITH TAILINGS, AN UNPERFORATED 16 in DIA. SDR-32 PIPE CAN BE USED TO ROUTE CYCLONE OVERFLOW TO THE TAILING SURFACE

SCALE: N.T.S. **2**
29
ENERGY DISSIPATOR/SCOUR PROTECTION CYCLONE OVERFLOW DISTRIBUTION SYSTEM



SCALE: N.T.S. **3**
29
PIPE PERFORATION DETAIL



SCALE: N.T.S. **H**
29
PIPE PERFORATION SECTION

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REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

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30K TPD TAILINGS STORAGE FACILITY
FEASIBILITY DESIGN
SIERRA COUNTY, NEW MEXICO

TITLE
TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS

PROJECT No.	103-92557	FILE No.	10392557K022
DESIGN	DMW	2013-04-08	SCALE AS SHOWN
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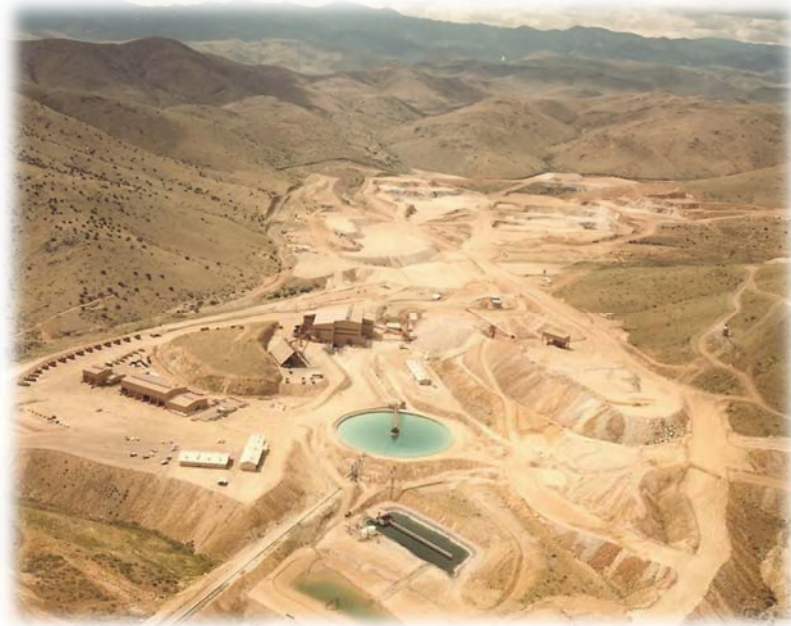
Golder Associates Inc.
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Albuquerque, NM 87113 USA
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Fax: (505) 821-5273



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Copper Flat Project



Impoundment Design Report

Prepared For:

THEMAC
RESOURCES 

Certified Professional Engineer Seal

This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.

Harry Lewsley
Signature



Exp. 12/31/2017
Date 12/7/2015

IMPOUNDMENT DESIGN REPORT
COPPER FLAT PROJECT

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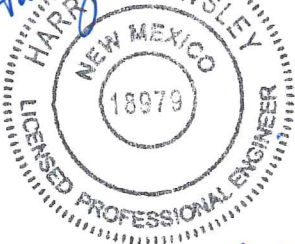
Harry J. Lewisley 12/7/2015
HARRY J. LEWSLEY
NEW MEXICO
18979
LICENSED PROFESSIONAL ENGINEER
Exp. 12/31/2017

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Harry J. Lewsley 12/7/2015

Exp. 12/31/2017

1 INTRODUCTION

The Copper Flat Project is located in South Central New Mexico, near the town of Hillsboro, approximately 150 miles south of Albuquerque, and approximately 20 miles southwest of Truth or Consequences (straight-line distances) (Figure 1). The Project is owned and operated by New Mexico Copper Corporation (NMCC), a wholly owned subsidiary of THEMAC Resources Group Limited.

The State of New Mexico has promulgated regulations pertaining to groundwater protection at copper mining facilities (New Mexico Administrative Code Title 20, Chapter 6, Part 7 [20.6.7 NMAC], the "Copper Rule"), the stated purpose of which is "to control discharges of water contaminants specific to copper mine facilities and their operations to prevent water pollution."

This report provides the design criteria, location, purpose, operation, and performance of certain elements of the project identified in Section 2 of this report to comply with 20.6.7 NMAC. This report excludes the design considerations for the Tailings Storage Facility (TSF), i.e., the tailings impoundment, underdrain collection pond, surge pond and the secondary containment trench from the processing facility to the TSF, which have been completed by others and are reported separately.

2 SYSTEM DESCRIPTIONS

Impacted Stormwater Impoundments are designed to receive surface drainage that potentially has come in contact with water contaminants on a copper mine facility. These systems consist of a network of diversion channels designed to convey to the impoundment at minimum the peak from a 100-year-return-interval storm with at least 6 inches of freeboard per 20.6.7.17.D.(2).(f). The Impacted Stormwater Impoundments are designed to store impacted stormwater for less than 30 days and include an engineered liner system, as described in Section 3.2.

The Process Water Reservoir is designed to receive reclaimed process water from a variety of sources including the TSF, impacted stormwater impoundments and freshwater supply system conveyed via pipelines. The reservoir also receives direct precipitation to the pond surface and embankment crest area. The Process Water Reservoir is designed with an engineered liner system, leak collection system, and subgrade bedding, as described in Section 3.3.

3 BASIS OF DESIGN

3.1 GENERAL

All impoundments for the Copper Flat Project will be considered "new" impoundments as defined by NMAC 20.6.7.17 (D).

Outside Slopes	20.6.7.17.D.(1).(a) NMAC	Maximum 2:1 (H:V)
Static factor of safety	20.6.7.17.D.(1).(a) NMAC	Minimum 1.3
Liner Sidewall seams	20.6.7.17.D.(1).(e) NMAC	Vertical only
Capacity	20.6.7.17.D.(2) NMAC	Contain 100-year return interval storm event plus minimum 2 ft of freeboard

3.2 IMPACTED STORMWATER IMPOUNDMENTS

Impacted stormwater impoundments are designed to hold impacted stormwater for less than 30 days in accordance with NMAC 20.6.7.17 (D) (4) and (7).

Liner system	20.6.7.17.D.(4).(a) NMAC	Compacted minimum 6-inch subbase overlain by 60 mil HDPE liner system
Wind protection	20.6.7.17.D.(4).(d) NMAC	Weighting system to limit liner damage in high winds
Spillway design	20.6.7.17.D.(7) NMAC	Safely discharge peak flow from 24-hour storm event with 25-year return

3.3 PROCESS WATER RESERVOIR

Process Water Impoundments/Reservoirs are designed to hold process water at design capacity plus impacted stormwater for more than 30 days in accordance with NMAC 20.6.7.17 (D) (3).

Liner system	20.6.7.17.D.(3).(a) and 20.6.7.17.D.(3).(c) NMAC	Primary 60 mil HDPE liner over a secondary 60 mil HDPE liner with drainage layer over a compacted minimum 6-inch subbase
Leakage collection system	20.6.7.17.D.(3).(d) NMAC	Drainage layer between primary and secondary liners with fluid removal system
Drainage layer	20.6.7.17.D.(3).(d) NMAC	Granular soil material or geosynthetic drainage net
Drainage layer slope	20.6.7.17.D.(3).(d) NMAC	At least 2 percent
Drainage layer permeability	20.6.7.17.D.(3).(d) NMAC	At least 1×10^{-2} centimeters per second (cm/s)
Collection sump	20.6.7.17.D.(3).(d) NMAC	At confluence drainage layer with dedicated automatic pump system with totalizing flow meter and automated failure alarm system
Spillway design	20.6.7.17.D.(7) NMAC	No discharge to ground surface, safely discharge peak process flows

4 DESIGN AND CONSTRUCTION OF IMPACTED STORMWATER IMPOUNDMENTS AND PROCESS WATER RESERVOIR

The Copper Flat Project permit boundary (Figure 2) incorporates the mine pit, processing plant area, waste rock stockpiles (WRSPs), and the TSF. The TSF and related facilities including the cyclone plant, surge pond, and underdrain collection pond are described by others. The mine and process plant area includes five developed watershed (WS) areas (Figure 3) that are managed as part of this plan. The facilities described below are designed to manage process and impacted stormwater to prevent releases from the site to surface water and groundwater (Figure 4).

4.1 FACILITIES

WS A includes the process plant, maintenance, and administrative areas of the Copper Flat Project. It also includes WS E, which is the Process Water Reservoir as a separate, internal area of stormwater and process water management (Sec. 4.1.5). During precipitation events, sheet flow of stormwater is directed (Figure 5) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment A (Figures 6 and 7). The impoundment is designed with a spillway that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment A are presented in Table 1.

WS B includes runoff from the western flank of Animas Peak and proposed new waste rock stockpiles (WRSP-1) (Figure 3). During precipitation events, sheet flow of stormwater is directed (Figure 3) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment B (Figures 8 and 9). The impoundment is designed with a spillway to the mine pit that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment B are presented in Table 1.

WS C includes runoff from the eastern flank of Animas Peak and proposed new waste rock stockpiles (WRSP-2 and 3) (Figure 3). During precipitation events, sheet flow of stormwater is directed (Figure 3) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment C (Figures 10 and 11). The impoundment is designed with a spillway that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment C are presented in Table 1.

WS D includes runoff from uphill slopes and existing waste rock stockpiles (EWRSP-1 and -2b) to the mine pit (Figure 3). During precipitation events, sheet flow of stormwater is directed by natural drainage channels and open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to the mine pit.

WS E consists of direct precipitation onto the lined surfaces of the Process Water Reservoir and unlined perimeter road that is directed to the reservoir (Figure 3). The amount of direct precipitation to the pond is small (9.5 cubic feet per second [cfs]) in comparison to the design throughput of process solutions through the pond (100,000 cfs). The design freeboard of 2 ft is more than adequate to handle the additional flux from a precipitation event. The design capacity of the pond is 726,400 cubic feet (ft³) with 2 ft of freeboard and the ultimate capacity is 938,000 ft³ (Figures 12 and 13). Overtopping of the reservoir is controlled by an alarm system and emergency shutoff system. Overtopping flows, in the event of system failure, are directed to the lined tailings conveyance trench to the lined tailings impoundment. Design criteria for the Process Water Reservoir are presented in Table 1.

4.2 SURFACE WATER CONTROL

Surface areas draining to the Impacted Stormwater Impoundments (A, B, and C), mine pit, and Process Water Reservoir will be shielded from run-on surface drainage by site diversions as described in a separate report.

4.3 GEOLOGIC HAZARDS

No geologic hazards are known to exist in the vicinities of the Impacted Stormwater Impoundments or Process Water Reservoir. Impacted Stormwater Impoundment B is located on the eastern wall of the ultimate mine pit (Figure 11J-3). In the event of a pit slope failure, any liquids contained in Impacted Stormwater Impoundment B would be contained in the mine pit.

4.4 SOLUTION CHARACTERIZATION

Liquids routinely expected to enter the Impacted Stormwater Impoundments (A, B, and C) are direct precipitation and stormwater runoff from areas impacted by mining activities including mining, hauling, waste rock stockpiling, mineral processing, and shipping and receiving of goods and products. The Impacted Stormwater Impoundments will be typically empty. Impacted stormwater collected in the impoundments will be pumped out and used as process makeup water.

Liquids routinely expected to enter the Process Water Reservoir include direct precipitation, water reclaimed from the Copper-Moly (Cu-Mo) Thickener, fresh make-up water from the water supply wellfield, and reclaimed water from the Tailings Impoundment and Underdrain Collection Pond. The Process Water Reservoir is typically maintained at a nearly full operational level at all times to ensure continuity of the process during short-term interruptions of return or makeup flows. The physical characteristics of these constituents are expected to be neutral to slightly alkaline and completely compatible with the liner materials. Flows from upset conditions in the concentrator do not flow directly to the Process Water Reservoir, but would eventually contribute to the water reclaimed from the Tailings Impoundment and Underdrain Collection Pond.

4.5 CAPACITY AND STORAGE DESIGN

The capacity and storage design of the subject impoundments and reservoir are provided in Table 1. The impacted water impoundments are designed to contain the runoff from a 100-year, 24-hour storm event with a minimum of 2 ft of freeboard.

The Process Water Reservoir is designed to contain the maximum design process flow plus stormwater runoff from the reservoir catchment area with a minimum of 2 ft of freeboard.

4.6 SPILLWAY DESIGN

Spillways for Impacted Stormwater Impoundments A, B, and C are designed to safely discharge the peak runoff of a 25-year, 24-hour precipitation event. The spillways for Impacted Stormwater Impoundments A and C are designed as open channel spillways with slopes that are suitable for vehicle access on the perimeter road. The spillway for Impacted Stormwater Impoundment B is designed as a culvert beneath the haul road. The culvert(s) will have sufficient capacity to safely pass peak runoff from the prescribed precipitation event.

Overflow protection for the process water reservoir is accomplished via a designed solution conveyance to the lined tailings conveyance trench, which conveys any upset flows that exceed the maximum capacity without compromising the integrity of the structure.

4.7 SITE PREPARATION

The pond areas will be cleared and grubbed of vegetation. Any unsuitable foundation materials within the pond footprint will be excavated and replaced. Bedding soil will be placed, moisture conditioned, and compacted pursuant to 20.6.7.17.D.(3) and (4). The bedding soil must be free of sharp rock, vegetation, and stubble to a depth of at least 6 inches. The bedding surface must be smooth to ensure good contact between the liner and the bedding. The liner must be placed on a layer of sand or fine soil. The floor of the bedding surface will be sloped to collection sump at grades of up to 1 percent to facilitate removal of the contents. Side slopes will be less than 2H:1V to permit proper installation of the liner system. The liner bedding shall have an acceptance certificate prior to installation of the liner.

4.8 LINER SYSTEMS

Pursuant to 20.6.7.17.D.(4), the liner system of the Impacted Stormwater Impoundments consists of a single 60 mil HDPE textured geomembrane liner that is certified as UV resistant in accordance with a Construction Quality Assurance and Construction Quality Control (CQA/CQC) Plan, which will be generated and approved prior to construction. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature. These impoundments are typically empty and the liner will be weighted to prevent wind damage. The liner shall be secured in an anchor trench (Figure 14, Detail 3).

Pursuant to 20.6.7.17.D. (3), the liner system for the Process Water Reservoir consists of a secondary liner, drainage layer, and primary liner. The drainage layer connects directly to the fluid collection sump and fluid removal system to alleviate the need for fluid collection pipes. This reservoir typically contains solution and will not require the liner to be weighted unless there is a prolonged period when the reservoir will be empty and susceptible to wind damage. The liner system shall be secured in an anchor trench (Figure 14, Detail 1).

The lower (secondary) liner consists of a single 60 mil HDPE geomembrane AGRU® drainage liner, or equivalent, that is installed in accordance with an approved CQA/CQC Plan. This type of secondary liner, paired with a primary liner, doubles as a drainage layer with a coefficient of permeability of 1×10^{-2} cm/s on a design slope of 2 percent. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature.

The primary liner for the Process Water Reservoir consists of a single 60 mil HDPE textured geomembrane liner that is certified as UV resistant and installed in accordance with an approved CQA/CQC Plan. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature.

A CQA/CQC plan will be developed by the design engineer and the liner installation contractor and for approval by the appropriate agency as part of the final design prior to construction. The plan includes the following elements.

- Identification of persons and entities responsible for overseeing the program.
- Inspection protocols for subgrade, materials, placement, anchoring, welding, testing, and repairing.
- Identification of field and laboratory testing equipment and testing entities.
- Procedures for observing and testing liner, subgrade, bedding, etc.
- Verification protocol for manufacturer's QC testing.
- Procedures for reviewing results of testing and inspection.
- Corrective actions for material repair, subgrade and bedding deficiencies, weld testing failures, or other construction defects.
- Seaming procedures, qualification, testing, and inspection.
- QA/QC reporting procedures, schedules, and certifications.

- Guidelines, schedules, contents, and certifications for submission of a CQA/CQC report.

4.9 LEAK COLLECTION SYSTEM

Pursuant to 20.6.7.17.D.(3).(d), the liner drainage layer of the process water reservoir discharges directly into a leakage collection sump (Figure 14, Detail 6) which is part of the fluid removal system. The sump contains granular fill materials to convey the drainage fluid to the fluid removal pipe and pump system. The fluid removal pipe consists of a 6" Sch. 80 polyvinyl chloride (PVC) pipe with 3 ft of slotted screen at the bottom for water collection. The fluid removal pipe can be cleaned using conventional pipe cleaning equipment. An automated fluid removal pump is installed at the bottom of the pipe to enable removal of leakage. The pump is activated in the presence of drainage fluid in the sump and is turned off when the fluid has been removed. A totalizing flow meter records the volume of fluid removed from the sump. The pump also has an alarm system to notify the operator of system failure.

4.10 PERFORMANCE INSPECTIONS AND OPERATIONAL MONITORING

Routine inspections of the Process Water Reservoir and Impacted Stormwater Impoundments begin at the time of construction and proceed quarterly. Additional inspections are prescribed in the event of a process upset or a significant stormwater flow event. Inspections include visual assessment of integrity and physical assessment of pond capacity. Water levels in the ponds are noted with respect to the freeboard. Totalizing meter readings are recorded from fluid removal pumps from the leakage collection sump and from the impacted stormwater impoundment pumps.

TABLES

Table 1: Impoundment Design Criteria

Impoundment ID	Stormwater Impoundment A	Stormwater Impoundment B	Stormwater Impoundment C	Process Water Reservoir
Catchment Area (ac)	91.06	98.52	198.66	1.80
Peak Flow, Q100 (cfs) ¹	200.25	176.88	315.76	9.54
Pond Size - Approx, Surface area (ac)	1.98	2.12	6.37	1.80
Freeboard Requirement (ft)	2.0	2.0	2.0	2.0
Capacity at Freeboard (ft ³)	976,800	748,400	1,405,500	726,400
Design Capacity at spillway/crest (ft ³)	1,280,500	913,200	1,802,100	938,000
Primary Liner Specifications ²	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent Certified UV resistant
Secondary Liner Specifications ²	N/A	N/A	N/A	60 mil HDPE or equivalent 6" fine soil subgrade
Drainage Layer Specifications ³	N/A	N/A	N/A	Geonet drainage layer Slope min. 2% Perm. min. 1 x 10 ⁻² cm/s
Perforated Fluid Collection System ⁴	N/A	N/A	N/A	Geonet drainage layer
Fluid Removal System ²	N/A	N/A	N/A	Automatic pump Totalizing flow meter Automated failure alarm
Design Flow for Conveyance Structures (cfs)	Q100 = 200.25	Q100 = 176.88	Q100 = 315.76	Q100 = 9.54
Design Storm for Pond & Source	100-yr, 24hr rainfall event, WS A	100-yr, 24hr rainfall event, WS B	100-yr, 24hr rainfall event, WS C	100-yr, 24hr rainfall event, WS E
Design Storm for Spillway & Source	200-yr, 24hr rainfall event, WS A	200-yr, 24hr rainfall event, WS B	200-yr, 24hr rainfall event, WS C	N/A
Peak Flow, Q200 for Spillway (cfs) ⁵	6.16	6.37	8.80	N/A
Bank Slopes	2:1 (H:V) Max	2:1 (H:V) Max	2:1 (H:V) Max	2:1 (H:V) Max

¹ Precipitation data is per NOAA Atlas 14, Volume 1, Version 5; Hillsboro, NM, Station ID: 29-4009

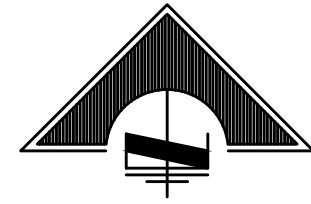
² Specifications are per 20.6.7.17.D.(3) and (4)(c)

³ Specifications are per 20.6.7.17.D.(3)(d)

⁴ Geonet layer drains directly into collection sump and fluid removal system

⁵ Design Flow for spillway is approximate flow from pond to spillway during the 200-yr event assuming the spillway elevation is at the 100-yr WSEL

FIGURES



NEW MEXICO COPPER CORPORATION

COPPER FLAT PROJECT SIERRA COUNTY, NEW MEXICO

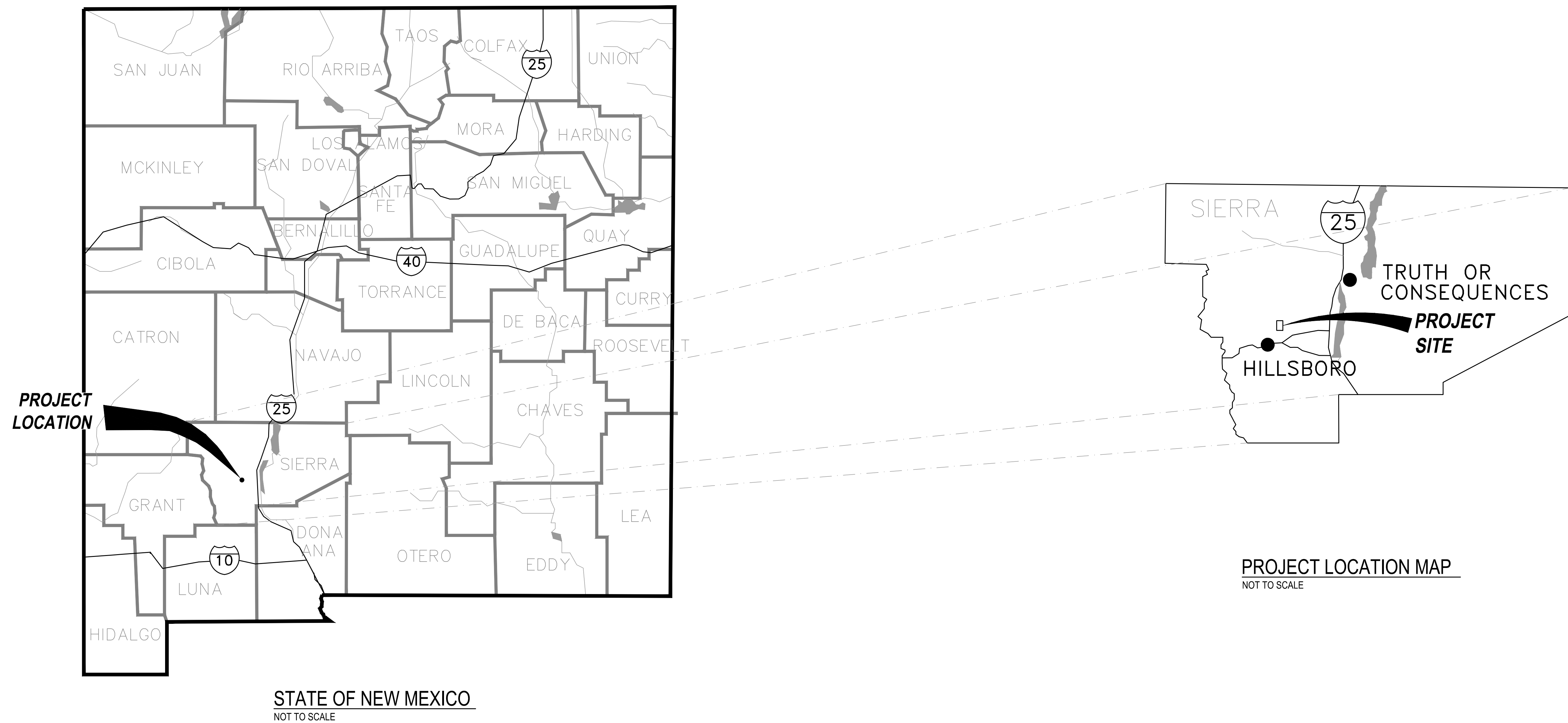


FIGURE 1

PRELIMINARY
FOR AGENCY REVIEW



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COPPER FLAT PROJECT		JOB NO. M3 PN-120085	
SITE GENERAL CIVIL PROJECT AREA SITE LOCATION PLAN		DWG. NO.	0000-CI-001
		REV. NO.	DATE
P 4	05 MAR 13		

File: P:\2013\20085\CIVIL (044)\544.2 - Design\Drawings for Permit\00000000.dwg LAST UPDATE: 12/22/2015 10:38 AM BY: JN699



FIGURE 2

EWRSP = EXISTING WASTE ROCK STOCKPILE
WRSP = WASTE ROCK STOCKPILE

SITE PLAN
 SCALE: 1:500

500' 250' 0' 500' 1000'
 1" = 500'
 DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
 FOR AGENCY REVIEW

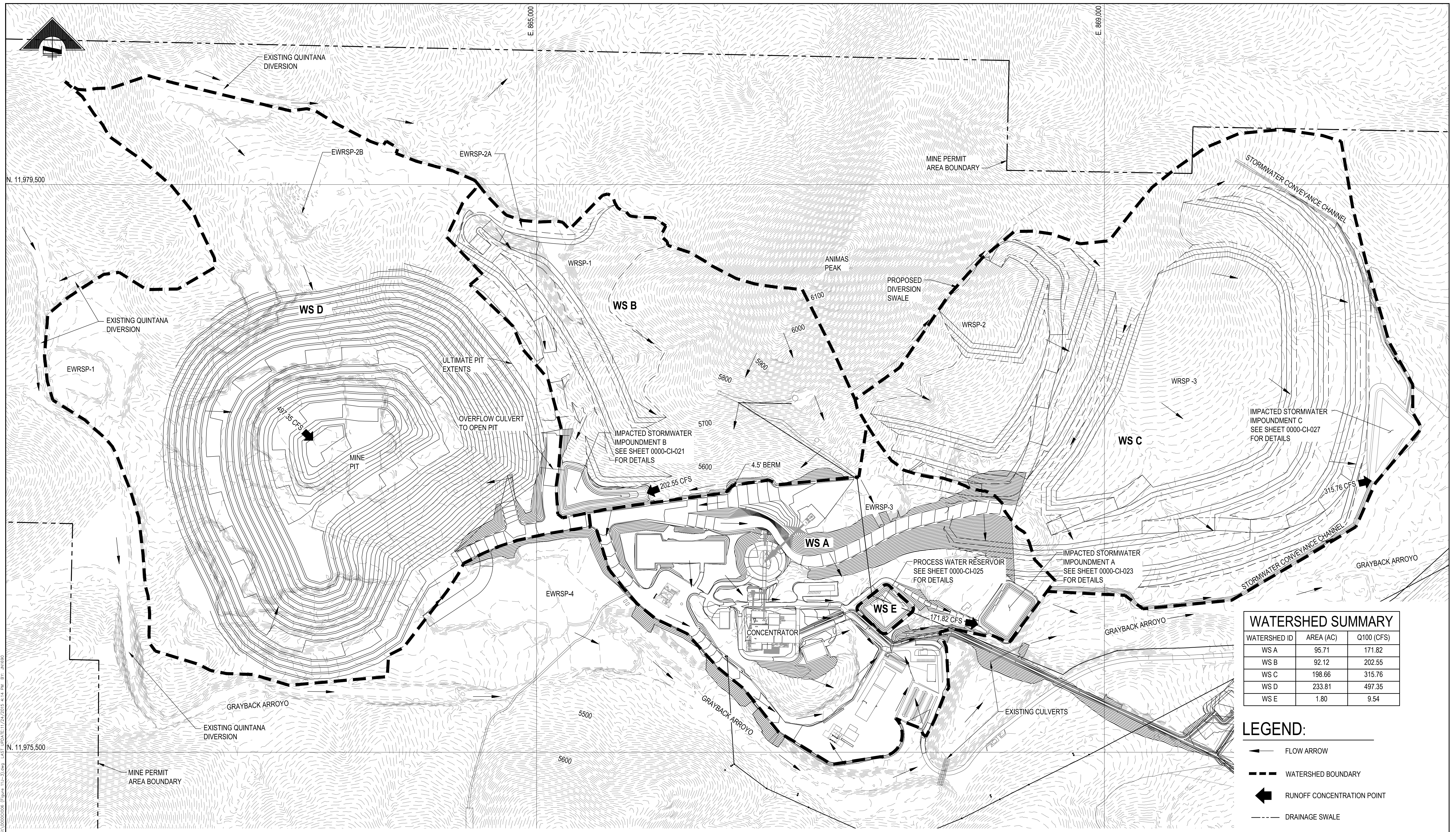


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COPPER FLAT PROJECT
SITE GENERAL CIVIL PROJECT AREA PROPOSED SITE PLAN
 JOB NO. M3 PN-120085
 DWG. NO. **FIGURE 11J-1**
 REV. NO. P18 DATE 16 NOV 15

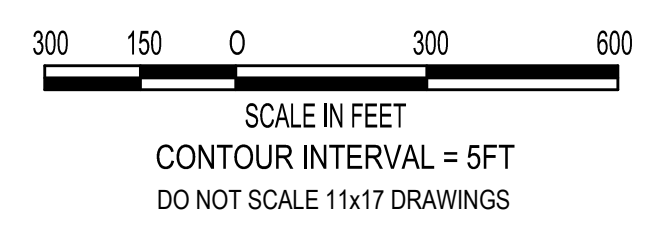
File: P:\2013\200805\Civil_0441544.2 - Design\Drawings for Permit\00000002 - Figure 11J-1.dwg, LAST UPDATED: 12/24/2015, 4:15 PM, BY: AN060



WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
WS A	95.71	171.82
WS B	92.12	202.55
WS C	198.66	315.76
WS D	233.81	497.35
WS E	1.80	9.54

- LEGEND:**
- FLOW ARROW
 - - - WATERSHED BOUNDARY
 - ◀ RUNOFF CONCENTRATION POINT
 - - - DRAINAGE SWALE

PLAN VIEW
SCALE: 1" = 300'



PRELIMINARY
FOR AGENCY REVIEW

FIGURE 3



EWRS = EXISTING WASTE ROCK STOCKPILE
WRSP = WASTE ROCK STOCKPILE

REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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0000-CI-023	IMPACTED STORMWATER IMPOUNDMENT B PLAN VIEW														
0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT D PLAN VIEW														

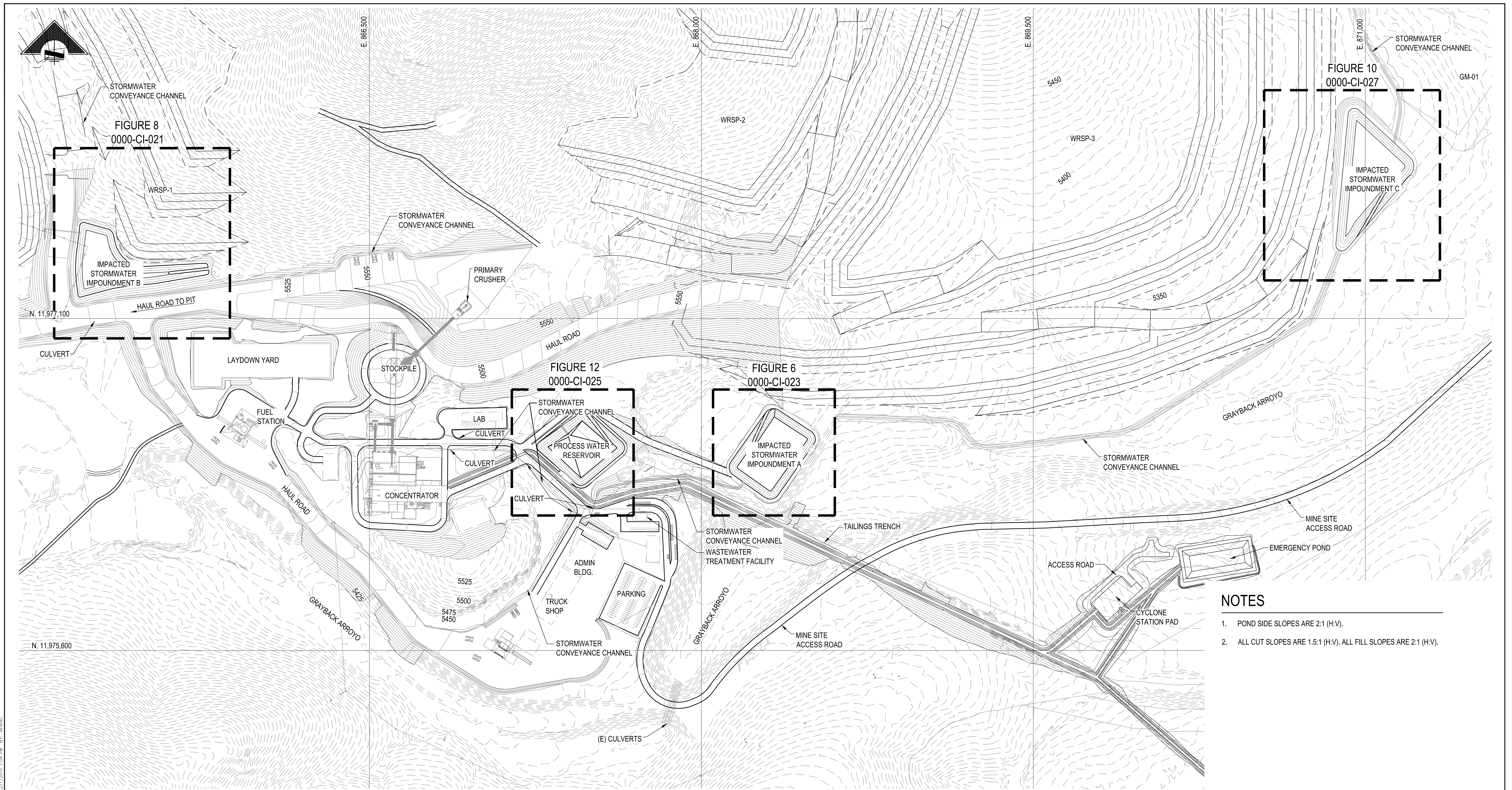
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GENERAL SITE CIVIL
MINE AREA
DEVELOPED WATERSHED AREAS

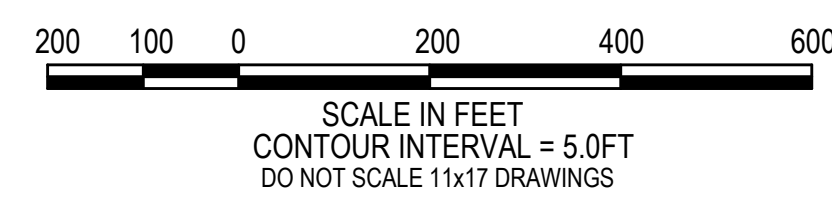
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DWG NO. **FIGURE 11J-3**
REV NO. P10 DATE 29 OCT 15

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SITE PLAN
SCALE: 1:200

- NOTES**
1. POND SIDE SLOPES ARE 2:1 (H:V).
 2. ALL CUT SLOPES ARE 1.5:1 (H:V); ALL FILL SLOPES ARE 2:1 (H:V).



PRELIMINARY
FOR AGENCY REVIEW

FIGURE 4



REFERENCES		REFERENCES		REVISIONS						REVISIONS					
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0000-CV-023	IMPACTED STORMWATER IMPOUNDMENT A, PLAN VIEW														
0000-CV-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CV-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW														

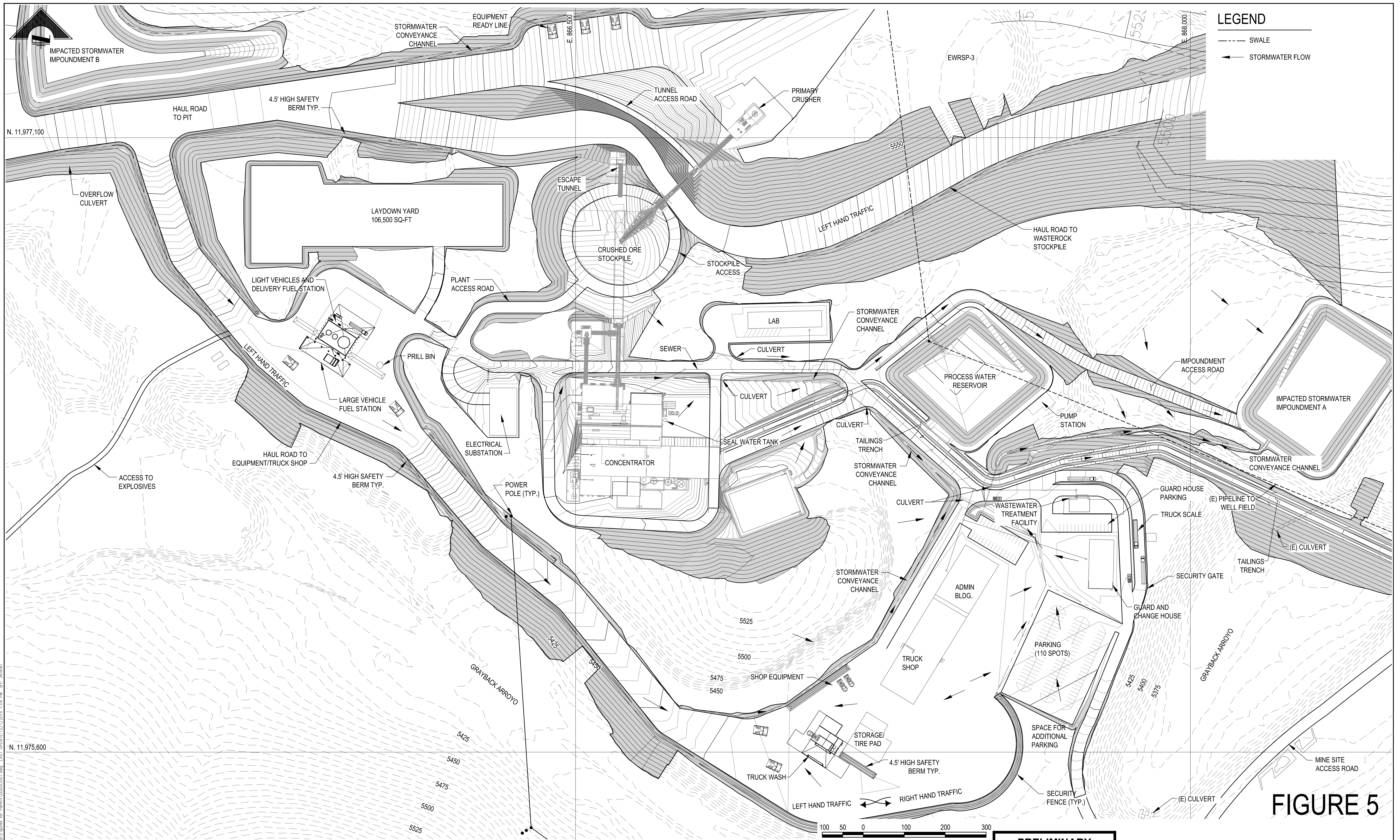
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SITE GENERAL CIVIL
STORMWATER & PROCESS WATER PONDS
OVERALL PLAN

JOB NO. M3 PN-120085
DWG. NO. **0000-CI-020**
REV. NO. P3 DATE 09 OCT 15

File: E:\2013\200805\Civil_0441\544.2_Dwg\00000200.dwg LAST UPDATE: 12/20/15 1:04 PM BY: JNK90



SITE PLAN
SCALE: 1:100

SCALE IN FEET
PROPOSED CONTOUR INTERVAL = 1.0FT
EXISTING CONTOUR INTERVAL = 5.0FT
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PRELIMINARY
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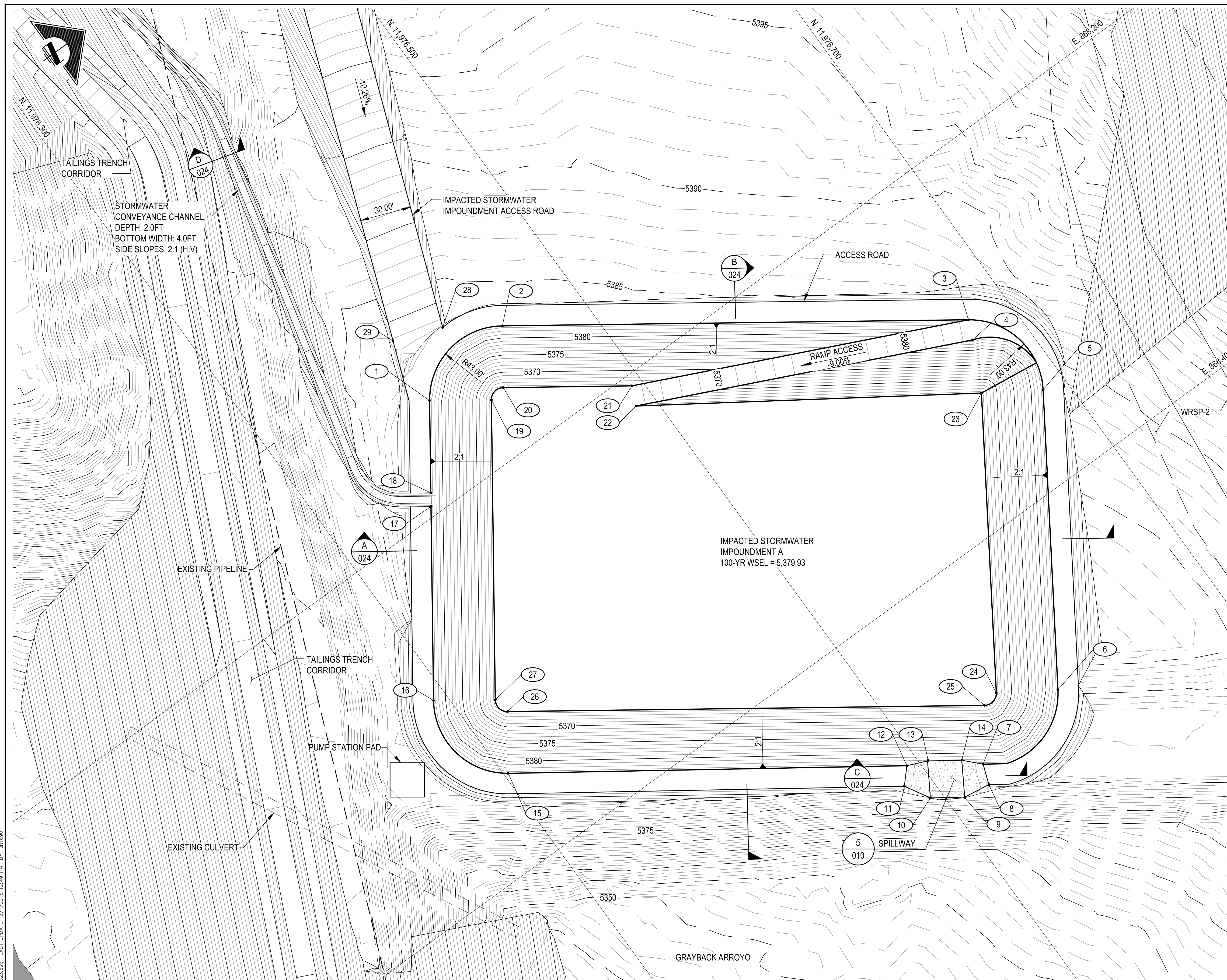


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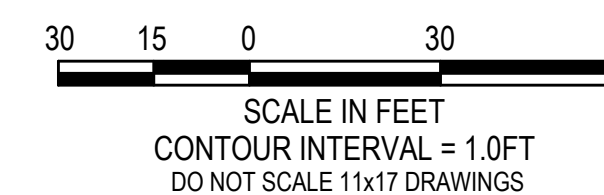
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	CONSTRUCTION MANAGEMENT	DRAWN BY SAM FEB 13
		CHECKED BY TDL FEB 13
		PROJECT MGR RKZ FEB 13
	CLIENT APPR.:	

COPPER FLAT PROJECT		JOB NO. M3 PN-12085
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		REV NO. DATE
		P9 09 OCT 15

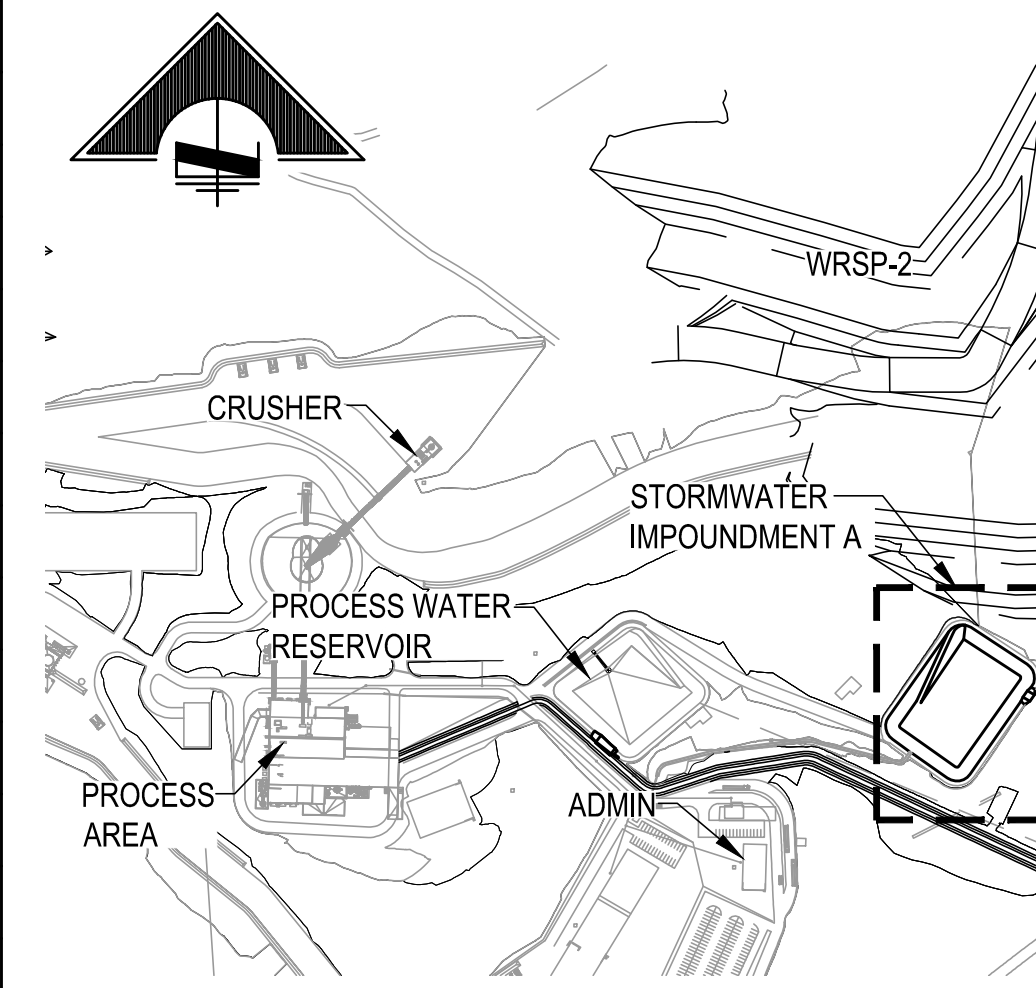
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PLAN
SCALE: 1" = 30'



POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION
1	11976393.13	868144.02	5383.50
2	11976453.06	868133.72	5383.50
3	11976675.02	868290.57	5383.50
4	11976670.08	868301.51	5383.50
5	11976686.19	868349.21	5383.50
6	11976590.39	868495.80	5383.50
7	11976529.56	868505.28	5383.50
8	11976525.25	868516.90	5383.50
9	11976509.58	868514.53	5381.96
10	11976493.13	868503.16	5381.96
11	11976484.86	868488.73	5383.50
12	11976493.10	868479.86	5383.50
13	11976504.45	868484.69	5382.18
14	11976520.85	868496.13	5382.18
15	11976302.43	868346.86	5383.50
16	11976292.13	868286.93	5383.50
17	11976357.47	868194.48	5383.50
18	11976362.08	868187.95	5383.50
19	11976422.53	868164.79	5365.50
20	11976432.28	868163.12	5365.50
21	11976493.65	868206.42	5365.50
22	11976488.60	868217.31	5365.50
23	11976656.01	868329.59	5365.50
24	11976500.26	868476.10	5365.50
25	11976550.36	868477.99	5365.50
26	11976323.21	868317.47	5365.50
27	11976321.53	868307.71	5365.50
28	11976424.35	868113.97	5383.50
29	11976396.34	868103.21	5383.50



KEY MAP
SCALE: 1" = 500'

NOTES:

1. STORMWATER IMPOUNDMENT A IS INTENDED TO CAPTURE STORMWATER RUNOFF FROM THE MINE SITE PROCESS AREA.
2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
3. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60ML HDPE LINER, PER DETAIL 4 ON SHEET 0000-CI-010, OR APPROVED EQUAL.
4. STORMWATER SPILLWAY IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT MINIMUM.
5. SPILLWAY IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

IMPOUNDMENT SUMMARY

	976,772	CU-FT
CAPACITY AT 100-YR WSEL		
ULTIMATE CAPACITY	1,280,516	CU-FT

FIGURE 6

PRELIMINARY
FOR AGENCY REVIEW



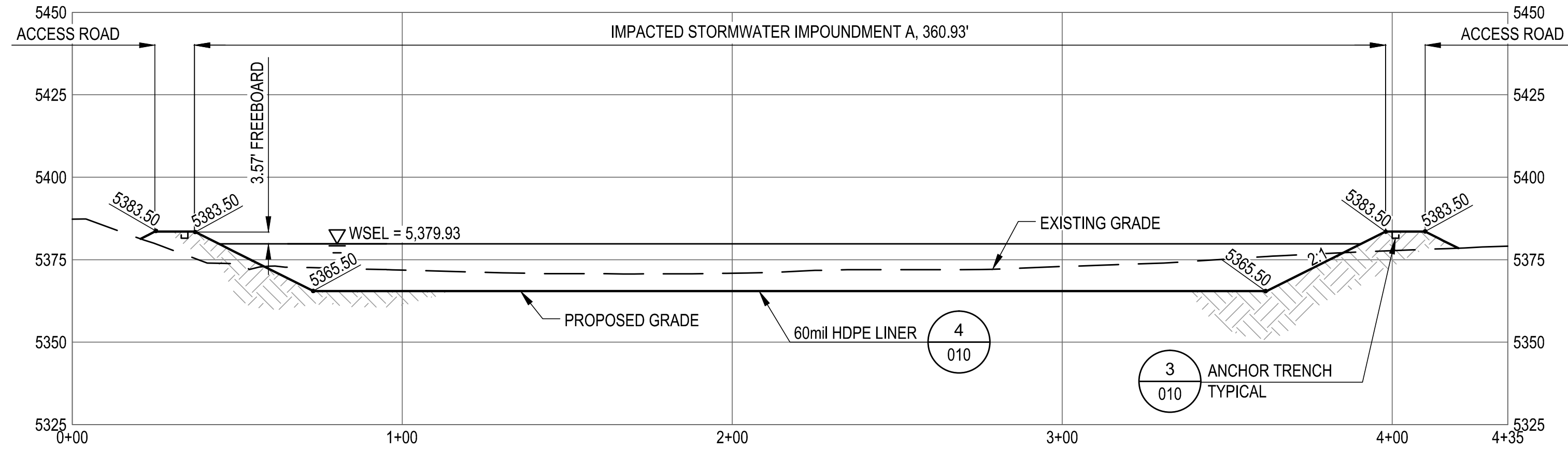
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IMPACTED STORMWATER IMPOUNDMENT A
PLAN VIEW

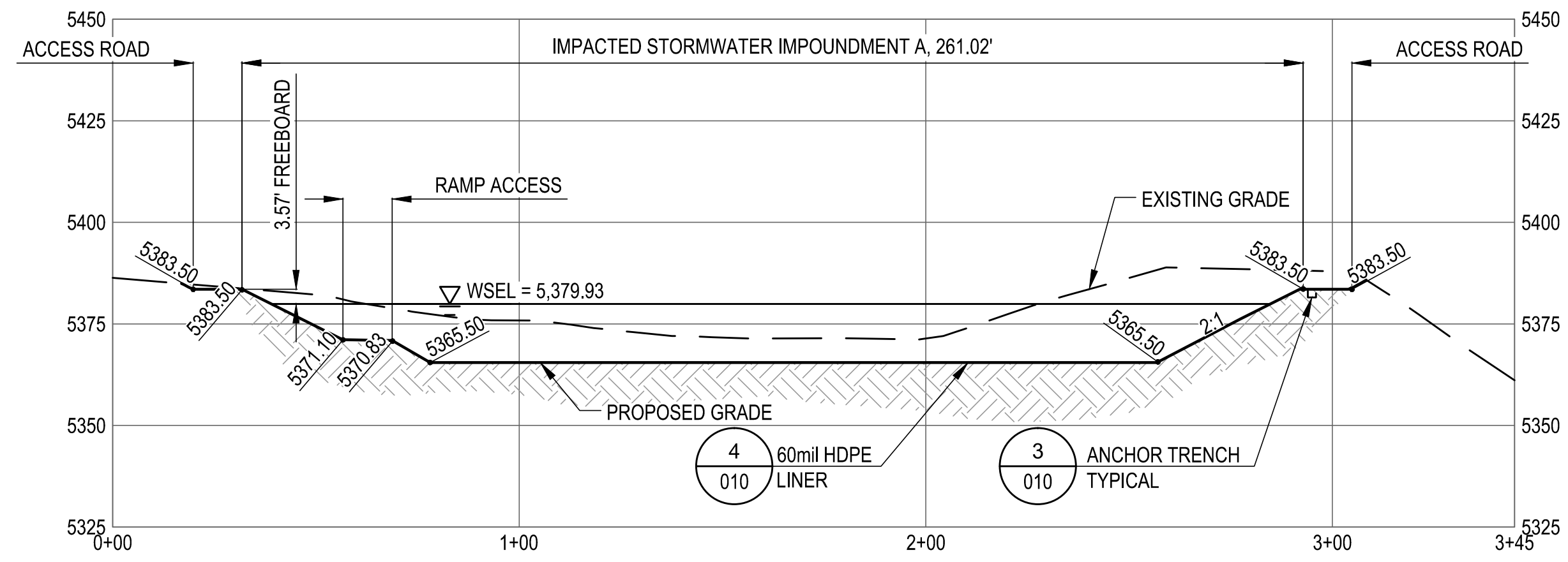
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REV NO. P3 DATE 09 OCT 15

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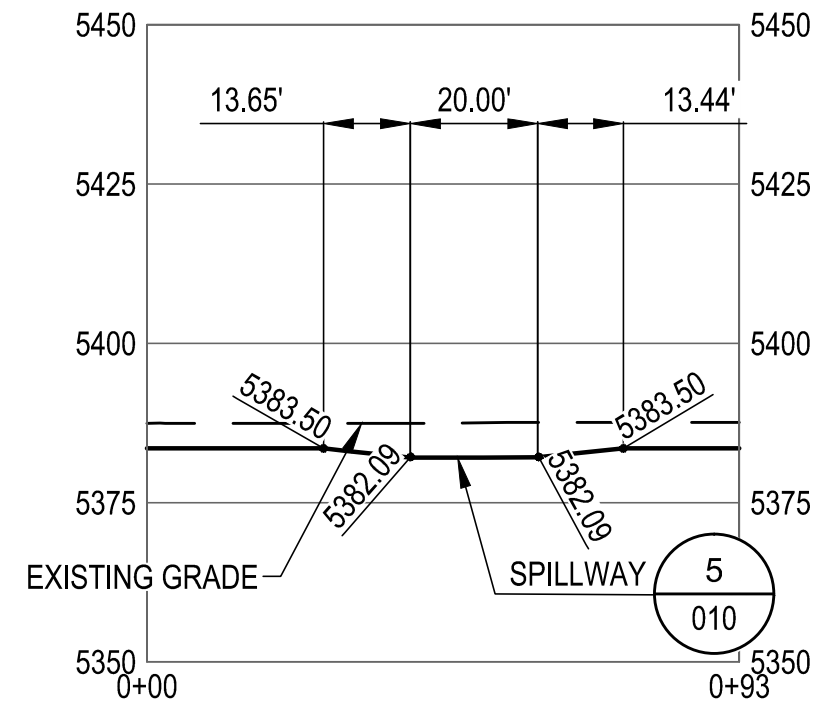
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0000-CI-010	STANDARD DETAILS SHEET 3														



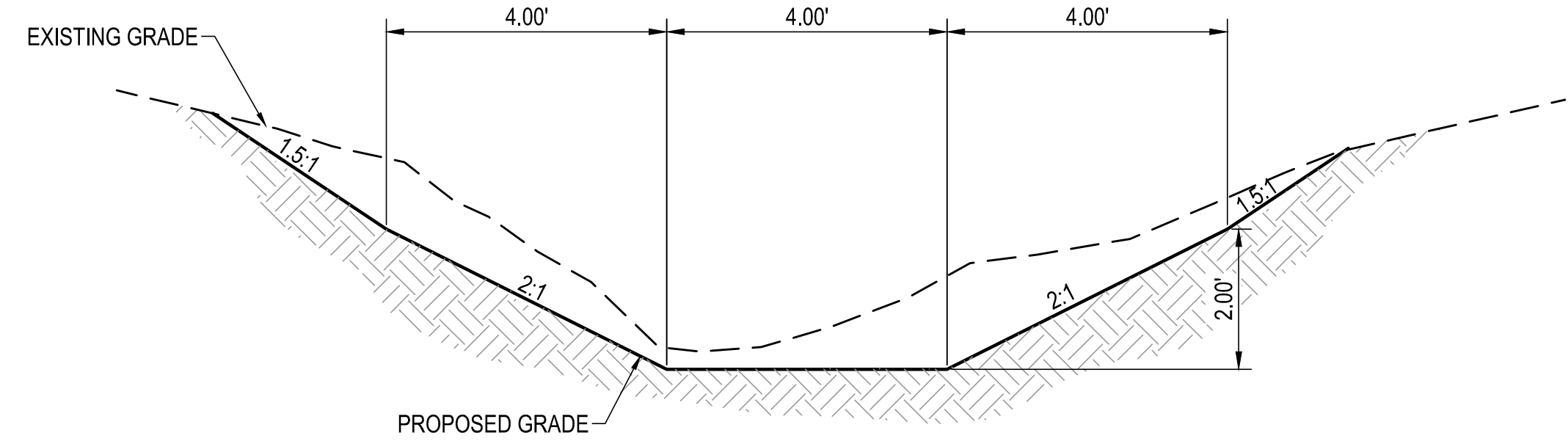
SECTION A
SCALE: 1" = 30'



SECTION B
SCALE: 1" = 30'



SECTION C
SCALE: 1" = 30'



SECTION D
SCALE: NTS

NOTES:

1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



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FIGURE 7

**THE MAC
RESOURCES**

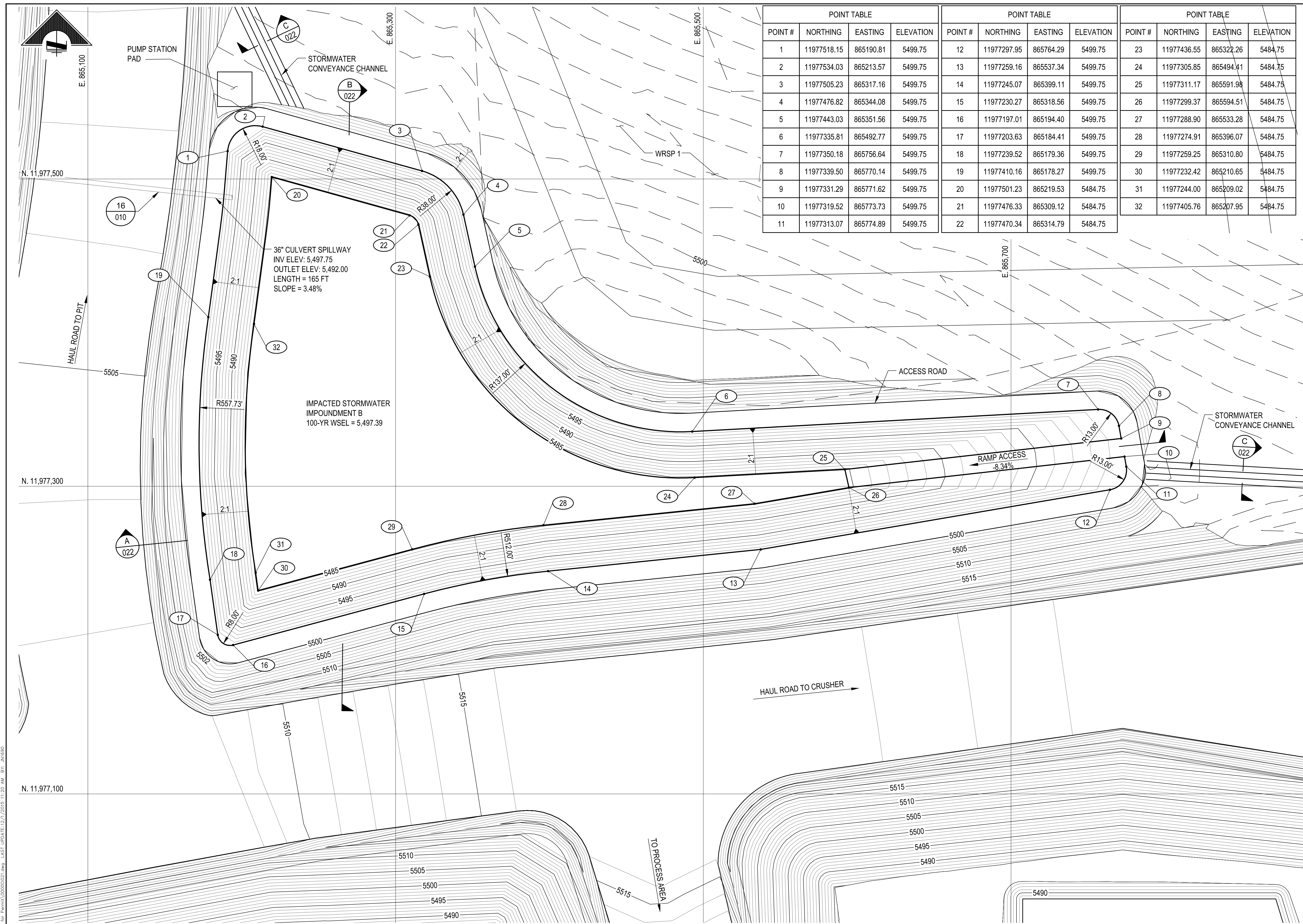
COPPER FLAT PROJECT

**GENERAL SITE
CIVIL
IMPACTED STORMWATER IMPOUNDMENT A
SECTIONS**

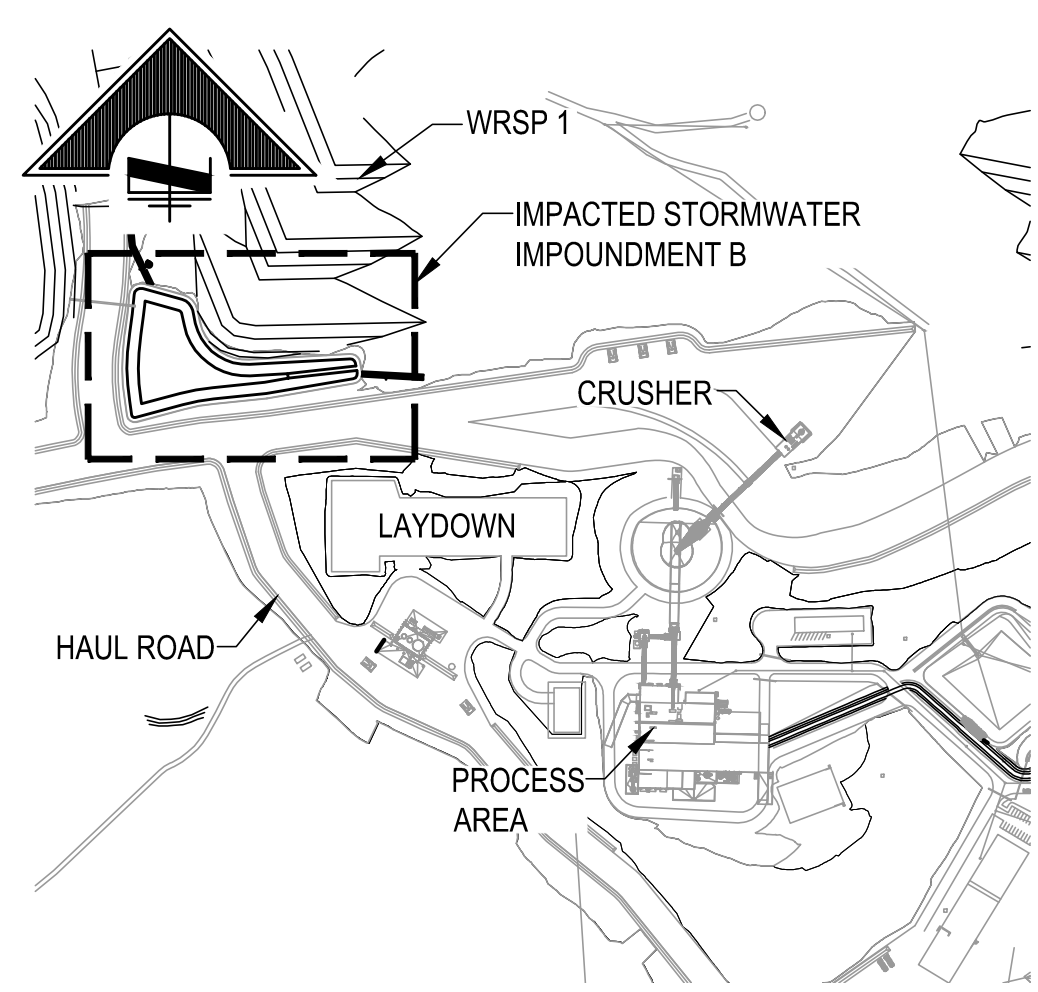
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DWG. NO. 0000-CI-024
REV. NO. P3
DATE 09 OCT 15

File: P:\2015\230863\CIVIL (644)\544.2 - Impacted Stormwater Impoundment A.dwg LAST UPDATE: 12/7/2015 11:20 AM BY: AN090

REFERENCES		REFERENCES		REVISIONS					REVISIONS					SCALE: AS NOTED		
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	DATE
0000-CI-023	IMPACTED STORMWATER IMPOUNDMENT A, PLAN VIEW															JUN 15
0000-CI-010	STANDARD DETAILS SHEET 3															JUN 15
																AUG 15
																AUG 15



POINT TABLE				POINT TABLE				POINT TABLE			
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1	11977518.15	865190.81	5499.75	12	11977297.95	865764.29	5499.75	23	11977436.55	865322.26	5484.75
2	11977534.03	865213.57	5499.75	13	11977259.16	865537.34	5499.75	24	11977305.85	865494.41	5484.75
3	11977505.23	865317.16	5499.75	14	11977245.07	865399.11	5499.75	25	11977311.17	865591.98	5484.75
4	11977476.82	865344.08	5499.75	15	11977230.27	865318.56	5499.75	26	11977299.37	865594.51	5484.75
5	11977443.03	865351.56	5499.75	16	11977197.01	865194.40	5499.75	27	11977288.90	865533.28	5484.75
6	11977335.81	865492.77	5499.75	17	11977203.63	865184.41	5499.75	28	11977274.91	865396.07	5484.75
7	11977350.18	865756.64	5499.75	18	11977239.52	865179.36	5499.75	29	11977259.25	865310.80	5484.75
8	11977339.50	865770.14	5499.75	19	11977410.16	865178.27	5499.75	30	11977232.42	865210.65	5484.75
9	11977331.29	865771.62	5499.75	20	11977501.23	865219.53	5484.75	31	11977244.00	865209.02	5484.75
10	11977319.52	865773.73	5499.75	21	11977476.33	865309.12	5484.75	32	11977405.76	865207.95	5484.75
11	11977313.07	865774.89	5499.75	22	11977470.34	865314.79	5484.75				



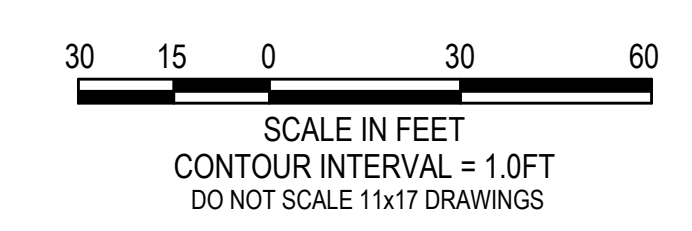
KEY MAP
SCALE: 1" = 500'

NOTES:

1. STORMWATER IMPOUNDMENT B IS INTENDED TO CAPTURE STORMWATER RUNOFF FROM THE WASTE ROCK STOCK PILE 1 (WRS P-1).
2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
3. STORMWATER CULVERT SPILLWAY IS DESIGNED FOR THE 25-YR, 24HR RAINFALL EVENT AT MINIMUM.
4. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60mil HDPE LINER PER DETAIL 4 ON DWG. 0000-CI-010, OR APPROVED EQUIVALENT.

IMPOUNDMENT SUMMARY		
CAPACITY AT 100-YR WSEL	748,445	CU-FT
ULTIMATE CAPACITY	913,160	CU-FT

PLAN
SCALE: 1" = 30'



PRELIMINARY
FOR AGENCY REVIEW

FIGURE 8



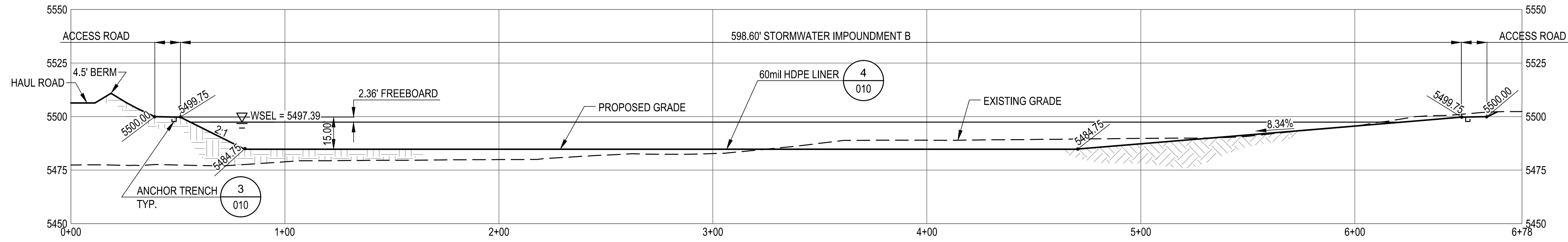
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DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT
0000-CI-022	IMPACTED STORMWATER IMPOUNDMENT B SECTIONS														
0000-CI-005	STANDARD DETAILS SHEET 2														

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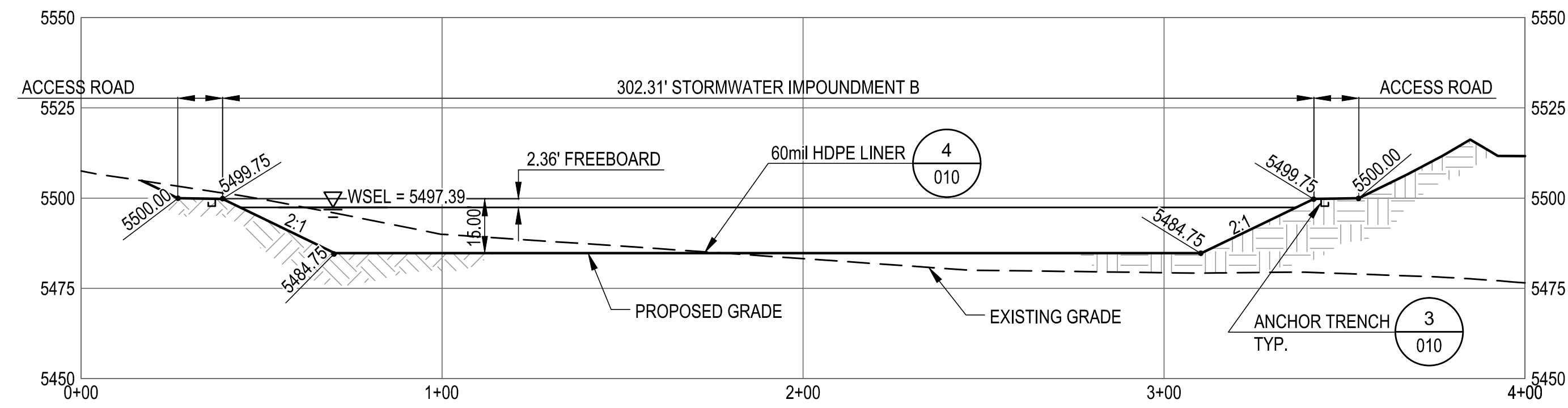
COPPER FLAT PROJECT
GENERAL SITE
CIVIL
IMPACTED STORMWATER IMPOUNDMENT B
PLAN VIEW

JOB NO. M3 PN-120085
DWG. NO. **0000-CI-021**
REV. NO. P4
DATE 27 OCT 15

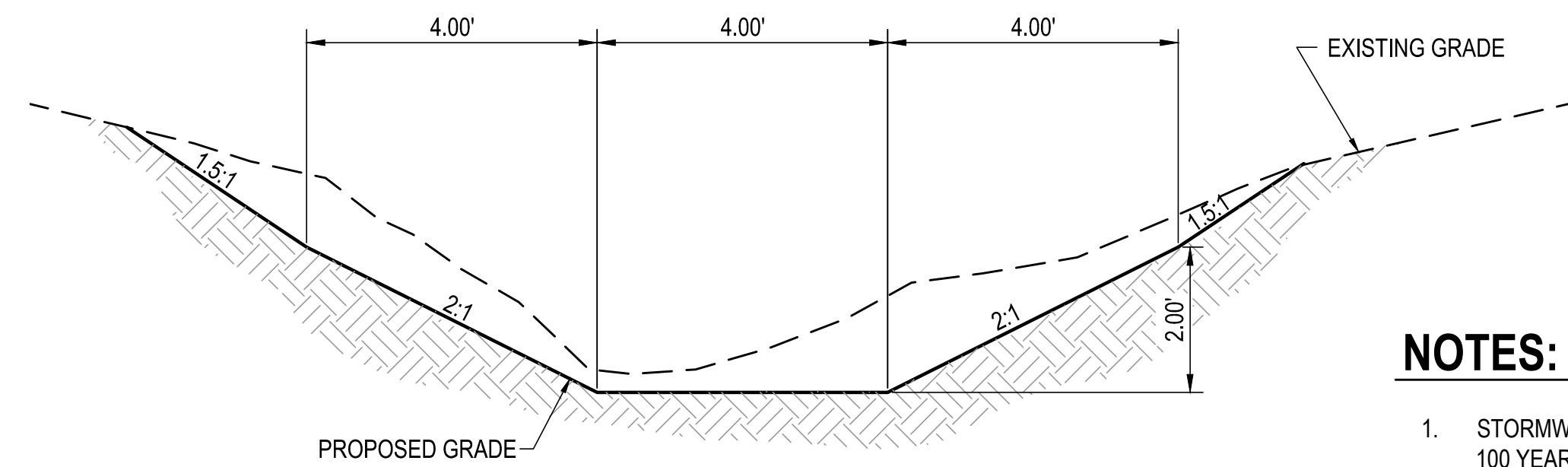
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SECTION A
SCALE: 1" = 30'



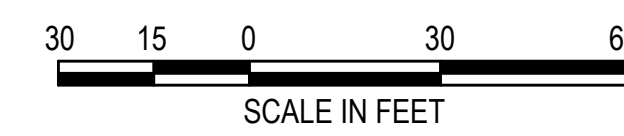
SECTION B
SCALE: 1" = 30'



SECTION C
SCALE: 1" = 30'

NOTES:

1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
NOT FOR CONSTRUCTION

FIGURE 9



File: P:\2013\200809\Civil (644)\544.2 - Impacted Stormwater Impoundment B.dwg, LAST UPDATE: 12/7/2015 11:20 AM, BY: AN090

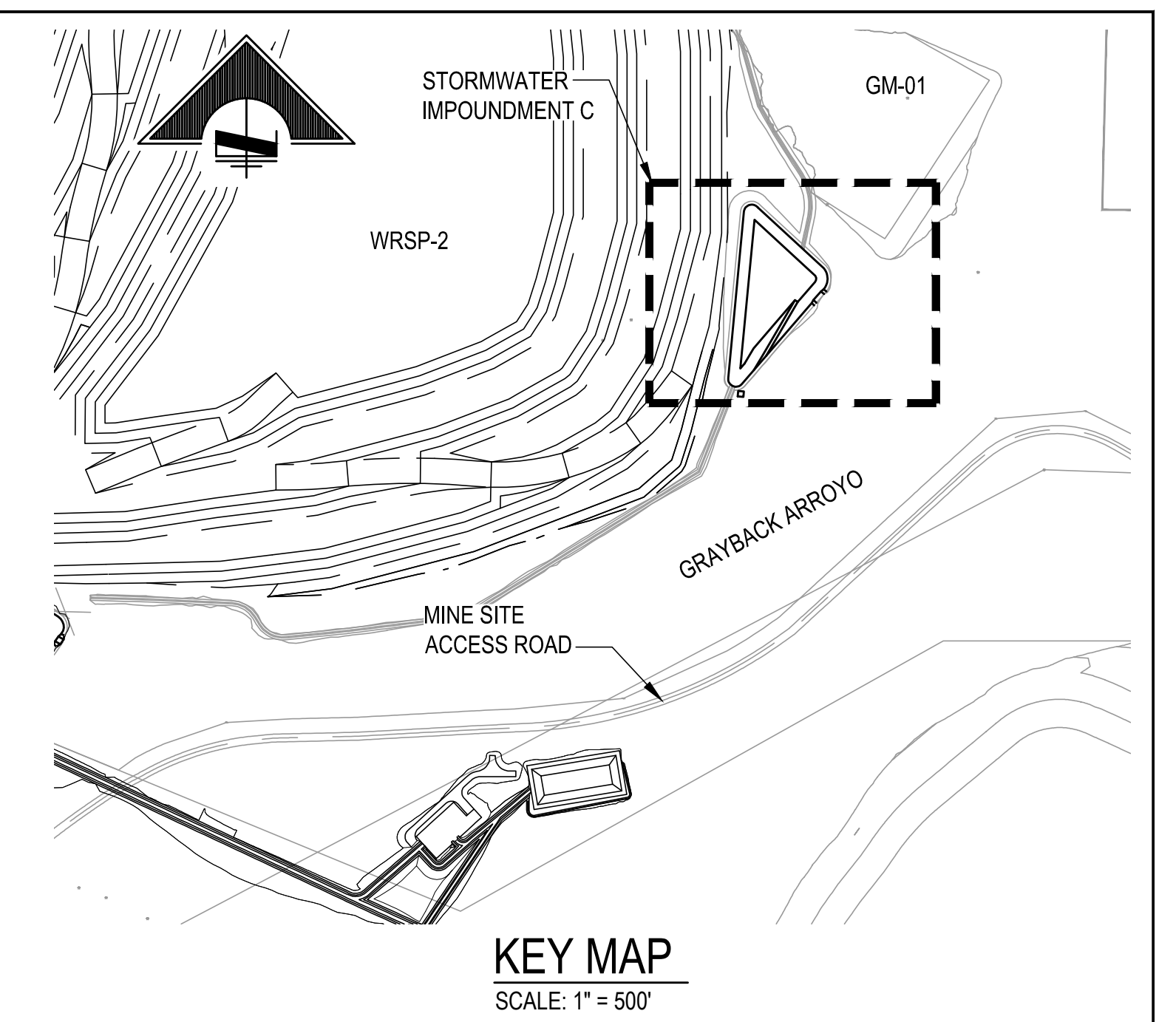
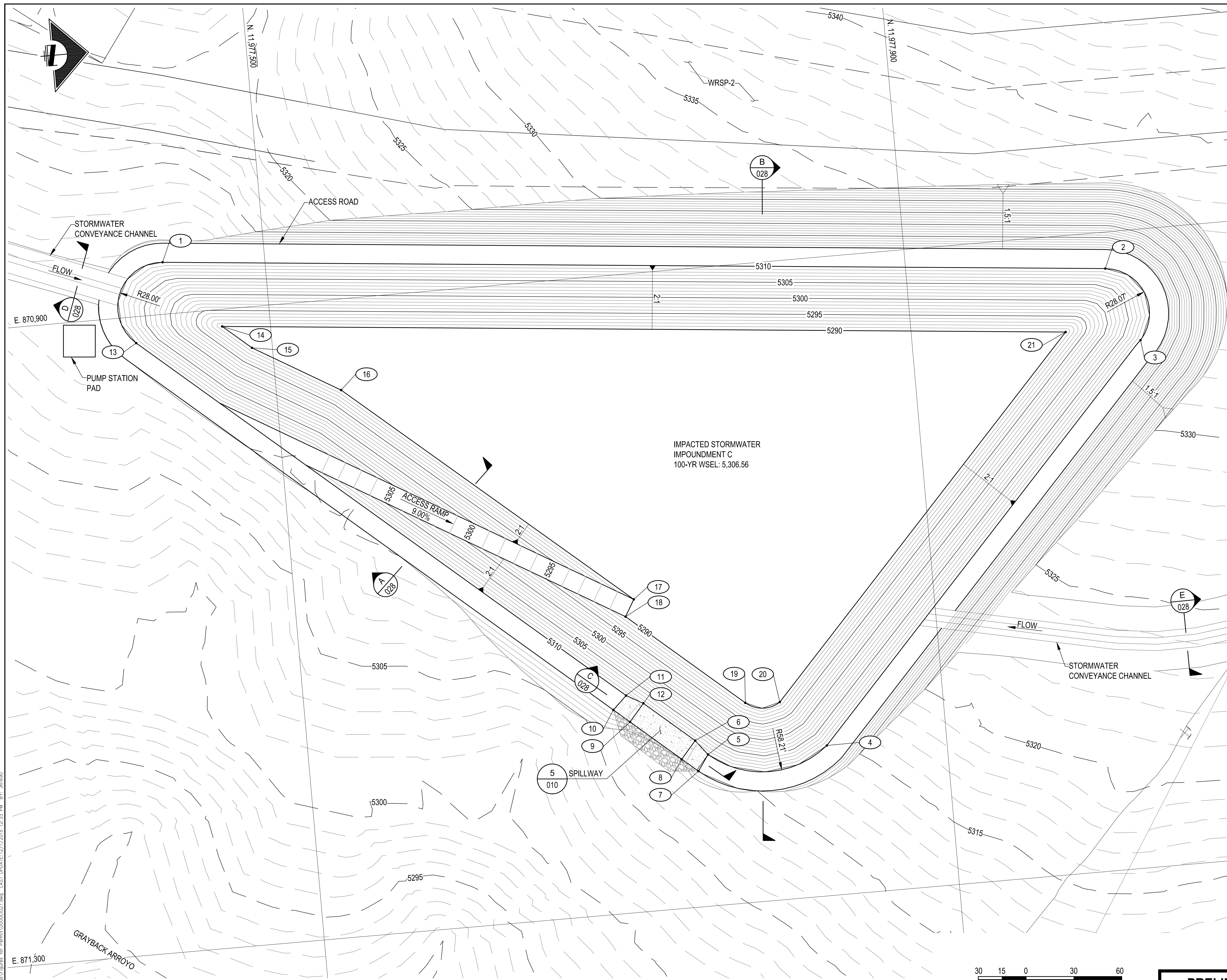
REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT
0000-CI-021	IMPACTED STORMWATER IMPOUNDMENT B PLAN VIEW														
0000-CI-010	STANDARD DETAILS SHEET 3														

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GENERAL SITE CIVIL IMPACTED STORMWATER IMPOUNDMENT B SECTIONS

JOB NO. M3 PN-120085
DWG. NO. **0000-CI-022**
REV. NO. P3 DATE 09 OCT 15

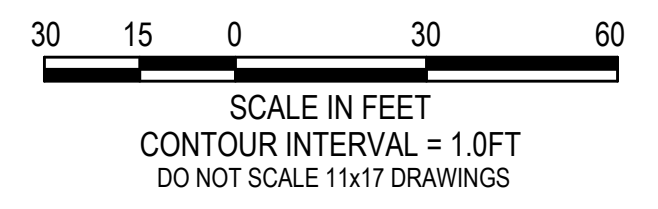


- NOTES:**
1. IMPACTED STORMWATER IMPOUNDMENT C IS INTENDED TO COLLECT AND RETAIN STORMWATER RUNOFF FROM WASTE ROCK STOCKPILE 2 (WRSP-2) AND WASTE ROCK STOCKPILE 3 (WRSP-3).
 2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
 3. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60mil HDPE PER DETAIL 4 ON SHEET 0000-CI-010, OR APPROVED EQUIVALENT.
 4. STORMWATER SPILLWAY IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT MINIMUM.
 5. SPILLWAY IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

IMPOUNDMENT SUMMARY			
CAPACITY AT 100-YR WSEL	1,405,507	CU-FT	
ULTIMATE CAPACITY	1,802,067	CU-FT	

POINT TABLE				POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION	POINT #	NORTHING	EASTING	ELEVATION
1	11977436.02	870867.41	5310.00	12	11977712.41	871169.49	5308.81
2	11978024.85	870922.90	5310.00	13	11977415.12	870916.50	5310.00
3	11978042.98	870969.57	5310.00	14	11977469.75	870910.76	5290.00
4	11977824.76	871205.86	5310.00	15	11977487.16	870925.76	5290.00
5	11977750.03	871205.03	5310.00	16	11977540.59	870957.08	5290.00
6	11977742.72	871195.59	5308.81	17	11977712.07	871103.95	5290.00
7	11977743.12	871214.89	5310.00	18	11977706.00	871114.30	5290.00
8	11977733.33	871206.49	5308.67	19	11977776.14	871174.73	5290.00
9	11977703.03	871180.38	5308.67	20	11977797.74	871176.15	5290.00
10	11977693.25	871171.96	5310.00	21	11977996.59	870960.41	5290.00
11	11977702.00	871163.65	5310.00				

PLAN
SCALE: 1" = 30'



PRELIMINARY
FOR AGENCY REVIEW

FIGURE 10



REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT
0000-CI-028	IMPACTED STORMWATER IMPOUNDMENT C SECTIONS														
0000-CI-005	STANDARD DETAILS SHEET 2														
0000-CI-010	STANDARD DETAILS SHEET 3														

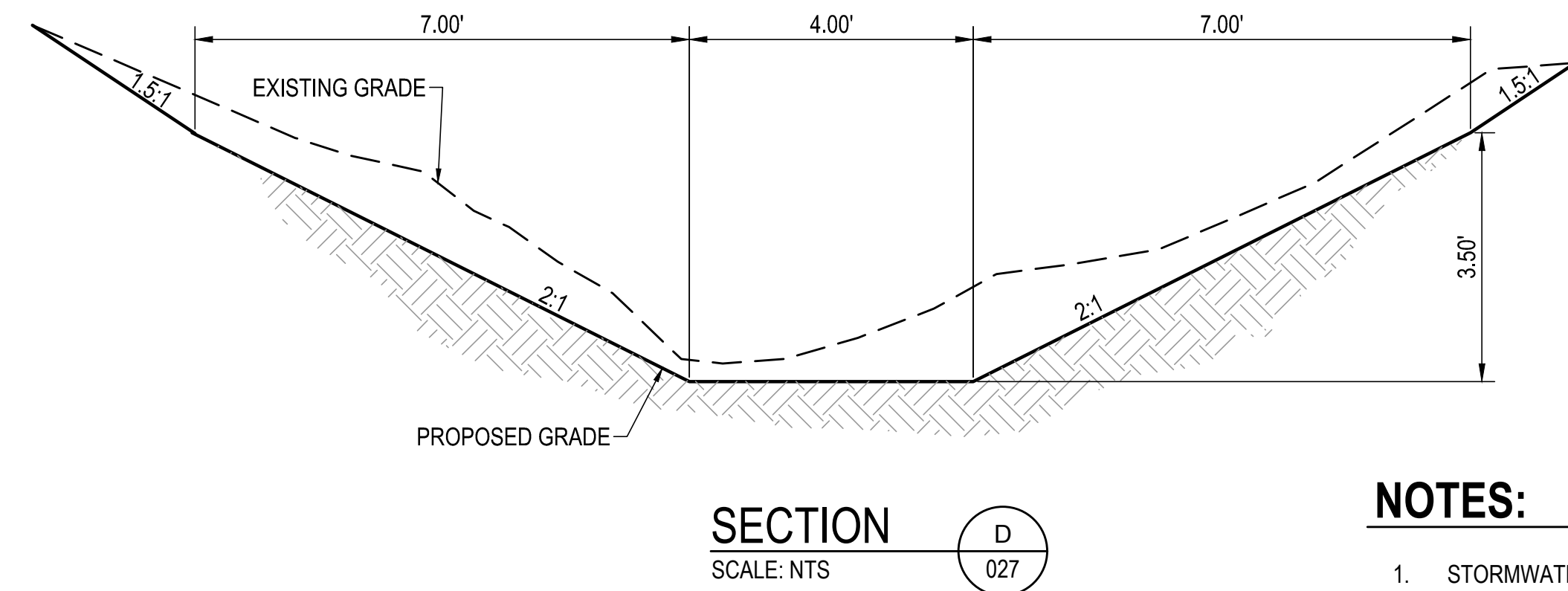
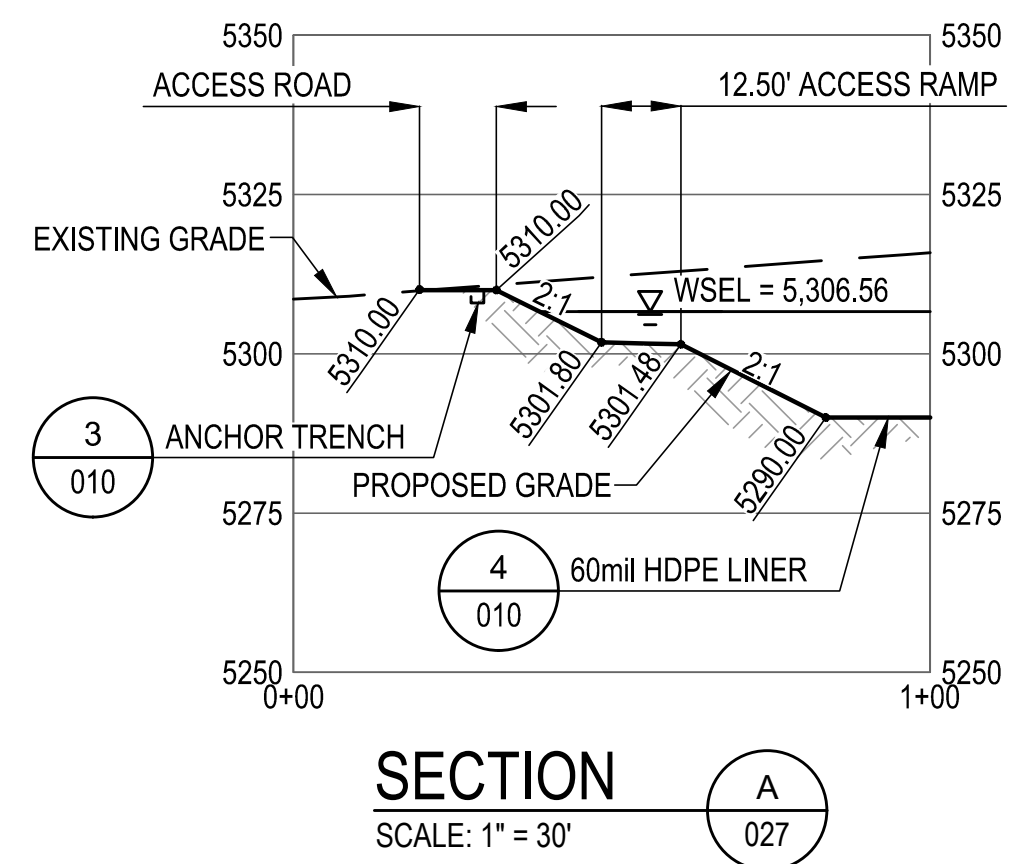
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COPPER FLAT PROJECT

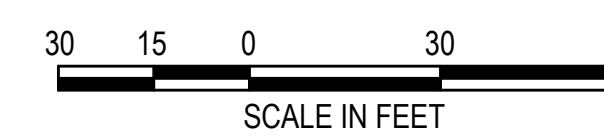
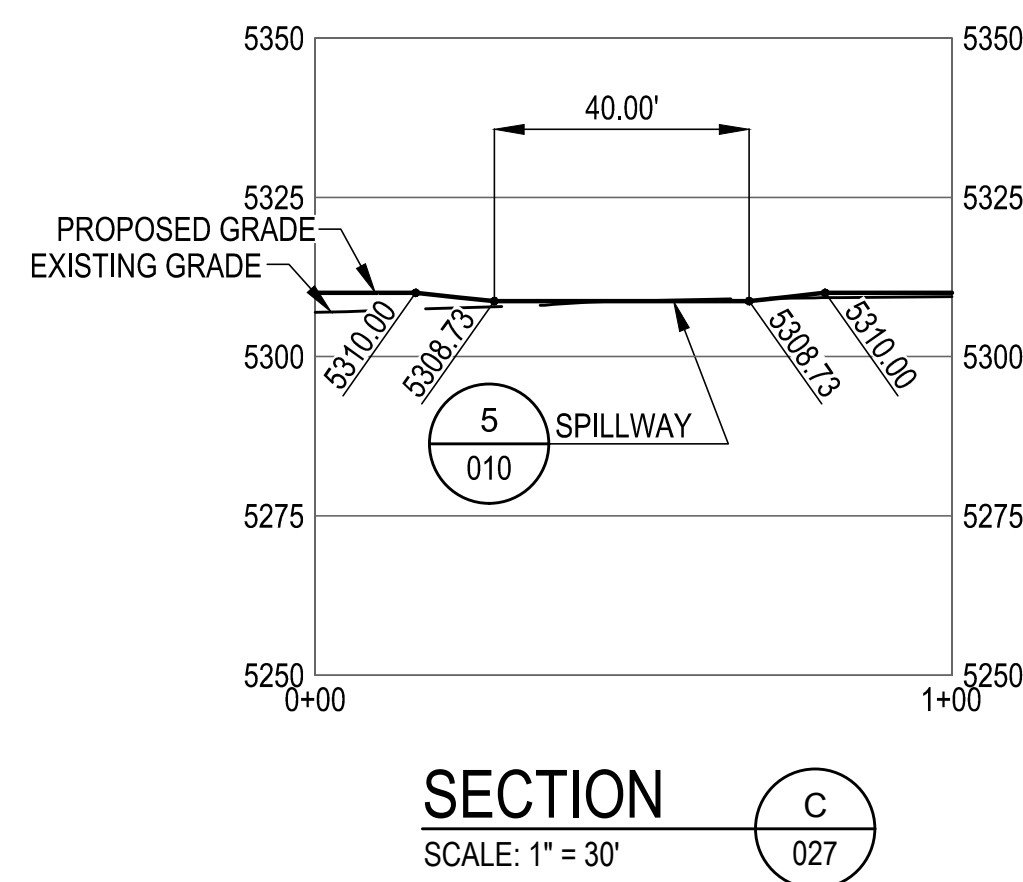
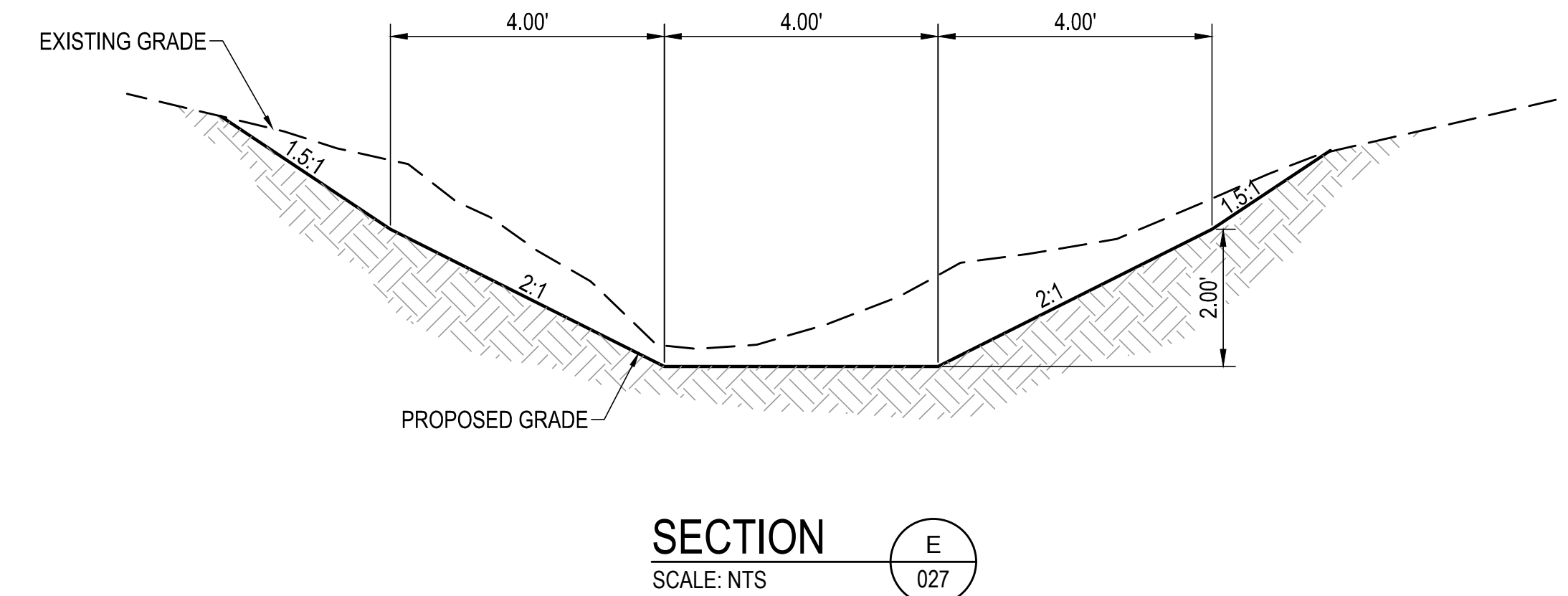
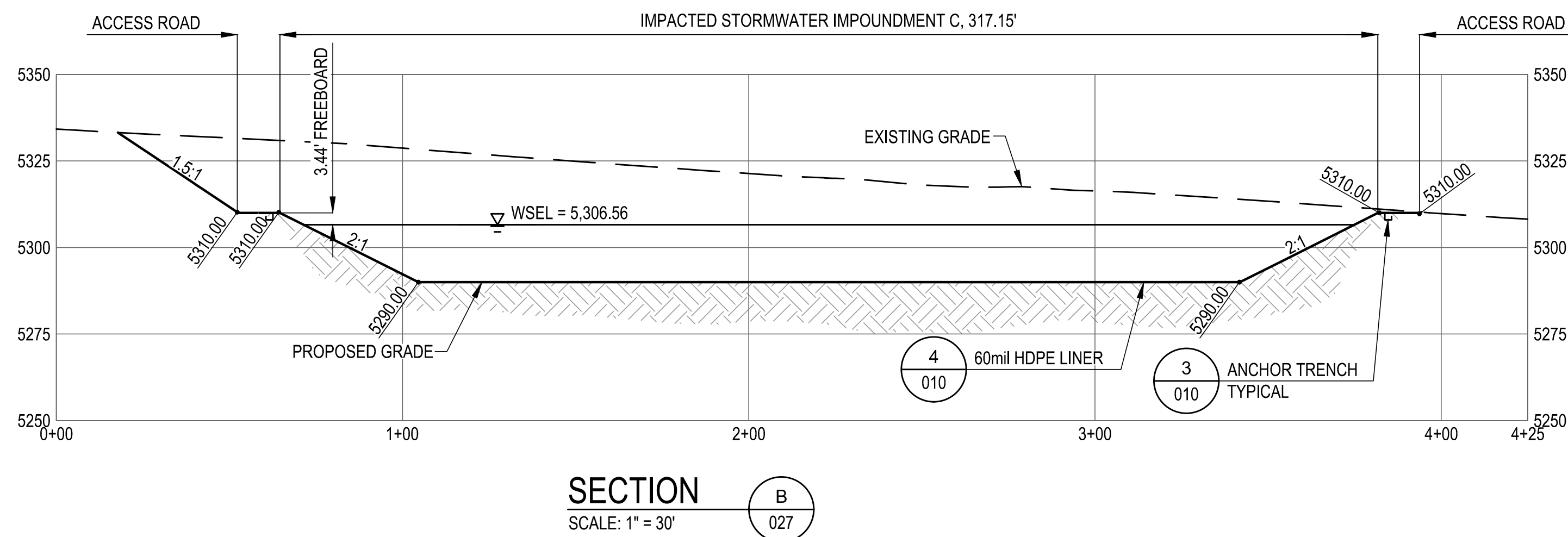
GENERAL SITE CIVIL
IMPACTED STORMWATER IMPOUNDMENT C
PLAN VIEW

JOB NO. M3 PN-120085
DWG NO. **0000-CI-027**
REV. NO. P3 DATE 09 OCT 15

File: E:\3003\3003003\Civil\0441\544.2 - Design\Drawings for Permit\0000CI027.dwg, LAST UPDATE: 12/7/2015 12:33 PM, BY: JN069



- NOTES:**
1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
 2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW

FIGURE 11



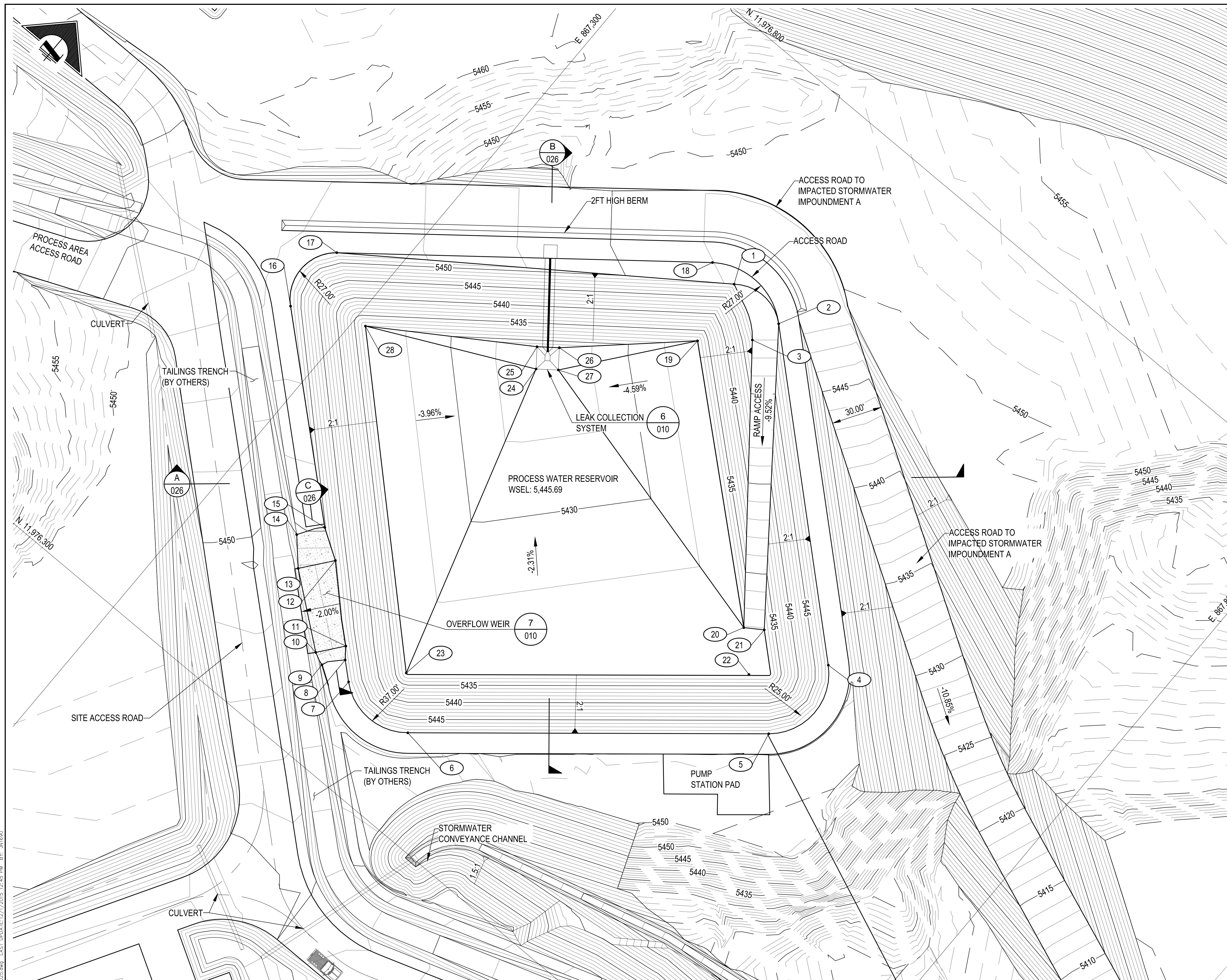
File: P:\2023\230808\Civil (644)\S44.2 - Imp\Updates for Permit\0000CI-028.dwg, LAST UPDATE: 12/7/2015 12:38 PM BY: AN090

REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED		DATE	
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW														
0000-CI-010	STANDARD DETAILS SHEET 3														

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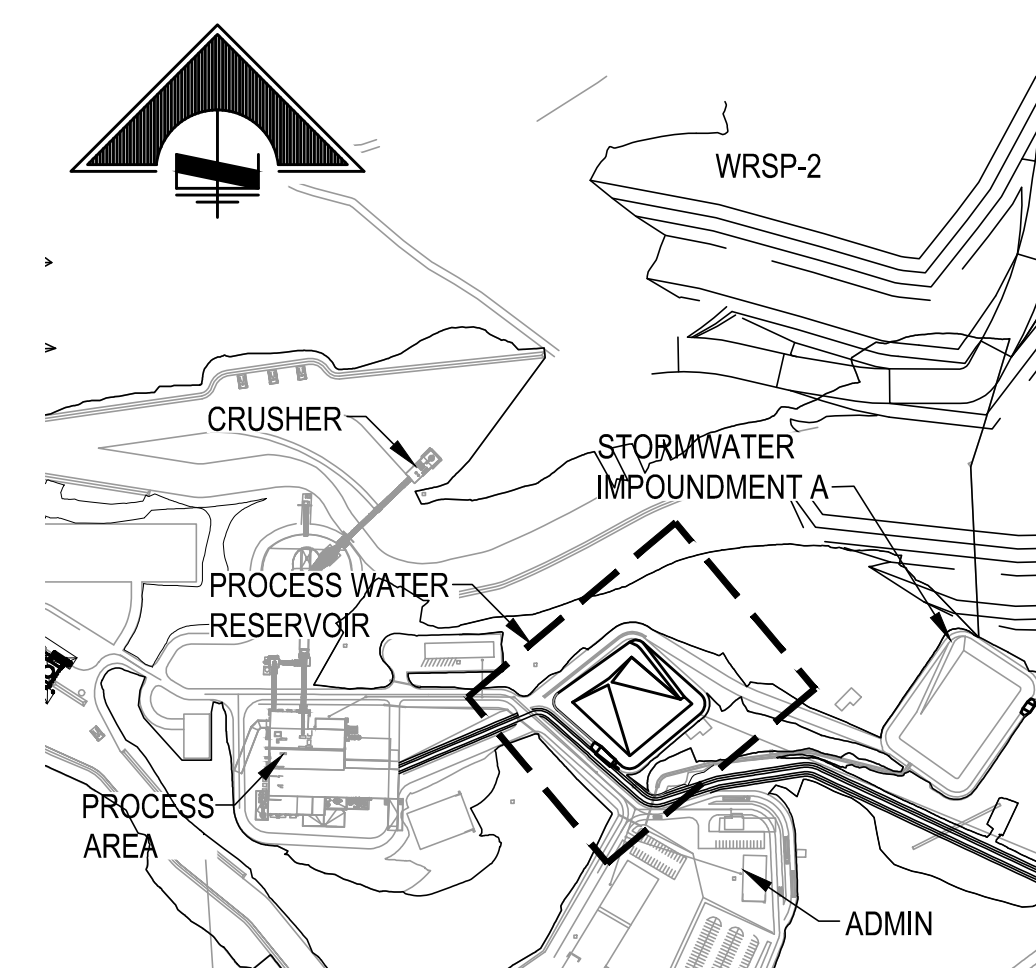
COPPER FLAT PROJECT
GENERAL SITE CIVIL
IMPACTED STORMWATER IMPOUNDMENT C SECTIONS

JOB NO. M3 PN-120085
DWG. NO. **0000-CI-028**
REV. NO. P3 DATE 09 OCT 15



PLAN
SCALE: 1" = 30'

POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION
1	11976677.01	867456.22	5448.90
2	11976676.01	867490.99	5448.90
3	11976658.90	867485.30	5448.00
4	11976525.20	867641.13	5444.30
5	11976488.75	867638.96	5448.61
6	11976354.25	867478.64	5448.90
7	11976354.80	867433.15	5448.90
8	11976363.32	867423.40	5448.90
9	11976352.45	867414.83	5448.90
10	11976355.69	867407.41	5447.80
11	11976369.69	867418.41	5448.16
12	11976404.09	867381.76	5448.24
13	11976386.62	867368.04	5447.80
14	11976401.16	867354.74	5449.81
15	11976414.90	867364.55	5450.22
16	11976500.94	867266.34	5452.92
17	11976542.17	867266.96	5452.92
18	11976678.55	867438.45	5449.02
19	11976637.98	867461.15	5431.90
20	11976519.41	867579.10	5431.84
21	11976526.05	867589.10	5431.90
22	11976508.16	867609.15	5431.90
23	11976380.38	867455.56	5431.90
24	11976564.87	867399.57	5428.10
25	11976575.19	867391.67	5428.10
26	11976583.10	867401.99	5428.10
27	11976572.78	867409.89	5428.10
28	11976520.09	867307.24	5431.90

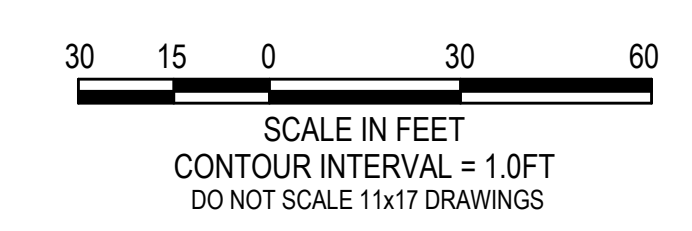


KEY MAP
SCALE: 1" = 500'

NOTES:

1. THE PROCESS WATER RESERVOIR IS INTENDED TO RETAIN PROCESS WATER, STORMWATER THAT FALLS DIRECTLY ON THE POND SURFACE, AND STORMWATER TRANSFERRED FROM OTHER IMPACTED STORMWATER IMPOUNDMENTS.
2. THE PROCESS WATER RESERVOIR IS SIZED TO RETAIN 12 HRS OF 7,200GPM INFLOW AND THE 100-YR, 24-HR RAINFALL EVENT PLUS 2 FEET OF FREEBOARD.
3. THE PROCESS WATER WATER RESERVOIR SHALL BE DOUBLE LINED WITH 60mil HDPE PER DETAIL 2 SHEET 0000-CI-010.
4. THE PROCESS WATER RESERVOIR OVERFLOW WEIR IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT CAPACITY (SEE NOTE 2) AT MINIMUM. THE WEIR CONVEYS PROCESS WATER INTO THE TAILINGS TRENCH AND TO THE TAILINGS IMPOUNDMENT.
5. OVERFLOW WEIR IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

IMPOUNDMENT SUMMARY		
CAPACITY AT 100-YR WSEL	726,365	CU-FT
ULTIMATE CAPACITY	937,998	CU-FT



PRELIMINARY
FOR AGENCY REVIEW

FIGURE 12



File: P:\2023\202308\01\Civil\0441\5442 - Design\Drawings for Permit\0000-CI-026.dwg, LAST UPDATE: 12/7/2015 12:45 PM, BY: AN080

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0000-CI-026	PROCESS WATER RESERVOIR SECTIONS														
0000-CI-010	STANDARD DETAILS SHEET 3														

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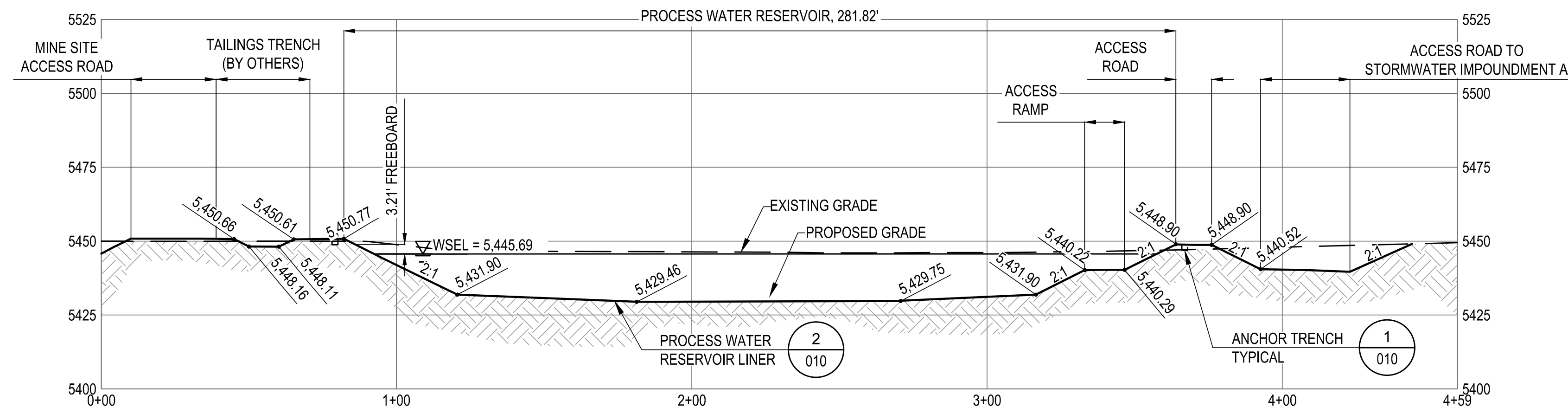
COPPER FLAT PROJECT

GENERAL SITE
CIVIL
PROCESS WATER RESERVOIR
PLAN VIEW

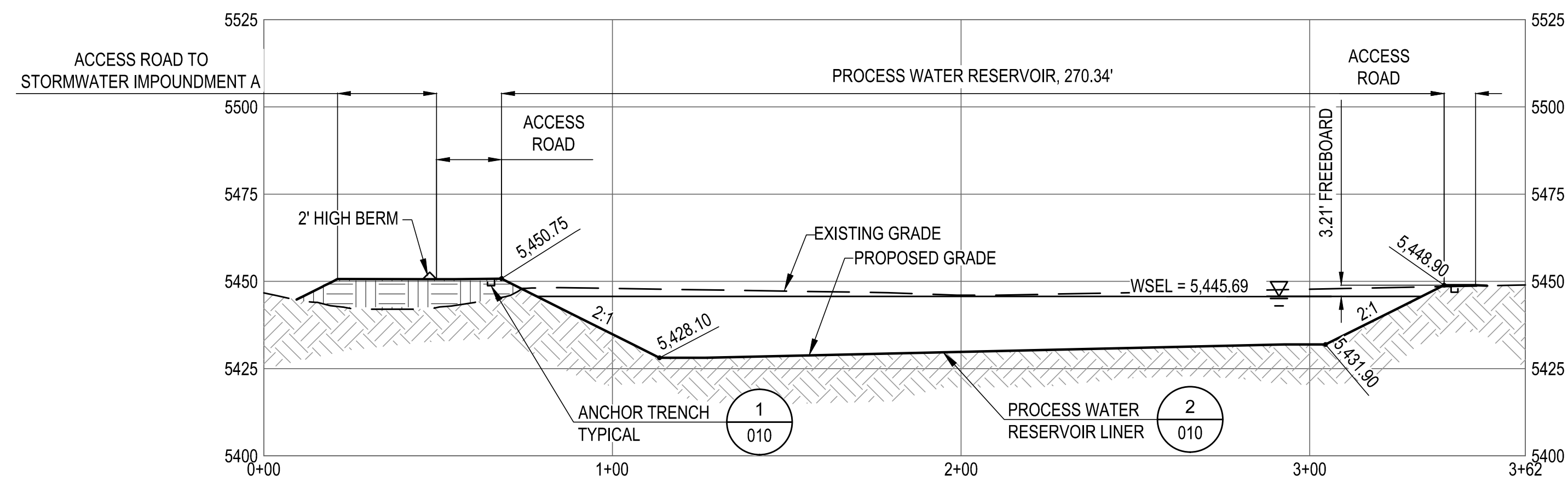
JOB NO. M3 PN-120085

DWG NO. **0000-CI-025**

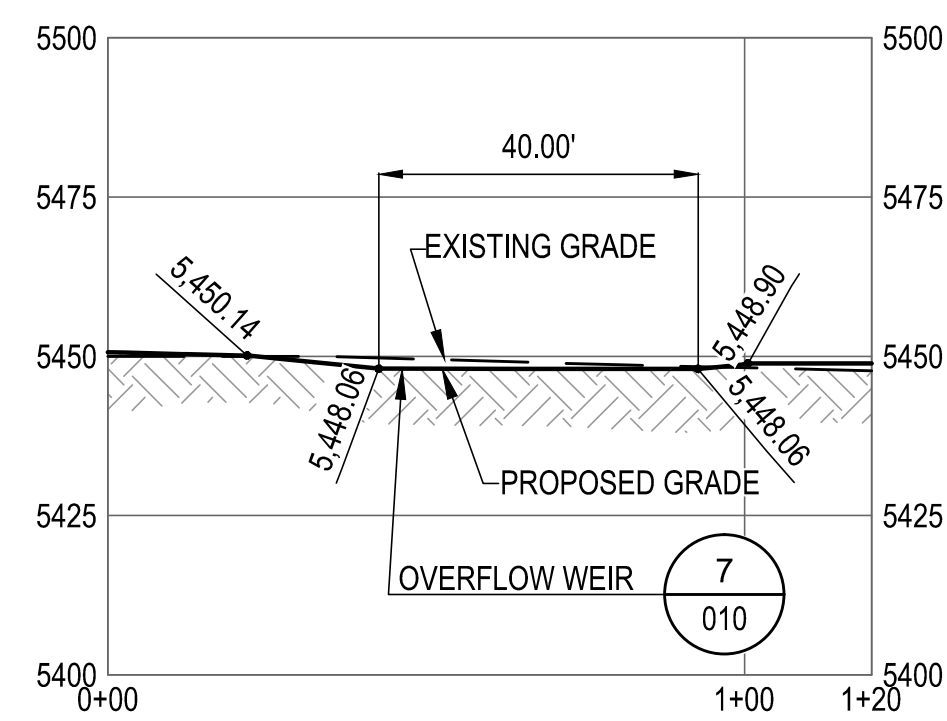
REV. NO. P3 DATE 09 OCT 15



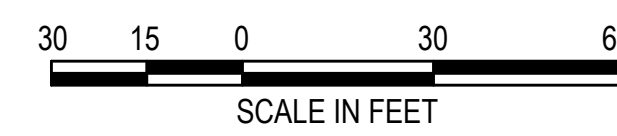
SECTION A
SCALE: 1" = 30'



SECTION B
SCALE: 1" = 30'



SECTION C
SCALE: 1" = 30'



DO NOT SCALE 11x17 DRAWINGS

PRELIMINARY
FOR AGENCY REVIEW



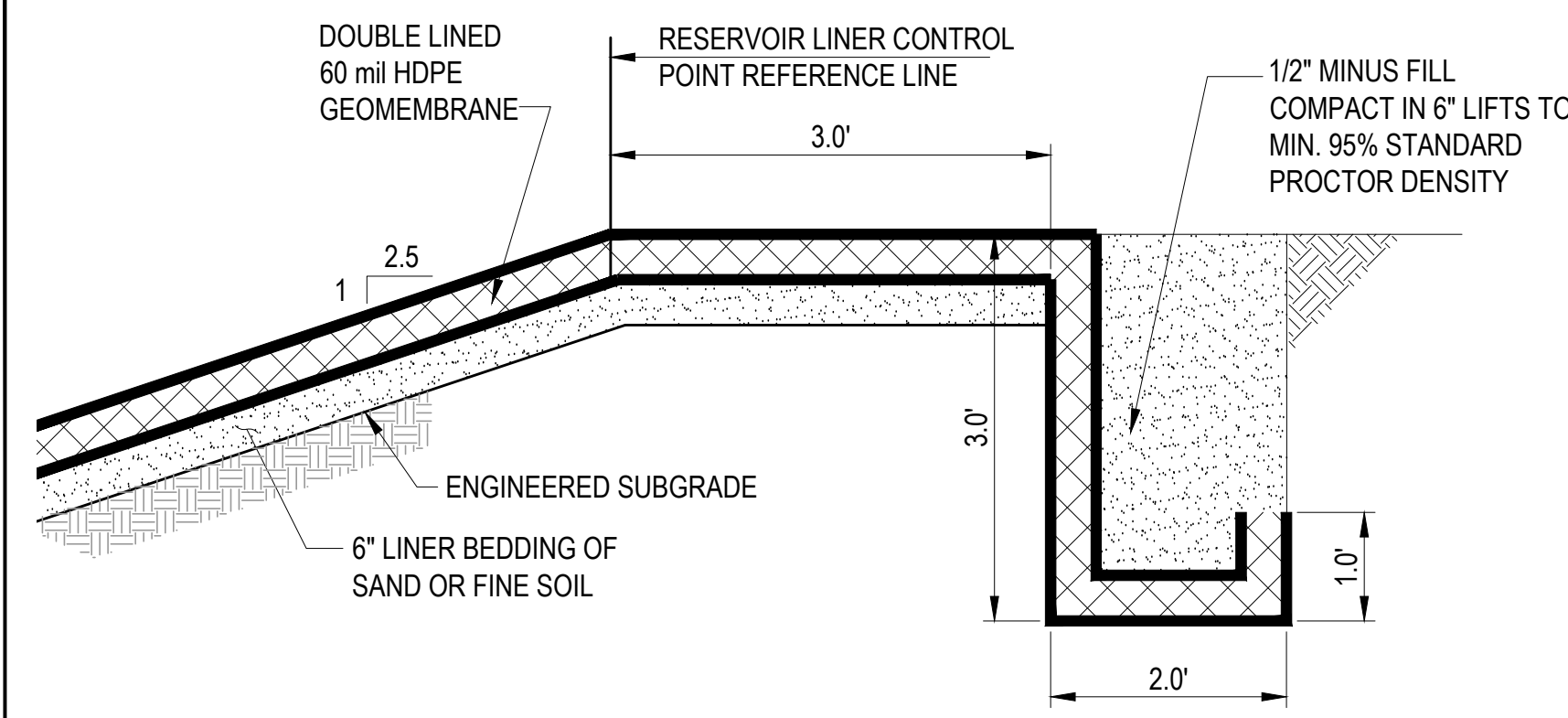
FIGURE 13

File: P:\2023\202308\CI-025 - Design\Drawings for Permit\0000-CI-026.dwg, LAST UPDATE: 12/7/2015 12:49 PM, BY: JN0909

REFERENCES		REFERENCES		REVISIONS						REVISIONS					
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0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-010	STANDARD DETAILS SHEET 3														

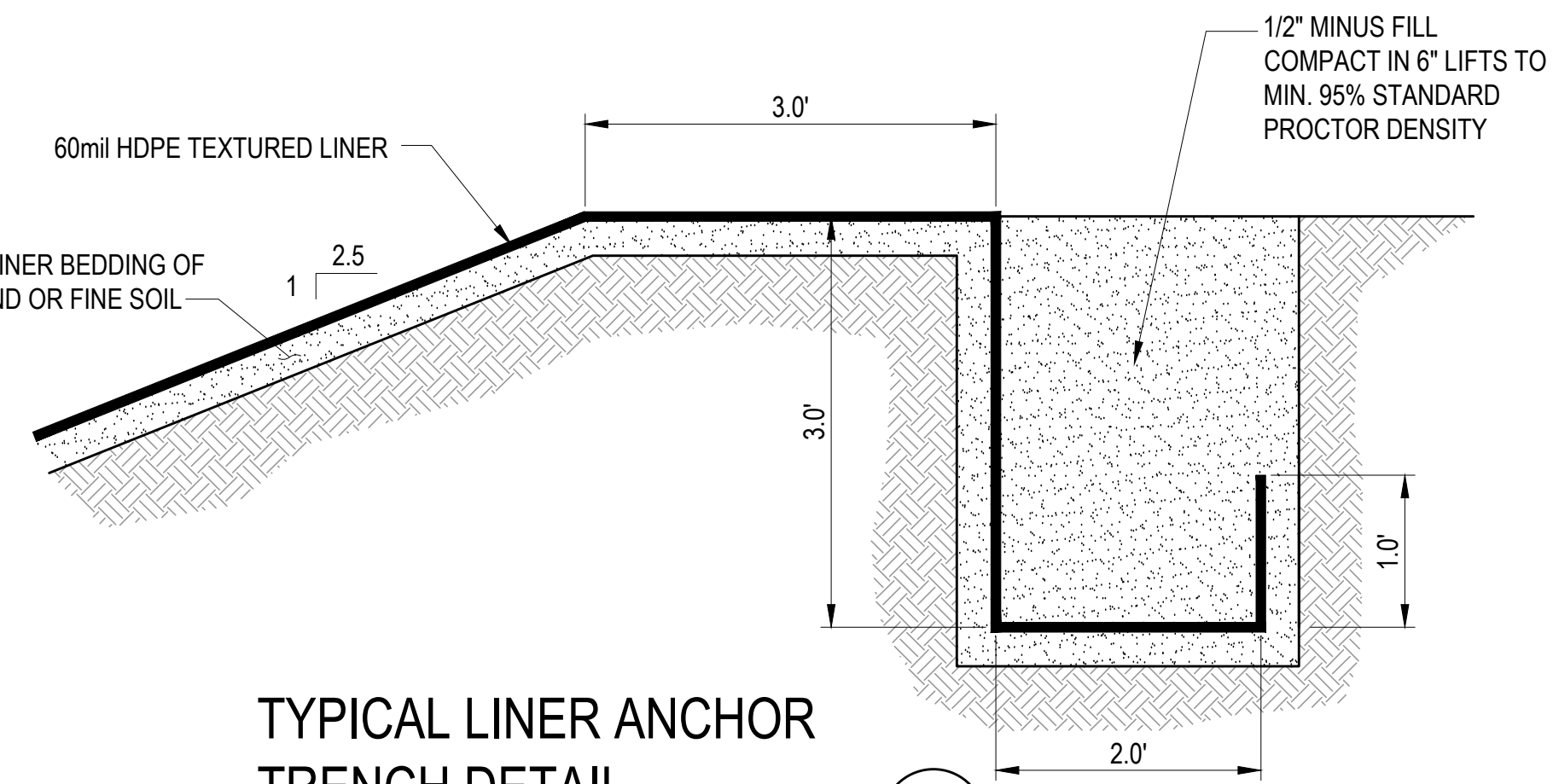
SCALE:	AS NOTED	DATE:	
DESIGNED BY:	SAM	DATE:	JUN 15
DRAWN BY:	SAM	DATE:	JUN 15
CHECKED BY:	JPN		
PROJECT MGR:	RKZ		
CLIENT APPR:			

COPPER FLAT PROJECT GENERAL SITE CIVIL PROCESS WATER RESERVOIR SECTIONS		JOB NO. M3 PN-120085
		DWG. NO. 0000-CI-026
REV. NO. P3	DATE 09 OCT 15	



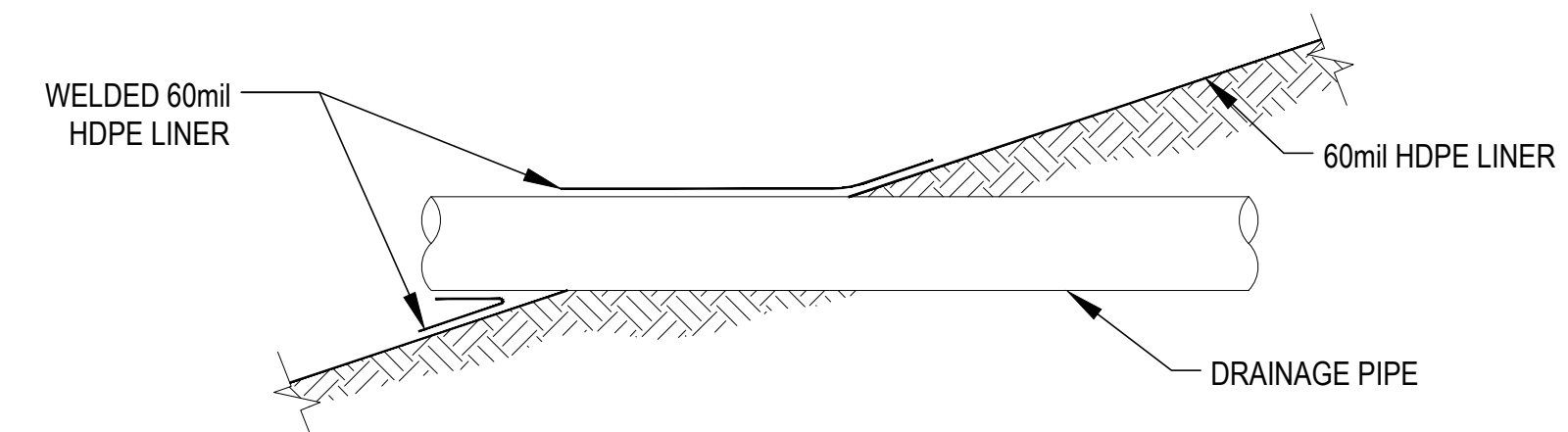
PROCESS WATER RESERVOIR LINER ANCHOR DETAIL

N.T.S. 1 026



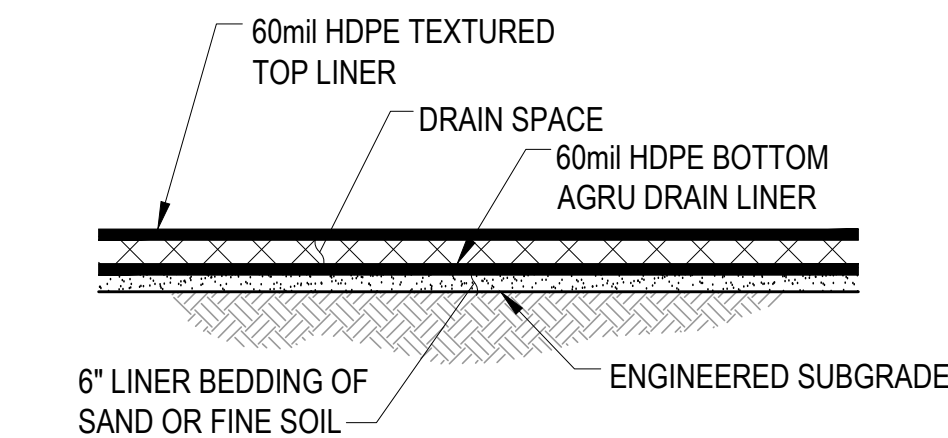
TYPICAL LINER ANCHOR TRENCH DETAIL

N.T.S. 3 022, 024, 028



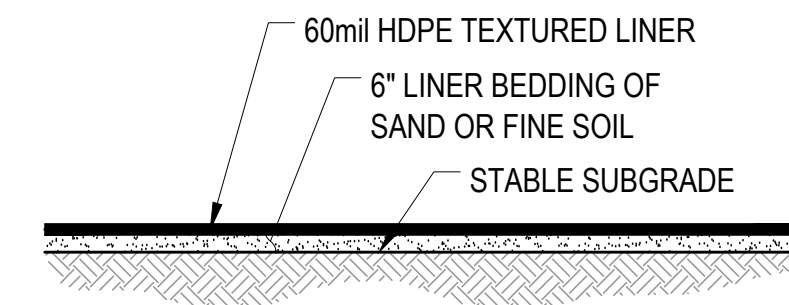
PIPE PENETRATING LINER

N.T.S. 16



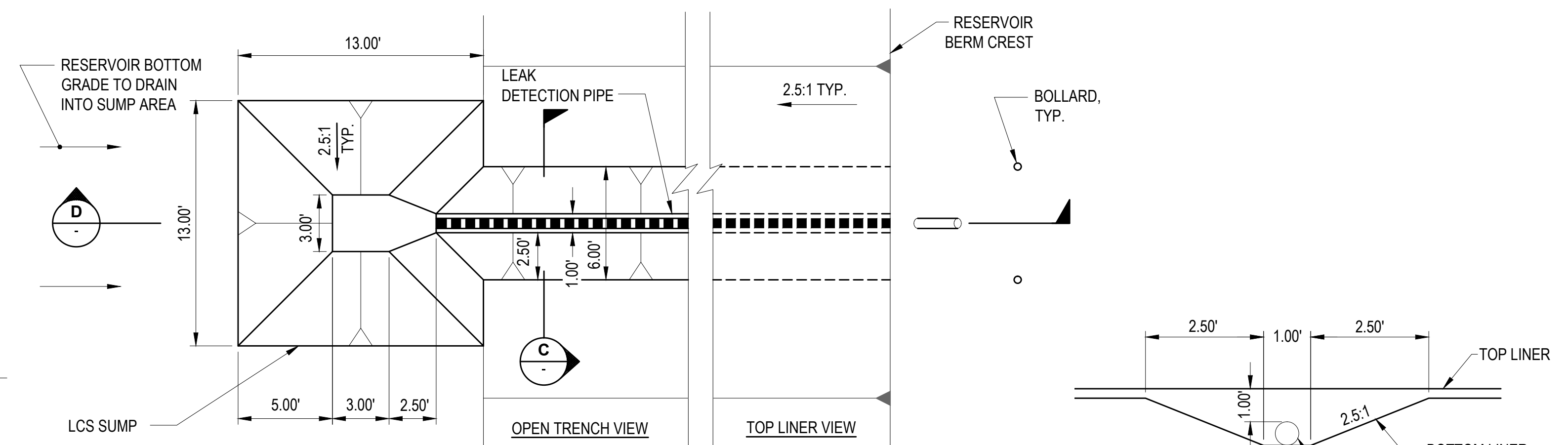
TYPICAL PROCESS WATER RESERVOIR LINER

N.T.S. 2 026



TYPICAL STORMWATER IMPOUNDMENT LINER

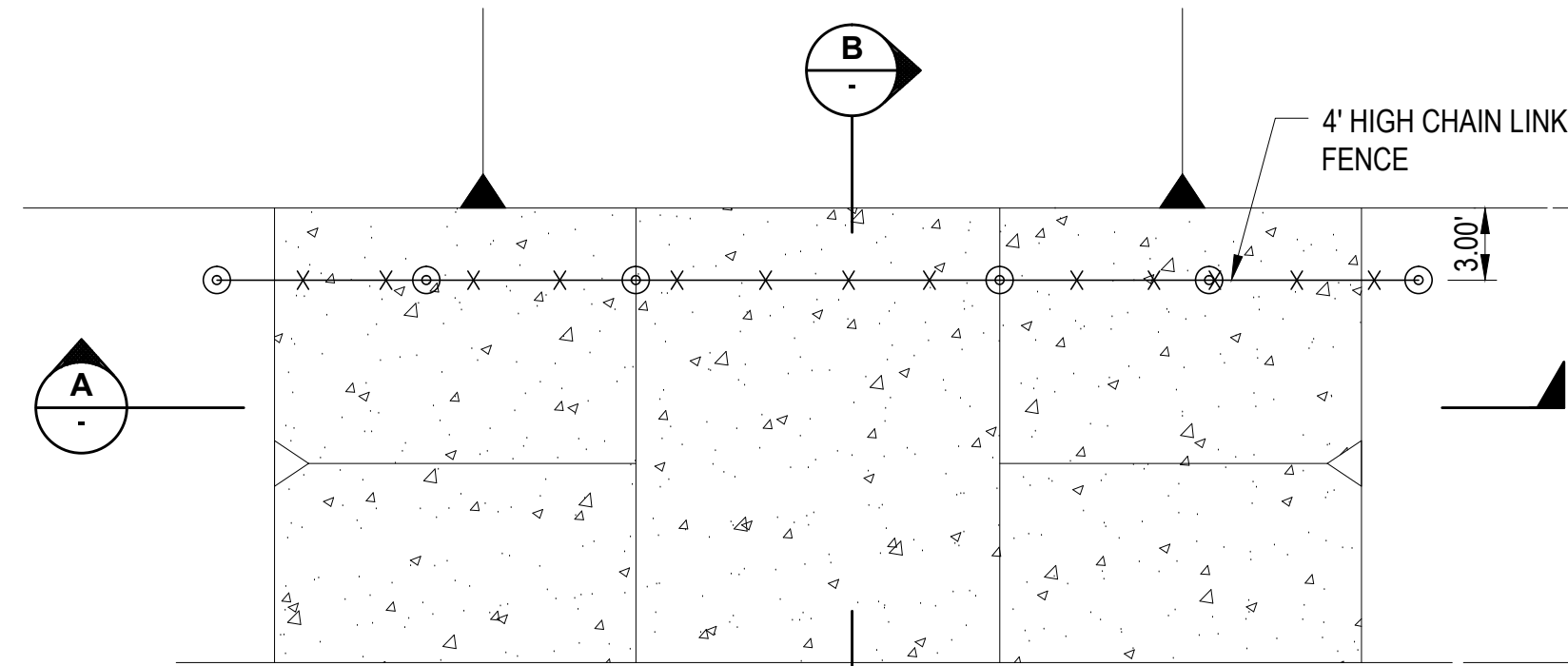
N.T.S. 4 022, 024, 028



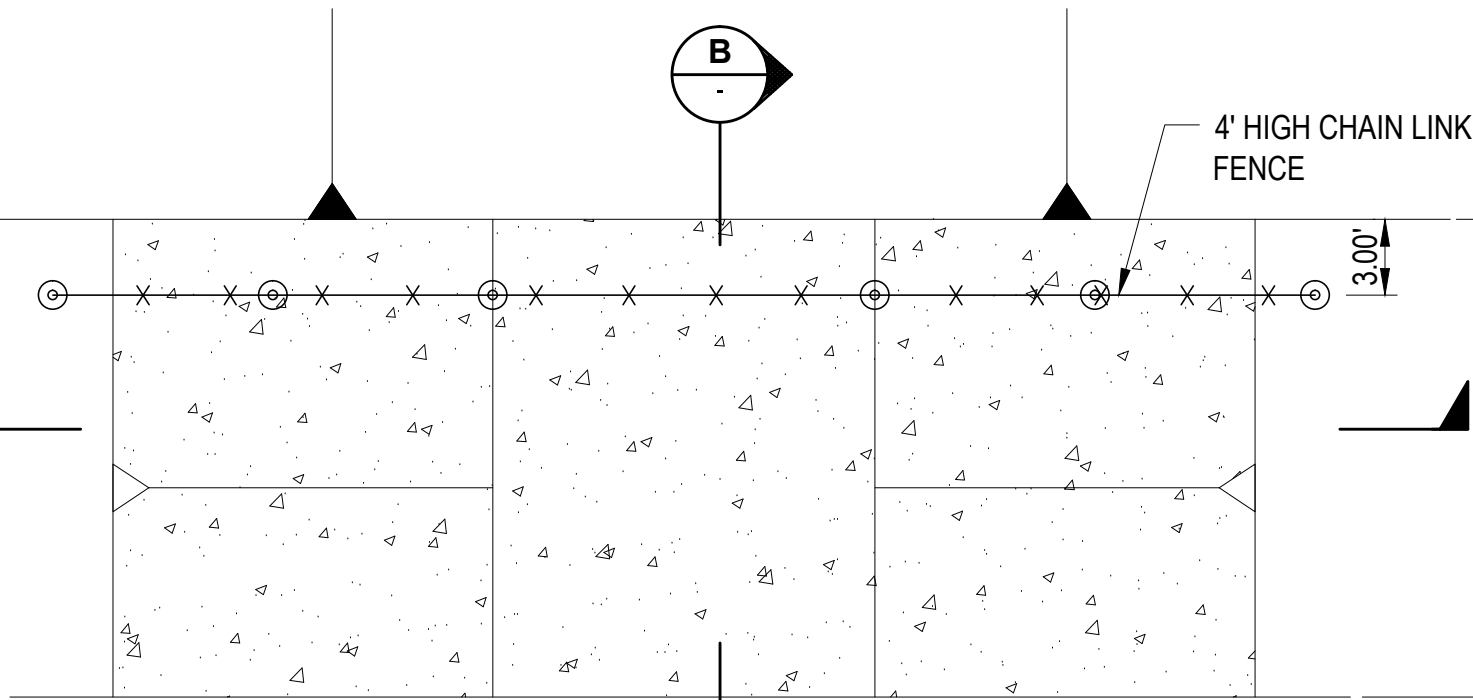
PROCESS WATER RESERVOIR - LCS PLAN

N.T.S. 6 025

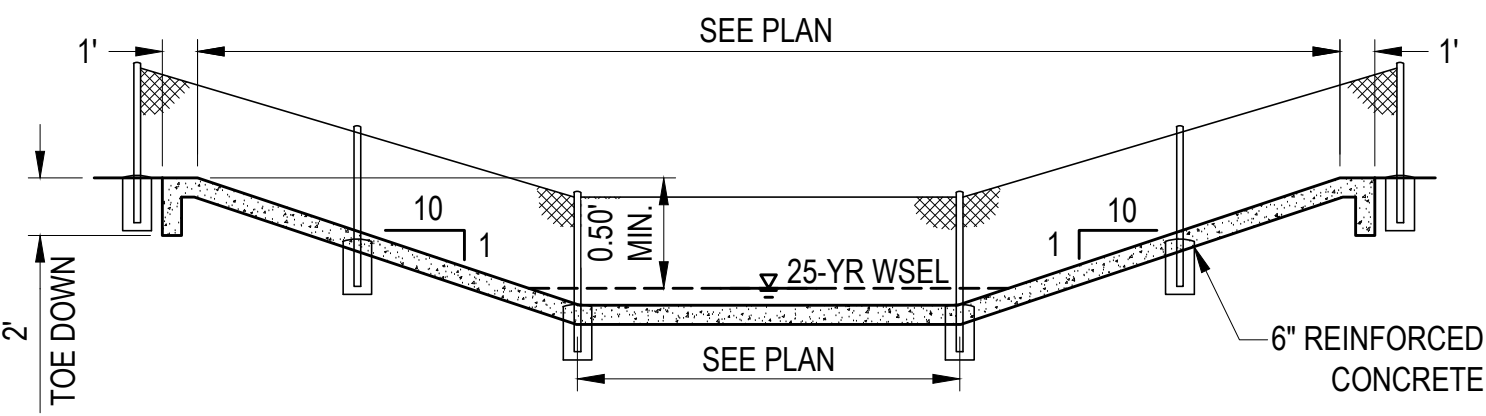
SECTION C



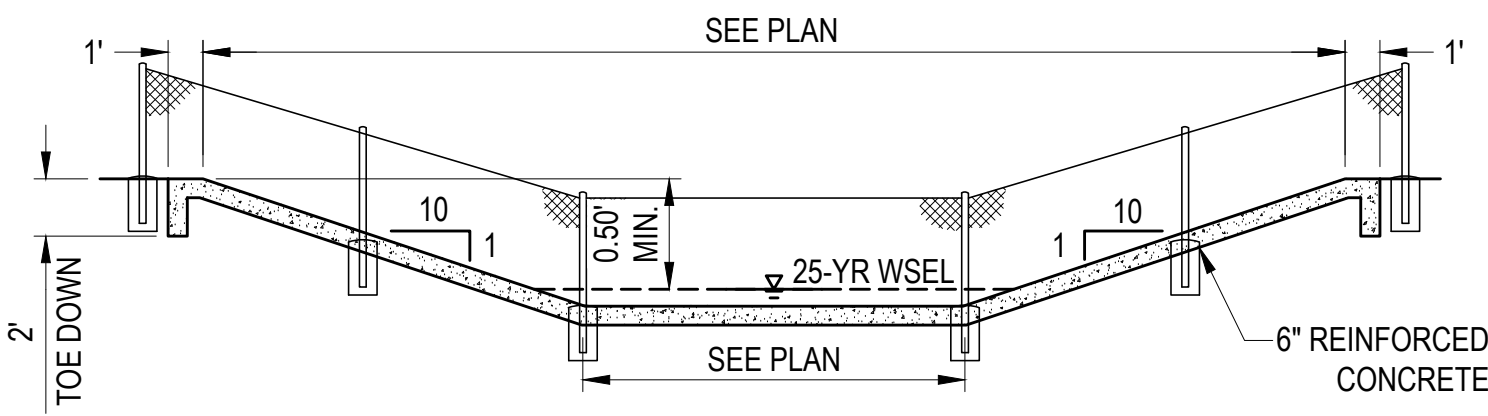
PLAN



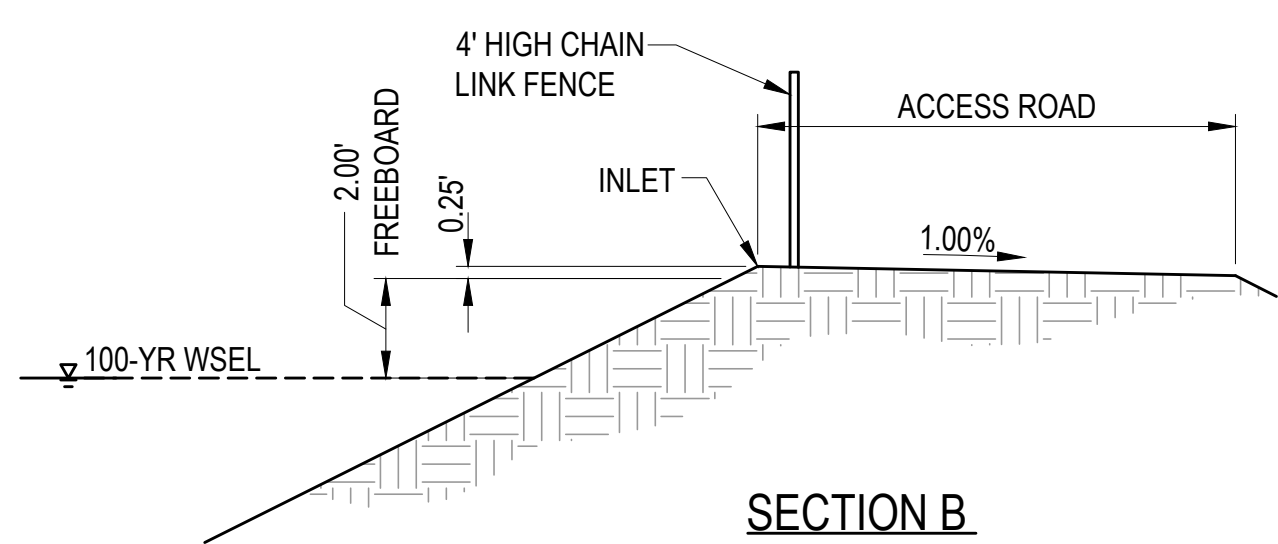
PLAN



SECTION A



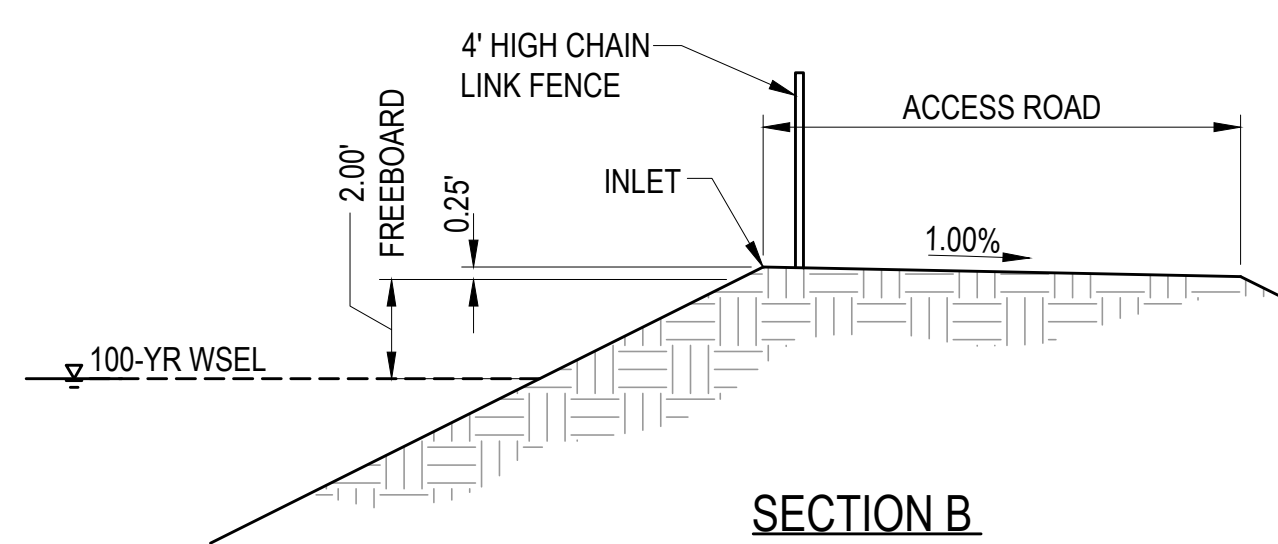
SECTION A



SECTION B

SPILLWAY

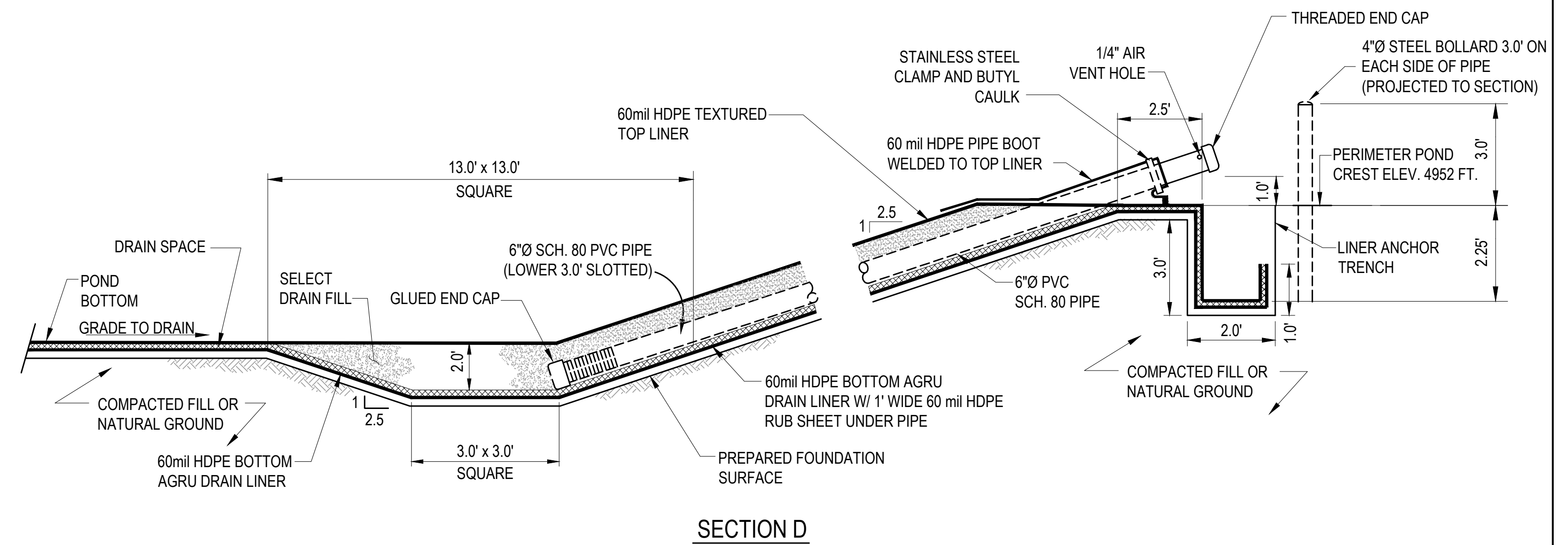
N.T.S. 5 021, 023, 027



SECTION B

OVERFLOW WEIR

N.T.S. 7 025



SECTION D

FIGURE 14

PRELIMINARY FOR AGENCY REVIEW

THEMAC RESOURCES

DO NOT SCALE 11x17 DRAWINGS

REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: NONE		DATE	
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT
0000-CI-023	IMPACTED STORMWATER IMPOUNDMENT A, PLAN VIEW														
0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW														

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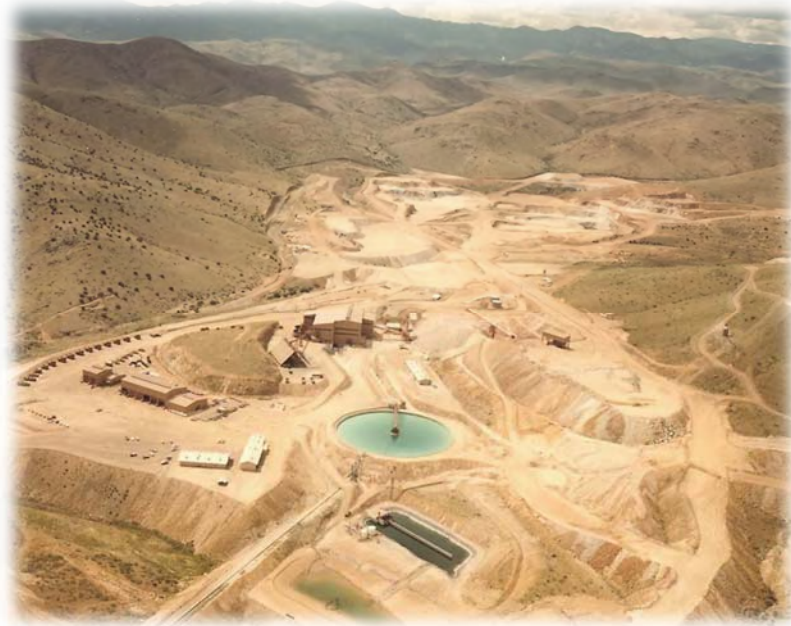
COPPER FLAT PROJECT

GENERAL/STANDARDS CIVIL STANDARD DETAILS SHEET 3

JOB NO. M3 PN-120085
DWG NO. 0000-CI-010
REV. NO. P3
DATE 09 OCT 15

File: P:\2023\230803\CIVIL (6443)44.2 - Dept\Updates for Permit\00000000.dwg LAST UPDATE: 12/7/2015 12:45 PM BY: AN060

Copper Flat Project



Process Facility Containment Report

Prepared For:

THEMAC
RESOURCES 

Certified Professional Engineer Seal

This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.


Signature




Exp. 12/31/2017

Date 12/7/2015

PROCESS FACILITY CONTAINMENT REPORT
COPPER FLAT PROJECT

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4 PROCESS FACILITY CONTAINMENT AREAS	4
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Harry J. Lewisley 12/7/2015

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1 INTRODUCTION

The Copper Flat Project is located in South Central New Mexico, near the town of Hillsboro, approximately 150 miles south of Albuquerque, and approximately 20 miles southwest of Truth or Consequences (straight-line distances) as illustrated in Drawing (Dwg.) 000-CI-001. The Project is owned and operated by New Mexico Copper Corporation (NMCC), a wholly owned subsidiary of THEMAC Resources Group Limited.

The State of New Mexico has promulgated regulations pertaining to groundwater protection at copper mining facilities (New Mexico Administrative Code Title 20, Chapter 6, Part 7 [20.6.7 NMAC], the "Copper Rule"), the stated purpose of which is "to control discharges of water contaminants specific to copper mine facilities and their operations to prevent water pollution."

This report provides the design criteria, location, and capacity of containment systems for the process areas identified in Section 2 of this report to comply with 20.6.7 NMAC, Sections 22, 23, and 26. This report excludes design considerations for the Impacted Stormwater Impoundments, Process Water Reservoir, and Tailings Storage Facility (TSF), i.e., the tailings impoundment, underdrain collection pond, surge pond and the secondary containment trench from the processing facility to the TSF, which have been completed by others and are reported separately.

2 PROCESS AREA IDENTIFICATION

Pursuant to 20.6.17.22 and .23, tanks and pipelines for new construction as proposed for the Copper Flat Project are required to be designed with adequate containment to prevent migration of process solutions to groundwater. Specific areas of the project for which the containment designs are described in this report include the following areas with tanks that require secondary containment.

- Concentrator Area
- Truck Shop
- Fuel Station
- Process Water Storage Tank

This report also describes containment for the following areas that contain process equipment and/or liquids that require containment to prevent release to the environment.

- Crusher
- Coarse Ore Stockpile and Ore Reclaim Tunnel
- Truck and Equipment Washing Unit
- Analytical Laboratory
- Domestic Sewage Treatment Facility

Discussion of containment of facilities related to the tailings storage facility (TSF) (tailings pipelines, tailings storage facility), impacted stormwater impoundments, and the process water reservoir is not included in this report.

The following containment areas will be covered by a roof and therefore will not collect appreciable precipitation.

- Grinding
- Copper Flotation
- Molybdenum Flotation
- Copper Filtration
- Diesel Reagents
- Flotation Reagents
- Sodium Hydrosulfide Reagents
- Truck Shop Tank Farm
- Assay Laboratory
- Wastewater Treatment Facility

The following areas will be uncovered and will therefore collect precipitation. Monthly inspections of these facilities are required by 2.6.7.22 and .23 NMAC. However, after occurrence of a significant precipitation event, the collection sumps will be inspected and collected water will be evacuated within 30 days to maintain required containment capacity. Stormwater evacuated from these areas will be added to the process reservoir or process stream for disposal.

- Copper-Moly Thickening
- Copper Thickening
- Lime Reagents
- Wheel Wash
- Fuel Station
- Truck and Equipment Washing Unit
- Process Water Storage Tank

3 BASIS OF DESIGN

Tanks and pipelines for the Copper Flat Project are designed in accordance with NMAC 20.6.7.23 (A). Most process pipelines are located within buildings with concrete floors sloped to drain to containment sumps. Process pipelines located outside of buildings will have secondary containment for management of leaks and spills. Pipelines are designed in accordance with the following specifications.

Construction Materials	20.6.7.23.A.(1).(a) NMAC	Impermeable materials compatible with contents and resistant to corrosion or degradation
Leak Detection	20.6.7.23.A.(1).(b) NMAC	Mechanism for monitoring integrity
Containment	20.6.7.23.A.(1).(c) NMAC	Secondary containment provided by building floors, double-walled piping, or lined trenches.

Tanks containing process solutions, reagents, chemicals, and petroleum products are located within a building area or on concrete containment pads with curbs or stem walls that are designed to be impermeable to the contents of the contained tanks. Joints between containment curbs and floors will be sealed with approved water stops to seal against leaks. Tanks are designed to contain the stored material, prevent overflows, and are resistant to corrosion or degradation. Tanks for this project (Table 1) are above ground and designed in accordance with the following specifications, except where specifically noted.

Construction Materials	20.6.7.23.A.(2).(a) NMAC	Impermeable materials compatible with contents and resistant to corrosion or degradation
Foundation	20.6.7.23.A.(2).(b) NMAC	Constructed concrete foundation
Containment	20.6.7.23.A.(2).(c) and 20.6.7.23.A.(2).(d) NMAC	Concrete containment to prevent run-on. Containment volume sized to contain 110% of volume of largest tank in the enclosed area while accounting for unusable volume with the containment due to the presence of other tanks, equipment, pedestals, etc.

The majority of process solution pipelines will be placed inside secondary containment for tanks and process buildings. When process pipelines cannot be positioned within secondary containment for tanks or process buildings, the pipe will either be double walled and positioned to drain to an established containment area, or secondary containment specific to the pipeline will be constructed.

Containment sumps will be sealed with water stops at joints and coatings applied where necessary to provide a watertight seal in order to prevent leaking solution to the environment. A list of containment sumps and the destination of liquids collecting in the sumps is provided as Table 2.

4 PROCESS FACILITY CONTAINMENT AREAS

Containment of process solutions, reagents, and other potential groundwater contaminants associated with the Copper Flat Project is delineated below by process area. The areas described include the primary mineral processing circuit and other ancillary areas that are essential to the operation of the project, such as vehicle washing and maintenance, fueling operations, assaying, and sanitary wastewater treatment. The portions of the project area addressed in this report are identified in Dwg. 0000-CI-008. Containment of process solutions is discussed below by process area and by location of ancillary facilities. A list of tanks designed for installation at the Copper Flat Project is presented as Table 1. Table 1 provides location, dimensions, material of construction, and capacity of the tanks in gallons. These tank capacities are compared with the capacities of containments in which they are located to demonstrate adequate containment.

Containment is provided around tanks and in areas with process piping to prevent loss of contents, control leaks and spills, and prevent release of solutions to the surface or ground water. Concrete containment walls are used to contain leaks and spills where tanks are present. The net volume of the containment is designed to contain at least 110 percent of the volume of the largest tank or series of connected tanks in the system. Any leakage, spillage, or wash water within a containment area is directed by sloped concrete floors to a watertight drainage sump. A list of the sumps present in each of the containment areas and the disposition of the captured solutions is presented in Table 2. The sumps listed in Table 2 are containment sumps.

4.1 CONCENTRATOR AREA

Drawing 3000-FS-000 summarizes the Copper Flat ore process flowsheet. Processing of the ore begins with the Crusher. Crushed ore is conveyed to the Coarse Ore Stockpile to be used as mill feed. A concrete tunnel beneath the Coarse Ore Stockpile is used to reclaim the crushed ore and convey it to the Grinding Area. Coarse ore is combined with water in the grinding mills to create a slurry that flows by gravity to Copper Flotation. Concentrate containing copper (Cu) and molybdenum (Moly) is pumped to the Copper-Molybdenum Thickening area and then to the Molybdenum Flotation area for separation of Copper and Moly. Moly Concentrates are filtered, dried, and bagged for shipment in this area. The remaining Copper concentrate is pumped to the Copper Thickening area where excess water is removed. Thickened Copper concentrates are pumped to the Copper Filtration area for dewatering prior to loading onto trucks for offsite shipment. The wheels of trucks and trailers loaded with Copper concentrate are washed in the Wheel Wash area to prevent the loss of concentrate and contamination of roadways.

Chemicals used in the process described above are stored in the Reagents area. Several containment areas are present in the Reagent area to isolate chemicals that may be incompatible or require special handling. Containment areas in the Reagents area include Lime Containment, Diesel Containment, General Reagents Containment, and Sodium Hydrosulfide (NaHS) Containment. The sumps are equipped with a dedicated sump pump to remove the contained liquids and transfer them to an appropriate location in the process.

An overall plan of the Concentrator Area is provided in Dwg. 0000-GA-050. Locations of tanks and sumps are shown on the drawing along with flow arrows depicting the slope of the floor surfaces to demonstrate flows to the collection sumps for the identified areas. A table on the drawing provides the volume of the largest tank in a given area and compares it with the capacity of the containment sump in which it resides.

The concentrator perimeter will be enclosed by curbing to prevent migration of process solutions away from the concentrator building. Unless specified below, general containment curbing at the perimeter of the concentrator will be at least 4 inches tall.

4.1.1 Crushing Area

The Crusher Area (Dwg. 0000-CI-008) does not contain any tanks. The crusher takes large blocks of ore recovered from the mine pit and crushes it to sizes that can be efficiently handled in the grinding area. Crushing is a dry process, but the crusher dump pocket is open to precipitation. Water is used in the crusher to suppress dust during the transfer of crushed ore to the conveying system to feed the Coarse Ore Stockpile. Water is used to wash down areas within the crusher that accumulate dust and dirt. Precipitation and the moisture content of the ore also contribute minor amounts of water that will collect at the bottom of the crusher structure.

A collection sump located in the lowest level of the concrete crusher facility collects water within the primary crusher and pump it to the Coarse Ore Reclaim Tunnel sump using a dedicated sump pump. The sump has a capacity of approximately 560 gallons. The sump pump will be configured for automatic start when solution is detected and the capacity of the crusher sump is appropriate for the potential solution flows.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Crushing Area				
No Storage Tanks	na	na	na	na
Containment Volume (gallons)			560	

4.1.2 Coarse Ore Stockpile and Ore Reclaim Tunnel Area

The Coarse Ore Stockpile and Reclaim Tunnel Area (Dwg. 0000-CI-008) does not contain any tanks. The Coarse Ore Stockpile accumulates crushed ore for delivery to the grinding area of the concentrator for processing. An existing concrete Reclaim Tunnel lies beneath this area and will be used to recover crushed ore at a controllable rate to supply feed to the grinding circuit. Process water and fresh water piping to the Concentrator pass through the Reclaim Tunnel.

Water used for wash down and dust suppression, as well as the water pumped from Crusher sump, drains to a sump in the bottom of the Reclaim Tunnel. Process water or fresh water solution coming from piping in the tunnel and moisture from precipitation or excess water contained in the crushed ore will drain to the Reclaim Tunnel sump.

A dedicated sump pump will be used to transfer collected water from the Reclaim Tunnel sump to the concentrator through the Primary Cyclone Feed system. The Reclaim Tunnel Sump has a capacity of approximately 6,400 gallons. The sump pump will be configured for automatic start when solution is detected and the capacity of the Reclaim Tunnel sump is appropriate for the potential solution flows. The installed pumping capacity will be sufficient to evacuate the sump at a flow rate equal to the rate that may be generated by a broken water line.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Reclaim Tunnel Area				
No Storage Tanks	na	na	na	na
Containment Volume (gallons)			6,400	

4.1.3 Grinding Area

A conveyor from the Reclaim Tunnel delivers crushed ore to the semi-autogenous grinding (SAG) mill. Water is added to the SAG mill to begin the grinding process. Discharge from the SAG mill is pumped to the Primary Cyclone classifiers in closed circuit with the Ball Mill. The slurry is circulated through the Ball Mill until the particles are fine enough to flow out of the top of the cyclones and on the Rougher Flotation Conditioning Tank. A portion of the circulating slurry will go through a gravity concentration circuit to recover separable gold prior to fine grinding.

The only tank in the Grinding Area (Dwg. 0000-GA-050) is the Gravity Concentrate Tank. This tank accumulates concentrate from gravity gold separation and will typically be filled with 8,000 gallons of wet solids. The Grinding Area containment volume is 200,000 gallons. The large containment volume in this area is not based on required capacity but rather results from requirements for setup and configuration of the grinding equipment. Therefore, the resulting containment capacity is more than adequate to contain the contents of the gravity concentrate tank and any operating upsets or maintenance conditions that may occur in the grinding circuit or elsewhere in the concentrator area.

All leakage or spillage of liquids or water used to wash down equipment and floors in this area reports to the Grinding Area Containment sump. The sump is equipped with a dedicated pump and automatic start/stop controls. Liquids accumulated in this sump are pumped back into the Primary Cyclone Feed system.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Grinding Area Containment				
Gravity Concentrate Tank	12.00	9.50	8,000	Carbon Steel
Containment Volume (gallons)			200,000	

4.1.4 Copper Flotation Area

Copper Flotation (Dwg. 0000-GA-050) includes the Rougher Conditioning Tank, rougher flotation cells, and other equipment to recover the sulfide minerals in the process slurry. The floor in the Copper Flotation area is sloped to facilitate gravity flow from one flotation cell to the next.

The floor slopes are directed to the Copper Flotation Area sump. Solutions accumulated in the Copper Flotation Area sump are returned to the Rougher Flotation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest tank the Copper Flotation Area is the Rougher Flotation Conditioning Tank with a capacity of 69,300 gallons, which requires a containment capacity of approximately 76,200 gallons at 110% of capacity. The net containment volume in the Copper Flotation Area is 79,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Copper Flotation Area Containment				
Rougher Flotation Conditioning Tank	22.00	25.00	69,300	Carbon Steel
Containment Volume (gallons)			79,000	

4.1.5 Copper-Molybdenum Thickening Area

The Copper-molybdenum (Copper-Moly) Thickening Area (Dwg. 0000-GA-050) recovers water from the rougher flotation concentrate in preparation for separating the moly into a saleable product. The area includes the flocculant mixing and distribution system, Copper-Moly thickener, and Copper-Moly thickener overflow tank.

The floor slopes are directed to the Copper-Moly Thickening Area sump. Solutions accumulated in the Copper-Moly Thickening Area sump are returned to the Copper-Moly Separation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Copper-Moly Concentrate Thickener at a capacity of 17,700 gallons., which requires a containment capacity of approximately 19,500 gallons at 110% of capacity. The designed containment volume is 29,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Copper-Moly Thickening Area Containment				
Flocculant Mix Tank	12.00	7.25	6,100	Carbon Steel
Flocculant Distribution Tank	12.00	7.25	6,100	Carbon Steel
Copper-Moly Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Copper-Moly Concentrate Thickener Overflow Tank	4.00	10.67	1,000	Carbon Steel
Containment Volume (gallons)			29,000	

4.1.6 Molybdenum Flotation Area

Molybdenum (Moly) Flotation (Dwg. 0000-GA-050) separates the moly from the copper concentrate and refines it through a series of flotation and cleaning steps into a saleable product that is filtered, dried, and bagged for shipment to the buyer. The area includes the copper-moly separation conditioning tank, moly filter feed tank, and moly filter cloth wash tank.

The floor slopes are directed to the Moly Flotation Area sump. Solutions accumulated in the Moly Flotation Area sump are returned to the Copper-Moly Separation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Moly Filter Feed Tank at a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons at 110% of capacity. The designed containment volume is 8,300 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Moly Flotation Area Containment				
Copper-Moly Separation Conditioning Tank	4.00	10.67	1,000	Carbon Steel
Moly Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Moly Filter Feed Tank	8.00	6.00	2,000	Carbon Steel

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Containment Volume (gallons)	8,300
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4.1.7 Copper Thickening Area

The Copper Thickening Area (Dwg. 0000-GA-050) receives the copper concentrate from the moly flotation circuit and dewateres it in preparation for filtration to make a shippable concentrate. The area includes a stock tank for storage of thickened concentrates to act as surge capacity for production. Copper Filtrate tanks receive water recovered from the filters for recycling through the thickener.

The floor slopes are directed to the Copper Thickening Area sump. Solutions accumulated in the Copper Thickening Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Copper Concentrate Stock Tank at a capacity of 41,760 gallons., which requires a containment capacity of approximately 46,000 gallons at 110% of capacity. The designed containment volume is 50,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Copper Thickening Area Containment				
Copper Concentrate Thickener Overflow Tank	4.00	4.00	375	Carbon Steel
Copper Concentrate Stock Tank	17.00	25.00	41,760	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Concentrate Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			50,000	

4.1.8 Copper Filtration Area

The Copper Filtration Area (Dwg. 0000-GA-050) does not contain any tanks. Thickened copper concentrate from the stock tank is pumped to a pair of plate-and-frame filters that produce a filter cake suitable for bulk shipment. The water recovered from the filters returns to the copper thickener via the Copper Filtrate Tanks that are located with the thickener. The filter cake drops to the floor in the Concentrate Loadout area and is loaded into concentrate trailers for shipment offsite.

The floor slopes are directed to the Copper Loadout Area sump. Solutions accumulated in the Copper Loadout Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The designed containment in the Copper Filtration Area is approximately 25,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Copper Filtration and Concentrate Loadout Area Containment				

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No Storage Tanks	na	na	na	na
Containment Volume (gallons)			25,000	

4.1.9 Wheel Wash Area

The Wheel Wash Area (Dwg. 0000-GA-050) consists of the wheel wash tank and pump area and a concrete containment area in which the wheels of the concentrate trucks are washed to remove adhering copper concentrates before driving off onto the site roads.

The floor slopes are directed to the Wheel Wash Area sump. Solutions accumulated in the Wheel Wash Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Wheel Wash Surge Tank at a capacity of 560 gallons., which requires a containment capacity of approximately 620 gallons at 110% of capacity. The designed containment volume is 15,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Wheel Wash Area Containment				
Wheel Wash Surge Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			15,000	

4.1.10 Reagents Area

The Reagents Area (Dwg. 0000-GA-050) includes equipment and storage tanks to contain reagents used in the process. This area is configured into four separate containments to separate and manage Lime Reagent, Diesel Reagent, Flotation Reagent, and Sodium Hydrosulfide (NaHS) Reagent.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Concentrator: Lime Reagent Containment				
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Containment Volume (gallons)			30,000	
Concentrator: Diesel Reagent Containment				
No. 2 Diesel Storage Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gallons)			4,800	
Concentrator: Flotation Reagent Containment				

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Pax Mix Tank	8.00	10.67	4,000	Carbon Steel
Pax Distribution Tank	8.00	10.67	4,000	Carbon Steel
MIBC Storage Tank	8.00	6.00	2,000	Carbon Steel
AERO 238 Mix Tank	8.00	10.67	4,000	Carbon Steel
AERO 238 Distribution Tank	8.00	10.67	4,000	Carbon Steel
Moly Collector Mix Tank	8.00	6.00	2,300	Carbon Steel
Moly Collector Distribution Tank	8.00	6.00	2,300	Carbon Steel
Containment Volume (gallons)			10,200	
Concentrator: Sodium Hydrosulfide Reagent Containment				
NaHS Mix Tank	8.00	10.67	4,000	Stainless Steel
NaHS Distribution Tank	8.00	10.67	4,000	Stainless Steel
NaHS Stock Tank	8.00	10.67	4,000	Stainless Steel
Containment Volume (gallons)			4,900	

4.1.10.1 Lime Reagent Area

The Lime Reagent Area (Dwg. 0000-GA-050) consists of a Lime Silo, Lime Slaker, and Milk of Lime tanks to receive pebble lime from the supplier and convert it to a lime slurry that can be used to raise the pH of process solutions.

The floor slopes are directed to the Lime Reagent Area sump. Solutions accumulated in the Lime Reagent Area sump are returned to the milk of lime distribution box. The containment will be inspected on a regular basis per regulatory requirements and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The largest vessels are the Milk of Lime Tanks at a capacity of 20,000 gallons., which requires a containment capacity of approximately 22,000 gallons at 110% of capacity. The designed containment volume is 30,000 gallons.

4.1.10.2 Diesel Containment Area

Diesel fuel is used as a reagent in the flotation process. It is stored in the Diesel Tank in the Diesel Containment (Dwg. 0000-GA-050) and has its own containment area and sump.

The floor slopes are directed to the Diesel Containment Area sump. Leaked or spilled diesel fuel accumulating in the sump is pumped out using a portable pump and deposited in the Used Oil Storage Tank (Sec. 4.2). The containment will be inspected on a regular basis per regulatory requirements and solutions evacuated as soon as observed, and the source will be investigated and addressed.

The Diesel Containment Tank has a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons. The designed containment volume is 4,800 gallons.

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4.1.10.3 Flotation Reagent Area

The Flotation (General) Reagents Area contains tanks with compatible aqueous reagents that are used in the process.

The floor slopes are directed to the Flotation Reagent Area sump. Solutions accumulating in the sump are pumped to the tailings box to be combined with tailings and report to the TSF. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. The sump is equipped with a dedicated pump and manual start/stop control.

The largest vessels are the Aero 238 Tanks at a capacity of 4,000 gallons., which requires a containment capacity of approximately 4,400 gallons. The designed containment volume is 10,200 gallons.

4.2 TRUCK SHOP

The Truck Shop is located south east of the concentrator (Dwg. 0000-CI-008) and provides maintenance services for the mining and mobile equipment fleet. Tanks will be constructed at the truck shop (Dwg. 1010-AR-012) to store required maintenance fluids such as: motor oil, gear oil, hydraulic oil, engine coolant, and used oil and coolant for recycling.

The floor slopes are directed to the Truck Shop Tank Farm Area sump. Leaked or spilled diesel fuel accumulating in the sump is pumped out using a portable pump and deposited in the Used Oil Storage Tank for recycling. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The Used Oil Storage Tank has a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons. The designed containment volume is 5,600 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Truck Shop Tank Farm Containment				
Used Oil Storage Tank	8.00	6.00	2,000	Carbon Steel
Used Anti-Freeze Storage Tank	8.00	6.00	2,000	Carbon Steel
ATF Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Hydraulic Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Engine Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Gear Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Anti-Freeze/Coolant Storage Tank	6.00	6.00	1,000	Carbon Steel
Containment Volume (gallons)			5,600	

4.3 FUEL STATION

A Fuel Station with secondary containment for project vehicles will be constructed on the haulage road west of the concentrator (Dwg. 0000-CI-008). Tanks, pumps, and piping associated with the fuel station will be positioned inside

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the containment facility (Dwg. 1010-GA-010). During fueling and fuel offloading operations, vehicles and equipment will be parked on concrete pads that are sloped to drain into solution containment sumps. Liquid from the sumps will be evacuated into a portable used oil tank for transfer to the used oil holding facility for recycling.

The floor slopes at the fueling areas and within the tank containment are directed to the Fuel Station Area sumps. Solutions accumulated in the Fuel Station Area sumps are pumped out and transported offsite by a Certified used oil recycler. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The largest vessel is the Off Road Diesel Fuel Storage Tank at a capacity of 100,000 gallons., which requires a containment capacity of approximately 110,000 gallons at 110% of capacity. The designed containment volume is 119,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Fuel Station Containment				
Off Road Diesel Fuel Storage Tank	28.00	24.00	100,000	Carbon Steel
On Road Diesel Storage Tank	12.00	12.00	10,000	Carbon Steel
Gasoline Storage Tank	12.00	12.00	10,000	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			119,000	

4.4 TRUCK AND EQUIPMENT WASHING UNIT

A Truck and Equipment Washing Unit (Truck Wash) will be constructed along the haulage road from the mine to the Truck Shop (Dwg. 0000-CI-008). This facility is designed in accordance with 20.6.7.26 NMAC. The washing unit includes a concrete pad on which equipment will be parked for washing (Dwg. 1010-GA-001). The pad will be sloped to drain into a concrete basin for separating water, solids, oil, and grease. The water from the basin will be recycled and reused for washing equipment. Oil and grease will be skimmed from the settling basin and stored in sealed drums for proper disposal. After draining, sediment will be removed from the basin and either loaded directly into a dump truck for disposal at tailings or stored on a concrete pad adjacent to the facility for re-handling and disposal at the TSF. The settling basin will be designed to provide at least 12 inches of freeboard during operation. During periods of precipitation, excess water will be removed from the settling basin by a water truck and transported to the a process water impoundment for reuse.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Equipment Wash Pad				
Settling Basin	40' W x 50' L x 3' D		50,000	Concrete
Containment Volume (gallons)			50,000	

4.5 ASSAY LABORATORY

The Assay Laboratory (Dwg. 0000-CI-008) uses a variety of reagents for sample process and analytical testing of mine and process samples. Sinks and drains in areas with potential chemical use are piped to a below-ground holding tank (Dwg. 3010-AR-003) so that they are not commingled with sanitary wastes from the building's restrooms and breakroom area. The chemical holding tank will be installed in a concrete, water tight vault as secondary containment.

The floor slopes in the Assay Lab Chemical Waste tank containment are directed to a collection sump. Solutions accumulated in the Assay Lab Chemical Waste sump are pumped out and taken to the TSF or transferred to a portable tank and removed from site by a certified disposal contractor. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The only vessel is the Chemical Waste Tank at a capacity of 1,200 gallons., which requires a containment capacity of approximately 1,300 gallons at 110% of capacity. The designed containment volume is 7,500 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Assay Laboratory				
Assay Lab Chemical Waste Tank	4.00	14.00	1,200	Polypropylene
Containment Volume (gallons)			7,700	

4.6 DOMESTIC WASTEWATER TREATMENT FACILITY

A packaged wastewater treatment facility (WWTF) for treatment of domestic sanitary will be installed on an existing concrete slab located near the Guardhouse at the main entrance (Dwg. 0000-CI-008). Concrete, water tight curbing will be installed at the perimeter of the foundation slab to provide a minimum volume of 5,000 gallons of secondary containment for the facility. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Sanitary Wastewater Treatment Facility				
Sewage Treatment Equalization Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Treatment Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Solids Holding Tank	9.60	8.00	4,300	Carbon Steel
Containment Volume (gallons)			5,000	

**PROCESS FACILITY CONTAINMENT REPORT
COPPER FLAT PROJECT**

4.7 PROCESS WATER STORAGE TANK

Process water from the Process Water Reservoir will be pumped to the Process Water Storage Tank (Dwg. 0000-CI-008) on the slopes of Animas Peak to provide consistent line pressure using gravity head to supply water for use in the process. The tank will be installed within a lined earthen basin with a secondary containment capacity of at least 190,000 gallons, plus 2 feet of freeboard. The containment basin will be lined with 60 mil HDPE liner or equivalent. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump. Any water collecting in the basin will be pumped out within 30 days and deposited in the Process Water Reservoir.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Miscellaneous Site				
Process Water Head Tank	30.00	32.00	170,000	Carbon Steel

TABLES

Table 1: List of Tanks and Process Solution Containment Areas

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
Concentrator: Grinding Area Containment				
Gravity Concentrator Concentrate Tank	12.00	9.50	8,000	Carbon Steel
Containment Volume (gal)			200,000	
Concentrator: Copper Flotation Area Containment				
Rougher Flotation Conditioning Tank	22.00	25.00	69,300	Carbon Steel
Containment Volume (gal)			79,000	
Concentrator: Copper Regrind Area Containment				
Copper 1st Cleaner Conditioning Tank	12.00	7.25	6,000	Carbon Steel
Containment Volume (gal)			8,900	
Concentrator: Moly Flotation Area Containment				
Cu-Mo Separation Conditioning Tank	4.00	10.67	1,000	Carbon Steel
Moly Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Moly Filter Feed Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gal)			8,300	
Concentrator: Copper-Moly Thickening Area Containment				
Flocculant Mix Tank	12.00	7.25	6,100	Carbon Steel
Flocculant Distribution Tank	12.00	7.25	6,100	Carbon Steel
Cu-Mo Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Cu-Mo Concentrate Thickener Overflow Tank	4.00	10.67	1,000	Carbon Steel
Containment Volume (gal)			13,000	
Concentrator: Copper Thickening Area Containment				
Cu-Mo Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Cu Concentrate Thickener Overflow Tank	4.00	4.00	375	Carbon Steel
Cu Concentrate Stock Tank	17.00	25.00	41,760	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Cu Concentrate Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			50,000	
Concentrator: Copper Filtration and Concentrate Loadout Area Containment				
No Storage Tanks				
Containment Volume (gal)			25,000	

Table 1: List of Tanks and Process Solution Containment Areas

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
Concentrator: Copper Filtration Wheel Wash Area Containment				
Wheel Wash Surge Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			15,000	
Concentrator: Lime Reagent Containment				
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Containment Volume (gal)			30,000	
Concentrator: Diesel Reagent Containment				
No. 2 Diesel Storage Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gal)			4,800	
Concentrator: General Reagent Containment				
Pax Mix Tank	8.00	10.67	4,000	Carbon Steel
Pax Distribution Tank	8.00	10.67	4,000	Carbon Steel
MIBC Storage Tank	8.00	6.00	2,000	Carbon Steel
AERO 238 Mix Tank	8.00	10.67	4,000	Carbon Steel
AERO 238 Distribution Tank	8.00	10.67	4,000	Carbon Steel
Moly Collector Mix Tank	8.00	6.00	2,300	Carbon Steel
Moly Collector Distribution Tank	8.00	6.00	2,300	Carbon Steel
Containment Volume (gal)			10,200	
Concentrator: Sodium Hydrosulfide Reagent Containment				
NaHS Mix Tank	8.00	10.67	4,000	Stainless Steel
NaHS Distribution Tank	8.00	10.67	4,000	Stainless Steel
NaHS Stock Tank	8.00	10.67	4,000	Stainless Steel
Containment Volume (gal)			4,900	

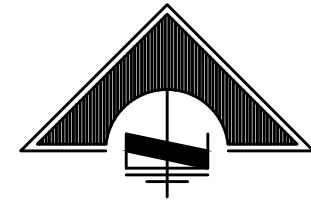
Table 1: List of Tanks and Process Solution Containment Areas

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
Truck Shop Tank Farm Containment				
Used Oil Storage Tank	8.00	6.00	2,000	Carbon Steel
Used Anti-Freeze Storage Tank	8.00	6.00	2,000	Carbon Steel
ATF Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Hydraulic Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Engine Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Gear Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Anti-Freeze/Coolant Storage Tank	6.00	6.00	1,000	Carbon Steel
Containment Volume (gal)			5,600	
Fuel Station Containment				
Off Road Diesel Fuel Storage Tank	28.00	24.00	100,000	Carbon Steel
On Road Diesel Storage Tank	12.00	12.00	10,000	Carbon Steel
Gasoline Storage Tank	12.00	12.00	10,000	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			119,000	
Equipment Wash Pad				
Settling Basin	40' W x 50' L x 3' D		50,000	Concrete
Containment Volume (gal)			50,000	
Miscellaneous Site				
Process Water Head Tank	30.00	32.00	170,000	Carbon Steel
Assay Lab Chemical Waste Tank	4.00	14.00	1,200	Polypropylene
Sewage Treatment Equalization Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Treatment Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Solids Holding Tank	9.60	8.00	4,300	Carbon Steel

Table 2: Containment and Cleanout Sumps

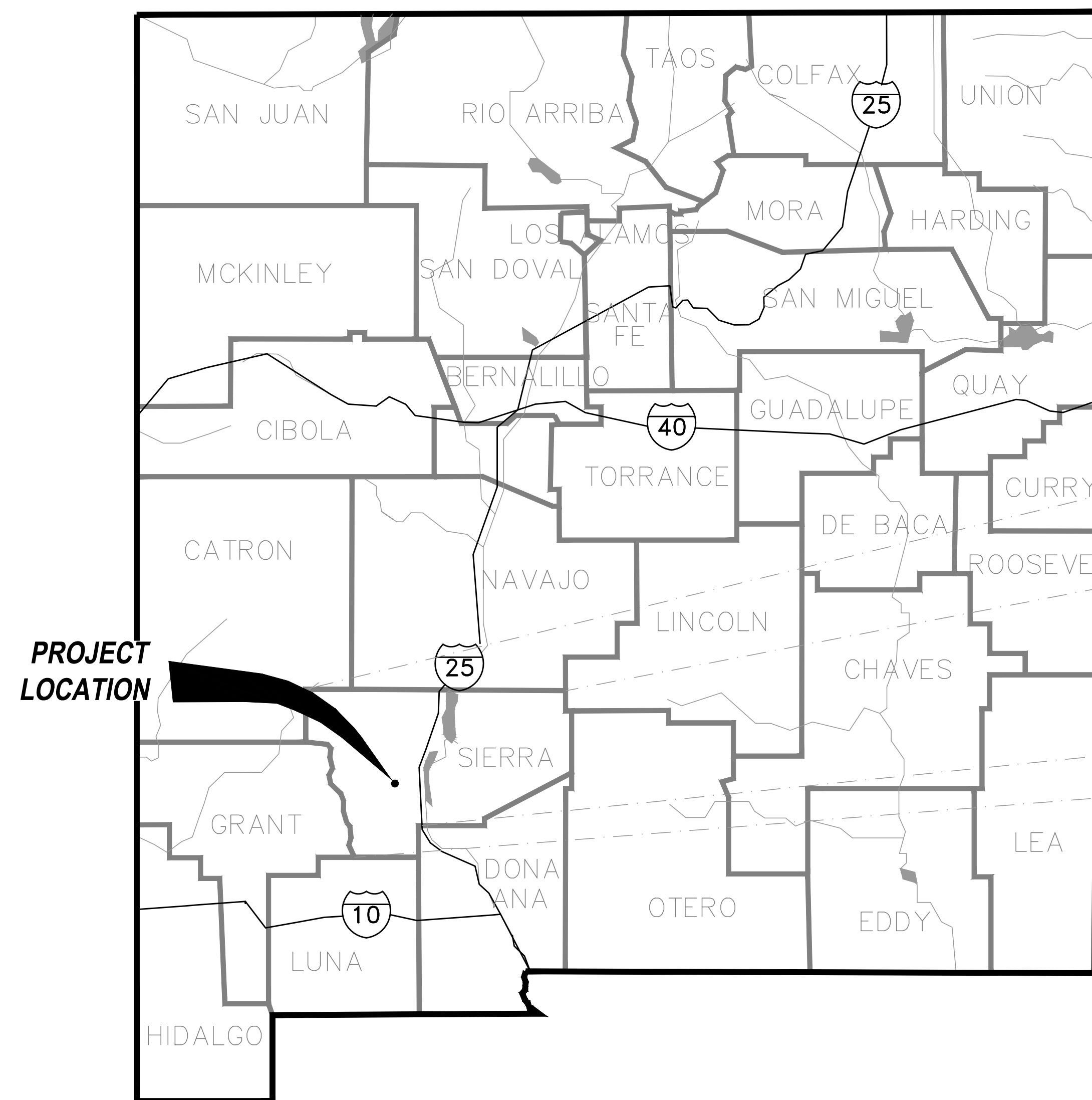
Location	Reports To
Crusher	Recalim Tunnel Sump
Reclaim Tunnel	Primary Cyclone Feed System
Grinding Area	Primary Cyclone Feed System
Copper Flotation Area	Rougher Flotation Conditioning Tank
Copper Regrind Area	Copper Regrind Cyclone Feed Box
Mo Flotation Area	Copper-Molybdenum Separation Conditioning Tank
Copper-Molybdenum Thickening Area	Copper-Molybdenum Thickener
Copper Thickening Area	Copper Concentrate Thickener
Copper Filtration and Concentrate Loadout Area	Copper Concentrate Thickener
Copper Filtration Wheel Wash Area	Copper Concentrate Thickener
Lime Containment Area	Lime Distribution Box
Diesel Reagent Containment Area	Used Oil Tank
General Reagent Containment Area	Tailings Sump
Sodium Hydrosulfide Reagent Containment Area	Copper-Molybdenum Separation Conditioning Tank
Mobile Equipment Shop Containment	Certified Used Oil Recycler
Fuel Station Containment	Certified Used Oil Recycler
Heavy Equipment Fuel Pad	Certified Used Oil Recycler
Light Vehicle Fuel/Fuel Offload Pad	Certified Used Oil Recycler
Equipment Wash Pad Settling Basin	Closed Basin; Zero Discharge
Process Water Head Tank Containment	Process Water Reservoir
Sewage Treatment Plant	Sewage Treatment Plant or Sanitary Waste Disposal Contractor
Chemical Waste Containment	TSF or Certified Contractor Disposal

DRAWINGS

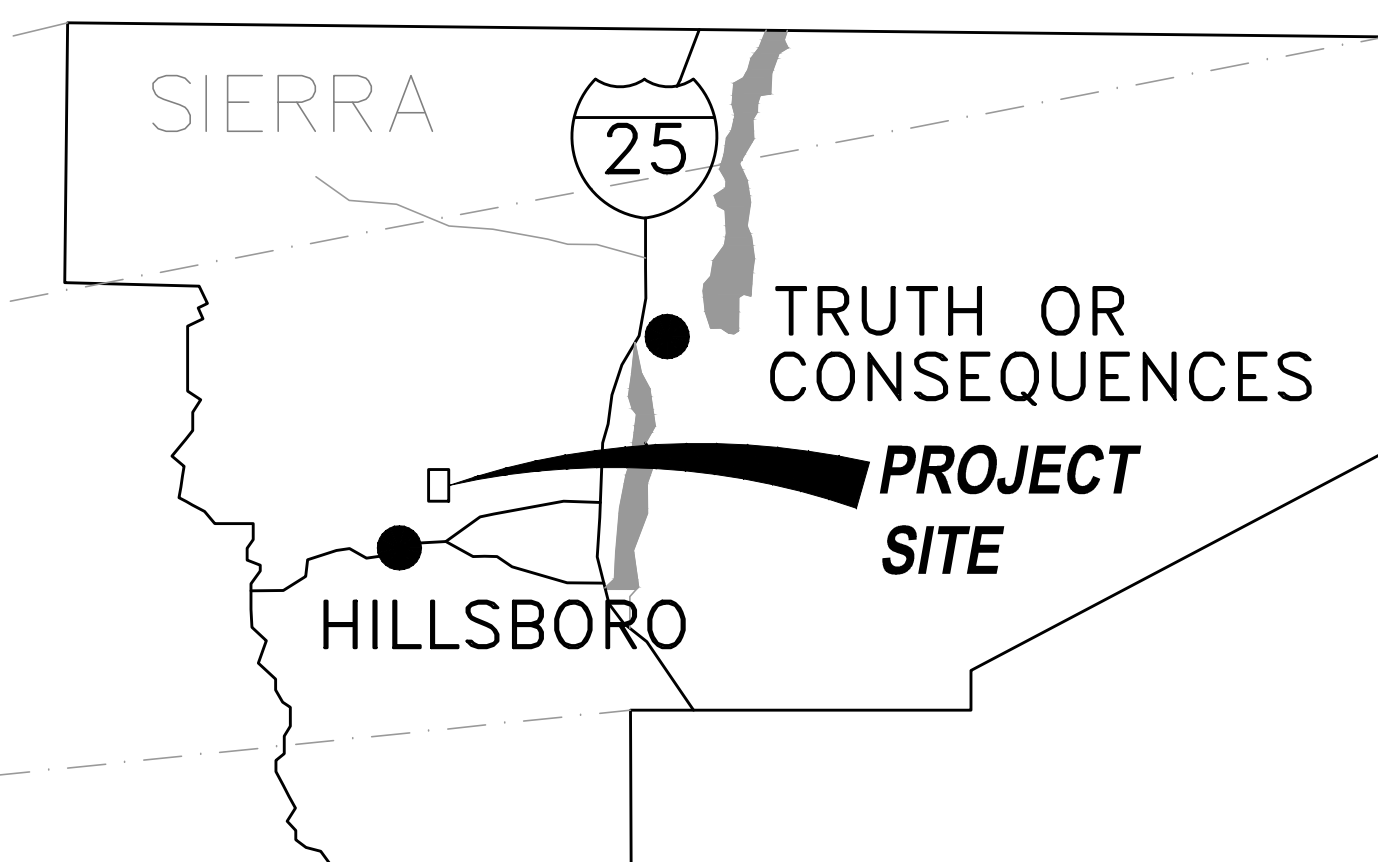


NEW MEXICO COPPER CORPORATION

COPPER FLAT PROJECT SIERRA COUNTY, NEW MEXICO



STATE OF NEW MEXICO
NOT TO SCALE



PROJECT LOCATION MAP
NOT TO SCALE

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FOR AGENCY REVIEW



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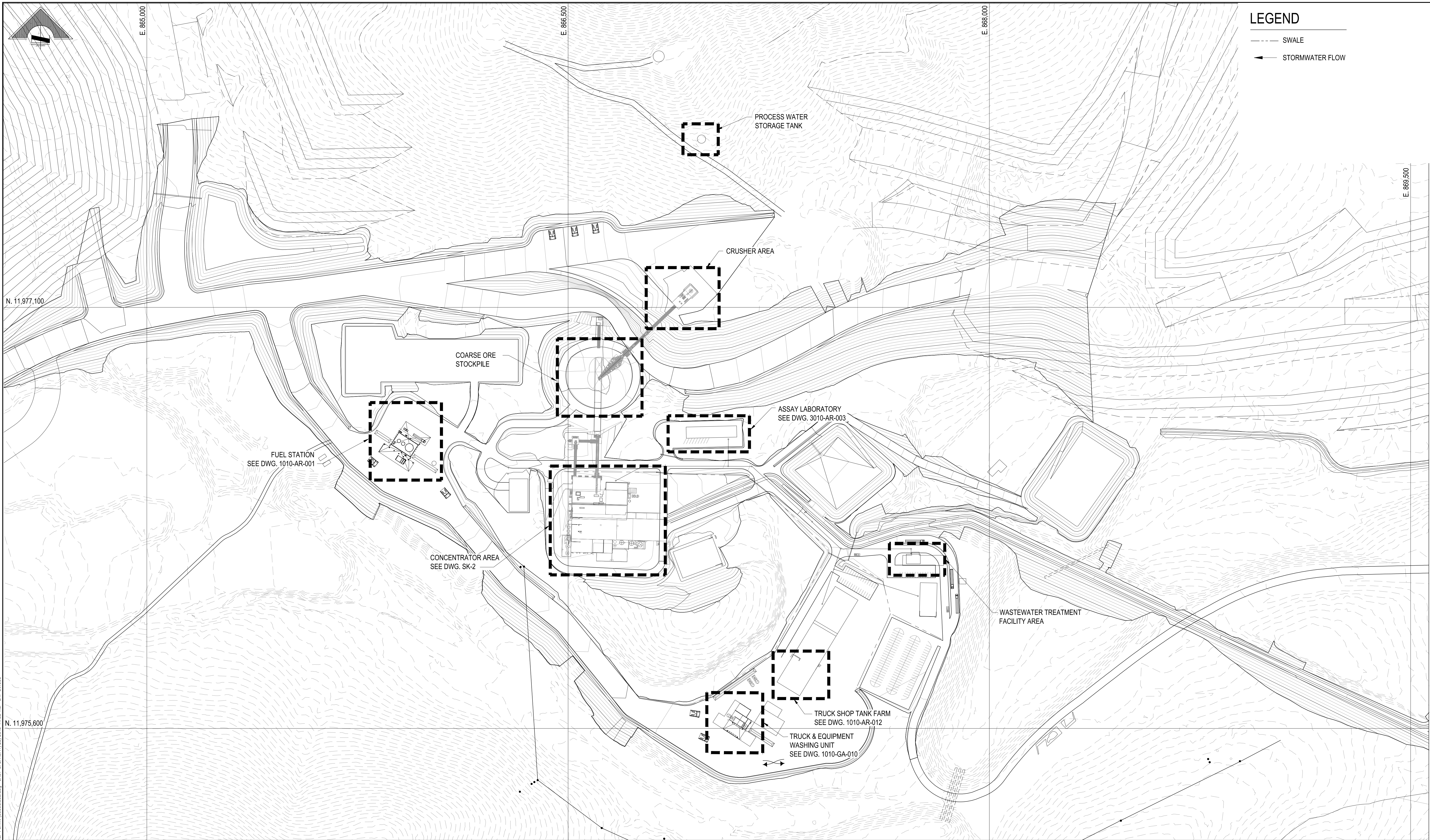


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COPPER FLAT PROJECT

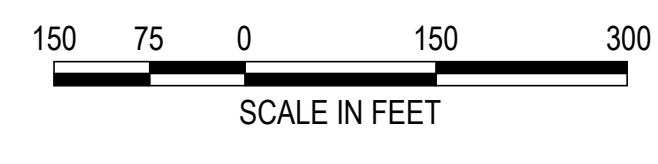
**SITE GENERAL
CIVIL
PROJECT AREA
SITE LOCATION PLAN**

JOB NO. M3 PN-120085	
DWG. NO. 0000-CI-001	DATE
REV. NO. P 4	DATE 05 MAR 13



LEGEND
 - - - - - SWALE
 ← STORMWATER FLOW

SITE PLAN
 SCALE: 1:150



PRELIMINARY
 FOR AGENCY REVIEW



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DRAWN BY:	SAM	DATE:	FEB 13
CHECKED BY:	TDL	DATE:	FEB 13
PROJECT MGR:	RKZ	DATE:	FEB 13
CLIENT APPR:			

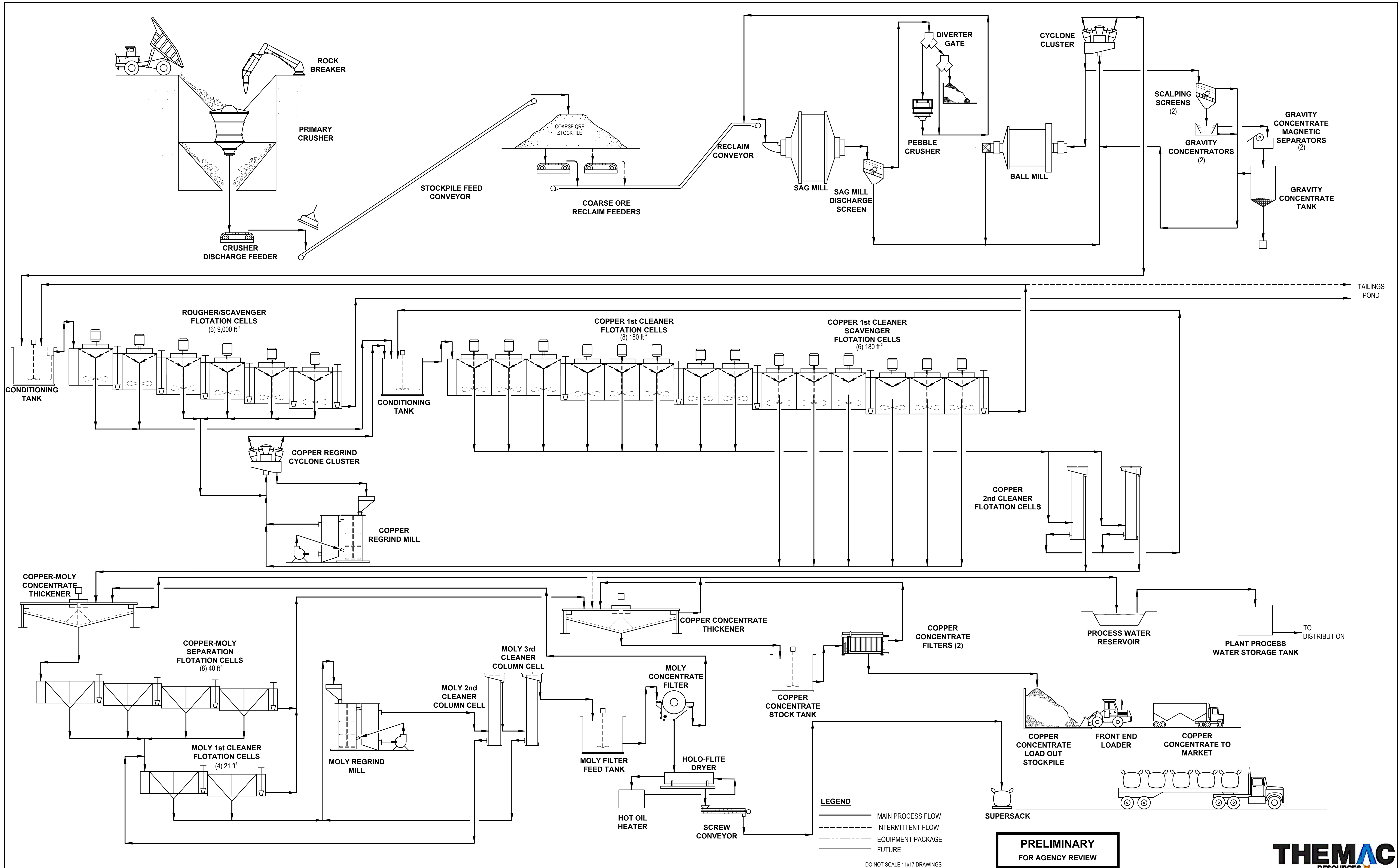
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COPPER FLAT PROJECT

**SITE GENERAL
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 PROJECT AREA
 PROCESS FACILITY CONTAINMENT AREAS**

JOB NO. M3 PN-120085
 DWG NO. **0000-CI-008**
 REV NO. p1
 DATE 25 NOV 15

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FOR AGENCY REVIEW

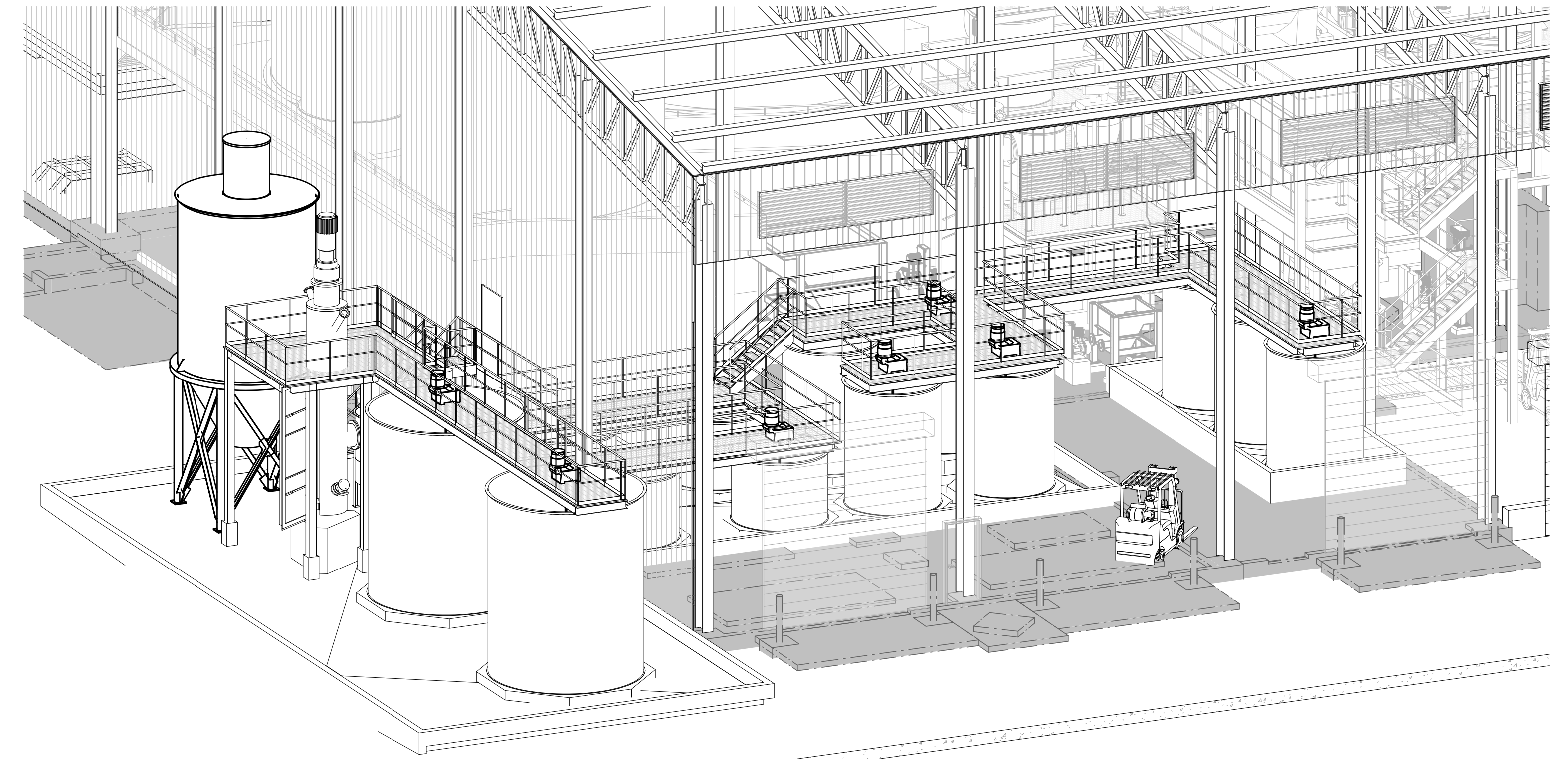


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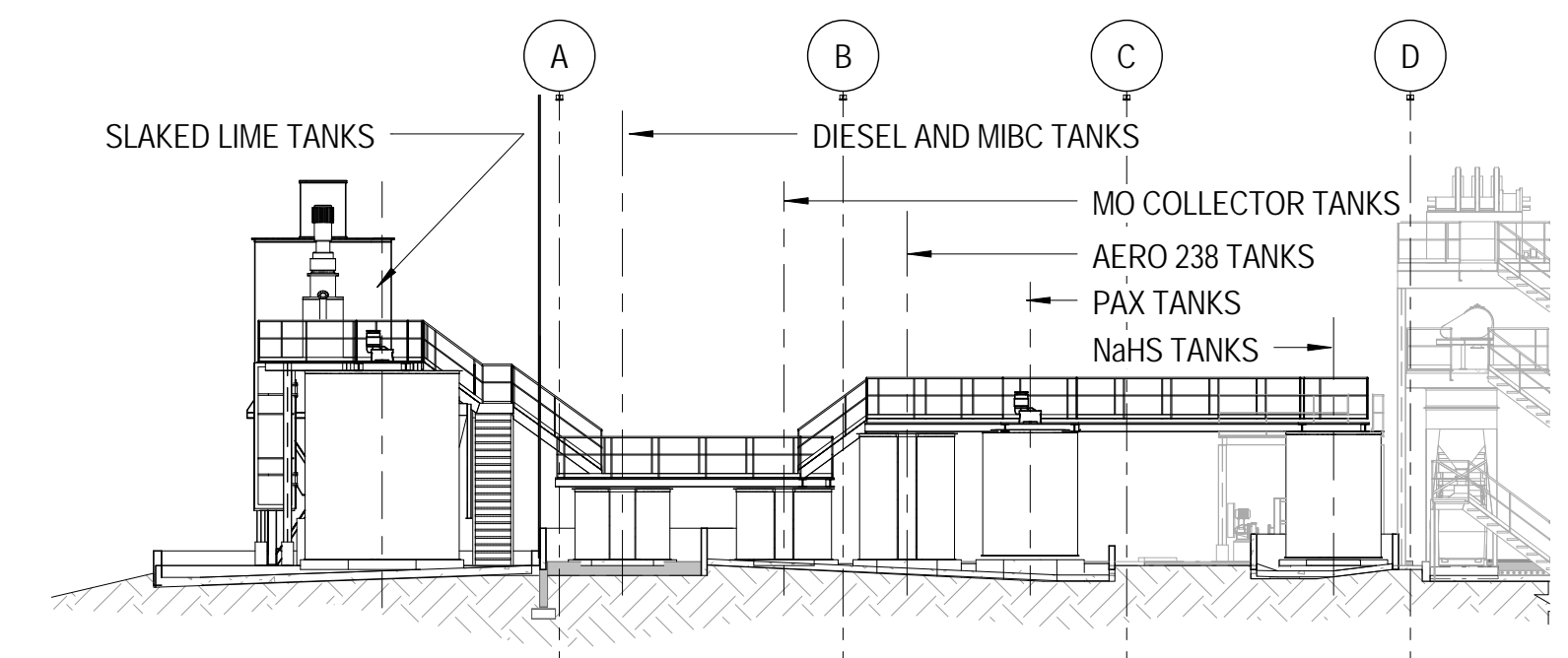
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COPPER FLAT PROJECT
PROCESSING PLANT
30,000 TPD
CONVENTIONAL TAILINGS
OVERALL FLOW SHEET

JOB NO. M3 PN-120065
DWG NO. **3000-FS-000**
REV NO. P6 DATE 18 MAR 13

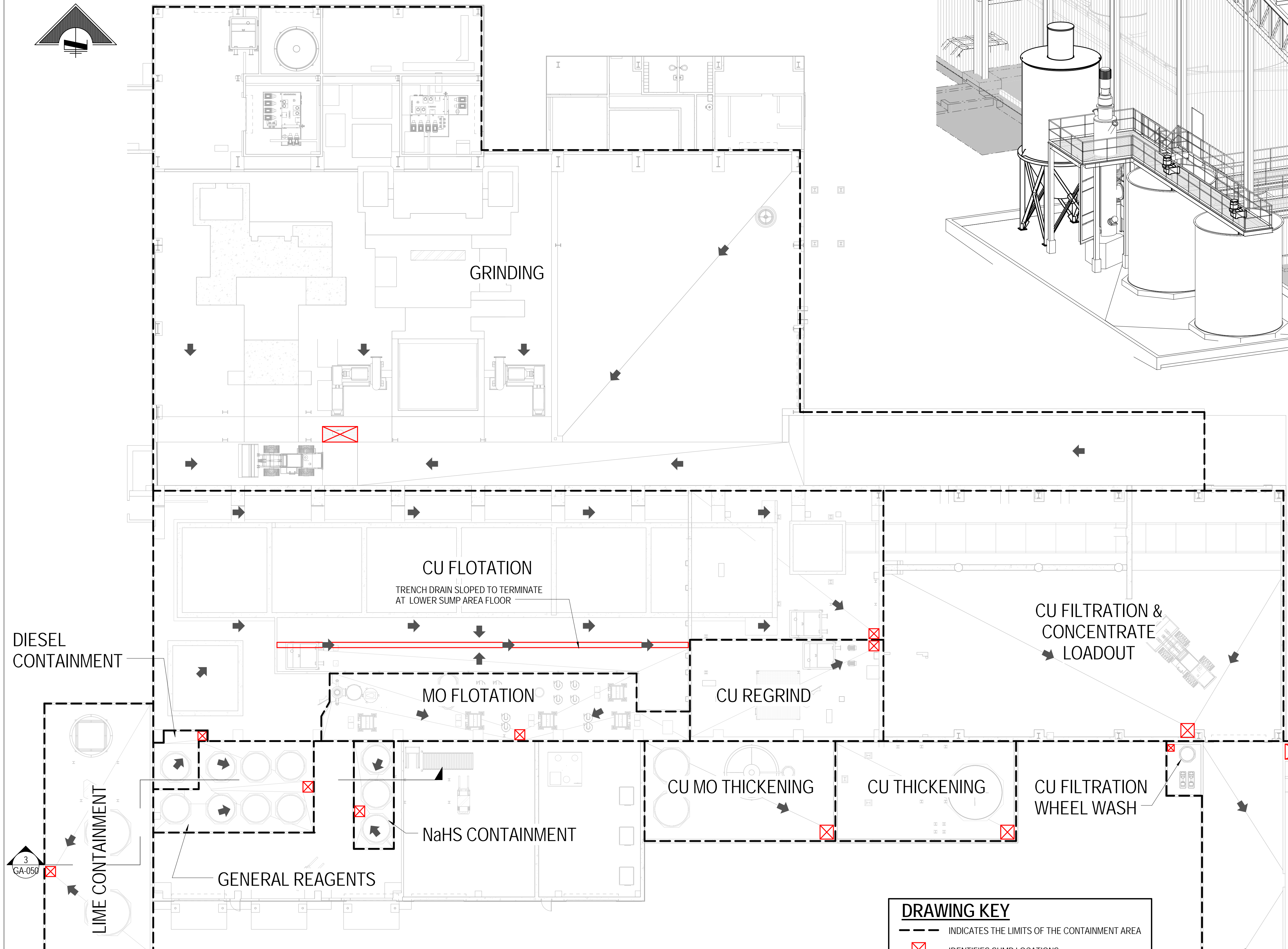


REAGENTS AREA ISOMETRIC



REAGENTS SECTION
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3
GA-050



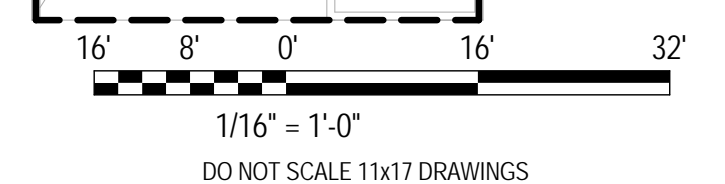
CONCENTRATOR CONTAINMENT PLAN

DRAWING KEY

- INDICATES THE LIMITS OF THE CONTAINMENT AREA
- ⊠ IDENTIFIES SUMP LOCATIONS
- ➔ DEMONSTRATES FLOW DIRECTION

Containment Area	Tank Name	Tank Volume (gal)	Containment Volume	
			Required (gal)	Provided (gal)
General Reagents	PAX Mix Tank*	4,000	4,400	10,200
	PAX Distribution Tank	4,000		
	MIBC Storage Tank	2,000		
	Moly Collector Mix Tank	2,300		
	Moly Collector Distribution Tank	2,300		
	AERO 238 Mix Tank	4,000		
	AERO 238 Distribution Tank	4,000		
Diesel Containment	No. 2 Diesel Storage Tank*	2,000	2,200	4,800
Lime Containment	Milk of Lime Storage Tank*	20,000	22,000	30,000
	Milk of Lime Storage Tank	20,000		
NaHS Containment	NaHS Distribution Tank*	4,000	4,400	4,900
	NaHS Mix Tank	4,000		
	NaHS Stock Tank	4,000		
Cu Flotation	Rougher Flotation Tank*	66,000	72,600	79,000
Cu Regrind	Cu 1st Cleaner Conditioning Tank*	1,000	1,100	8,900
Mo Flotation	Filter Feed Tank*	2,000	2,200	8,300
Cu Thickening	Cu Concentrate Stock Tank*	41,800	46,000	50,000
Cu-Mo Thickening	Cu-Mo Concentrate Thickener*	17,700	19,500	29,000
Cu Filtration-Wheel Wash	Wheel Wash Surge Tank*	560	620	750

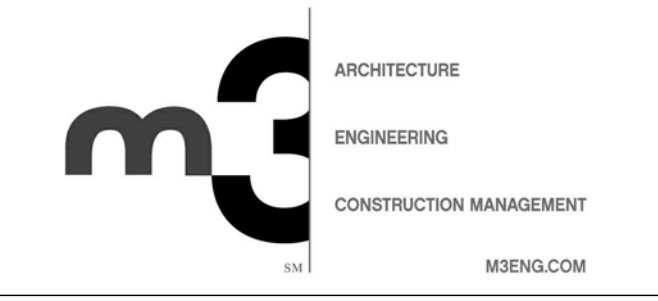
* - Denotes largest tank in containment area



PRELIMINARY
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DESIGNED BY AF	NOV 15
DRAWN BY AF	NOV 15
CHECKED BY AF	NOV 15
PROJECT MGR	
CLIENT APPR	

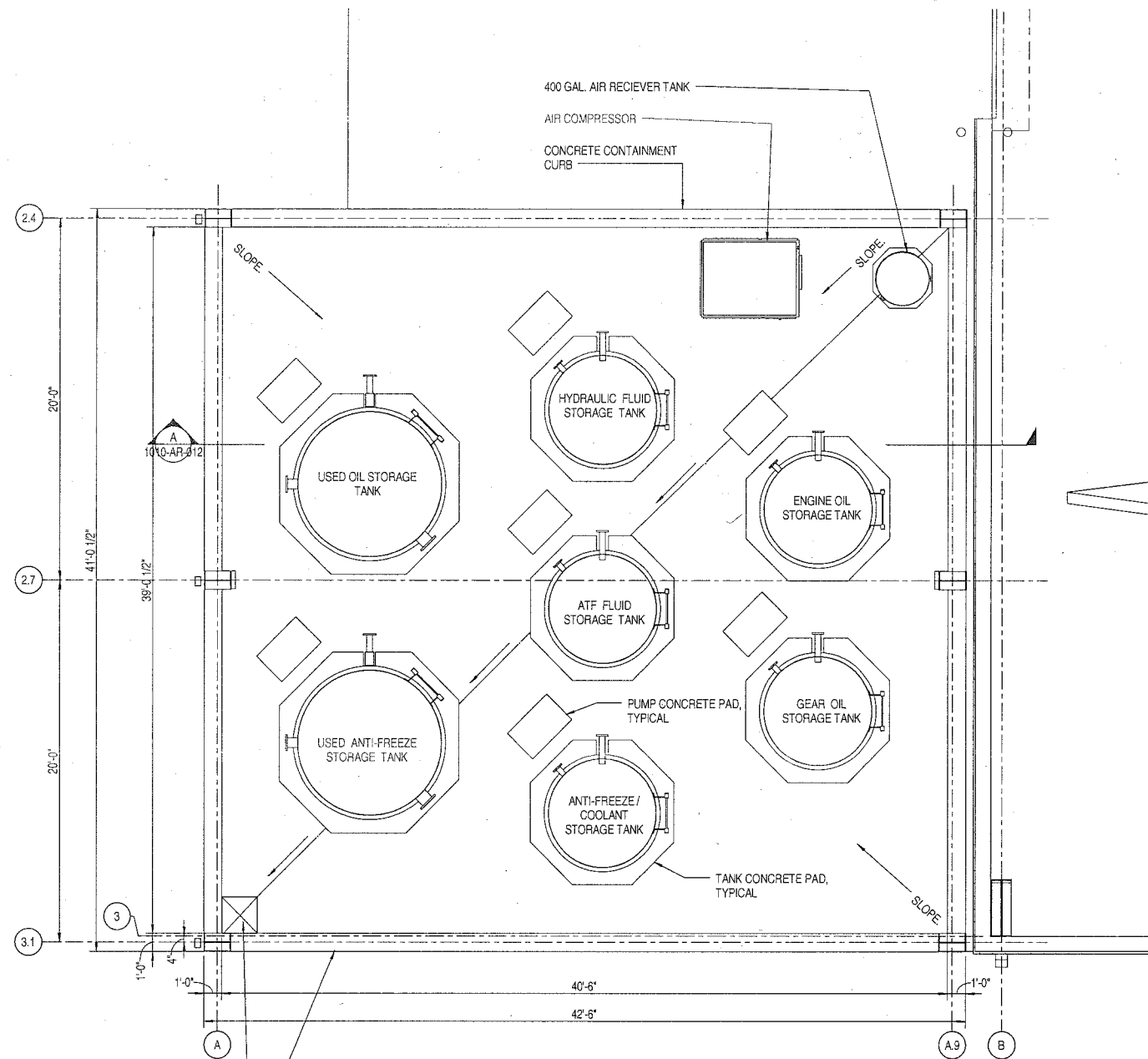


THE MAC RESOURCES

COPPER FLAT PROJECT

PROJECT PERMITTING GENERAL ARRANGEMENT CONCENTRATOR AREA CONTAINMENT PLAN

PROJECT NO. M3-PN120085
DWG NO. **0000-GA-050**
REV NO. P1 DATE 11/30/15

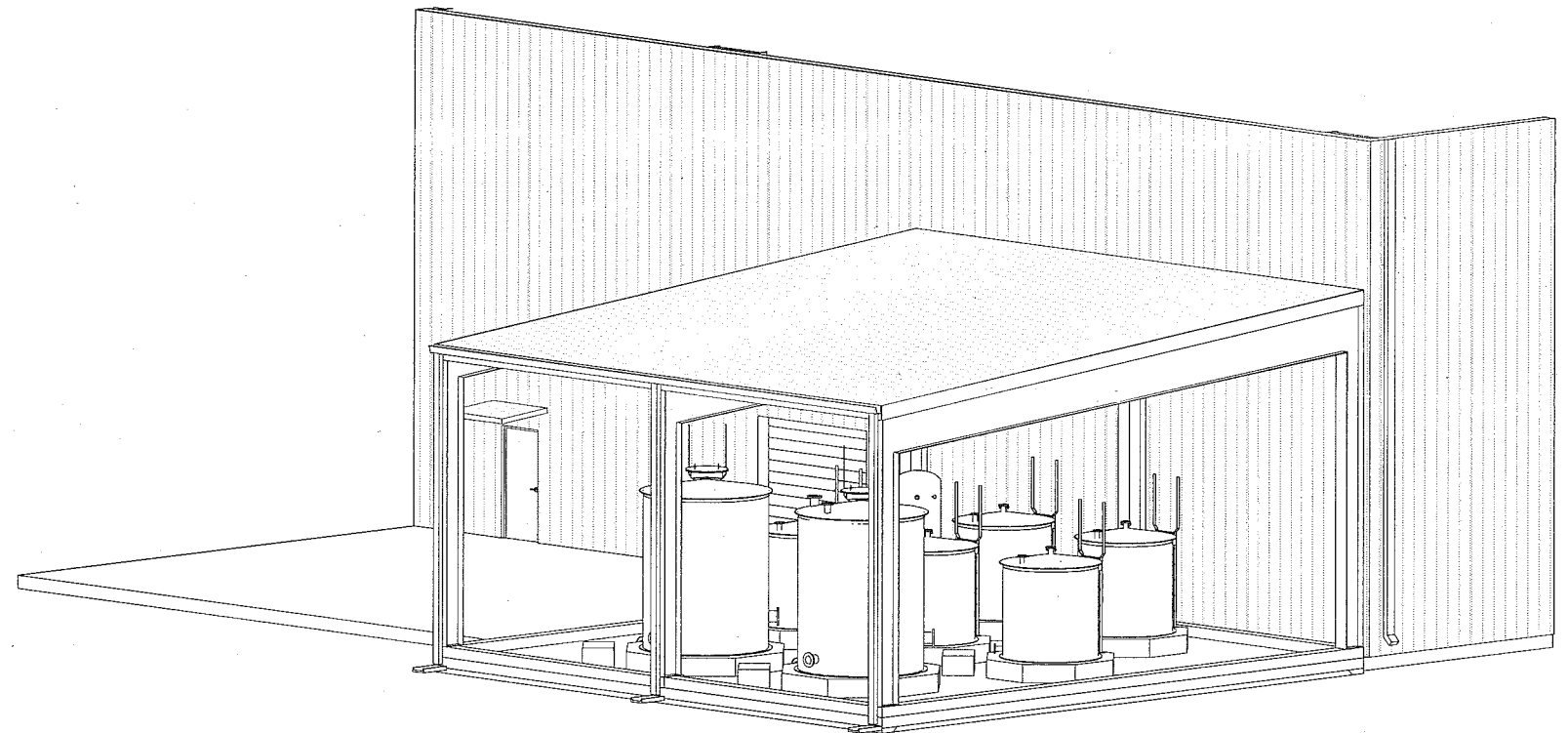


TANK FARM CONTAINMENT FLOOR PLAN

SCALE: 1/4" = 1'-0"

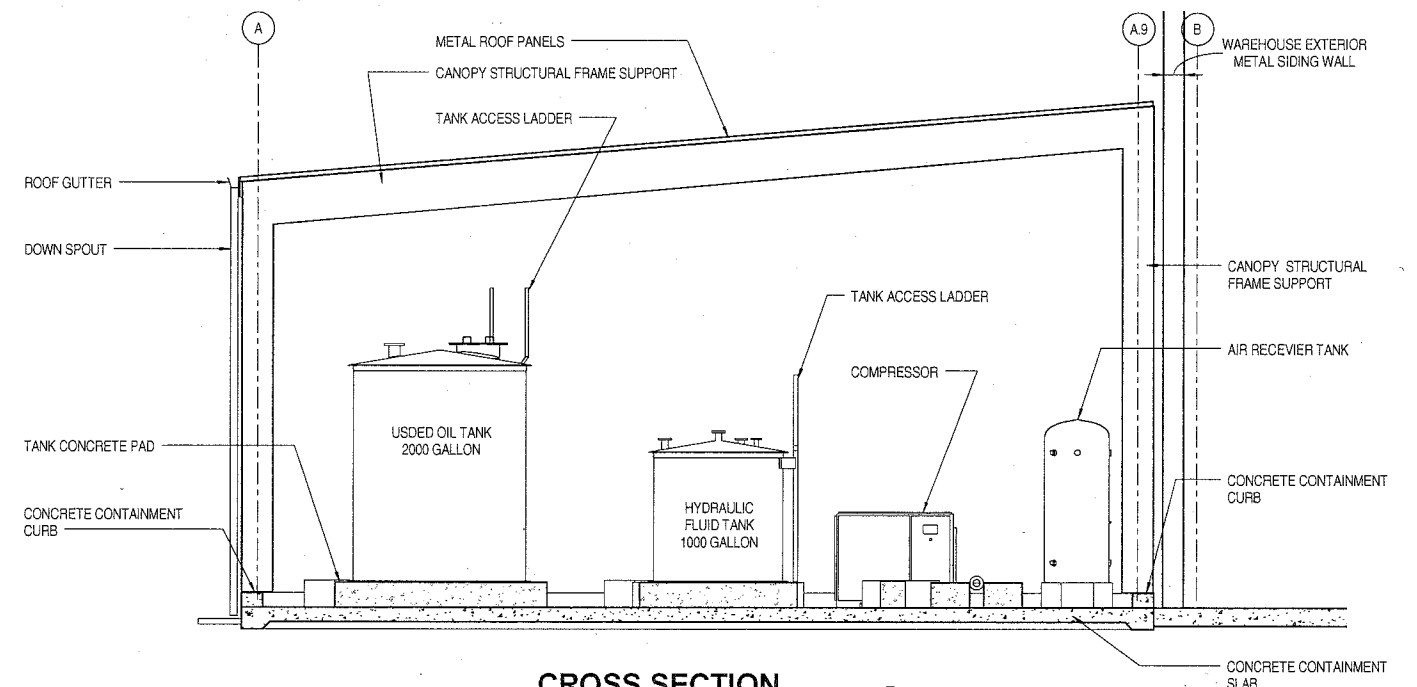
CONTAINMENT AREA	TANK NAME	TANK VOLUME (GAL.)	CONTAINMENT VOLUME	
			REQUIRED (GAL.)	PROVIDED (GAL.)
TRUCK SHOP TANK FARM.	USED OIL STORAGE TANK *	2,000	2,200	5,600
	USED ANTI-FREEZE STORAGE TANK	2,000		
	ATF FLUID STORAGE TANK	1,000		
	HYDRAULIC FLUID STORAGE TANK	1,000		
	ENGINE OIL STORAGE TANK	1,000		
	GEAR OIL STORAGE TANK	1,000		
	ANTI-FREEZE/COOLANT STORAGE TANK	1,000		

* DENOTES LARGEST TANK IN CONTAINMENT AREA



TRUCK SHOP - TANK FARM CONTAINMENT

SCALE:



CROSS SECTION

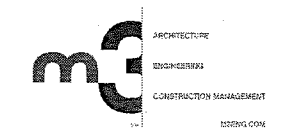
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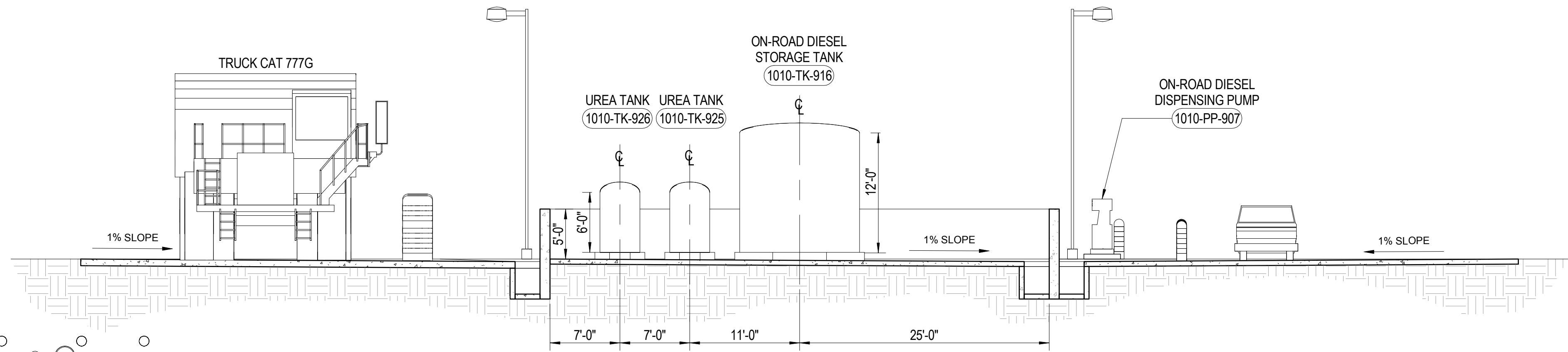
THEMAC
RESOURCES

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SCALE: AS NOTED	DATE
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DRAWN BY R.R.	NOV. 15
CHECKED BY RSS	NOV. 15
PROJECT MGR	
CLIENT APPR	



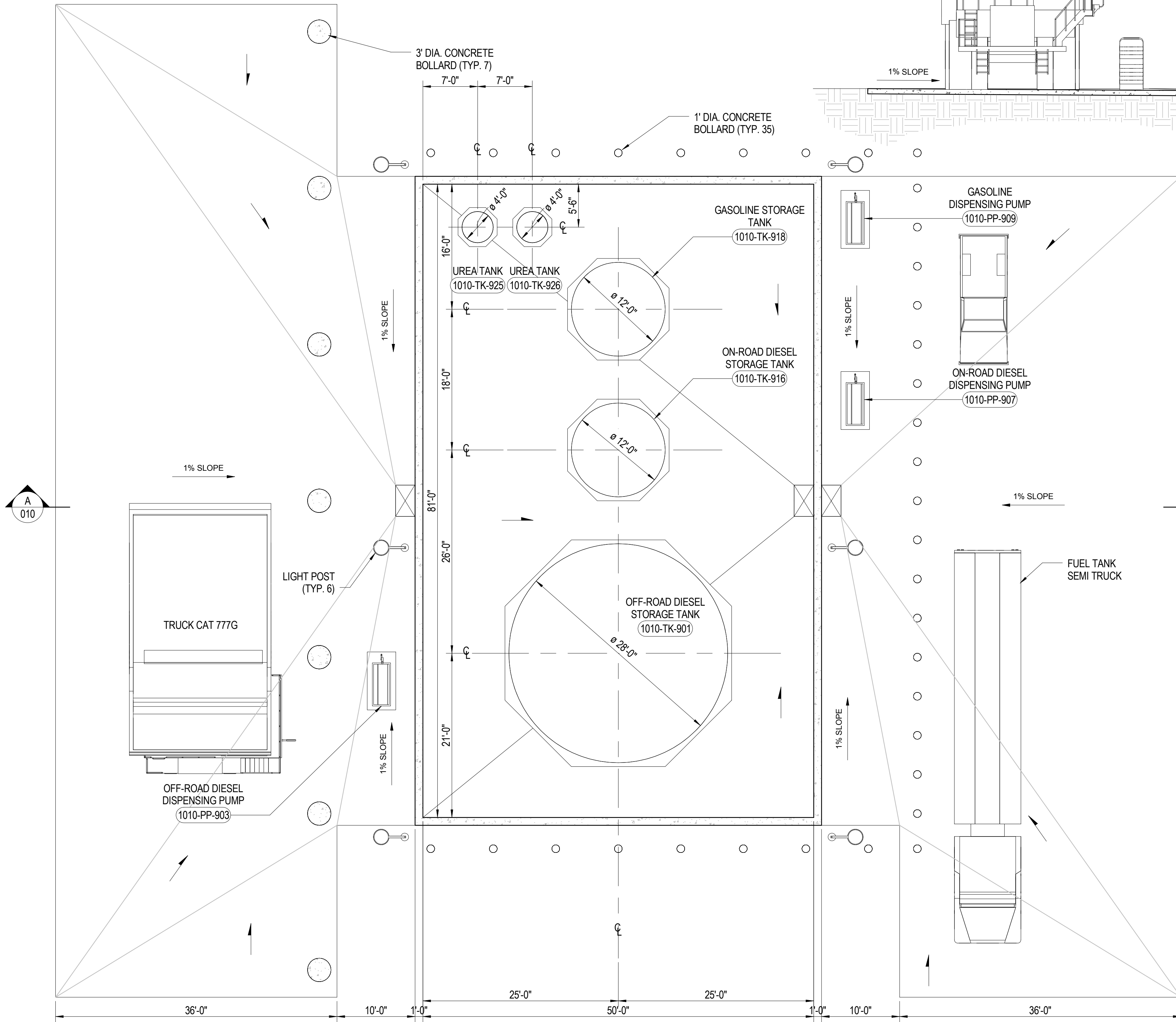
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ADMINISTRATION AREA ARCHITECTURAL TRUCK SHOP TANK FARM CONTAINMENT		DWG. NO. 1010-AR-012
REV. NO. P2	DATE 11/18/15	



SECTION

SCALE: 1/8" = 1'-0"

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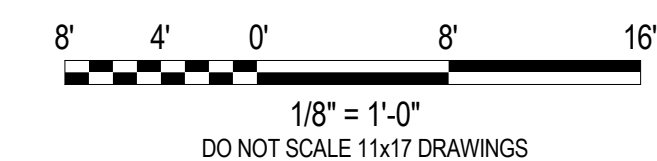
DRAWING KEY	
	CONTAINMENT SUMP
	DEMONSTRATES FLOW DIRECTION

CONTAINMENT AREA	TANK NAME	TANK VOLUME	CONTAINMENT VOLUME	
		(GAL.)	REQUIRED (GAL.)	PROVIDED (GAL.)
FUEL STATION TANK FARM	OFF-ROAD DIESEL STORAGE TANK *	100,000	110,000	119,000
	ON-ROAD DIESEL STORAGE TANK	10,000		
	GASOLINE STORAGE TANK	10,000		
	UREA TANK	560		
	UREA TANK	560		

* DENOTES LARGEST TANK IN CONTAINMENT AREA

FUEL STATION CONTAINMENT PLAN

SCALE: 1/8" = 1'-0"



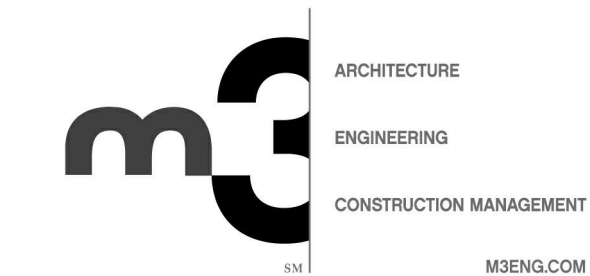
PRELIMINARY
NOT FOR CONSTRUCTION

THE MAC
RESOURCES

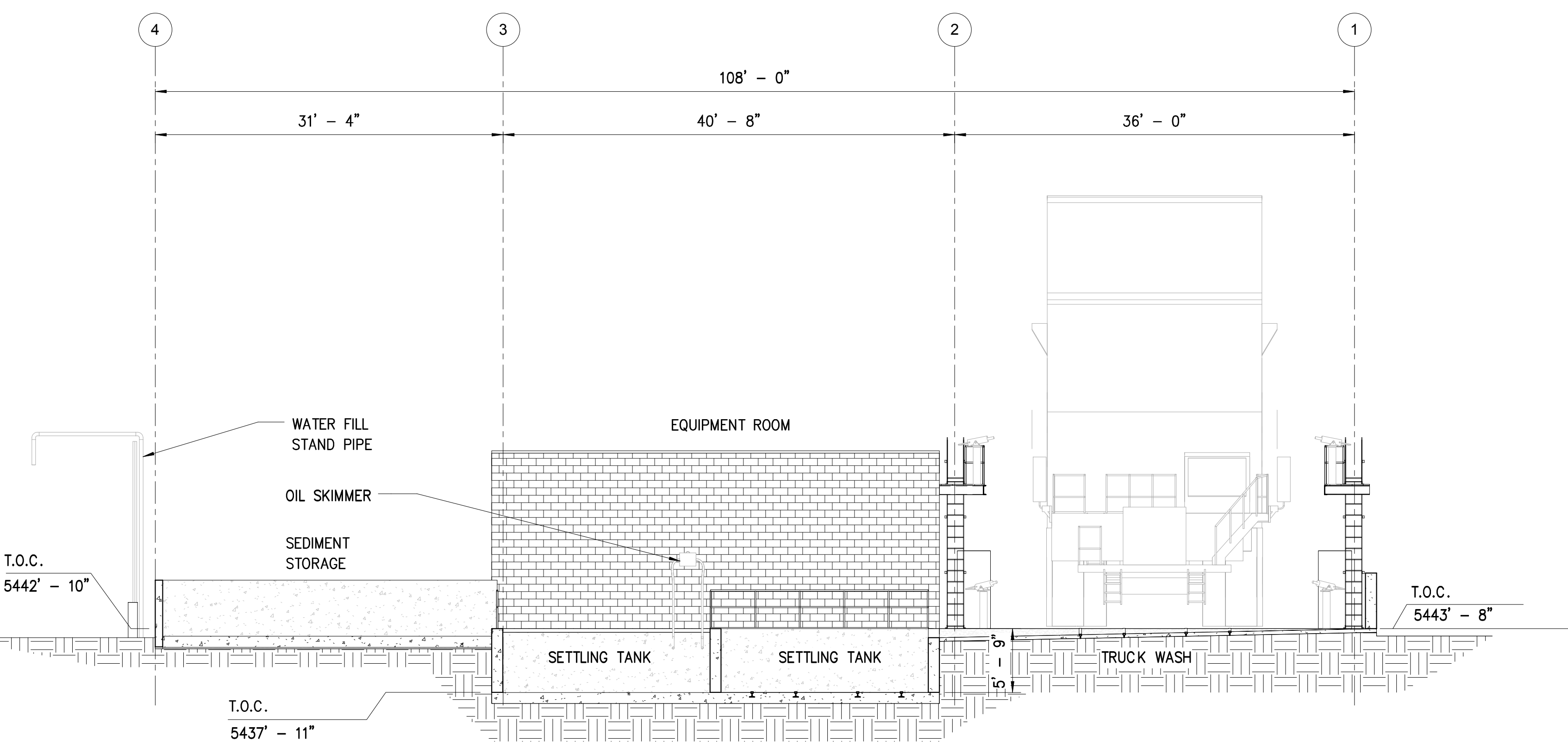
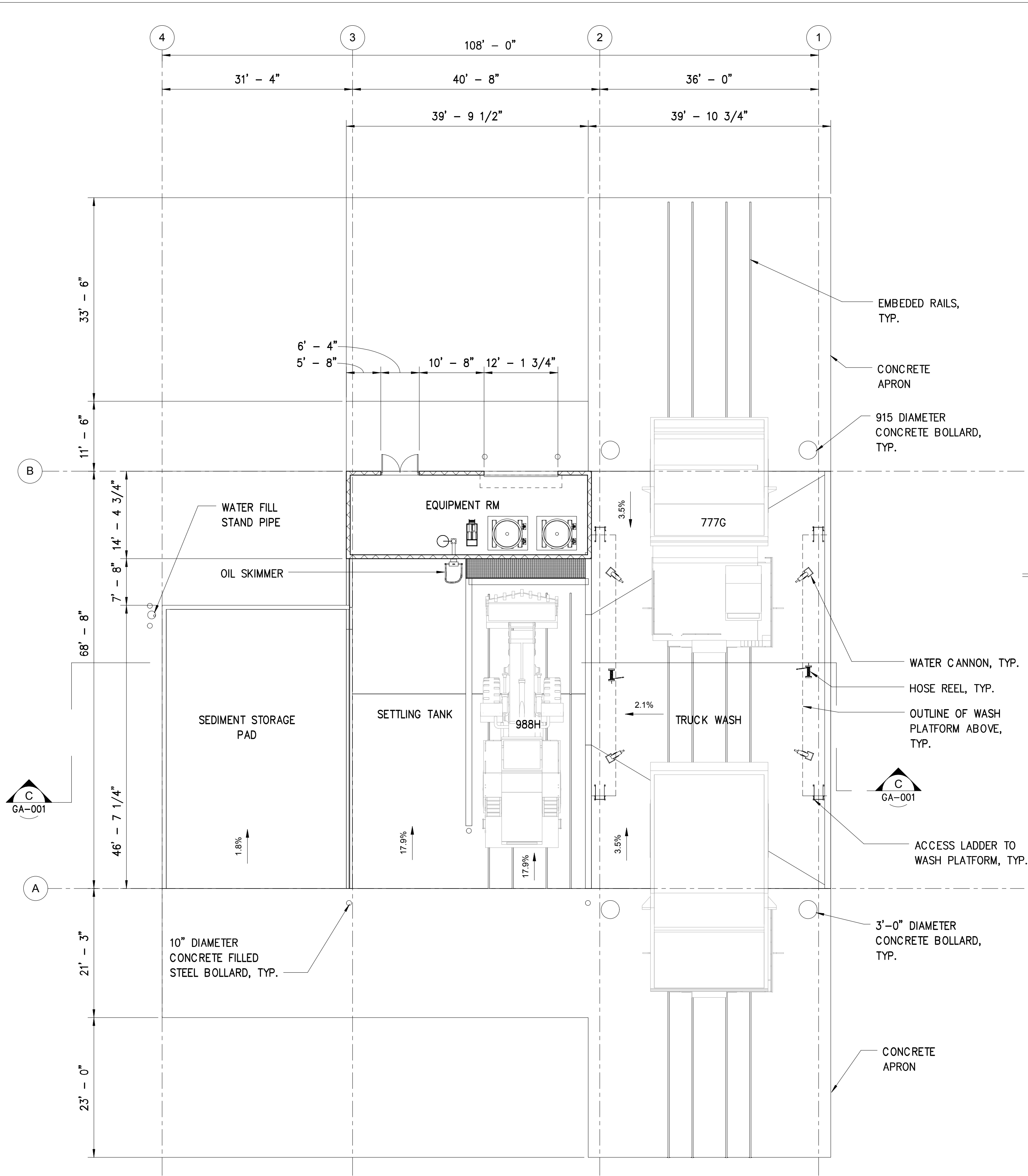
COPPER FLAT PROJECT
ADMINISTRATION AREA
GENERAL ARRANGEMENT
FUEL STATION
CONTAINMENT PLAN

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SCALE: AS NOTED	DATE
DESIGNED BY: RZ	NOV 15
DRAWN BY: JC	NOV 15
CHECKED BY:	
PROJECT MGR:	
CLIENT APPR:	



PROJECT NO. M3-PN120085
DWG NO. 1010-GA-010
REV NO. P1
DATE 19 NOV 15



FLOOR PLAN
SCALE: 3/32" = 1'-0"

SECTION C-C
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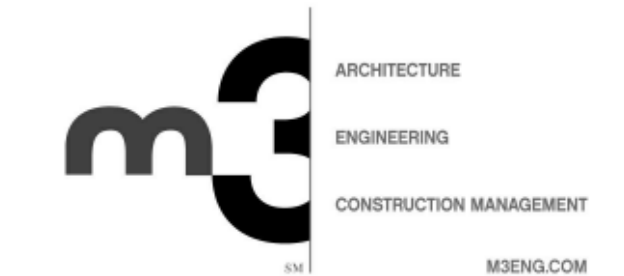
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REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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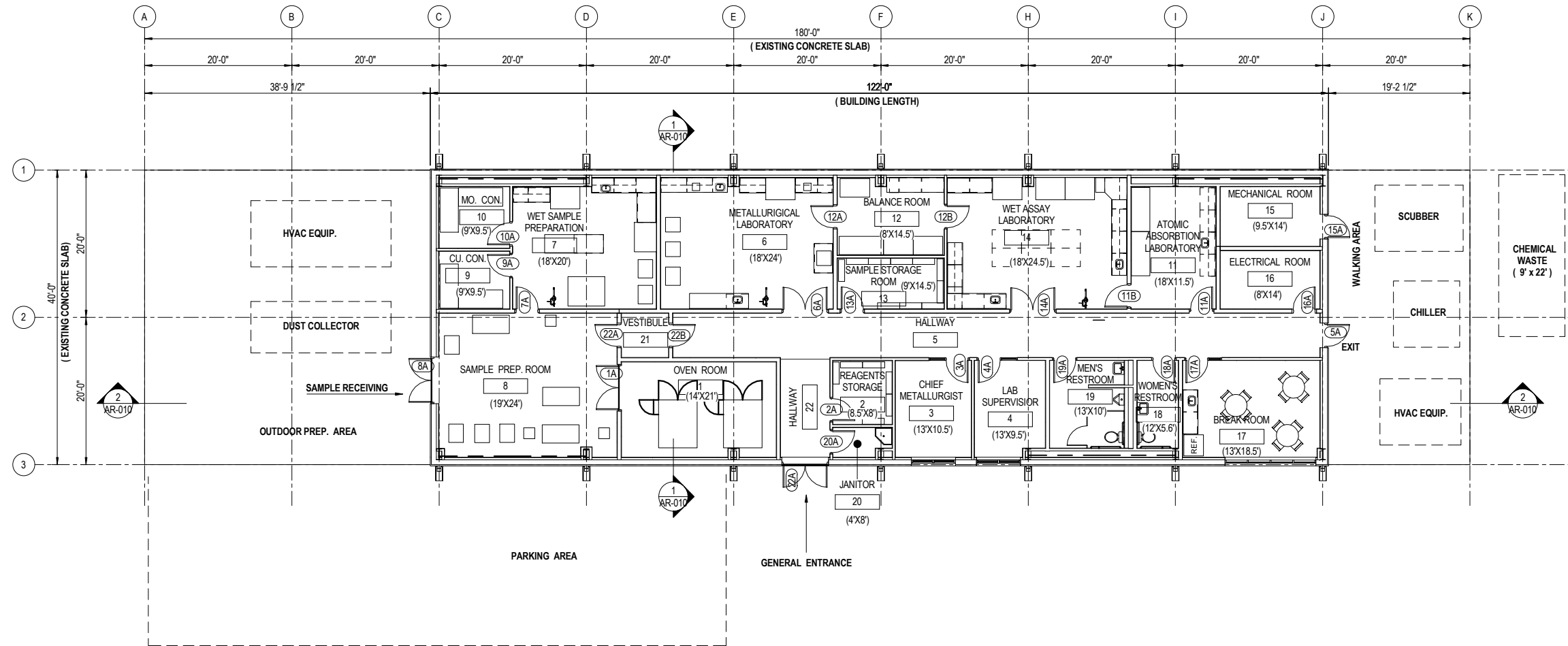
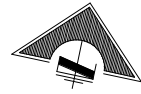
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PROJECT MGR	
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COPPER FLAT PROJECT

**ADMINISTRATION AREA
GENERAL ARRANGEMENT
TRUCK WASH
FLOOR PLAN & SECTION**

PROJECT NO. M3-PN120085
DWG NO. **1010-GA-001**
REV NO. P1 DATE 20 NOV 15

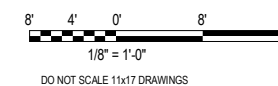


FLOOR PLAN

SCALE: 1/8" = 1'-0"

ANALYSIS:

1. VENDOR ENGINEERED PREFABRICATED METAL BUILDING.
2. NEW BUILDING SIZE: 122'-0" x 40'-0" = 4,880 SQ. FT.
3. EXISTING CONCRETE SLAB SQUARE FOOTAGE = 7,200 SQ. FT.
4. COLUMN LINE SPACING SAME AS PRIOR BUILDING, UTILIZING EXISTING FOUNDATIONS.



PRELIMINARY
NOT FOR CONSTRUCTION



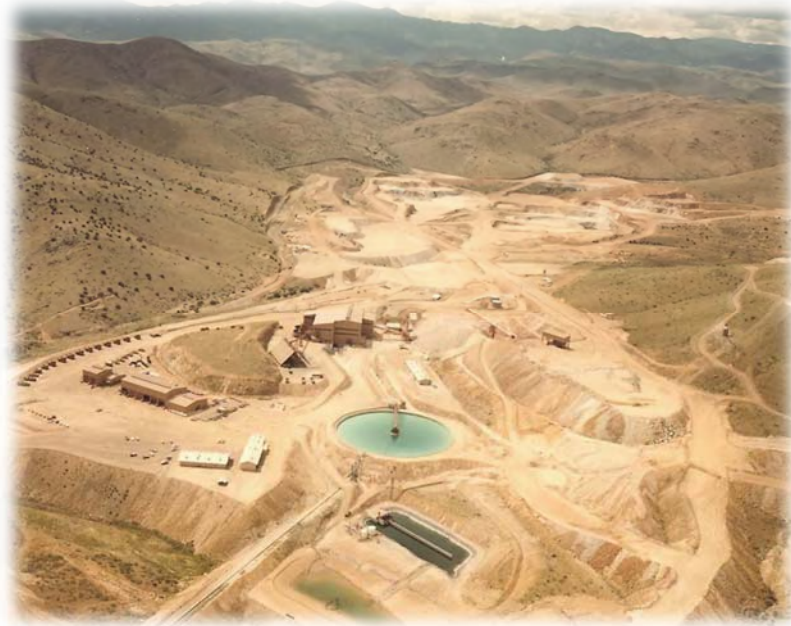
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																PROJECT MGR			
																CLIENT APPR			



COPPER FLAT PROJECT
CONCENTRATOR AREA
ARCHITECTURAL
LABORATORY
FLOOR PLAN

PROJECT NO.	M3-PN120085
DWG. NO.	3010-AR-003
REV. NO.	P2
DATE	5 DEC. 12

Copper Flat Project



Site Diversion Analysis Report

Prepared For:

THEMAC
RESOURCES 

Certified Professional Engineer Seal

This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.



Harry Lewsley
Signature


Exp. 12/31/2017
Date 1/4/2015

SITE DIVERSION ANALYSIS
COPPER FLAT PROJECT

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Harry J. Lewsley
12/7/2015




HARRY J. LEWSLEY
NEW MEXICO
18979
LICENSED PROFESSIONAL ENGINEER
Exp. 12/31/2017

LIST OF FIGURES

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12/7/2015

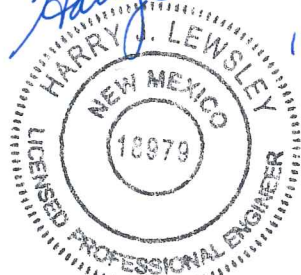


Exp. 12/31/2017

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B	HydroCAD Results for Pre-Quintana Watersheds
C	HydroCAD Results for Post-Quintana Watersheds
D	HydroCAD Results for Post-Quintana Culvert Analysis
E	HY-8 Culvert Analysis Results

Harry J. Lewisley
12/7/2015



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18979
LICENSED PROFESSIONAL ENGINEER
Exp. 12/31/2017

1 PURPOSE AND SCOPE OF ANALYSIS

The Copper Flat Project proposed for mine development by THEMAC Resources, Inc. through its subsidiary, New Mexico Copper Company (NMCC), lies within the Greenhorn Arroyo sub-basin in Sierra County, New Mexico (Dwg. 0000-CI-100).

Previous development of the mining property was conducted by Quintana Minerals Company (Quintana). The natural drainage pattern in the area (Dwg. 0000-CI-101) was modified by Quintana to divert surface drainages away from the proposed mine pit and protect the operations from flooding (Dwg. 0000-CI-102). In addition to the diversions, Quintana placed large diameter culverts beneath two crossings of Grayback Arroyo near the mine entrance. These structures are still in place and will be used by NMCC to limit stormwater run-on and prevent the site impacted areas from interacting with uncontained surface water flows for the planned mining operation (Dwg. 0000-CI-103).

The analysis completed was to evaluate existing diversions and water conveyance features at the Copper Flat Project with regard to their adequacy in conveying flows from storm events and protecting the site from flooding. In order to evaluate the adequacy of the existing diversions it was necessary to identify and evaluate surface water flow prior to the existence of the diversions (Dwg. 000-CI-104). Analytical methods were used to calculate the peak discharges, runoff volumes, and to determine if the existing site diversions and Grayback Arroyo culverts had the adequate capacity to convey the peak discharges and runoff around and away from the proposed site. Peak flows were analyzed for each sub-basin contributing to surface water flows upstream of the project area using the the HydroCAD program which uses both Soil Conservation Service (SCS) TR-20 methods (SCS, 1982) and TR-55 methods (USDA, 1986). An SCS 24-hour Type II storm was selected for simulation due to the project's location. Upstream watersheds were delineated as depicted on Dwg. 0000-CI-104.

The result of this evaluation demonstrates that existing diversions and culverts are adequate to prevent run-on or flooding of the mine site.

2 TYPE OF ANALYSIS

2.1 HYDROLOGY

Peak discharge and volume analysis was performed for drainage areas contributing to the Grayback Arroyo located within the Copper Flat project area. The return periods used were:

- 100-year, 24-hour storm, (Q100)
- 200-year, 24-hour storm, (Q200)
- 500-year, 24-hour storm, (Q500)

2.2 HYDRAULICS

Culvert and channel capacity analysis was conducted for the Grayback Arroyo to determine water surface elevations during the design storm for the two existing culvert crossings.

3 KEY ASSUMPTIONS

- Minimum of 10 percent impervious area assumed for all watersheds.
- Minimum time of concentration set to 5 minutes for developed watersheds and 10 minutes for natural watersheds.
- Soil Class D assumed because fine to medium sand and clay dominate beneath the top layer of soil, which will likely be excavated, in accordance with THEMAC Conceptual Model of Groundwater Flow.
- Surface description used for natural watersheds is “Desert Shrub Range” with a curve number (CN) of 86.
- Existing culverts were modeled assuming that they are in good condition and free of any debris.

4 PROCEDURES AND METHODS USED:

- Peak Discharges:
 - Calculated by the SCS methods using HydroCAD.
 - Calculated the existing discharges for the 100-, 200-, and 500-year, 24-hour storm events.
 - The peak discharges were found using an antecedent moisture condition (AMC) of II to represent normal conditions.
 - Time of concentration was calculated using the SCS Lag time method.
 - The length of longest watercourse, L_c , and mean watershed slope, Sc , were determined by using a combination of site surveys and USGS topographic maps.

- Culvert Capacity Calculations:
 - Calculated the existing culvert capacity for the 100-, 200-, and 500-year, 24-hour storm events.
 - Culvert capacity calculations completed using the Federal Highway Administration (FHWA) HY-8 program for Crossing #1. HY-8 was chosen for the first existing culvert crossing as the culverts will be subject to the peak discharge of the entire watershed reporting to them.
 - Culvert capacity calculations for Crossing #2 were computed utilizing HydroCAD due to the upstream ponding, which will mitigate the peak discharge and thus make the existing culvert at Crossing #2 act as a orifice outlet instead of a standard culvert.
 - Combined peak discharges at each crossing are less than the cumulative respective discharges due to the “routing” effect of Grayback Arroyo.

5 TECHNICAL DATA

- Precipitation depths are per the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 Precipitation Frequency Estimates (NOAA, 2006) near Hillsboro, NM.
- The watershed characteristic summary for the existing watersheds can be found in Appendix A.
- The files for the HydroCAD analysis for Pre-Quintana & Post-Quintana watersheds can be found in Appendices B and C, respectively.
- The HydroCAD analysis for the culvert crossings can be found in Appendix D.
- The HY-8 culvert analysis can be found in Appendix E.

6 RESULTS

Analysis of the pre-mine development topography and drainage pattern identified sixteen watersheds (Dwg. 0000-CI-105) contributing to Grayback Arroyo in the project area. These watersheds were analyzed using HydroCAD and the results are shown in Table 1 in cubic feet per second (cfs) for each watershed. The upstream drainages merged in the central portion of the current Copper Flat Project area and passed through to the eastern boundary via Grayback Arroyo.

Table 1: Pre-Quintana Watershed Characteristics

Watershed ID	Area (ac)	Lc (ft)	Avg. Sc (ft/ft)	Tc (min)	Q100 (cfs)	Q200 (cfs)	Q500 (cfs)
1	352.00	7,396	0.164	30.70	708.72	813.94	952.23
2	117.79	3,175	0.231	13.10	377.27	432.11	504.05
3	100.96	4,242	0.194	18.10	277.70	318.42	371.87
4	85.03	3,389	0.304	12.10	280.90	321.62	375.03
5	107.36	4,088	0.346	13.10	343.86	393.85	459.42
6	377.55	9,338	0.267	29.00	788.24	906.94	1060.97
7	144.47	5,064	0.277	17.40	405.48	464.86	542.79
8	92.01	3,617	0.273	13.40	291.83	334.29	389.99
9	235.91	7,005	0.267	23.00	567.91	651.70	761.76
10	333.62	10,278	0.262	31.50	660.28	758.38	887.32
11	397.83	7,149	0.182	28.30	843.39	968.42	1132.73
12	236.95	6,590	0.292	21.00	600.47	688.82	804.86
13	282.46	7,744	0.258	25.30	641.72	736.61	861.28
14	161.96	5,608	0.160	24.90	371.20	426.07	498.15
15	55.11	2,582	0.184	12.50	179.83	205.93	240.16
16	83.15	3,076	0.078	22.10	204.89	235.08	274.73

Lc = Length of longest waterpath
Sc = Slope

Tc = Time of concentration
Q = Flowrate

Diversion of surface drainages away from the prospective mining area was accomplished by Quintana using a number of diversions shown in Dwg. 0000-CI-105. Diversion 1 is an earthen diversion that re-routed a portion of Watershed 14 (Dwg. 0000 CI 105) to the east into an existing drainage that wraps around the north side of Animas Peak. Diversion 2 consists of earthen diversions routing Watersheds 12 and 13 southward to Diversion 3. Diversion 3 is a composite earthen diversion and bedrock cut that re-routes Watershed 10, 12, and 13 south into Watershed 6, where it joins the ancestral Grayback Arroyo channel south of the mine area (Dwg. 0000-CI-105). Diversion of surface drainages away from the mine area allows for surface water in the mine area to be managed in a manner that prevents impacted stormwater from migrating offsite or impacting groundwater.

Table 2 lists the watersheds that exist after the Quintana diversions, as altered by the NMCC development plan. The Quintana diversions and NMCC development plan alters the drainage pattern in the project area to facilitate control of impacted stormwater. As a result, Watersheds 15 and 16 (Dwg. 0000-CI-104) are completely within the project stormwater control area and are eliminated as tributaries to Grayback Arroyo and portions of Watersheds 1, 2, 3, and 14 are included in the project stormwater control area and isolated from interaction with Grayback Arroyo.

Table 2: Final Watershed Characteristics

Watershed ID	Area (ac)	Lc (ft)	Avg. Sc (ft/ft)	Tc (min)	Q100 (cfs)	Q200 (cfs)	Q500 (cfs)
1	28.21	N/A	0.249	5.00	117.20	133.97	155.94
2	106.56	3,068	0.314	11.00	363.66	415.35	484.31
3	34.99	1,603	0.177	8.70	129.54	148.20	172.66
4	75.02	3,047	0.339	10.50	259.92	297.56	346.92
5	124.19	5,976	0.340	18.00	342.49	392.70	458.61
6	331.22	8,173	0.310	24.20	774.13	888.50	1038.74
7	144.47	5,064	0.296	16.90	411.06	471.20	550.14
8	92.01	3,617	0.274	13.40	291.83	334.29	389.99
9	235.91	7,005	0.268	23.00	567.91	651.70	761.76
10	330.41	10,278	0.276	30.80	663.73	762.30	891.85
11	397.83	7,149	0.182	28.30	843.39	968.42	1132.73
12	227.42	6,590	0.315	20.20	588.30	674.80	789.72
13	275.51	7,744	0.293	23.80	649.87	745.85	871.93
14	79.97	3,545	0.224	14.60	243.90	279.49	326.19

Lc = Length of longest waterpath
Sc = Slope

Tc = Time of concentration
Q = Flowrate

In addition to the Diversions described above, Quintana installed culverts to convey water beneath two earthen crossings over the Grayback Arroyo channel southeast of the process plant area (Dwg. 0000-CI-103). The characteristics of these culverts were evaluated with respect to the calculated flows from the diverted and natural drainages upstream from the proposed mine area. Those characteristics and flows are presented in Tables 3 and 4. The modeled flows through the composite Grayback Arroyo drainage upstream of the culverts were evaluated to ensure that the culverts were adequate to transmit the necessary flows without risk of damage to the structures and flooding of the proposed mine site.

Table 3: Existing Culvert Characteristics and Flows

Crossing No.	Culvert CP Location	Description	End Treatment	Q100 (cfs)	Q200 (cfs)	Q500 (cfs)
1	2	3 – 177" CMPs	Projecting	3,552.39	4,191.95	5,046.09
2	1	1 – 177" CMP	Projecting	3,066.00	3,418.33	3,856.73

Table 4: Existing Culvert Elevations, Lengths, and Slopes

Crossing No.	Inlet Elev. (ft)	Outlet Elev. (ft)	Length (ft)	Slope (ft/ft)	Q100 Headwater Elev. (ft)	Q200 Headwater Elev. (ft)	Q500 Headwater Elev. (ft)	Top of Roadway Elev. (ft)
1	5372.0	5370.0	194.40	0.0103	5391.64	5397.05	5406.01	5419.0
2	5352.0	5344.0	255.4	0.0313	5386.54	5391.04	5396.71	5403.0

7 CONCLUSIONS

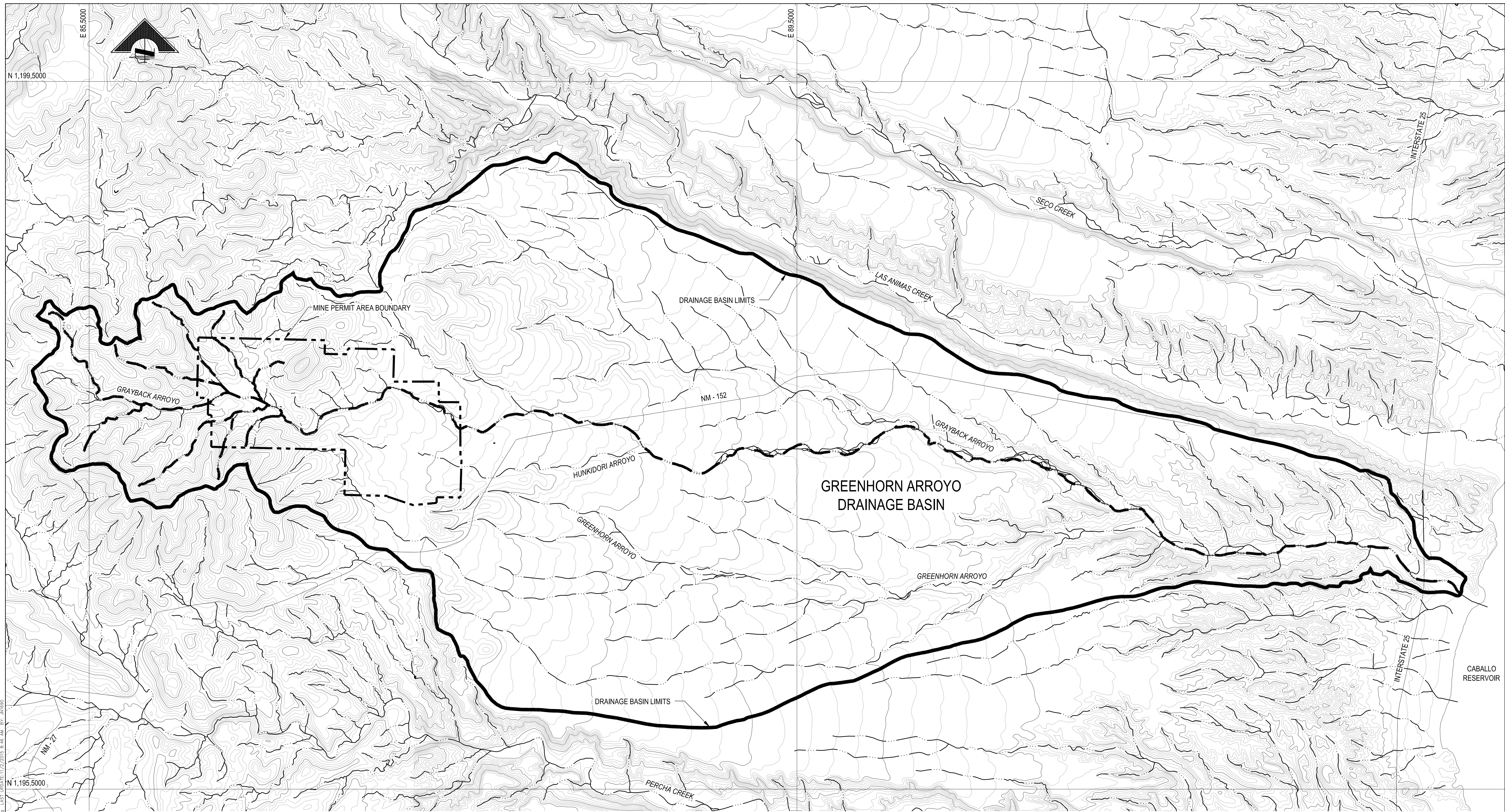
The 24-hour storm flows for 100-year, 200-year, and 500-year return periods were evaluated for the watersheds as they exist at present. Analysis of the results of the hydrologic analyses presented in this report demonstrates that the existing diversions and control structures are adequate to protect the site from flooding during the modeled flow events.

The analyses conducted also determined that both culvert crossings are capable of conveying the Q500 without overtopping the adjacent roadway or pipeline corridor. This conclusion is based on the assumption that the culverts analyzed are in good working condition. Field inspection of the culverts completed by M3 found the body of the existing culverts to be in good condition; repair will be required at the upstream inlets and vegetation removal undertaken at culvert inlets and outlets in order to meet the conditions of this analysis.

8 REFERENCES

- Bonin, GM, Martin, D., Lin, B., Parzybok, T., Yekta, M., and Riley, D., 2000. Precipitation Frequency Atlas of the United States, NOAA Atlas 14 Addendum, Volume 1, Version 4.0: Semiarid Southwest (Arizona, Southeast California, Nevada, New Mexico, Utah) Addendum – Update to Version 3.0. U.S. Department of Commerce, National Oceanic and Atmospheric Administration, National Weather Service, Silver Spring, Maryland, 2004 revised 2006.
- USDA, 1986. Urban Hydrology for Small Watersheds. U.S. Department of Agriculture, National Resources Conservation Service, Conservation Engineering Division. Technical Release (TR) 55. June, SCS, 1986.
- SCS, 1982. [Draft] Computer Program Co. Project Formulation – Hydrology. Soil Conservation Service Technical Release 20. Washington, DC.

DRAWINGS



LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN

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PRELIMINARY
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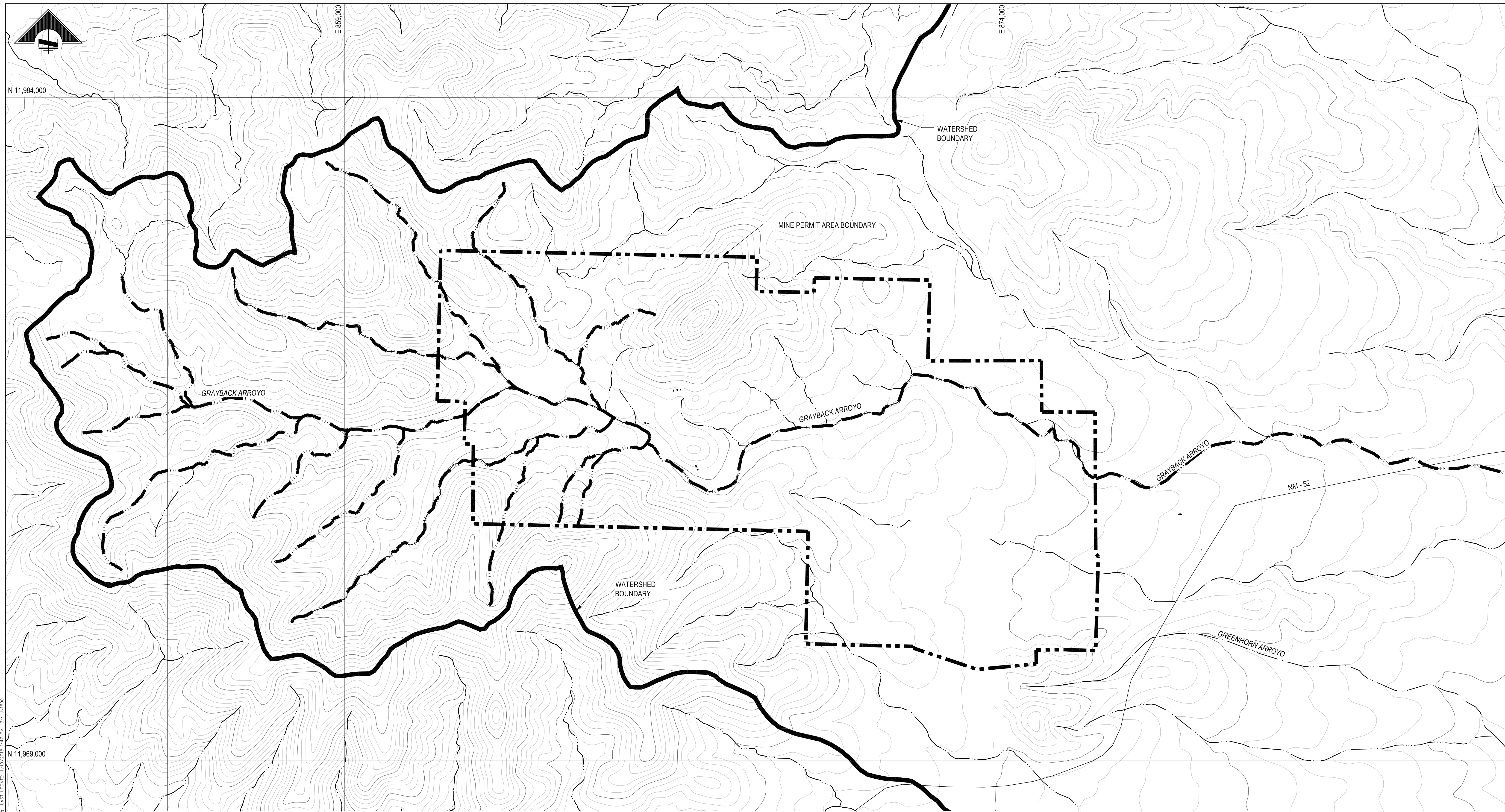
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COPPER FLAT PROJECT

**GENERAL SITE
CIVIL
GREENHORN ARROYO
PRE-MINING WATERSHED BASIN**

JOB NO. M3 PN-12085
DWG. NO. **0000-CI-100**
REV. NO. P2 DATE 20 NOV 15



LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN
SCALE: 1" = 1000'



SCALE IN FEET
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COPPER FLAT PROJECT

**GENERAL SITE
COPPER FLAT SITE
EXISTING HYDROLOGY
PRE-QUINTANA MINING**

JOB NO. M3 PN-120085
DWG. NO. **0000-CI-101**
REV. NO. P1 DATE 20 NOV 15

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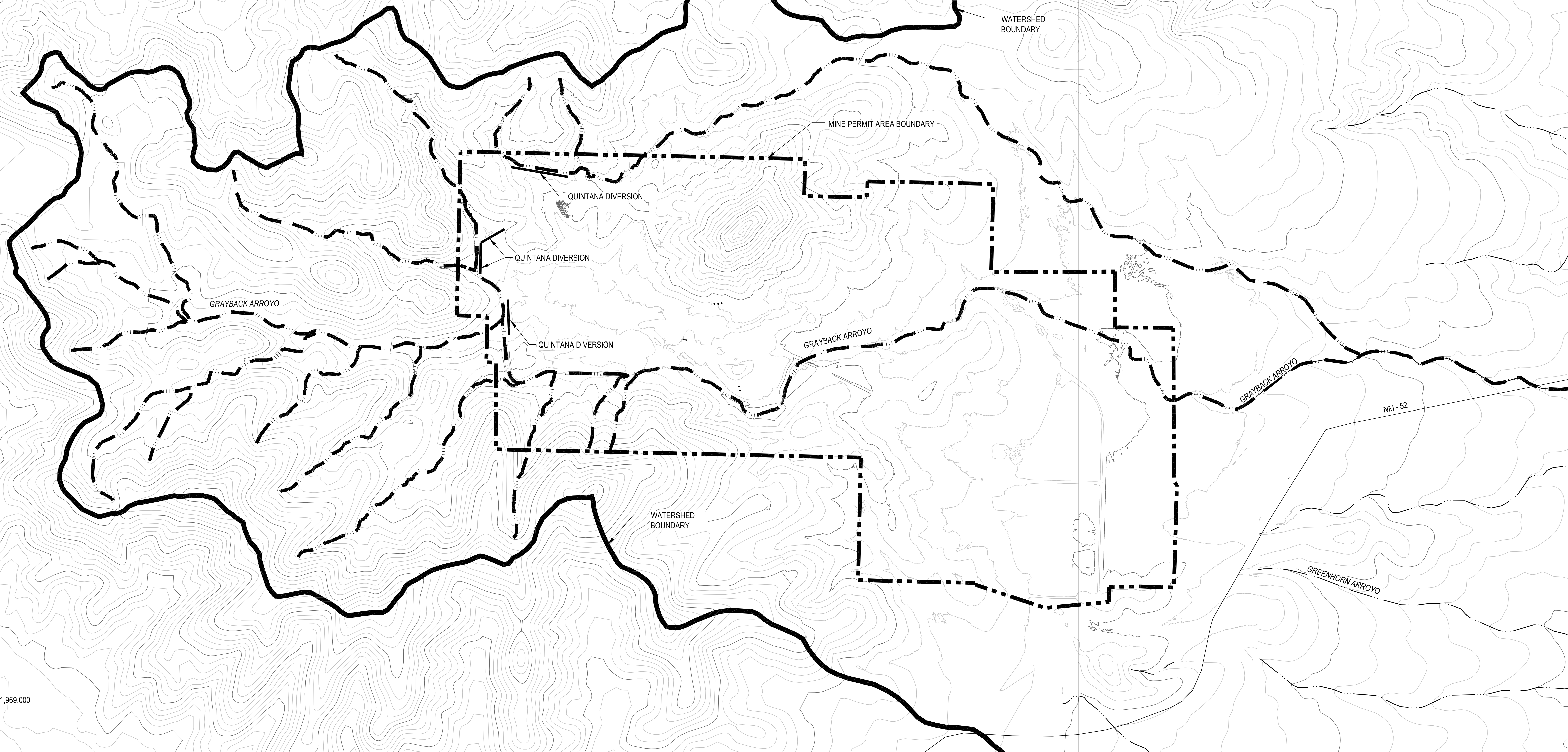
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LEGEND

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

SITE PLAN

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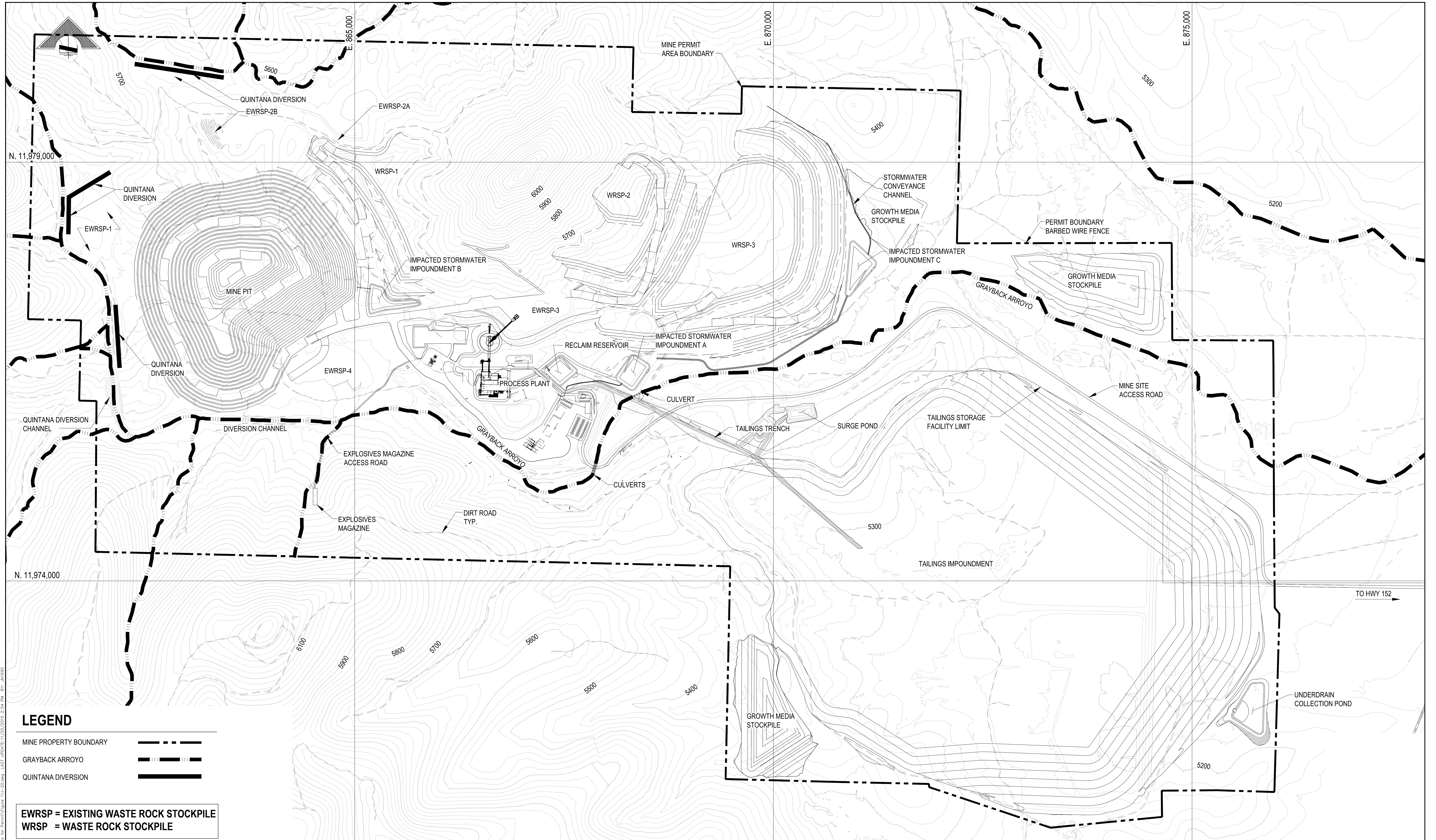
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**GENERAL SITE
COPPER FLAT SITE
EXISTING HYDROLOGY
POST QUINTANA MINING**

JOB NO. M3 PN-120085
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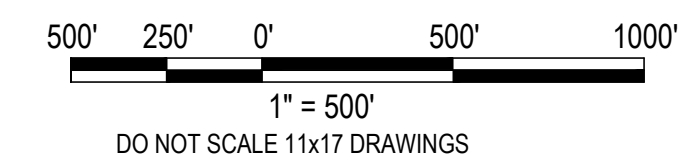


LEGEND

- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- QUINTANA DIVERSION

EWRSP = EXISTING WASTE ROCK STOCKPILE
WRSP = WASTE ROCK STOCKPILE

SITE PLAN
SCALE: 1:500



PRELIMINARY
FOR AGENCY REVIEW

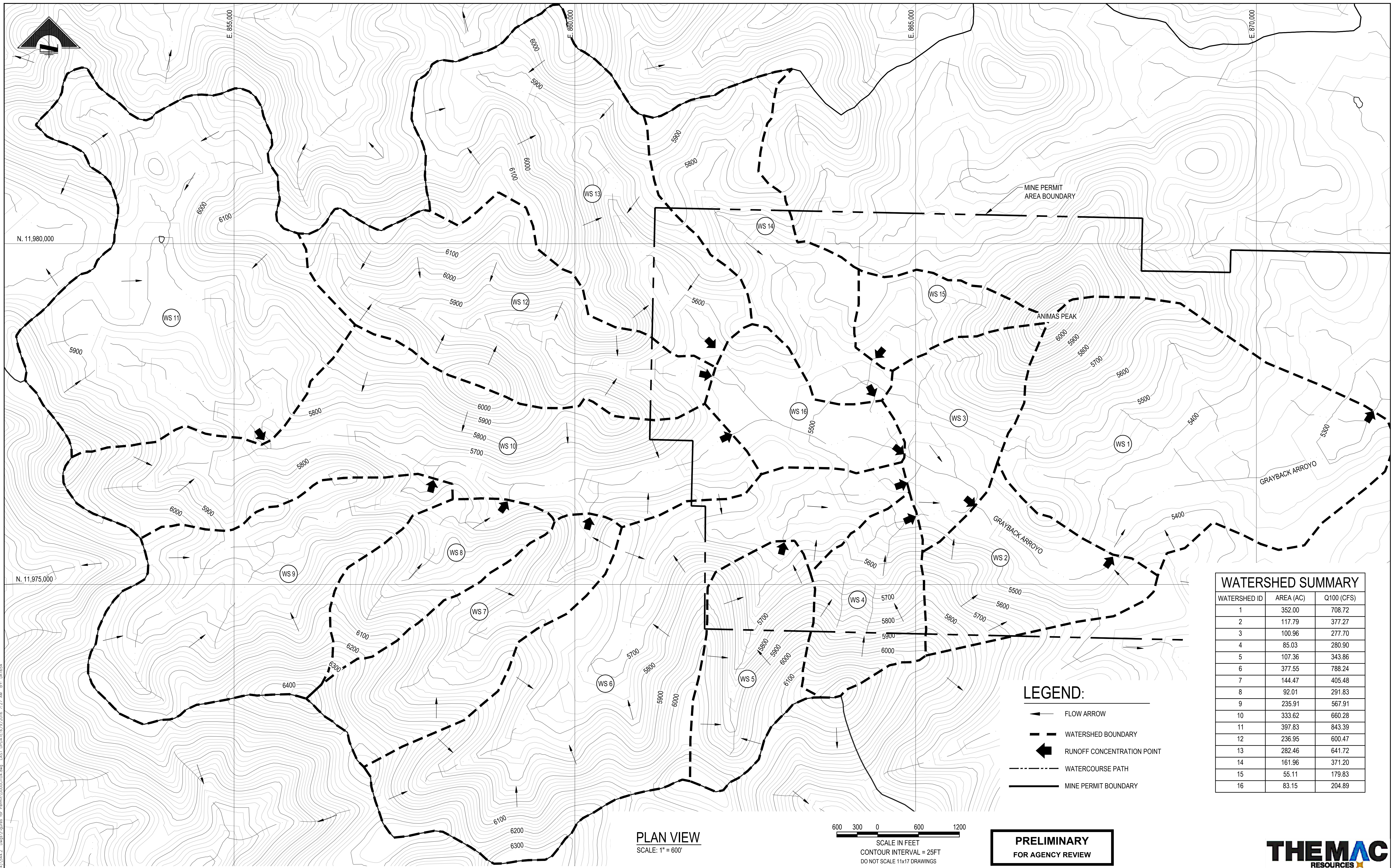


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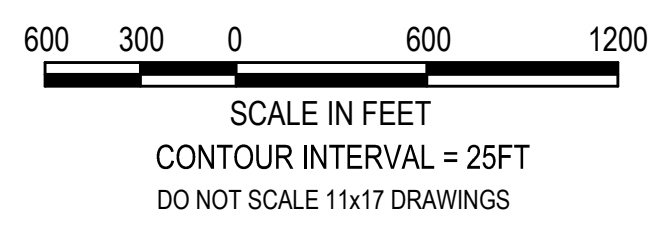
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WATERSHED SUMMARY		
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3	100.96	277.70
4	85.03	280.90
5	107.36	343.86
6	377.55	788.24
7	144.47	405.48
8	92.01	291.83
9	235.91	567.91
10	333.62	660.28
11	397.83	843.39
12	236.95	600.47
13	282.46	641.72
14	161.96	371.20
15	55.11	179.83
16	83.15	204.89

- LEGEND:**
- FLOW ARROW
 - - - WATERSHED BOUNDARY
 - ▶ RUNOFF CONCENTRATION POINT
 - - - WATERCOURSE PATH
 - MINE PERMIT BOUNDARY



PLAN VIEW
SCALE: 1" = 600'

PRELIMINARY
FOR AGENCY REVIEW



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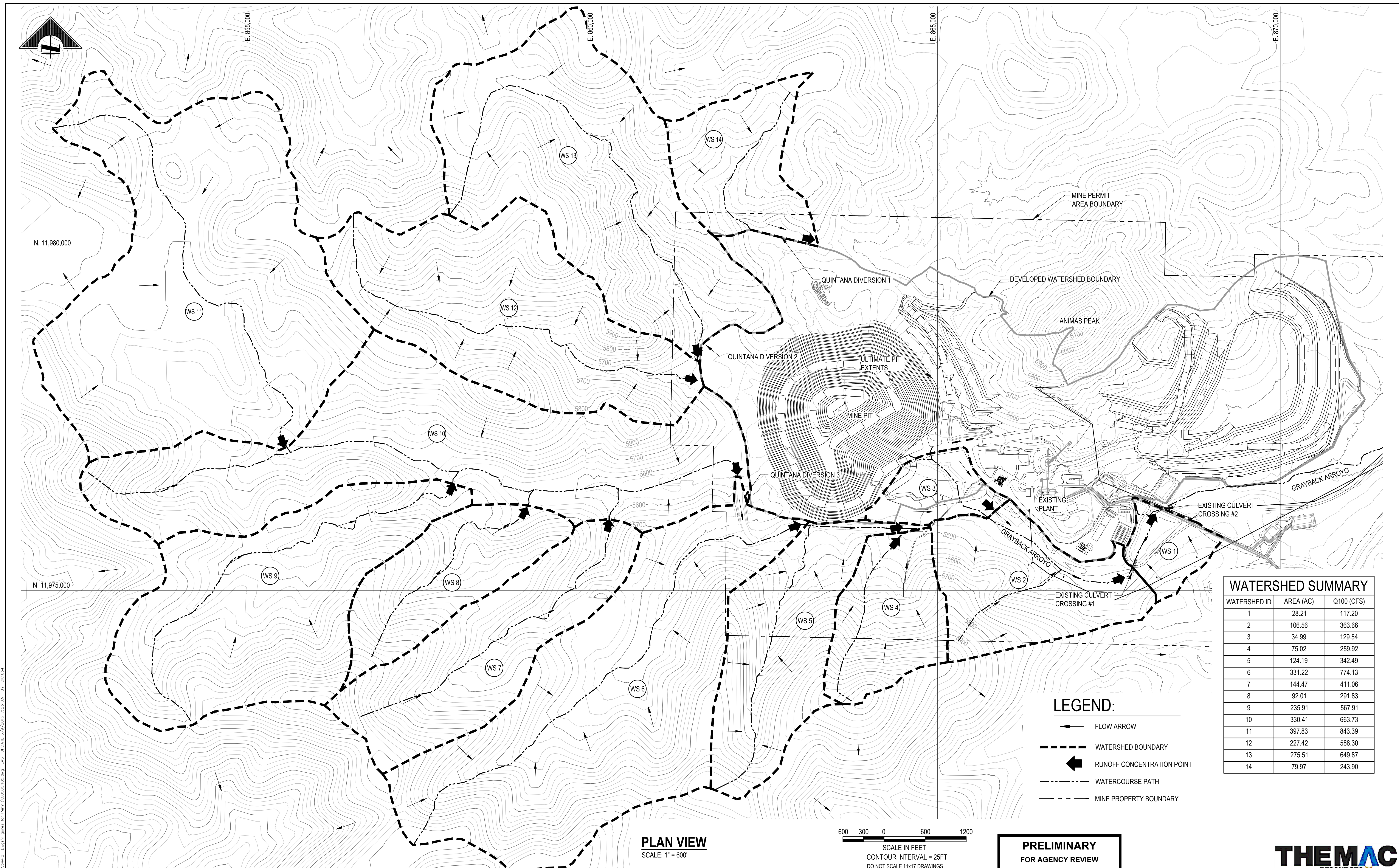
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COPPER FLAT PROJECT

GENERAL SITE CIVIL
PRE QUINTANA
EXISTING WATERSHED AREAS

JOB NO. M3 PN-12085
 DWG NO. **0000-CI-104**
 REV NO. P2 DATE 09 JUN 16



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N. 11,975,000

E. 865,000

E. 860,000

E. 865,000

E. 870,000

PLAN VIEW
SCALE: 1" = 600'



SCALE IN FEET
CONTOUR INTERVAL = 25FT
DO NOT SCALE 11x17 DRAWINGS

- LEGEND:**
- ← FLOW ARROW
 - WATERSHED BOUNDARY
 - ◀ RUNOFF CONCENTRATION POINT
 - - - WATERCOURSE PATH
 - MINE PROPERTY BOUNDARY

WATERSHED SUMMARY

WATERSHED ID	AREA (AC)	Q100 (CFS)
1	28.21	117.20
2	106.56	363.66
3	34.99	129.54
4	75.02	259.92
5	124.19	342.49
6	331.22	774.13
7	144.47	411.06
8	92.01	291.83
9	235.91	567.91
10	330.41	663.73
11	397.83	843.39
12	227.42	588.30
13	275.51	649.87
14	79.97	243.90

PRELIMINARY
FOR AGENCY REVIEW



COPPER FLAT PROJECT
GENERAL SITE CIVIL
POST QUINTANA
EXISTING WATERSHED AREAS

JOB NO. M3 PN-12085
DWG NO. **0000-CI-105**
REV NO. P2 DATE 09 JUN 16

REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: 1" = 600'		DATE	
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT

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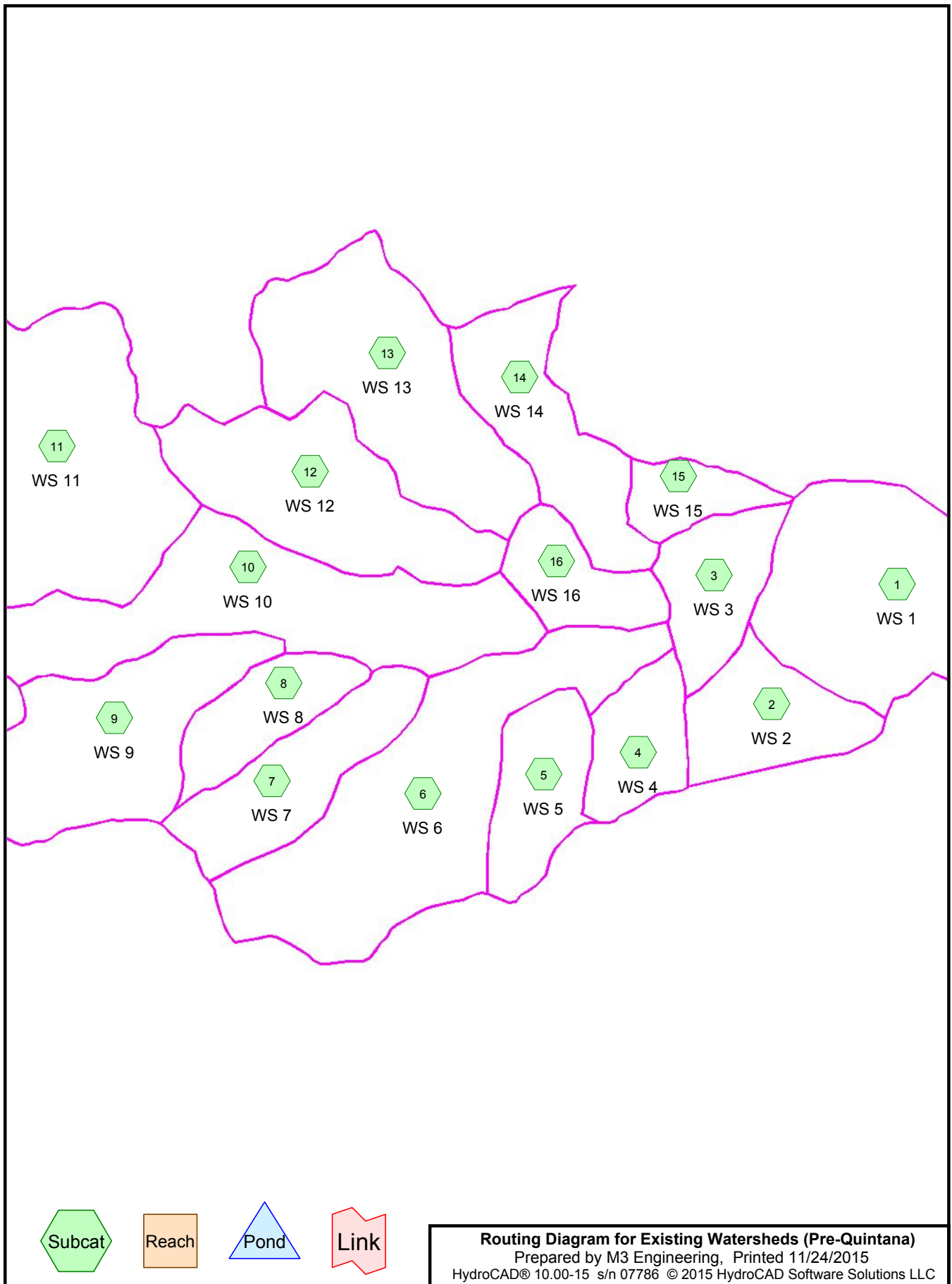
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APPENDIX A

Pre-Quintana Existing												
Watershed ID	Area	Total Area	Desert Shrub CN=86	Impervious Area CN=98	Pit Material CN=80	L _c	Avg. S _c	Q ₁₀₀ - SCS	Tc Method	Tc	Weighted SCS CN	Soil Type
--	<i>sq. feet</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>ft</i>	<i>ft/ft</i>	<i>cfs</i>	--	<i>min</i>	--	--
1	15,333,000	352.00	316.80	35.20	0.00	7,396	0.164	708.72	SCS Lag	30.70	87	D
2	5,131,142	117.79	106.01	11.78	0.00	3,175	0.231	377.27	SCS Lag	13.10	87	D
3	4,397,931	100.96	90.87	10.10	0.00	4,242	0.194	277.70	SCS Lag	18.10	87	D
4	3,703,864	85.03	76.53	8.50	0.00	3,389	0.304	280.9	SCS Lag	12.10	87	D
5	4,676,541	107.36	96.62	10.74	0.00	4,088	0.346	343.86	SCS Lag	13.10	87	D
6	16,446,115	377.55	339.79	37.75	0.00	9,338	0.267	788.24	SCS Lag	29.00	87	D
7	6,292,945	144.47	130.02	14.45	0.00	5,064	0.277	405.48	SCS Lag	17.40	87	D
8	4,008,126	92.01	82.81	9.20	0.00	3,617	0.273	291.83	SCS Lag	13.40	87	D
9	10,276,128	235.91	212.32	23.59	0.00	7,005	0.267	567.91	SCS Lag	23.00	87	D
10	14,532,660	333.62	300.26	33.36	0.00	10,278	0.262	660.28	SCS Lag	31.50	87	D
11	17,329,719	397.83	358.05	39.78	0.00	7,149	0.182	843.39	SCS Lag	28.30	87	D
12	10,321,490	236.95	213.25	23.69	0.00	6,590	0.292	600.47	SCS Lag	21.00	87	D
13	12,303,894	282.46	254.21	28.25	0.00	7,744	0.258	641.72	SCS Lag	25.30	87	D
14	7,054,905	161.96	145.76	16.20	0.00	5,608	0.160	371.2	SCS Lag	24.90	87	D
15	2,400,616	55.11	49.60	5.51	0.00	2,582	0.184	179.83	SCS Lag	12.50	87	D
16	3,621,863	83.15	74.83	8.31	0.00	3,076	0.078	204.89	SCS Lag	22.10	87	D

Post-Quintana Existing												
Watershed ID	Area	Total Area	Desert Shrub CN=86	Impervious Area CN=98	Pit Material CN=80	L _c	Avg. S _c	Q ₁₀₀ - SCS	Tc Method	Tc	Weighted SCS CN	Soil Type
--	<i>sq. feet</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>ft</i>	<i>ft/ft</i>	<i>cfs</i>	--	<i>min</i>	--	--
1 (pond)	1,228,747	28.21	25.39	2.82	0.00	0	0.249	117.20	Minimum	5.00	87	D
2	4,641,978	106.56	95.91	10.66	0.00	3,068	0.314	363.66	SCS Lag	11.00	87	D
3	1,523,968	34.99	31.49	3.50	0.00	1,603	0.177	129.54	SCS Lag	8.70	87	D
4	3,267,841	75.02	67.52	7.50	0.00	3,047	0.339	259.92	SCS Lag	10.50	87	D
5	5,409,568	124.19	111.77	12.42	0.00	5,976	0.340	342.49	SCS Lag	18.00	87	D
6	14,427,914	331.22	298.10	33.12	0.00	8,173	0.310	774.13	SCS Lag	24.20	87	D
7	6,292,945	144.47	130.02	14.45	0.00	5,064	0.296	411.06	SCS Lag	16.90	87	D
8	4,008,126	92.01	82.81	9.20	0.00	3,617	0.274	291.83	SCS Lag	13.40	87	D
9	10,276,128	235.91	212.32	23.59	0.00	7,005	0.268	567.91	SCS Lag	23.00	87	D
10	14,392,904	330.41	297.37	33.04	0.00	10,278	0.276	663.73	SCS Lag	30.80	87	D
11	17,329,719	397.83	358.05	39.78	0.00	7,149	0.182	843.39	SCS Lag	28.30	87	D
12	9,906,558	227.42	204.68	22.74	0.00	6,590	0.315	588.30	SCS Lag	20.20	87	D
13	12,001,134	275.51	247.96	27.55	0.00	7,744	0.293	649.87	SCS Lag	23.80	87	D
14	3,483,496	79.97	71.97	8.00	0.00	3,545	0.224	243.90	SCS Lag	14.60	87	D

APPENDIX B



Existing Watersheds (Pre-Quintana)

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2,847.730	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16)
316.410	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16)
3,164.140	87	TOTAL AREA

Existing Watersheds (Pre-Quintana)

Prepared by M3 Engineering

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Page 3

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
3,164.140	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
0.000	Other	
3,164.140		TOTAL AREA

Existing Watersheds (Pre-Quintana)

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,847.730	0.000	2,847.730	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
0.000	0.000	0.000	316.410	0.000	316.410	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
0.000	0.000	0.000	3,164.140	0.000	3,164.140	TOTAL AREA	

Existing Watersheds (Pre-Quintana)*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=708.72 cfs	69.315 af	
Subcatchment2: WS 2	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=377.27 cfs	23.195 af	
Subcatchment3: WS 3	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=277.70 cfs	19.883 af	
Subcatchment4: WS 4	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=280.90 cfs	16.744 af	
Subcatchment5: WS 5	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=343.86 cfs	21.141 af	
Subcatchment6: WS 6	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=788.24 cfs	74.344 af	
Subcatchment7: WS 7	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=405.48 cfs	28.449 af	
Subcatchment8: WS 8	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=291.83 cfs	18.118 af	
Subcatchment9: WS 9	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=567.91 cfs	46.455 af	
Subcatchment10: WS 10	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=660.28 cfs	65.695 af	
Subcatchment11: WS 11	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=843.39 cfs	78.339 af	
Subcatchment12: WS 12	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=600.47 cfs	46.657 af	
Subcatchment13: WS 13	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=641.72 cfs	55.621 af	
Subcatchment14: WS 14	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=371.20 cfs	31.893 af	
Subcatchment15: WS 15	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=179.83 cfs	10.852 af	
Subcatchment16: WS 16	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=204.89 cfs	16.372 af	

Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Total Runoff Area = 3,164.140 ac Runoff Volume = 623.071 af Average Runoff Depth = 2.36"
90.00% Pervious = 2,847.730 ac 10.00% Impervious = 316.410 ac

Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 1: WS 1

Runoff = 708.72 cfs @ 12.25 hrs, Volume= 69.315 af, Depth= 2.36"

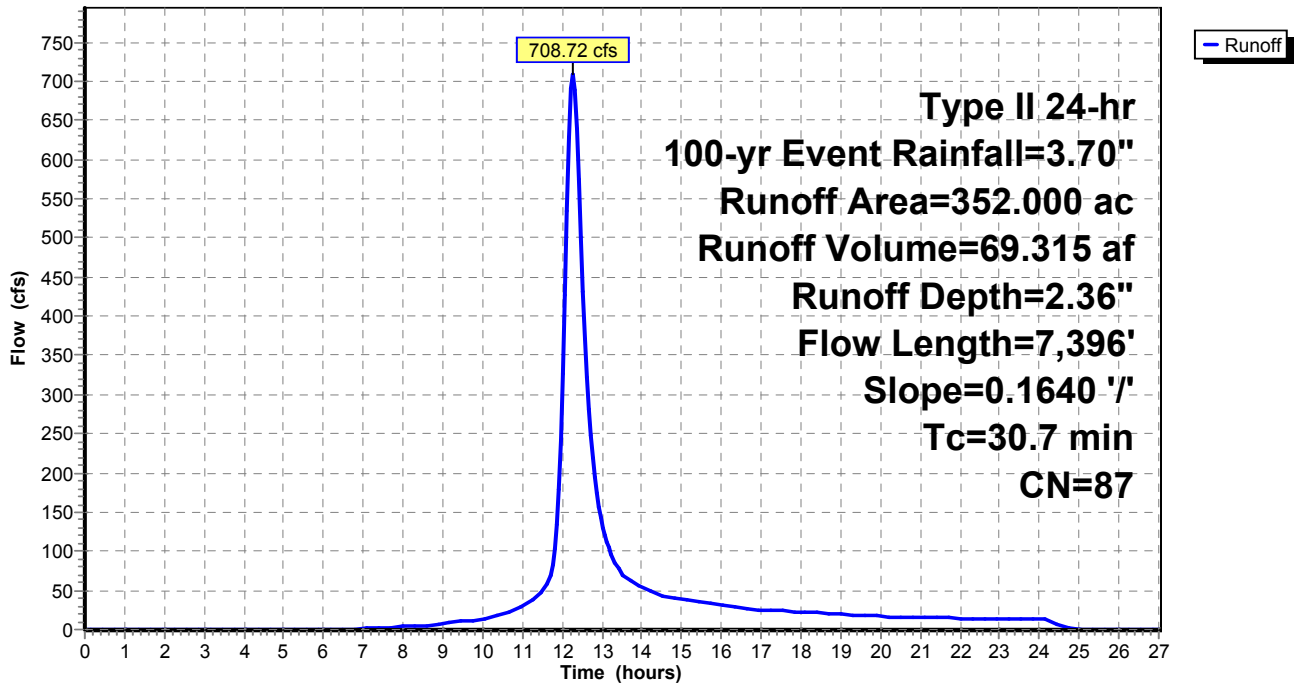
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

Subcatchment 1: WS 1

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 2: WS 2

Runoff = 377.27 cfs @ 12.05 hrs, Volume= 23.195 af, Depth= 2.36"

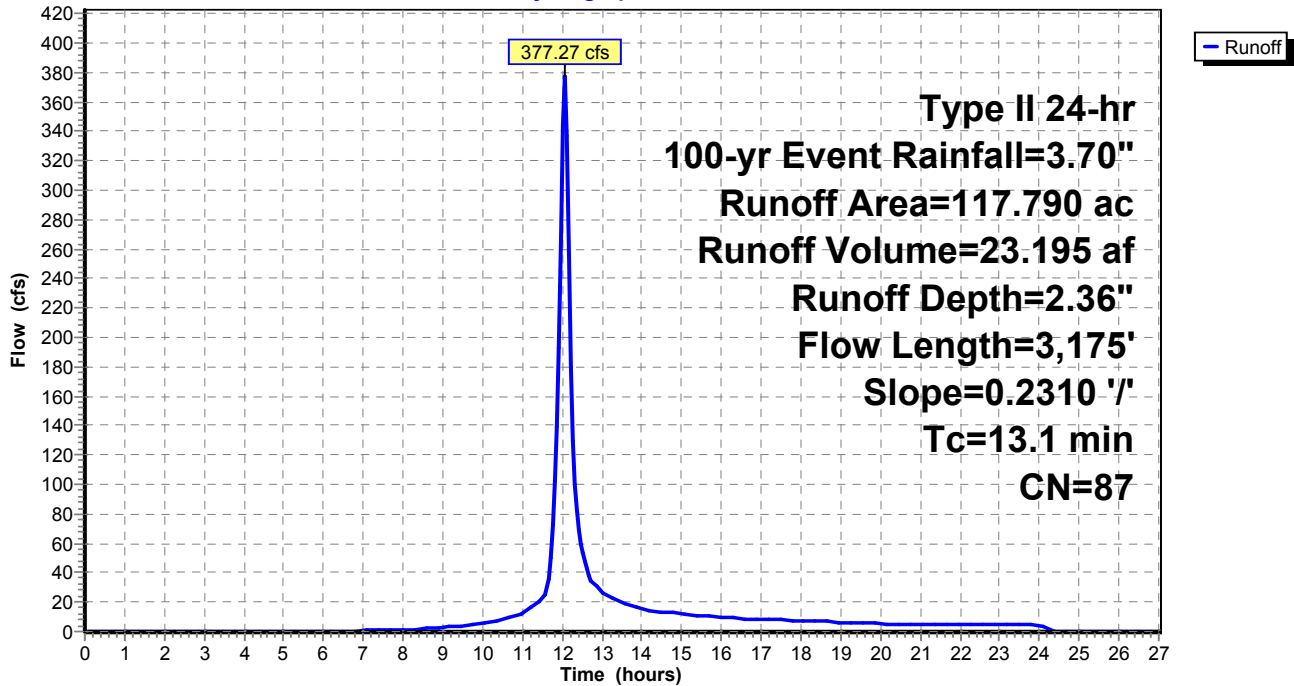
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 3: WS 3

Runoff = 277.70 cfs @ 12.10 hrs, Volume= 19.883 af, Depth= 2.36"

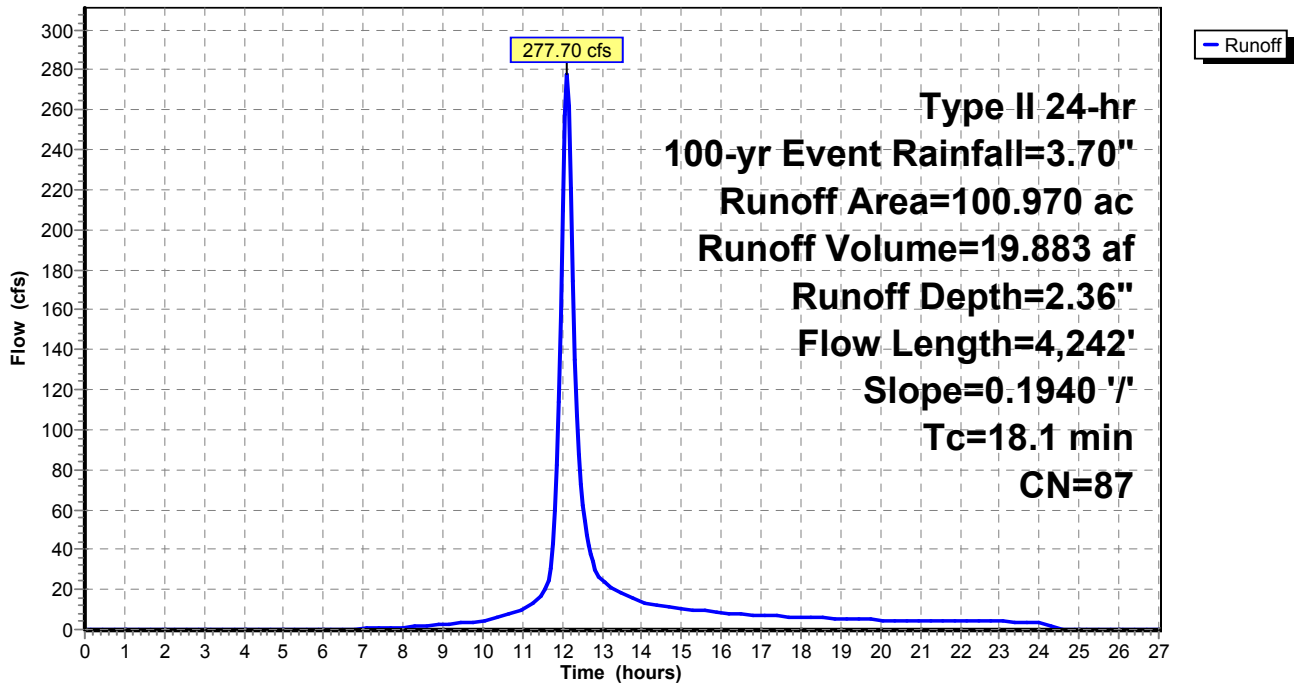
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 4: WS 4

Runoff = 280.90 cfs @ 12.04 hrs, Volume= 16.744 af, Depth= 2.36"

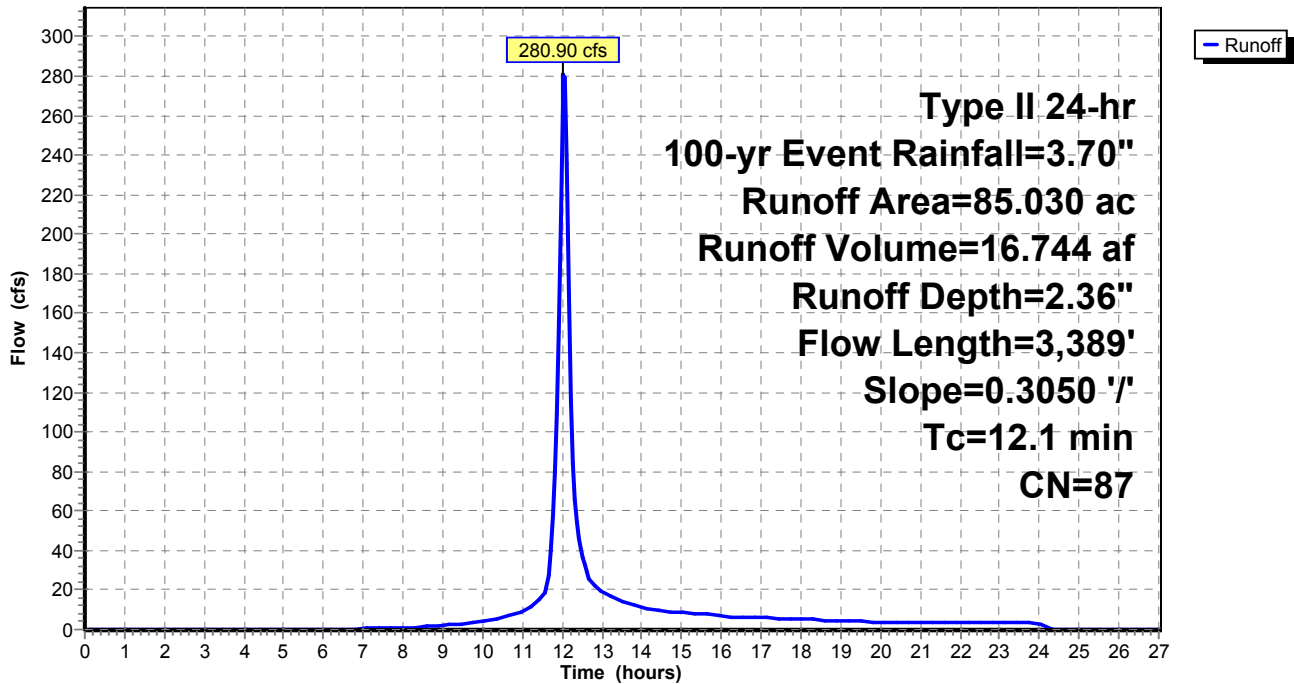
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 5: WS 5

Runoff = 343.86 cfs @ 12.05 hrs, Volume= 21.141 af, Depth= 2.36"

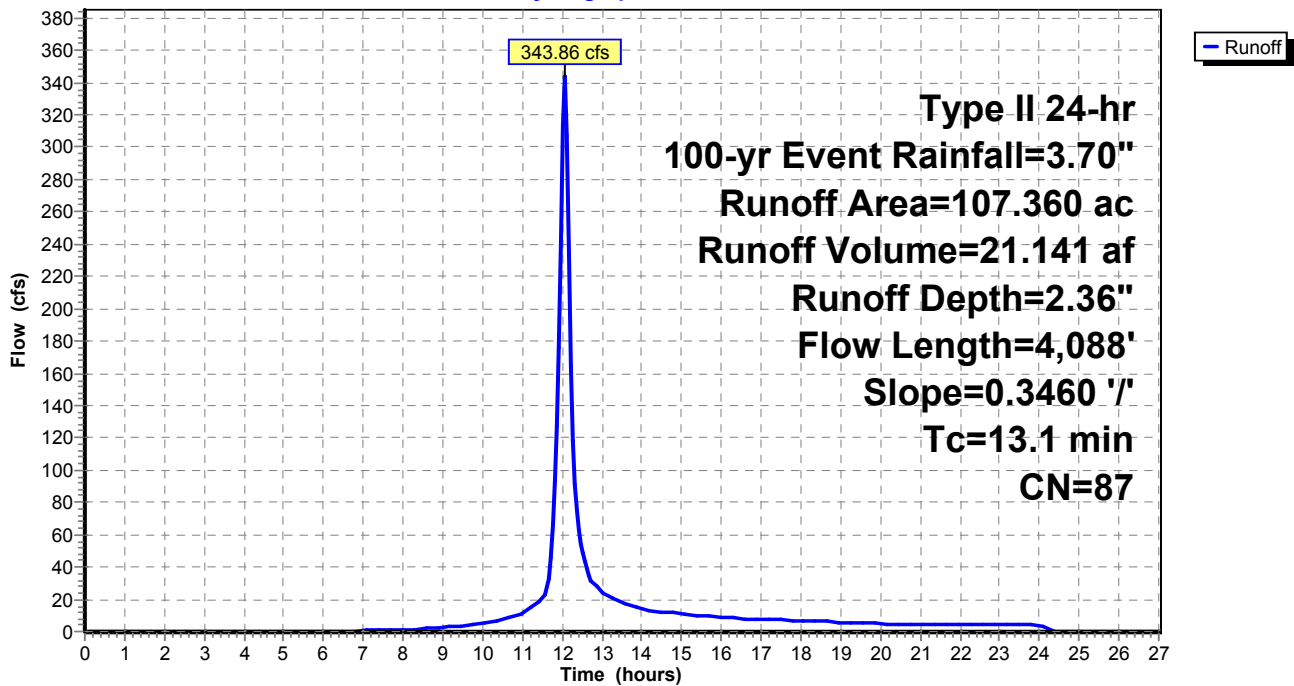
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 6: WS 6

Runoff = 788.24 cfs @ 12.23 hrs, Volume= 74.344 af, Depth= 2.36"

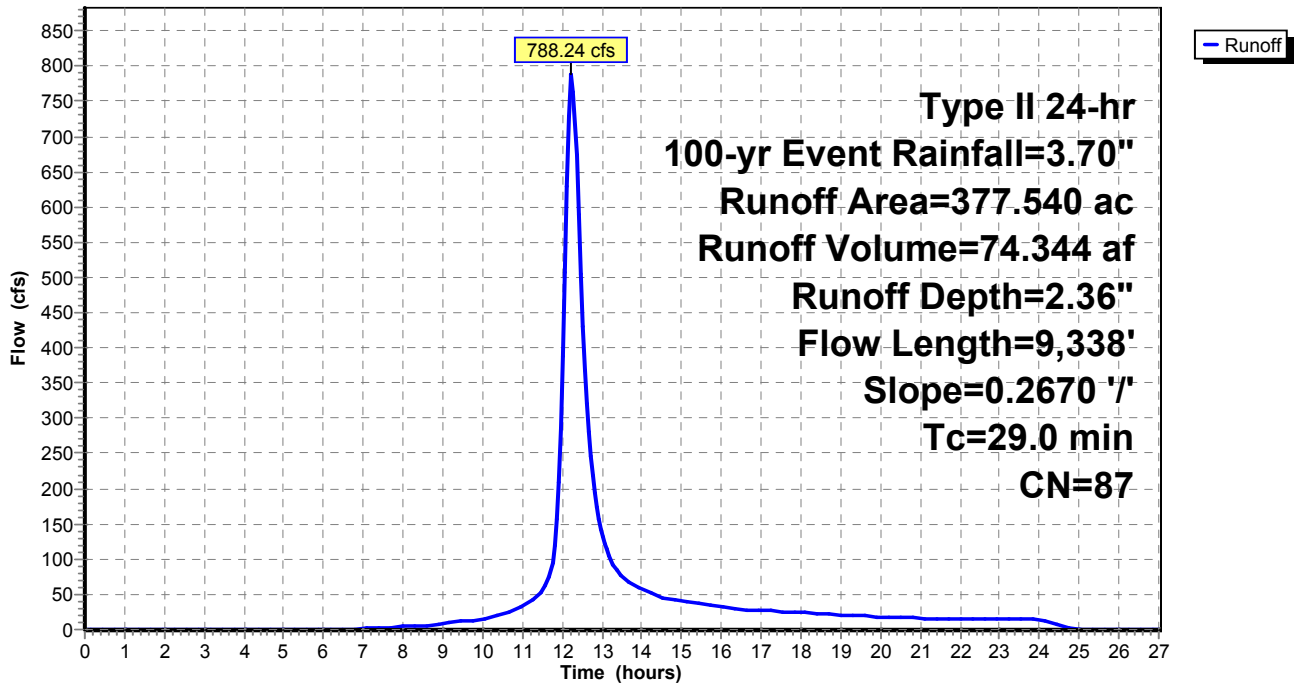
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 7: WS 7

Runoff = 405.48 cfs @ 12.10 hrs, Volume= 28.449 af, Depth= 2.36"

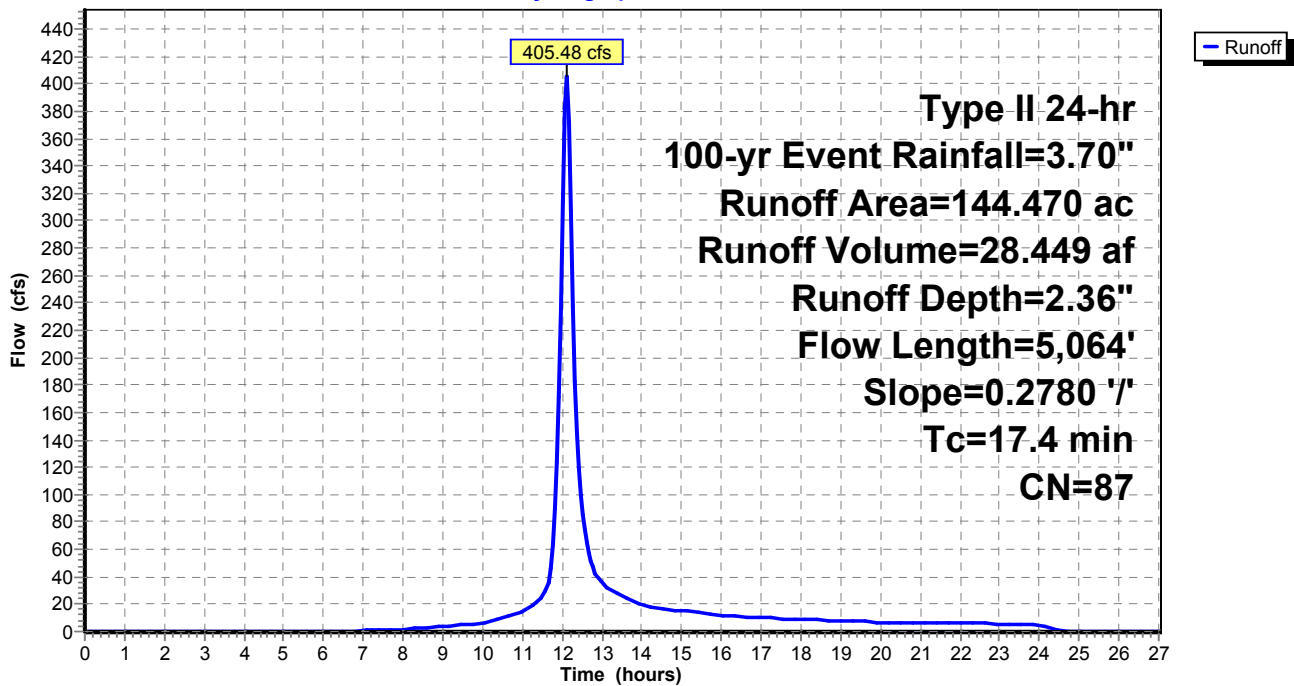
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 8: WS 8

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

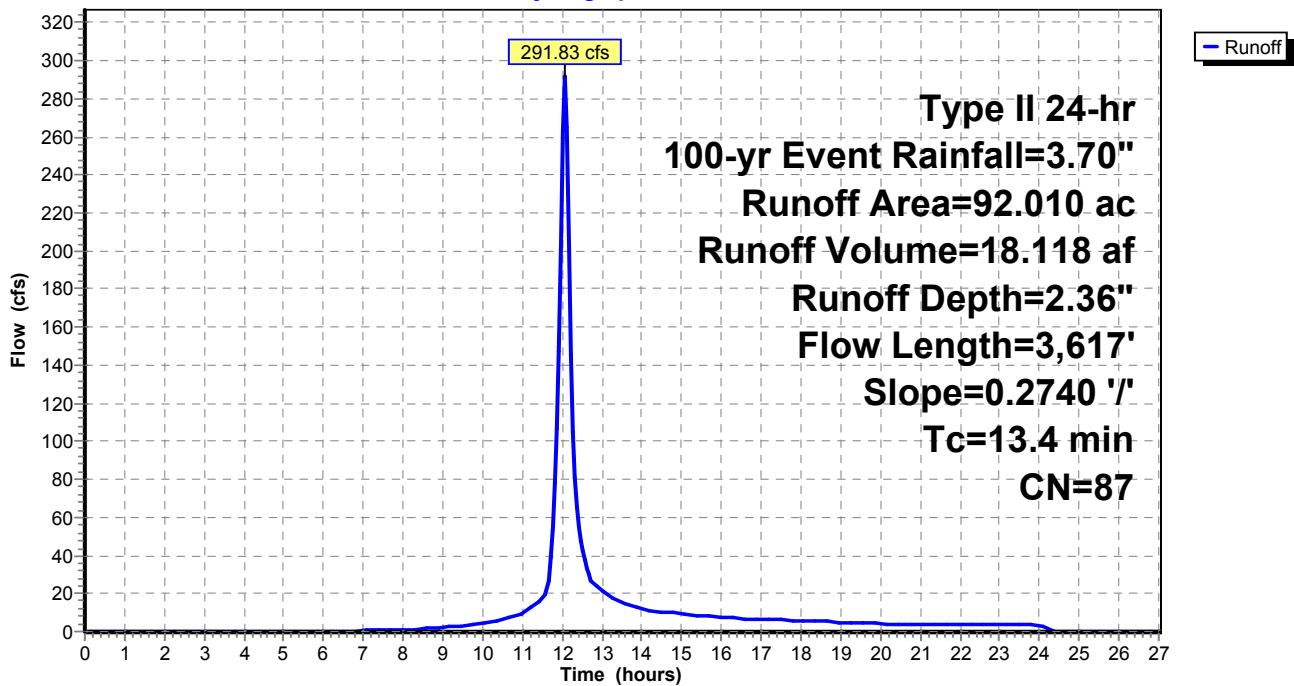
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 9: WS 9

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

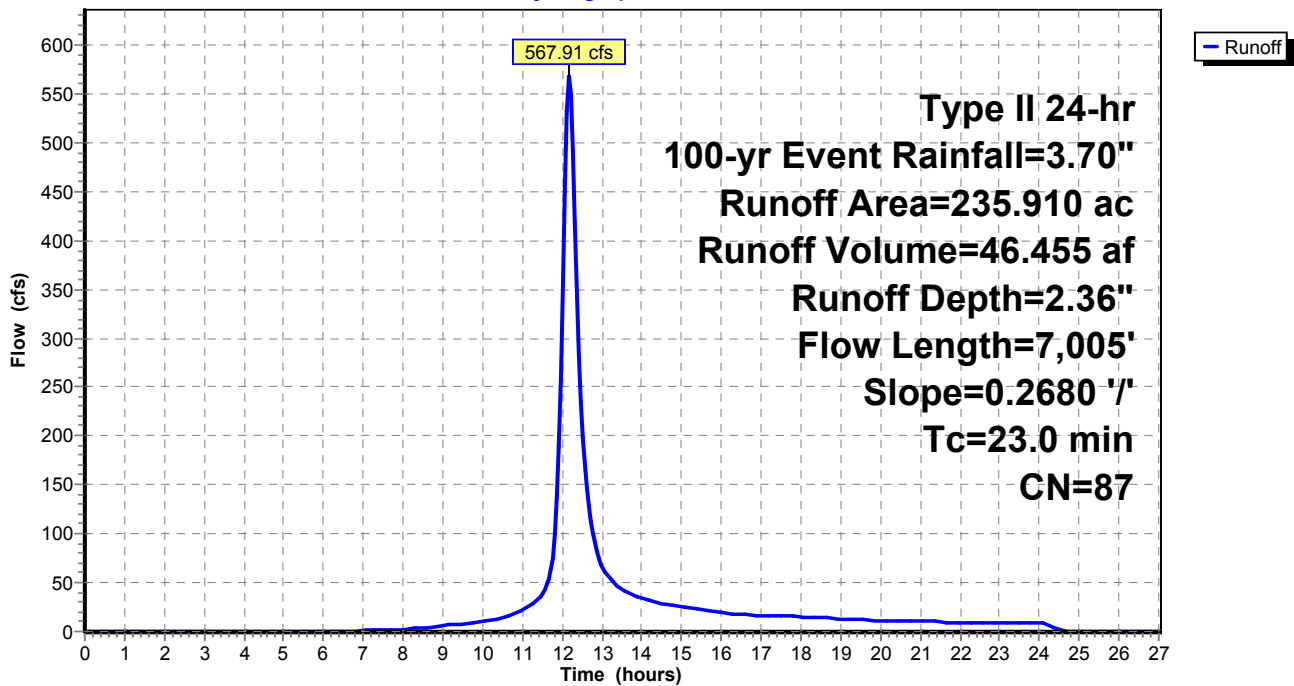
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 10: WS 10

Runoff = 660.28 cfs @ 12.26 hrs, Volume= 65.695 af, Depth= 2.36"

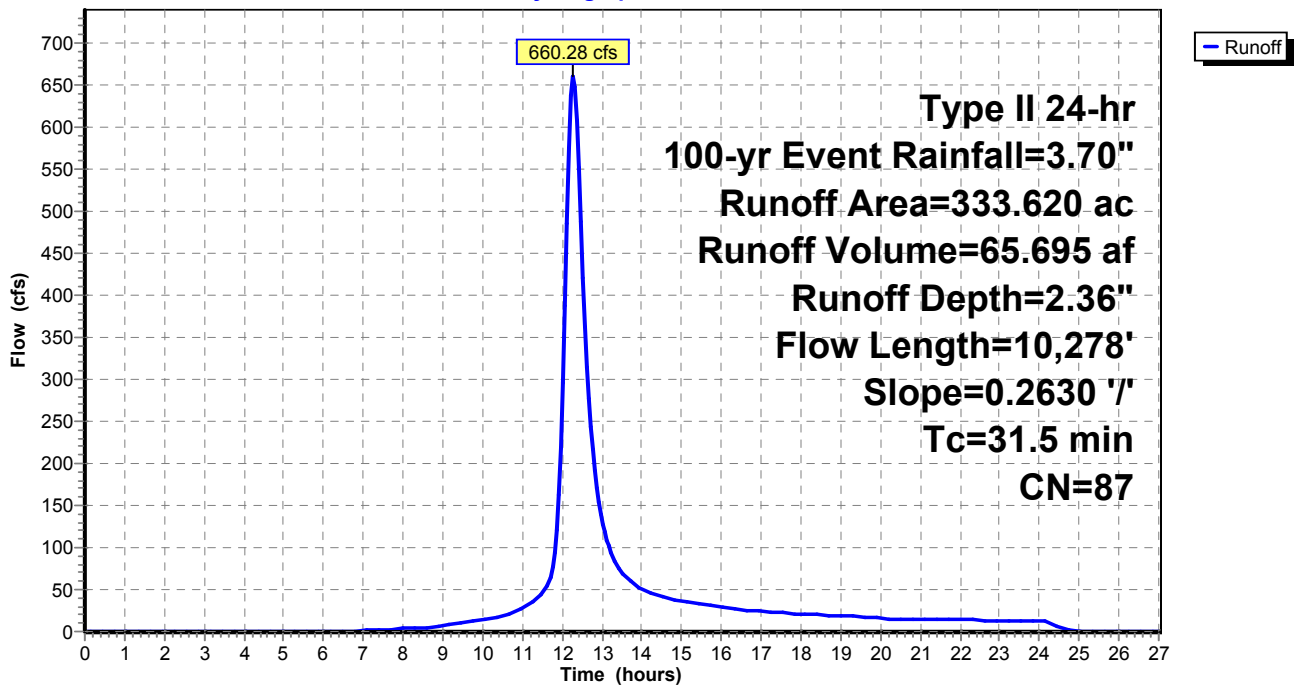
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 11: WS 11

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

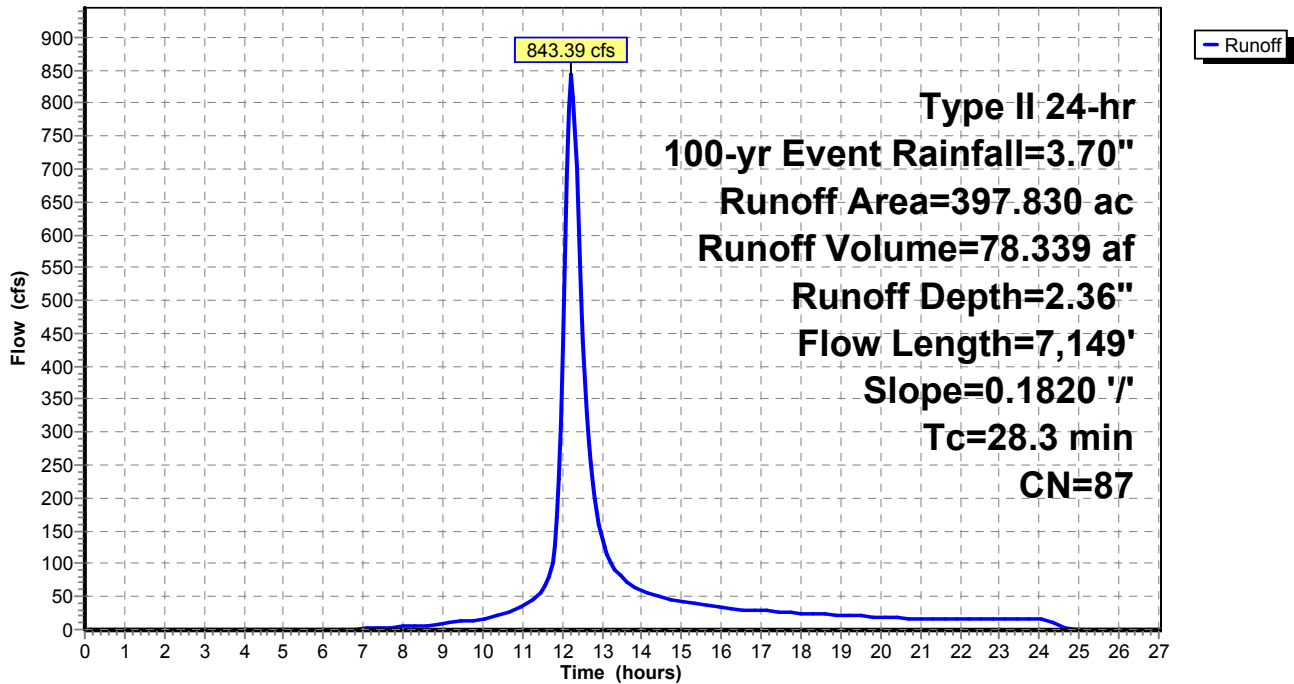
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 12: WS 12

Runoff = 600.47 cfs @ 12.14 hrs, Volume= 46.657 af, Depth= 2.36"

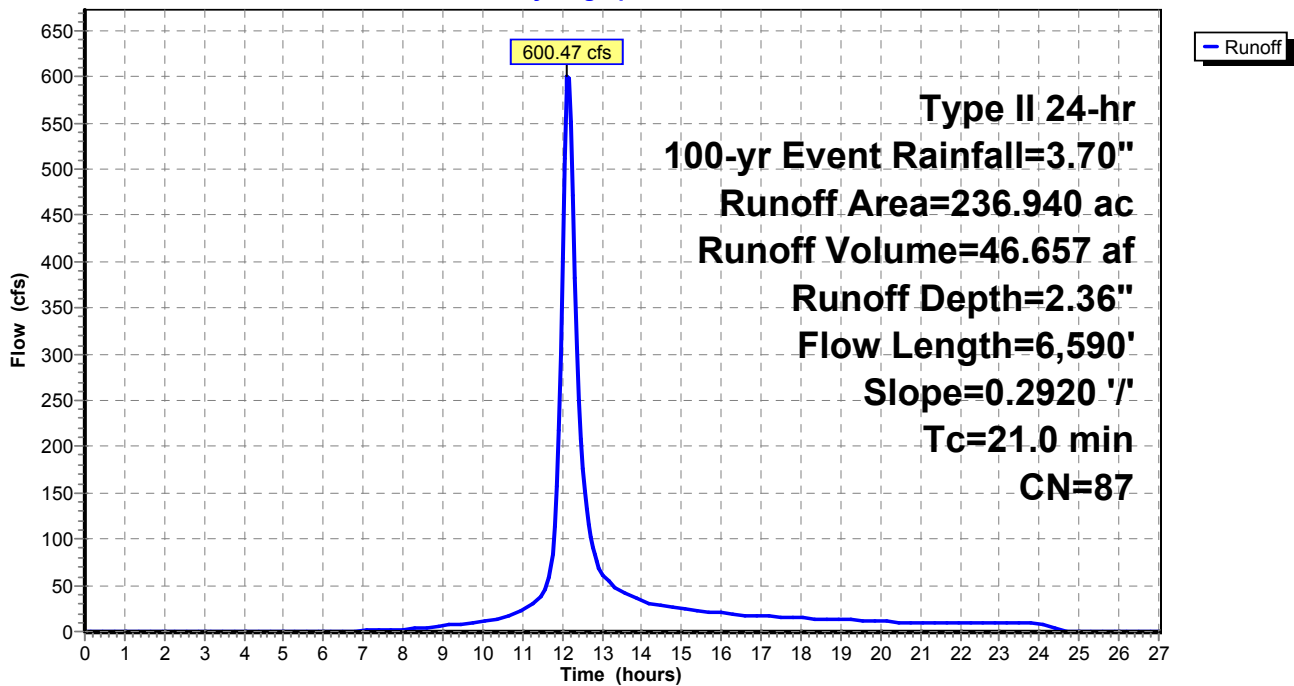
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

Subcatchment 12: WS 12

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 13: WS 13

Runoff = 641.72 cfs @ 12.18 hrs, Volume= 55.621 af, Depth= 2.36"

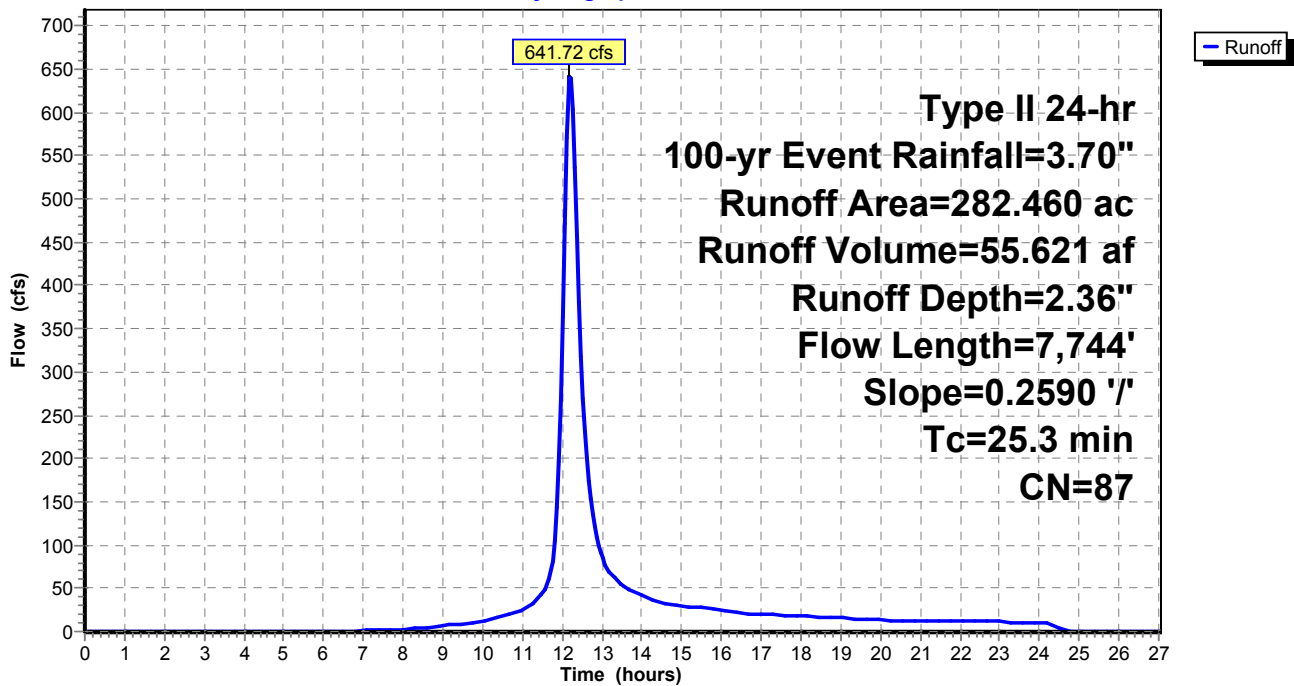
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 14: WS 14

Runoff = 371.20 cfs @ 12.18 hrs, Volume= 31.893 af, Depth= 2.36"

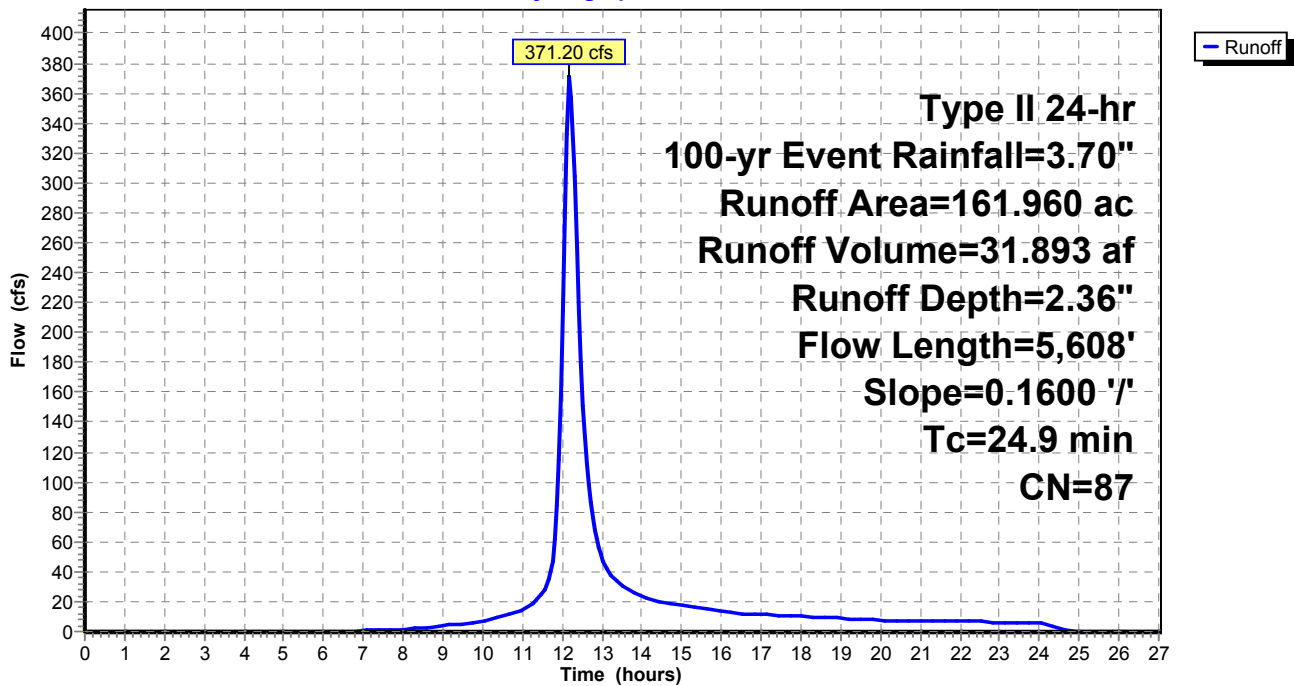
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 15: WS 15

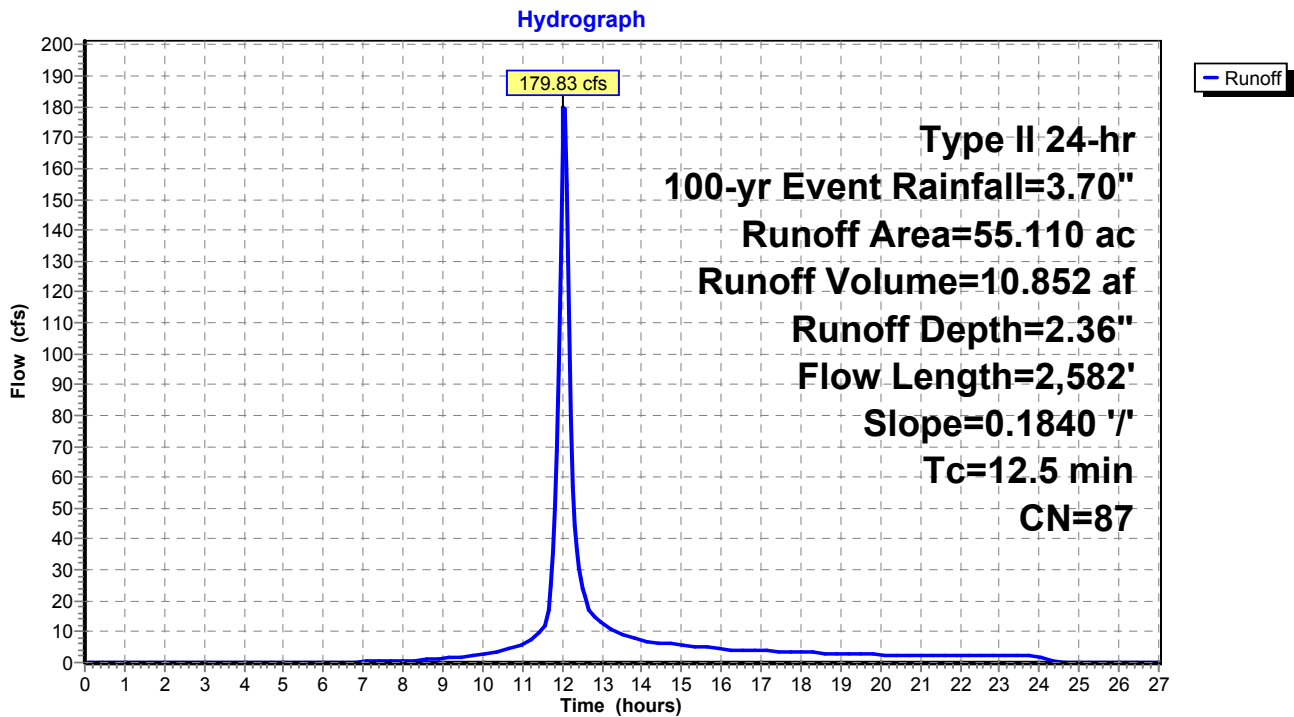
Runoff = 179.83 cfs @ 12.04 hrs, Volume= 10.852 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

Subcatchment 15: WS 15



Existing Watersheds (Pre-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 16: WS 16

Runoff = 204.89 cfs @ 12.15 hrs, Volume= 16.372 af, Depth= 2.36"

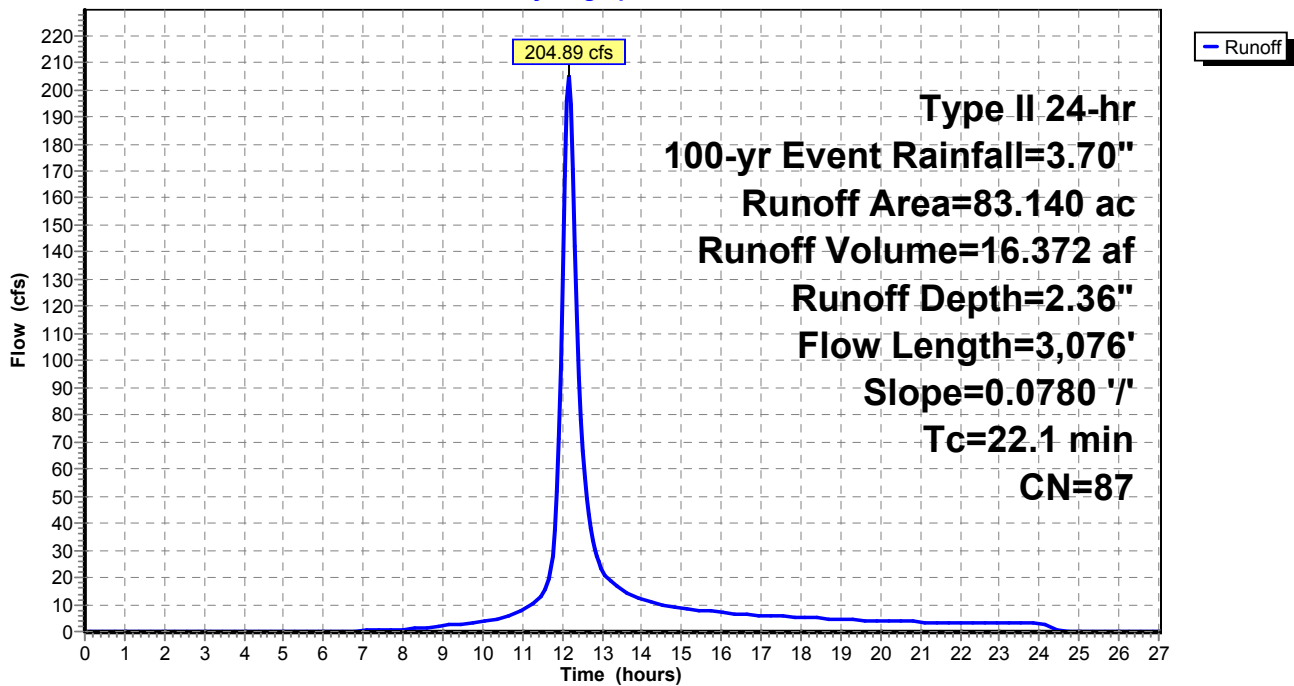
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

Subcatchment 16: WS 16

Hydrograph



Existing Watersheds (Pre-Quintana)*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=813.94 cfs	79.767 af	
Subcatchment2: WS 2	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=432.11 cfs	26.693 af	
Subcatchment3: WS 3	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=318.42 cfs	22.881 af	
Subcatchment4: WS 4	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=321.62 cfs	19.269 af	
Subcatchment5: WS 5	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=393.85 cfs	24.329 af	
Subcatchment6: WS 6	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=906.94 cfs	85.555 af	
Subcatchment7: WS 7	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=464.86 cfs	32.739 af	
Subcatchment8: WS 8	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=334.29 cfs	20.851 af	
Subcatchment9: WS 9	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=651.70 cfs	53.460 af	
Subcatchment10: WS 10	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=758.38 cfs	75.602 af	
Subcatchment11: WS 11	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=968.42 cfs	90.153 af	
Subcatchment12: WS 12	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=688.82 cfs	53.693 af	
Subcatchment13: WS 13	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=736.61 cfs	64.009 af	
Subcatchment14: WS 14	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=426.07 cfs	36.702 af	
Subcatchment15: WS 15	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=205.93 cfs	12.489 af	
Subcatchment16: WS 16	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=235.08 cfs	18.841 af	

Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Total Runoff Area = 3,164.140 ac Runoff Volume = 717.031 af Average Runoff Depth = 2.72"
90.00% Pervious = 2,847.730 ac 10.00% Impervious = 316.410 ac

Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 1: WS 1

Runoff = 813.94 cfs @ 12.25 hrs, Volume= 79.767 af, Depth= 2.72"

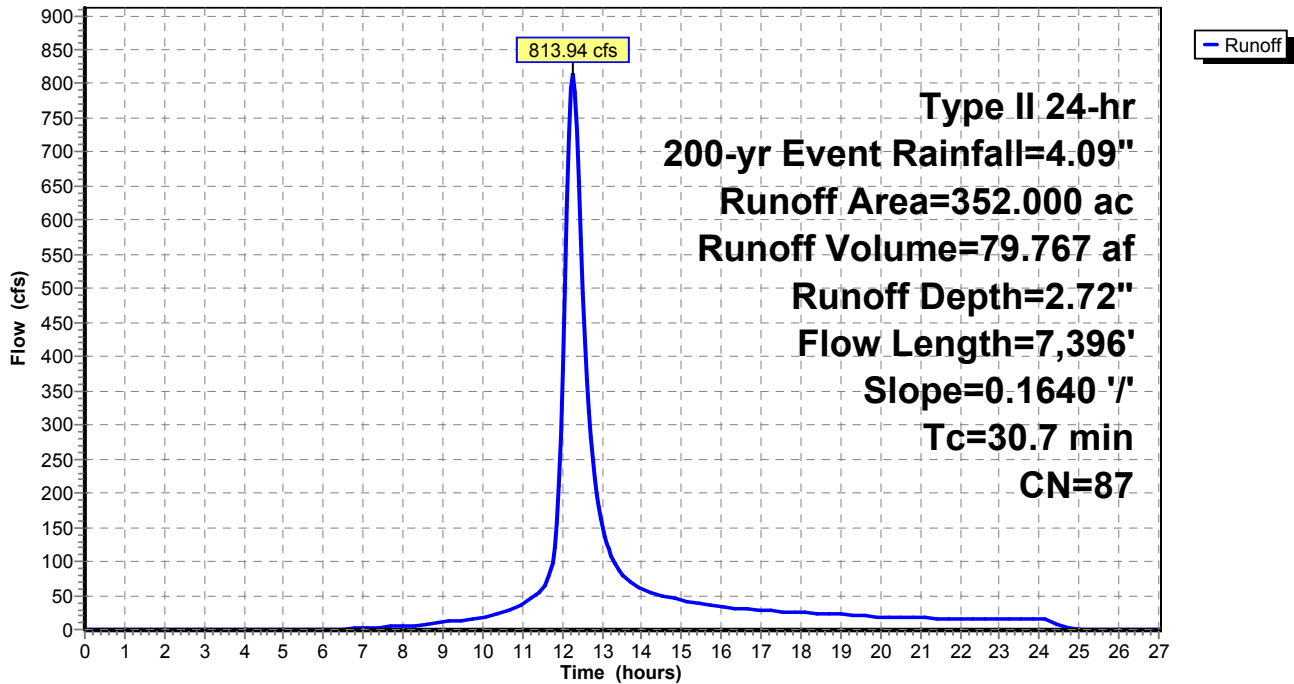
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

Subcatchment 1: WS 1

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 2: WS 2

Runoff = 432.11 cfs @ 12.05 hrs, Volume= 26.693 af, Depth= 2.72"

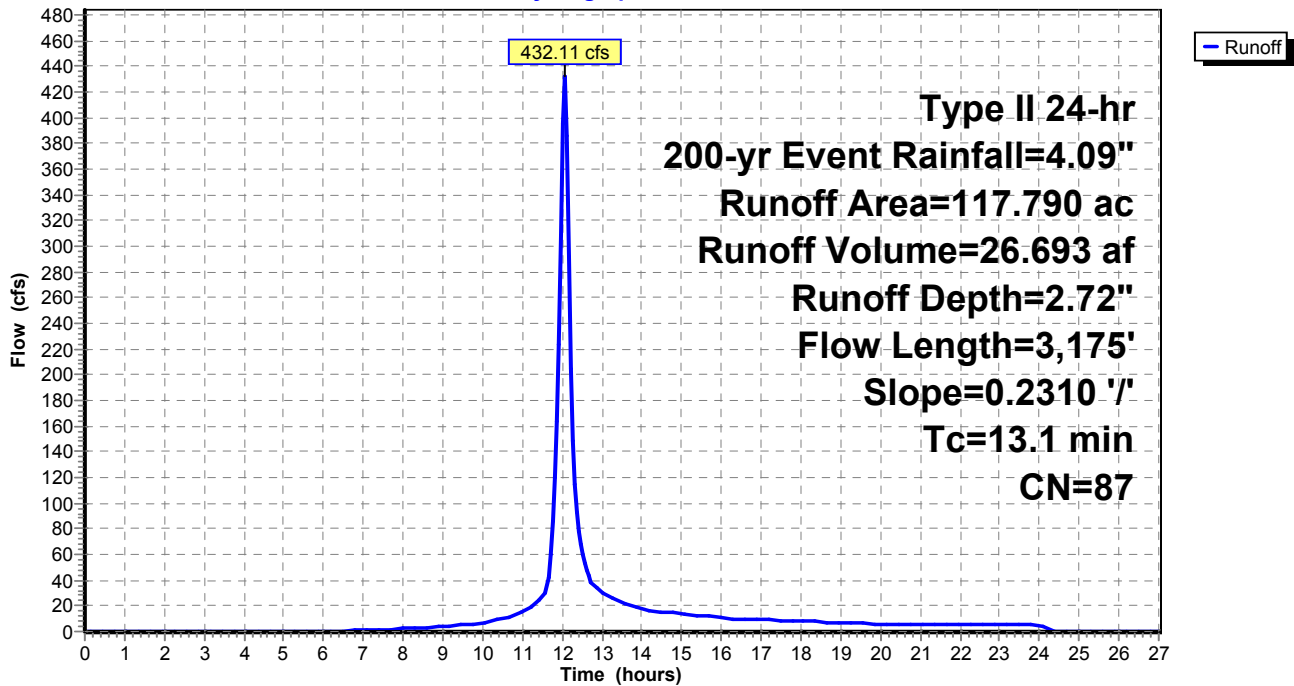
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 3: WS 3

Runoff = 318.42 cfs @ 12.10 hrs, Volume= 22.881 af, Depth= 2.72"

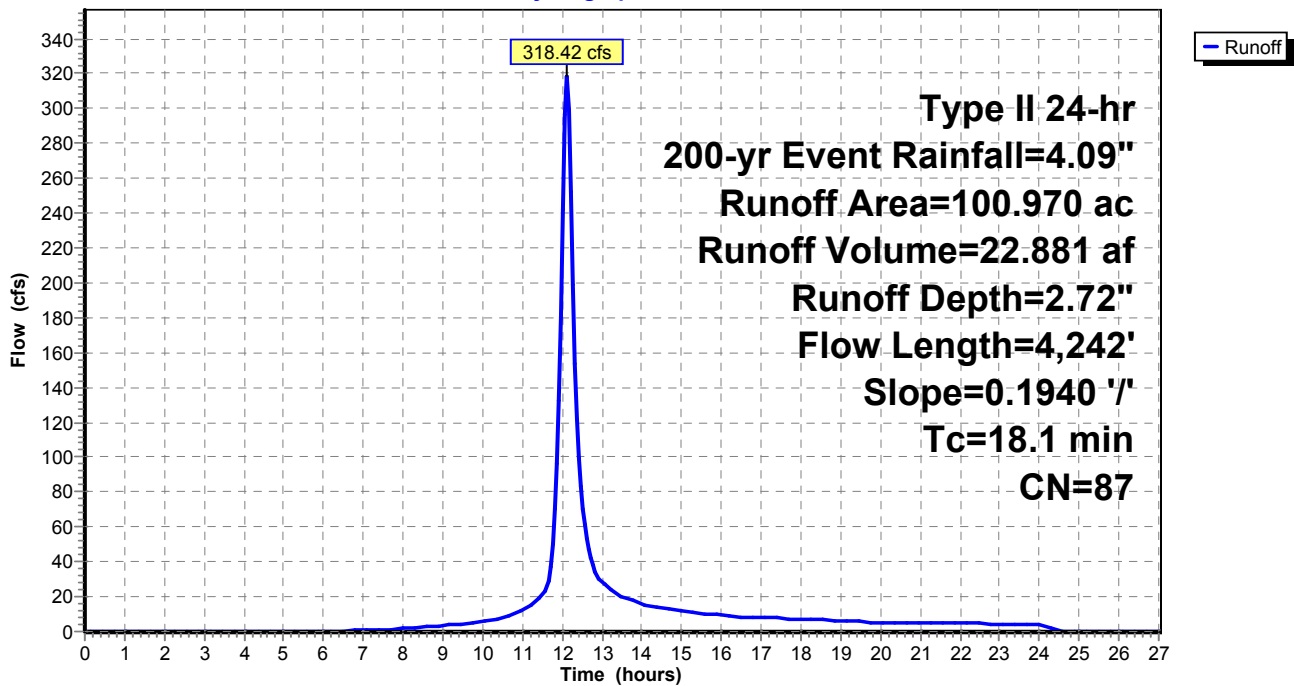
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 4: WS 4

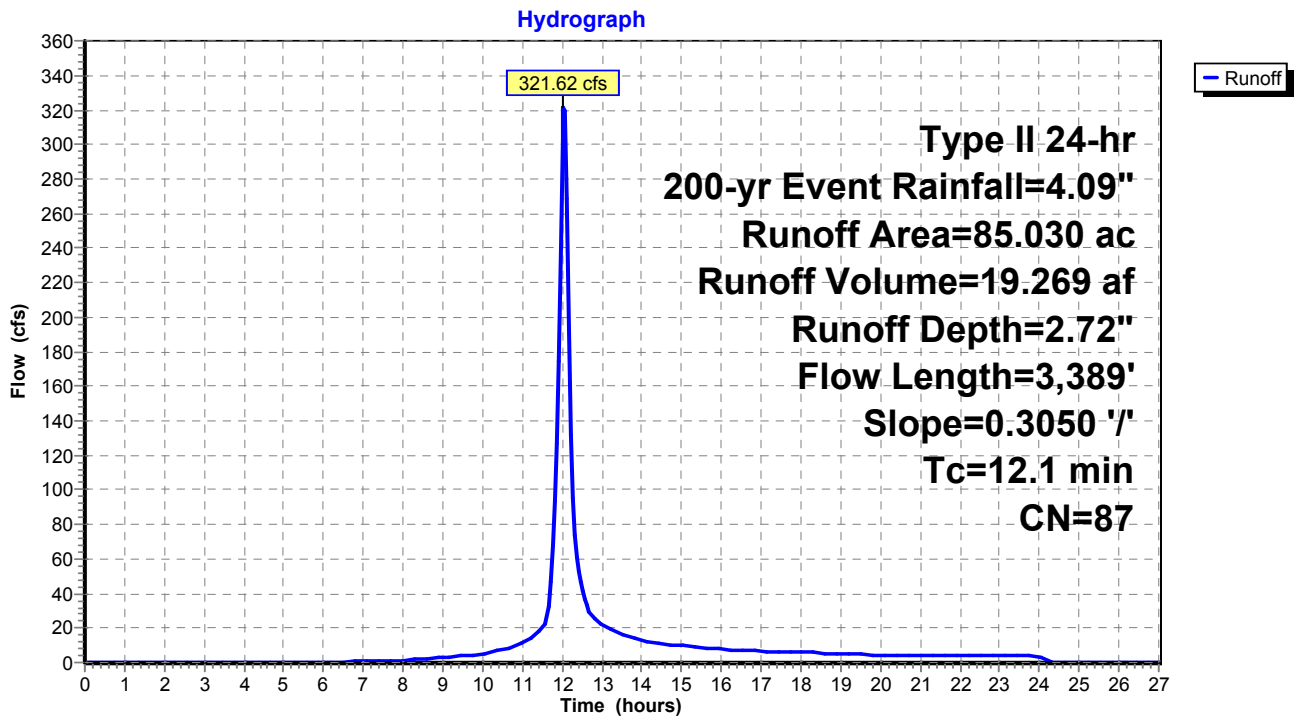
Runoff = 321.62 cfs @ 12.04 hrs, Volume= 19.269 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

Subcatchment 4: WS 4



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 5: WS 5

Runoff = 393.85 cfs @ 12.05 hrs, Volume= 24.329 af, Depth= 2.72"

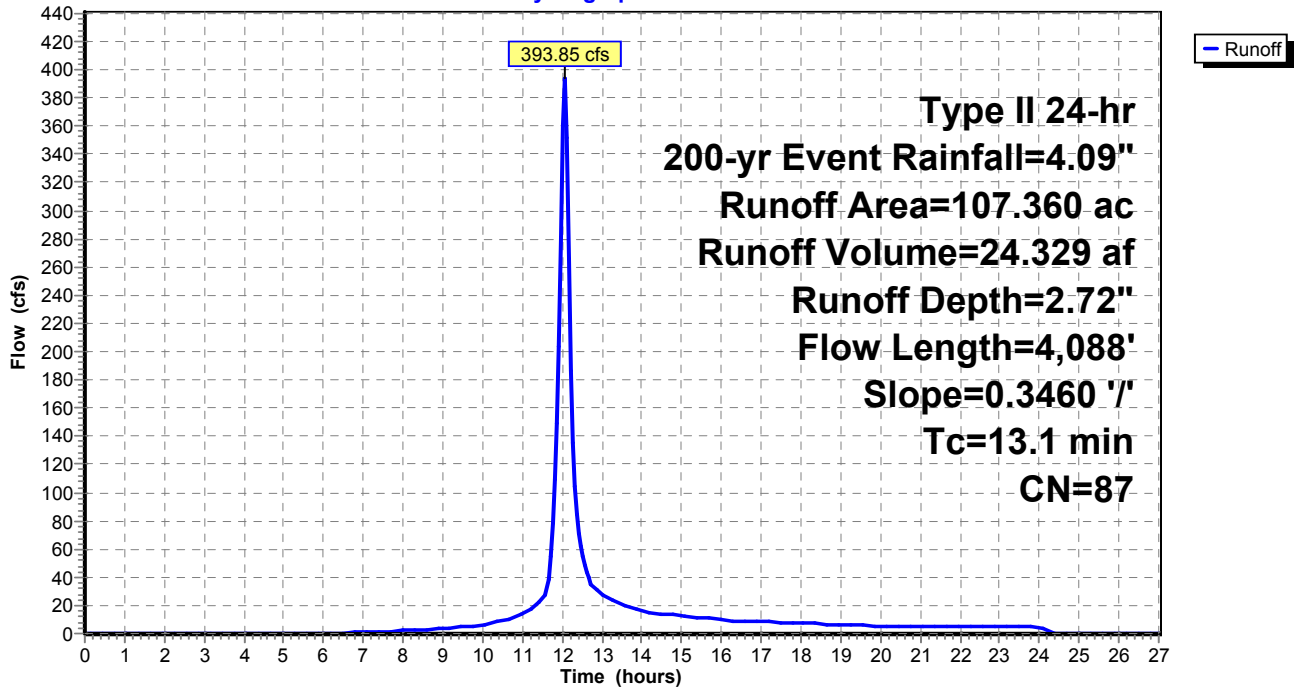
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 6: WS 6

Runoff = 906.94 cfs @ 12.22 hrs, Volume= 85.555 af, Depth= 2.72"

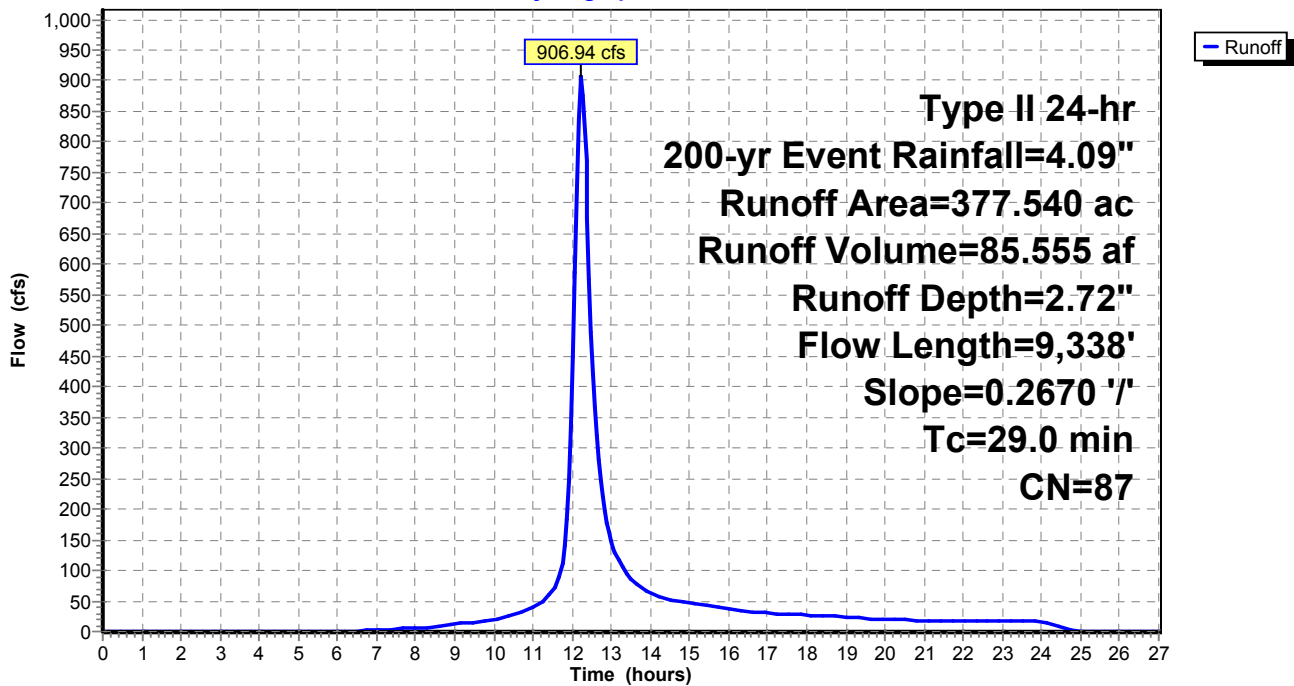
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 7: WS 7

Runoff = 464.86 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

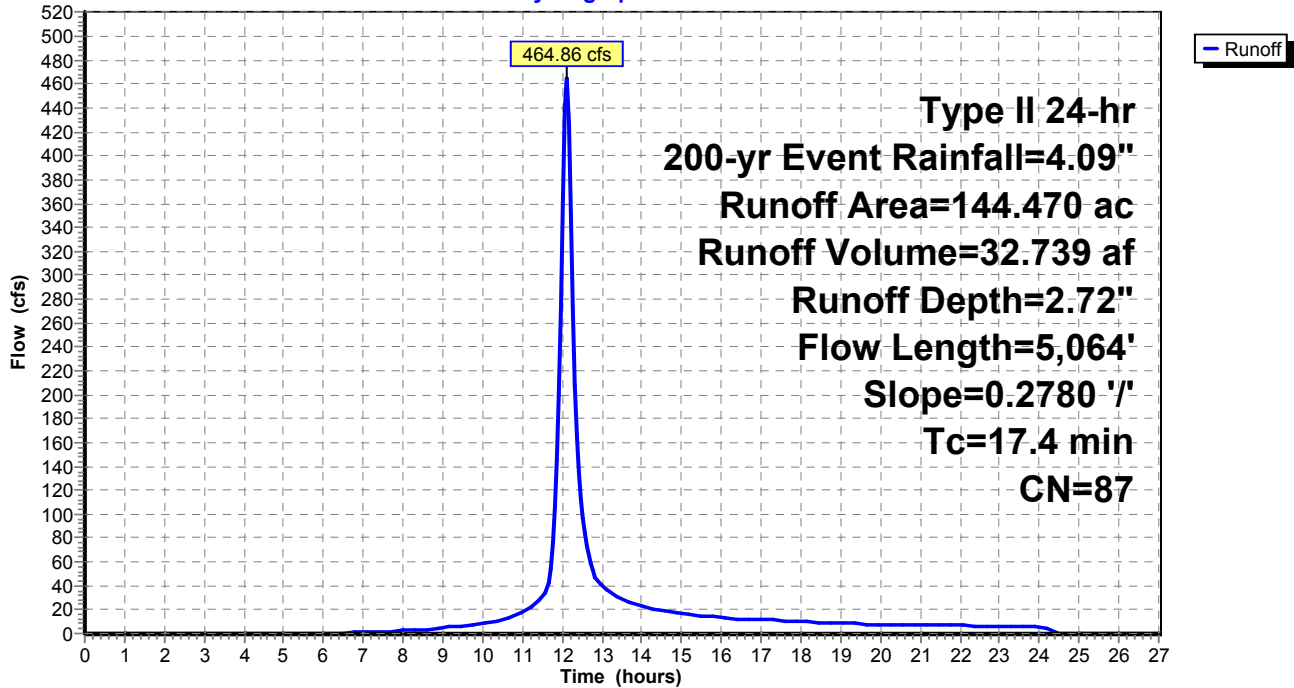
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 8: WS 8

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

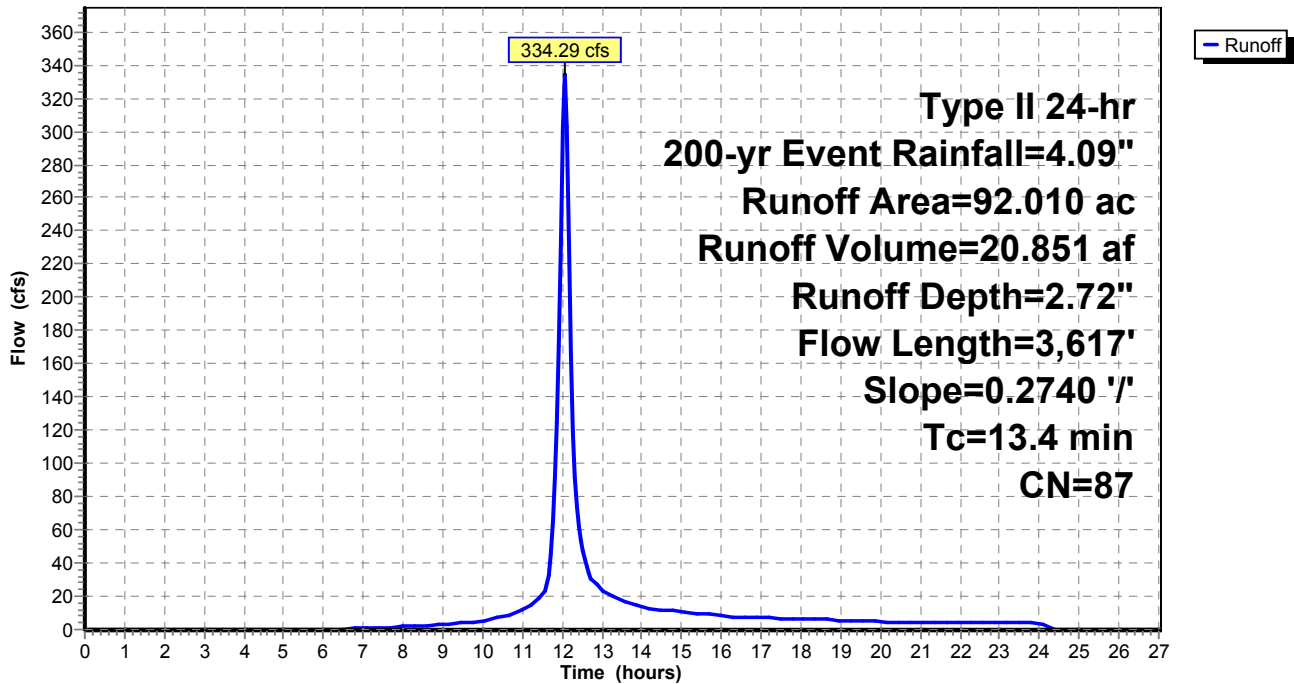
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 9: WS 9

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

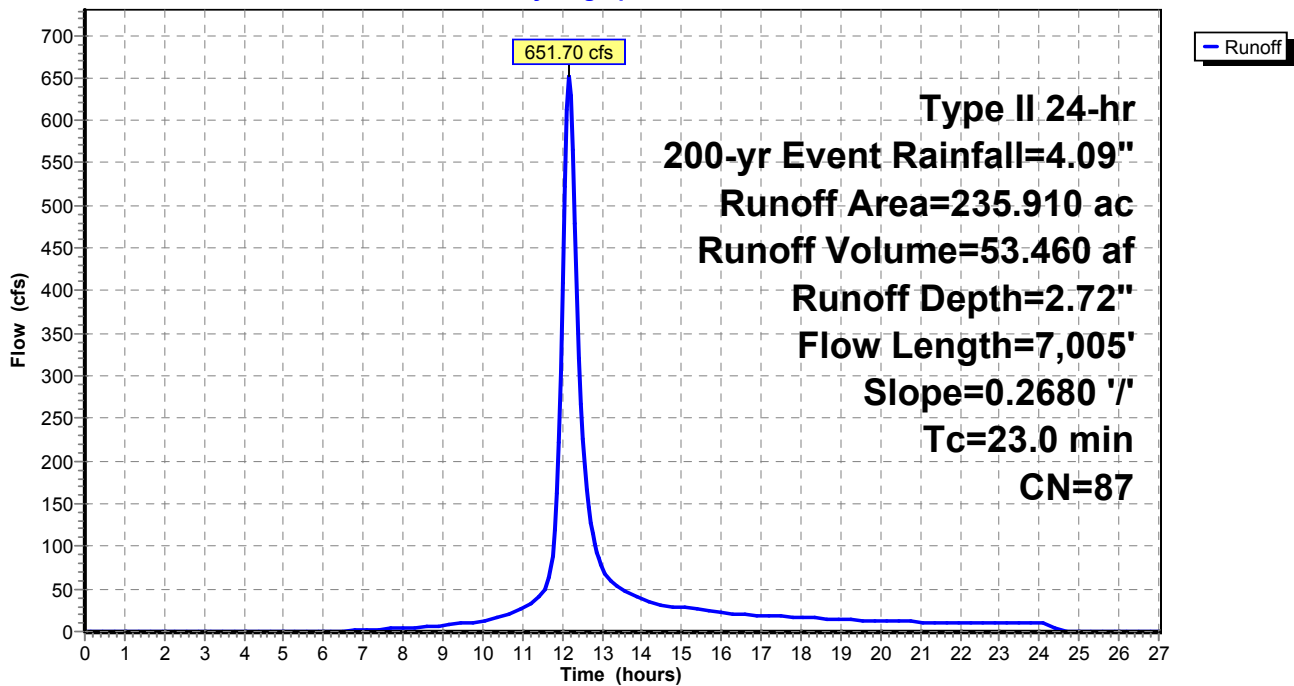
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 10: WS 10

Runoff = 758.38 cfs @ 12.26 hrs, Volume= 75.602 af, Depth= 2.72"

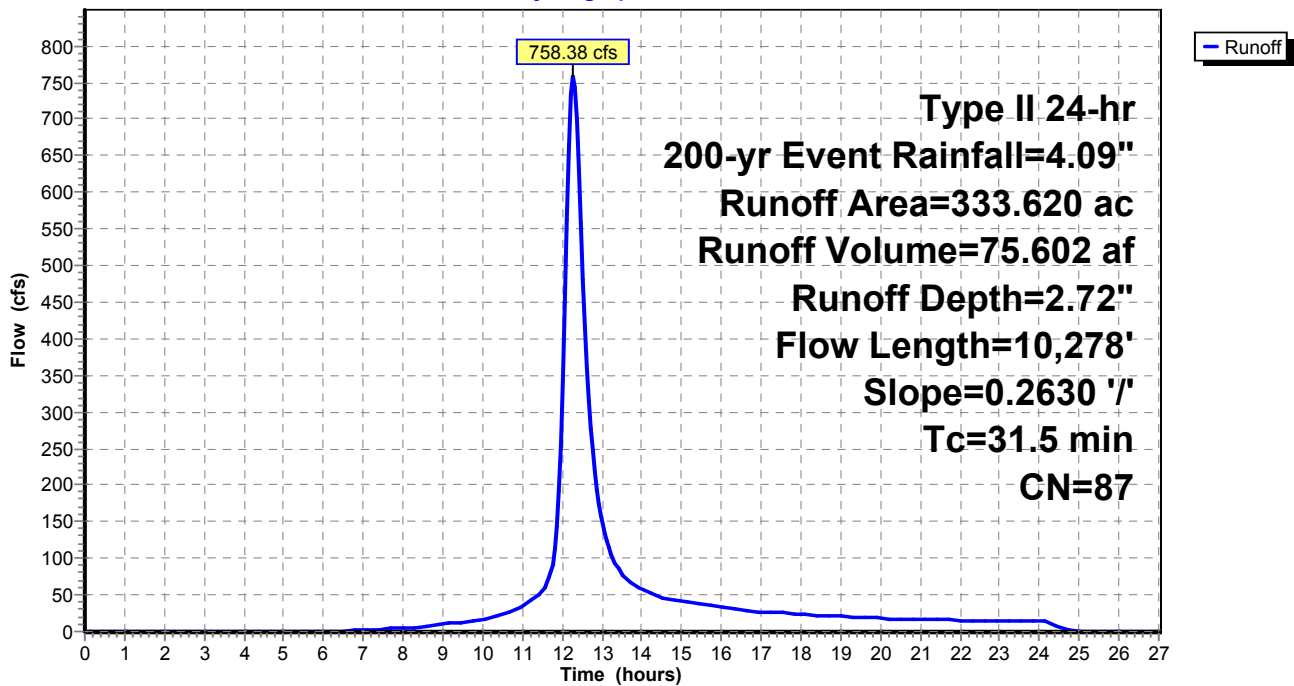
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 11: WS 11

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

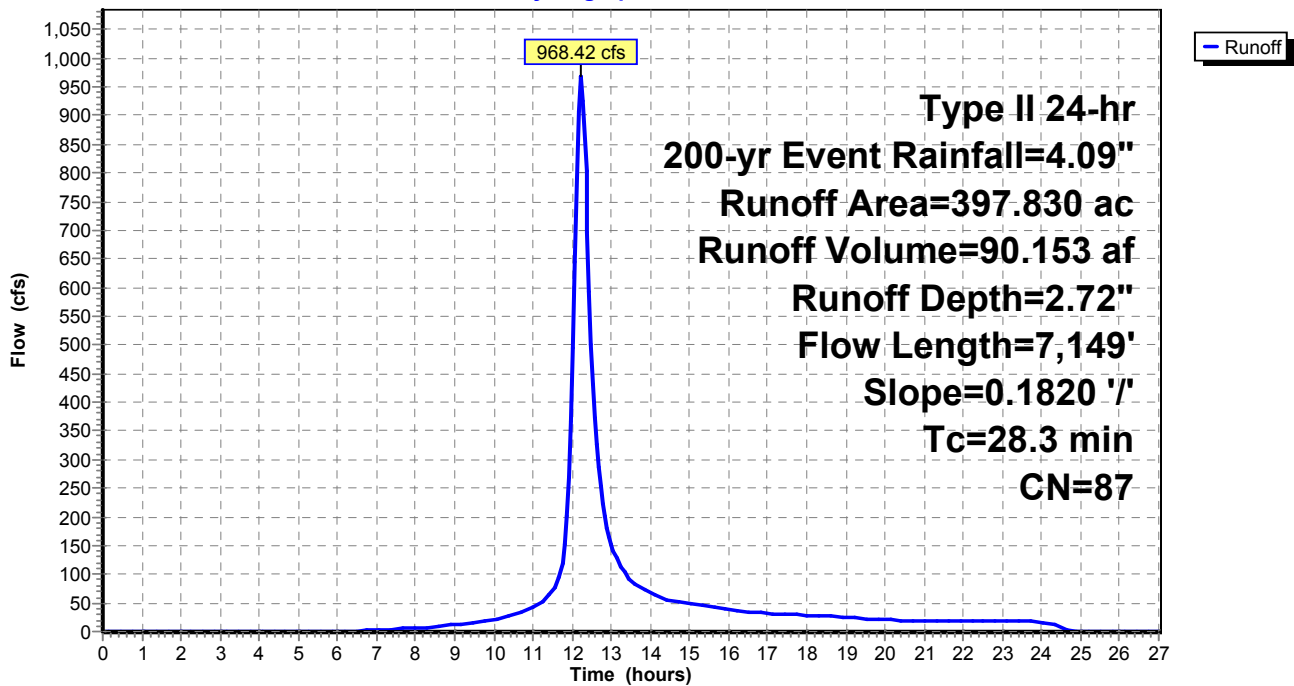
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 12: WS 12

Runoff = 688.82 cfs @ 12.13 hrs, Volume= 53.693 af, Depth= 2.72"

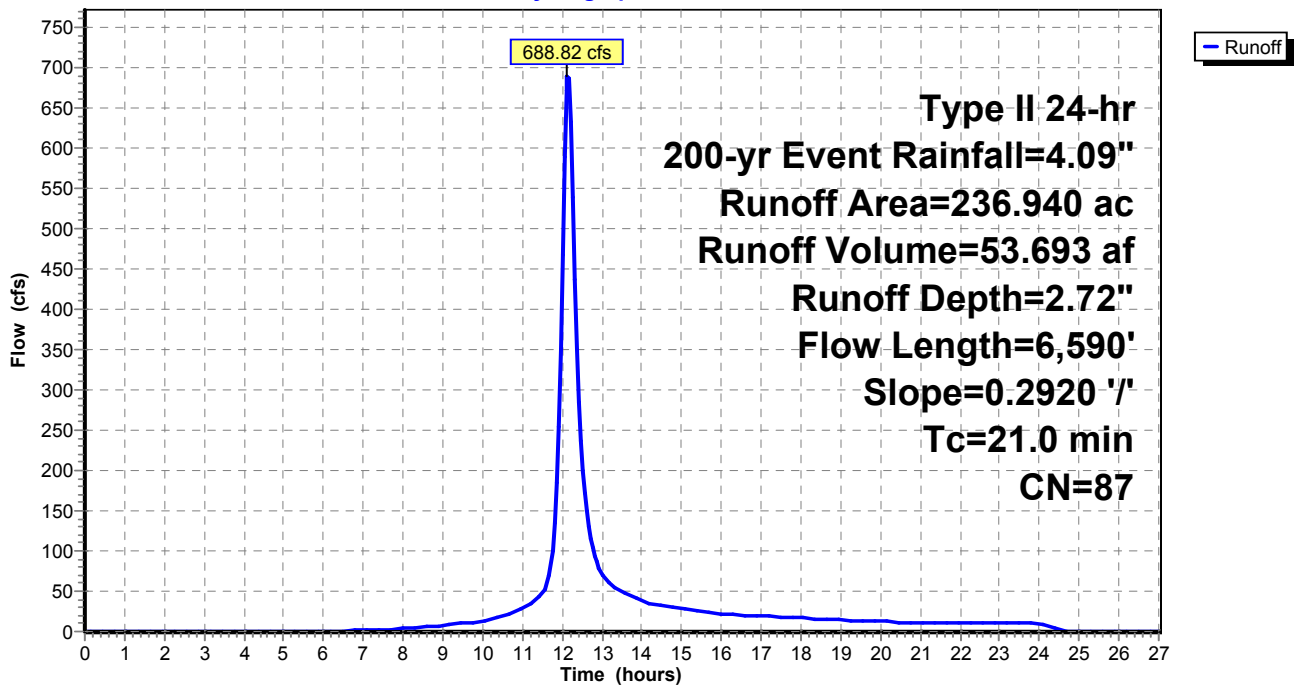
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

Subcatchment 12: WS 12

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 13: WS 13

Runoff = 736.61 cfs @ 12.18 hrs, Volume= 64.009 af, Depth= 2.72"

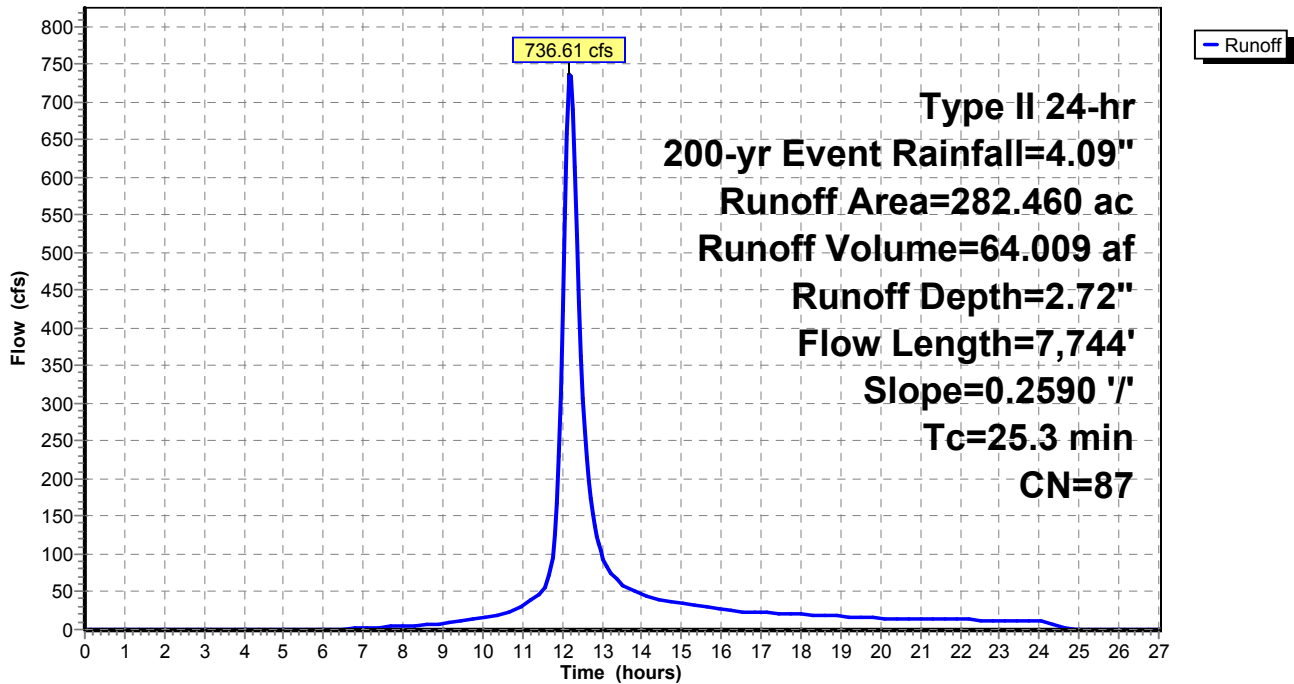
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 14: WS 14

Runoff = 426.07 cfs @ 12.18 hrs, Volume= 36.702 af, Depth= 2.72"

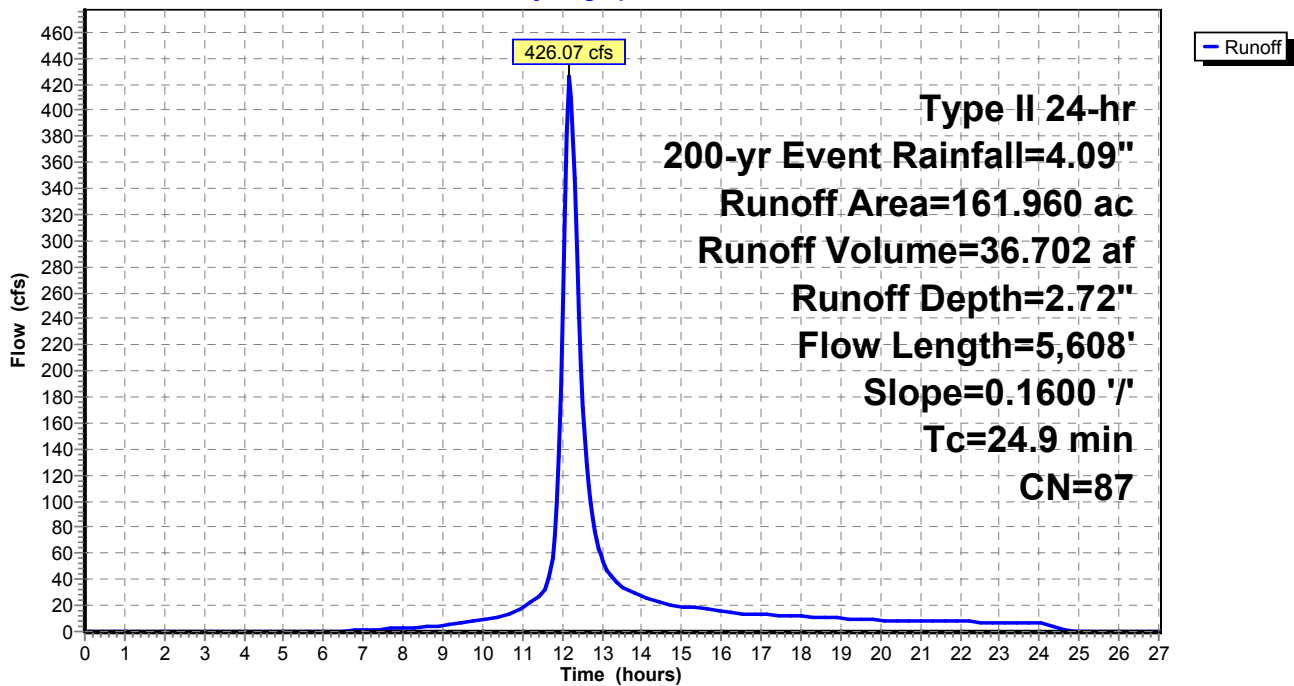
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 15: WS 15

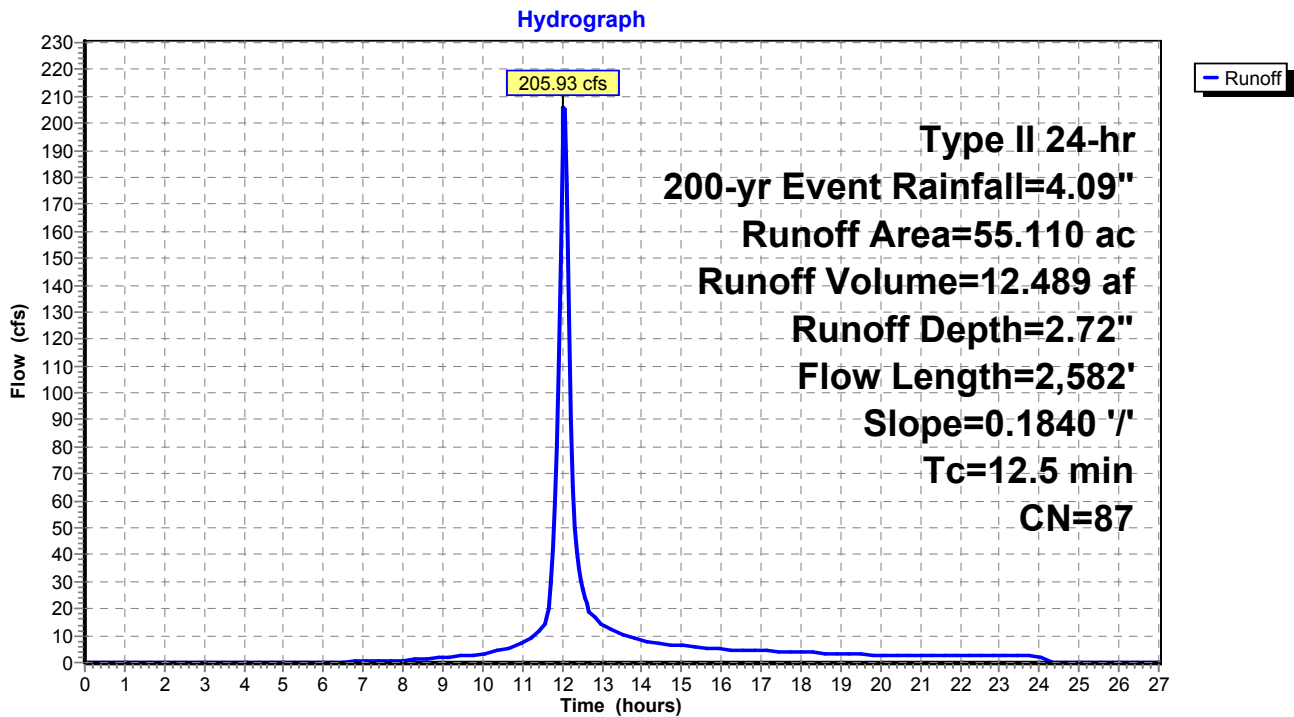
Runoff = 205.93 cfs @ 12.04 hrs, Volume= 12.489 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

Subcatchment 15: WS 15



Existing Watersheds (Pre-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 16: WS 16

Runoff = 235.08 cfs @ 12.15 hrs, Volume= 18.841 af, Depth= 2.72"

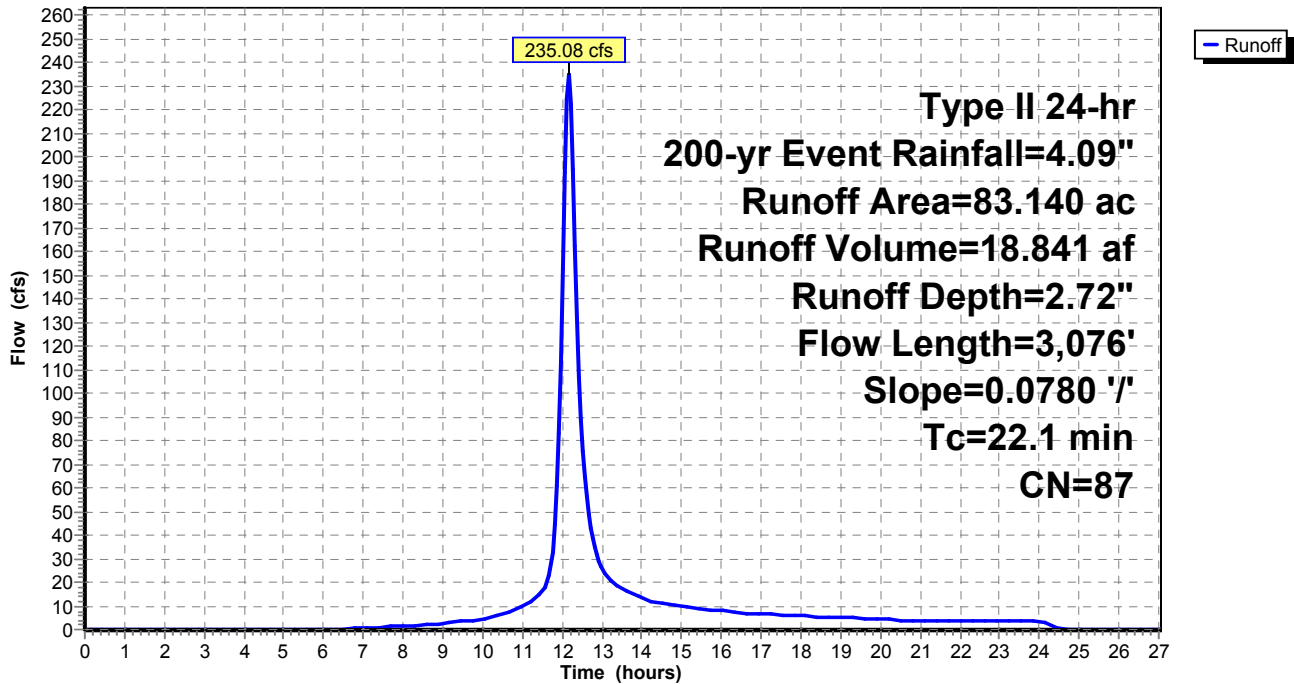
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

Subcatchment 16: WS 16

Hydrograph



Existing Watersheds (Pre-Quintana)*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=952.23 cfs	93.637 af	
Subcatchment2: WS 2	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=504.05 cfs	31.334 af	
Subcatchment3: WS 3	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=371.87 cfs	26.859 af	
Subcatchment4: WS 4	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=375.03 cfs	22.619 af	
Subcatchment5: WS 5	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=459.42 cfs	28.559 af	
Subcatchment6: WS 6	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=1,060.97 cfs	100.431 af	
Subcatchment7: WS 7	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=542.79 cfs	38.431 af	
Subcatchment8: WS 8	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=389.99 cfs	24.476 af	
Subcatchment9: WS 9	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=761.76 cfs	62.755 af	
Subcatchment10: WS 10	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=887.32 cfs	88.748 af	
Subcatchment11: WS 11	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=1,132.73 cfs	105.828 af	
Subcatchment12: WS 12	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=804.86 cfs	63.029 af	
Subcatchment13: WS 13	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=861.28 cfs	75.138 af	
Subcatchment14: WS 14	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=498.15 cfs	43.084 af	
Subcatchment15: WS 15	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=240.16 cfs	14.660 af	
Subcatchment16: WS 16	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=274.73 cfs	22.116 af	

Existing Watersheds (Pre-Quintana)*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Total Runoff Area = 3,164.140 ac Runoff Volume = 841.705 af Average Runoff Depth = 3.19"
90.00% Pervious = 2,847.730 ac 10.00% Impervious = 316.410 ac

Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 1: WS 1

Runoff = 952.23 cfs @ 12.25 hrs, Volume= 93.637 af, Depth= 3.19"

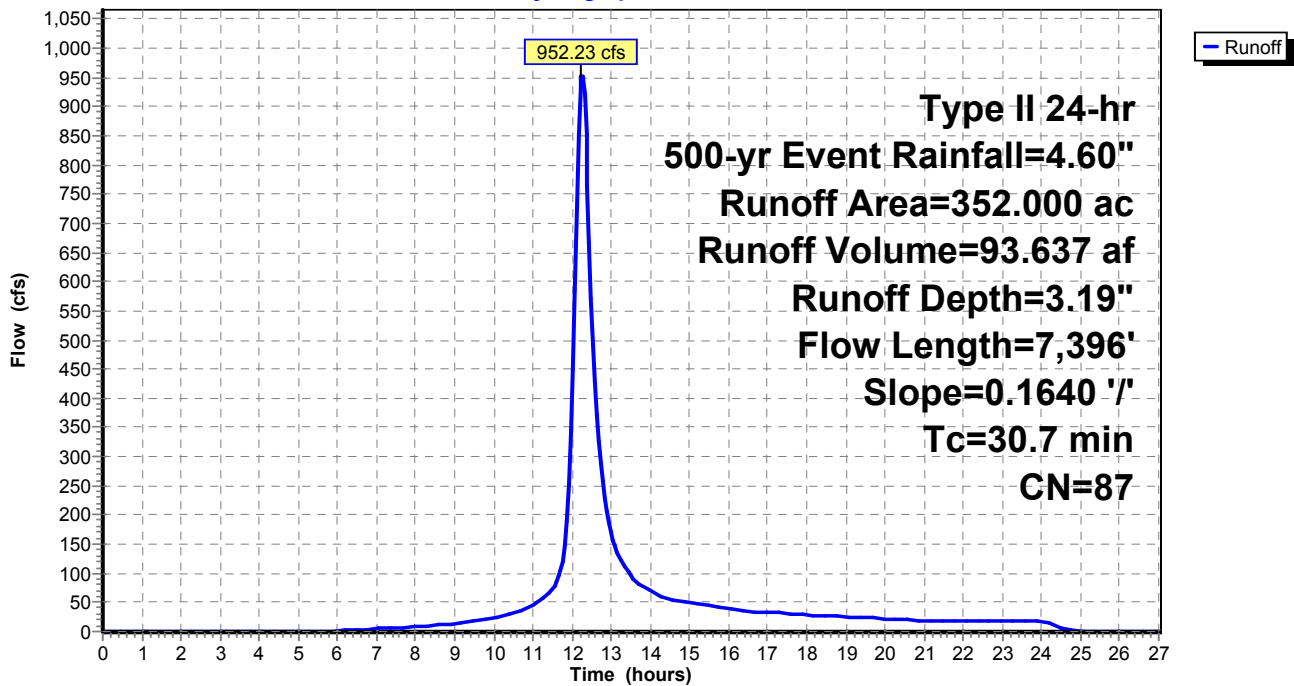
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

Subcatchment 1: WS 1

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 2: WS 2

Runoff = 504.05 cfs @ 12.05 hrs, Volume= 31.334 af, Depth= 3.19"

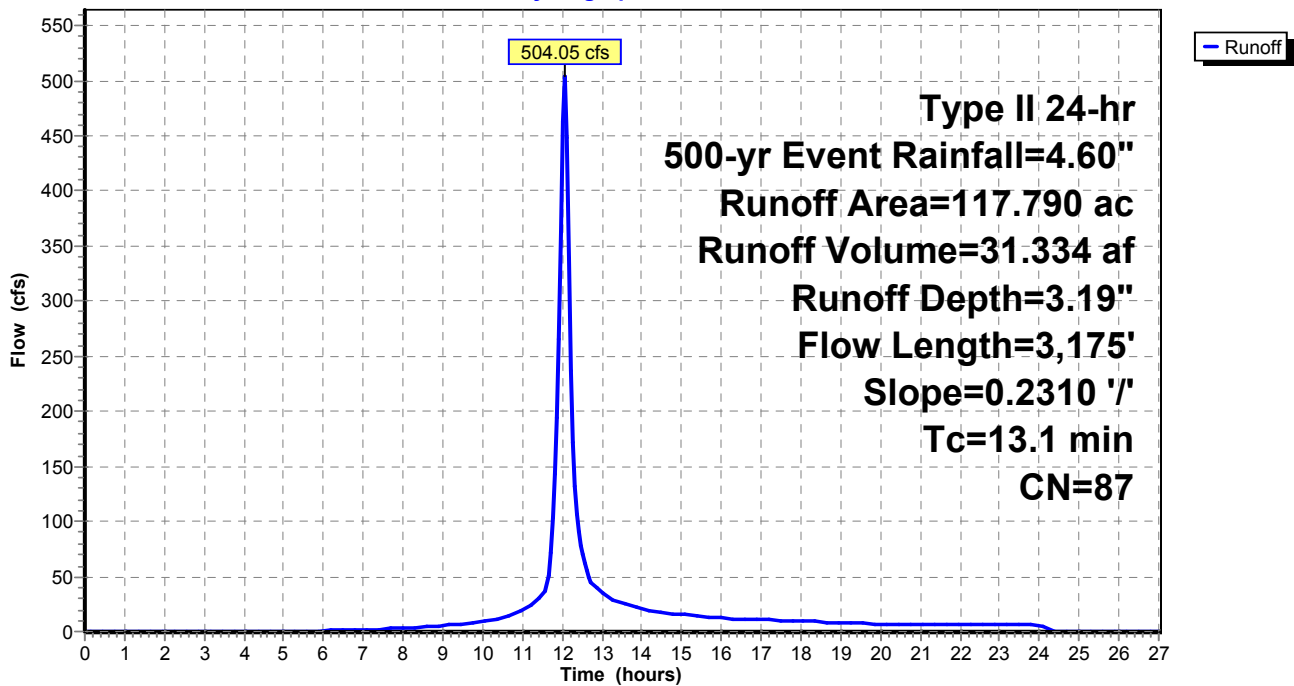
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 3: WS 3

Runoff = 371.87 cfs @ 12.10 hrs, Volume= 26.859 af, Depth= 3.19"

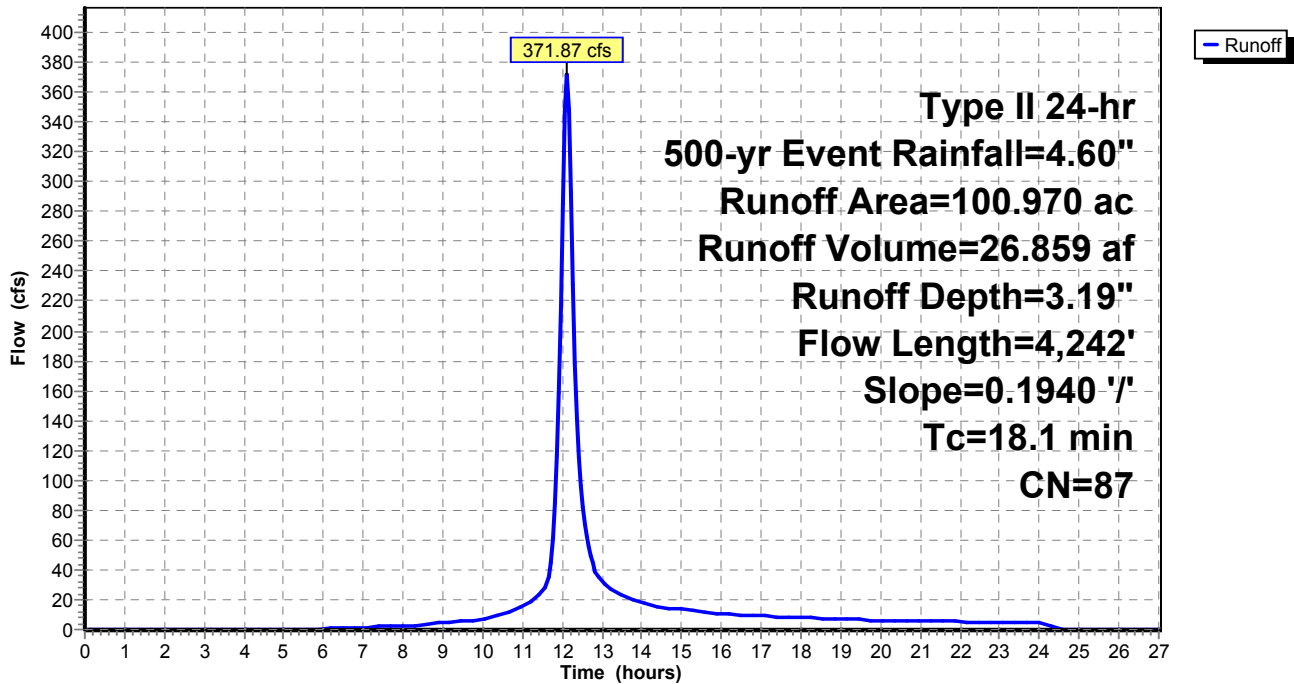
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 4: WS 4

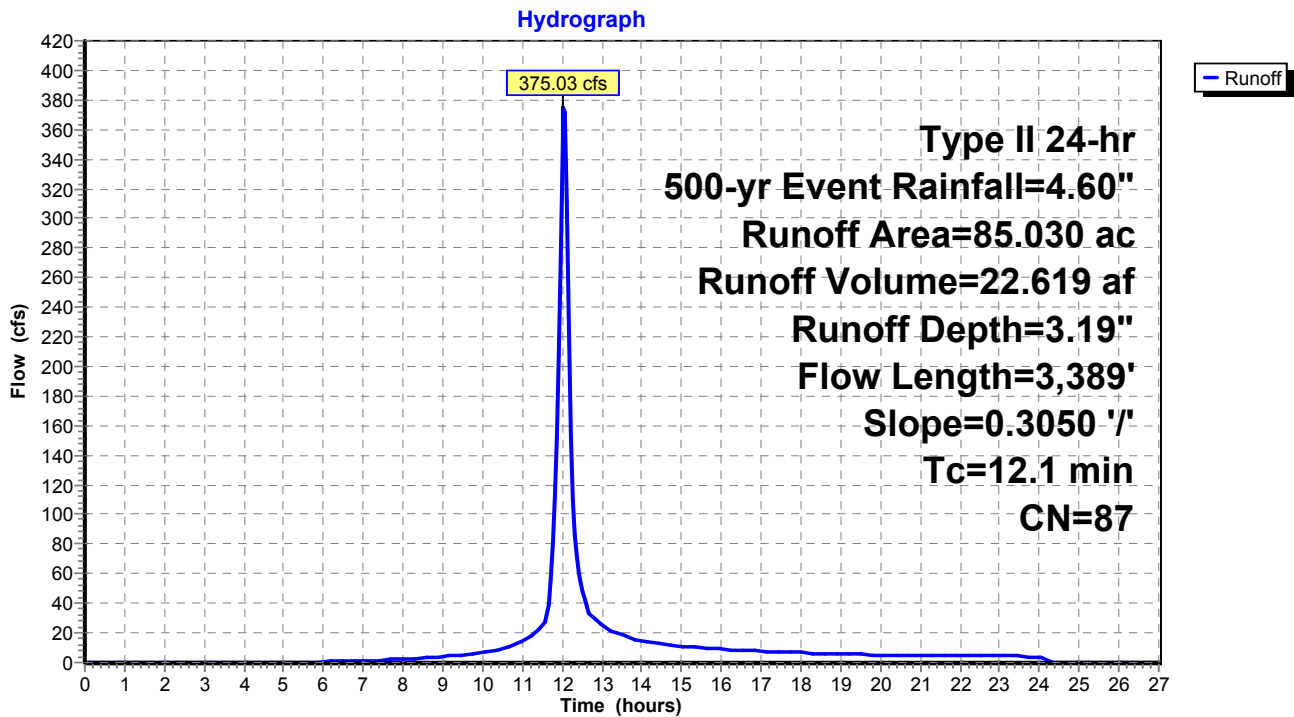
Runoff = 375.03 cfs @ 12.04 hrs, Volume= 22.619 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

Subcatchment 4: WS 4



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 5: WS 5

Runoff = 459.42 cfs @ 12.05 hrs, Volume= 28.559 af, Depth= 3.19"

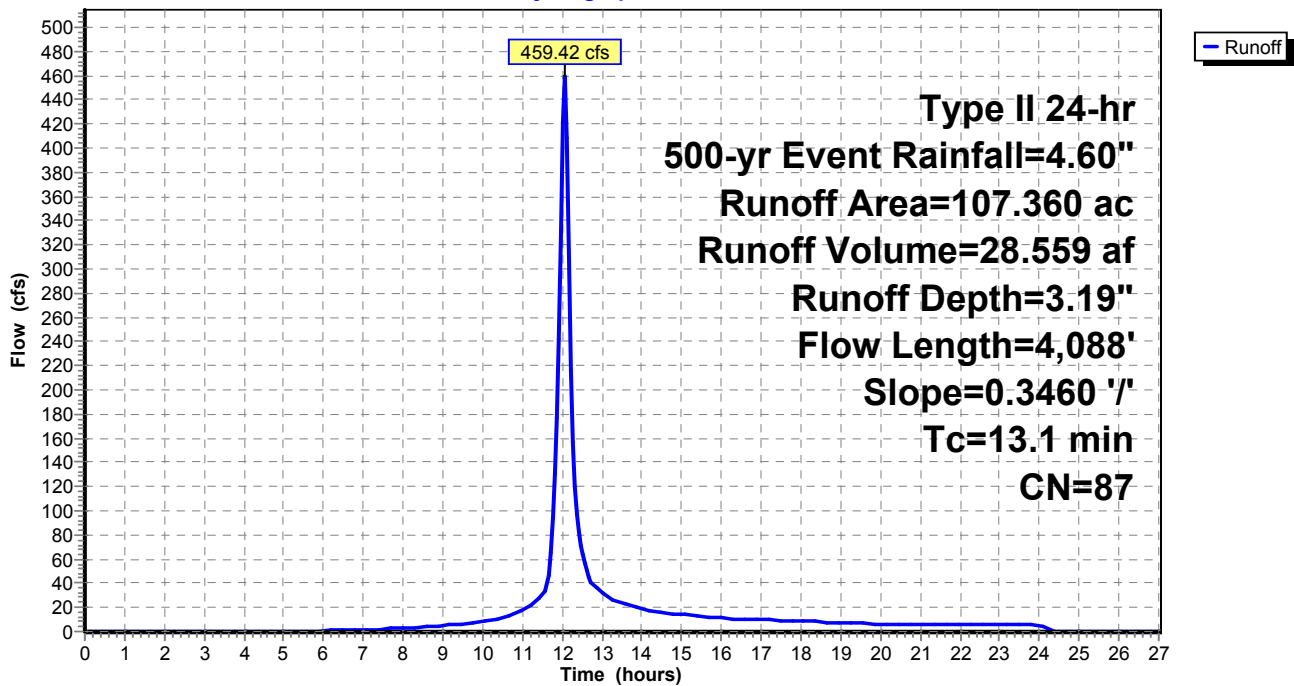
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 6: WS 6

Runoff = 1,060.97 cfs @ 12.22 hrs, Volume= 100.431 af, Depth= 3.19"

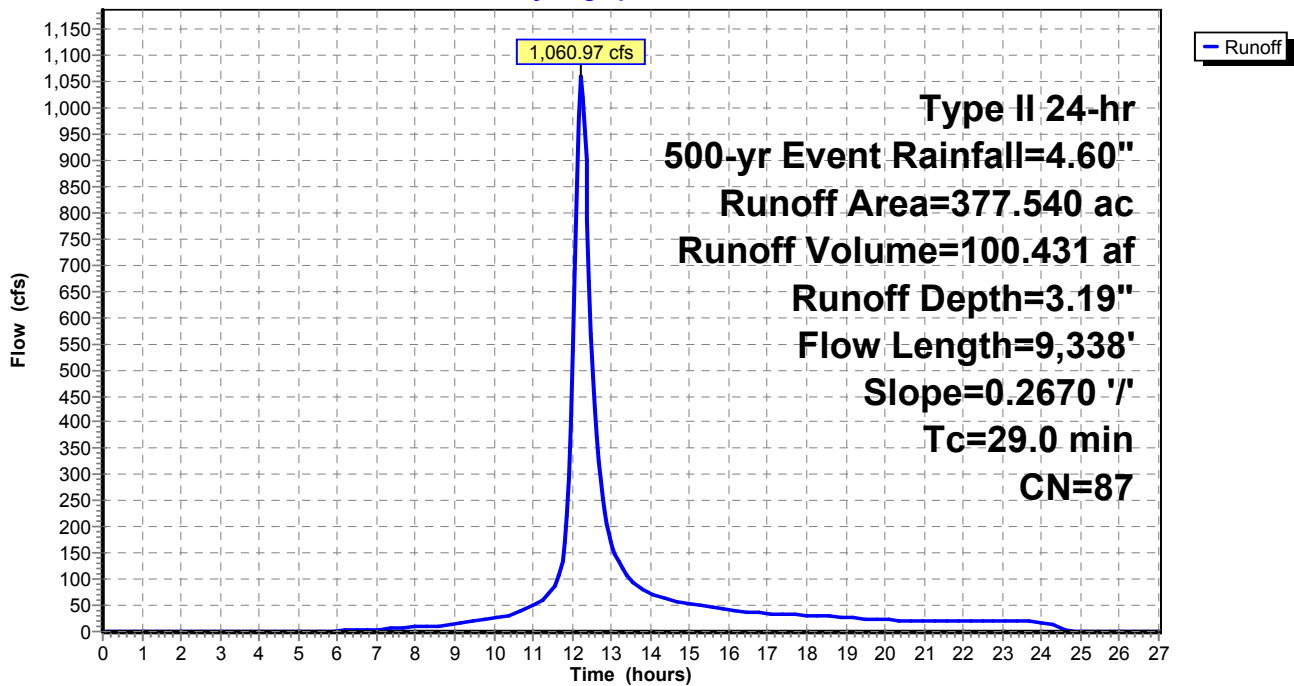
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 7: WS 7

Runoff = 542.79 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

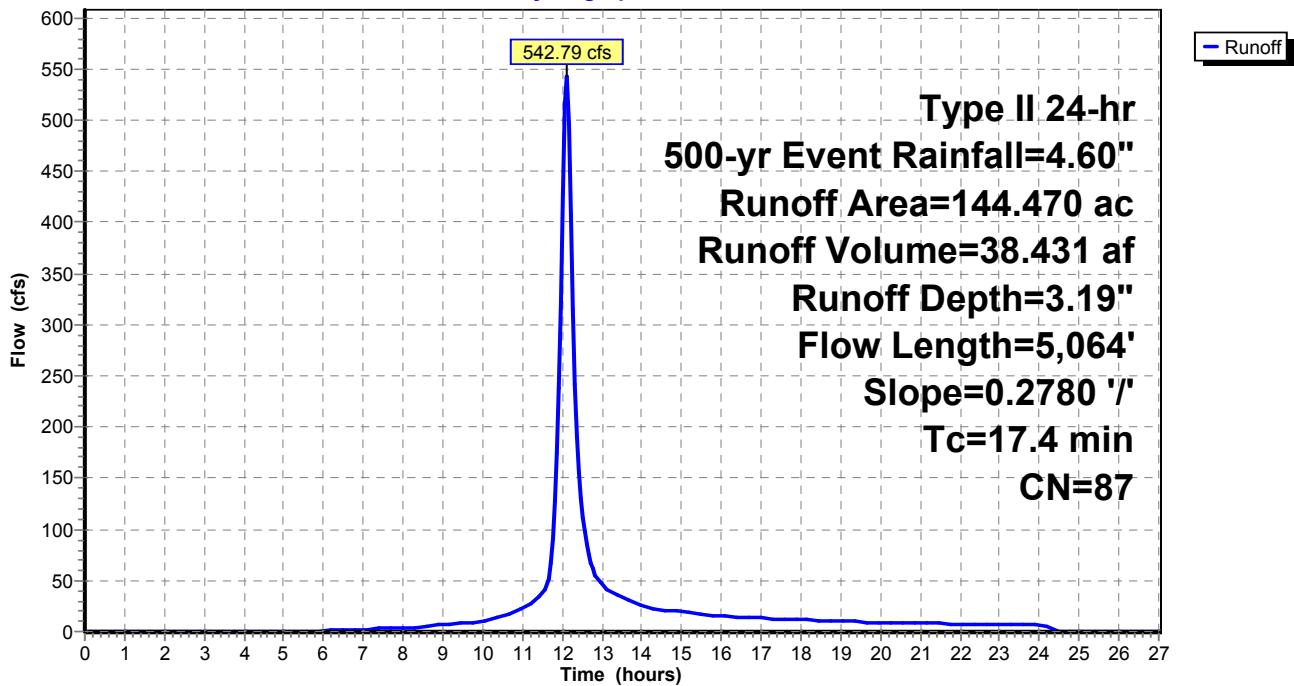
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 8: WS 8

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

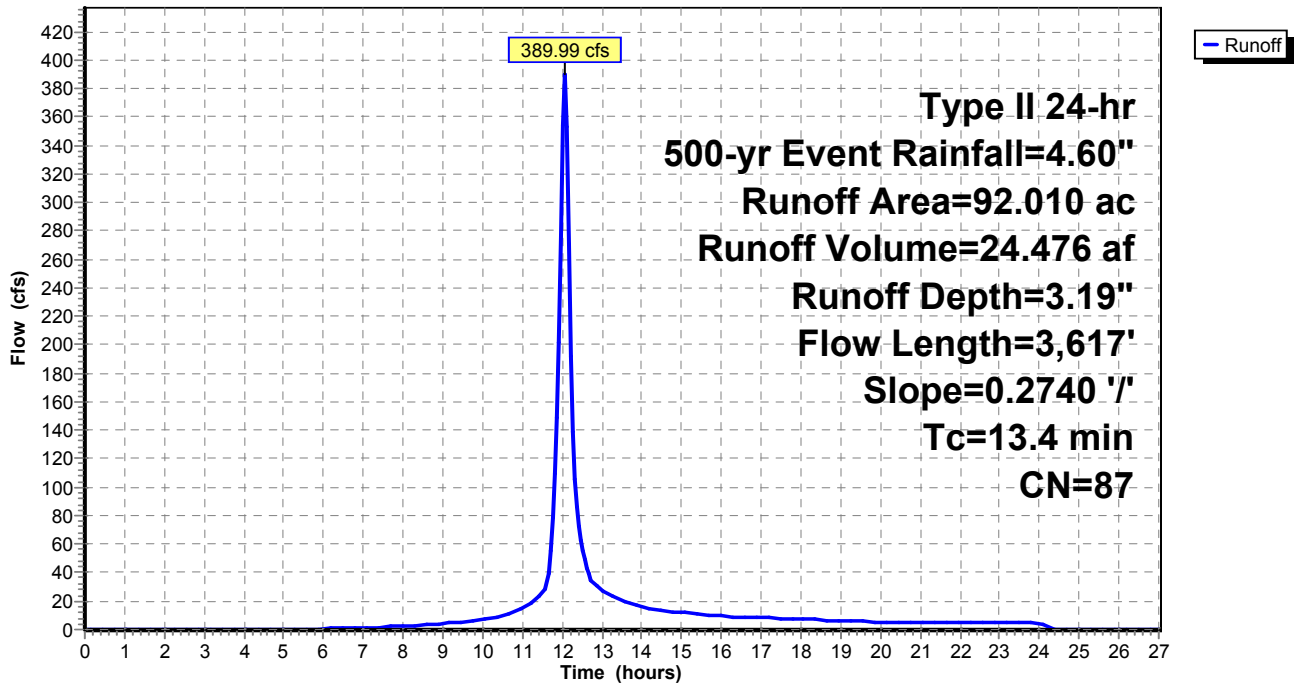
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 9: WS 9

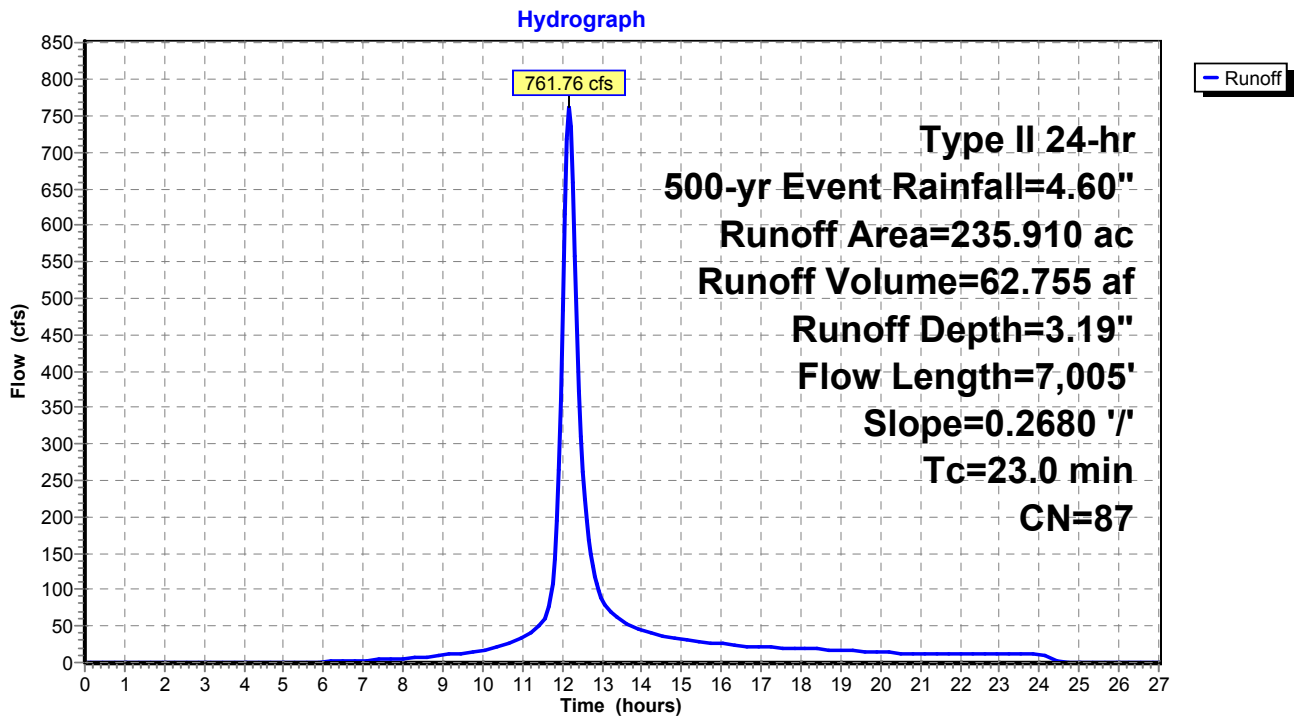
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 10: WS 10

Runoff = 887.32 cfs @ 12.26 hrs, Volume= 88.748 af, Depth= 3.19"

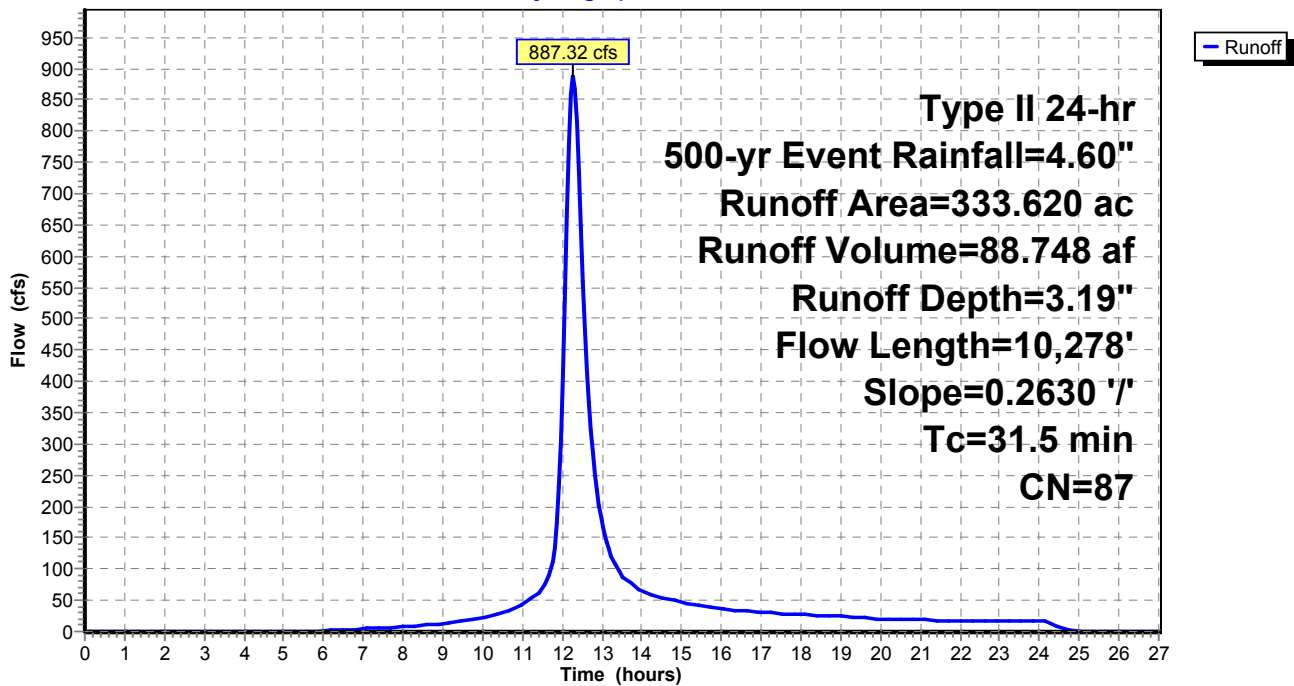
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 11: WS 11

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

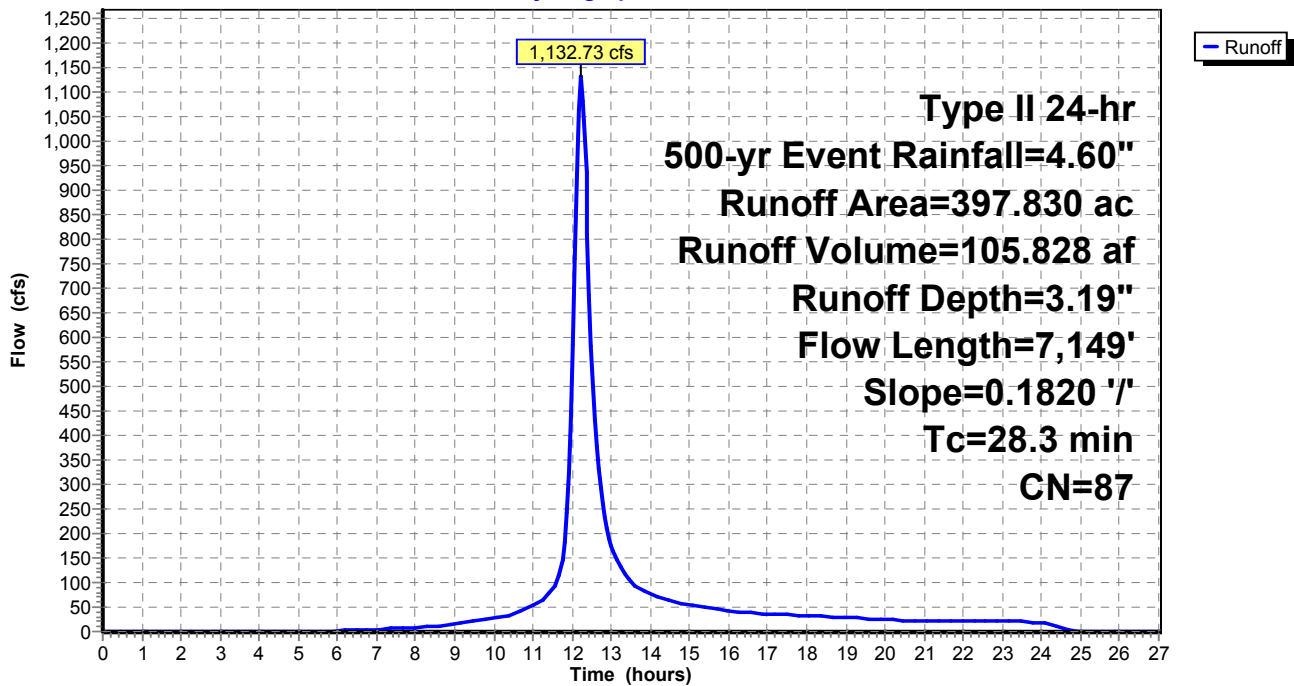
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 12: WS 12

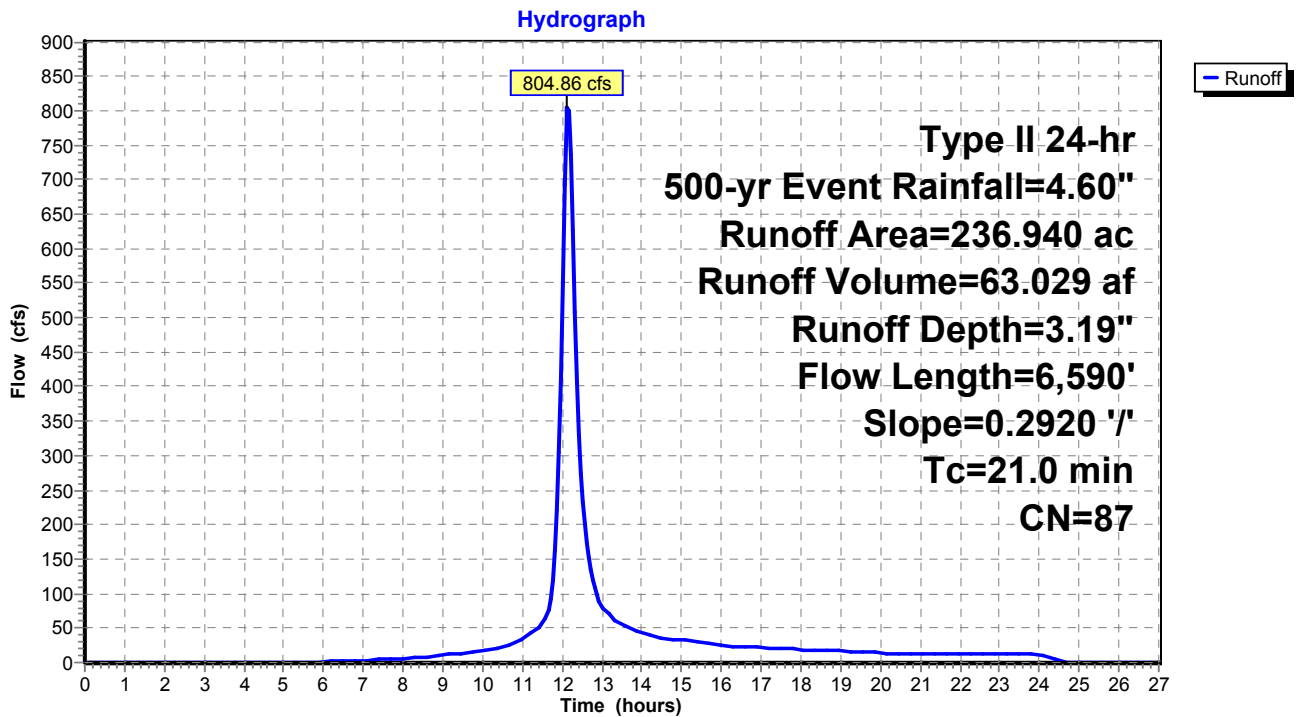
Runoff = 804.86 cfs @ 12.13 hrs, Volume= 63.029 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

Subcatchment 12: WS 12



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 13: WS 13

Runoff = 861.28 cfs @ 12.18 hrs, Volume= 75.138 af, Depth= 3.19"

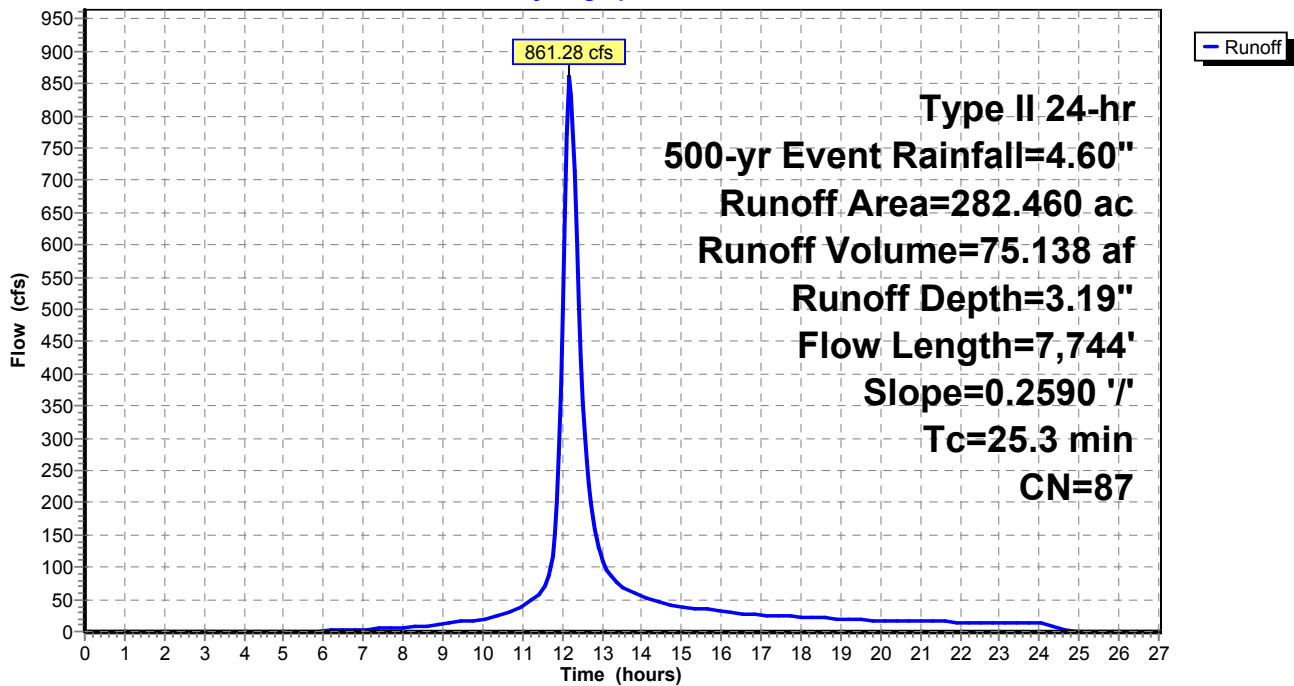
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 14: WS 14

Runoff = 498.15 cfs @ 12.18 hrs, Volume= 43.084 af, Depth= 3.19"

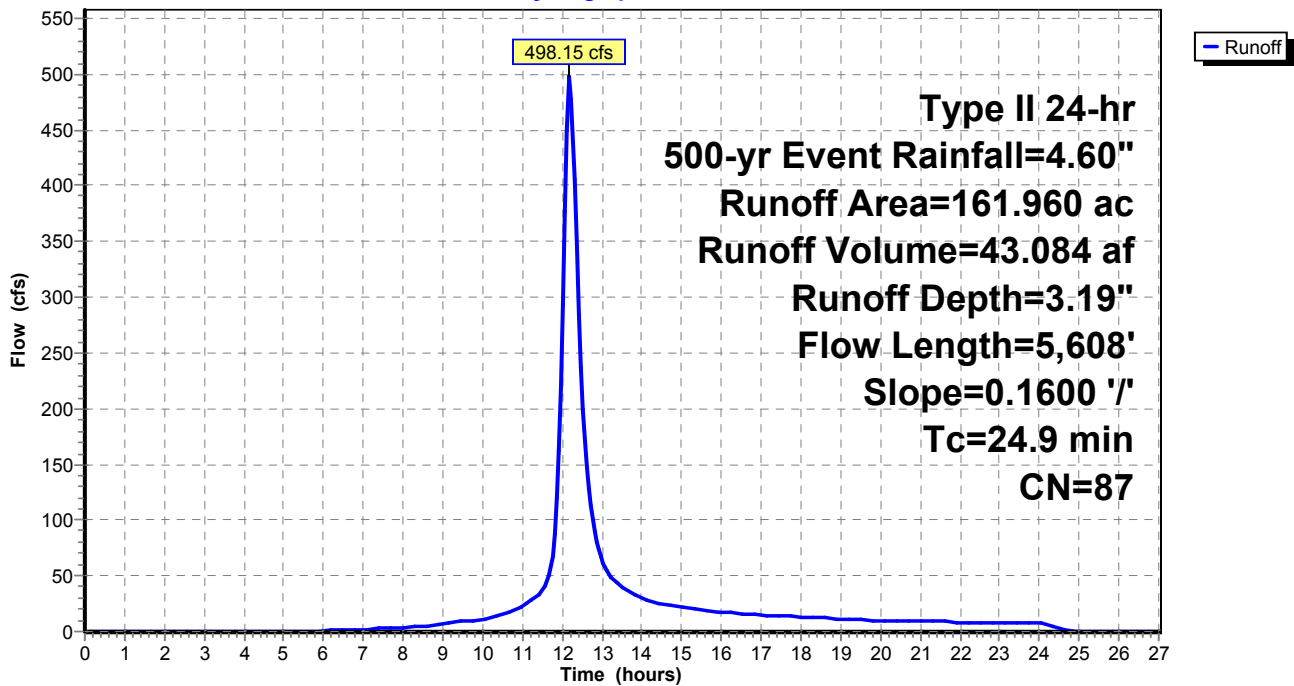
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 15: WS 15

Runoff = 240.16 cfs @ 12.04 hrs, Volume= 14.660 af, Depth= 3.19"

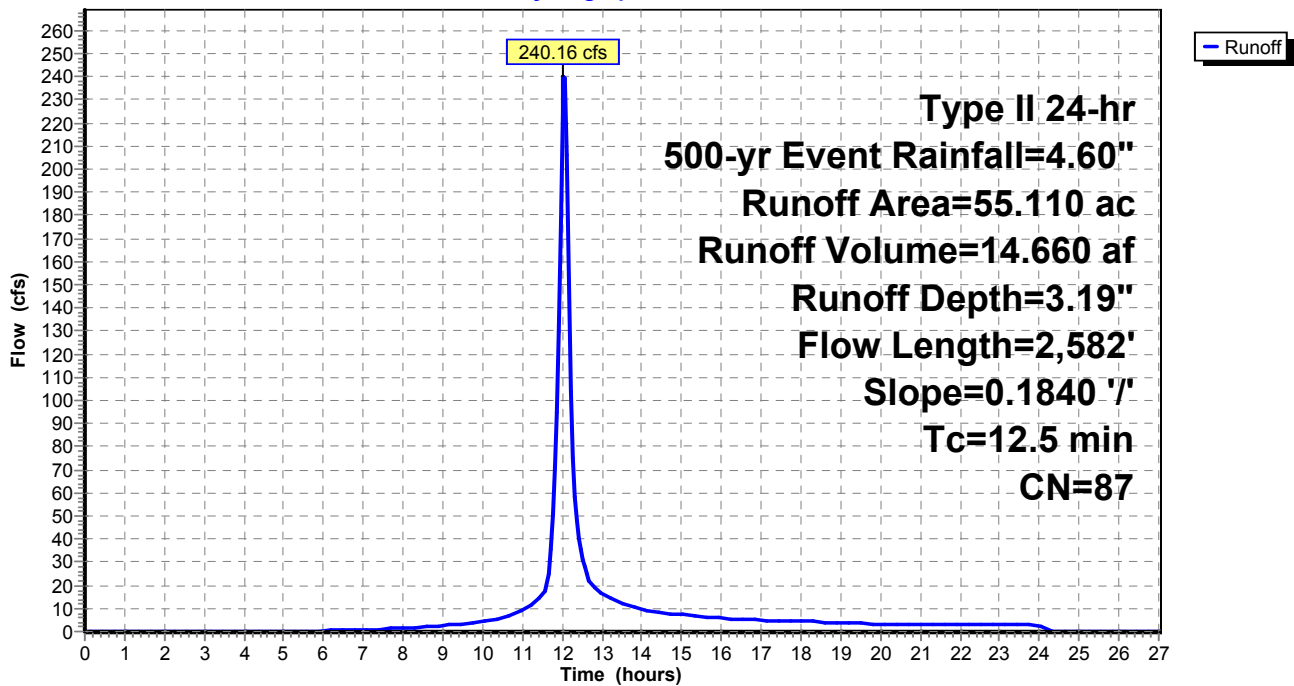
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

Subcatchment 15: WS 15

Hydrograph



Existing Watersheds (Pre-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 16: WS 16

Runoff = 274.73 cfs @ 12.15 hrs, Volume= 22.116 af, Depth= 3.19"

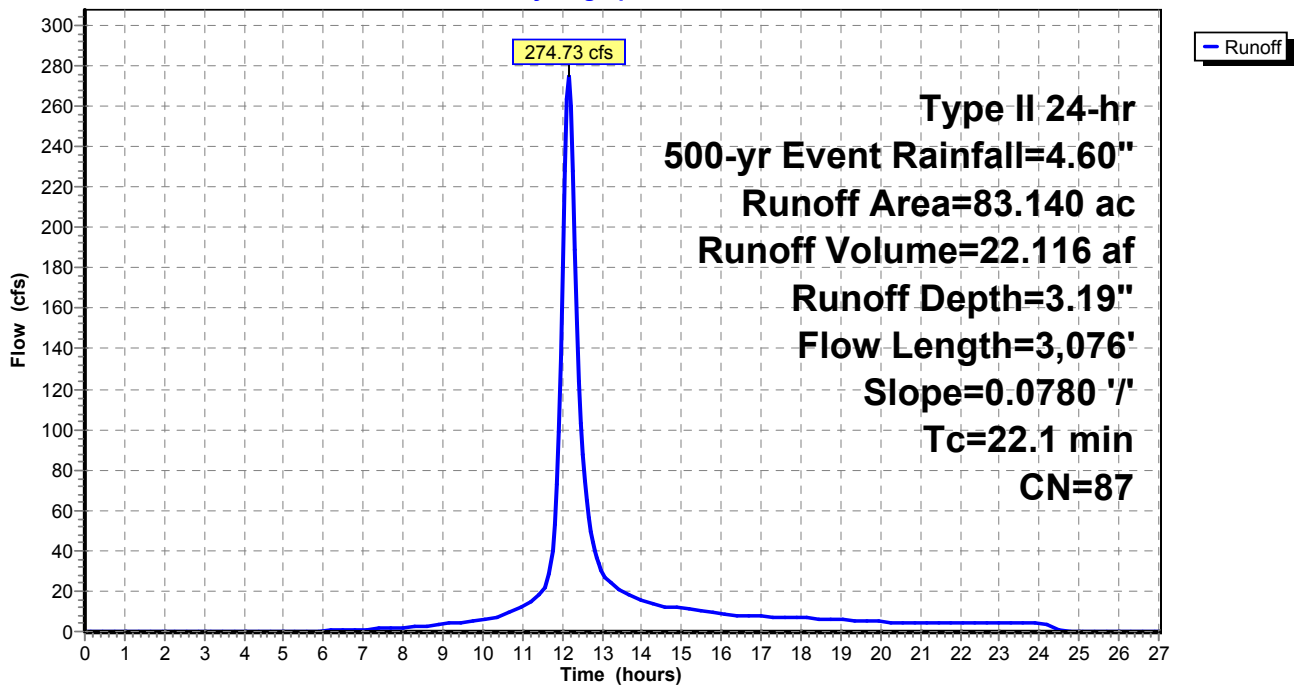
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

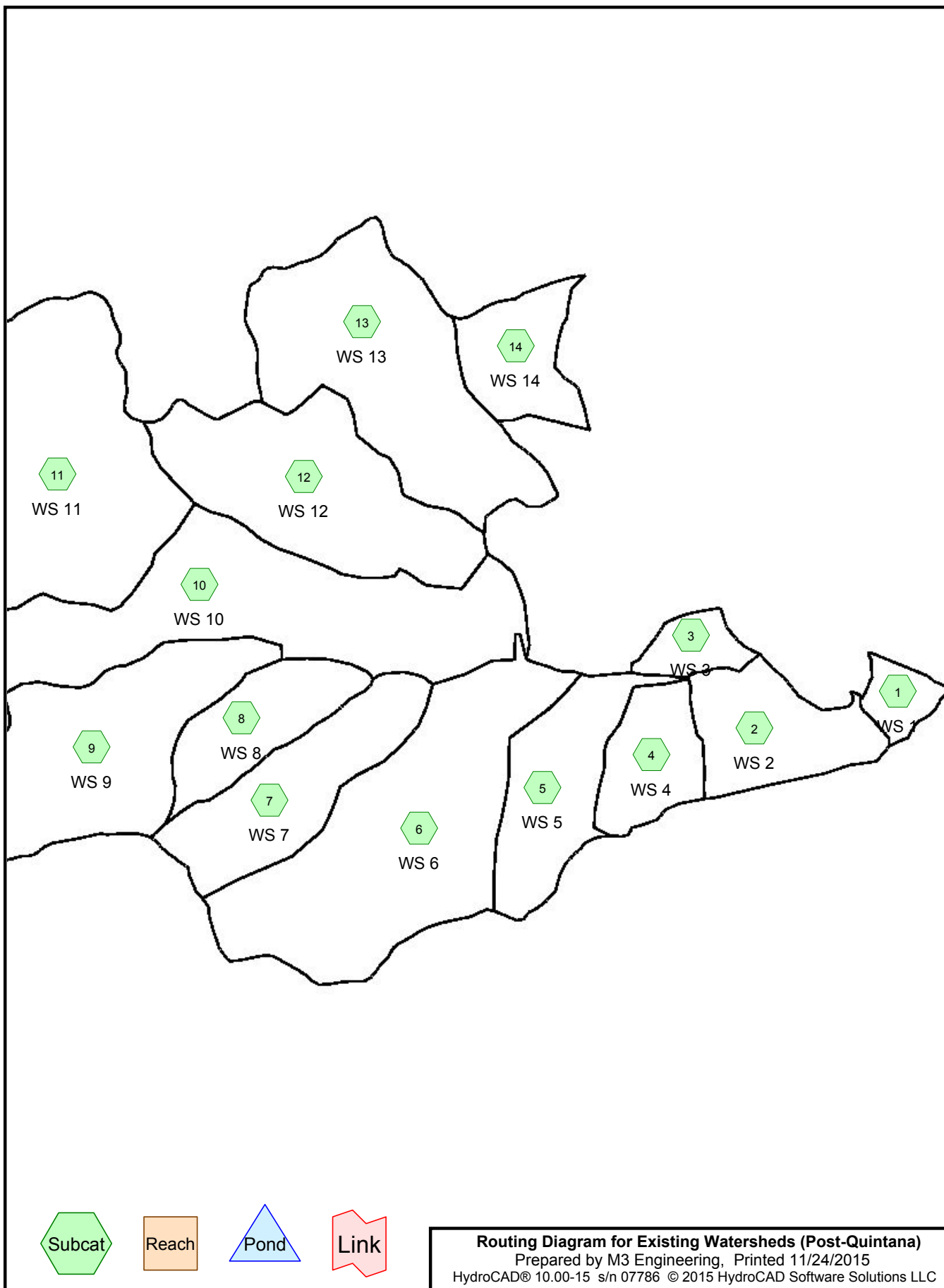
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

Subcatchment 16: WS 16

Hydrograph



APPENDIX C



Existing Watersheds (Post-Quintana)

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2,235.360	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14)
248.370	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14)
2,483.730	87	TOTAL AREA

Existing Watersheds (Post-Quintana)

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2,483.730	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
0.000	Other	
2,483.730		TOTAL AREA

Existing Watersheds (Post-Quintana)

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,235.360	0.000	2,235.360	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
0.000	0.000	0.000	248.370	0.000	248.370	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
0.000	0.000	0.000	2,483.730	0.000	2,483.730	TOTAL AREA	

Existing Watersheds (Post-Quintana)*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.36" Tc=5.0 min CN=87 Runoff=117.20 cfs 5.555 af
Subcatchment2: WS 2	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=363.66 cfs 20.985 af
Subcatchment3: WS 3	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=129.54 cfs 6.890 af
Subcatchment4: WS 4	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=259.92 cfs 14.773 af
Subcatchment5: WS 5	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=342.49 cfs 24.455 af
Subcatchment6: WS 6	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=774.13 cfs 65.223 af
Subcatchment7: WS 7	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=411.06 cfs 28.449 af
Subcatchment8: WS 8	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=291.83 cfs 18.118 af
Subcatchment9: WS 9	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=567.91 cfs 46.455 af
Subcatchment10: WS 10	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=663.73 cfs 65.063 af
Subcatchment11: WS 11	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=843.39 cfs 78.339 af
Subcatchment12: WS 12	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=588.30 cfs 44.783 af
Subcatchment13: WS 13	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=649.87 cfs 54.252 af
Subcatchment14: WS 14	Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=243.90 cfs 15.747 af

Total Runoff Area = 2,483.730 ac Runoff Volume = 489.087 af Average Runoff Depth = 2.36"
90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac

Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 117.20 cfs @ 11.95 hrs, Volume= 5.555 af, Depth= 2.36"

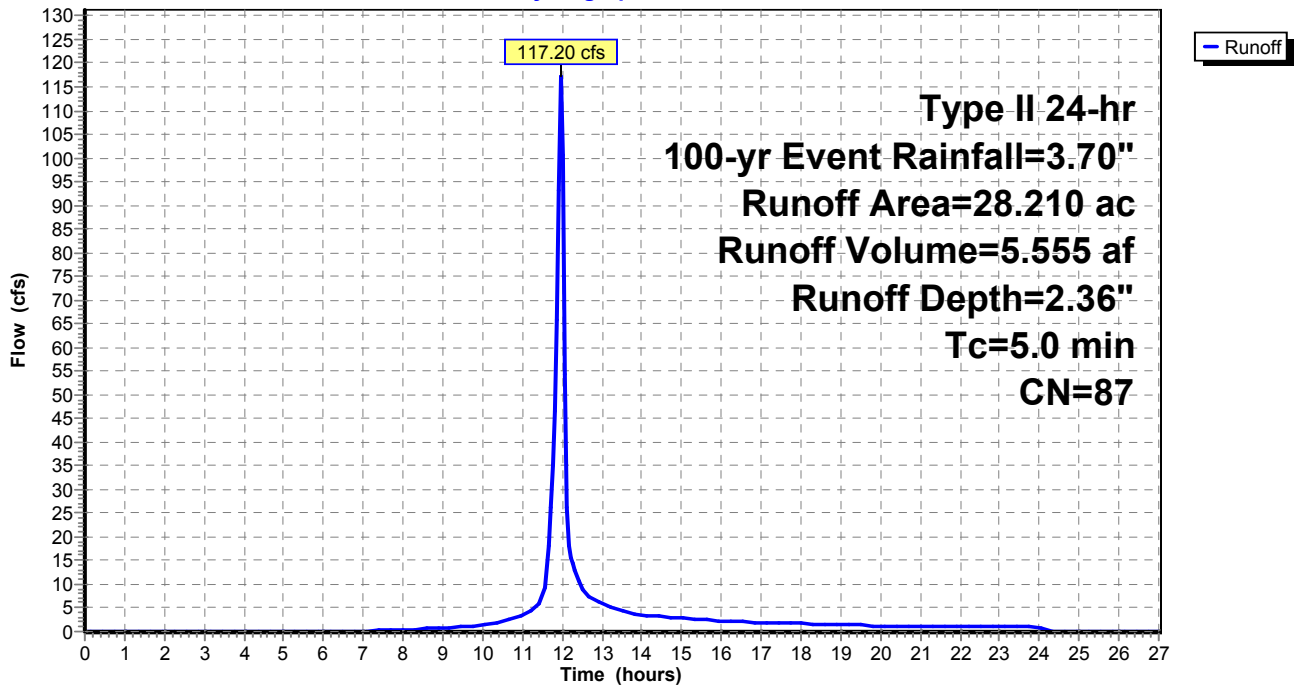
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 2: WS 2

Runoff = 363.66 cfs @ 12.03 hrs, Volume= 20.985 af, Depth= 2.36"

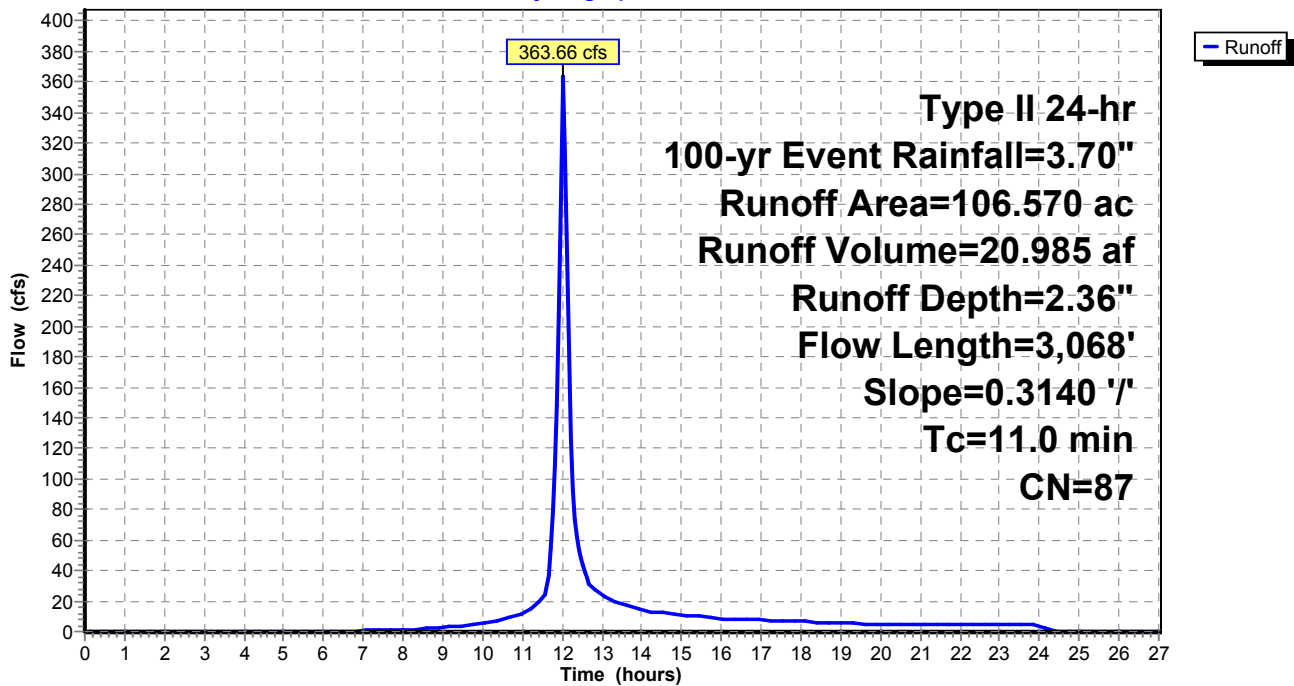
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 3: WS 3

Runoff = 129.54 cfs @ 12.00 hrs, Volume= 6.890 af, Depth= 2.36"

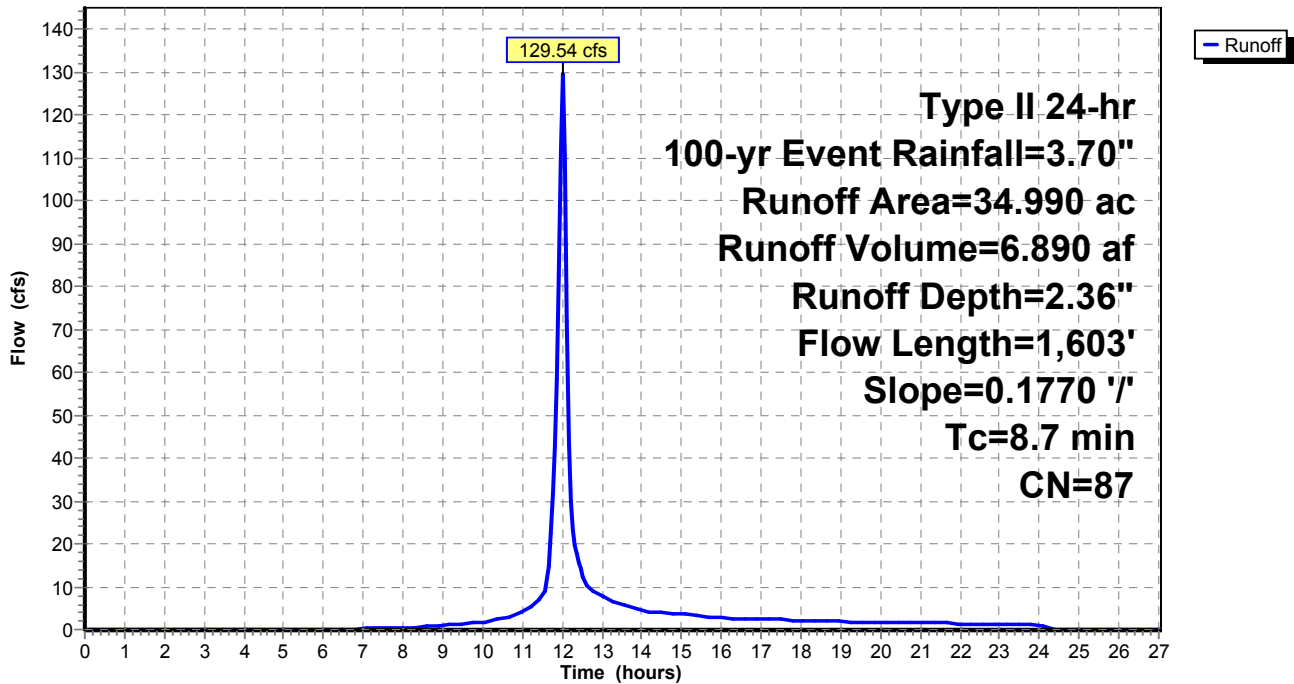
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 4: WS 4

Runoff = 259.92 cfs @ 12.02 hrs, Volume= 14.773 af, Depth= 2.36"

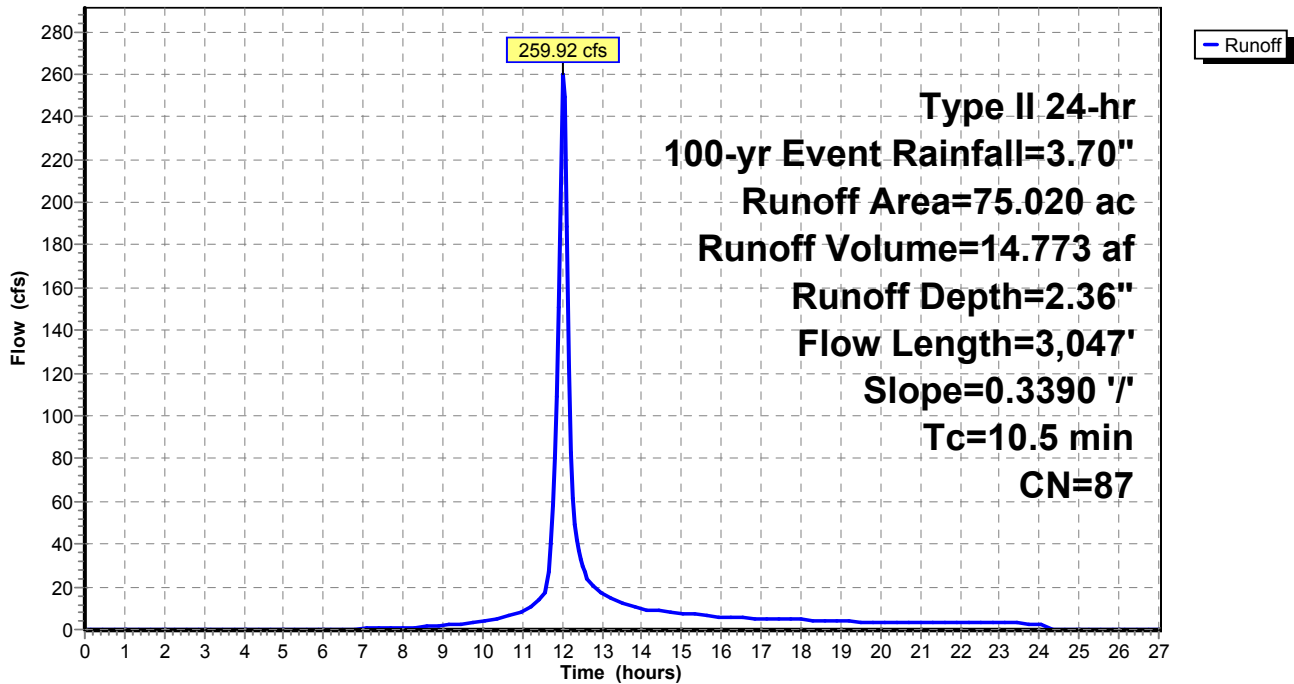
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 5: WS 5

Runoff = 342.49 cfs @ 12.10 hrs, Volume= 24.455 af, Depth= 2.36"

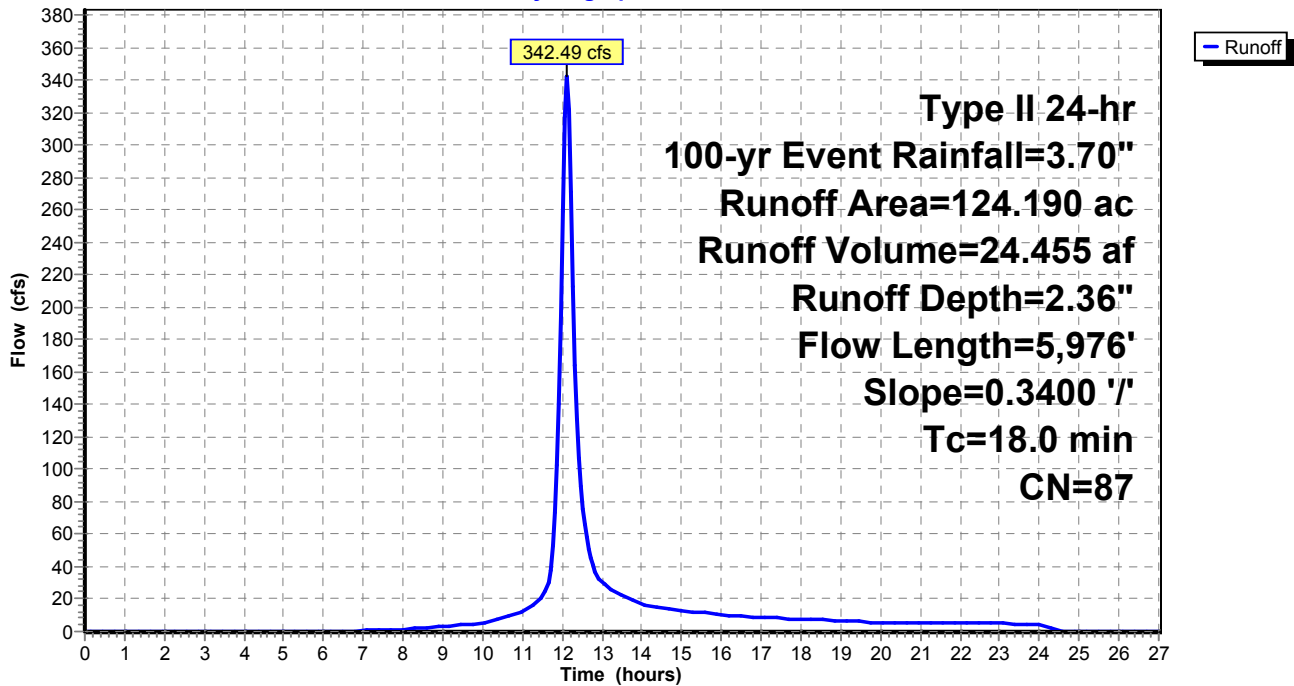
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 6: WS 6

Runoff = 774.13 cfs @ 12.17 hrs, Volume= 65.223 af, Depth= 2.36"

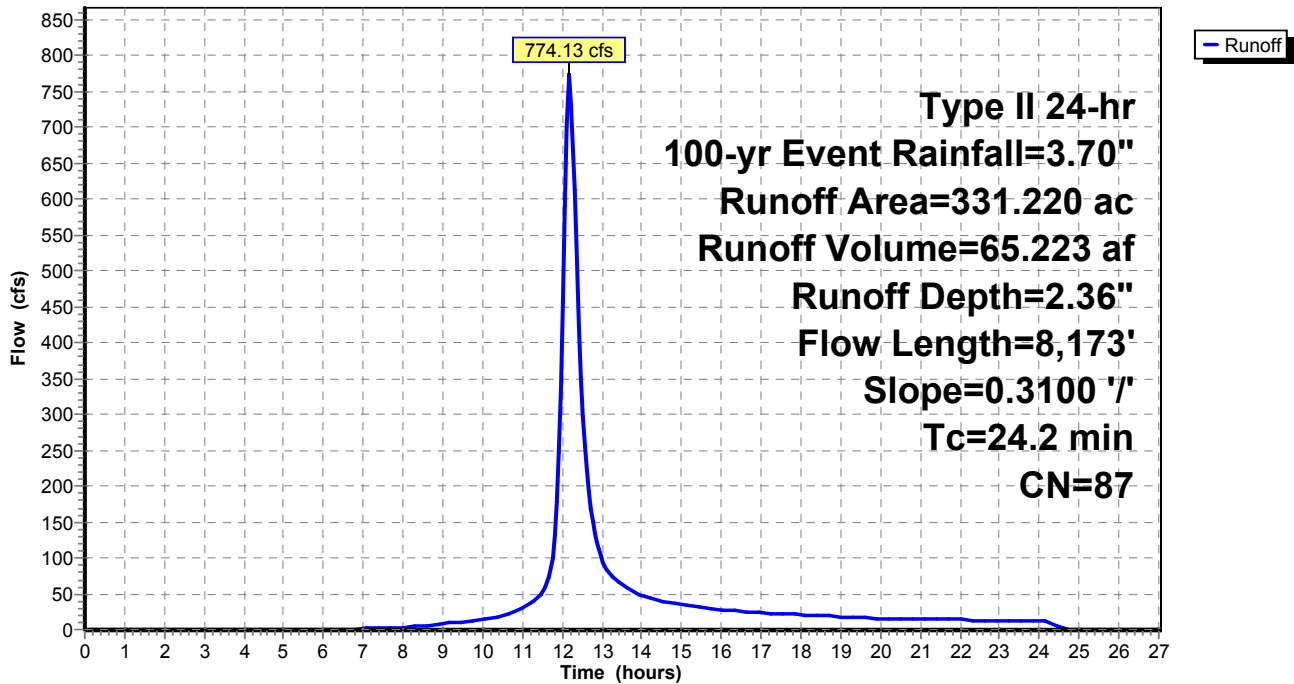
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 7: WS 7

Runoff = 411.06 cfs @ 12.09 hrs, Volume= 28.449 af, Depth= 2.36"

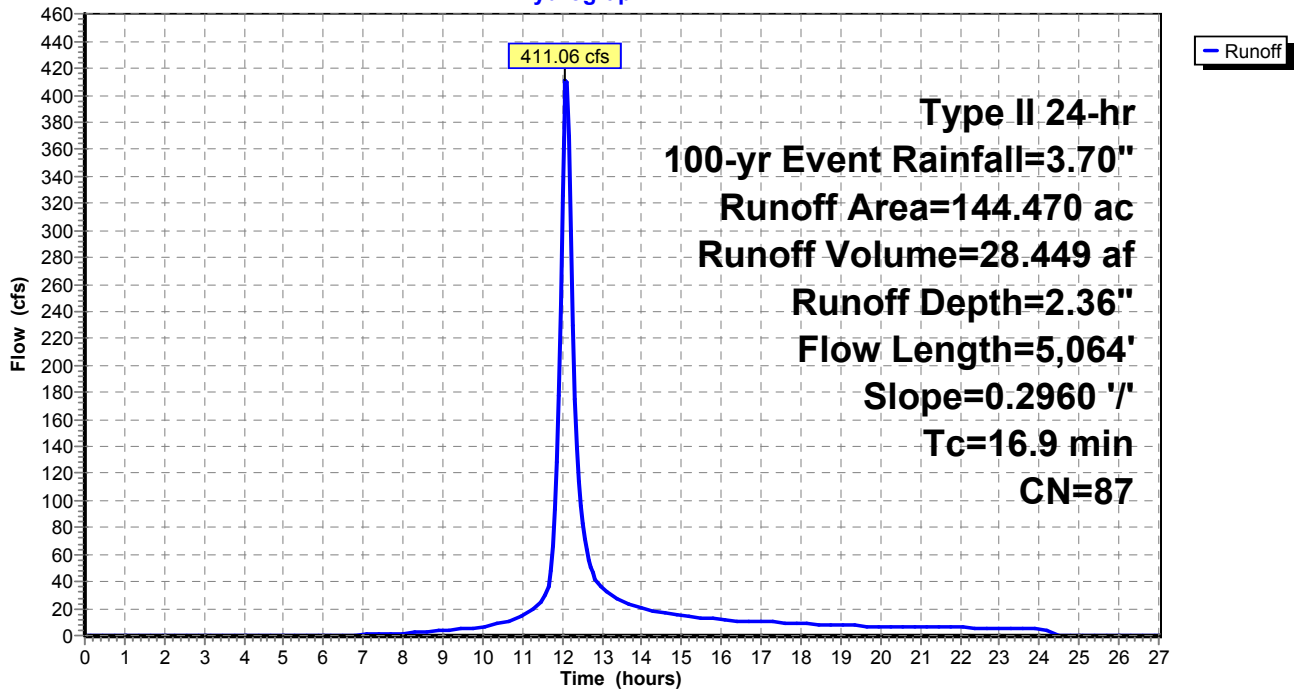
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 8: WS 8

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

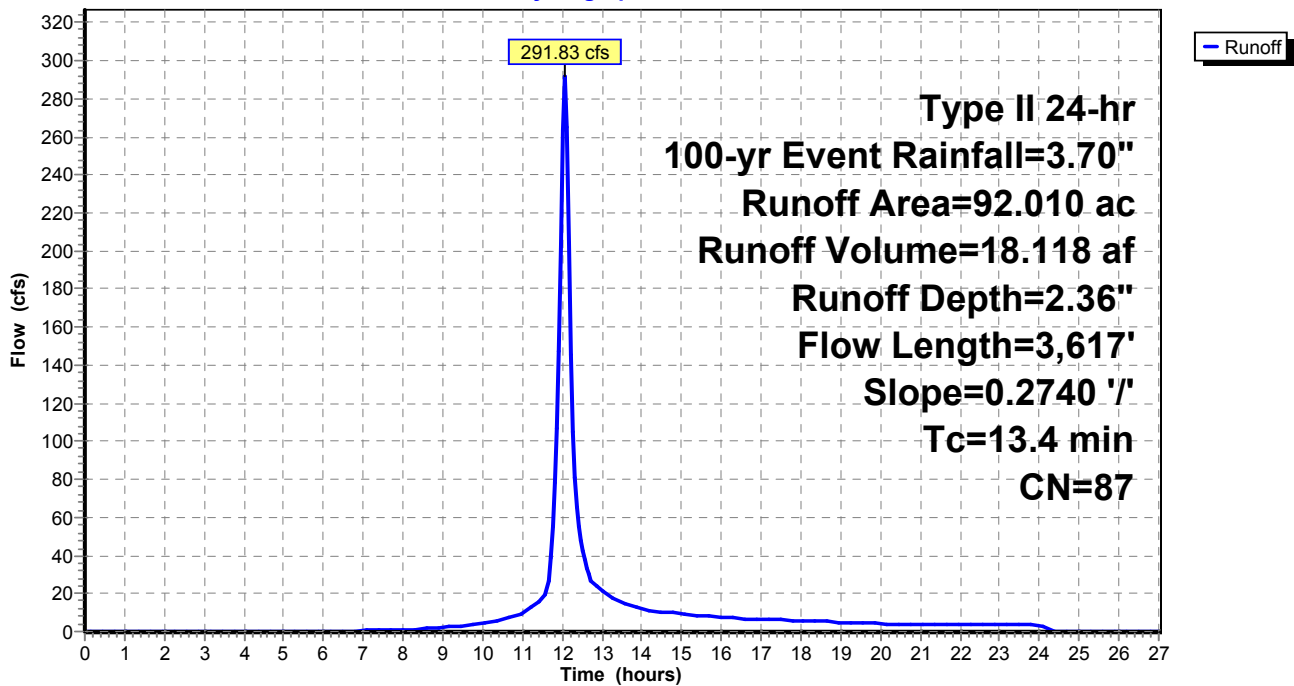
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 9: WS 9

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

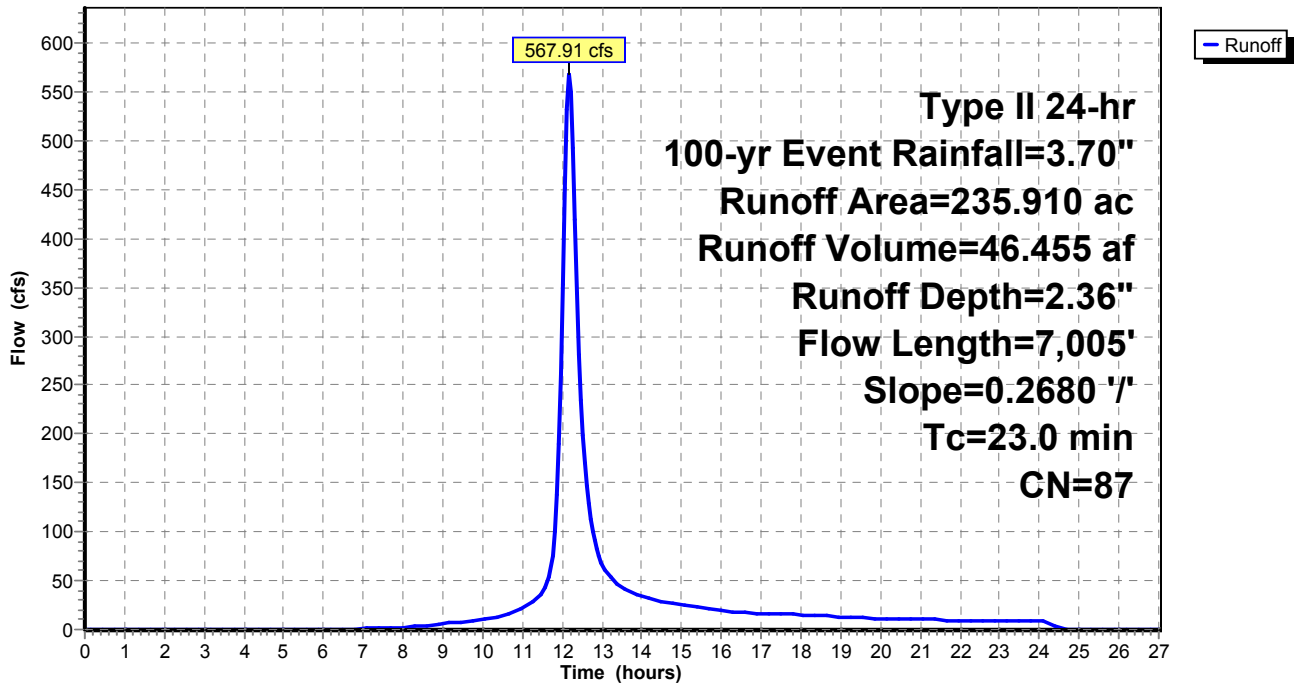
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 10: WS 10

Runoff = 663.73 cfs @ 12.25 hrs, Volume= 65.063 af, Depth= 2.36"

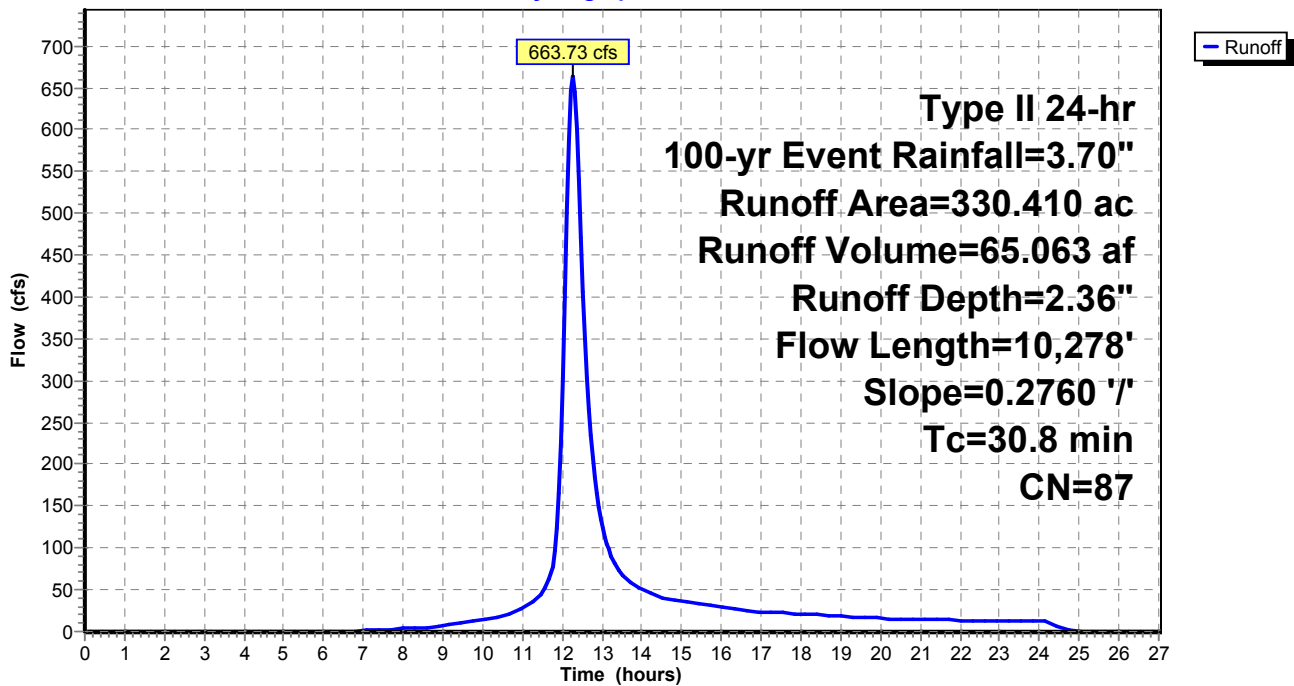
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 11: WS 11

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

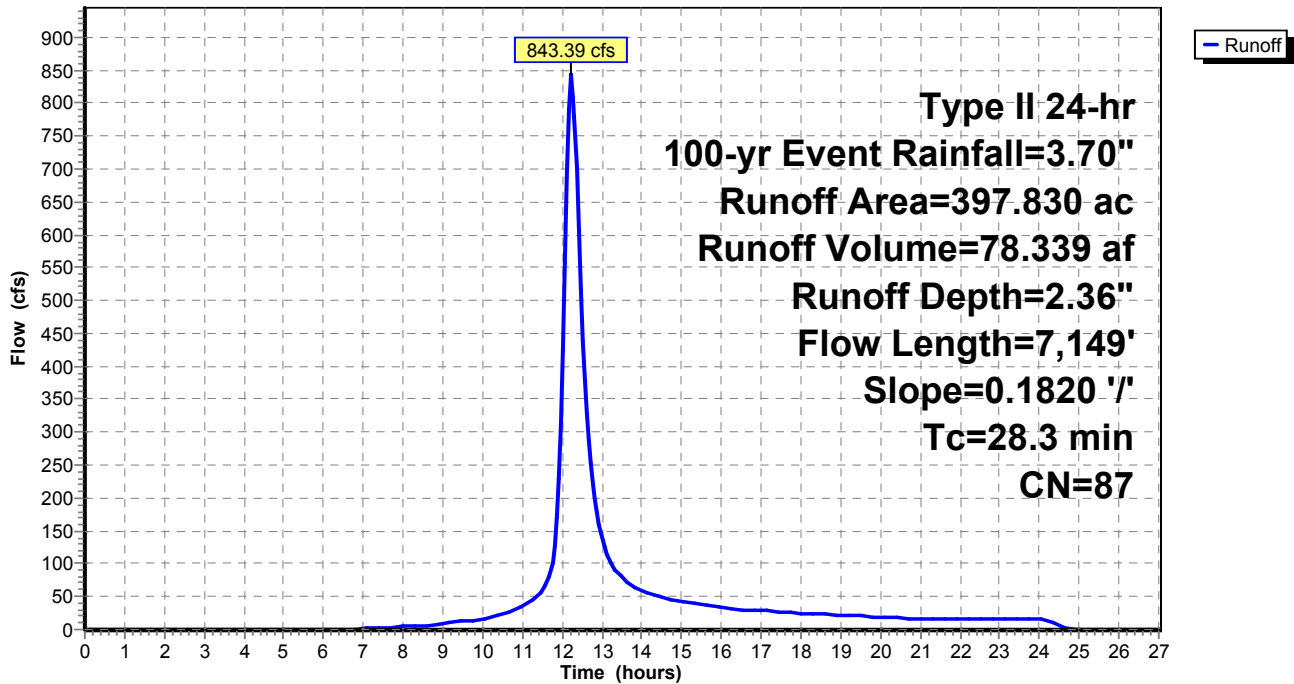
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 12: WS 12

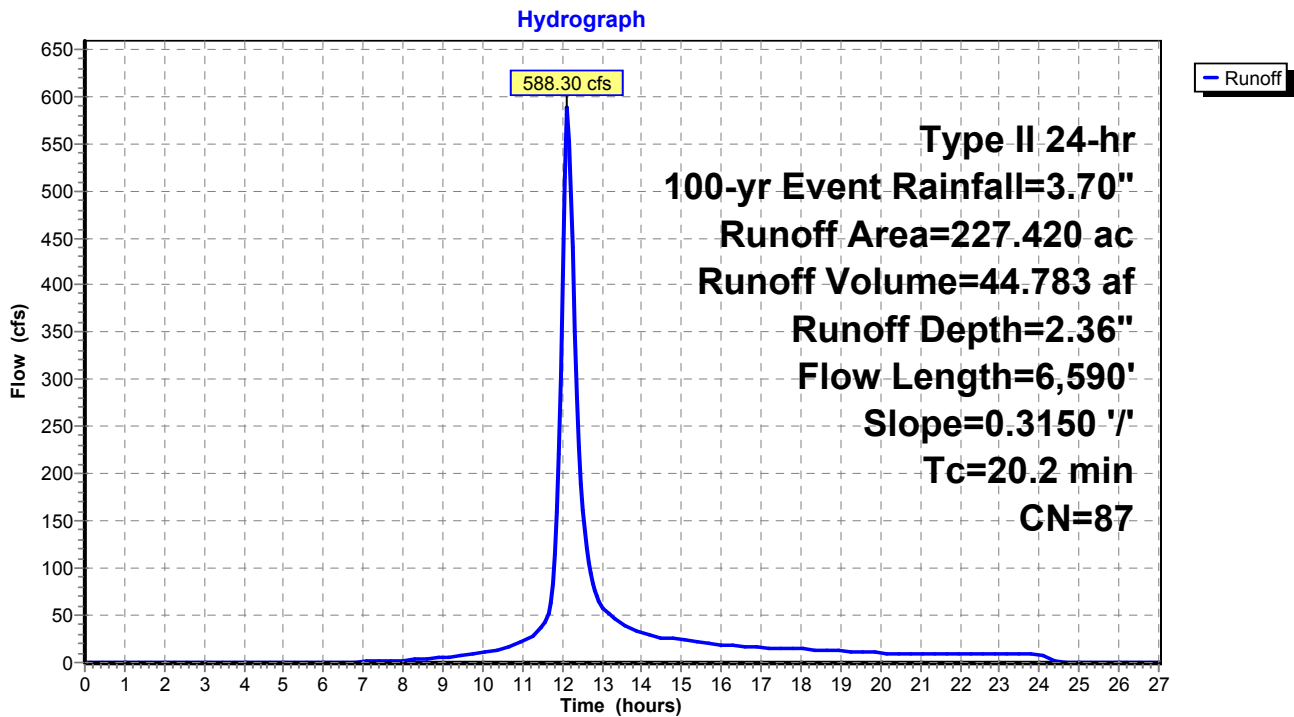
Runoff = 588.30 cfs @ 12.13 hrs, Volume= 44.783 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 13: WS 13

Runoff = 649.87 cfs @ 12.17 hrs, Volume= 54.252 af, Depth= 2.36"

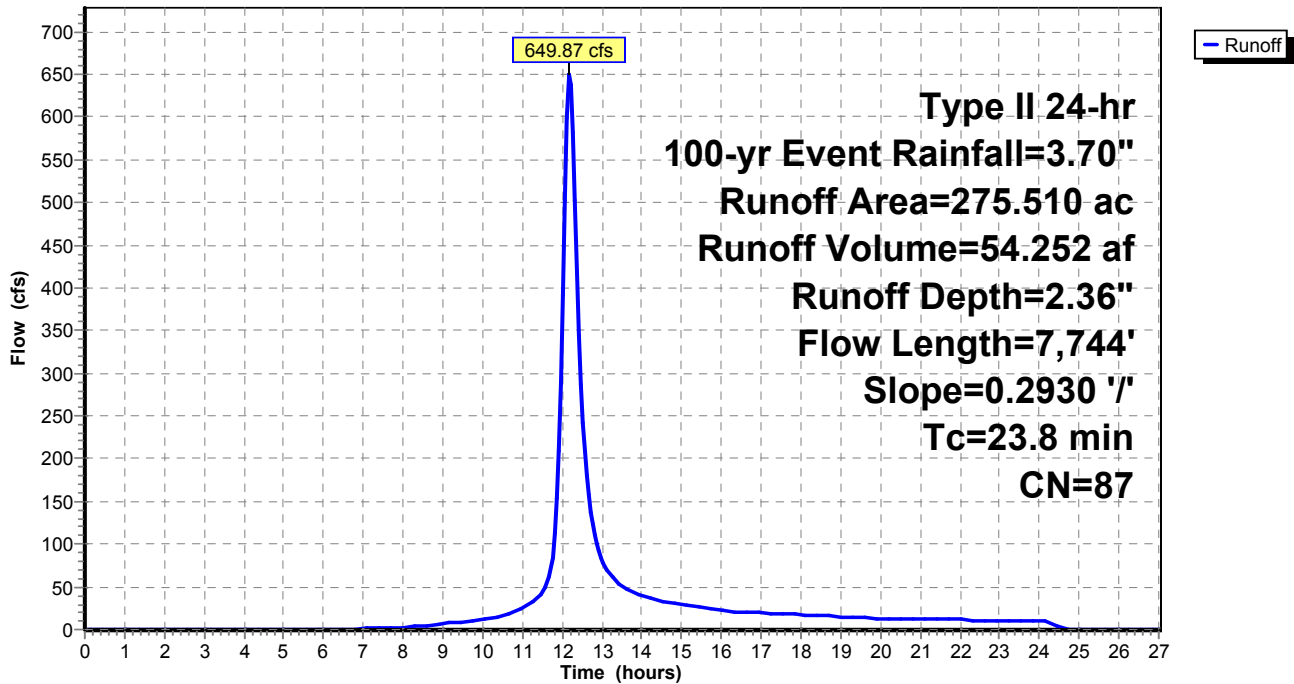
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 14: WS 14

Runoff = 243.90 cfs @ 12.06 hrs, Volume= 15.747 af, Depth= 2.36"

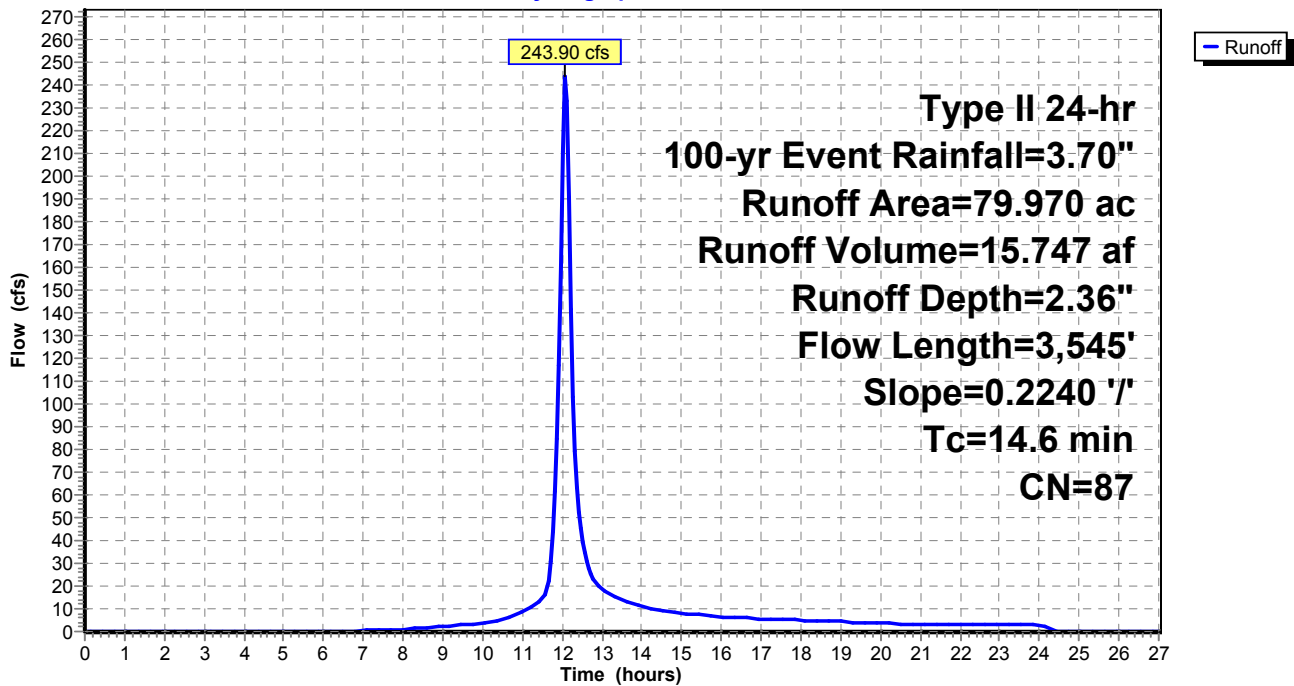
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



Existing Watersheds (Post-Quintana)*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.72" Tc=5.0 min CN=87 Runoff=133.97 cfs 6.393 af
Subcatchment2: WS 2	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=415.35 cfs 24.150 af
Subcatchment3: WS 3	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=148.20 cfs 7.929 af
Subcatchment4: WS 4	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=297.56 cfs 17.000 af
Subcatchment5: WS 5	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=392.70 cfs 28.143 af
Subcatchment6: WS 6	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=888.50 cfs 75.058 af
Subcatchment7: WS 7	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=471.20 cfs 32.739 af
Subcatchment8: WS 8	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=334.29 cfs 20.851 af
Subcatchment9: WS 9	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=651.70 cfs 53.460 af
Subcatchment10: WS 10	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=762.30 cfs 74.875 af
Subcatchment11: WS 11	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=968.42 cfs 90.153 af
Subcatchment12: WS 12	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=674.80 cfs 51.536 af
Subcatchment13: WS 13	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=745.85 cfs 62.434 af
Subcatchment14: WS 14	Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=279.49 cfs 18.122 af

Total Runoff Area = 2,483.730 ac Runoff Volume = 562.843 af Average Runoff Depth = 2.72"
90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac

Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

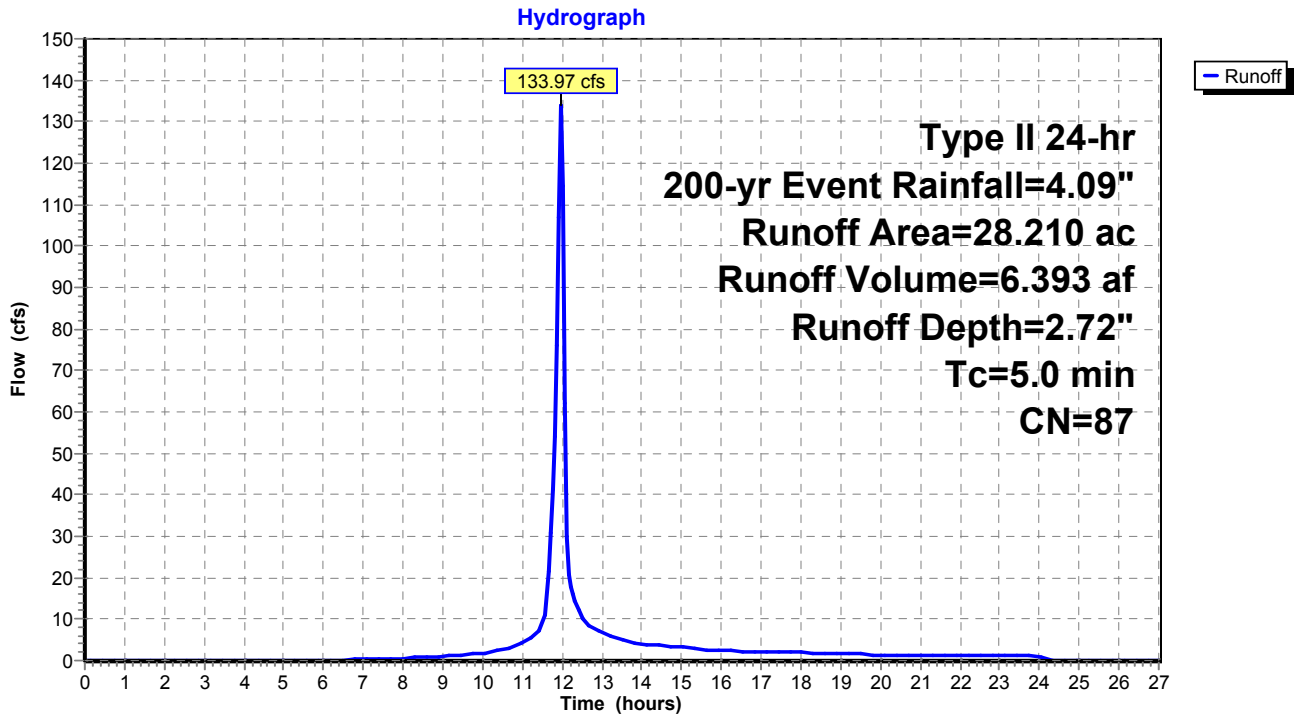
Runoff = 133.97 cfs @ 11.95 hrs, Volume= 6.393 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 2: WS 2

Runoff = 415.35 cfs @ 12.02 hrs, Volume= 24.150 af, Depth= 2.72"

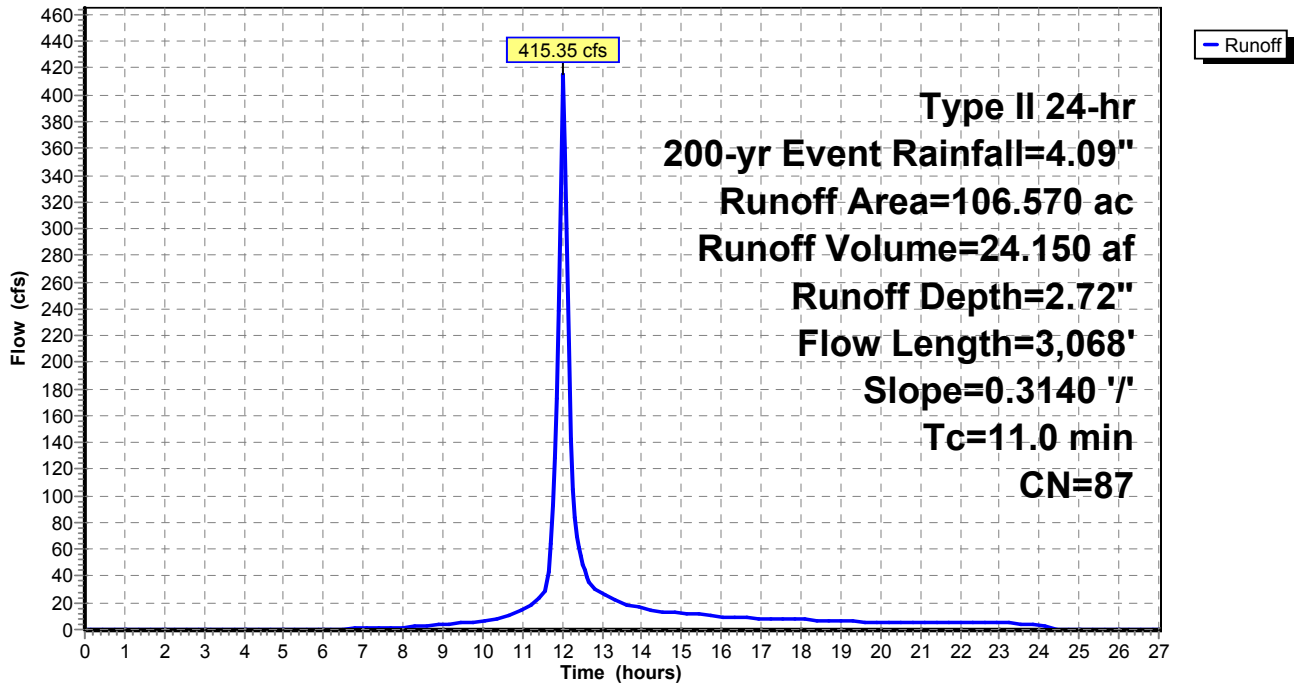
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 3: WS 3

Runoff = 148.20 cfs @ 12.00 hrs, Volume= 7.929 af, Depth= 2.72"

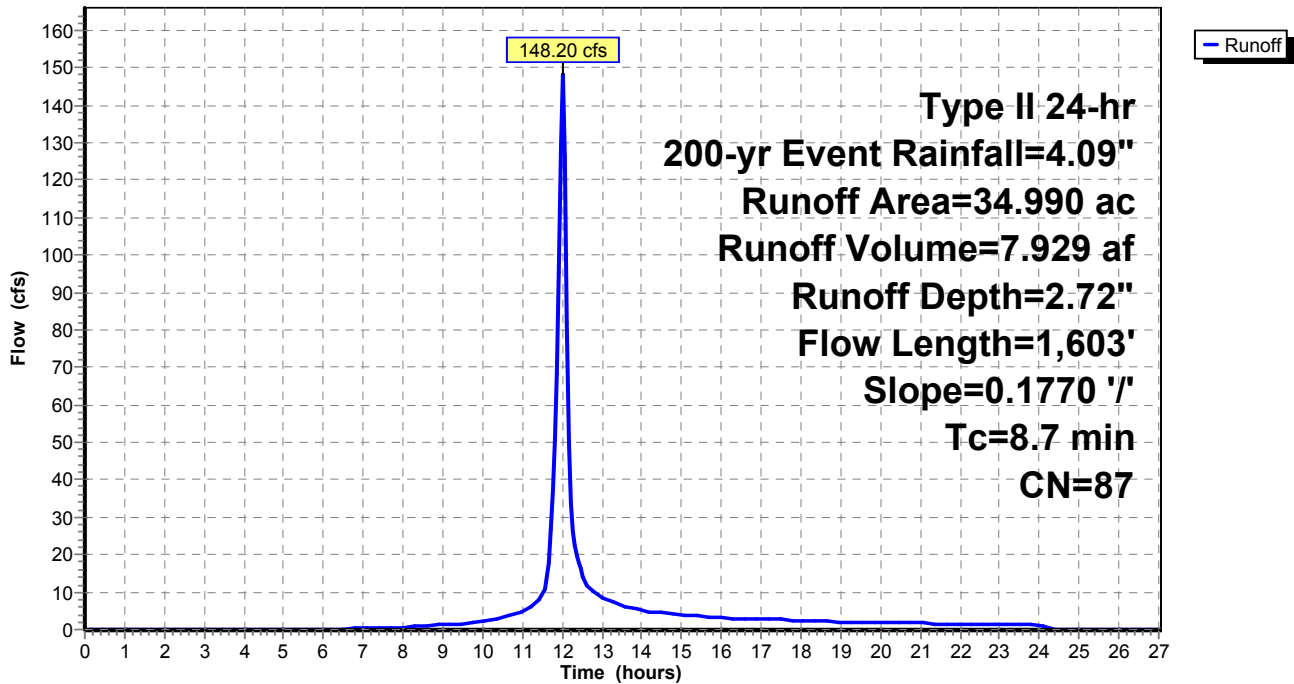
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 4: WS 4

Runoff = 297.56 cfs @ 12.02 hrs, Volume= 17.000 af, Depth= 2.72"

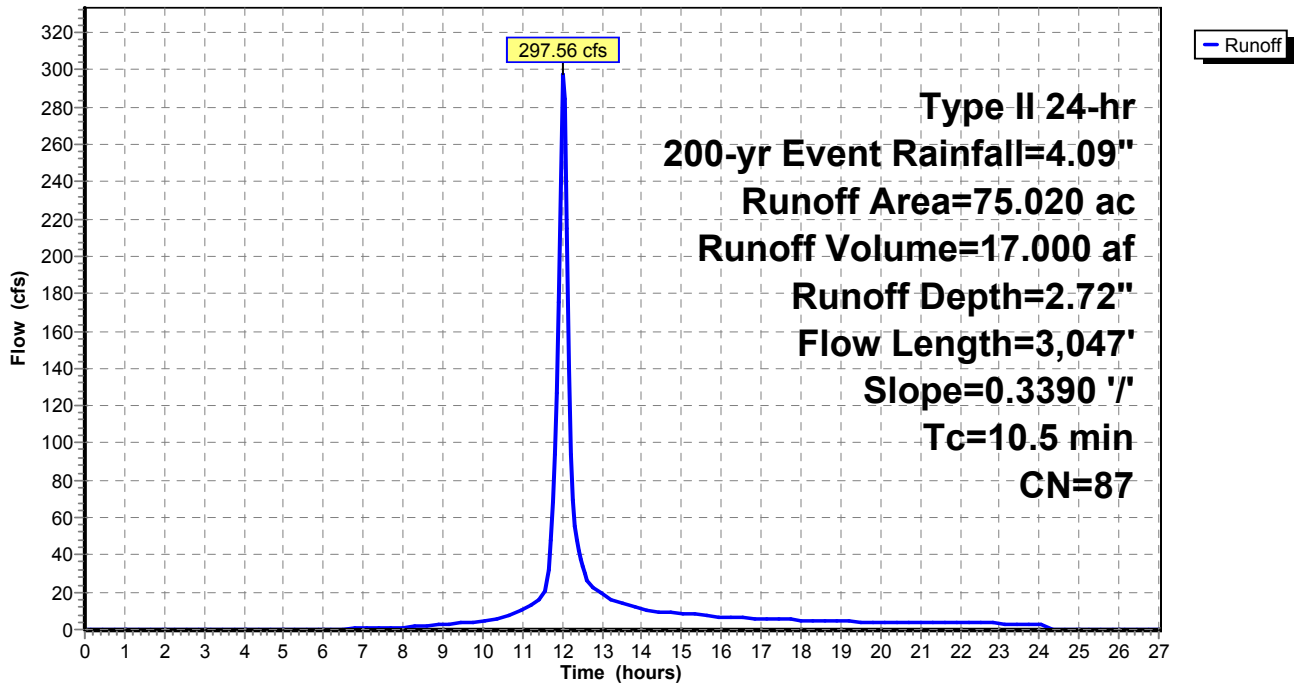
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 5: WS 5

Runoff = 392.70 cfs @ 12.10 hrs, Volume= 28.143 af, Depth= 2.72"

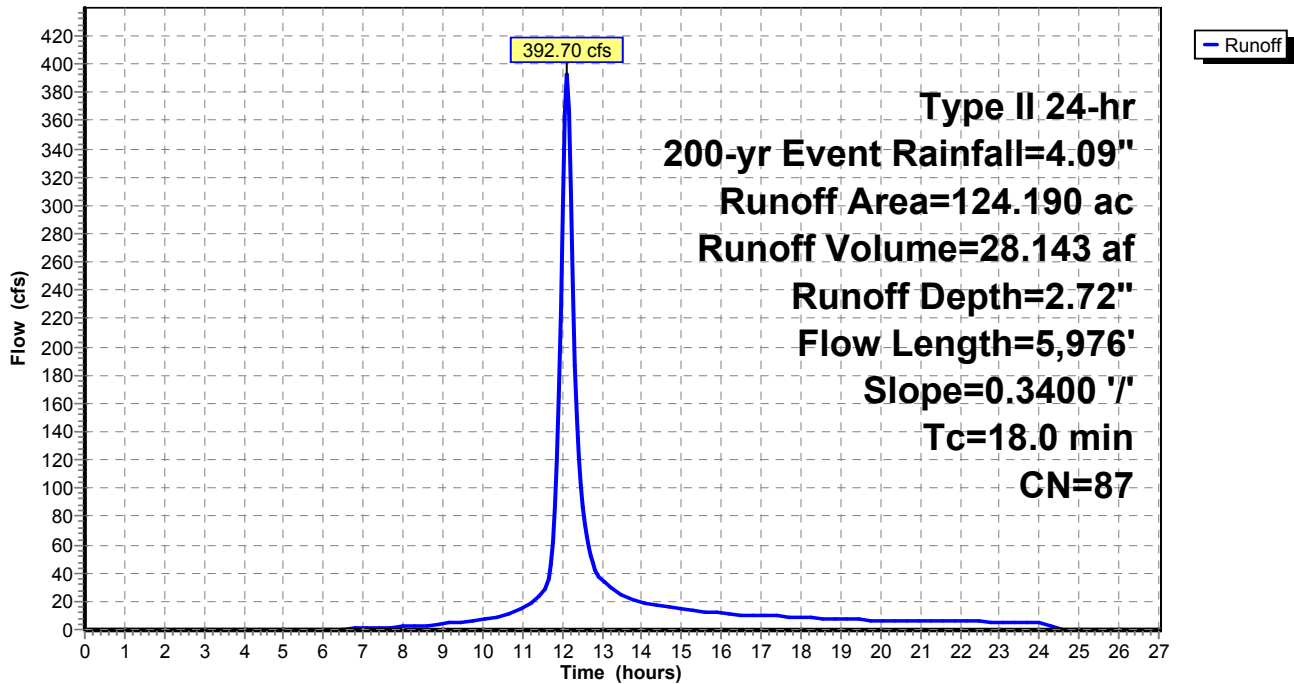
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 6: WS 6

Runoff = 888.50 cfs @ 12.17 hrs, Volume= 75.058 af, Depth= 2.72"

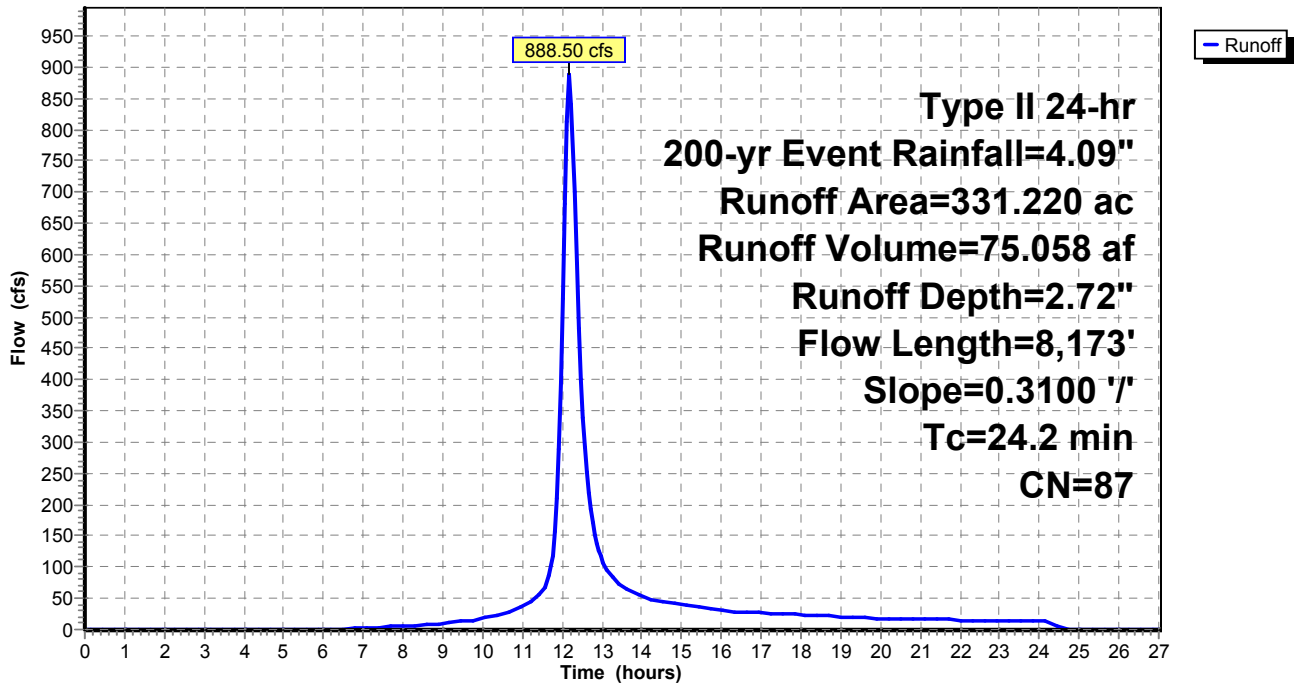
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 7: WS 7

Runoff = 471.20 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

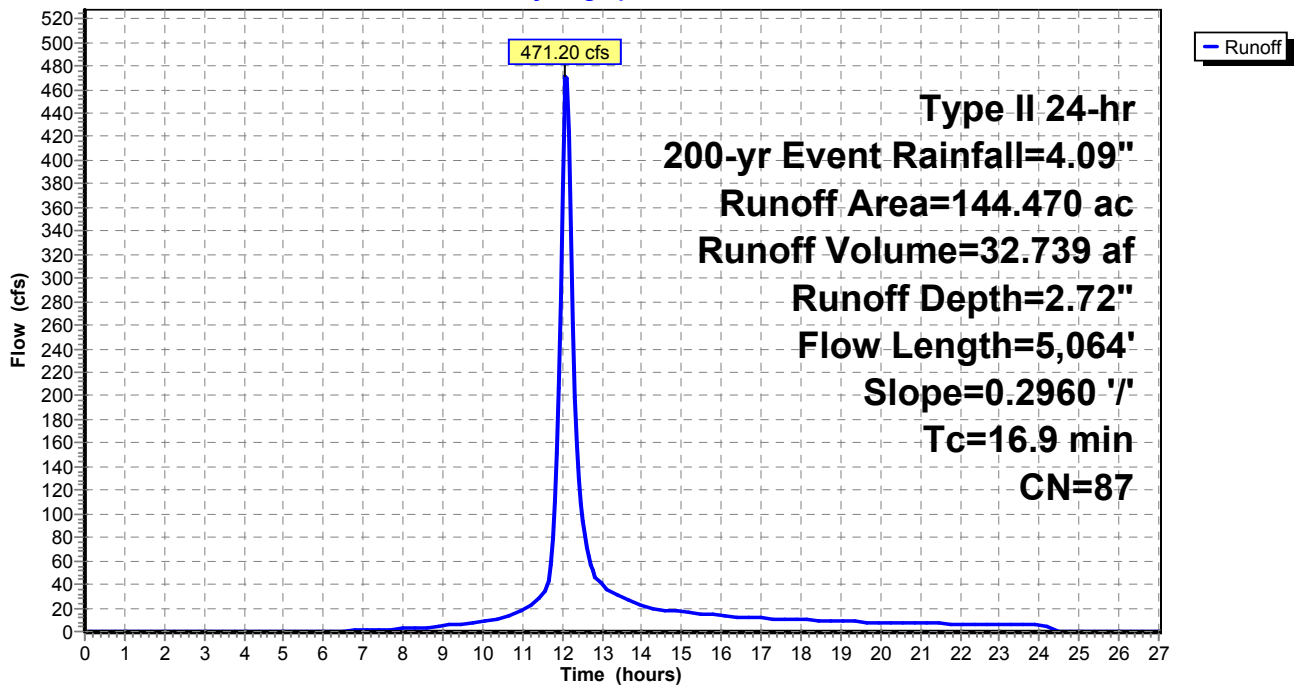
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 8: WS 8

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

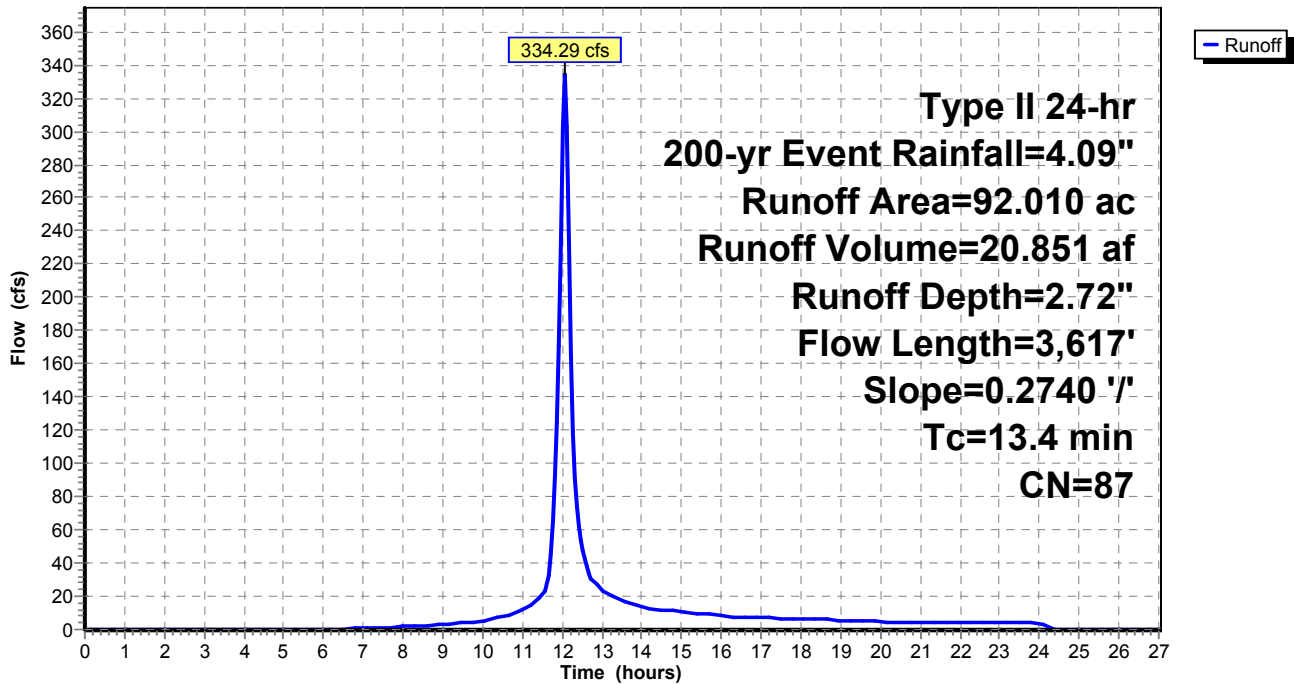
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 9: WS 9

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

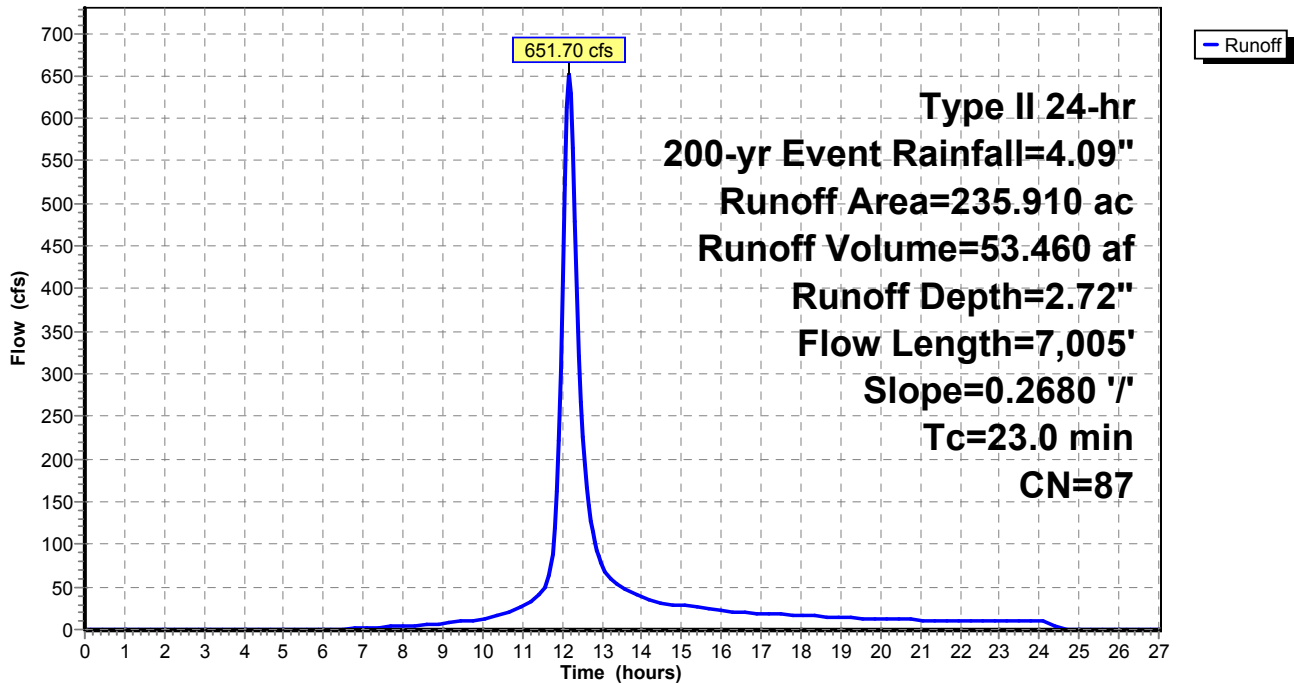
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 10: WS 10

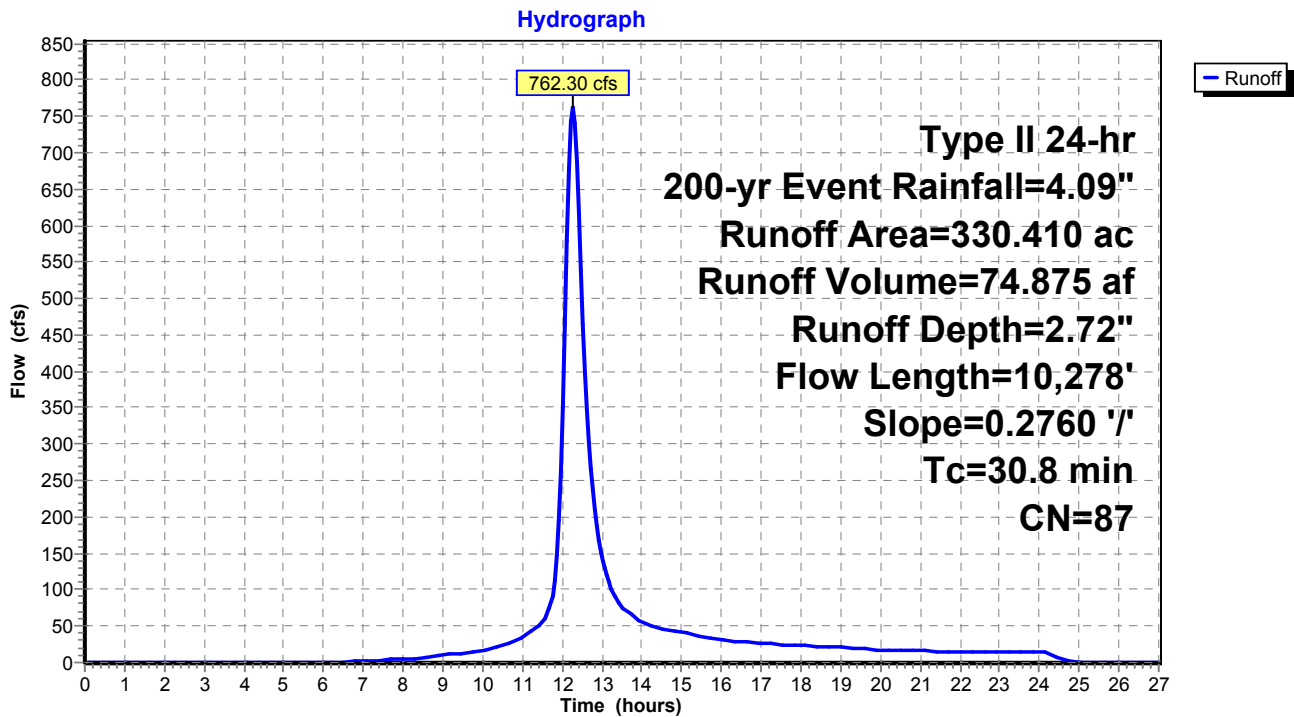
Runoff = 762.30 cfs @ 12.25 hrs, Volume= 74.875 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 11: WS 11

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

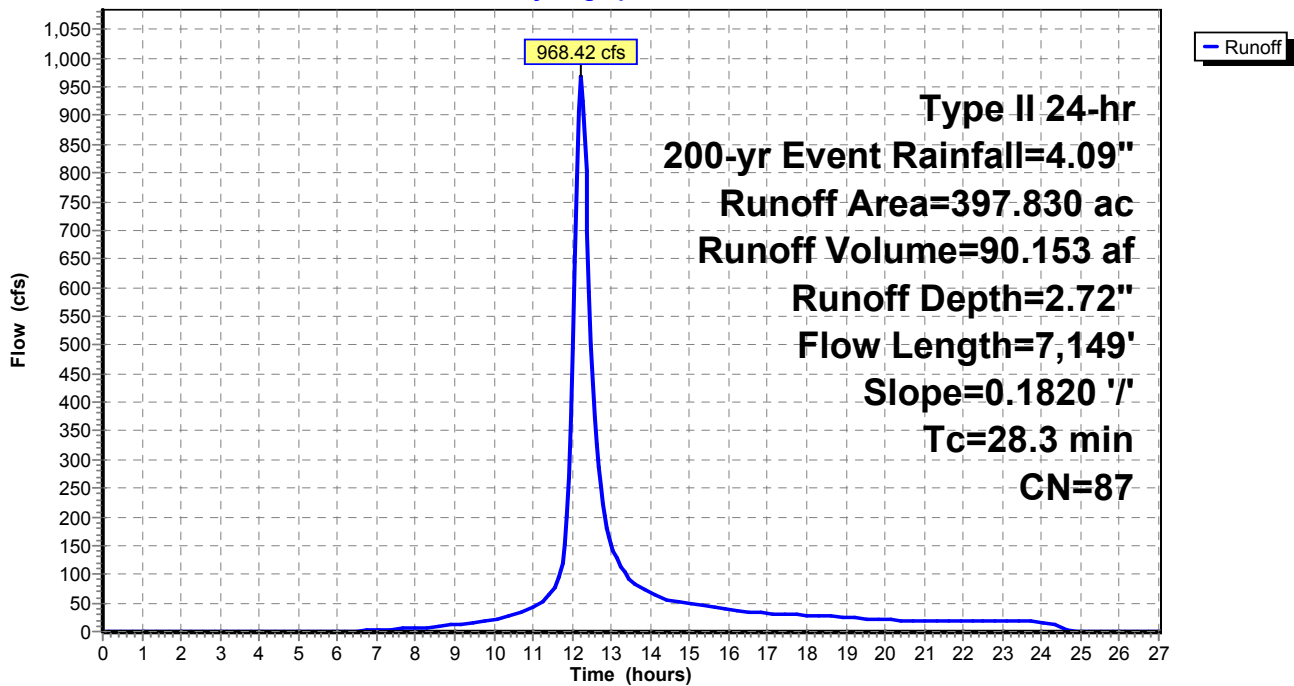
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 12: WS 12

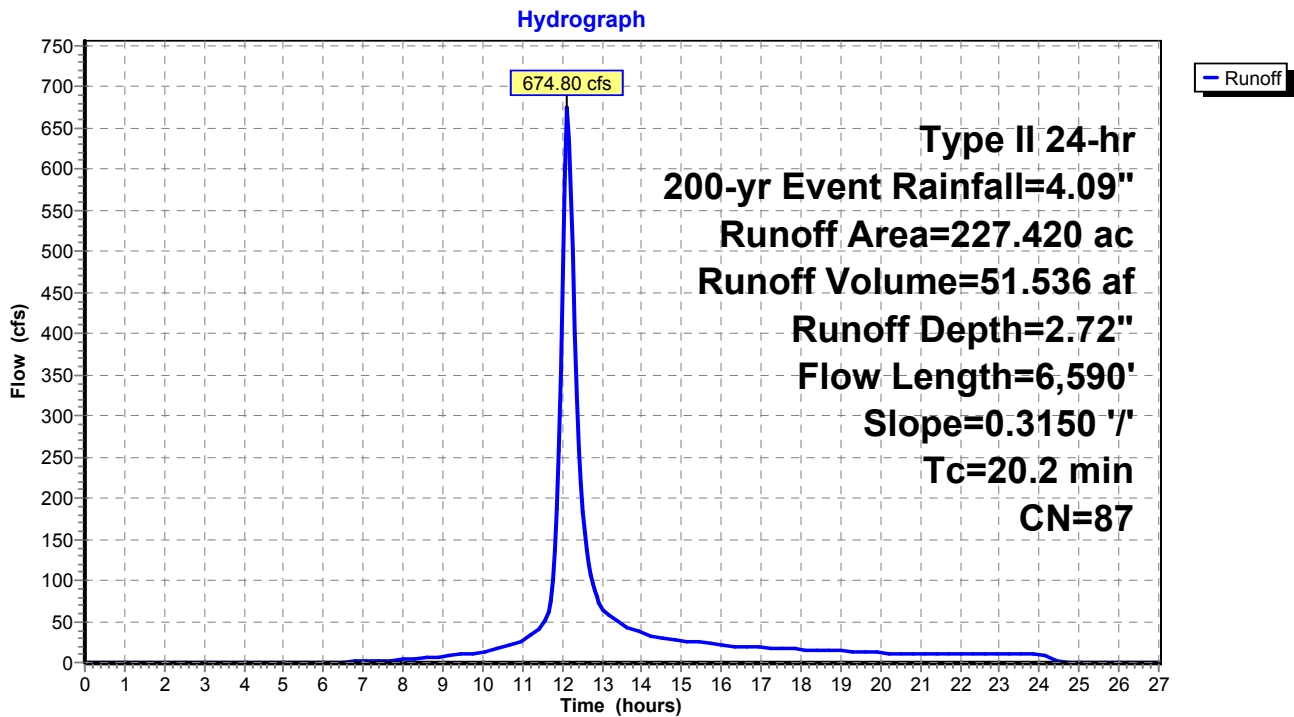
Runoff = 674.80 cfs @ 12.13 hrs, Volume= 51.536 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 13: WS 13

Runoff = 745.85 cfs @ 12.17 hrs, Volume= 62.434 af, Depth= 2.72"

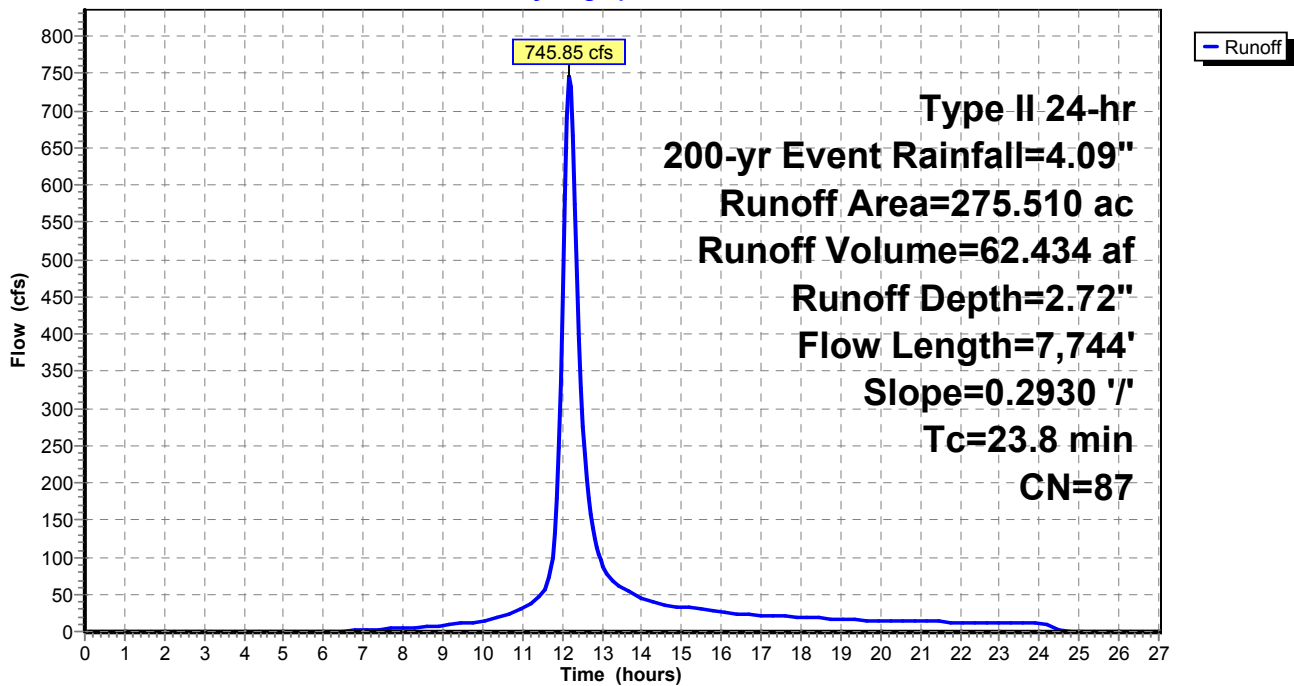
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 14: WS 14

Runoff = 279.49 cfs @ 12.06 hrs, Volume= 18.122 af, Depth= 2.72"

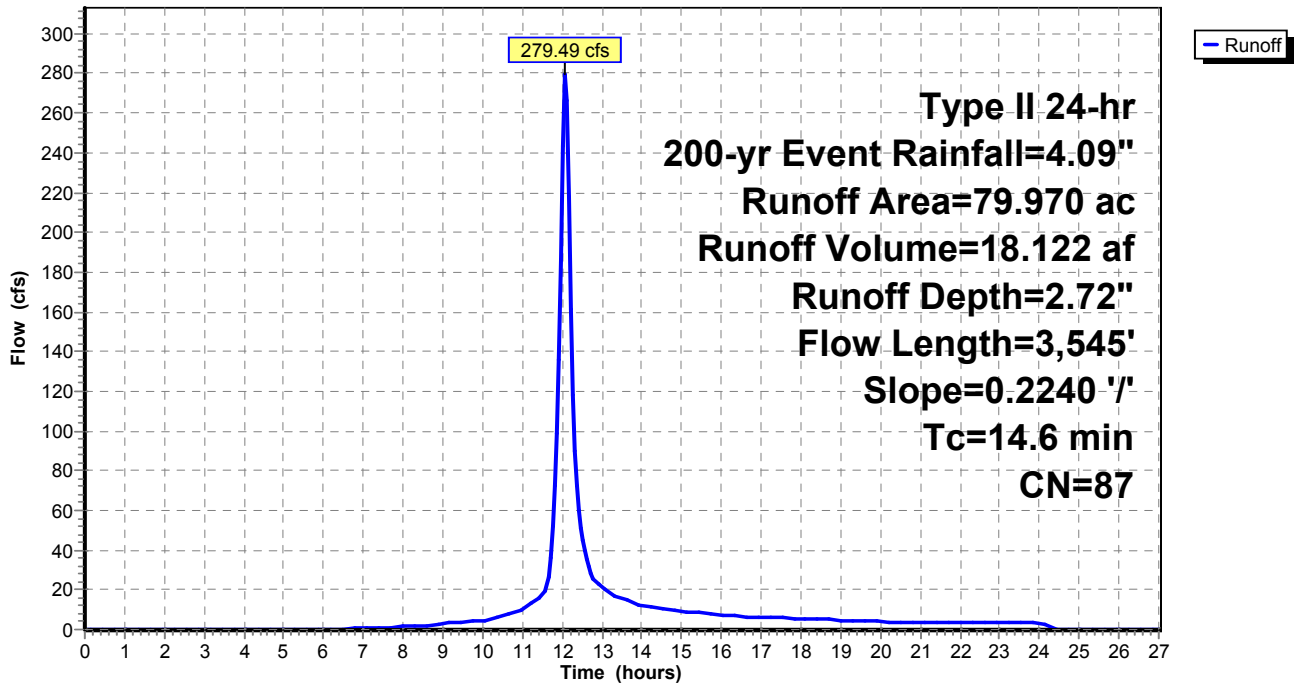
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



Existing Watersheds (Post-Quintana)*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1 Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=3.19"
 Tc=5.0 min CN=87 Runoff=155.94 cfs 7.504 af

Subcatchment2: WS 2 Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=484.31 cfs 28.349 af

Subcatchment3: WS 3 Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=172.66 cfs 9.308 af

Subcatchment4: WS 4 Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=346.92 cfs 19.956 af

Subcatchment5: WS 5 Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=458.61 cfs 33.036 af

Subcatchment6: WS 6 Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=1,038.74 cfs 88.109 af

Subcatchment7: WS 7 Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=550.14 cfs 38.431 af

Subcatchment8: WS 8 Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=389.99 cfs 24.476 af

Subcatchment9: WS 9 Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=761.76 cfs 62.755 af

Subcatchment10: WS 10 Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=891.85 cfs 87.894 af

Subcatchment11: WS 11 Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=1,132.73 cfs 105.828 af

Subcatchment12: WS 12 Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=789.72 cfs 60.497 af

Subcatchment13: WS 13 Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=871.93 cfs 73.289 af

Subcatchment14: WS 14 Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=3.19"
 Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=326.19 cfs 21.273 af

Total Runoff Area = 2,483.730 ac Runoff Volume = 660.707 af Average Runoff Depth = 3.19"
90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac

Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 155.94 cfs @ 11.95 hrs, Volume= 7.504 af, Depth= 3.19"

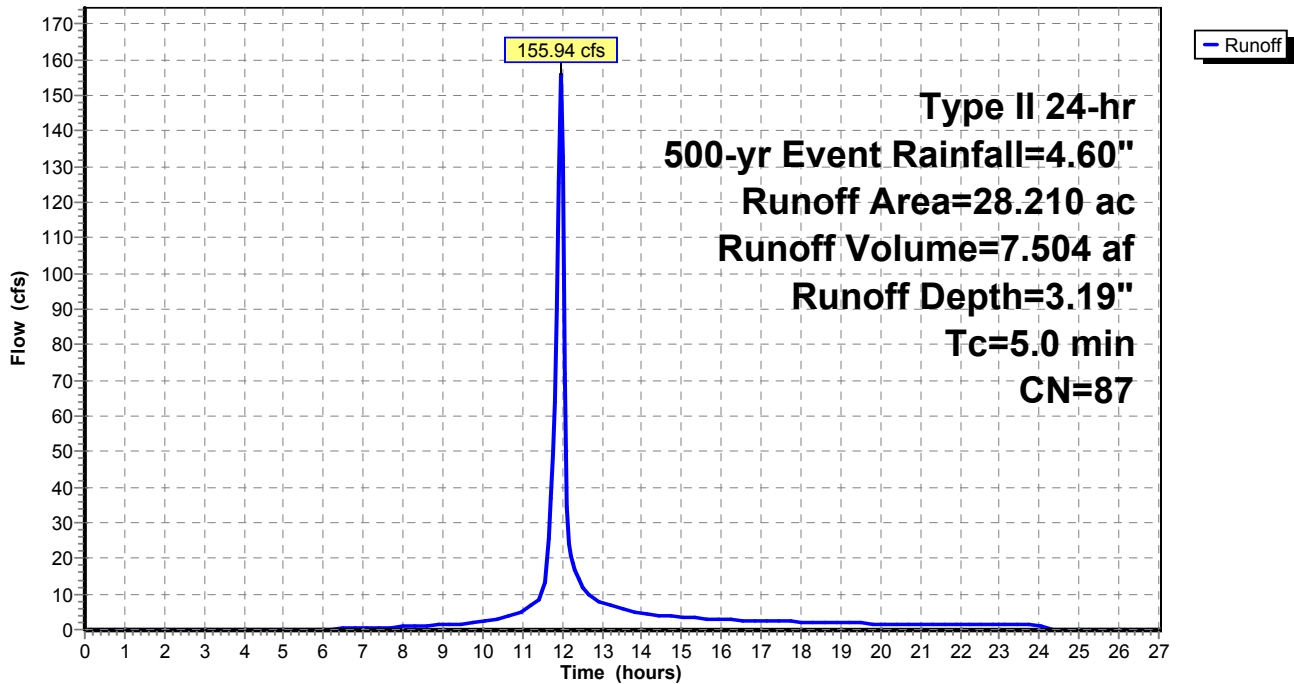
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 2: WS 2

Runoff = 484.31 cfs @ 12.02 hrs, Volume= 28.349 af, Depth= 3.19"

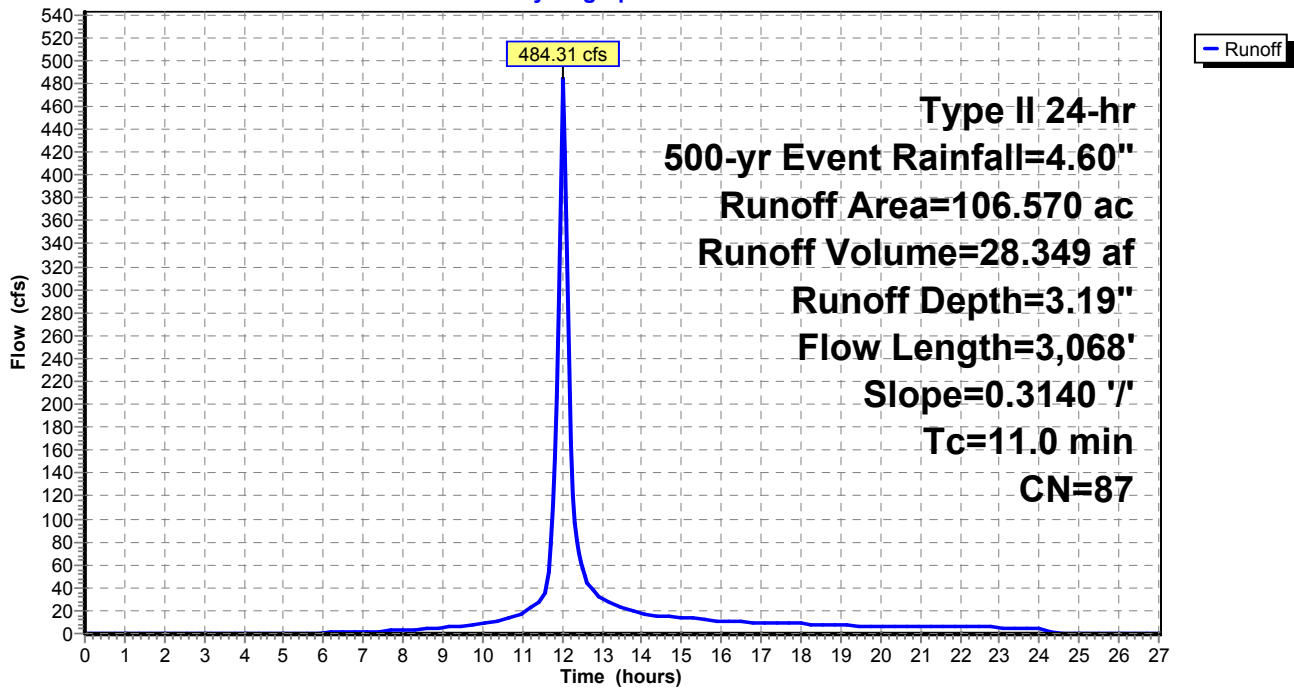
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 3: WS 3

Runoff = 172.66 cfs @ 12.00 hrs, Volume= 9.308 af, Depth= 3.19"

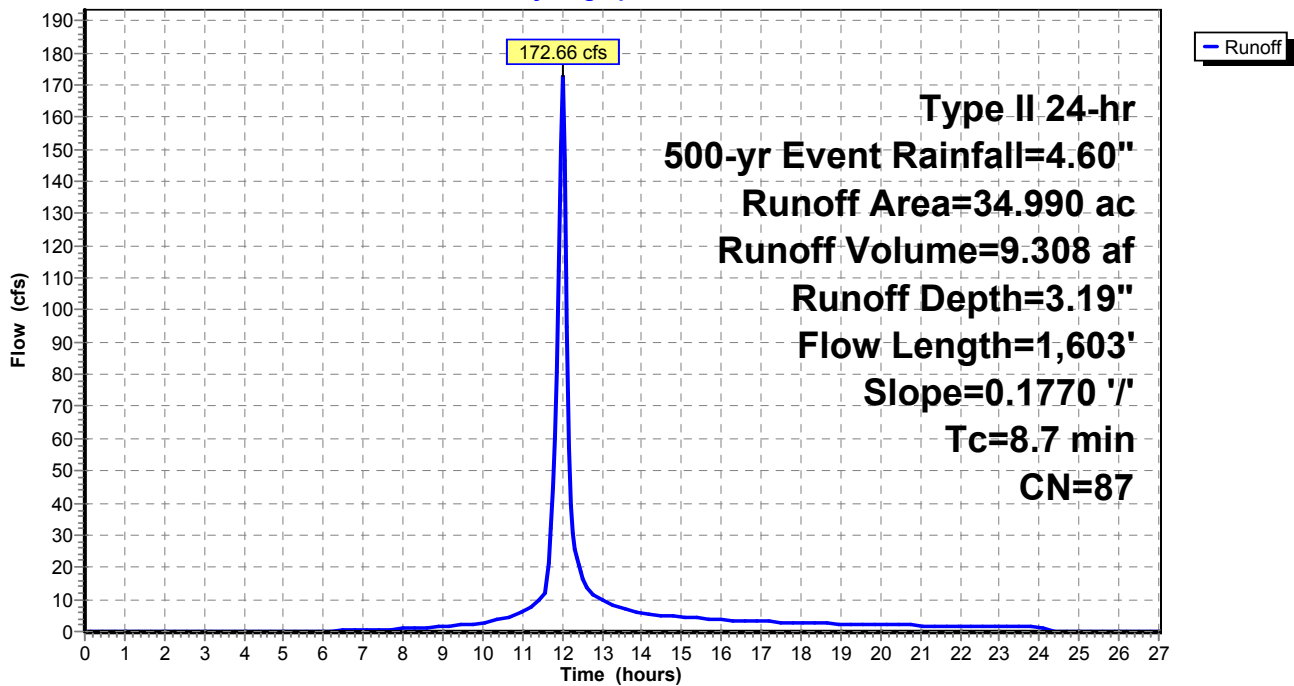
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 4: WS 4

Runoff = 346.92 cfs @ 12.02 hrs, Volume= 19.956 af, Depth= 3.19"

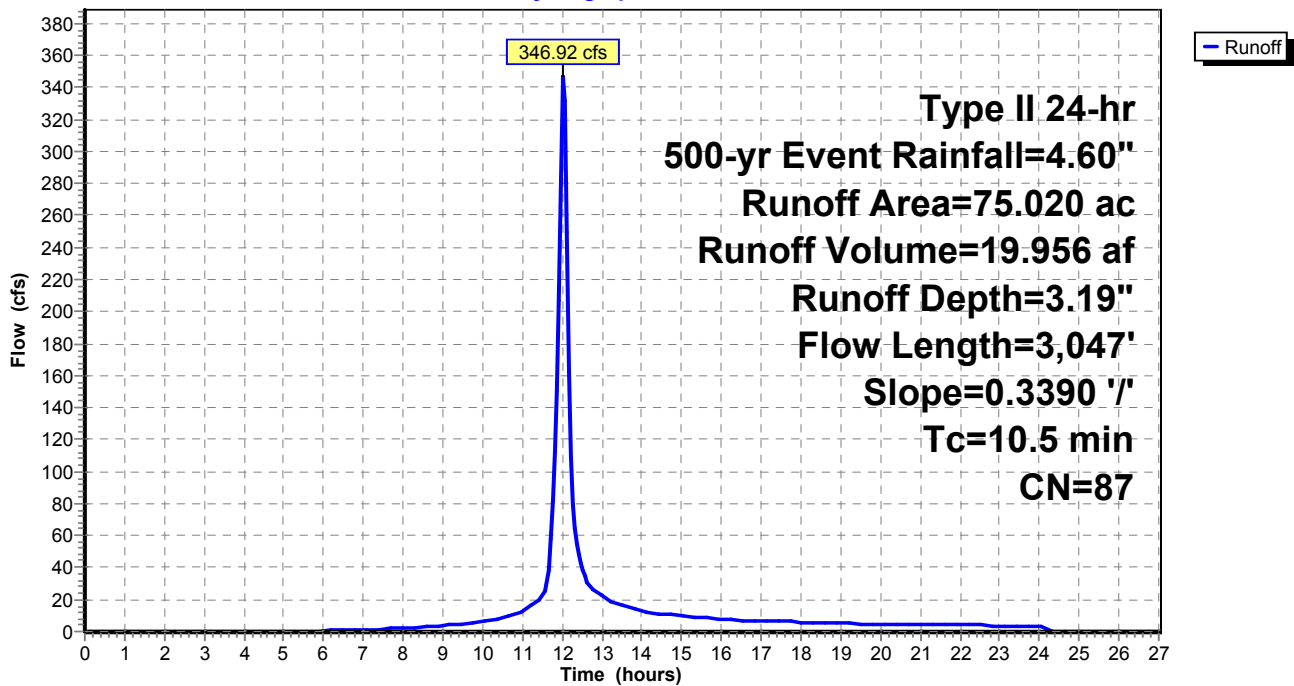
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 5: WS 5

Runoff = 458.61 cfs @ 12.10 hrs, Volume= 33.036 af, Depth= 3.19"

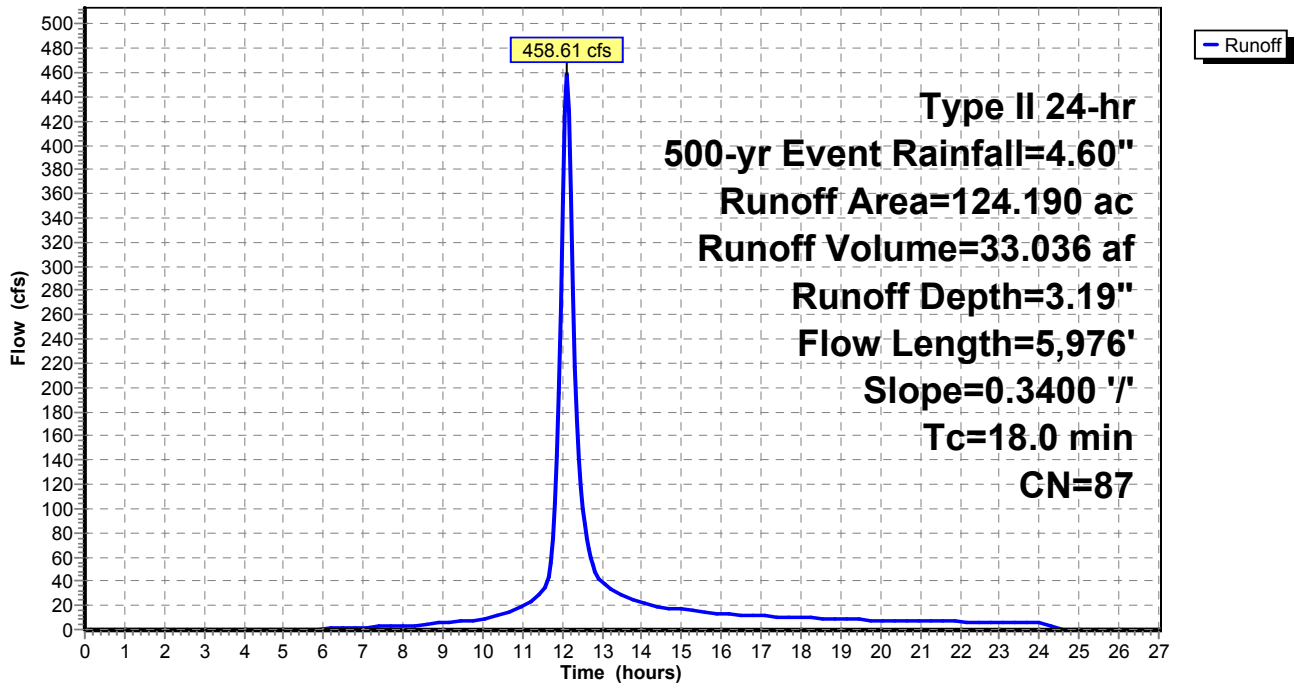
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 6: WS 6

Runoff = 1,038.74 cfs @ 12.17 hrs, Volume= 88.109 af, Depth= 3.19"

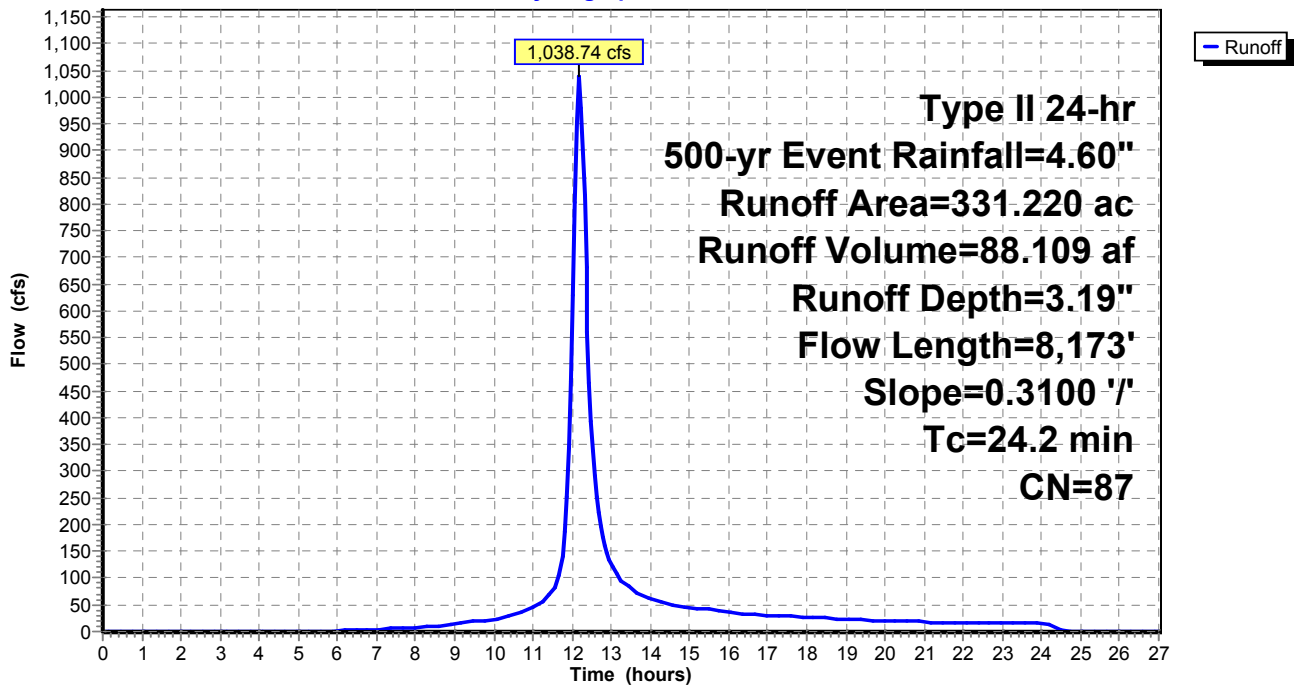
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 7: WS 7

Runoff = 550.14 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

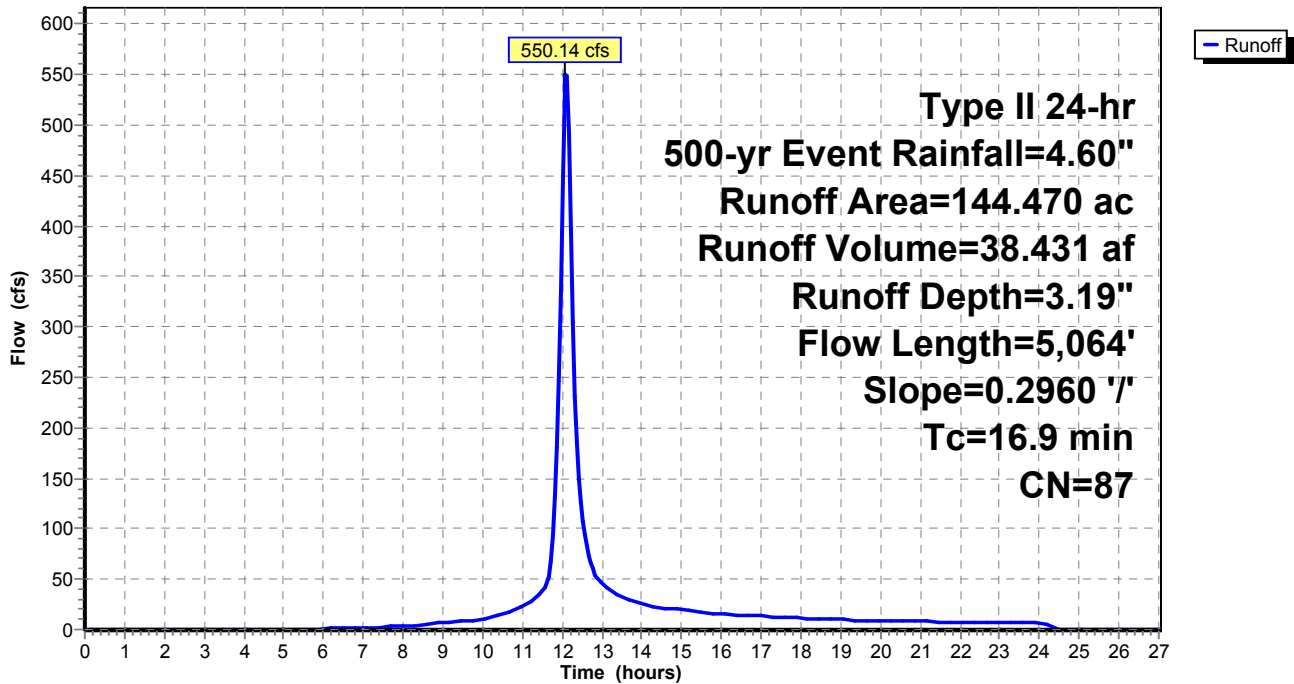
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 8: WS 8

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

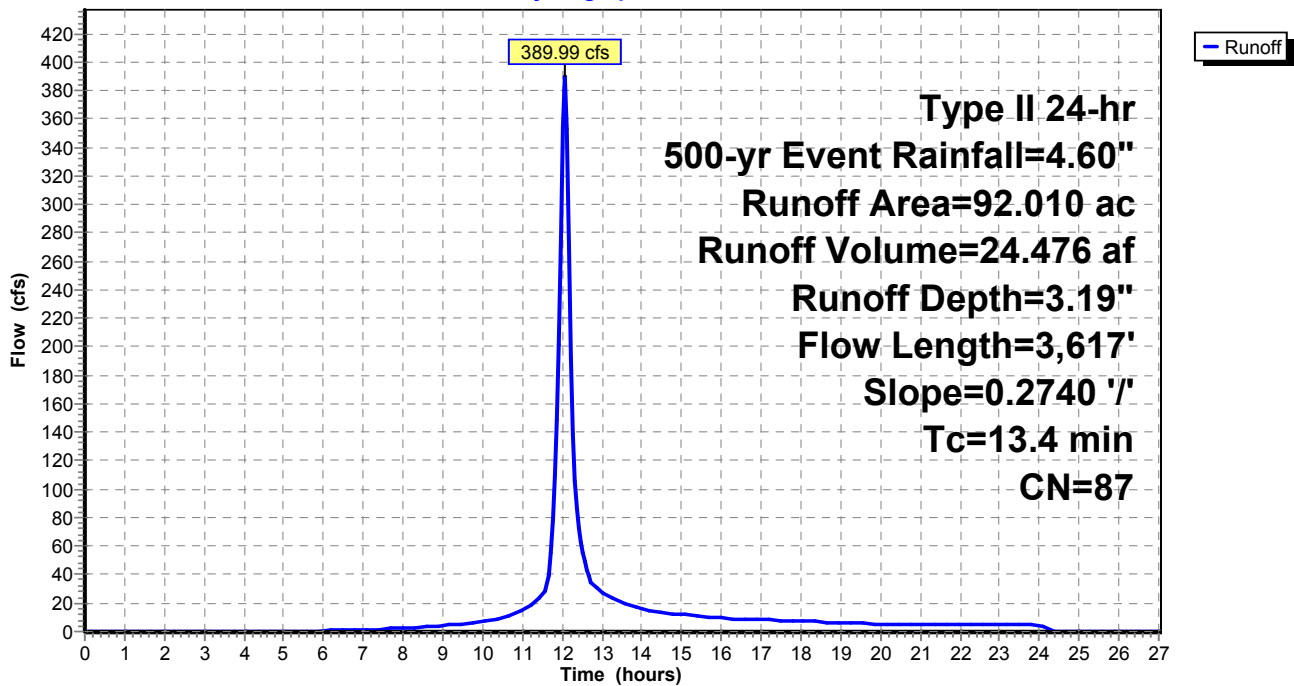
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 9: WS 9

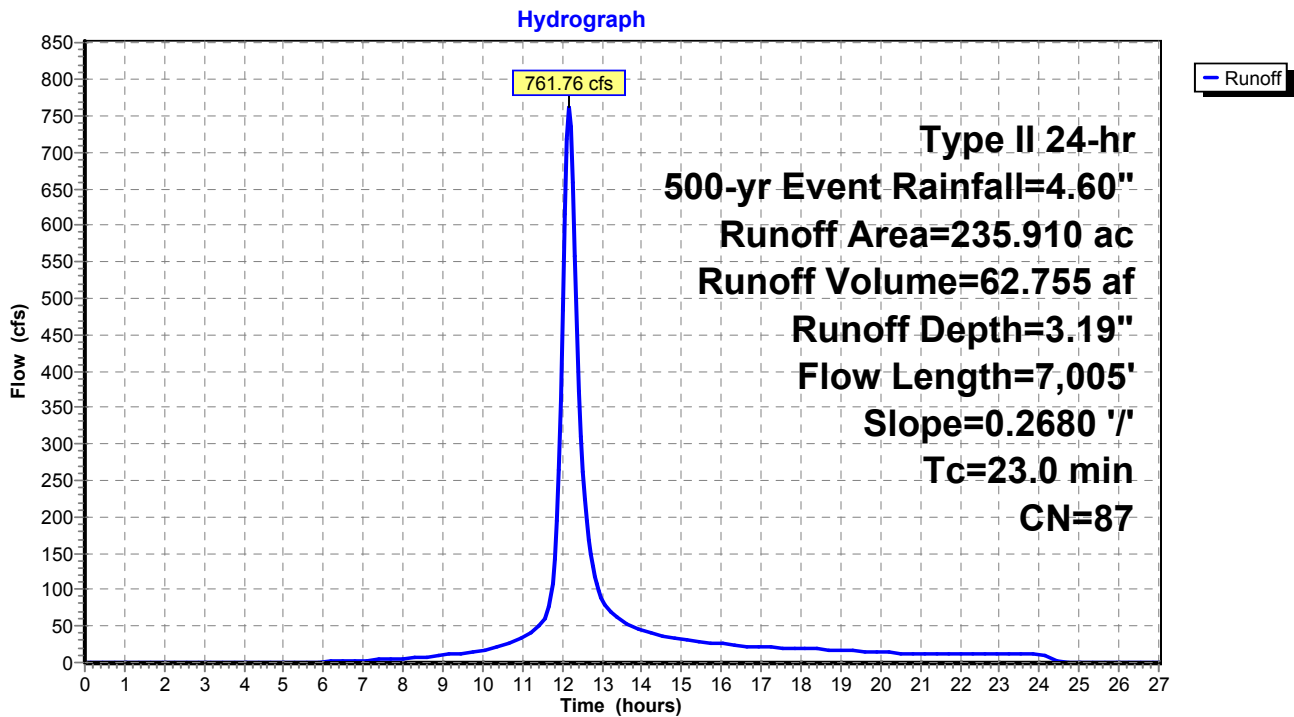
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 10: WS 10

Runoff = 891.85 cfs @ 12.25 hrs, Volume= 87.894 af, Depth= 3.19"

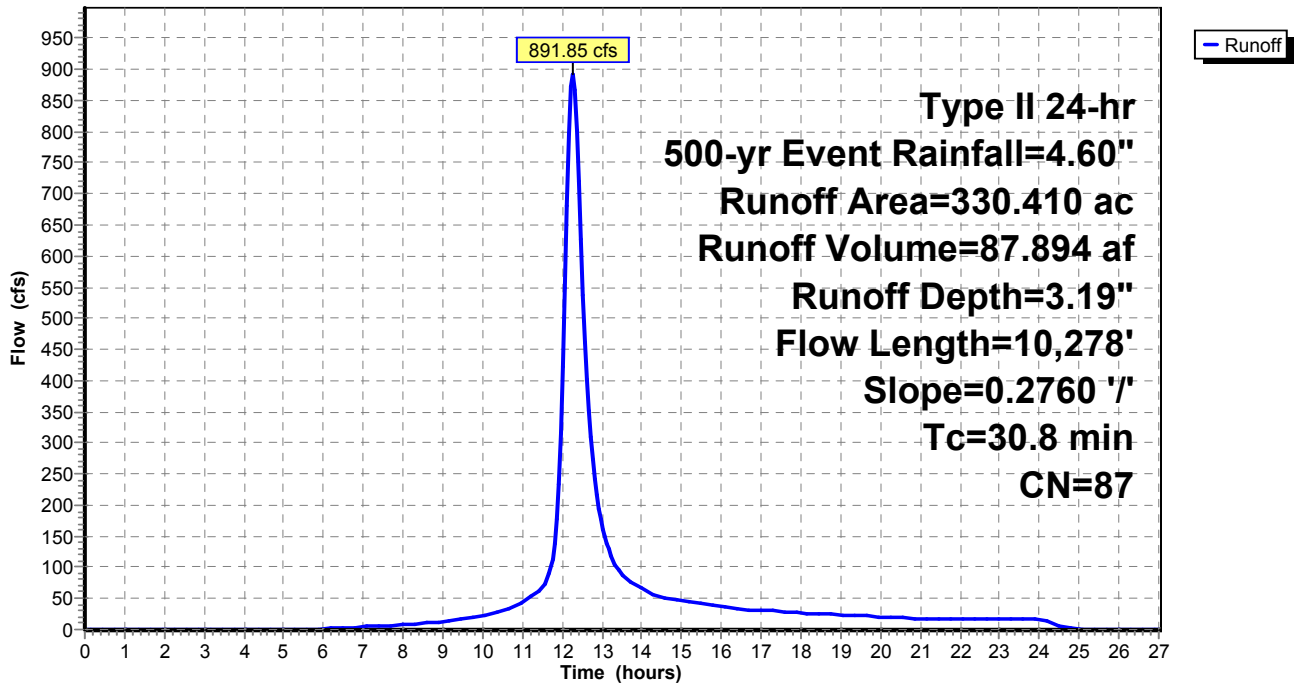
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 11: WS 11

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

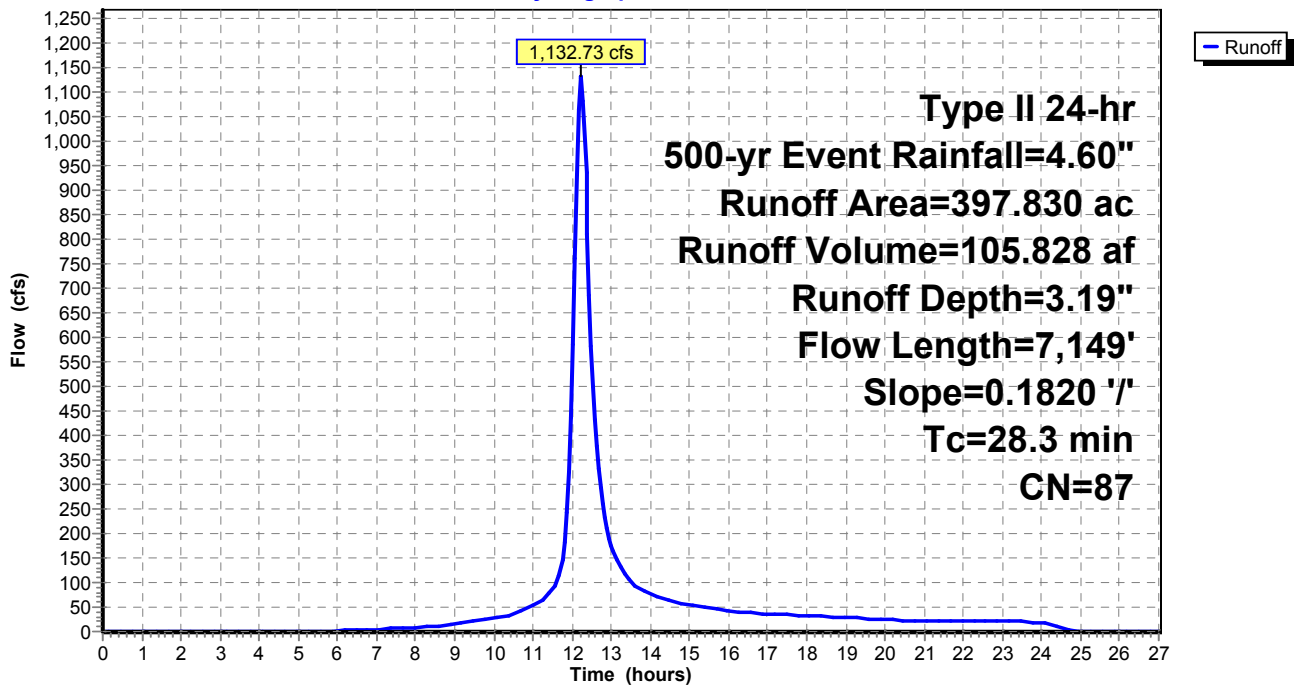
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 12: WS 12

Runoff = 789.72 cfs @ 12.12 hrs, Volume= 60.497 af, Depth= 3.19"

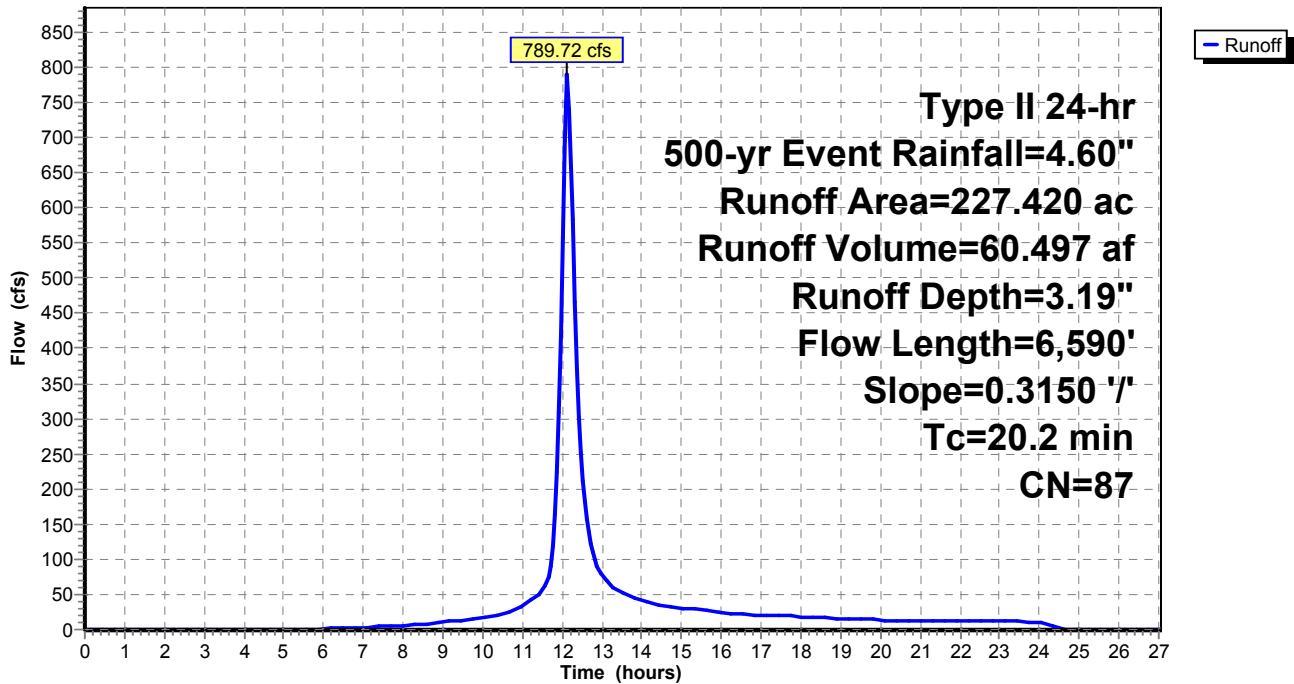
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 13: WS 13

Runoff = 871.93 cfs @ 12.16 hrs, Volume= 73.289 af, Depth= 3.19"

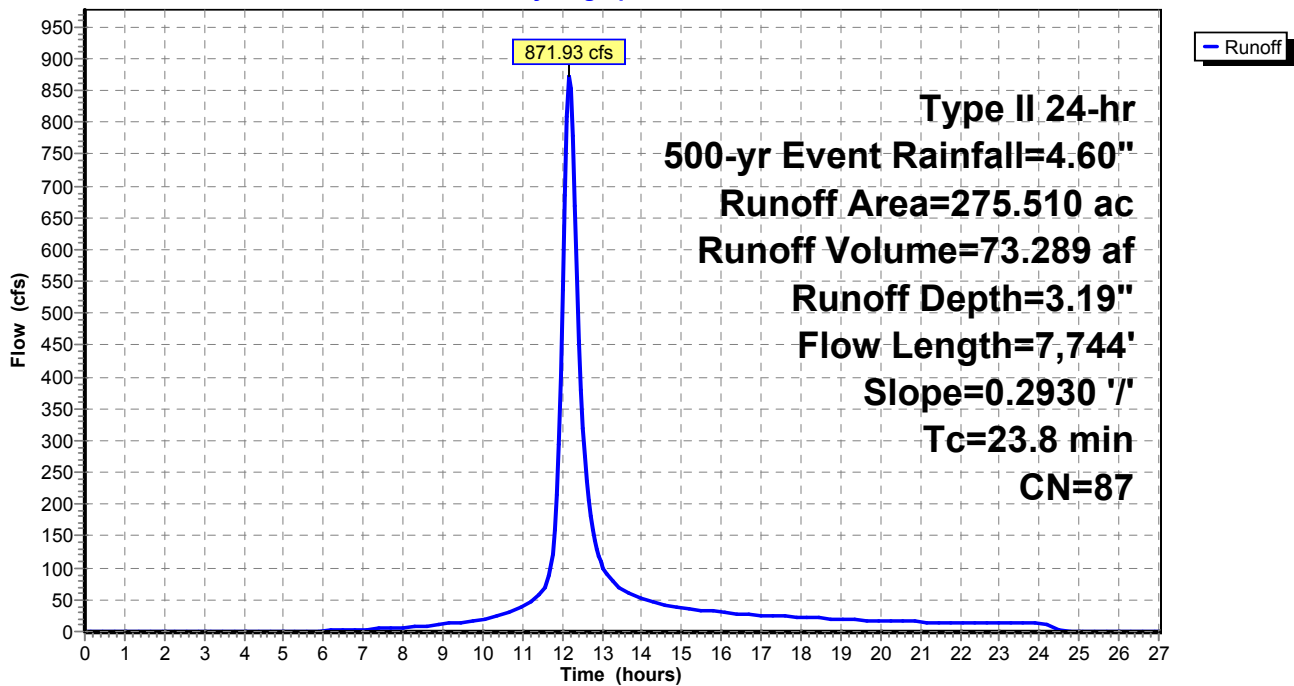
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Existing Watersheds (Post-Quintana)

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 14: WS 14

Runoff = 326.19 cfs @ 12.06 hrs, Volume= 21.273 af, Depth= 3.19"

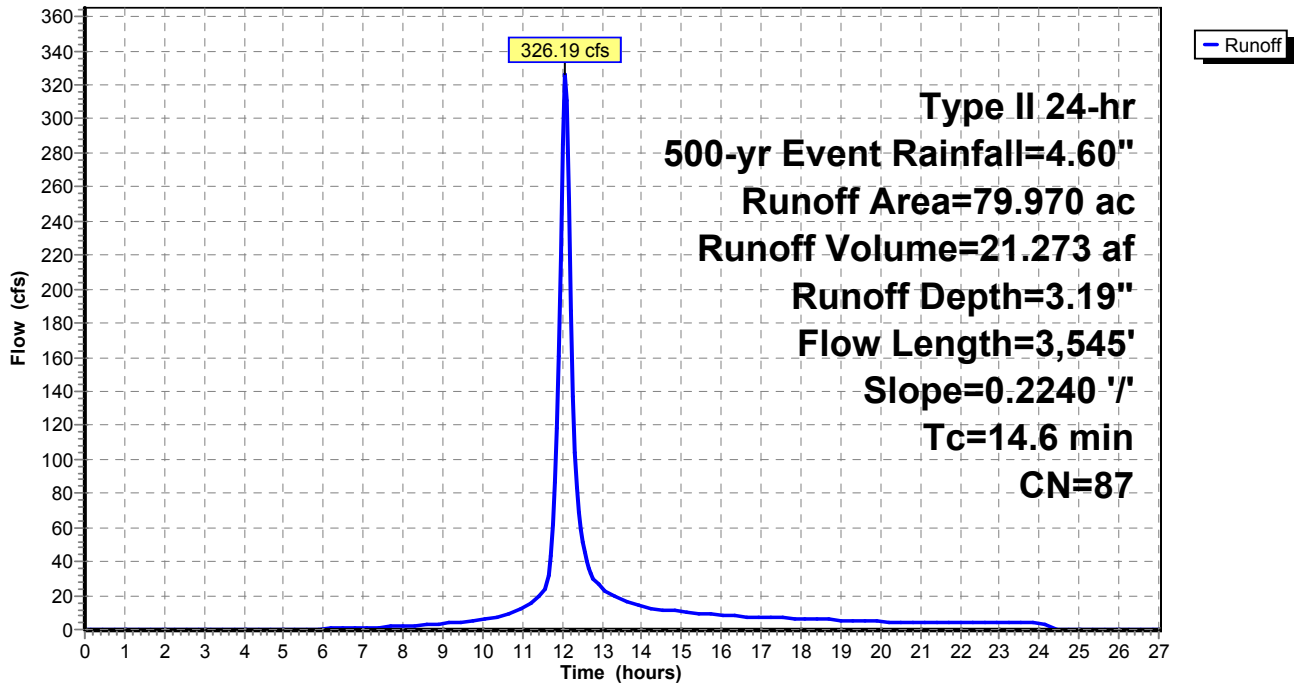
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

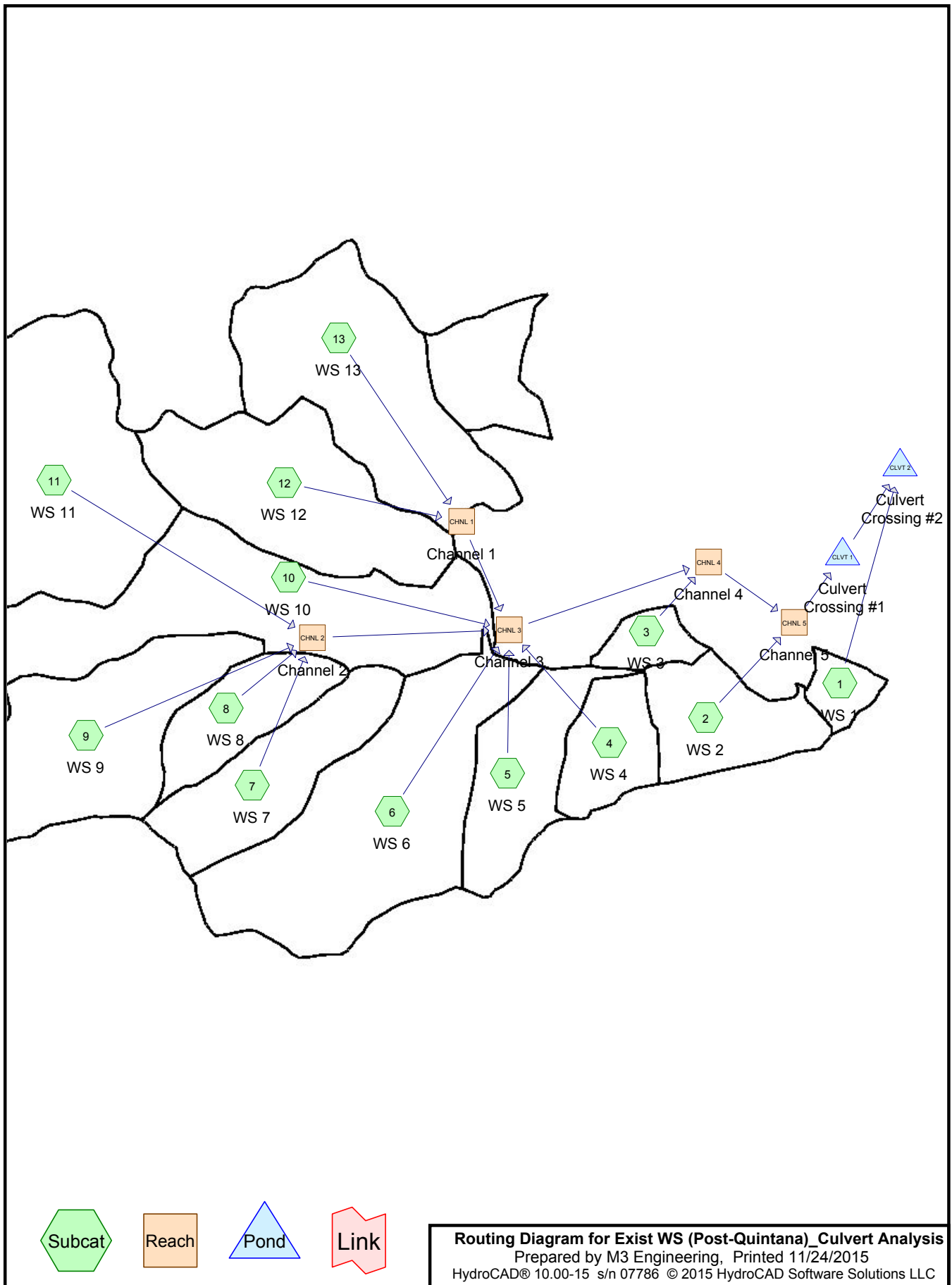
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

Subcatchment 14: WS 14

Hydrograph



APPENDIX D



Exist WS (Post-Quintana)_ Culvert Analysis

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2,163.390	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13)
240.370	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13)
2,403.760	87	TOTAL AREA

Exist WS (Post-Quintana)_Culvert Analysis

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2,403.760	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
0.000	Other	
2,403.760		TOTAL AREA

Exist WS (Post-Quintana)_ Culvert Analysis

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,163.390	0.000	2,163.390	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
0.000	0.000	0.000	240.370	0.000	240.370	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
0.000	0.000	0.000	2,403.760	0.000	2,403.760	TOTAL AREA	

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.36" Tc=5.0 min CN=87 Runoff=117.20 cfs 5.555 af
Subcatchment2: WS 2	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=363.66 cfs 20.985 af
Subcatchment3: WS 3	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=129.54 cfs 6.890 af
Subcatchment4: WS 4	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=259.92 cfs 14.773 af
Subcatchment5: WS 5	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=342.49 cfs 24.455 af
Subcatchment6: WS 6	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=774.13 cfs 65.223 af
Subcatchment7: WS 7	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=411.06 cfs 28.449 af
Subcatchment8: WS 8	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=291.83 cfs 18.118 af
Subcatchment9: WS 9	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=567.91 cfs 46.455 af
Subcatchment10: WS 10	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=663.73 cfs 65.063 af
Subcatchment11: WS 11	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=843.39 cfs 78.339 af
Subcatchment12: WS 12	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=588.30 cfs 44.783 af
Subcatchment13: WS 13	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=649.87 cfs 54.252 af
Reach CHNL 1: Channel 1	Avg. Flow Depth=2.49' Max Vel=8.88 fps Inflow=1,228.49 cfs 99.035 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,184.09 cfs 99.033 af
Reach CHNL 2: Channel 2	Avg. Flow Depth=1.68' Max Vel=9.03 fps Inflow=1,933.74 cfs 171.361 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=1,565.27 cfs 170.940 af
Reach CHNL 3: Channel 3	Avg. Flow Depth=5.30' Max Vel=13.90 fps Inflow=3,580.90 cfs 439.487 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=3,520.63 cfs 439.082 af

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Reach CHNL 4: Channel 4 Avg. Flow Depth=3.72' Max Vel=20.81 fps Inflow=3,535.74 cfs 445.972 af
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=3,523.29 cfs 445.885 af

Reach CHNL 5: Channel 5 Avg. Flow Depth=6.13' Max Vel=16.97 fps Inflow=3,572.05 cfs 466.870 af
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=3,552.39 cfs 466.654 af

Pond CLVT 1: Culvert Crossing Peak Elev=5,389.78' Storage=1,214,495 cf Inflow=3,552.39 cfs 466.654 af
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,058.70 cfs 466.511 af

Pond CLVT 2: Culvert Crossing Peak Elev=5,386.54' Storage=5,175,763 cf Inflow=3,066.00 cfs 472.066 af
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,545.71 cfs 472.015 af

Total Runoff Area = 2,403.760 ac Runoff Volume = 473.340 af Average Runoff Depth = 2.36"
90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 117.20 cfs @ 11.95 hrs, Volume= 5.555 af, Depth= 2.36"

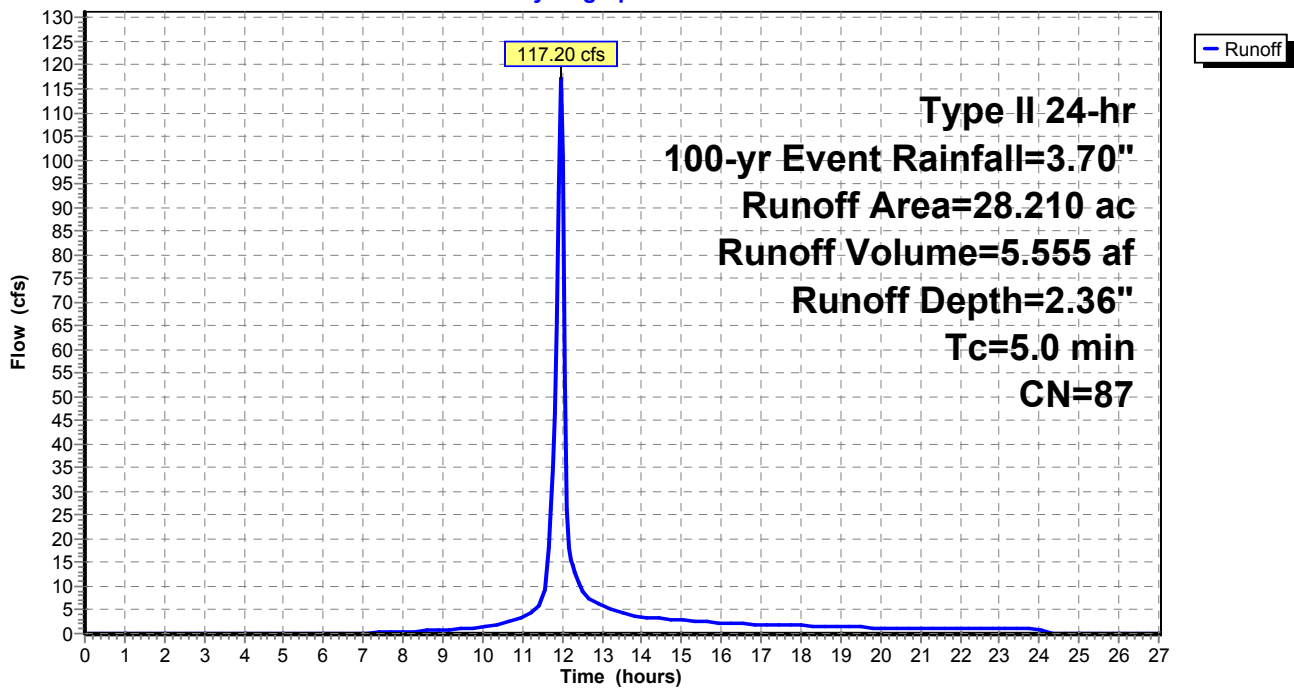
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 2: WS 2

Runoff = 363.66 cfs @ 12.03 hrs, Volume= 20.985 af, Depth= 2.36"

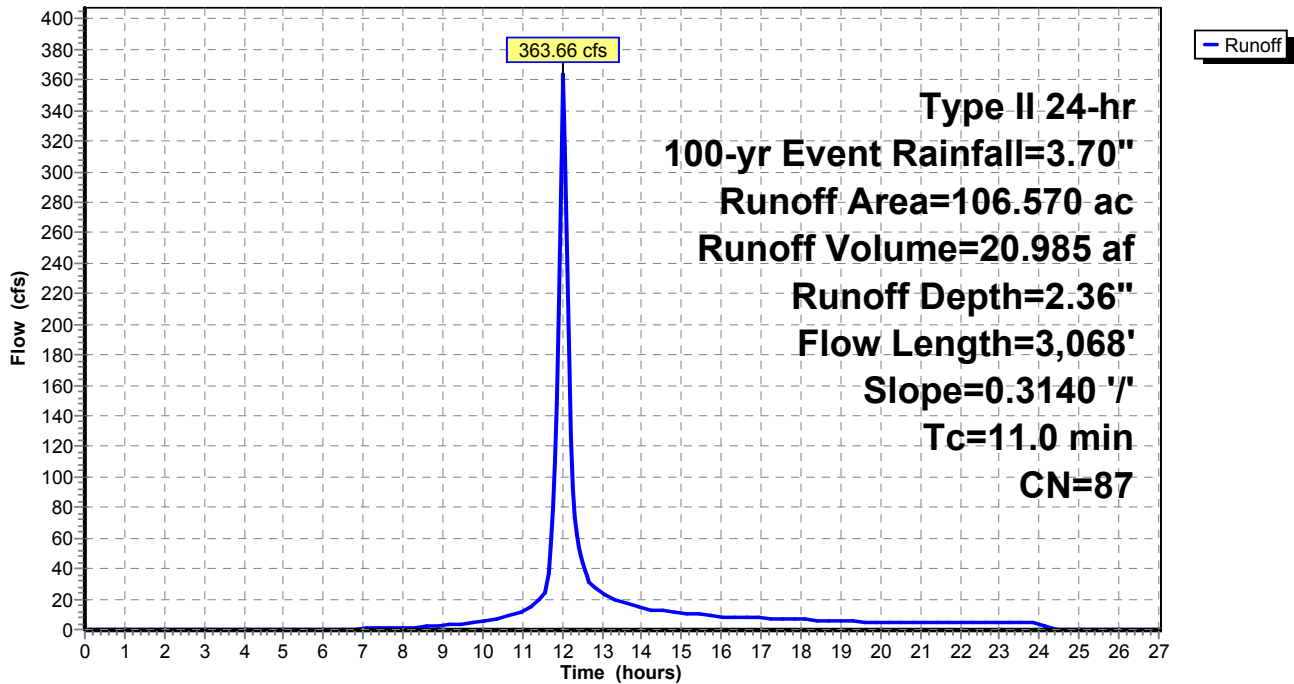
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 3: WS 3

Runoff = 129.54 cfs @ 12.00 hrs, Volume= 6.890 af, Depth= 2.36"

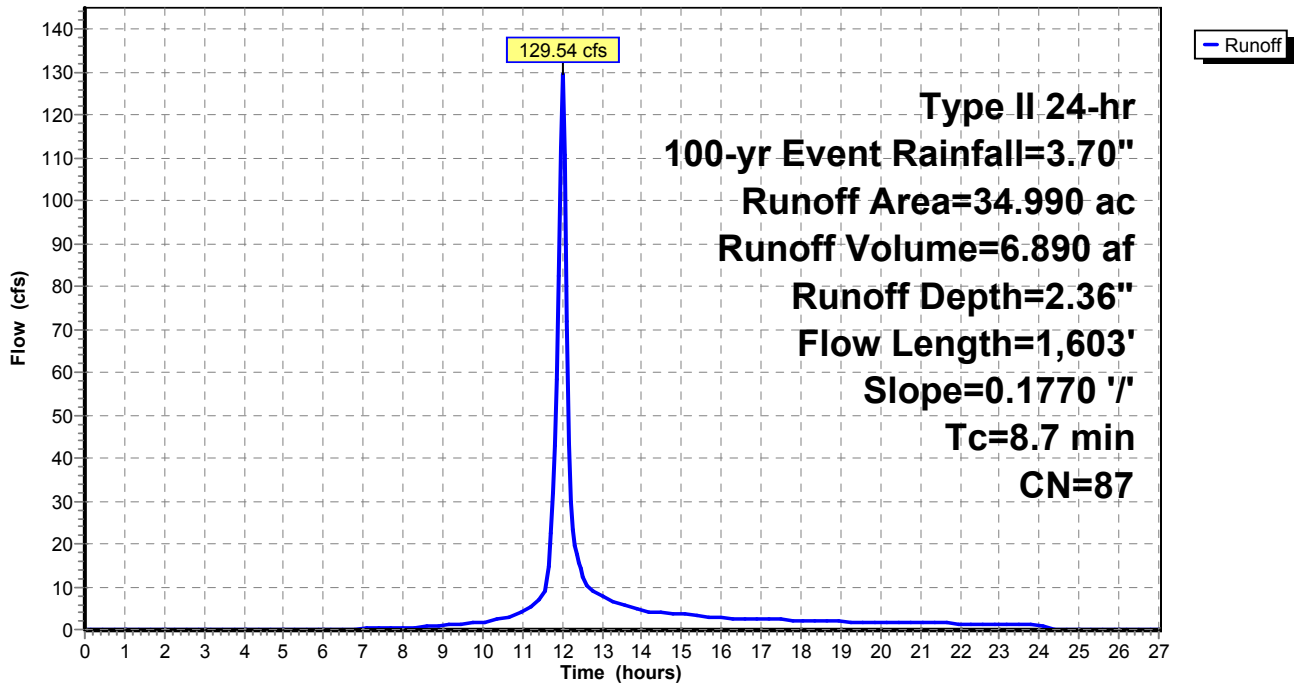
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 4: WS 4

Runoff = 259.92 cfs @ 12.02 hrs, Volume= 14.773 af, Depth= 2.36"

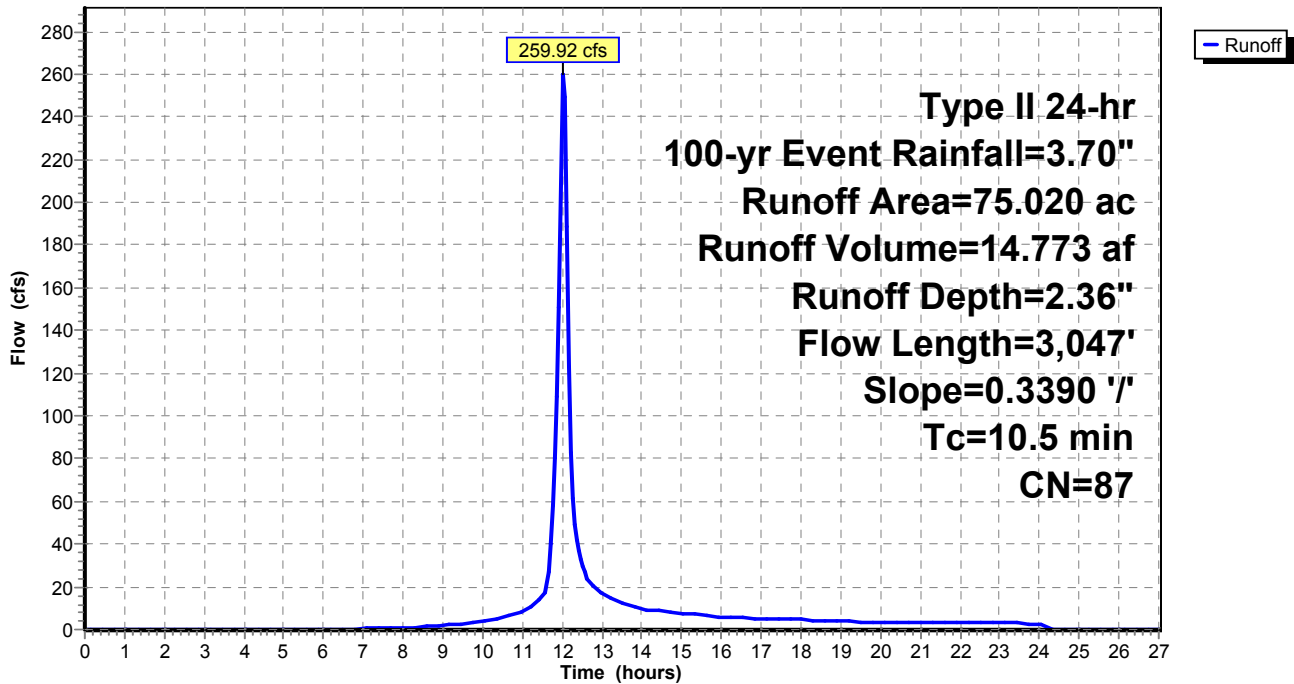
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 5: WS 5

Runoff = 342.49 cfs @ 12.10 hrs, Volume= 24.455 af, Depth= 2.36"

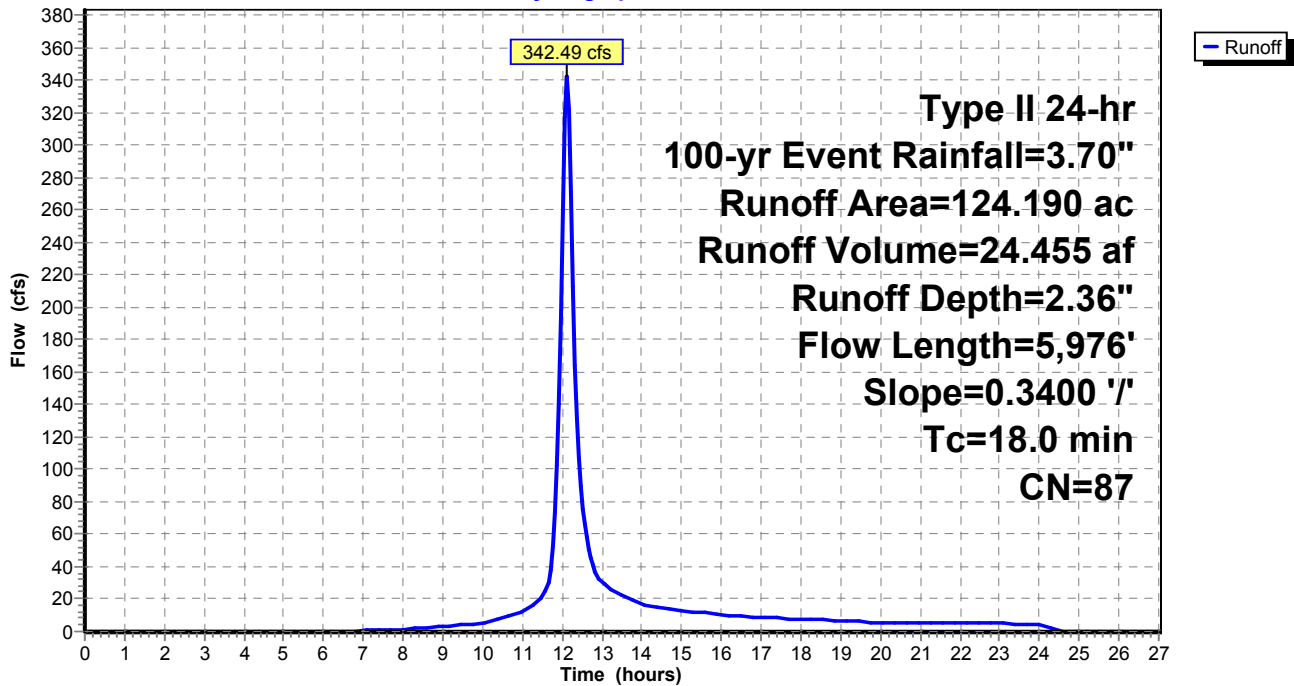
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 6: WS 6

Runoff = 774.13 cfs @ 12.17 hrs, Volume= 65.223 af, Depth= 2.36"

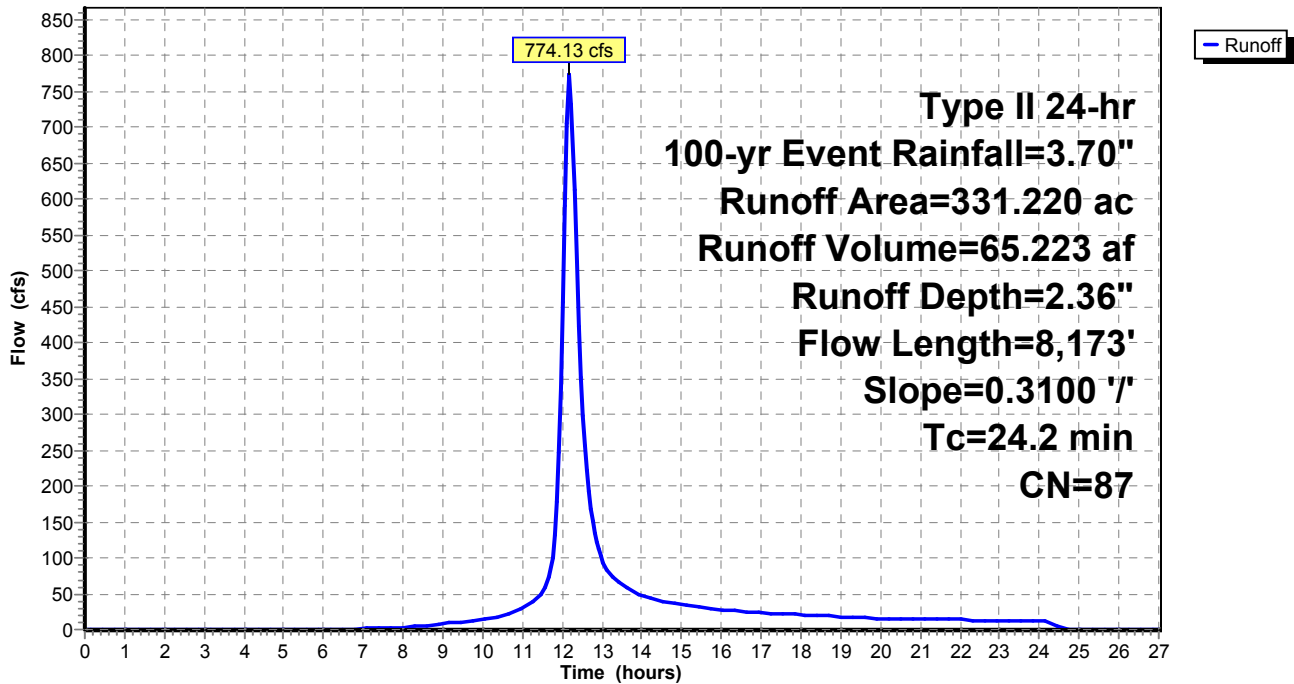
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 7: WS 7

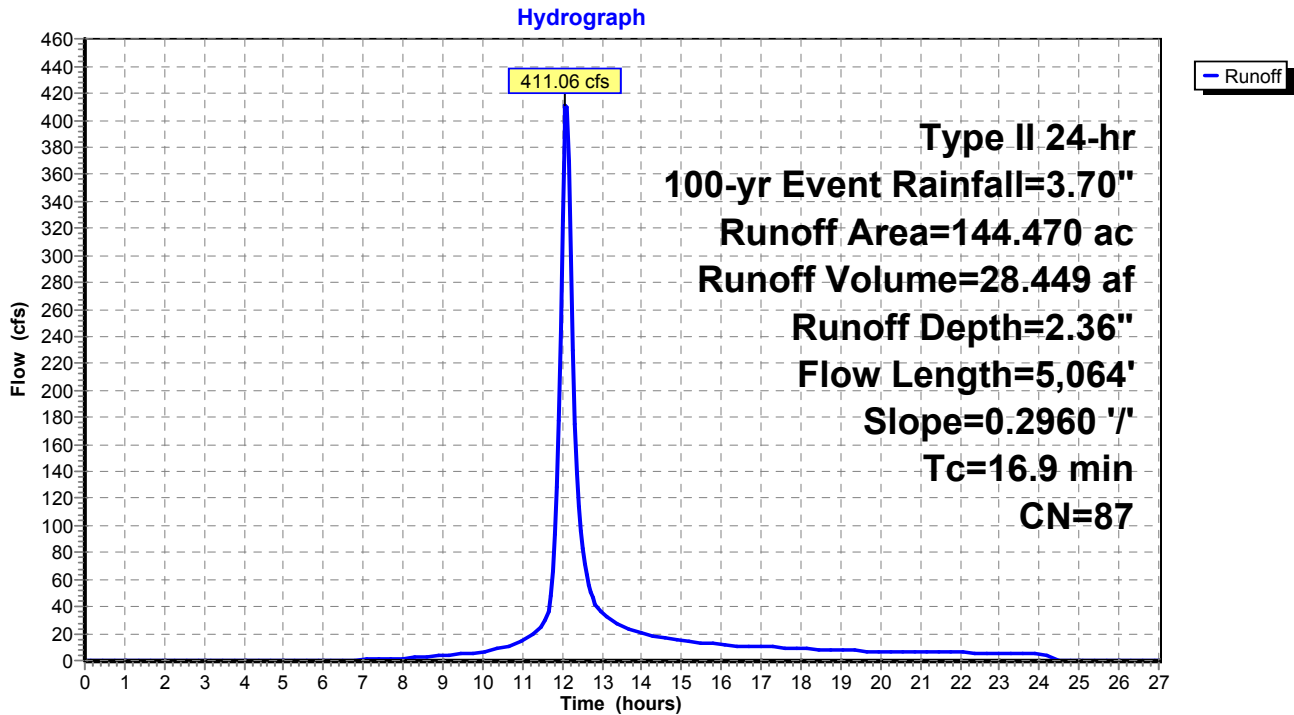
Runoff = 411.06 cfs @ 12.09 hrs, Volume= 28.449 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 8: WS 8

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

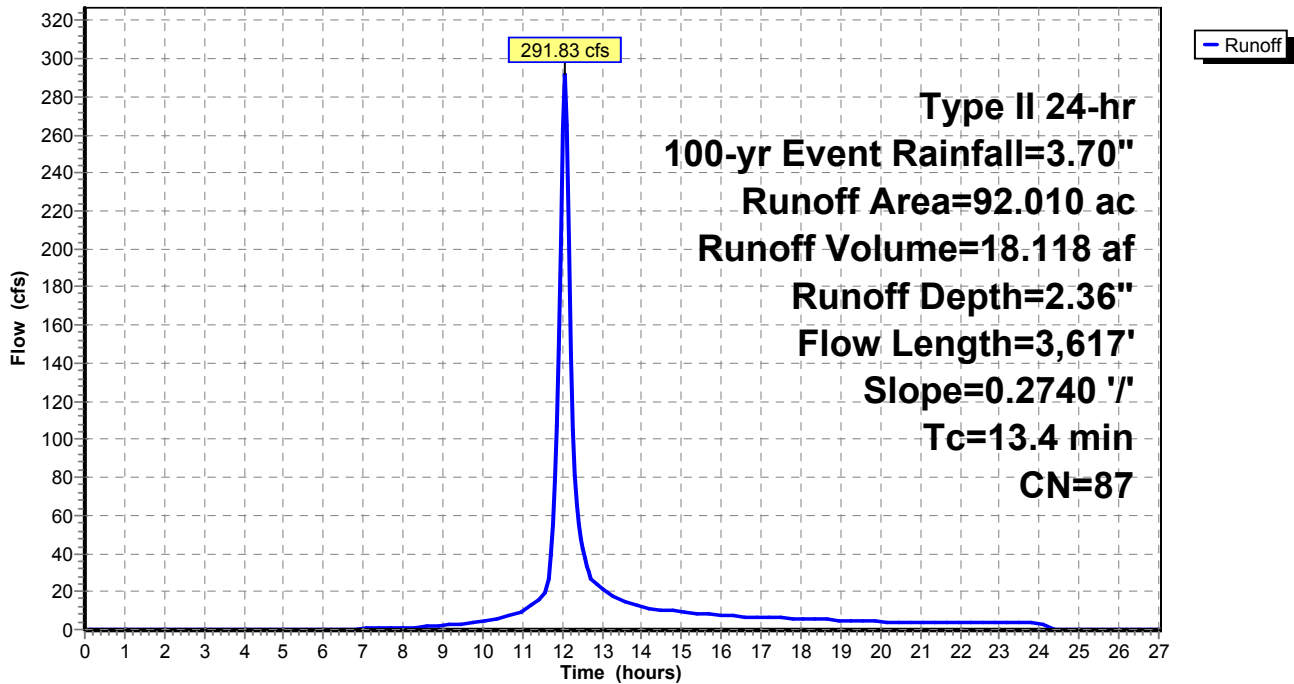
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 9: WS 9

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

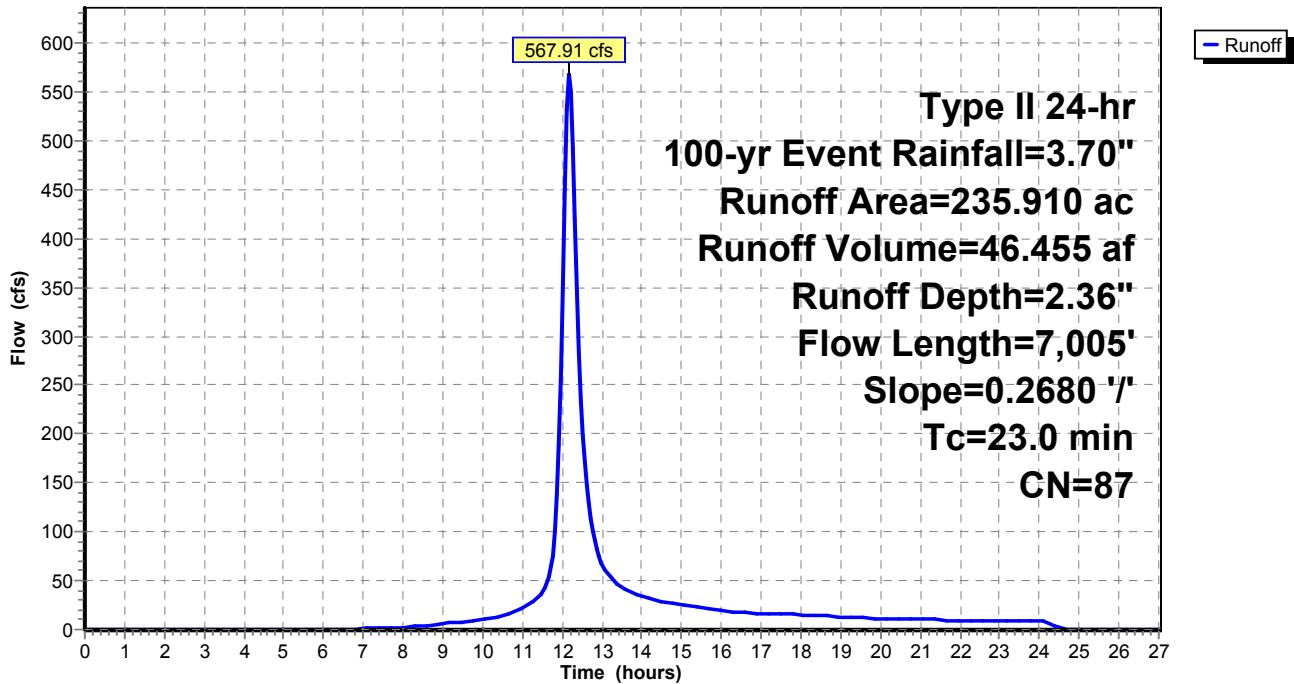
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 10: WS 10

Runoff = 663.73 cfs @ 12.25 hrs, Volume= 65.063 af, Depth= 2.36"

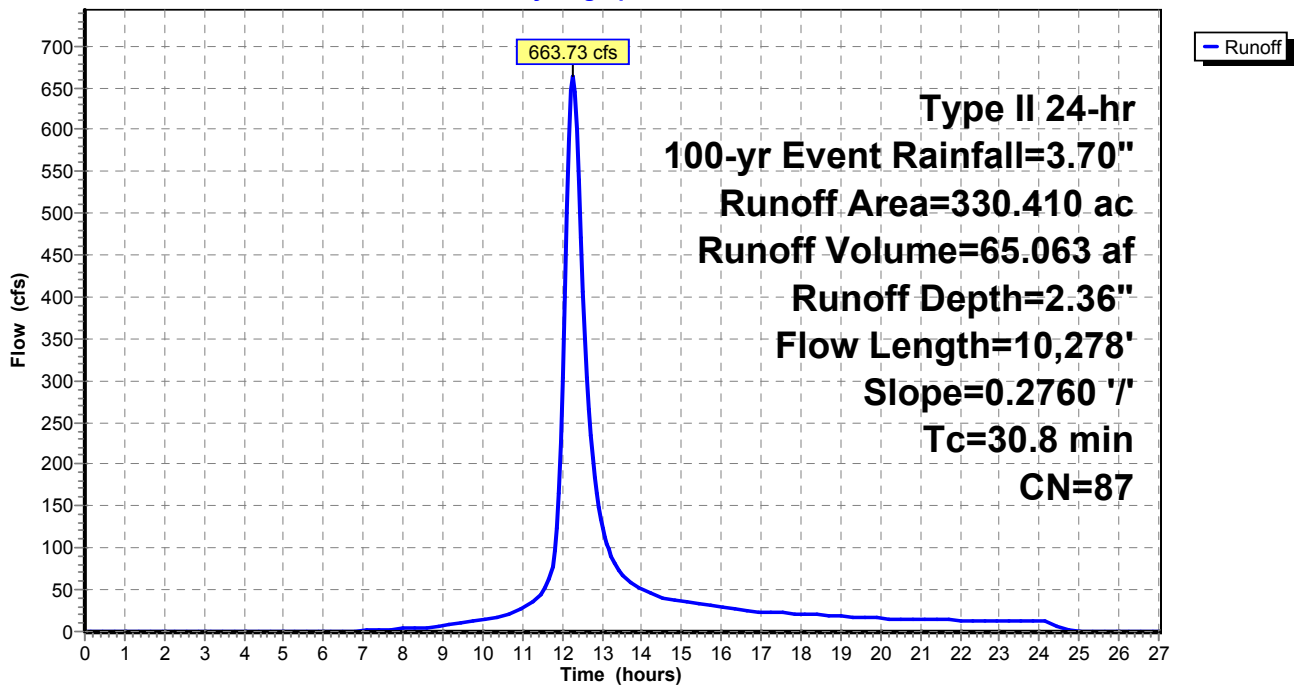
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 11: WS 11

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

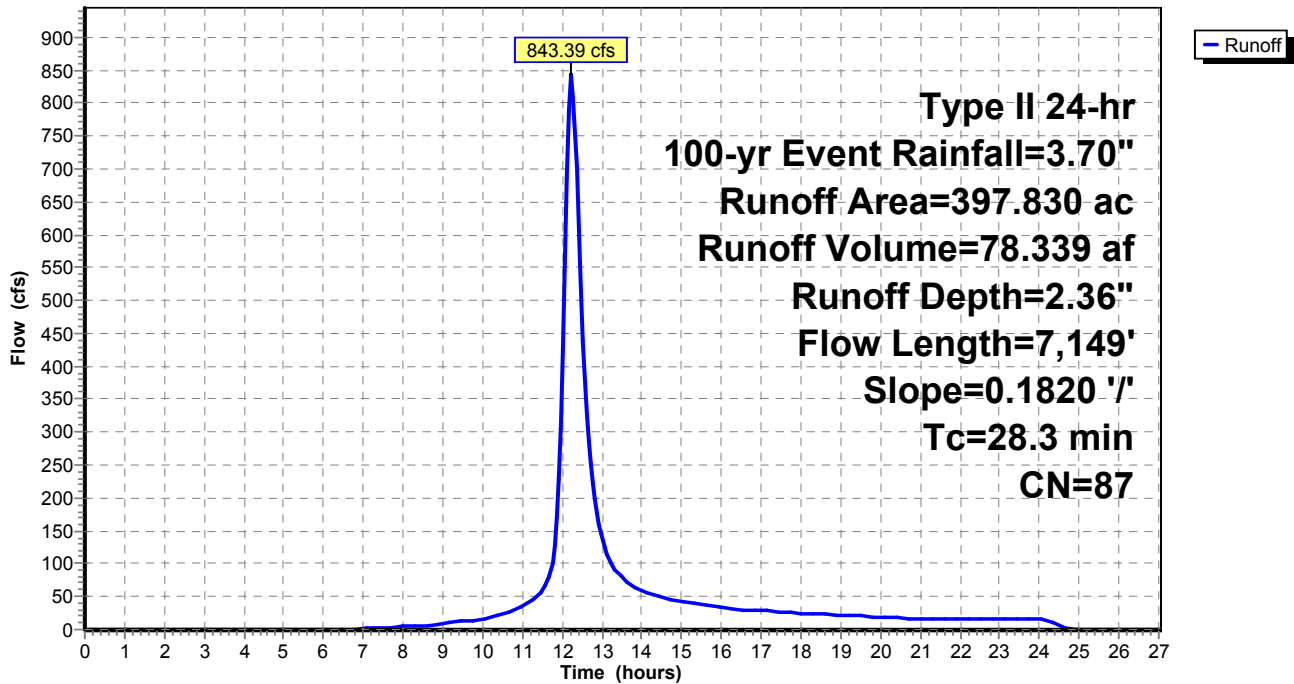
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 12: WS 12

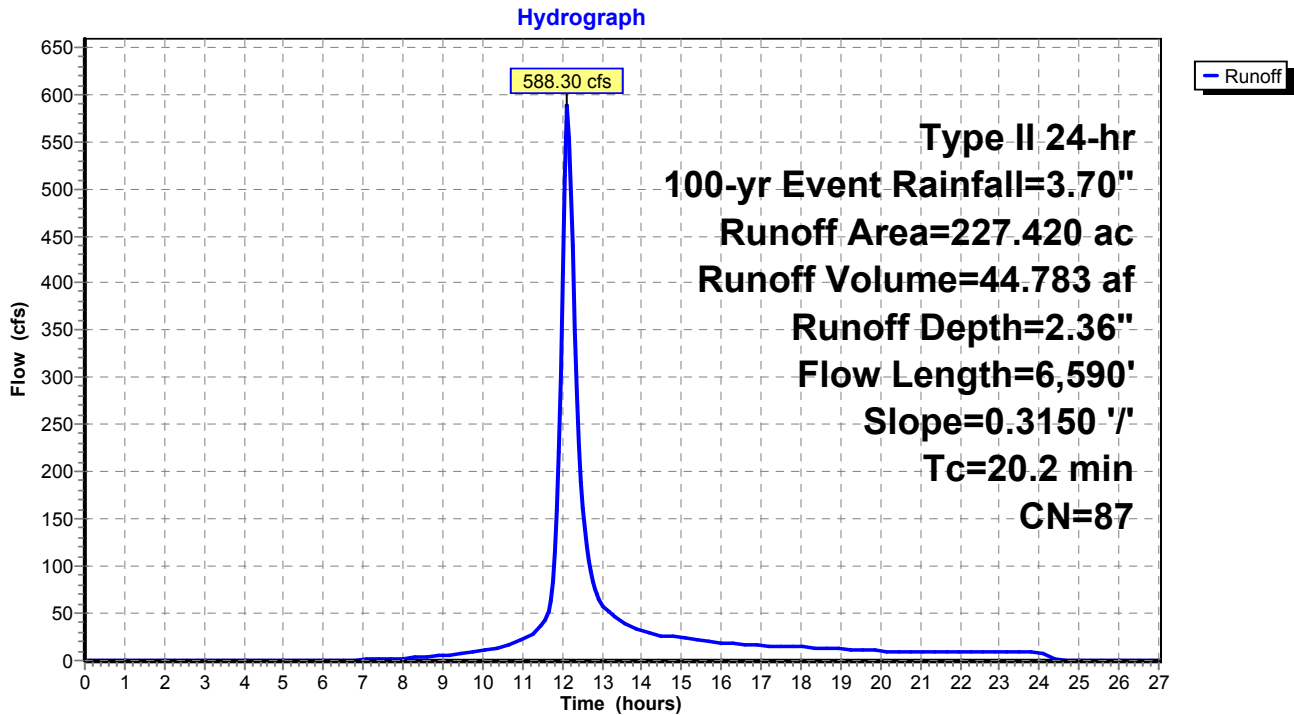
Runoff = 588.30 cfs @ 12.13 hrs, Volume= 44.783 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Subcatchment 13: WS 13

Runoff = 649.87 cfs @ 12.17 hrs, Volume= 54.252 af, Depth= 2.36"

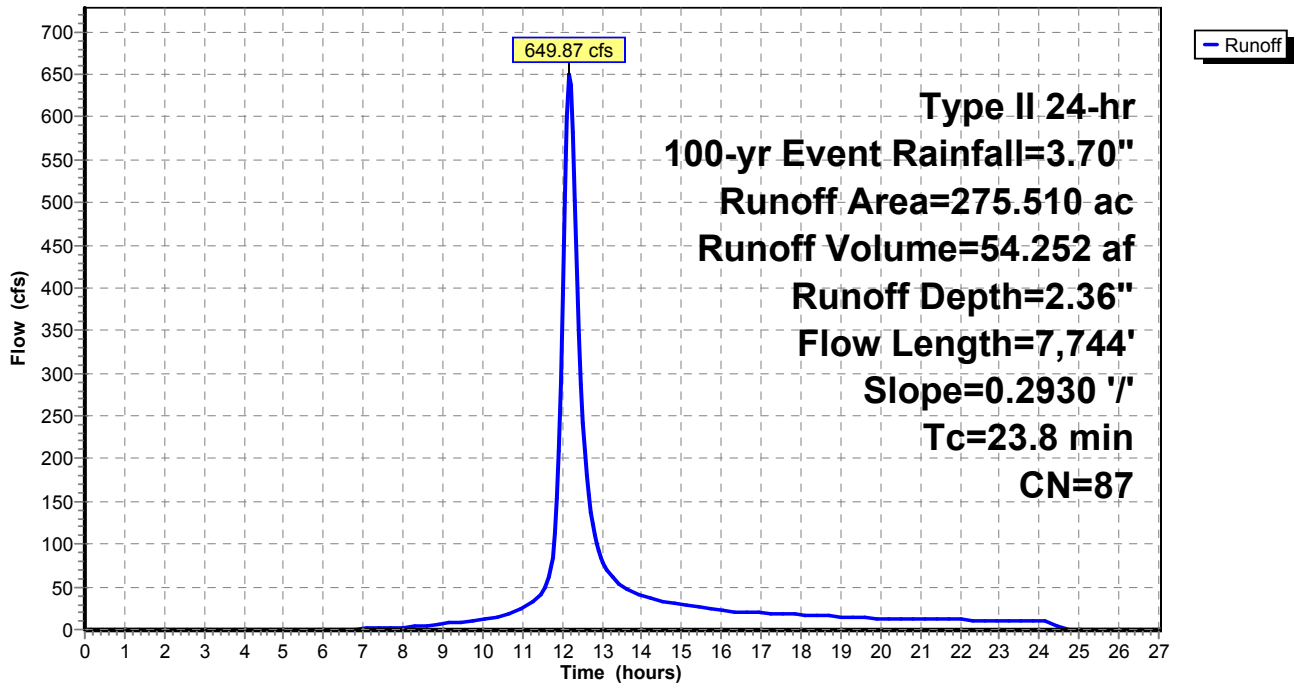
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Reach CHNL 1: Channel 1

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 2.36" for 100-yr Event event
 Inflow = 1,228.49 cfs @ 12.15 hrs, Volume= 99.035 af
 Outflow = 1,184.09 cfs @ 12.25 hrs, Volume= 99.033 af, Atten= 4%, Lag= 6.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 8.88 fps, Min. Travel Time= 3.6 min
 Avg. Velocity = 2.21 fps, Avg. Travel Time= 14.4 min

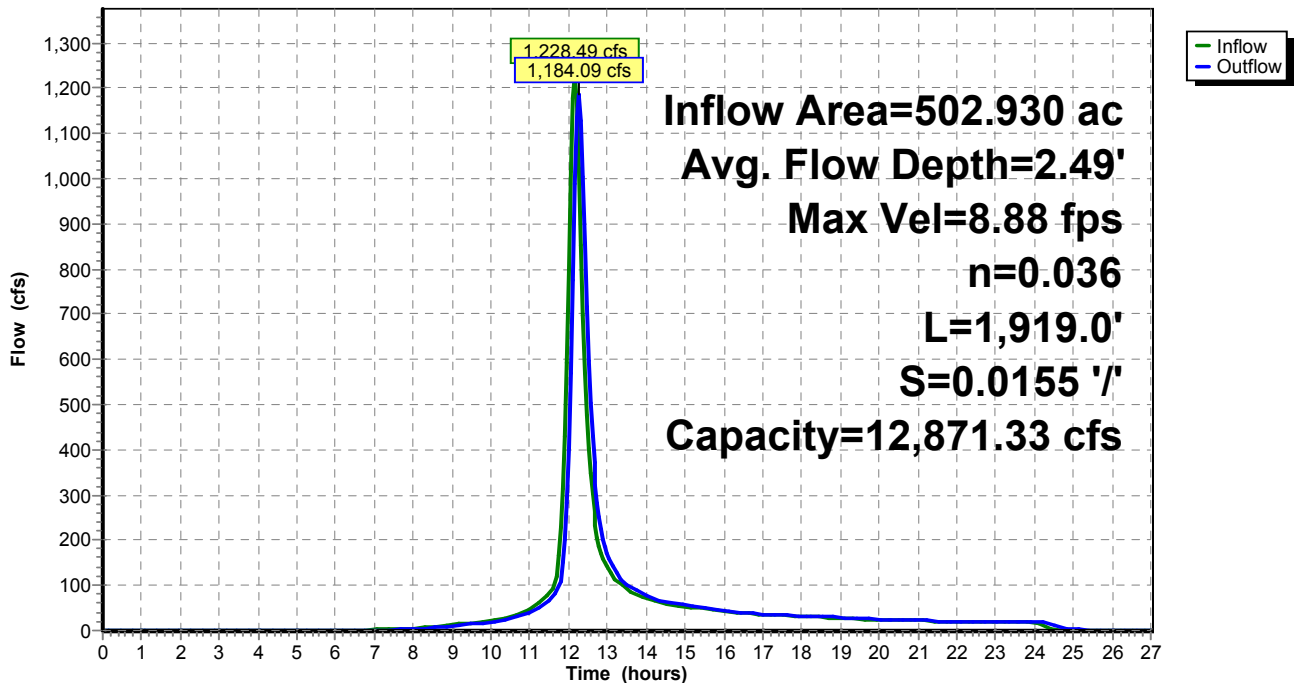
Peak Storage= 257,291 cf @ 12.19 hrs
 Average Depth at Peak Storage= 2.49'
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 80.00'
 Length= 1,919.0' Slope= 0.0155 '/'
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



Reach CHNL 1: Channel 1

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Reach CHNL 2: Channel 2

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 2.36" for 100-yr Event event
 Inflow = 1,933.74 cfs @ 12.14 hrs, Volume= 171.361 af
 Outflow = 1,565.27 cfs @ 12.49 hrs, Volume= 170.940 af, Atten= 19%, Lag= 21.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 9.03 fps, Min. Travel Time= 13.0 min
 Avg. Velocity = 2.51 fps, Avg. Travel Time= 46.9 min

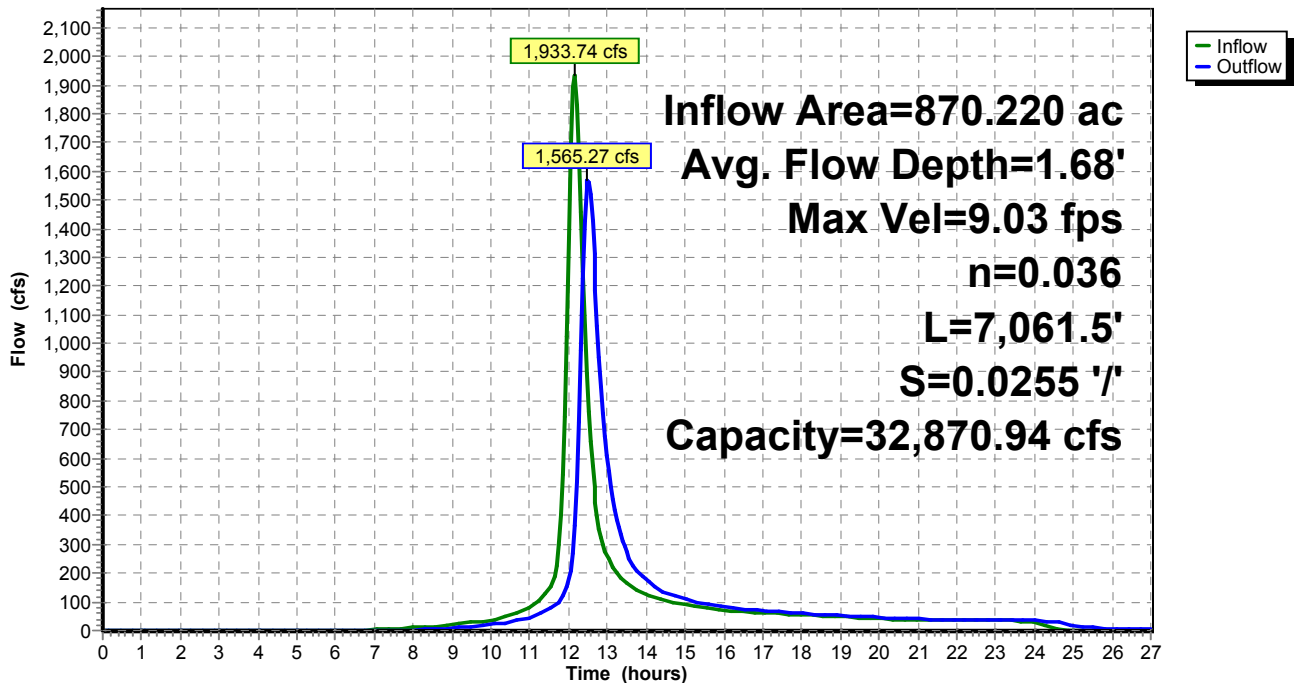
Peak Storage= 1,228,925 cf @ 12.27 hrs
 Average Depth at Peak Storage= 1.68'
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 2.5 2.0 ' / ' Top Width= 145.00'
 Length= 7,061.5' Slope= 0.0255 ' / '
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



Reach CHNL 2: Channel 2

Hydrograph



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Summary for Reach CHNL 3: Channel 3

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 3.53' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 3.70' @ 12.40 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event
 Inflow = 3,580.90 cfs @ 12.29 hrs, Volume= 439.487 af
 Outflow = 3,520.63 cfs @ 12.42 hrs, Volume= 439.082 af, Atten= 2%, Lag= 7.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 13.90 fps, Min. Travel Time= 4.4 min
 Avg. Velocity = 4.18 fps, Avg. Travel Time= 14.7 min

Peak Storage= 935,003 cf @ 12.35 hrs
 Average Depth at Peak Storage= 5.30'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 3,680.5' Slope= 0.0162 '/'
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

Prepared by M3 Engineering

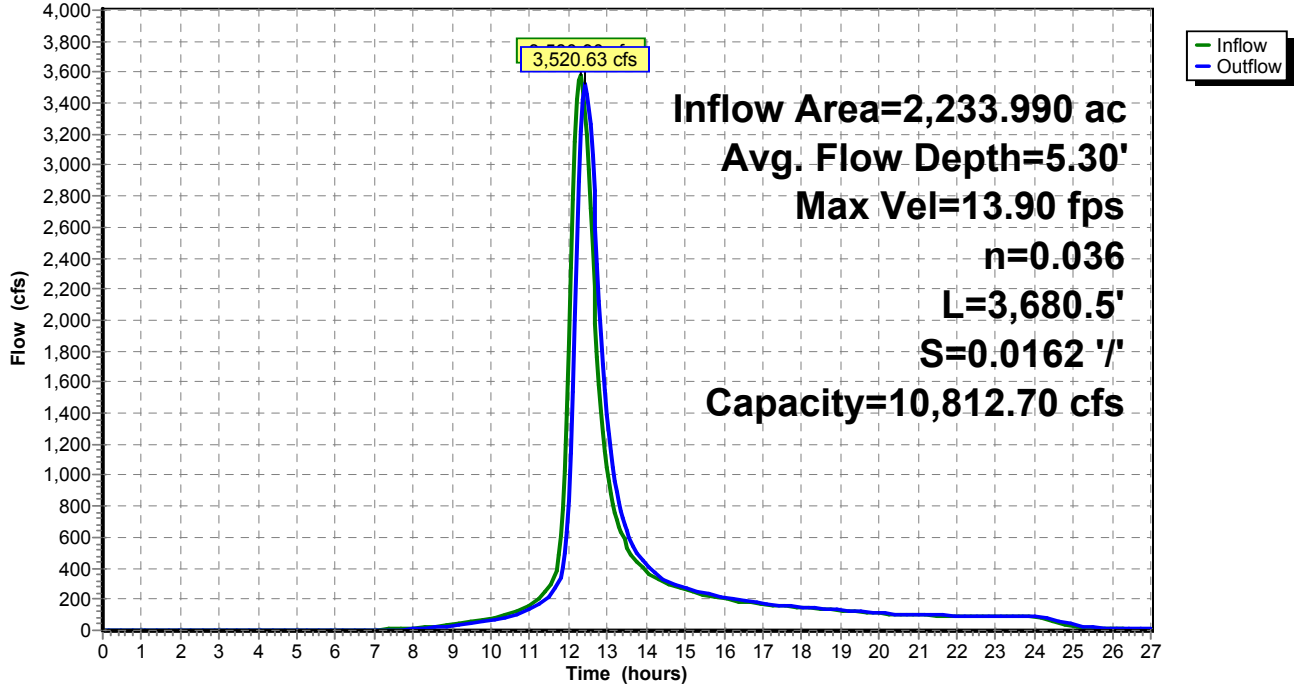
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Reach CHNL 3: Channel 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Reach CHNL 4: Channel 4

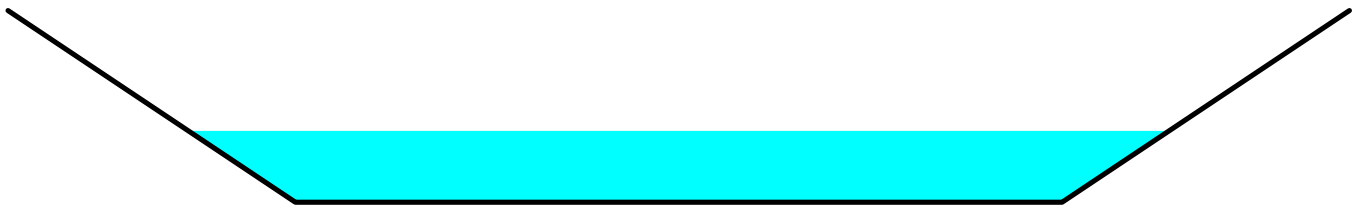
[61] Hint: Exceeded Reach CHNL 3 outlet invert by 3.72' @ 12.45 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event
 Inflow = 3,535.74 cfs @ 12.42 hrs, Volume= 445.972 af
 Outflow = 3,523.29 cfs @ 12.45 hrs, Volume= 445.885 af, Atten= 0%, Lag= 1.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 20.81 fps, Min. Travel Time= 0.9 min
 Avg. Velocity = 6.18 fps, Avg. Travel Time= 3.0 min

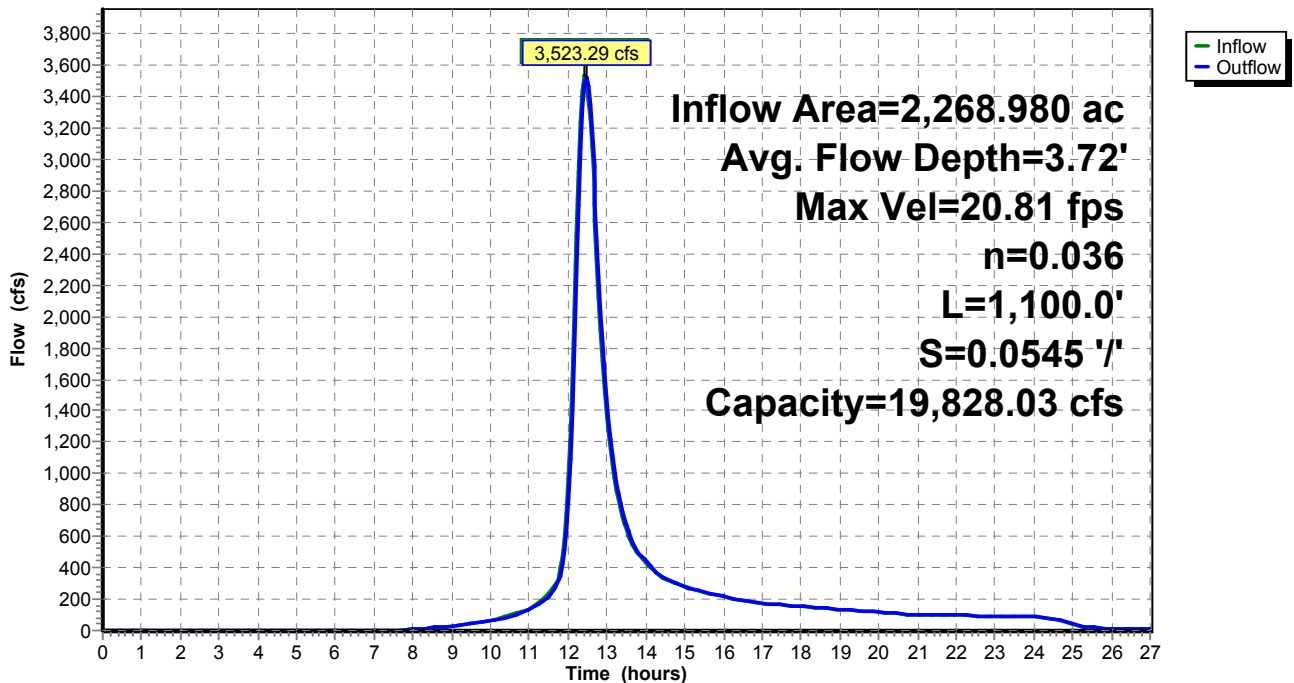
Peak Storage= 186,563 cf @ 12.43 hrs
 Average Depth at Peak Storage= 3.72'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 1,100.0' Slope= 0.0545 '/'
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



Reach CHNL 4: Channel 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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Summary for Reach CHNL 5: Channel 5

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.45' @ 12.55 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event
 Inflow = 3,572.05 cfs @ 12.44 hrs, Volume= 466.870 af
 Outflow = 3,552.39 cfs @ 12.51 hrs, Volume= 466.654 af, Atten= 1%, Lag= 3.8 min

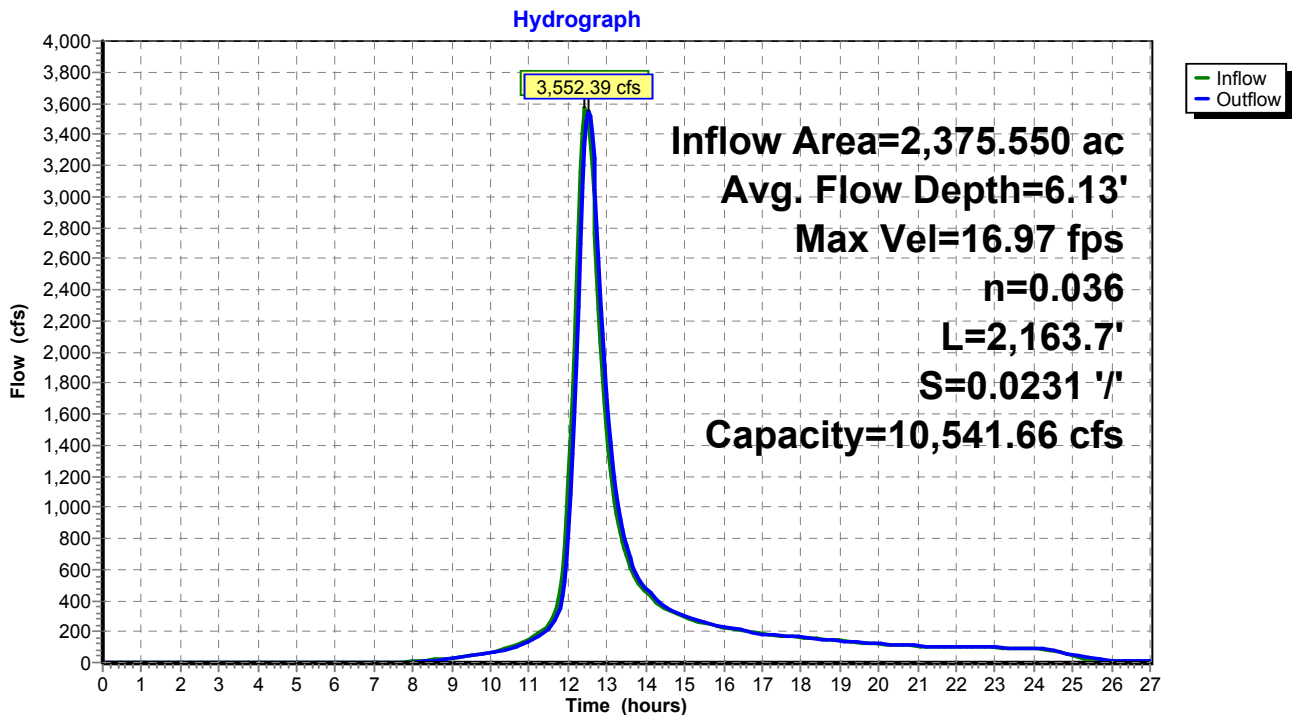
Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 16.97 fps, Min. Travel Time= 2.1 min
 Avg. Velocity = 5.55 fps, Avg. Travel Time= 6.5 min

Peak Storage= 453,859 cf @ 12.47 hrs
 Average Depth at Peak Storage= 6.13'
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 58.00'
 Length= 2,163.7' Slope= 0.0231 '/'
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



Reach CHNL 5: Channel 5



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Summary for Pond CLVT 1: Culvert Crossing #1

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 14.44' @ 12.75 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event
 Inflow = 3,552.39 cfs @ 12.51 hrs, Volume= 466.654 af
 Outflow = 3,058.70 cfs @ 12.70 hrs, Volume= 466.511 af, Atten= 14%, Lag= 11.4 min
 Primary = 3,058.70 cfs @ 12.70 hrs, Volume= 466.511 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,389.78' @ 12.70 hrs Surf.Area= 0 sf Storage= 1,214,495 cf
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 3.5 min calculated for 465.649 af (100% of inflow)
 Center-of-Mass det. time= 3.3 min (865.7 - 862.5)

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	Culvert Crossing #1 Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

Exist WS (Post-Quintana)_ Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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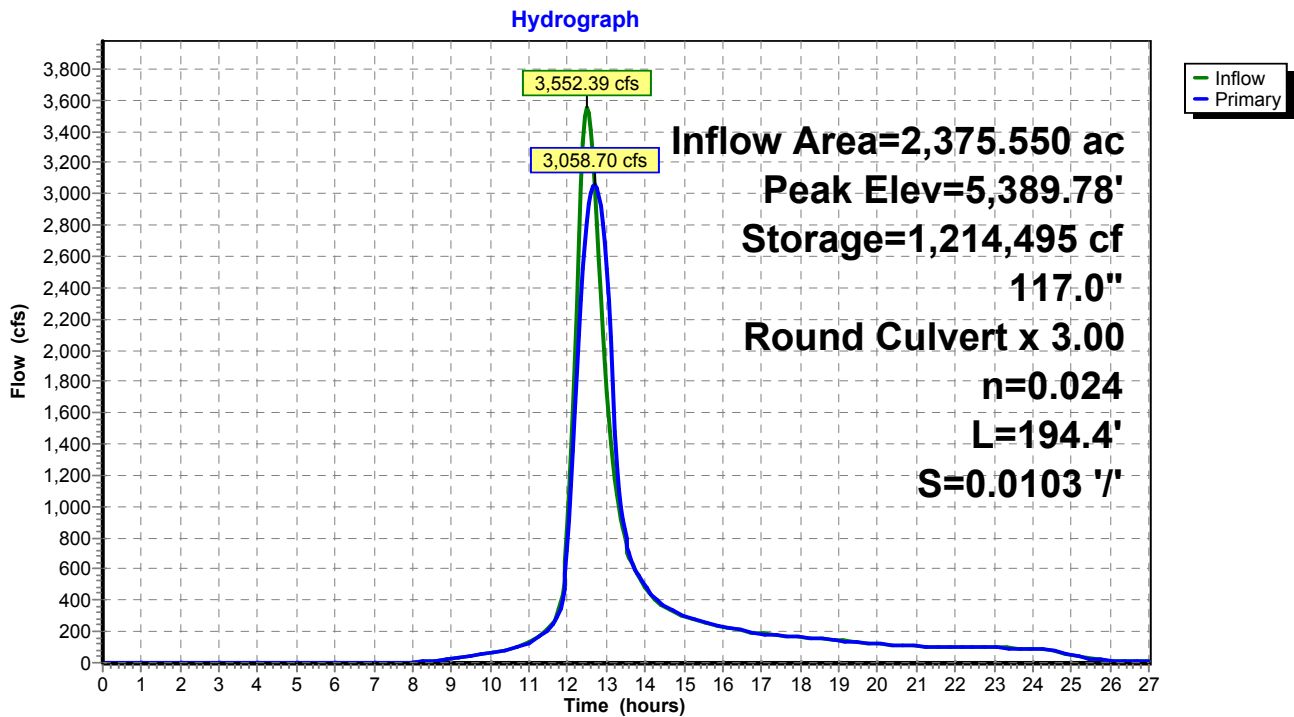
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	117.0" Round Culvert X 3.00 L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=3,057.57 cfs @ 12.70 hrs HW=5,389.77' (Free Discharge)
 ←1=Culvert (Inlet Controls 3,057.57 cfs @ 13.65 fps)

Pond CLVT 1: Culvert Crossing #1



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Summary for Pond CLVT 2: Culvert Crossing #2

[81] Warning: Exceeded Pond CLVT 1 by 7.40' @ 13.35 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event
 Inflow = 3,066.00 cfs @ 12.70 hrs, Volume= 472.066 af
 Outflow = 1,545.71 cfs @ 13.19 hrs, Volume= 472.015 af, Atten= 50%, Lag= 29.9 min
 Primary = 1,545.71 cfs @ 13.19 hrs, Volume= 472.015 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,386.54' @ 13.19 hrs Surf.Area= 0 sf Storage= 5,175,763 cf
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 26.2 min calculated for 471.143 af (100% of inflow)
 Center-of-Mass det. time= 26.0 min (891.1 - 865.1)

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	Existing Pond Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 100-yr Event Rainfall=3.70"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

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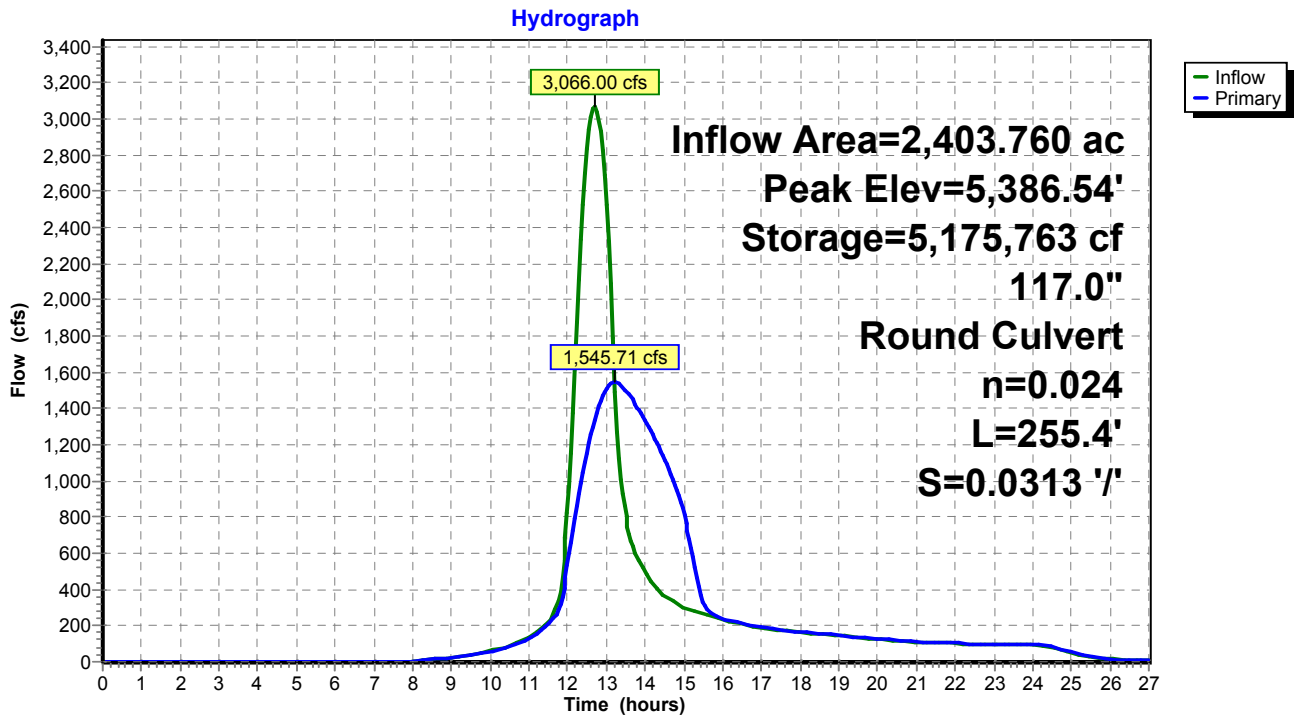
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5,402.00 9,814,540
 5,403.00 10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	117.0" Round Culvert L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=1,545.45 cfs @ 13.19 hrs HW=5,386.53' (Free Discharge)
 ←**1=Culvert** (Inlet Controls 1,545.45 cfs @ 20.70 fps)

Pond CLVT 2: Culvert Crossing #2



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.72" Tc=5.0 min CN=87 Runoff=133.97 cfs 6.393 af
Subcatchment2: WS 2	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=415.35 cfs 24.150 af
Subcatchment3: WS 3	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=148.20 cfs 7.929 af
Subcatchment4: WS 4	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=297.56 cfs 17.000 af
Subcatchment5: WS 5	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=392.70 cfs 28.143 af
Subcatchment6: WS 6	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=888.50 cfs 75.058 af
Subcatchment7: WS 7	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=471.20 cfs 32.739 af
Subcatchment8: WS 8	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=334.29 cfs 20.851 af
Subcatchment9: WS 9	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=651.70 cfs 53.460 af
Subcatchment10: WS 10	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=762.30 cfs 74.875 af
Subcatchment11: WS 11	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=968.42 cfs 90.153 af
Subcatchment12: WS 12	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=674.80 cfs 51.536 af
Subcatchment13: WS 13	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=745.85 cfs 62.434 af
Reach CHNL 1: Channel 1	Avg. Flow Depth=2.71' Max Vel=9.34 fps Inflow=1,409.59 cfs 113.970 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,364.28 cfs 113.968 af
Reach CHNL 2: Channel 2	Avg. Flow Depth=1.84' Max Vel=9.59 fps Inflow=2,220.40 cfs 197.202 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=1,835.68 cfs 196.763 af
Reach CHNL 3: Channel 3	Avg. Flow Depth=5.83' Max Vel=14.67 fps Inflow=4,230.35 cfs 505.807 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=4,159.08 cfs 505.394 af

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Reach CHNL 4: Channel 4 Avg. Flow Depth=4.10' Max Vel=22.02 fps Inflow=4,176.49 cfs 513.323 af
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=4,160.94 cfs 513.234 af

Reach CHNL 5: Channel 5 Avg. Flow Depth=6.72' Max Vel=17.83 fps Inflow=4,217.67 cfs 537.384 af
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=4,191.95 cfs 537.163 af

Pond CLVT 1: Culvert Crossing Peak Elev=5,392.92' Storage=1,758,861 cf Inflow=4,191.95 cfs 537.163 af
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,410.14 cfs 537.020 af

Pond CLVT 2: Culvert Crossing Peak Elev=5,391.04' Storage=6,444,314 cf Inflow=3,418.33 cfs 543.413 af
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,658.81 cfs 543.361 af

Total Runoff Area = 2,403.760 ac Runoff Volume = 544.720 af Average Runoff Depth = 2.72"
90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

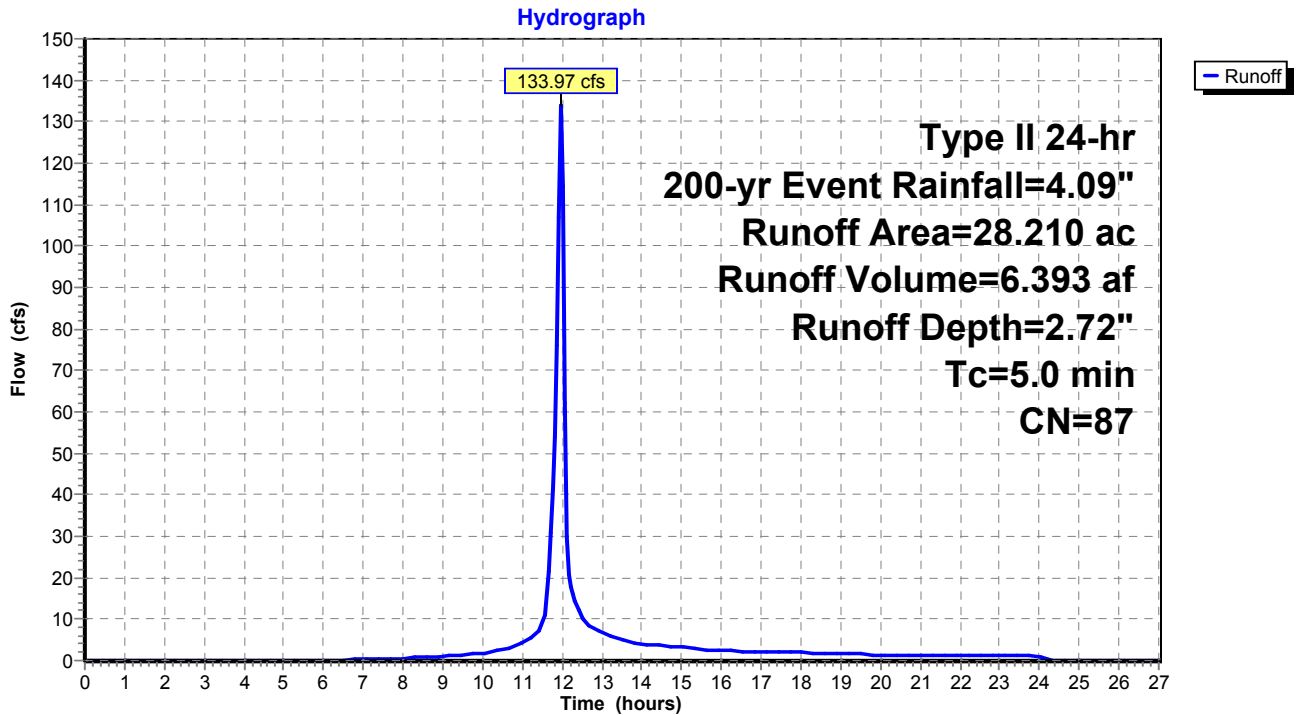
Runoff = 133.97 cfs @ 11.95 hrs, Volume= 6.393 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 2: WS 2

Runoff = 415.35 cfs @ 12.02 hrs, Volume= 24.150 af, Depth= 2.72"

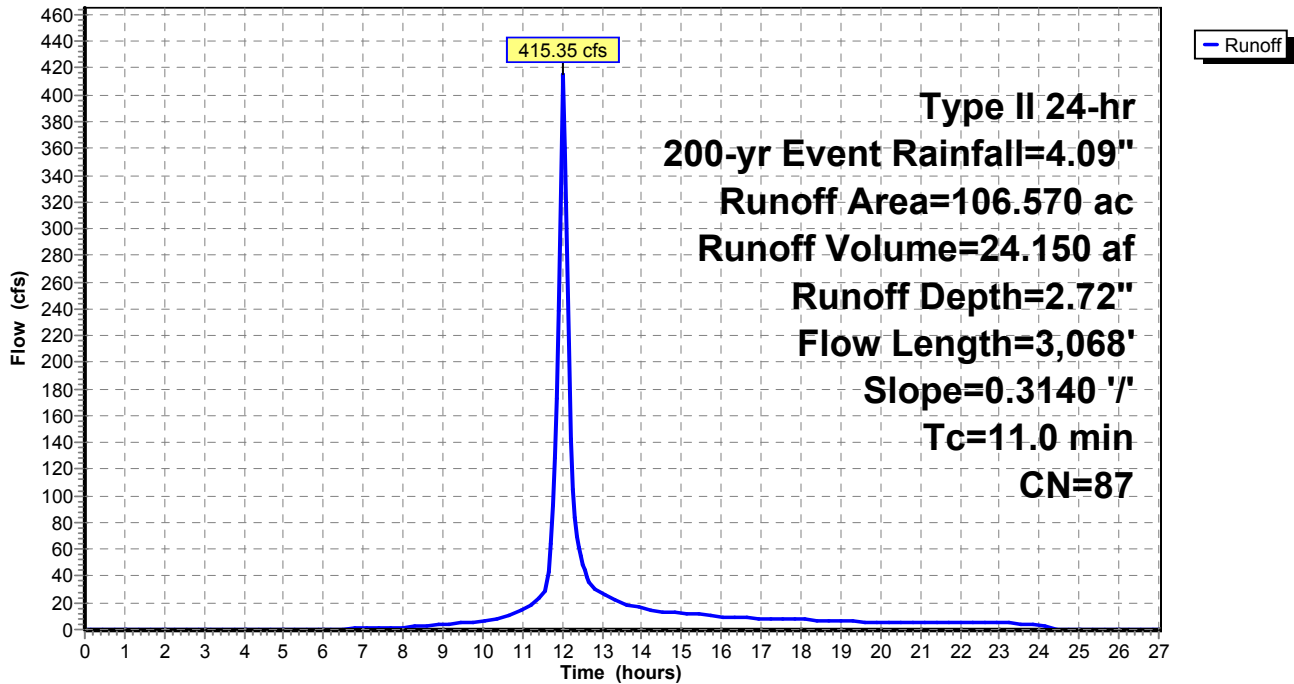
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 3: WS 3

Runoff = 148.20 cfs @ 12.00 hrs, Volume= 7.929 af, Depth= 2.72"

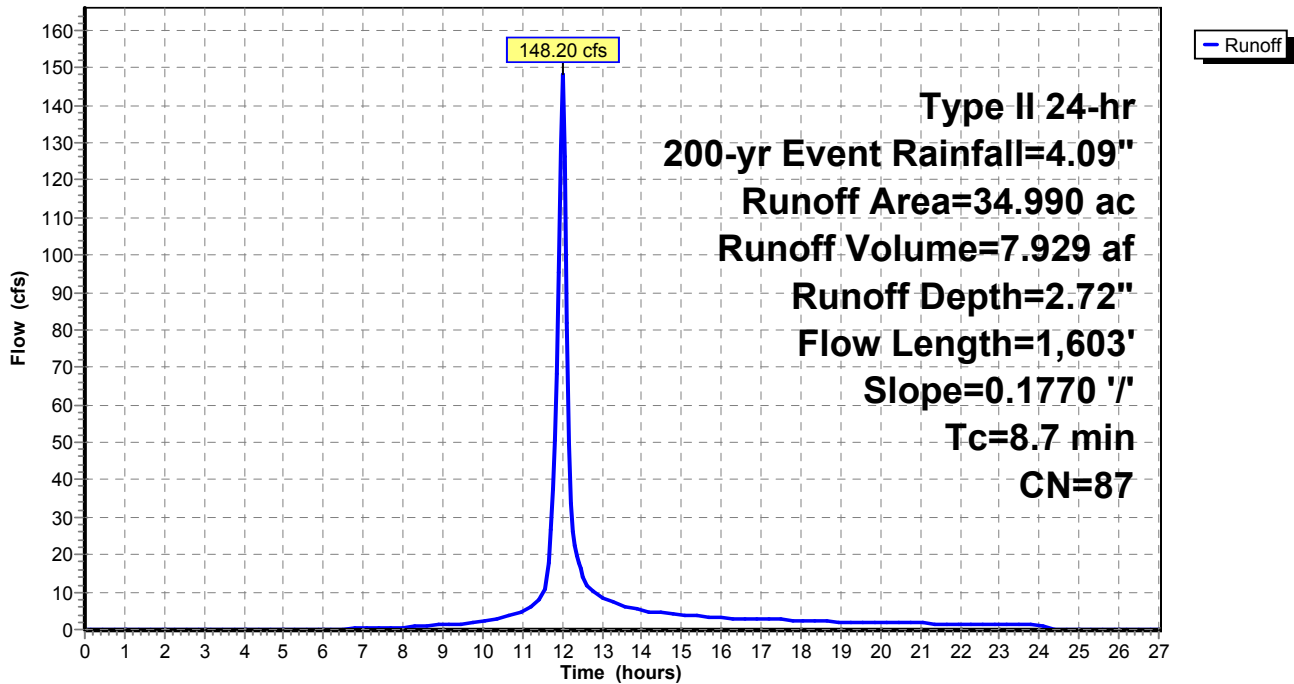
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 4: WS 4

Runoff = 297.56 cfs @ 12.02 hrs, Volume= 17.000 af, Depth= 2.72"

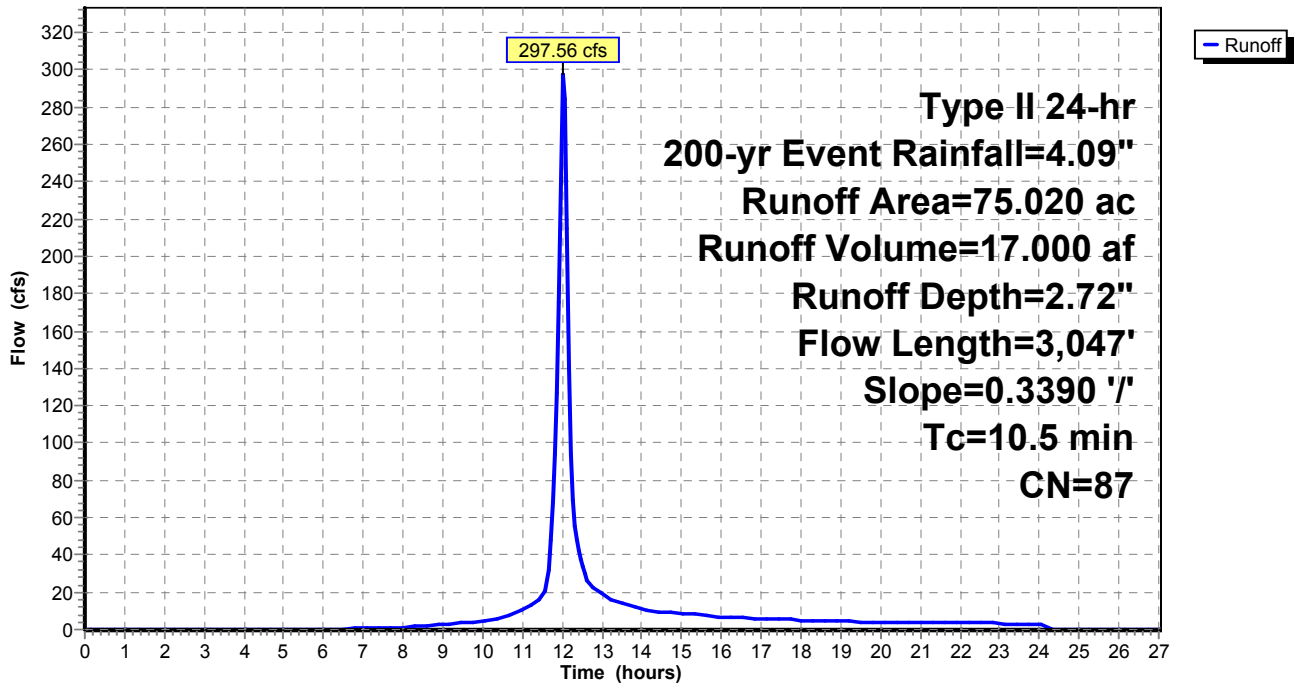
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 5: WS 5

Runoff = 392.70 cfs @ 12.10 hrs, Volume= 28.143 af, Depth= 2.72"

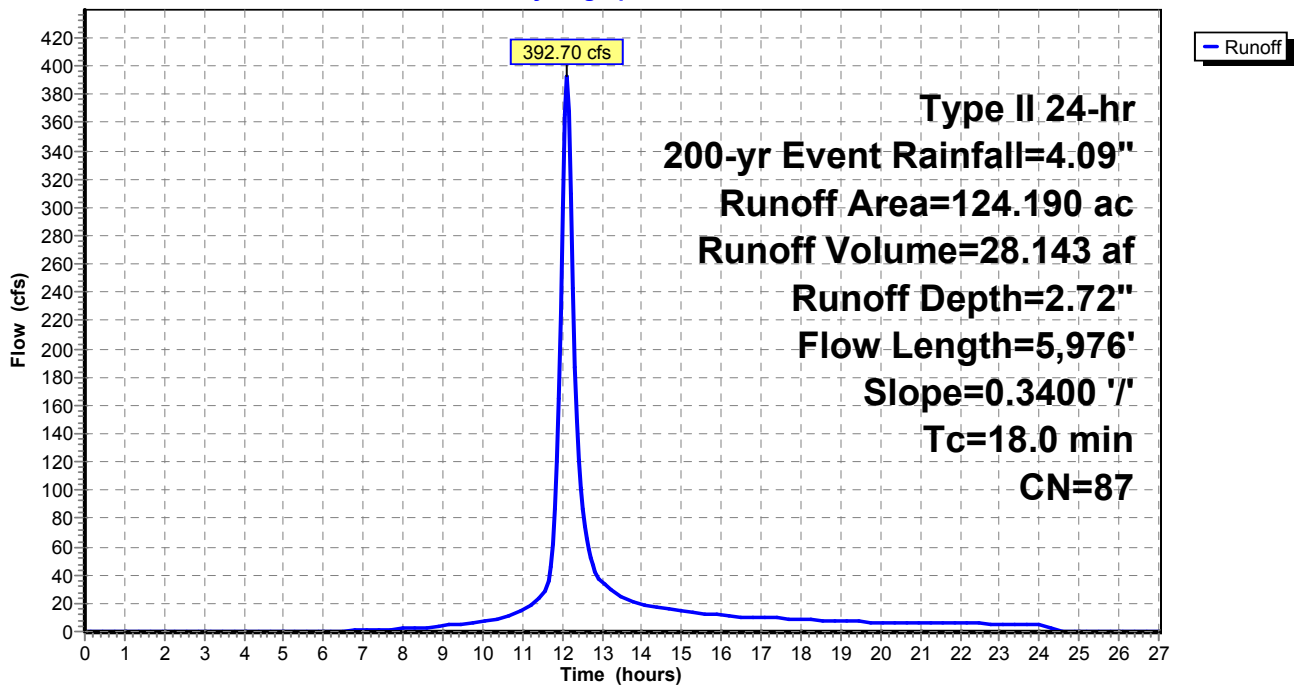
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 6: WS 6

Runoff = 888.50 cfs @ 12.17 hrs, Volume= 75.058 af, Depth= 2.72"

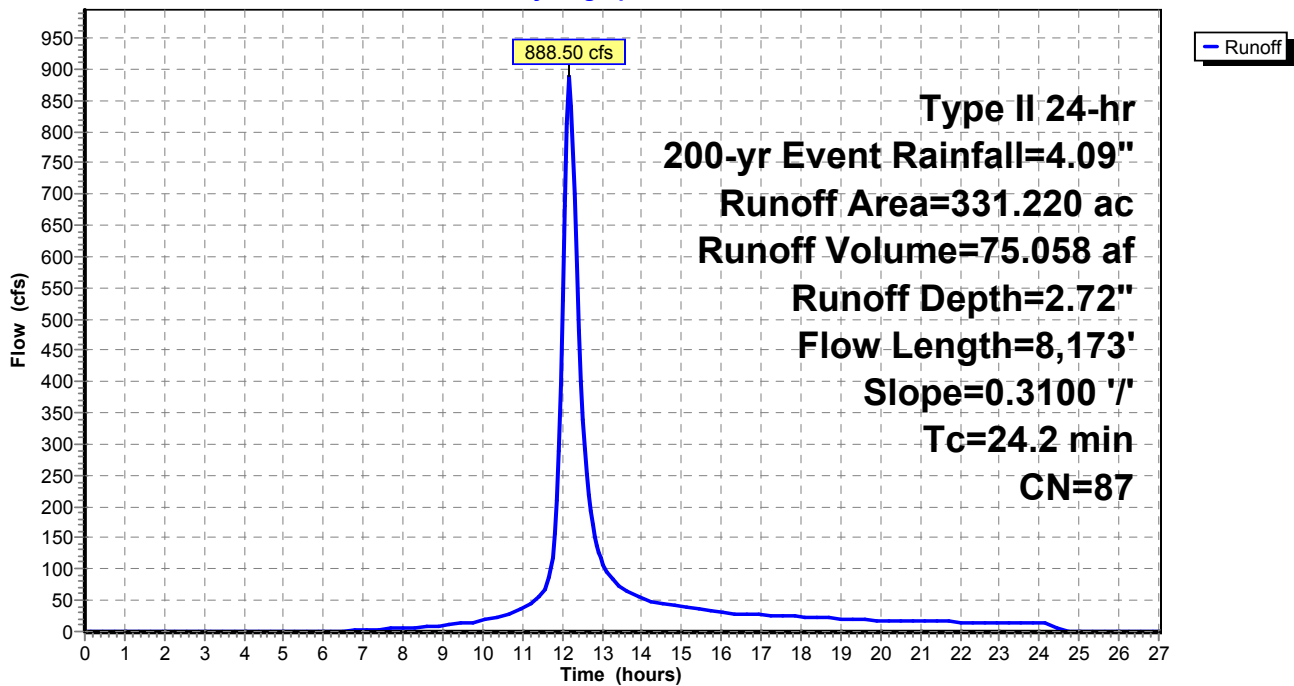
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 7: WS 7

Runoff = 471.20 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

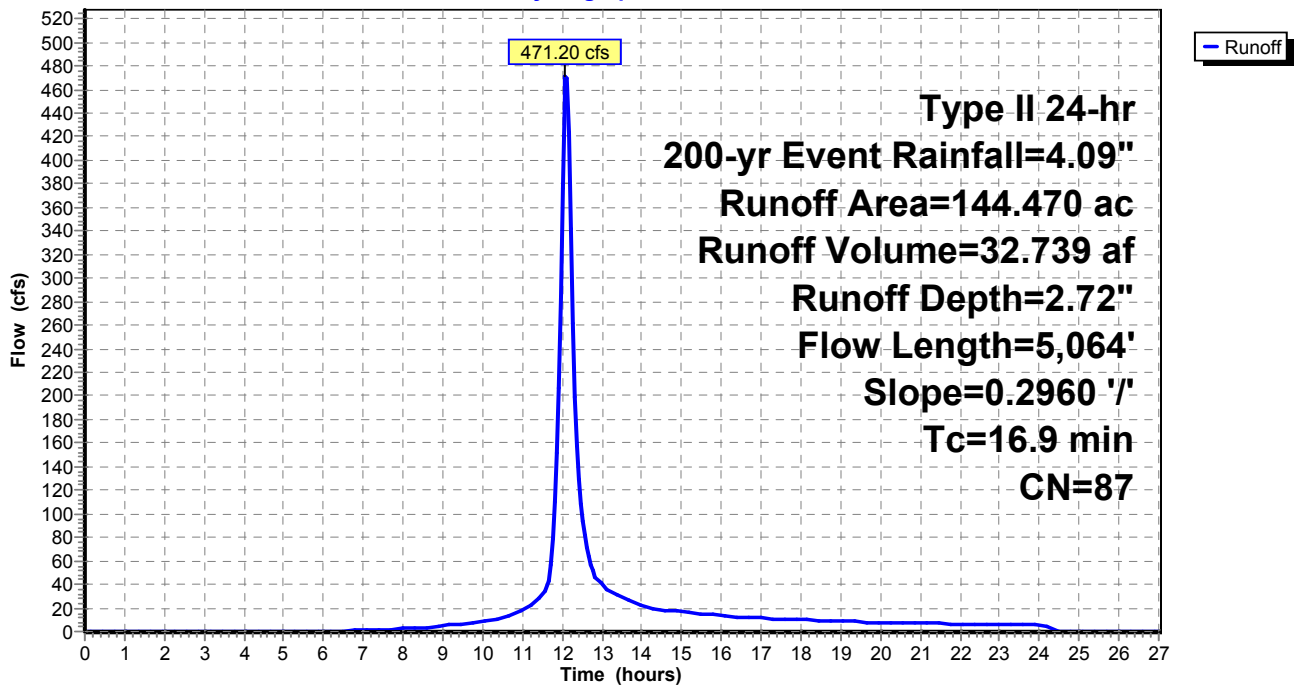
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 8: WS 8

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

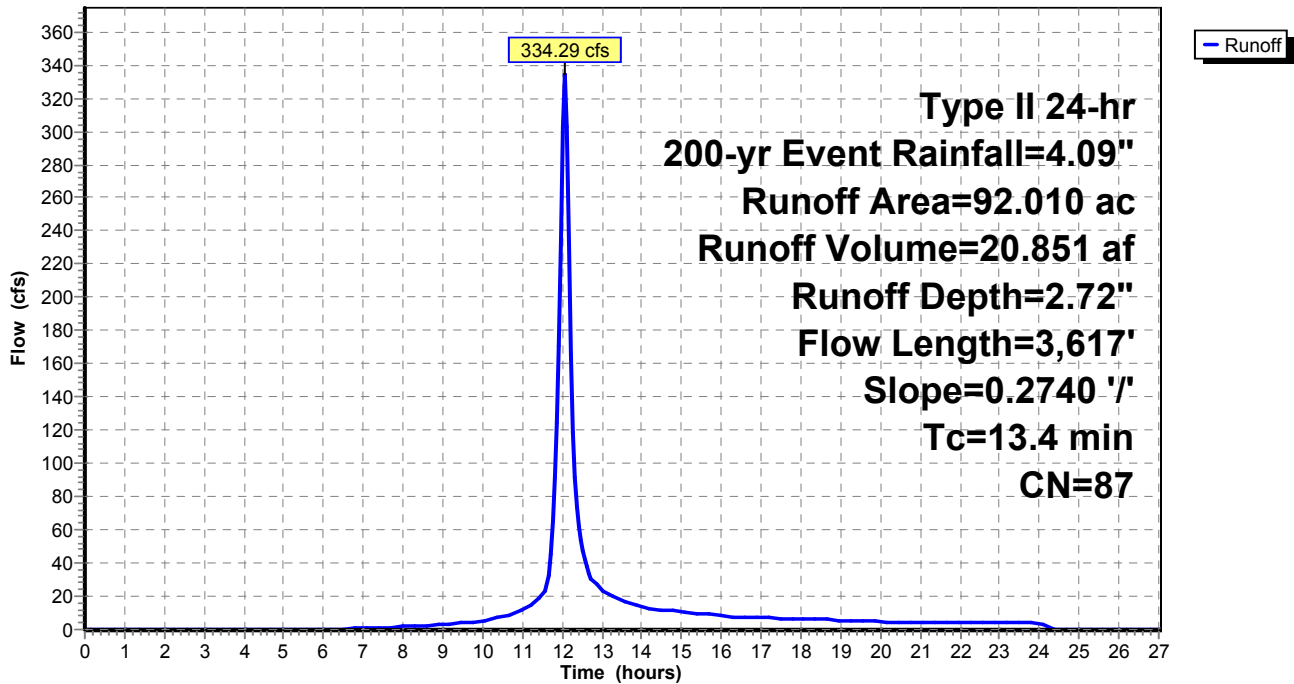
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

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Summary for Subcatchment 9: WS 9

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

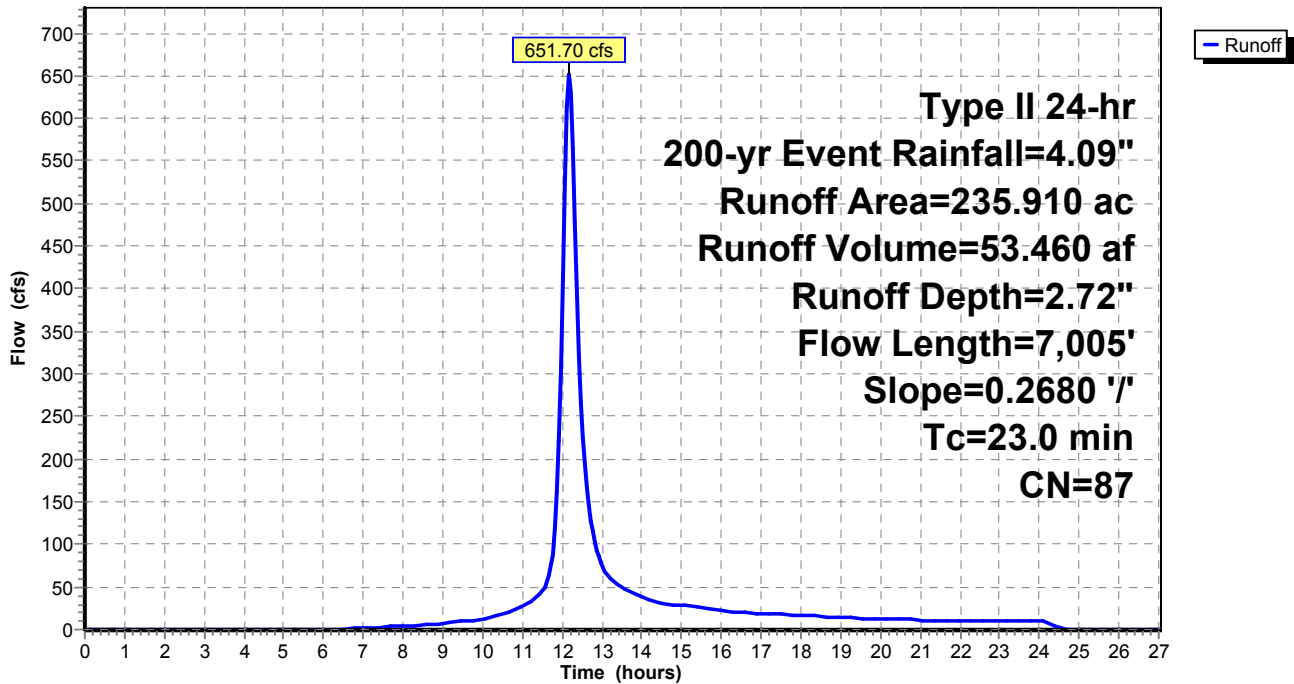
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 10: WS 10

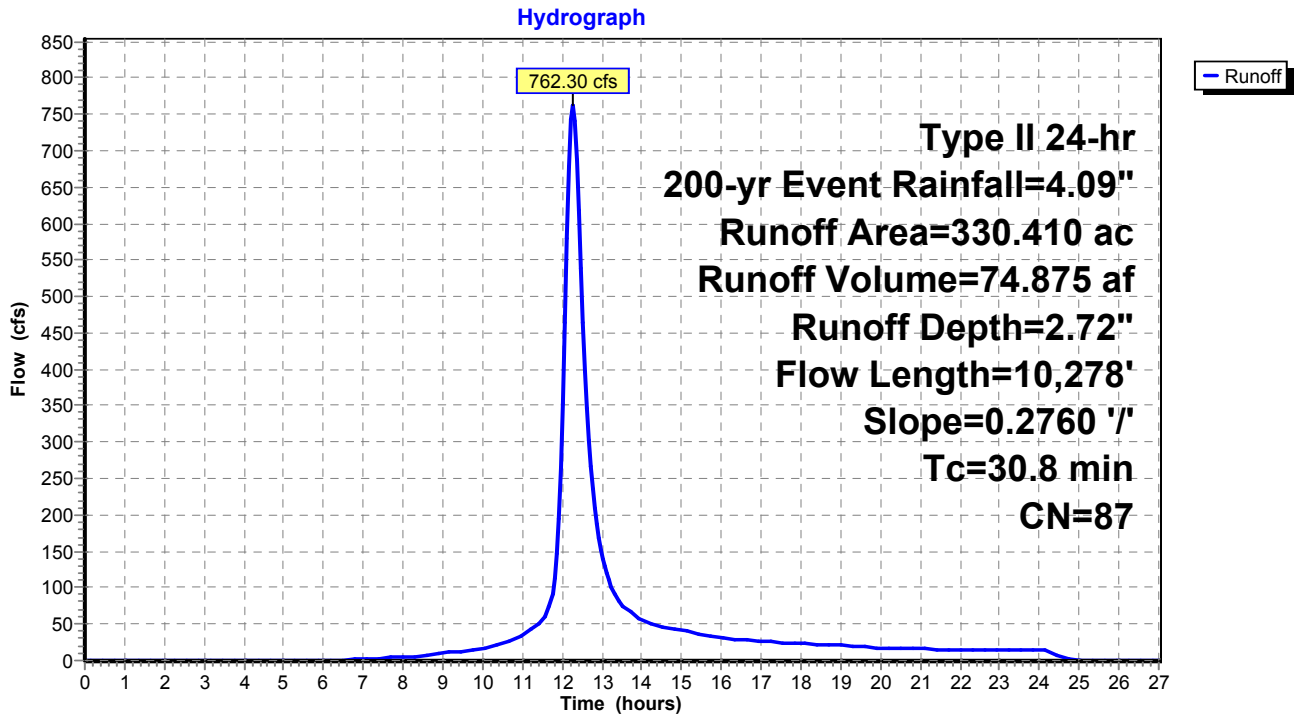
Runoff = 762.30 cfs @ 12.25 hrs, Volume= 74.875 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 11: WS 11

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

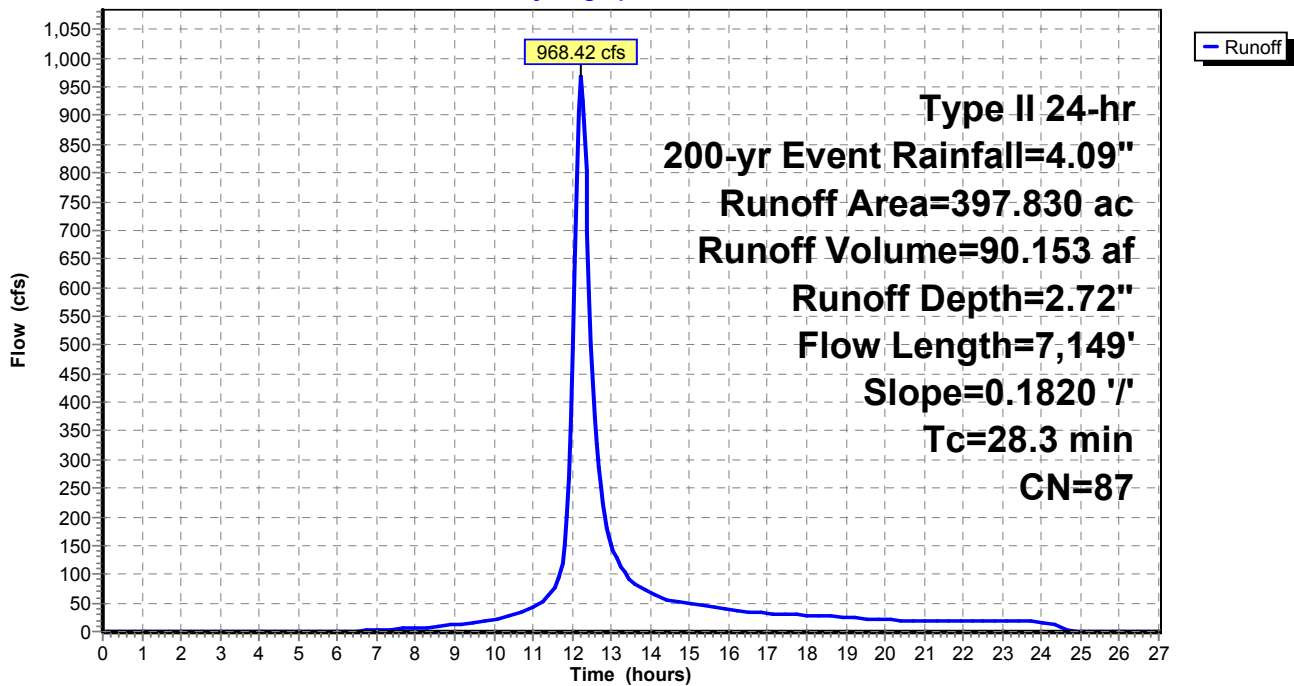
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 12: WS 12

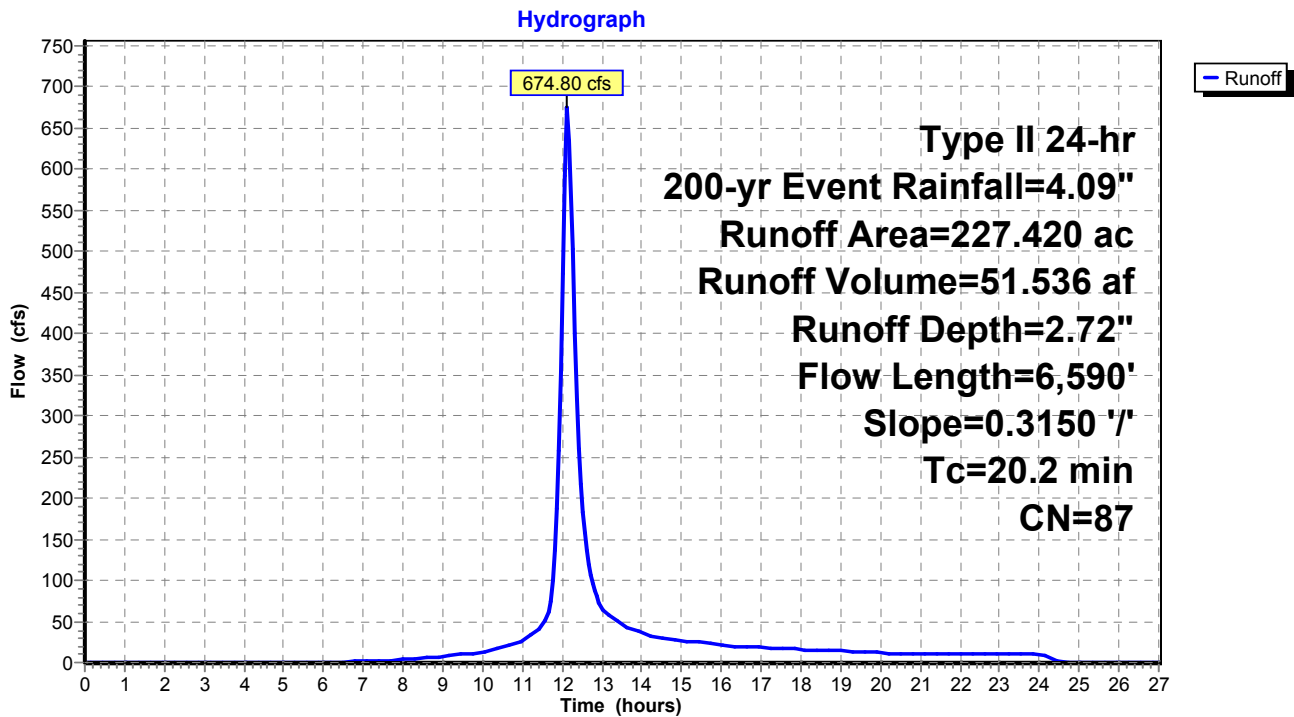
Runoff = 674.80 cfs @ 12.13 hrs, Volume= 51.536 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Subcatchment 13: WS 13

Runoff = 745.85 cfs @ 12.17 hrs, Volume= 62.434 af, Depth= 2.72"

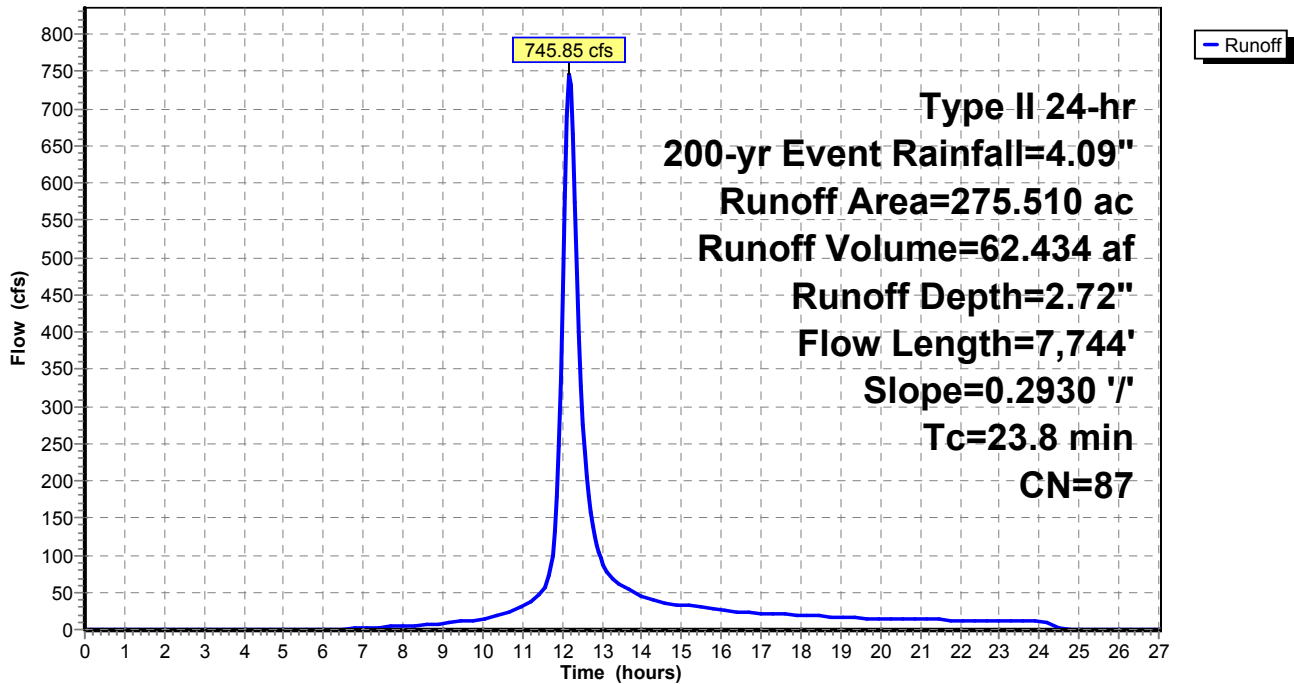
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Reach CHNL 1: Channel 1

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 2.72" for 200-yr Event event
 Inflow = 1,409.59 cfs @ 12.15 hrs, Volume= 113.970 af
 Outflow = 1,364.28 cfs @ 12.24 hrs, Volume= 113.968 af, Atten= 3%, Lag= 5.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 9.34 fps, Min. Travel Time= 3.4 min
 Avg. Velocity = 2.31 fps, Avg. Travel Time= 13.9 min

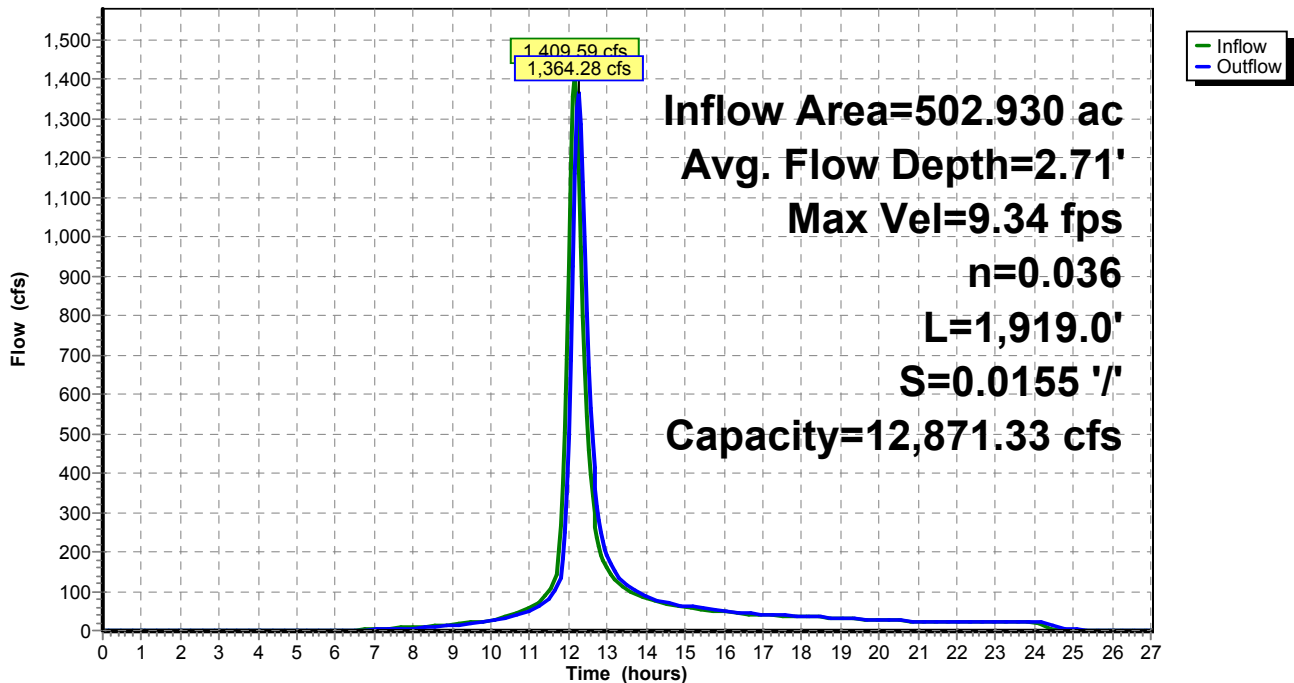
Peak Storage= 281,335 cf @ 12.18 hrs
 Average Depth at Peak Storage= 2.71'
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 ' / ' Top Width= 80.00'
 Length= 1,919.0' Slope= 0.0155 ' / '
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



Reach CHNL 1: Channel 1

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Reach CHNL 2: Channel 2

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 2.72" for 200-yr Event event
 Inflow = 2,220.40 cfs @ 12.14 hrs, Volume= 197.202 af
 Outflow = 1,835.68 cfs @ 12.47 hrs, Volume= 196.763 af, Atten= 17%, Lag= 20.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 9.59 fps, Min. Travel Time= 12.3 min
 Avg. Velocity = 2.61 fps, Avg. Travel Time= 45.2 min

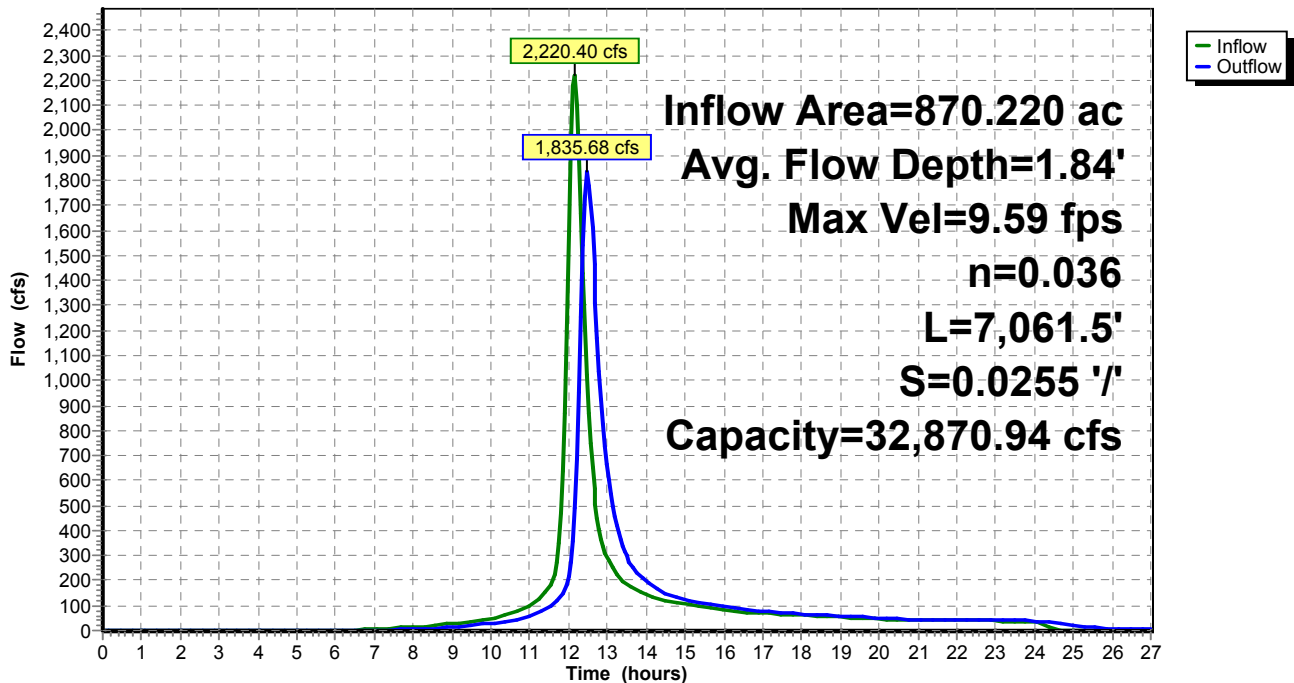
Peak Storage= 1,352,568 cf @ 12.27 hrs
 Average Depth at Peak Storage= 1.84'
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 2.5 2.0 ' / ' Top Width= 145.00'
 Length= 7,061.5' Slope= 0.0255 ' / '
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



Reach CHNL 2: Channel 2

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Summary for Reach CHNL 3: Channel 3

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 3.89' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 4.08' @ 12.40 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 2.72" for 200-yr Event event
 Inflow = 4,230.35 cfs @ 12.29 hrs, Volume= 505.807 af
 Outflow = 4,159.08 cfs @ 12.41 hrs, Volume= 505.394 af, Atten= 2%, Lag= 7.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 14.67 fps, Min. Travel Time= 4.2 min
 Avg. Velocity = 4.37 fps, Avg. Travel Time= 14.1 min

Peak Storage= 1,046,926 cf @ 12.34 hrs
 Average Depth at Peak Storage= 5.83'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 3,680.5' Slope= 0.0162 '/'
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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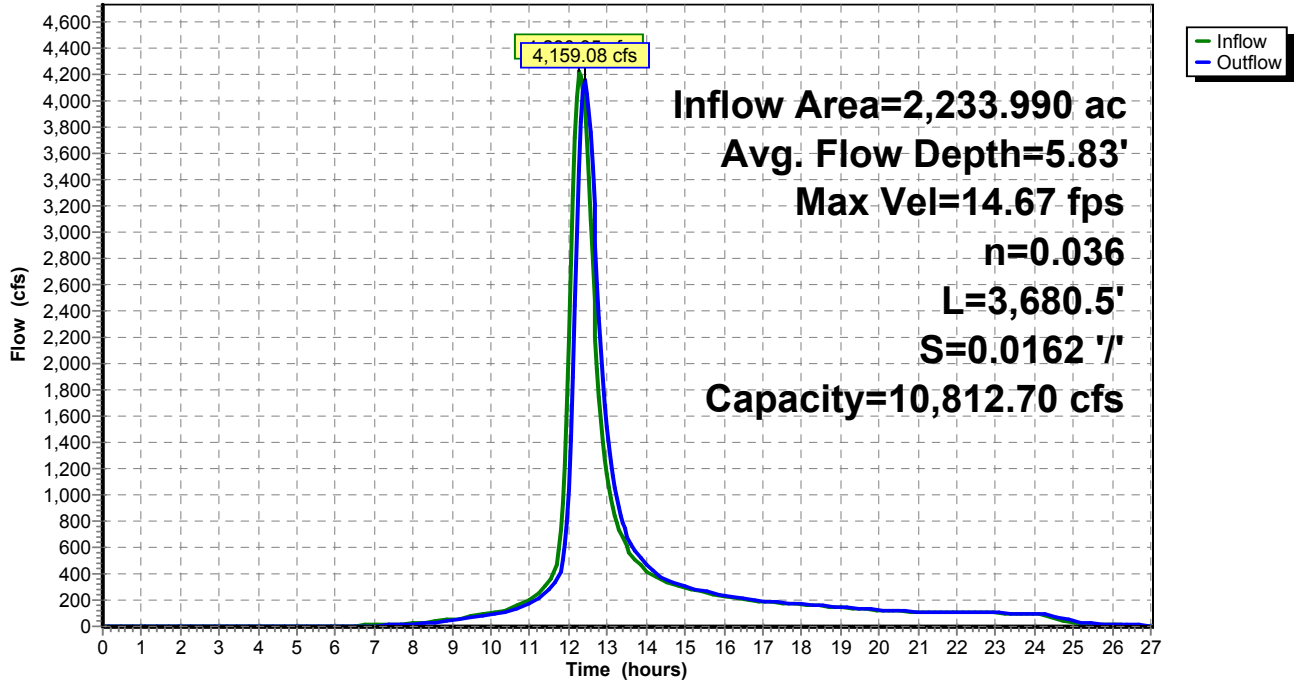
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Reach CHNL 3: Channel 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Reach CHNL 4: Channel 4

[61] Hint: Exceeded Reach CHNL 3 outlet invert by 4.10' @ 12.40 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event
 Inflow = 4,176.49 cfs @ 12.41 hrs, Volume= 513.323 af
 Outflow = 4,160.94 cfs @ 12.43 hrs, Volume= 513.234 af, Atten= 0%, Lag= 1.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 22.02 fps, Min. Travel Time= 0.8 min
 Avg. Velocity = 6.44 fps, Avg. Travel Time= 2.8 min

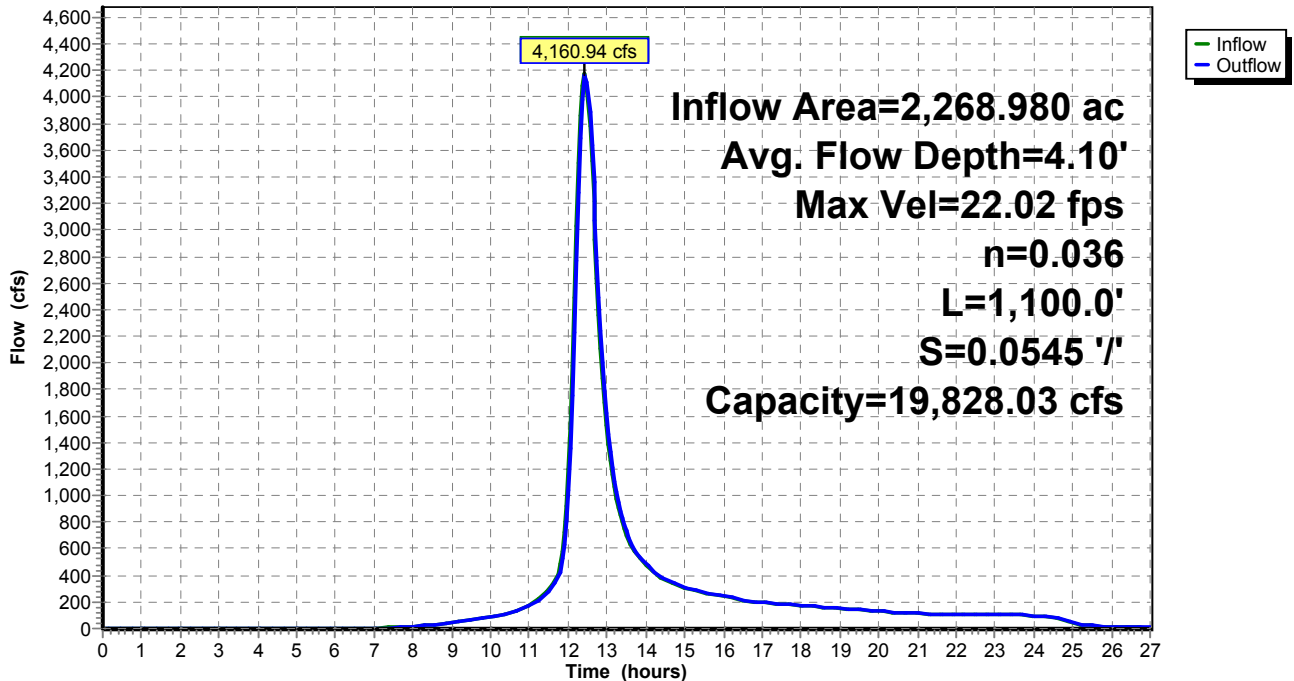
Peak Storage= 208,374 cf @ 12.42 hrs
 Average Depth at Peak Storage= 4.10'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 1,100.0' Slope= 0.0545 '/'
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



Reach CHNL 4: Channel 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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Summary for Reach CHNL 5: Channel 5

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.66' @ 12.50 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event
 Inflow = 4,217.67 cfs @ 12.43 hrs, Volume= 537.384 af
 Outflow = 4,191.95 cfs @ 12.49 hrs, Volume= 537.163 af, Atten= 1%, Lag= 3.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 17.83 fps, Min. Travel Time= 2.0 min
 Avg. Velocity = 5.77 fps, Avg. Travel Time= 6.2 min

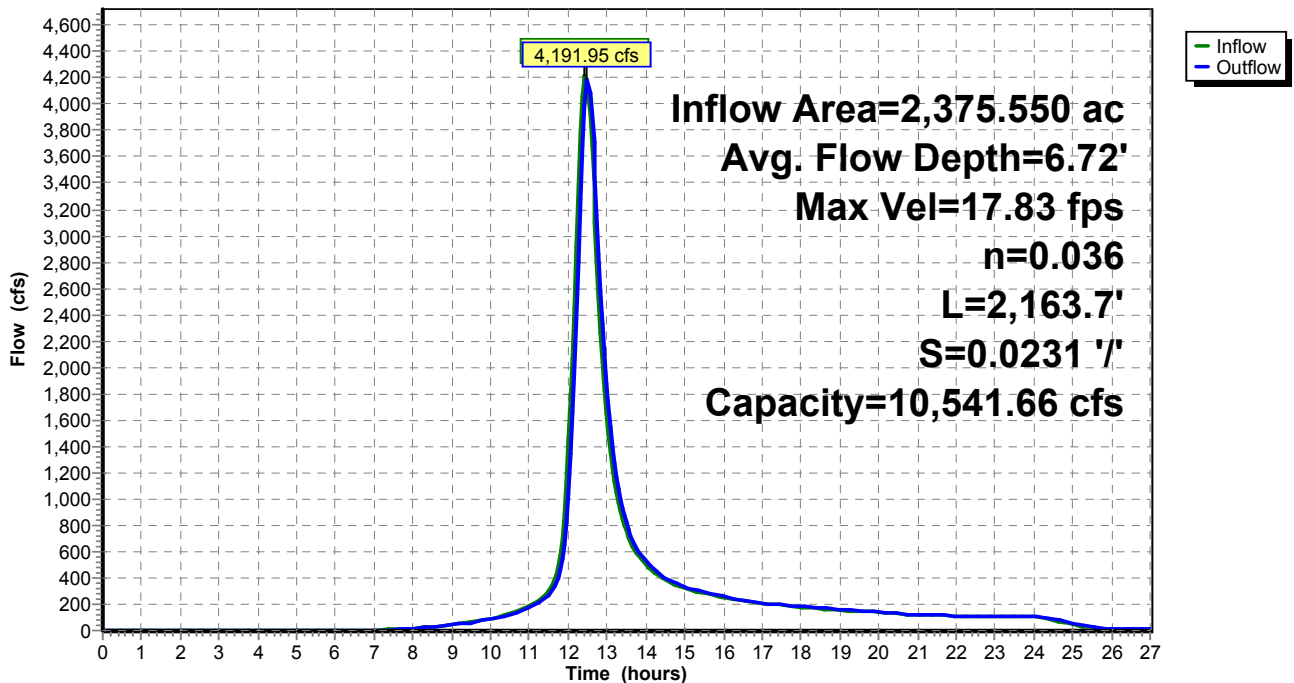
Peak Storage= 510,334 cf @ 12.46 hrs
 Average Depth at Peak Storage= 6.72'
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 58.00'
 Length= 2,163.7' Slope= 0.0231 '/'
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



Reach CHNL 5: Channel 5

Hydrograph



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Summary for Pond CLVT 1: Culvert Crossing #1

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 17.28' @ 12.75 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event
 Inflow = 4,191.95 cfs @ 12.49 hrs, Volume= 537.163 af
 Outflow = 3,410.14 cfs @ 12.71 hrs, Volume= 537.020 af, Atten= 19%, Lag= 12.9 min
 Primary = 3,410.14 cfs @ 12.71 hrs, Volume= 537.020 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,392.92' @ 12.71 hrs Surf.Area= 0 sf Storage= 1,758,861 cf
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 4.1 min calculated for 537.020 af (100% of inflow)
 Center-of-Mass det. time= 4.0 min (860.7 - 856.8)

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	Culvert Crossing #1 Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

Exist WS (Post-Quintana)_ Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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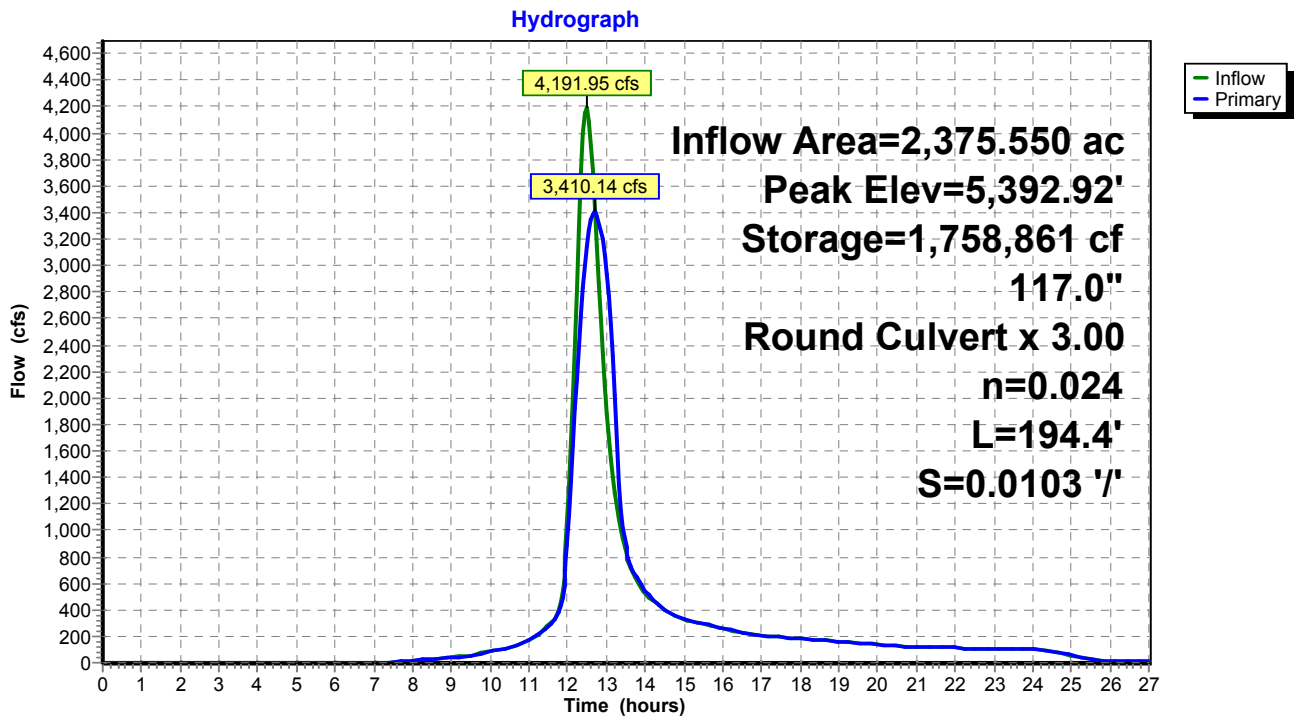
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	117.0" Round Culvert X 3.00 L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=3,408.57 cfs @ 12.71 hrs HW=5,392.90' (Free Discharge)
 ↳ **1=Culvert** (Inlet Controls 3,408.57 cfs @ 15.22 fps)

Pond CLVT 1: Culvert Crossing #1



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Summary for Pond CLVT 2: Culvert Crossing #2

[81] Warning: Exceeded Pond CLVT 1 by 11.78' @ 13.40 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event
 Inflow = 3,418.33 cfs @ 12.71 hrs, Volume= 543.413 af
 Outflow = 1,658.81 cfs @ 13.25 hrs, Volume= 543.361 af, Atten= 51%, Lag= 32.8 min
 Primary = 1,658.81 cfs @ 13.25 hrs, Volume= 543.361 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,391.04' @ 13.25 hrs Surf.Area= 0 sf Storage= 6,444,314 cf
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 30.6 min calculated for 543.361 af (100% of inflow)
 Center-of-Mass det. time= 30.5 min (890.6 - 860.1)

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	Existing Pond Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 200-yr Event Rainfall=4.09"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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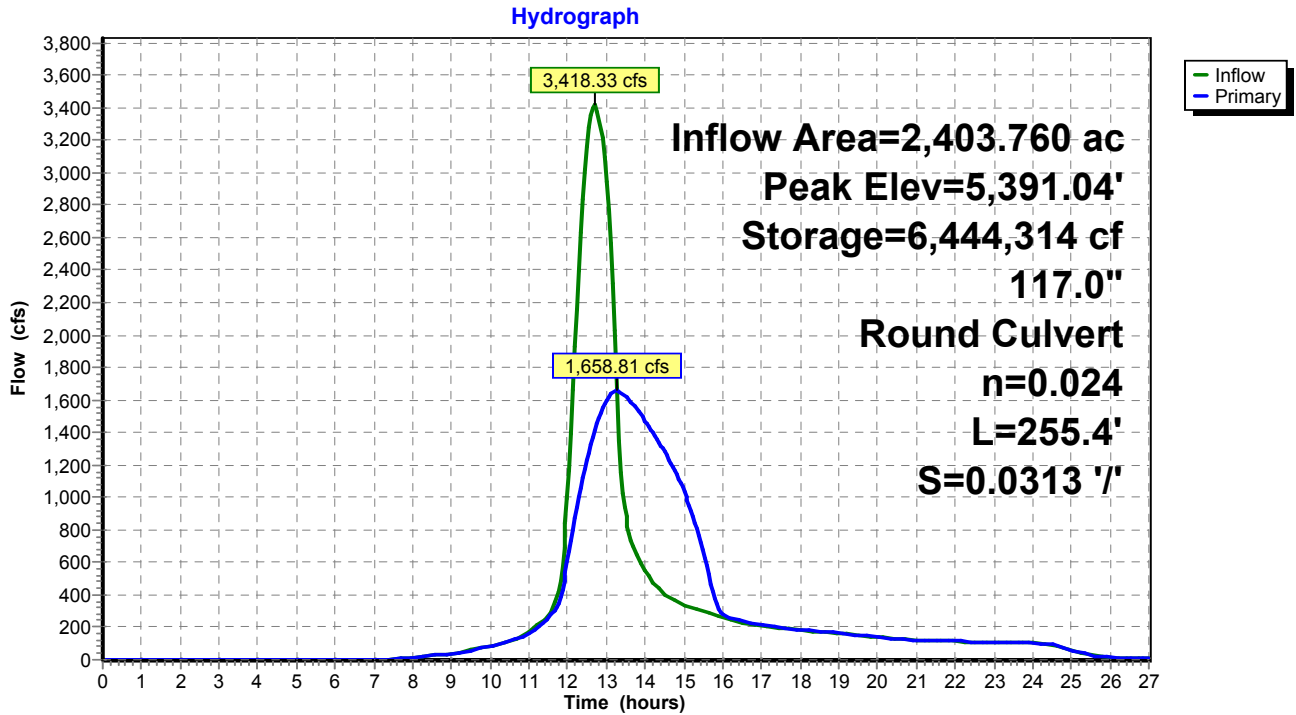
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5,402.00 9,814,540
 5,403.00 10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	117.0" Round Culvert L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=1,658.65 cfs @ 13.25 hrs HW=5,391.03' (Free Discharge)
 ←**1=Culvert** (Inlet Controls 1,658.65 cfs @ 22.22 fps)

Pond CLVT 2: Culvert Crossing #2



Exist WS (Post-Quintana)_Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: WS 1	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=3.19" Tc=5.0 min CN=87 Runoff=155.94 cfs 7.504 af
Subcatchment2: WS 2	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=484.31 cfs 28.349 af
Subcatchment3: WS 3	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=172.66 cfs 9.308 af
Subcatchment4: WS 4	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=346.92 cfs 19.956 af
Subcatchment5: WS 5	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=458.61 cfs 33.036 af
Subcatchment6: WS 6	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=1,038.74 cfs 88.109 af
Subcatchment7: WS 7	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=550.14 cfs 38.431 af
Subcatchment8: WS 8	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=389.99 cfs 24.476 af
Subcatchment9: WS 9	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=761.76 cfs 62.755 af
Subcatchment10: WS 10	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=891.85 cfs 87.894 af
Subcatchment11: WS 11	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=1,132.73 cfs 105.828 af
Subcatchment12: WS 12	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=789.72 cfs 60.497 af
Subcatchment13: WS 13	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=871.93 cfs 73.289 af
Reach CHNL 1: Channel 1	Avg. Flow Depth=2.98' Max Vel=9.88 fps Inflow=1,647.46 cfs 133.786 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,601.86 cfs 133.784 af
Reach CHNL 2: Channel 2	Avg. Flow Depth=2.04' Max Vel=10.26 fps Inflow=2,597.10 cfs 231.491 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=2,180.53 cfs 231.028 af
Reach CHNL 3: Channel 3	Avg. Flow Depth=6.49' Max Vel=15.55 fps Inflow=5,094.93 cfs 593.807 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=5,008.23 cfs 593.385 af

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Reach CHNL 4: Channel 4 Avg. Flow Depth=4.57' Max Vel=23.45 fps Inflow=5,028.84 cfs 602.692 af
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=5,012.38 cfs 602.599 af

Reach CHNL 5: Channel 5 Avg. Flow Depth=7.44' Max Vel=18.81 fps Inflow=5,080.00 cfs 630.949 af
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=5,046.09 cfs 630.724 af

Pond CLVT 1: Culvert Crossing Peak Elev=5,397.29' Storage=2,570,184 cf Inflow=5,046.09 cfs 630.724 af
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,847.35 cfs 630.580 af

Pond CLVT 2: Culvert Crossing Peak Elev=5,396.71' Storage=8,137,086 cf Inflow=3,856.73 cfs 638.084 af
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,791.19 cfs 638.031 af

Total Runoff Area = 2,403.760 ac Runoff Volume = 639.433 af Average Runoff Depth = 3.19"
90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 1: WS 1

[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 155.94 cfs @ 11.95 hrs, Volume= 7.504 af, Depth= 3.19"

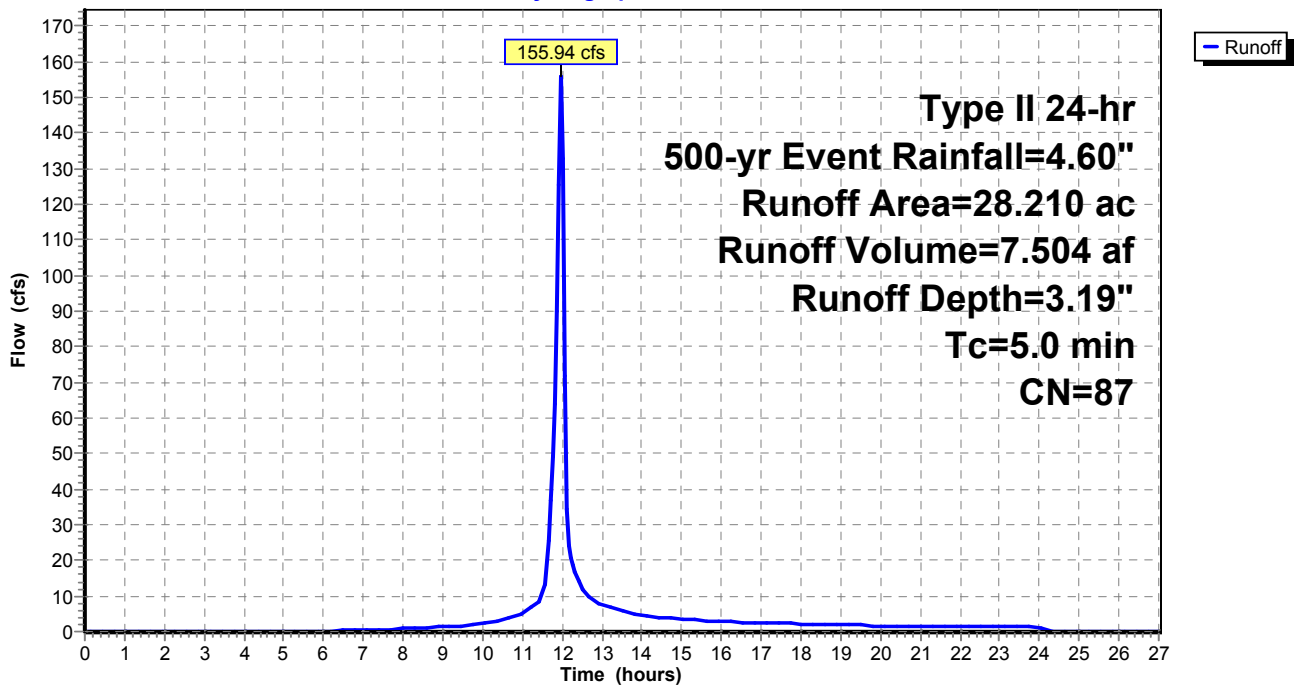
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment 1: WS 1

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 2: WS 2

Runoff = 484.31 cfs @ 12.02 hrs, Volume= 28.349 af, Depth= 3.19"

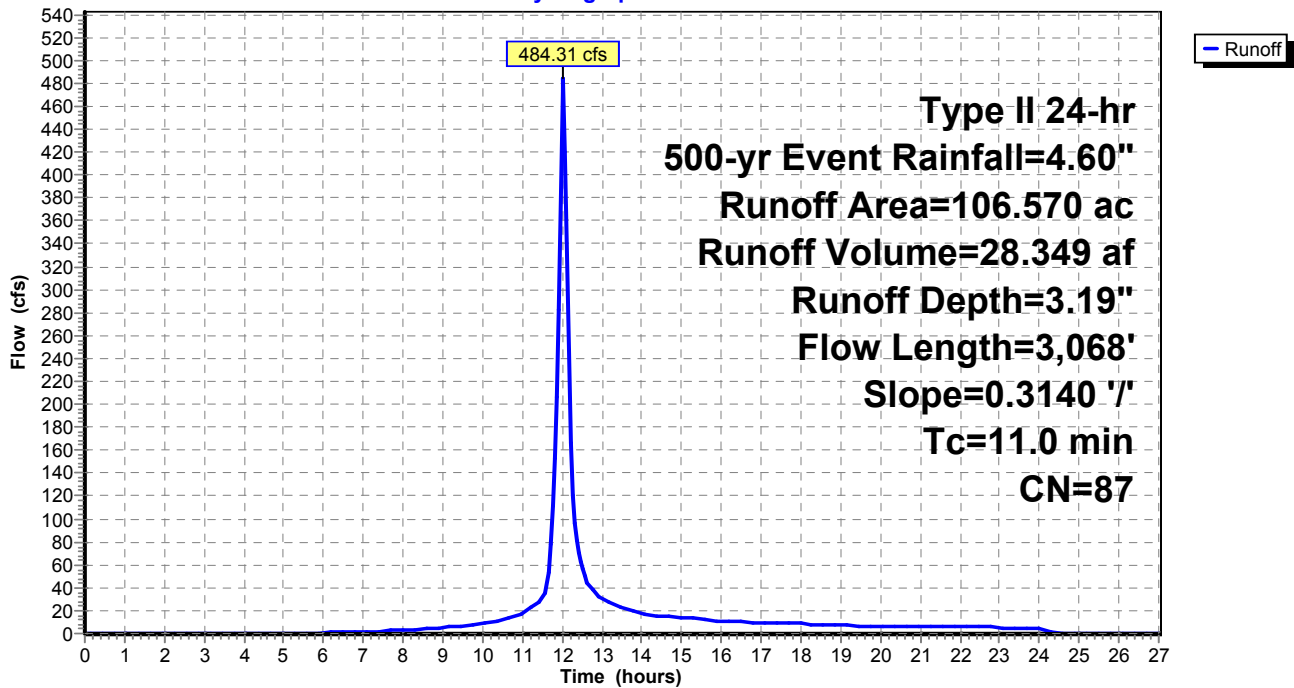
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

Subcatchment 2: WS 2

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 3: WS 3

Runoff = 172.66 cfs @ 12.00 hrs, Volume= 9.308 af, Depth= 3.19"

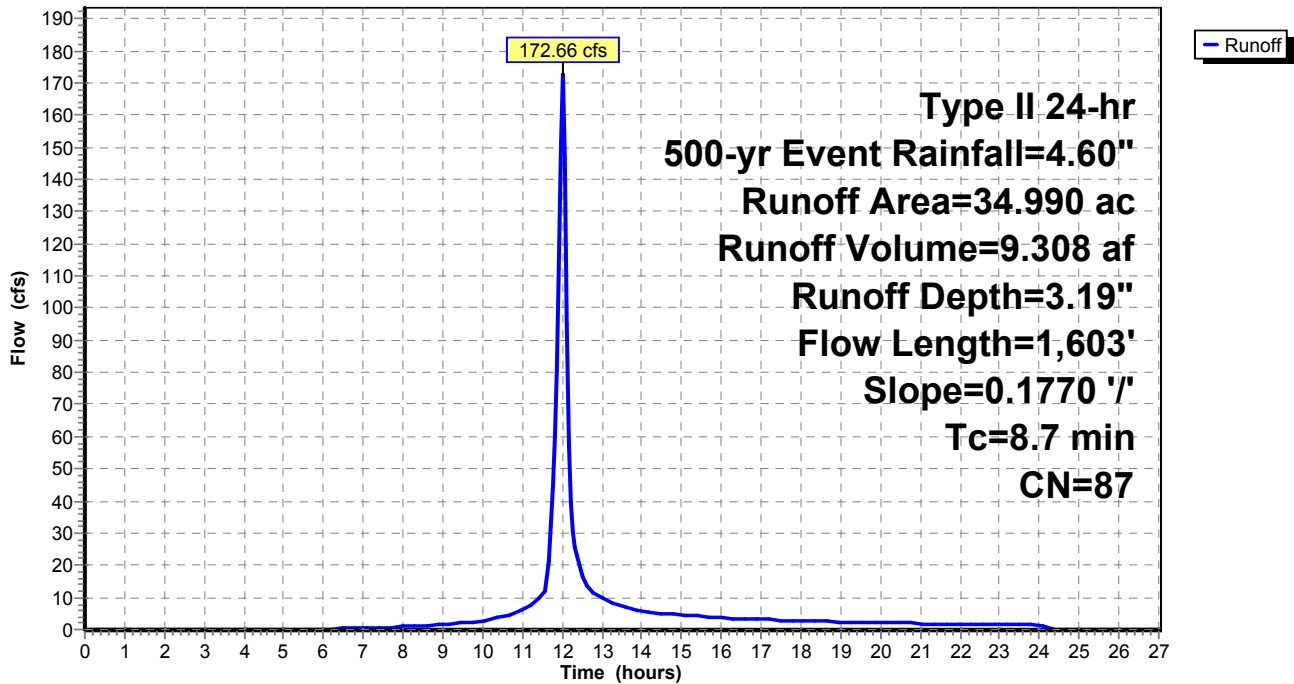
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

Subcatchment 3: WS 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 4: WS 4

Runoff = 346.92 cfs @ 12.02 hrs, Volume= 19.956 af, Depth= 3.19"

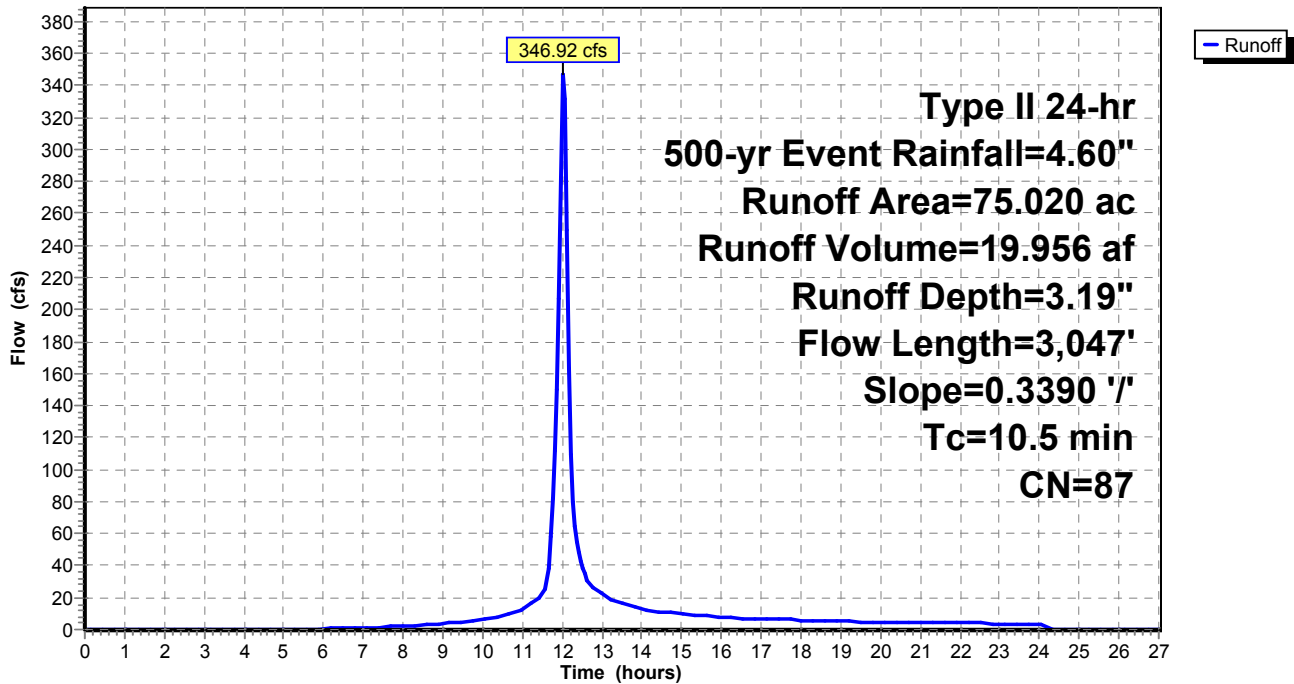
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

Subcatchment 4: WS 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 5: WS 5

Runoff = 458.61 cfs @ 12.10 hrs, Volume= 33.036 af, Depth= 3.19"

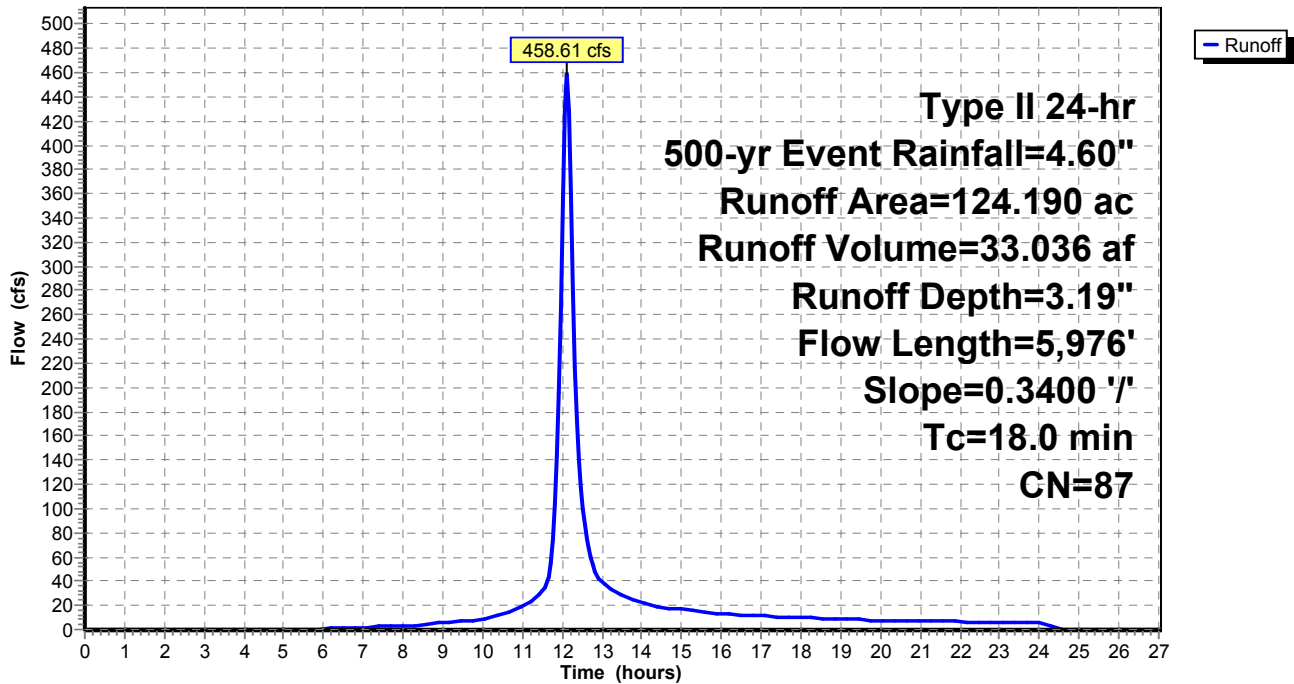
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

Subcatchment 5: WS 5

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 6: WS 6

Runoff = 1,038.74 cfs @ 12.17 hrs, Volume= 88.109 af, Depth= 3.19"

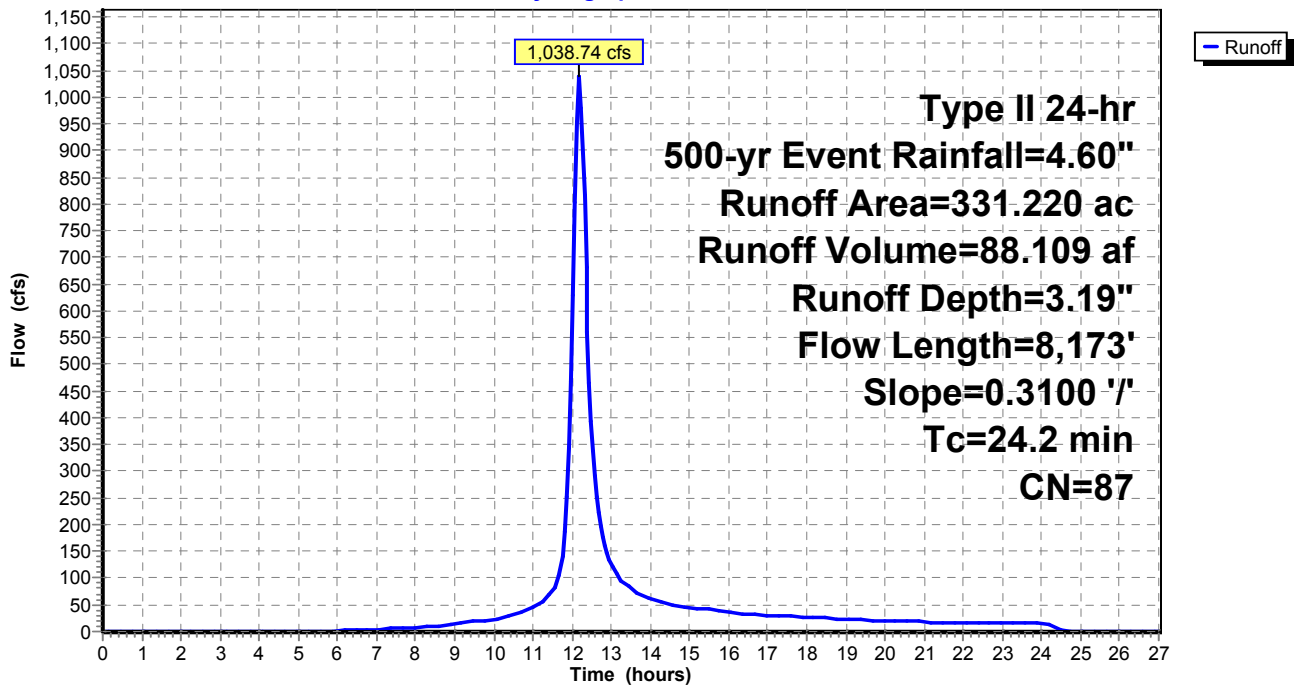
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

Subcatchment 6: WS 6

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 7: WS 7

Runoff = 550.14 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

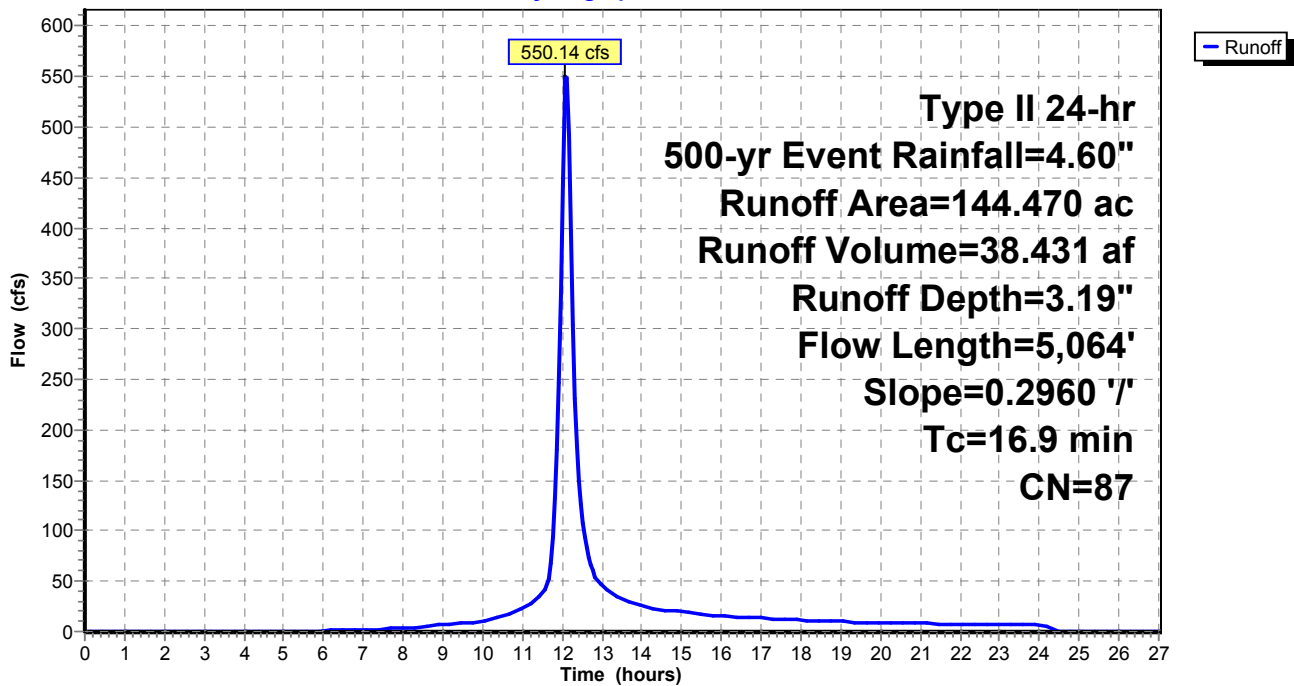
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

Subcatchment 7: WS 7

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 8: WS 8

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

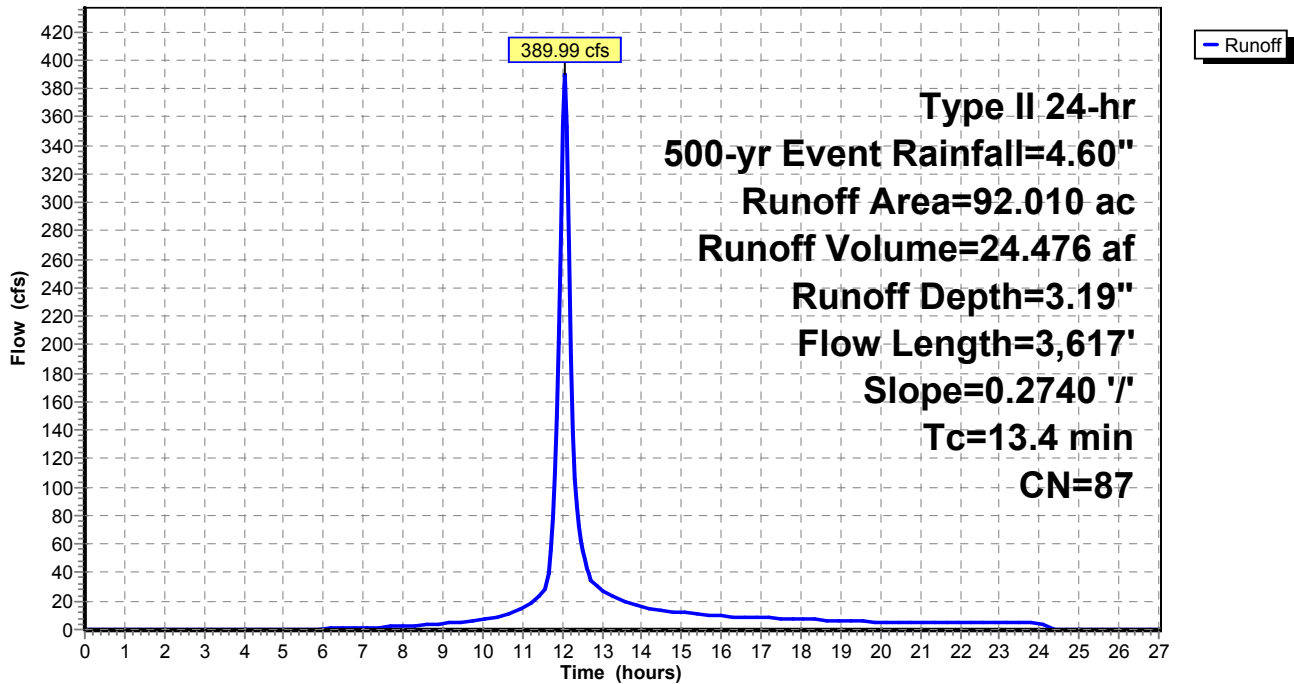
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

Subcatchment 8: WS 8

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 9: WS 9

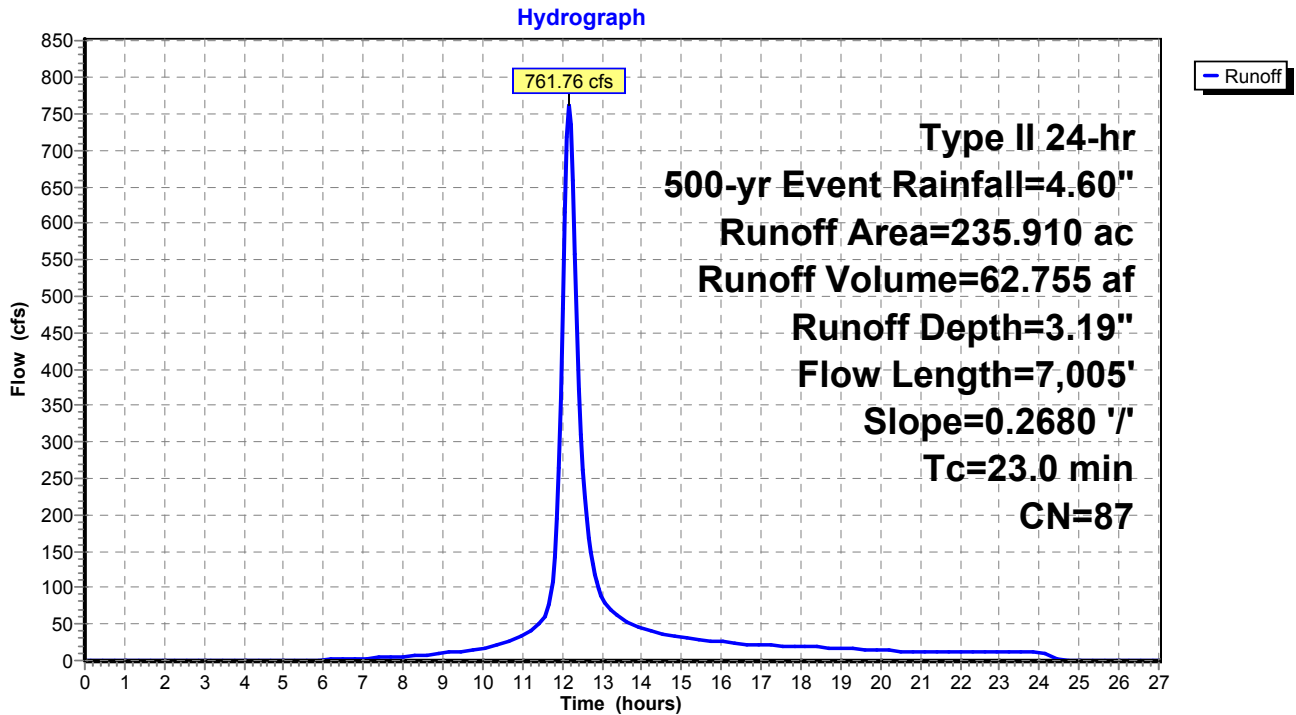
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

Subcatchment 9: WS 9



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 10: WS 10

Runoff = 891.85 cfs @ 12.25 hrs, Volume= 87.894 af, Depth= 3.19"

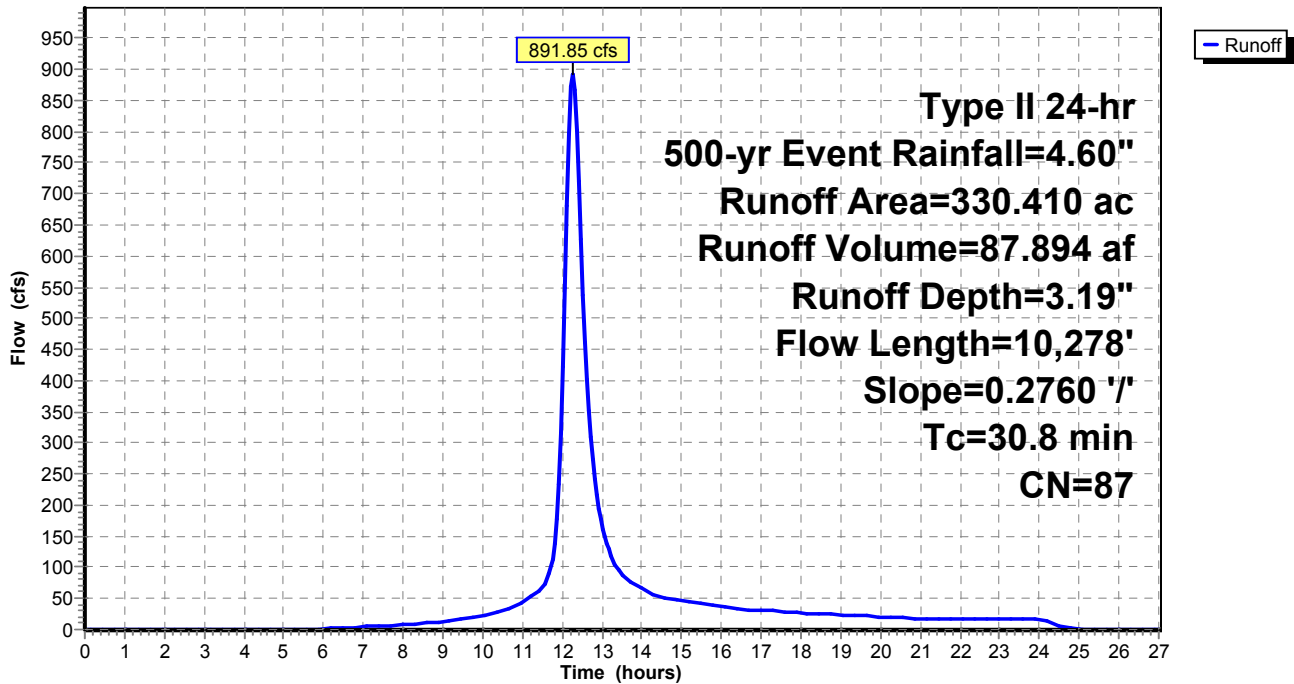
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

Subcatchment 10: WS 10

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 11: WS 11

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

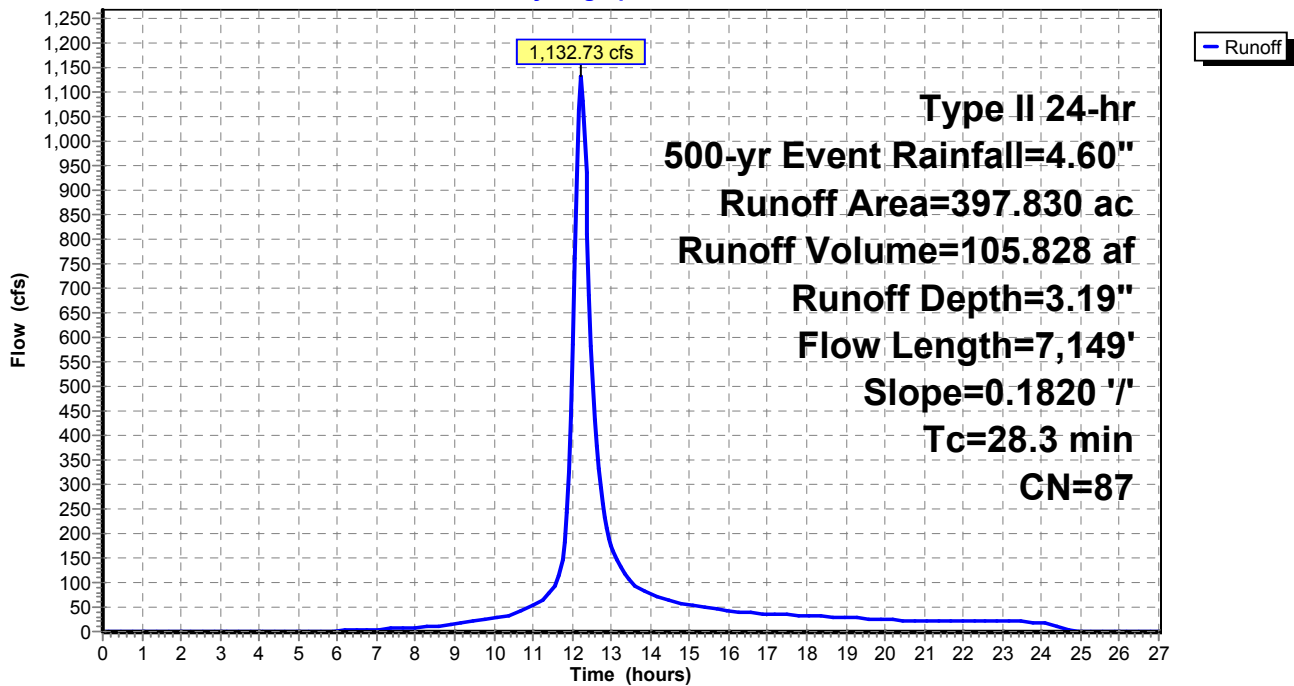
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

Subcatchment 11: WS 11

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 12: WS 12

Runoff = 789.72 cfs @ 12.12 hrs, Volume= 60.497 af, Depth= 3.19"

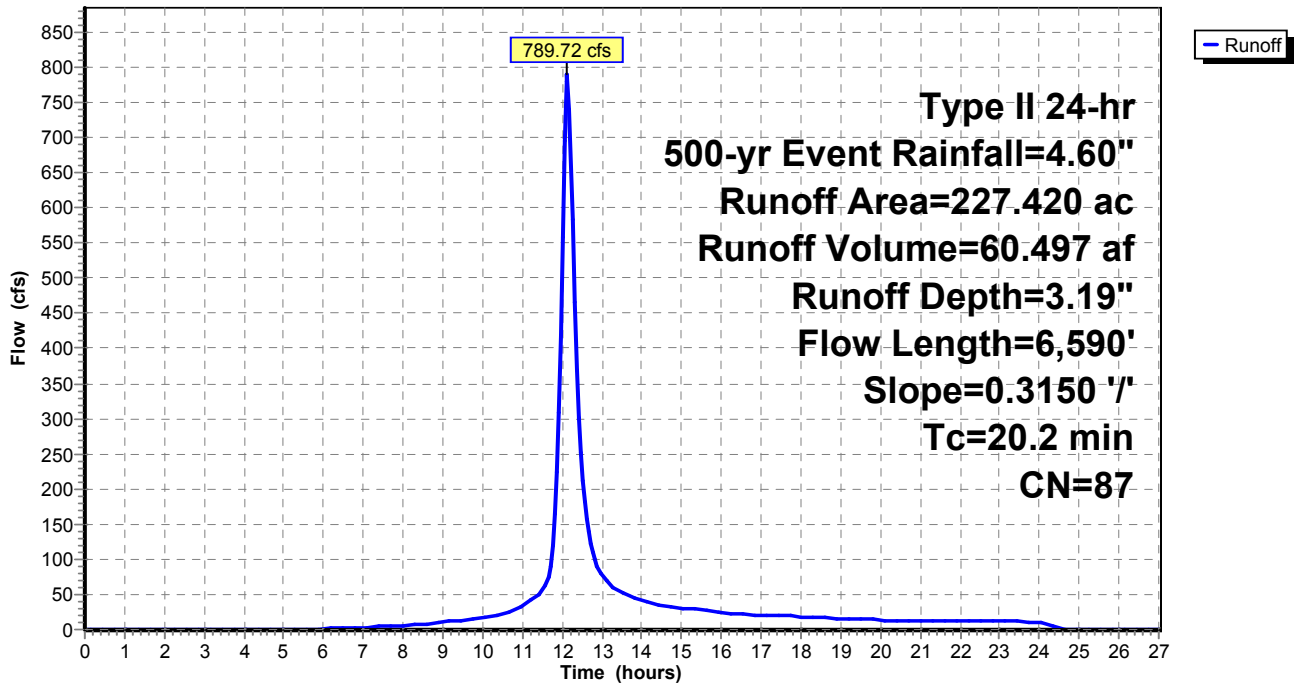
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

Subcatchment 12: WS 12

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Subcatchment 13: WS 13

Runoff = 871.93 cfs @ 12.16 hrs, Volume= 73.289 af, Depth= 3.19"

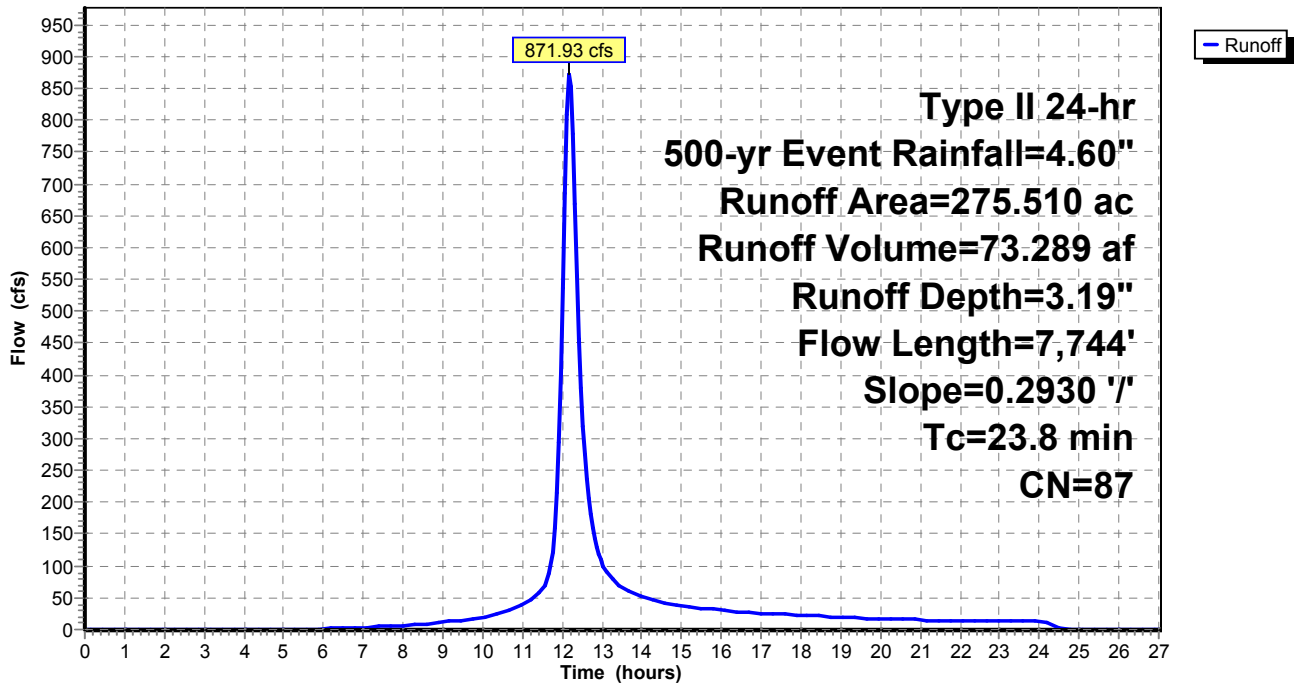
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

Subcatchment 13: WS 13

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Reach CHNL 1: Channel 1

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 3.19" for 500-yr Event event
 Inflow = 1,647.46 cfs @ 12.14 hrs, Volume= 133.786 af
 Outflow = 1,601.86 cfs @ 12.23 hrs, Volume= 133.784 af, Atten= 3%, Lag= 5.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 9.88 fps, Min. Travel Time= 3.2 min
 Avg. Velocity = 2.42 fps, Avg. Travel Time= 13.2 min

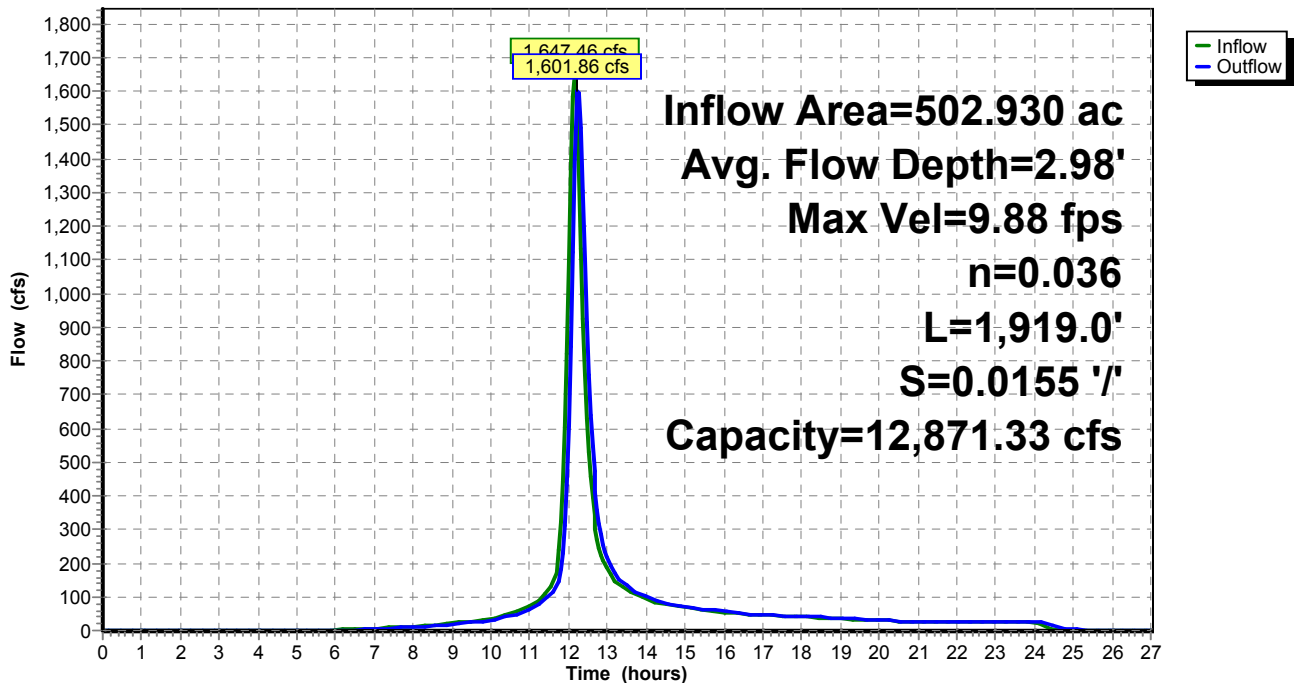
Peak Storage= 311,381 cf @ 12.18 hrs
 Average Depth at Peak Storage= 2.98'
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 ' / ' Top Width= 80.00'
 Length= 1,919.0' Slope= 0.0155 ' / '
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



Reach CHNL 1: Channel 1

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Reach CHNL 2: Channel 2

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 3.19" for 500-yr Event event
 Inflow = 2,597.10 cfs @ 12.14 hrs, Volume= 231.491 af
 Outflow = 2,180.53 cfs @ 12.45 hrs, Volume= 231.028 af, Atten= 16%, Lag= 18.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 10.26 fps, Min. Travel Time= 11.5 min
 Avg. Velocity = 2.72 fps, Avg. Travel Time= 43.2 min

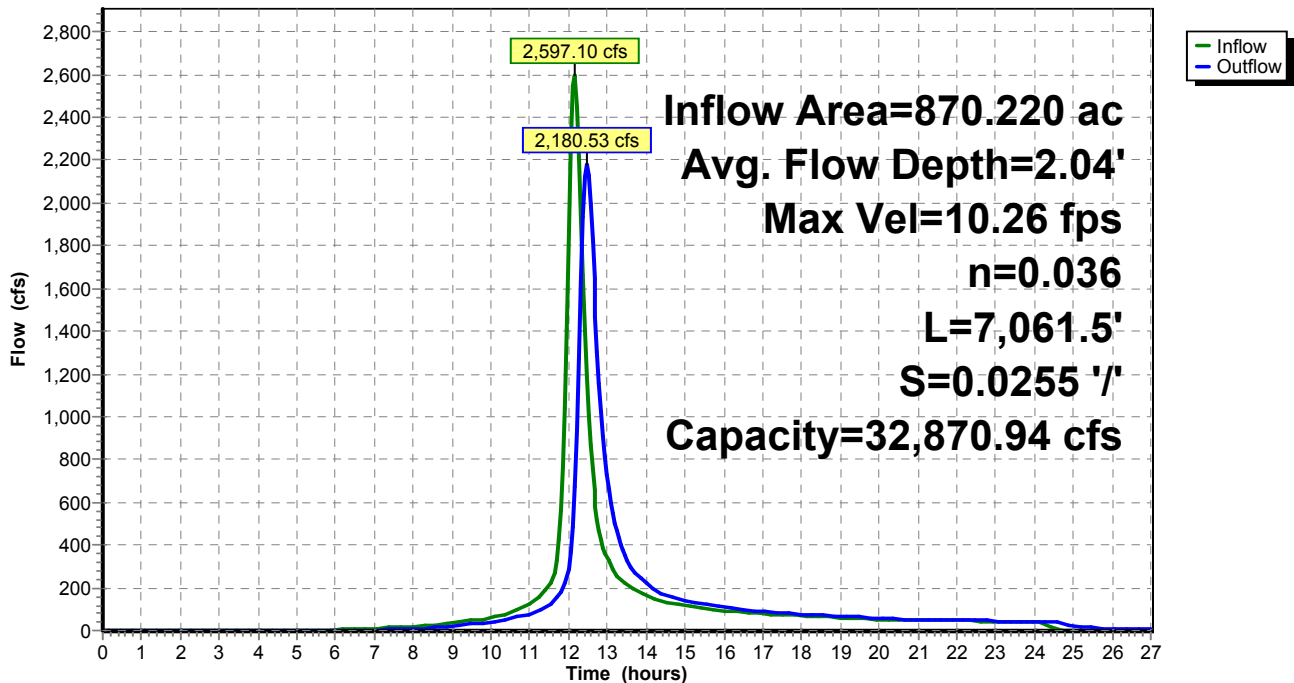
Peak Storage= 1,506,746 cf @ 12.26 hrs
 Average Depth at Peak Storage= 2.04'
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 2.5 2.0 ' / ' Top Width= 145.00'
 Length= 7,061.5' Slope= 0.0255 ' / '
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



Reach CHNL 2: Channel 2

Hydrograph



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Summary for Reach CHNL 3: Channel 3

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 4.30' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 4.53' @ 12.35 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event
 Inflow = 5,094.93 cfs @ 12.28 hrs, Volume= 593.807 af
 Outflow = 5,008.23 cfs @ 12.40 hrs, Volume= 593.385 af, Atten= 2%, Lag= 6.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 15.55 fps, Min. Travel Time= 3.9 min
 Avg. Velocity = 4.58 fps, Avg. Travel Time= 13.4 min

Peak Storage= 1,188,451 cf @ 12.33 hrs
 Average Depth at Peak Storage= 6.49'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 3,680.5' Slope= 0.0162 '/'
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

Prepared by M3 Engineering

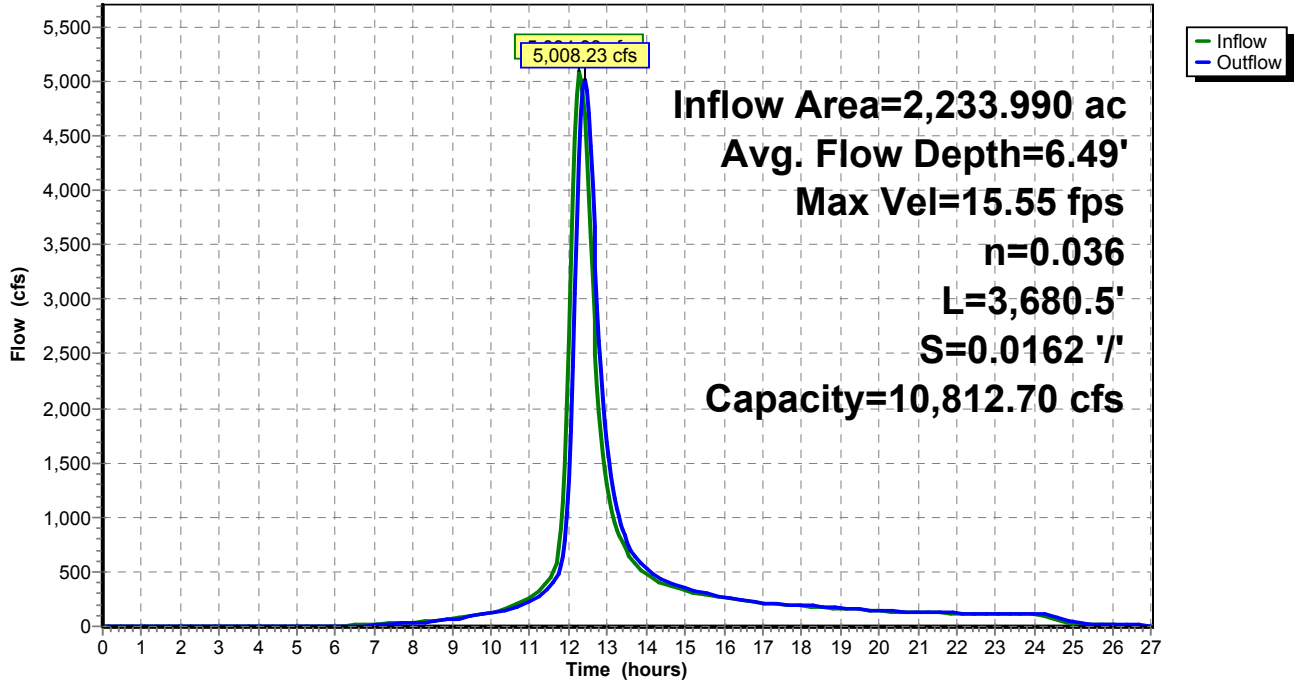
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Reach CHNL 3: Channel 3

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Reach CHNL 4: Channel 4

[61] Hint: Exceeded Reach CHNL 3 outlet invert by 4.57' @ 12.40 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event
 Inflow = 5,028.84 cfs @ 12.40 hrs, Volume= 602.692 af
 Outflow = 5,012.38 cfs @ 12.42 hrs, Volume= 602.599 af, Atten= 0%, Lag= 1.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 23.45 fps, Min. Travel Time= 0.8 min
 Avg. Velocity = 6.76 fps, Avg. Travel Time= 2.7 min

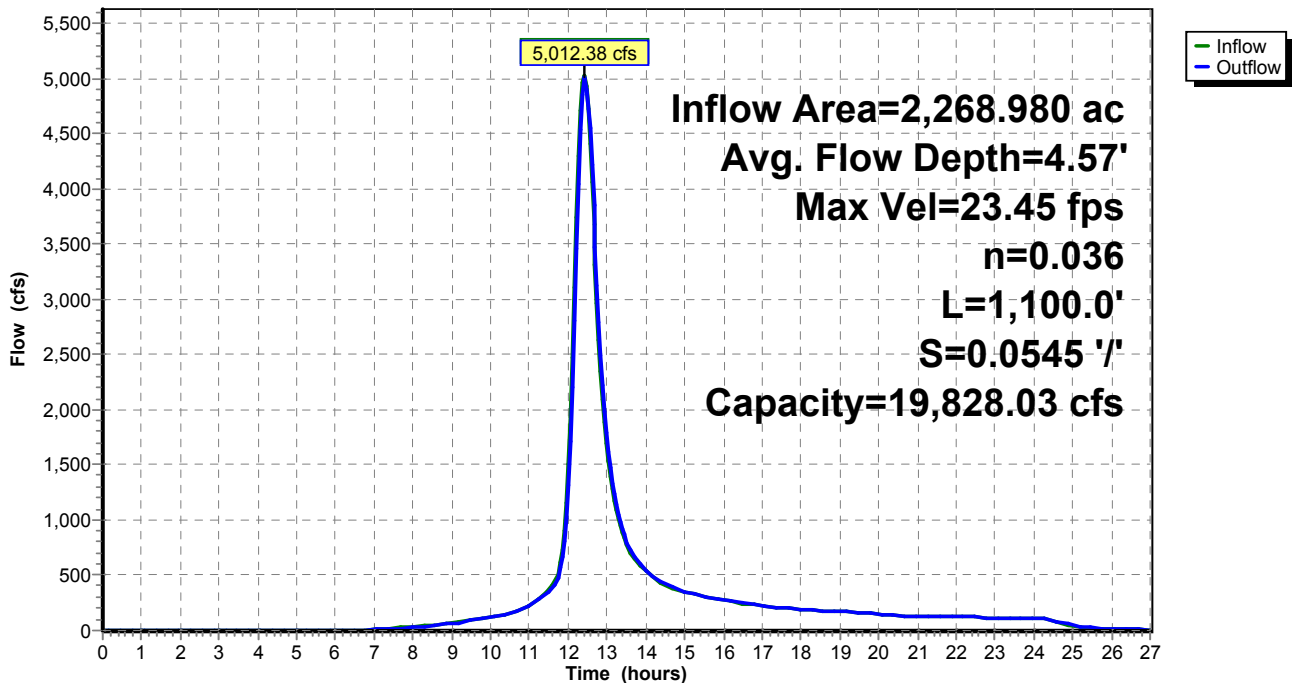
Peak Storage= 235,792 cf @ 12.41 hrs
 Average Depth at Peak Storage= 4.57'
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'
 Length= 1,100.0' Slope= 0.0545 '/'
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



Reach CHNL 4: Channel 4

Hydrograph



Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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Summary for Reach CHNL 5: Channel 5

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.92' @ 12.50 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event
 Inflow = 5,080.00 cfs @ 12.42 hrs, Volume= 630.949 af
 Outflow = 5,046.09 cfs @ 12.47 hrs, Volume= 630.724 af, Atten= 1%, Lag= 3.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Max. Velocity= 18.81 fps, Min. Travel Time= 1.9 min
 Avg. Velocity = 6.05 fps, Avg. Travel Time= 6.0 min

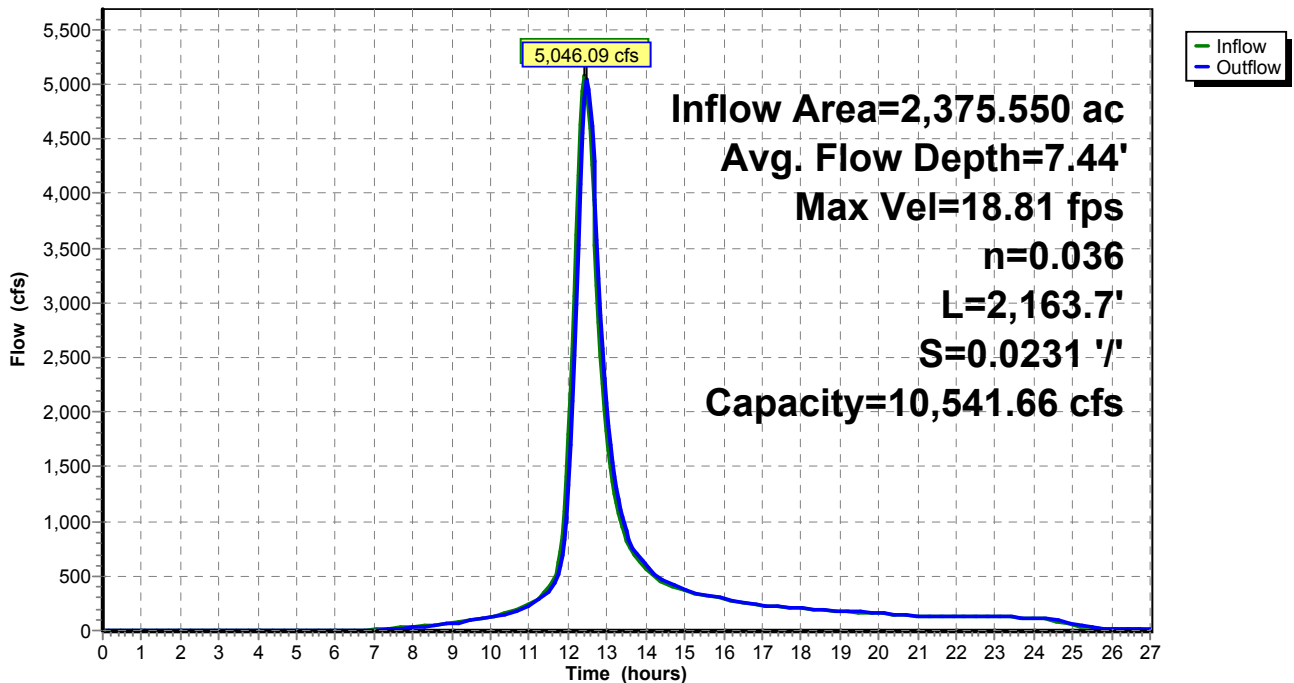
Peak Storage= 582,162 cf @ 12.44 hrs
 Average Depth at Peak Storage= 7.44'
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036
 Side Slope Z-value= 1.5 ' ' Top Width= 58.00'
 Length= 2,163.7' Slope= 0.0231 ' '
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



Reach CHNL 5: Channel 5

Hydrograph



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Summary for Pond CLVT 1: Culvert Crossing #1

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 21.32' @ 12.80 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event
 Inflow = 5,046.09 cfs @ 12.47 hrs, Volume= 630.724 af
 Outflow = 3,847.35 cfs @ 12.71 hrs, Volume= 630.580 af, Atten= 24%, Lag= 14.4 min
 Primary = 3,847.35 cfs @ 12.71 hrs, Volume= 630.580 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,397.29' @ 12.71 hrs Surf.Area= 0 sf Storage= 2,570,184 cf
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 5.1 min calculated for 629.414 af (100% of inflow)
 Center-of-Mass det. time= 4.9 min (855.3 - 850.4)

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	Culvert Crossing #1 Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

Exist WS (Post-Quintana)_ Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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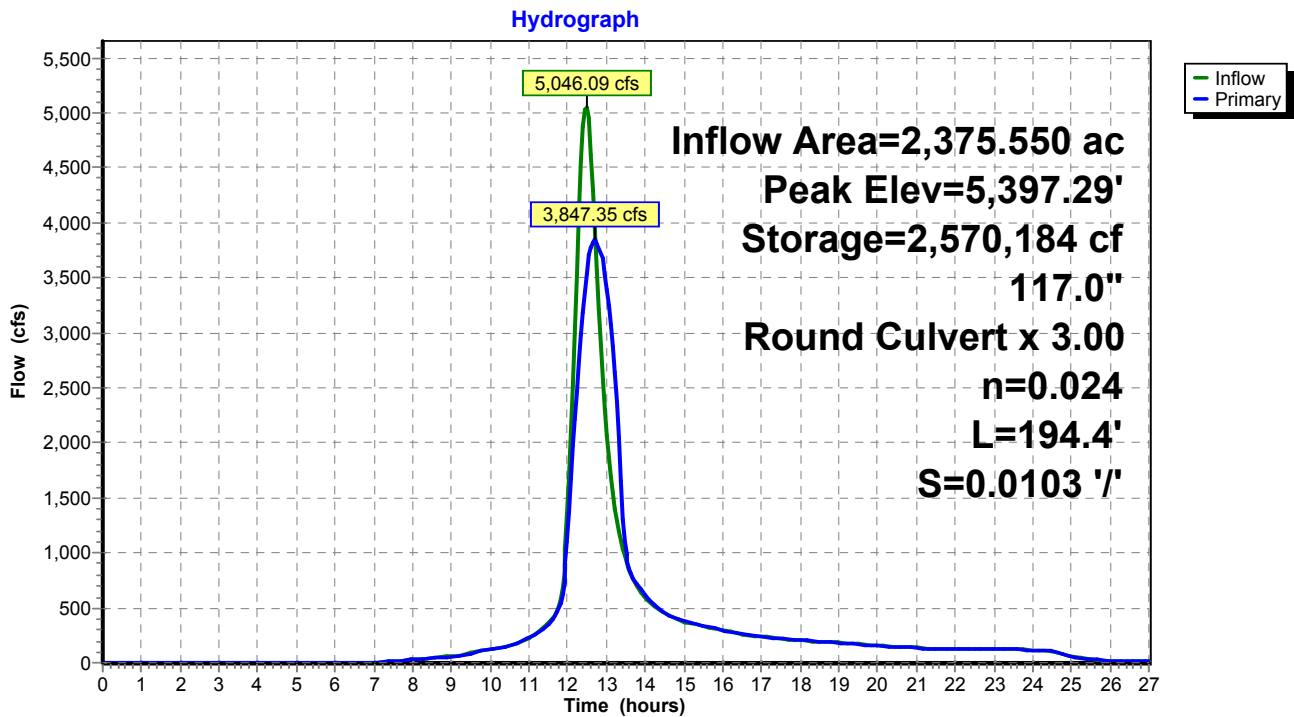
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	117.0" Round Culvert X 3.00 L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=3,844.98 cfs @ 12.71 hrs HW=5,397.27' (Free Discharge)
 ↳ **1=Culvert** (Inlet Controls 3,844.98 cfs @ 17.17 fps)

Pond CLVT 1: Culvert Crossing #1



Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Summary for Pond CLVT 2: Culvert Crossing #2

[81] Warning: Exceeded Pond CLVT 1 by 17.33' @ 13.50 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event
 Inflow = 3,856.73 cfs @ 12.71 hrs, Volume= 638.084 af
 Outflow = 1,791.19 cfs @ 13.33 hrs, Volume= 638.031 af, Atten= 54%, Lag= 37.4 min
 Primary = 1,791.19 cfs @ 13.33 hrs, Volume= 638.031 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs
 Peak Elev= 5,396.71' @ 13.33 hrs Surf.Area= 0 sf Storage= 8,137,086 cf
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 36.3 min calculated for 636.852 af (100% of inflow)
 Center-of-Mass det. time= 36.2 min (890.9 - 854.7)

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	Existing Pond Listed below

Exist WS (Post-Quintana)_ Culvert Analysis*Type II 24-hr 500-yr Event Rainfall=4.60"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

Exist WS (Post-Quintana)_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

Prepared by M3 Engineering

Printed 11/24/2015

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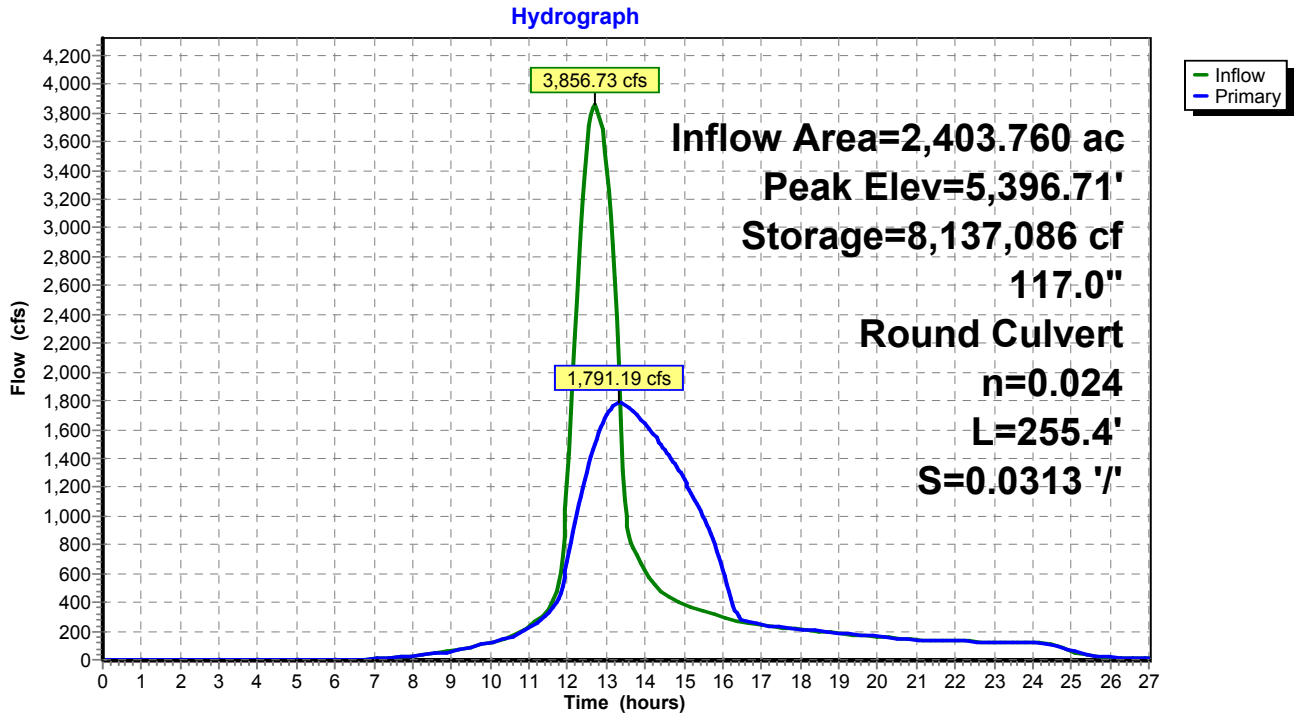
Page 85

5,402.00 9,814,540
 5,403.00 10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	117.0" Round Culvert L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

Primary OutFlow Max=1,790.68 cfs @ 13.33 hrs HW=5,396.68' (Free Discharge)
 ←**1=Culvert** (Inlet Controls 1,790.68 cfs @ 23.98 fps)

Pond CLVT 2: Culvert Crossing #2



APPENDIX E

HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 3552.39 cfs

Maximum Flow: 3552.39 cfs

Table 1 - Summary of Culvert Flows at Crossing: Crossing #1 (Q100)

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.14	355.24	355.24	0.00	1
5379.56	710.48	710.48	0.00	1
5380.24	1065.72	1065.72	0.00	1
5381.15	1420.96	1420.96	0.00	1
5382.25	1776.19	1776.19	0.00	1
5383.46	2131.43	2131.43	0.00	1
5384.93	2486.67	2486.67	0.00	1
5386.88	2841.91	2841.91	0.00	1
5389.10	3197.15	3197.15	0.00	1
5391.64	3552.39	3552.39	0.00	1
5419.00	6067.40	6067.40	0.00	Overtopping

Table 2 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	355.24	355.24	5379.14	3.732	7.141	1-S1t	2.429	2.542	9.000	9.000	1.661
	710.48	710.48	5379.56	5.429	7.560	1-S1t	3.514	3.636	9.000	9.000	3.322
	1065.72	1065.72	5380.24	6.895	8.241	1-S1t	4.391	4.498	9.000	9.000	4.983
	1420.96	1420.96	5381.15	8.285	9.153	1-S1t	5.196	5.224	9.000	9.000	6.645
	1776.19	1776.19	5382.25	9.701	10.246	7-M1t	5.989	5.872	9.000	9.000	8.221
	2131.43	2131.43	5383.46	11.227	11.459	7-M1t	6.823	6.452	9.000	9.000	9.865
	2486.67	2486.67	5384.93	12.934	12.754	7-M1t	7.797	6.982	9.000	9.000	11.509
	2841.91	2841.91	5386.88	14.878	14.187	3-M2t	9.750	7.458	9.000	9.000	13.153
	3197.15	3197.15	5389.10	17.102	16.162	7-M2t	9.750	7.882	9.000	9.000	14.797
	3552.39	3552.39	5391.64	19.638	18.453	7-M2t	9.750	8.252	9.000	9.000	16.441

Straight Culvert
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft
Culvert Length: 194.41 ft, Culvert Slope: 0.0103

Site Data - Culvert 1

Site Data Option: Culvert Invert Data
Inlet Station: 0.00 ft
Inlet Elevation: 5372.00 ft
Outlet Station: 194.40 ft
Outlet Elevation: 5370.00 ft
Number of Barrels: 3

Culvert Data Summary - Culvert 1

Barrel Shape: Circular
Barrel Diameter: 9.75 ft
Barrel Material: Corrugated Steel
Embedment: 0.00 in
Barrel Manning's n: 0.0240
Culvert Type: Straight
Inlet Configuration: Thin Edge Projecting
Inlet Depression: NONE

Table 3 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q100))

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
355.24	5379.00	9.00
710.48	5379.00	9.00
1065.72	5379.00	9.00
1420.96	5379.00	9.00
1776.19	5379.00	9.00
2131.43	5379.00	9.00
2486.67	5379.00	9.00
2841.91	5379.00	9.00
3197.15	5379.00	9.00
3552.39	5379.00	9.00

Tailwater Channel Data - Crossing #1 (Q100)

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

Roadway Data for Crossing: Crossing #1 (Q100)

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 4191.95 cfs

Maximum Flow: 4191.95 cfs

Table 4 - Summary of Culvert Flows at Crossing: Crossing #1 (Q200)

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.20	419.19	419.19	0.00	1
5379.78	838.39	838.39	0.00	1
5380.71	1257.59	1257.59	0.00	1
5381.93	1676.78	1676.78	0.00	1
5383.33	2095.97	2095.97	0.00	1
5385.08	2515.17	2515.17	0.00	1
5387.43	2934.36	2934.36	0.00	1
5390.18	3353.56	3353.56	0.00	1
5393.38	3772.76	3772.76	0.00	1
5397.05	4191.95	4191.95	0.00	1
5419.00	6067.40	6067.40	0.00	Overtopping

Table 5 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	419.19	419.19	5379.20	4.077	7.197	1-S1t	2.671	2.769	9.000	9.000	1.960
	838.39	838.39	5379.78	5.973	7.777	1-S1t	3.833	3.962	9.000	9.000	3.920
	1257.59	1257.59	5380.71	7.648	8.708	1-S1t	4.827	4.903	9.000	9.000	5.881
	1676.78	1676.78	5381.93	9.297	9.926	7-M1t	5.767	5.696	9.000	9.000	7.761
	2095.97	2095.97	5383.33	11.067	11.334	7-M1t	6.738	6.397	9.000	9.000	9.701
	2515.17	2515.17	5385.08	13.080	12.862	7-M1t	7.879	7.022	9.000	9.000	11.641
	2934.36	2934.36	5387.43	15.428	14.613	3-M2t	9.750	7.573	9.000	9.000	13.581
	3353.56	3353.56	5390.18	18.179	17.154	7-M2t	9.750	8.052	9.000	9.000	15.521
	3772.76	3772.76	5393.38	21.377	19.962	7-M2t	9.750	8.453	9.000	9.000	17.461
	4191.95	4191.95	5397.05	25.048	23.060	7-M2t	9.750	8.776	9.000	9.000	19.402

Straight Culvert
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft
Culvert Length: 194.41 ft, Culvert Slope: 0.0103

Site Data - Culvert 1

Site Data Option: Culvert Invert Data
Inlet Station: 0.00 ft
Inlet Elevation: 5372.00 ft
Outlet Station: 194.40 ft
Outlet Elevation: 5370.00 ft
Number of Barrels: 3

Culvert Data Summary - Culvert 1

Barrel Shape: Circular
Barrel Diameter: 9.75 ft
Barrel Material: Corrugated Steel
Embedment: 0.00 in
Barrel Manning's n: 0.0240
Culvert Type: Straight
Inlet Configuration: Thin Edge Projecting
Inlet Depression: NONE

Table 6 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q200))

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
419.19	5379.00	9.00
838.39	5379.00	9.00
1257.59	5379.00	9.00
1676.78	5379.00	9.00
2095.97	5379.00	9.00
2515.17	5379.00	9.00
2934.36	5379.00	9.00
3353.56	5379.00	9.00
3772.76	5379.00	9.00
4191.95	5379.00	9.00

Tailwater Channel Data - Crossing #1 (Q200)

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

Roadway Data for Crossing: Crossing #1 (Q200)

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 5046.09 cfs

Maximum Flow: 5046.09 cfs

Table 7 - Summary of Culvert Flows at Crossing: Crossing #1 (Q500)

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.28	504.61	504.61	0.00	1
5380.12	1009.22	1009.22	0.00	1
5381.42	1513.83	1513.83	0.00	1
5383.06	2018.44	2018.44	0.00	1
5385.12	2523.05	2523.05	0.00	1
5388.00	3027.65	3027.65	0.00	1
5391.49	3532.26	3532.26	0.00	1
5395.63	4036.87	4036.87	0.00	1
5400.48	4541.48	4541.48	0.00	1
5406.01	5046.09	5046.09	0.00	1
5419.00	6067.41	6067.41	0.00	Overtopping

Table 8 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	504.61	504.61	5379.28	4.496	7.284	1-S1t	2.918	3.045	9.000	9.000	2.360
	1009.22	1009.22	5380.12	6.670	8.116	1-S1t	4.252	4.370	9.000	9.000	4.719
	1513.83	1513.83	5381.42	8.649	9.424	3-M1t	5.404	5.401	9.000	9.000	7.006
	2018.44	2018.44	5383.06	10.725	11.064	7-M1t	6.552	6.274	9.000	9.000	9.342
	2523.05	2523.05	5385.12	13.121	12.892	7-M1t	7.902	7.033	9.000	9.000	11.677
	3027.65	3027.65	5388.00	16.003	15.097	3-M2t	9.750	7.686	9.000	9.000	14.013
	3532.26	3532.26	5391.49	19.485	18.319	7-M2t	9.750	8.233	9.000	9.000	16.348
	4036.87	4036.87	5395.63	23.634	21.878	7-M2t	9.750	8.665	9.000	9.000	18.684
	4541.48	4541.48	5400.48	28.477	25.882	7-M2t	9.750	8.986	9.000	9.000	21.019
	5046.09	5046.09	5406.01	34.012	30.317	7-M2c	9.750	9.218	9.218	9.000	23.018

Straight Culvert
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft
Culvert Length: 194.41 ft, Culvert Slope: 0.0103

Site Data - Culvert 1

Site Data Option: Culvert Invert Data
Inlet Station: 0.00 ft
Inlet Elevation: 5372.00 ft
Outlet Station: 194.40 ft
Outlet Elevation: 5370.00 ft
Number of Barrels: 3

Culvert Data Summary - Culvert 1

Barrel Shape: Circular
Barrel Diameter: 9.75 ft
Barrel Material: Corrugated Steel
Embedment: 0.00 in
Barrel Manning's n: 0.0240
Culvert Type: Straight
Inlet Configuration: Thin Edge Projecting
Inlet Depression: NONE

Table 9 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q500))

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
504.61	5379.00	9.00
1009.22	5379.00	9.00
1513.83	5379.00	9.00
2018.44	5379.00	9.00
2523.05	5379.00	9.00
3027.65	5379.00	9.00
3532.26	5379.00	9.00
4036.87	5379.00	9.00
4541.48	5379.00	9.00
5046.09	5379.00	9.00

Tailwater Channel Data - Crossing #1 (Q500)

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

Roadway Data for Crossing: Crossing #1 (Q500)

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft



DRAFT REPORT

APPENDIX E

MINE RECLAMATION AND CLOSURE PLAN

COPPER FLAT MINE



Submitted To: Mr. Jeffrey Smith, PE
Chief Operating Officer
THEMAC Resources Group Ltd.
Copper Flat Mine Project
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Albuquerque, NM 87109

Submitted By: Golder Associates Inc.
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Albuquerque, NM 87113 USA

Distribution:
2 Copies – Mining and Minerals Division
2 Copies – New Mexico Environment Department
3 Copies – THEMAC Resources Group Ltd.
2 Copies – Golder Associates Inc.

October 7, 2016

1531453



This report documents the Reclamation and Closure Plan for all of the mine facilities and disturbed areas associated with New Mexico Copper Corporation's Copper Flat project located near Hillsboro, New Mexico in Sierra County. The designs included herein were developed at a level consistent with preliminary designs for agency review. Development of this report and associated preliminary reclamation and closure designs was conducted under the oversight of the following Golder staff:

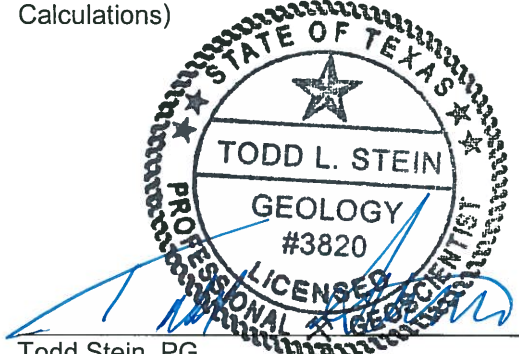


Jeffrey Clark, PE
(Senior Level Review of Preliminary Mine Reclamation and Closure Design Drawings)

10-14-16
Date

Heather Lammers, Project Engineer
(Preparation of Preliminary Mine Reclamation and Closure Design Drawings and Associated Engineering Calculations)

10/14/16
Date



Todd Stein, PG
(Preparation of Copper Flat Mine Reclamation and Closure Plan Document)

10/14/16
Date



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1.0 INTRODUCTION

Golder Associates has been contracted by New Mexico Copper Corporation (NMCC) to prepare a Mine Reclamation and Closure Plan for its Copper Flat project located near Hillsboro, New Mexico in Sierra County (**Figure E1**). This reclamation and closure plan describes the approach for reclamation of all of the disturbed areas described in NMCC's Mine Operations and Reclamation Plan document to which this document is attached as Appendix E. The reclamation and closure plan and associated design criteria conform to; (1) the closure requirements in the Copper Mine Rules (*Subsection A of 20.6.7.18 NMAC, 20.6.7.33 NMAC, 20.6.7.34 NMAC and 20.6.7.35 NMAC*); (2) reclamation requirements described 19.10.6.602.D(15) NMAC and 19.10.6.603 NMAC; and (3) applicable mine reclamation regulations set forth by the Bureau of Land Management (BLM) (3809.401(b)(3) and 3809.420(b)(3)). The goal for reclamation of the site is to re-establish the post-mining land uses consistent with the pre-mining land uses of the site and the surrounding area, i.e., wildlife habitat, grazing, mining, watershed and recreation as identified by the BLM in its approved Land Use Management Plan (BLM, 1986).

The Reclamation and Closure Plan and associated design criteria for the individual Copper Flat facilities are described in Section 2. Section 3 details how the plan will be implemented with regards to regrading operations, growth media placement and the revegetation plan. Section 4 describes the projected reclamation schedule and sequence of reclamation activities. Section 5 details the performance and reclamation standards and requirements. Section 6 provides the references associated with the citations included within the report.



2.0 RECLAMATION AND CLOSURE PLAN DESIGN CRITERIA

This Reclamation and Closure Plan was developed with consideration of the site-specific conditions that will exist at the Copper Flat Mine as a result of previous mine operations at the site by Quintana Minerals, and at the cessation of NMCC's proposed mining operations (end of mine life). The general setting of the Copper Flat Mine area is shown on **Figure E2** (existing features) and **Figure E3** (end of mine life features). The designs are depicted in the drawing set provided in **Attachment E1** of this Plan and have been developed to provide sufficient detail to calculate the financial assurance cost estimate when the Plans are deemed approvable.

The Reclamation and Closure Plan is subdivided into five major facility areas and several ancillary facilities, including:

Major Facility Areas:

- Existing Waste Rock Stockpiles (EWRSPs)
- Proposed Waste Rock Stockpiles (WRSPs)
- Tailings Storage Facility (TSF)
- Open Pit
- Plant Area

Ancillary Facilities:

- Surface Impoundments and Reservoirs
- Growth Media (i.e., Topdressing) Stockpiles
- Other Ancillary Facilities

Each of the major facility areas are discussed in Sections 2.1 through 2.5, and the ancillary facilities are discussed in Sections 2.6 through 2.8. The plans and methods developed herein represent detailed designs for reclamation of the facilities sufficient for agency review and approval. Construction design documents and construction quality assurance/construction quality control (CQA/CQC) plans will be prepared by NMCC for submittal to and approval by the State of New Mexico within 180 days of submission of a notice of intent to implement the closure plan per the Copper Rule (20.6.7.34.B, NMAC). The CQA/CQC plan will provide a detailed description of the work proposed to be performed and the final reclamation designs for the facilities to be closed. Post-closure monitoring activities will be conducted in accordance with Section 20.6.7.35 NMAC, and post-closure monitoring and maintenance any requirements that may be contained in the Copper Flat Mine Permit.

This Reclamation and Closure Plan describes: (1) contemporaneous reclamation that will be conducted, to the extent practicable, during mine operations; (2) facilities to be closed following cessation of mining operations; and (3) components that will be retained following closure of the site. A summary of the key design criteria for reclamation of the facilities to be closed is presented in **Table E1**.



Table E1. Proposed Copper Flat Reclamation Design Criteria

Facilities to be Reclaimed	Outslope Angle (Interbench) ^{1,2}	Cover Thickness ^{3,4}	Bench Width ^{2,5}	Interbench Slope Length ^{2,6}	Top Surface Slope ^{7,8}	Bench Cross Slope ⁵	Bench Longitudinal Slope ⁵	Surface Water Conveyances ^{2,9}	Comments		
Existing Waste Rock Stockpiles (EWRSPs)											
EWRSP-1	3.0H:1.0V max.	36" min.	NA	200' max.	Between 1.0% and 5.0%	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Requires a pullback from Grayback Diversion to maintain 25-foot setback from diversion. Top surfaces and outslopes to be graded to drain back to Grayback Diversion. Two small detention basins to be constructed to capture and evaporate minimal surface water runoff and provide riparian habitat. Will also regrade and cover safety/containment berm located south of EWRSP-1. There are no slopes greater than 200 feet in length so no benches are required.		
EWRSP-2A	NA	0" to 6"		NA	NA			NA	NA	Stockpile will get partially consumed by proposed WRSP-1. Portion that does not get consumed and located outside the OPSDA will get moved to EWRSP-2B or reclaimed as part of WRSP-1 reclamation. Disturbed surfaces will be graded, ripped and covered with 6-inches of suitable cover material where unsuitable growth media exists.	
EWRSP-2B	3.0H:1.0V max.	36" min.		200' max.	Between 1.0% and 5.0%			NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Waste material to the north of EWRSP-2B also gets graded and covered. Disturbed areas get graded, ripped, and seeded. There are no slopes greater than 200 feet in length so no benches are required.
EWRSP-3	3.0H:1.0V max.	36" min.		200' max.	Between 1.0% and 5.0%					Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Will be reclaimed as part of the Plant Area reclamation program. There are no slopes greater than 200 feet in length so no benches are required.
EWRSP-4	3.0H:1.0V max.	36" min.		200' max.	Between 1.0% and 5.0%					Armored channels designed to convey peak flows from 100-year, 24-hour storm event	The area between the two lobes of EWRSP-4 will be filled in with clean mine waste during the pre-production years. For reclamation design purposes it is assumed that the area is filled in prior to reclamation. Top surface to be graded to drain back toward the open pit, and the top surface will be used as an equipment and laydown area during operations. There are no slopes greater than 200 feet in length so no benches are required.



Table E1. Proposed Copper Flat Reclamation Design Criteria (continued)

Facilities to be Reclaimed	Outslope Angle (Interbench) ^{1,2}	Cover Thickness ^{3,4}	Bench Width ^{2,5}	Interbench Slope Length ^{2,6}	Top Surface Slope ^{7,8}	Bench Cross Slope ⁵	Bench Longitudinal Slope ⁵	Surface Water Conveyances ^{2,9}	Comments
New Proposed Waste Rock Stockpiles (WRSPs)									
WRSP-1	2.75H:1.0V max.	36" min.	25'	200' max.	Between 1.0% and 5.0%	1%	1%	Armored channels designed convey peak flows from 100-year, 24-hour storm event	This stockpile is located within the OPSDA and the outslopes will be graded to 2.75H:1V. Portion of EWRSP-2A located along northern perimeter of WRSP-1 will get consumed by this stockpile and reclaimed as part of WRSP-1.
WRSP-2	3.0H:1.0V max.			200' max.					WRSP's 2 and 3 will be reclaimed as one single facility. Southeastern toe needs to be set back a minimum of 50' from Grayback Arroyo.
WRSP-3				200' max.					
Surface Impoundments	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	HDPE liners will be ripped, folded over and buried in place, and impoundments backfilled with clean fill, surface to graded to drain and blend into the natural topography. Surface area to be covered with 6-inches of suitable cover material where unsuitable growth media exists after grading.
Growth Media Stockpiles (GMSPs)									
GMSP-1	NA	0"	NA	NA	Blended to natural topography	NA	NA	NA	The footprint areas of the growth media stockpiles will be graded to drain and recontoured to blend into the natural topography. It is anticipated that the only area that may require cover is GMSP-3 which is underlain by andesitic bedrock. The other two stockpile areas are underlain by alluvial materials (suitable growth media).
GMSP-2		0"							
GMSP-3		Up to 6"							
Tailing Storage Facility (TSF)									
Tailing Storage Facility	Between 3.0H:1.0V and 3.5H:1.0V	36" min.	25'	Between 200 feet (3H:1V) and 250 feet (3.5H:1V)	Between 1.0% and 5.0%	1%	1%	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	The toe of the TSF needs to stay where its at due to the underlying HDPE liner and associated HDPE lined toe berm.



Table E1. Proposed Copper Flat Reclamation Design Criteria (continued)

Facilities to be Reclaimed	Outslope Angle (Interbench) ^{1,2}	Cover Thickness ^{3,4}	Bench Width ^{2,5}	Interbench Slope Length ^{2,6}	Top Surface Slope ^{7,8}	Bench Cross Slope ⁵	Bench Longitudinal Slope ⁵	Surface Water Conveyances ^{2,9}	Comments
Tailing Storage Facility (TSF)									
Surface Impoundments	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	See above description in WRSPs.
Cyclone Plant Area	NA	6" to 36"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	All structures and equipment at the cyclone plant will be removed from the site and disposed of in an approved manner according to applicable federal and state laws; concrete foundations will be broken and covered with 36" of growth media; remaining disturbed areas will be graded and covered with 6" of growth media.
Open Pit									
Open Pit	NA	NA	NA	NA	NA	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Earthen berm will be constructed around the perimeter of the open pit to limit public access and reduce the physical safety hazard. Surface water conveyance channels will be constructed around the perimeter of the pit (immediately upstream of the perimeter berm/security fence) and along the existing haul road to direct surface water around and into the pit. Two small detention basins will be constructed on the western perimeter of the pit to capture and evaporate minimal surface runoff from the west perimeter of the pit. It is assumed there will be an approximate 100-foot-wide disturbance area around the pit that will be ripped and revegetated.
Plant Area									
Buildings and Structures	NA	6" to 36"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	All fuel tanks, reagent storage facilities, and equipment will be removed from the site and disposed of in an approved manner according to applicable federal and state laws; concrete foundations will be broken, walls toppled, backfilled, and covered with 36" of growth media; remaining disturbed areas will be graded and covered with 6" of growth media.



Table E1. Proposed Copper Flat Reclamation Design Criteria (continued)

Facilities to be Reclaimed	Outslope Angle (Interbench) ^{1,2}	Cover Thickness ^{3,4}	Bench Width ^{2,5}	Interbench Slope Length ^{2,6}	Top Surface Slope ^{7,8}	Bench Cross Slope ⁵	Bench Longitudinal Slope ⁵	Surface Water Conveyances ^{2,9}	Comments
Plant Area									
Surface Impoundments	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	See above description in WRSPs.
Pipelines, Pipeline Corridors	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Residual sediments and fluids will be flushed from the pipelines and placed in the TSF prior to reclamation of this facility, or at an approved location. Above-ground pipelines will be disposed of in the TSF prior to reclamation of this facility, or at a nearby approved construction and debris landfill. Buried pipelines will be capped at both ends. Surfaces will be graded, ripped and covered with 6-inches of suitable cover material where unsuitable growth media exists.
Electrical Infrastructure	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	Armored channels designed to convey peak flows from 100-year, 24-hour storm event	Removal of on-site overhead lines and power poles and disconnect from the 115kV line owned by Tri-State Generation and Transmission. The electrical substation and associated transmission lines will be closed once they are no longer needed. Disturbed surfaces along corridor will be graded, ripped and covered with 6-inches of suitable cover material where unsuitable growth media exists.
Ancillary Facilities									
Pipelines, Pipeline Corridors	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	See above description in Plant Area.



Table E1. Proposed Copper Flat Reclamation Design Criteria (continued)

Facilities to be Reclaimed	Outslope Angle (Interbench) ^{1,2}	Cover Thickness ^{3,4}	Bench Width ^{2,5}	Interbench Slope Length ^{2,6}	Top Surface Slope ^{7,8}	Bench Cross Slope ⁵	Bench Longitudinal Slope ⁵	Surface Water Conveyances ^{2,9}	Comments
Ancillary Facilities									
Surface Impoundments	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	See above description in WRSPs.
Mine Access Roads									Access roads not needed for closure and post-closure access will be reclaimed by ripping and revegetating the surfaces. Roads will be covered with 6-inches of suitable cover material where unsuitable growth media exists. Culverts will be removed if they are not needed for post-closure storm water management and disposed of in an approved manner.
Substation and Electrical Transmission Infrastructure	NA	0" to 6"	NA	NA	Blended to natural topography with a min. 1.0%	NA	NA	NA	See above description in Plant Area.
Other Disturbed Areas									Surfaces will be graded, ripped and covered with 6-inches of suitable cover material where unsuitable growth media exists.

Notes:

NA – Not Applicable

OPSDA – Open Pit Surface Drainage Area

¹ – Outslopes will be graded in accordance to 20.6.7.33.C(3) NMAC which states: "The outslopes of all tailing impoundments, waste rock and leach stockpiles at a copper mine facility shall be constructed to an interbench slope no steeper than three horizontal to one vertical (3H:1V). Alternative slope gradients may be allowed within an open pit surface drainage area, or if the permittee provides information showing that the cover performance objectives in Subsection F of this Section are met and the exception is approved by the department."

² – For the growth media stockpiles, it is assumed that the majority of the growth media will be used for cover on the reclaimed facilities and that there will be little to no growth media remaining following closure of these facilities (no outslopes, benches, or surface water conveyances).

³ – Cover systems for the EWRSPs, WRSPs, and TSF will be in accordance to 20.6.7.33.F NMAC which states: "Any cover system installed at an existing copper mine facility after the effective date of the copper mine rule shall be a store and release earthen cover system with a thickness of thirty-six inches..."

⁴ – The growth media stockpile areas, disturbed areas, and other ancillary facilities will not require additional cover unless sufficient native growth media is not present in the area (such as area underlain by bedrock). In areas with insufficient residual growth media, an additional six inches of cover material will be added to promote vegetative growth as part of the reclamation plan. The surfaces will be graded to drain and recontoured to blend into the natural topography, the graded area will then be ripped to a depth of between 12 and 18 inches and then seeded.

⁵ – In accordance with 19.10.5.507 NMAC "final surface configuration of the disturbed area shall be suitable for achieving a self-sustaining ecosystem or approved post-mining land use".

⁶ – Slopes lengths will be in accordance with 20.6.7.33.C(4) NMAC which states: "The maximum uninterrupted slope lengths shall be no greater than 300 feet for 4.0:1, 200 feet for 3:1 slopes and 175 feet for 2.5:1 slopes."

⁷ – The top surfaces of the EWRSPs and WRSPs will be graded in accordance to 20.6.7.33C(2) NMAC which states: "The top surfaces of all waste rock and leach stockpiles at a copper mine facility shall be constructed to a minimum final grade of 1%."

⁸ – The top surface of the TSF will be graded in accordance to 20.6.7.33. C(1) NMAC which states: "The top surfaces of all tailing impoundments at a copper mine facility shall be constructed to a minimum final grade of 0.5% after accounting for the estimated magnitude and location of large scale settlement due to totaling consolidation or differential settlement."

⁹ – Surface water conveyances will be constructed in accordance to 20.6.7.33. A NMAC which states: "Permanent storm water conveyances, ditches, channels, and diversions required for closure of a discharging facility at a copper mine facility shall be designed to convey the peak flow generated by the 100 year return interval storm event."



2.1 Existing Waste Rock Stockpiles

There are four existing waste rock stockpiles at the site, EWRSP-1, EWRSP-2A and 2B, EWRSP-3 and EWRSP-4 (**Figure E2**). The size and volume of the four EWRSPs are listed in **Table E2**.

Table E2. EWRSP Details

Facility	Size ¹ (Acres)	Volume ² (tons)
EWRSP 1	15.34	486,000
EWRSP 2A & 2B	21.06	760,050
EWRSP 3	19.54	333,300
EWRSP 4	22.62	1,000,050

Notes:

¹ – Includes stockpile areas and associated disturbance areas.

² – Provided by NMCC

NMCC will conduct contemporaneous reclamation of these EWRSP's to the extent practicable. EWRSP-1 is located at the western edge of the site within the open pit surface drainage area (OPSDA) and contains approximately 324,000 cy of waste rock. EWRSP-2, consists of two waste rock disposal areas and associated disturbed areas (EWRSP-2A and EWRSP-2B) located northeast of the open pit. Combined, these two disposal areas are estimated to contain approximately 506,700 cy of waste rock. EWRSP-3, located immediately east of the QMC crusher foundation and approximately 3,000 feet east of the pit, is estimated to contain approximately 222,200 cy of waste rock and unprocessed ore remaining on-site at the end of Quintana's operations. EWRSP-4 is located southeast of the open pit and contains approximately 666,700 cy of waste rock. NMCC will use a portion of the top surface of EWRSP-4 for an equipment/supply laydown yard during the future mining operations.

EWRSP-1, EWRSP-2B, and a portion of EWRSP-2A will be reclaimed during the pre-production phase of mine operations. EWRSP-3 will be reclaimed at the end of mine life as part of the plant area reclamation. The outslope of EWRSP-4 will also be reclaimed during the pre-production phase of mine operations. The top surface of EWRSP-4 will be regraded to drain toward the open pit to capture and route surface runoff during operations to the mine pit. EWRSP-4 will be reclaimed at the end of mine life after the area is no longer needed for an equipment/supply laydown yard. An earthen berm of growth media will be installed around the perimeter of the EWRSP-4 top surface during mining operations as safety measure for mining equipment operating in the area. It will also prevent surface water run-on onto the top surface from the outer edges of the stockpile and prevent runoff from impacting the reclaimed outslope.

Reclamation designs for the EWRSPs are based on an overall slope of 3H:1V with maximum 200-foot slope lengths. Final reclamation designs for the EWRSPs will be prepared and submitted to the agencies along with CQA/CQC plans within 180 days of submission of a notice of intent to implement the reclamation plan and may adjust the designs presented herein. The reclamation design criteria for the EWRSPs are summarized in **Table E1** and the design strategy for closure of these facilities are described below. Reclamation design drawings for the facilities are presented in **Attachment E1**.



2.1.1 Components To Remain At Closure

There are certain components and related engineering controls associated with the EWRSPs and associated disturbance areas that will be used for post-closure purposes, including:

- The existing Grayback Diversion Channel;
- The clean water diversion channel located at the north perimeter of EWRSP-2B;
- Access for small vehicles for future monitoring of the reclaimed areas; and
- Groundwater monitoring wells and surface water samplers that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.1.2 EWRSP Reclamation and Closure Plan

The approach for reclamation and closure of the EWRSPs is summarized below and the associated reclamation schedule and sequence is summarized in Section 4.0. The reclamation designs for the EWRSPs are detailed in sheets C-001 through C-006, C-015 and C-016 in **Attachment E1**. The approach for reclamation and closure of the EWRSPs include the following:

- Waste rock adjacent to the Grayback Diversion pulled back from EWRSP-1 or moved to provide clear separation between the final toe of the reclaimed stockpile and the bank of the Grayback Diversion channel;
- The portion of the EWRSP-2A that lies within the footprint of proposed WRSP-1 incorporated into this new stockpile;
- The portion of EWRSP-2A located outside of the OPSDA boundary relocated to the top of EWRSP-2B located inside the OPSDA boundary;
- Grading of the EWRSP top surfaces to a final grade of between 1 and 5% to direct storm water to slope drainage channels;
- Grading of the EWRSP outslopes down to an overall slope of 3.0H:1V with maximum uninterrupted slope lengths of 200 feet;
- Covering of the top surfaces and outslopes of the EWRSPs with 36 inches of growth media;
- Construction of two small detention basins on the eastern perimeter of EWRSP-1 to capture and evaporate minimal surface runoff from the western perimeter of the pit to protect surface water runoff from eroding the pit wall. The soils at the bottom of the basin will be compacted to inhibit water from percolating into the underlying rock formation and potentially creating an acid-generating pathway into the pit lake;
- Construction of an earthen protective safety/containment berm around the outer edge of the equipment/supply laydown yard on top of EWRSP-4 to prevent surface water run-on onto the top surface and to provide a safety berm for mining equipment operating in the area;
- Construction of surface water conveyance channels on the top surfaces (where required) to direct surface water off the covered stockpile surfaces. Surface water directed to the OPSDA for the top surfaces and interior slopes of EWRSPs 2B, 3, and 4 and to the exterior for the exterior slopes. Surface water directed to the exterior to Grayback Diversion for the top surfaces and slopes of EWRSP-1;
- Construction of energy-dissipation structures at channel outlets to reduce erosive velocities where necessary. Where possible channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock to promote long-term integrity of the structures;



- Construction of diversion swales and/or surface water conveyance channels to prevent storm water run on onto the EWRSPs;
- Grading and ripping of the disturbed areas associated with the EWRSPs to provide positive drainage. Where adequate growth media does not exist, areas will be covered with 6-inches of growth media;
- Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD, seeding of the equipment and material storage area on top of EWRSP-4 will be completed as part of the plant area reclamation; and
- Installation, operation, and maintenance of groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.2 Proposed Waste Rock Stockpiles

Waste rock will be hauled from the mine pit and placed in three proposed new waste rock stockpile (WRSP) areas shown on **Figure E3**. The planned sizes and capacities of the three WRSPs at the end of mine life are listed in **Table E3**.

Table E3. Proposed WRSP Details

Facility	Size ¹ (Acres)	Storage Volume ² (MT)
WRSP 1	39.71	3.16
WRSP 2	48.78	8.64
WRSP 3	121.35	32.89

Notes:

¹ – Includes stockpile areas and associated disturbance areas.

² – Provided by NMCC

MT – Million tons

Proposed waste rock stockpiles WRSP-1, WRSP-2 and WRSP-3 will be constructed in areas of the site that are completely underlain by andesite bedrock, a geologic formation that has a transmissivity of less than 10^{-6} centimeters per second (SRK, 2013). The proposed WRSPs will cover approximately 210 acres at final built-out, and will be constructed by end dumping in lifts approximately 75 feet high. The out slopes of the stockpiles will be built at angle of repose with benches sufficiently wide enough on each lift to allow grading the final slope and allow sufficient space for cross slope ditches.

Reclamation of the WRSP areas will be completed after the cessation of mining operations. However, select areas may be reclaimed contemporaneous during mine operations. NMCC is committed to maximizing contemporaneous reclamation activities, to the extent practicable, at the Copper Flat Project that will reduce erosion, provide early impact mitigation, limit costs and reduce final reclamation work. For example, portions of WRSP-3 may begin to be reclaimed by regrading and contouring of the WRSP beginning on the north side and proceeding south after the area is filled to capacity at the end of mining. Also, some roads that are no longer required for operations may be decommissioned and reclaimed contemporaneously during the active mining operation. Reclamation efforts will be implemented as soon as practicable in areas where activities are discontinued. This includes recontouring, scarifying, placement of topdressing or other approved growth media, followed by revegetation.



Reclamation designs for the proposed WRSP-2 and WRSP-3 are based on an inter-bench slope of 3.0H:1V, 25-foot wide terrace benches, and maximum 200-foot inter-bench slope lengths to allow for flexibility in the final design of the terrace benches. With these designs, the overall outslope gradient from the crest to toe is generally 3.5H:1V. For WRSP-1, which is located within the OPSDA, reclamation designs are based on an inter-bench slope of 2.75H:1V, 25-foot wide terrace benches, and maximum 200-foot inter-bench slope lengths. With these designs, the overall outslope gradient from the crest to toe for WRSP-1 ranges between 3.0H:1V and 3.5H:1V. Final reclamation designs for the WRSPs will be prepared and submitted to the agencies along with CQA/CQC plans within 180 days of submission of a notice of intent to implement the reclamation plan and may adjust the overall slopes presented herein. The reclamation design criteria for the proposed WRSPs are summarized in **Table E1** and the design strategy for closure of these facilities are described below. Reclamation design drawings for the facilities are presented in **Attachment E1**.

2.2.1 Components To Remain At Closure

The closure components and related engineering controls associated with the proposed WRSPs and stockpile areas that will be used for post-closure purposes include:

- Access for small vehicles maintained for future monitoring of the reclaimed areas; and
- Groundwater monitoring wells and surface water samplers that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.2.2 WRSP Reclamation and Closure Plan

The approach for reclamation and closure of the WRSPs is summarized below and the associated reclamation schedule and sequence is summarized in Section 4.0. The reclamation designs for the WRSPs are detailed in sheets C-007 through C-010 in **Attachment E1**. The approach for reclamation and closure of the WRSPs include the following:

- Grading of the proposed WRSP top surfaces to a final grade of between 1 and 5% to direct storm water to slope drainage channels;
- Grading of proposed WRSP-2 and WRSP-3 outslopes down to interbench slopes of 3.0H:1V;
- Construction of 25 foot wide terrace benches on the outslopes of proposed WRSP-2 and WRSP-3 at maximum slope lengths of 200 feet;
- Grading of proposed WRSP-1 outslopes down to interbench slopes of 2.75H:1V;
- Construction of 25 foot wide terrace benches on the outslopes of proposed WRSP-1 at maximum slope lengths of 200 feet;
- Covering of the top surfaces and outslopes of the proposed WRSPs with 36 inches of growth media;
- Construction of surface water conveyance channels on the top surfaces (as needed), terrace benches, and downslope channels to direct surface water off the covered stockpile surfaces. Surface water will be directed to the OPSDA for the top surfaces and interior slopes of WRSP-1 and WRSP-2, and to the exterior of the mine for WRSP-3;



- Construction of diversion swales and/or surface water conveyance channels to prevent storm water run on onto the stockpiles;
- Construction of energy-dissipation structures at channel outlets to reduce erosive velocities where necessary. Where possible channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock to promote long-term integrity of the structures;
- Grading of the disturbed areas associated with the stockpiles to provide positive drainage;
- Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD; and
- Installation, operation, and maintenance of groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.3 Tailings Storage Facility

The proposed TSF will include an HDPE-lined tailings impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam, and a water reclaim system to maximize water reuse. The TSF will also include a cyclone plant to separate the tailings coarse and fine fractions and a surge pond to handle potential upset conditions at the Copper Flat Project. The proposed TSF and associated ancillary facilities will cover approximately 564 acres at full capacity (**Figure E3**) and will be constructed using the centerline construction method. The proposed TSF design will comply with the design and dam-safety guidelines of the New Mexico Office of the State Engineer (OSE) Dam Safety Bureau. The tailings impoundment is designed to store 113 million tons of tailings produced over approximately 11 years, with tailings deposition occurring at an average rate of approximately 27,900 tons per day over the life of the mine. Additional details on the TSF design are provided in the Feasibility Level Design Report for the TSF (Golder, 2015).

The Feasibility Level Design Report for the TSF indicates that the maximum down-drain flow rate at final buildout of the dam is anticipated to be approximately 448 gallons per minute (gpm) from the dam underdrain and 66 gpm from the impoundment underdrain. The TSF embankment (dam) will be constructed of cyclone underflow sand fill which is coarser and has a higher permeability than the cyclone overflow material that is deposited within the TSF impoundment. This means that the TSF embankment will drain quickly in comparison to the impoundment and is, therefore, anticipated to undergo reclamation sooner than the impoundment surface. It is currently estimated that it will take approximately 2 to 3 years for the TSF embankment to drain sufficiently to begin reclamation. It is also anticipated that some reclamation of the impoundment can begin within 5 years of cessation of operations as the impoundment continues to drain and dry, allowing construction equipment to be utilized to commence cover placement. The underdrain systems will continue to operate after cessation of operations. An “active” underdrain water management program will commence thereafter, including pumping captured water from underdrain collection pond back to the impoundment surface of the TSF and use of forced or enhanced evaporation equipment to reduce the volume of the water. The duration of continued operation of the “active” water management system will, be driven by the volume of water that continues to drain from the impoundment.



For planning purposes NMCC has assumed that there will be 5 years of operation of the active program followed by 20 years of “passive” drain-down water management. After decommissioning of the active program and full reclamation of the TSF, any water that may continue to drain from the TSF (at ever decreasing rates for an estimated 20 years) will be captured in an evaporation pond that will be constructed below the toe of the TSF within the mine permit area. The underdrain collection pond will be incorporated into this evaporation pond. Details of the TSF drain-down water management program, including the design of the evaporation ponds, are included in **Attachment E2**.

Reclamation designs for the proposed TSF are based on an inter-bench slopes ranging between of 3H:1V and 3.5H:1V, 25-foot wide terrace benches, and inter-bench slope lengths of between 200 feet (3H:1V) and 250 feet (3.5H:1V). With these designs, the overall outslope gradient from the crest to toe ranges between approximately 3.0H:1V and 3.6:1V. Final reclamation designs for the TSF will be prepared and submitted to the agencies along with CQA/CQC plans within 180 days of submission of a notice of intent to implement the reclamation plan and may adjust the overall slopes presented herein. The reclamation design criteria for the proposed TSF are summarized in **Table E1** and the approaches for closure of this facility are described below. Reclamation design drawings for the TSF are presented in **Attachment E1**.

2.3.1 Components To Remain At Closure

The closure components and related engineering controls associated with the TSF area that will be used for post-closure purposes include:

- Access for small vehicles maintained for future monitoring of the reclaimed areas; and
- Groundwater monitoring wells and surface water samplers that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.3.2 TSF Reclamation and Closure Plan

The approach for closure of the proposed TSF is summarized below and the associated reclamation schedule and sequence is summarized in Section 4.0. The reclamation designs for the TSF are detailed in sheets C-011 and C-012 in **Attachment E1**. The approach for closure of the TSF is outlined below. The approach contains two fundamental components to successful completion of reclamation, i.e., a short-term active or forced water evaporation component and a long-term passive operation component. Utilization of active evaporation will allow the cover to begin to be placed on those areas of the top of the impoundment that become sufficiently “dry” to accept machinery. The goal of the active phase of evaporation is to dry the top of the impoundment as soon as possible to allow as much of the cover to be placed as possible, and eventually placing all of the cover on the impoundment. The active evaporation phase includes the following:

- Maintain the HDPE-lined runoff collection trench constructed at the toe of the dam to capture surface water runoff from the outer slopes of the dam and route it to the TSF underdrain collection pond prior to cover placement on the TSF embankment. Following



- cover placement on the embankment, surface water runoff will be routed to the nearest natural drainage;
- The storm water diversion ditches will be maintained to limit surface water run on onto the TSF;
 - The TSF underdrain collection system will be maintained to collect drain down water from the TSF and direct it to the TSF underdrain collection pond during the active evaporation period;
 - Maintenance of the HDPE-lined runoff collection trench at the toe of the dam to capture surface water runoff from the outer slopes of the TSF and route it to the TSF underdrain collection pond prior to cover placement, and to an offsite drainage after cover placement on the outslope;
 - Maintenance of the HDPE-lined TSF underdrain collection pond and associated pumps and piping to collect drain down and storm water runoff from the TSF (prior to cover placement) and direct it to the top of the TSF for active evaporation, or for direct passive evaporation within the pond;
 - Grading of the TSF embankment outslope down to interbench slopes of between 3.0H:1V and 3.5H:1V (estimated to begin 2-3 years after cessation of operations);
 - Construction of 25 foot wide terrace benches on the outslope of the TSF embankment at maximum slope lengths of between 200 feet (3H:1V) and 250 feet (3.5:1V);
 - Covering of the embankment outslopes of the TSF with 36 inches of growth media and seeding;
 - Begin grading of TSF impoundment as conditions allow (estimated to begin within 5 years of cessation of operations). Placement of growth media cover to the extent possible as conditions allow;
 - Closure of surge pond at the cyclone plant as described in Section 2.6;
 - All structures, tanks, storage facilities, buildings and equipment will be removed from the cyclone plant and TSF areas and disposed of in an approved manner according to applicable federal and state laws;
 - Concrete foundations will be broken and covered with 36 inches of growth media;
 - Pipelines will be removed or capped as described below in Section 2.8.
 - The electrical power systems and associated transmission lines will be removed once they are no longer needed as described below in Section 2.8.
 - Grading and ripping of the disturbed areas associated with the cyclone plant area to provide positive drainage. Where adequate growth media does not exist, areas will be covered with 6-inches of growth media;
 - Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD;
 - Construction of HDPE-lined evaporation pond for management of long term drain down water from the TSF through passive evaporation at end of active evaporation phase. (This includes consolidation of the underdrain collection pond into the evaporation pond system).

Upon completion of placement of the cover on the impoundment, active evaporation will no longer be necessary. However, the impoundment will continue to drain at an ever-decreasing rate, requiring that it continue to be captured and treated. A passive evaporation phase will be implemented so that captured drain-down water can continue to be managed through evaporation and the water not be placed on the



clean impoundment cover. For planning purposes it is assumed that the passive evaporation phase may last as long as 20 years after cessation of operations. NMCC, in coordination with the MMD, NMED and BLM, will determine when sufficient drain down has occurred to allow closure and reclamation of the passive evaporation system. The passive evaporation phase includes the following;

- The TSF underdrain collection system maintained to collect continued drain down water long-term, from the TSF and direct it to the evaporation pond during the passive evaporation period;
- Continue grading of the TSF top surface to a final grade of between 1 and 5% to direct storm water to the back side of the TSF;
- Complete covering of the top surfaces of the TSF with 36 inches of growth media and seeding, as conditions allow;
- Construction of surface water conveyance channels on the top surface of the TSF and outlet structure from the top of the TSF to Grayback Arroyo to direct surface water off the covered TSF surfaces. Surface water will be directed to Grayback Arroyo and other natural drainages upon completion of cover placement;
- Construction of energy-dissipation structures at channel outlets to reduce erosive velocities where necessary. Where possible channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock, which will promote long-term integrity of the structures;
- Grading of the disturbed areas associated with the TSF to provide positive drainage;
- Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD;
- Installation, operation and maintenance of groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC;
- The TSF evaporation pond will remain in place until drain down from the facility is reduced to a point to where it is no longer required for passive evaporation of these waters. This point in time will be determined in collaboration with the Agencies; and
- The evaporation pond will be reclaimed once it is no longer needed as described below in Section 2.6.2.

2.4 Open Pit

The open pit will be mined with pit benches ranging between 16 and 28 feet wide, creating a terraced/benched pit wall. Over the 11-year life of the proposed project, approximately 113 million tons of copper ore and 45 million tons of waste rock will be mined and removed from the open pit. The proposed mining activities will enlarge the open pit over time to a diameter of approximately 2,800 feet and an area of approximately 165 acres (including pit perimeter disturbed area). The floor of the proposed open pit will be mined to an elevation of approximately 4,650 feet above mean sea level (AMSL), which will be approximately 900 feet beneath the original surface of the Copper Flat Basin in this area.

The open pit will remain a hydrologic sink capturing groundwater flowing from all directions during operations and post-closure (JSAI, 2012; JSAI, 2013; JSAI, 2014; JSAI, 2014b; JSAI, 2014c; JSAI, 2015). NMCC will conduct rapid filling of the mine pit with water provided from the off-site well field as the initial



step in commencing reclamation/closure. The purpose of rapid filling of the pit is to provide a source of good quality water and provide a mechanism by which the mineralized rock walls of the pit will be more quickly submerged under water, thus limiting the potential for mineral oxidation. Approximately 2,800 acre-feet of water will be required for the rapid fill, pumping into the pit over approximately 7 months. Thereafter, pumping water for rapid fill will be discontinued and the elevation of the pit lake will reach an average steady-state condition of 4,900 feet above MSL over the long-term. Surface water runoff from the reclaimed facilities located within the OPSDA will be routed into the pit. Surface water conveyance channels will be constructed around the northern and eastern pit perimeter to direct surface runoff water into the pit. Two small surface water detention basins will be constructed along the western perimeter of the pit to capture and evaporate minimal surface runoff from the west perimeter of the pit. These features will limit the potential for erosion into and around the pit, provide stability to the pit walls, and provide riparian habitat to replace that lost in the mining operation.

The reclamation design criteria for the open pit are summarized in **Table E1** and the design strategy for closure of this facility are described below. Reclamation design drawings for the open pit are presented in **Attachment E1**.

2.4.1 Components To Remain At Closure

The closure components and related engineering controls associated with the open pit area that will be used for post-closure purposes include:

- Maintenance of existing storm water diversion structures to limit surface water run on into the open pit;
- Access for small vehicles maintained on the portion of the haul road remaining following construction of the open pit conveyance channel for future monitoring within of the open pit; and
- Groundwater monitoring wells in the vicinity of the open pit that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.4.2 Mine Pit Reclamation and Closure Plan

The approach for closure of the open pit is summarized below and the associated reclamation schedule and sequence is summarized in Section 4.0. The reclamation designs for the open pit area are detailed in sheets C-013 and C-014 in **Attachment E1**. The approaches for closure of the open pit area include the following:

- Rapid filling of the pit with 2,800 acre-feet of water from the off-site well field over a period of approximately 7 months;
- Construction of an earthen berm around the perimeter of the open pit to limit public access and ensure that the pit area does not pose a current or future hazard to public health or safety. The berm will be constructed from local rock and soils and will be 15 to 20-foot wide and 5- to 6-feet high with side slopes angled at 1.5H:1V;



- Construction of surface water conveyance channels around the perimeter of the pit (immediately upstream of the perimeter berm/security fence) to direct surface water around the pit and to the newly constructed open pit conveyance channel;
- Construction of the open pit conveyance channel along the existing haul road to direct surface water flows from around the perimeter of the pit to the pit lake;
- Construction of energy-dissipation structures at channel outlets to reduce erosive velocities where necessary. Where possible the channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock, which will promote long-term integrity of the structures;
- Grading of the disturbed areas associated with the pit perimeter, perimeter channels, and safety berm construction. It is assumed there will be an approximate 100-foot-wide disturbance area around the pit;
- Removal of aboveground electrical systems and infrastructure, including pumps, lighting and transmission lines not necessary for post-closure site operations and maintenance;
- Installation of a security gate at the haul road entrance into the pit to allow access for operation, maintenance, and monitoring activities to authorized personnel;
- Installation of a barbed wire fence around the outside perimeter of the pit to exclude livestock and other large mammals;
- Signs will be posted at 500-ft intervals along the security fence/earthen berm and at all access points, warning of potential hazards present;
- Seeding of disturbed areas around the pit perimeter to reestablish vegetation using a seed mix approved by the BLM and MMD; and
- Installation, operation, and maintenance of groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.5 Plant Area

The Copper Flat plant area will cover an area of approximately 82 acres located southeast of the existing open pit (**Figure E3**). The plant area includes the process area, primary crusher, concentrator area, laydown yard, administration/warehouse/mine shop, water tanks, process water reservoir, impacted stormwater impoundment A, tailings and process water conveyances, ancillary roadways, haul roads on the northern and southern/southwestern portion of the area, parking lot, wastewater treatment facility, fuel station, truck wash, and facilities supporting the process area. Additionally, due to its physical location within the plant area, an additional 23 acres which include EWRSP-3 will be incorporated into grading and drainage reclamation plan for the plant area.

At closure, all surface facilities, equipment and buildings will be removed from the area. The foundations associated with these structures will be buried in place. The slopes of the land bridge will be stabilized and the top revegetated during reclamation. The land bridge currently supports a wetland area, estimated to be 1.5 acres in size, and will be maintained as part of the closure plan. The reclamation design criteria for the plant area are summarized in **Table E1** and the approach for closure of this facility area is described below. Reclamation design drawings for the plant area are presented in **Attachment E1**.



2.5.1 Components To Remain At Closure

The closure components and related engineering controls associated with the plant area that will be used for post-closure purposes include:

- The Grayback Diversion Channel located along the southern perimeter of the plant area;
- The land bridge which conveys the tailings pipeline along the southeast corner of the area;
- Access for vehicles maintained following regrading EWRSP-3 and the access road on the western portion of the plant area for future monitoring and maintenance activities. Access through the plant area to the open pit and WRSPs north and west of the plant area, and for access to public and private lands north and west of the mine property; and
- Groundwater monitoring wells in the vicinity of the plant area that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.5.2 Plant Process Area Reclamation and Closure Plan

The approach for closure of the plant area summarized below and the associated reclamation schedule and sequence is summarized in Section 4.0. The reclamation designs for the plant area are detailed in sheets C-015 and C-016 in **Attachment E1**. The approach for closure of the plant area includes the following:

- All fuel tanks, reagent storage facilities, buildings and equipment will be removed from the site and disposed of in an approved manner according to applicable federal and state laws;
- Concrete foundations will be broken, walls toppled, backfilled, and covered with 36 inches of growth media;
- Closure of the process water reservoir and impacted storm water impoundment A as described below in Section 2.6.
- Pipelines will be removed or capped as described below in Section 2.8.
- The electrical substation and associated transmission lines will be removed once they are no longer needed as described below in Section 2.8.
- Ripping and grading of the disturbed areas associated with the plant area following burial of concrete foundations, slabs, and footings to provide positive drainage;
- Grading of EWRSP-3 top surface to a final grade of between 1 and 5% to direct storm water to slope drainage channels;
- Grading of the EWRSP-3 outslope down to a slope of 3.0H:1V;
- Covering of the top surface and outslope of the EWRSP-3 with 36 inches of growth media;
- Covering of ripped and graded disturbed areas with 6-inches of growth media;
- Construction of surface water conveyance channels to direct surface water off the covered surfaces. Surface water runoff will be directed to Grayback Arroyo;
- Construction of energy-dissipation structures at channel outlets to reduce erosive velocities where necessary. Where possible channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock, which will promote long-term integrity of the structures;
- Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD; and



- Installation, operation, and maintenance of any additional groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.6 Surface Impoundments

Table E4 presents a summary of the surface impoundment list for the Copper Flat Project. There will be 6 surface impoundments present at Copper Flat at the end of mine life; 3 impacted storm water impoundments, one process water reservoir, one surge pond and one underdrain collection pond. The seventh impoundment will be a passive evaporation pond constructed after mine closure to provide long-term passive evaporation of residual fluid drainage from the TSF.

Table E4. Summary of Copper Flat Surface Impoundments

Impoundment	Size ¹ (Acres)	Storage Volume ² (Gallons)
Impacted Storm Water Impoundment A	2.90	7,306,971
Impacted Storm Water Impoundment B	2.69	5,598,421
Impacted Storm Water Impoundment C	4.44	10,513,870
Process Water Reservoir	2.12	5,433,849
Surge Pond	1.86	1,610,000
TSF Underdrain Collection Pond	7.90	12,240,000
TSF Evaporation Pond	21.60	21,934,379

Notes:

¹ –Surface impoundment areas also include disturbed areas (embankment, access road, etc) associated with each impoundment.

² – Surface impoundment storage volumes account for 2-feet of freeboard.

Typically the surface impoundments will be the last features to be closed at their respective locations after grading and cover placement of the adjacent facilities (e.g., WRSPs, TSF, plant area, mine pit).

2.6.1 Components To Remain At Closure

The closure components and related engineering controls associated with the surface impoundments that will be used for post-closure purposes include:

- Groundwater monitoring wells in the vicinity of the plant area that are required for post-closure monitoring in accordance with 20.6.7.35.B NMAC.

2.6.2 Surface Impoundment Reclamation and Closure Plan

All of the surface impoundments will be reclaimed after the facilities they services are reclaimed. The TSF underdrain collection pond will continue to collect water that drains from the TSF during the active evaporation phase discussed above and will be incorporated into a single evaporation pond constructed for the management of water collected during the passive evaporation phase. The evaporation pond will be constructed below the toe of the TSF to allow for passive evaporation of any residual drain down water from the TSF. The evaporation pond will remain in place until drain down from the TSF facility is reduced to a point to where it is no longer required. This point in time will be determined in collaboration with the



Agencies and the pond will be reclaimed thereafter. The reclamation and closure activities for the surface impoundments consist of the following:

- Flushing of all process water pipelines to remove residual solutions and disposing of the solutions in the TSF to evaporate;
- Removing and disposing of the above-ground process pipelines in the TSF or at a nearby approved construction and debris landfill;
- Pumping of remaining water in the impoundments into the TSF to evaporate;
- Removal of all above ground impoundment electrical systems, pumps, and infrastructure, including outdoor lighting and transmission lines;
- Capping all buried process water, tailings delivery, and water delivery pipelines;
- Ripping surface impoundment HDPE liners and folding over prior to backfilling;
- Grading impoundment berms into and backfilling the impoundments with clean fill, as necessary, in lifts and compacting each lift;
- Grading the backfilled areas to drain and recontour surfaces to blend into the natural topography;
- Covering of impoundments with 6-inches of suitable cover material where unsuitable growth media exists after grading;
- Seeding of covered and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD; and
- Installation, operation, and maintenance of groundwater monitoring wells that may be required for post-closure monitoring in accordance with 20.6.7.35.B NMCC.

2.7 Growth Media Stockpiles

The term “growth media” as referred to within this Plan is equivalent to topdressing as defined in 19.10.1.7.T(1) NMCC. **Figure E3** identifies the location of three growth media stockpiles (GMSP-1, GMSP-2, and GMSP-3) established as part of the site development and construction phases of operation. GMSP-1 will be located at the southwest corner of the proposed TSF, GMSP-2 will be located north of the proposed TSF and north of Grayback Arroyo, and GMSP-3 will be located east of WRSP-3. Combined these stockpiles will contain an estimated 4.5 million cy of suitable growth media/cover material (**Table E5**).

Table E5. Proposed Growth Media Stockpile Details

Facility	Size ¹ (Acres)	Storage Volume ² (cy)
Growth Media Stockpile 1	29.33	2,180,000
Growth Media Stockpile 2	31.55	1,813,000
Growth Media Stockpile 3	14.10	508,000

Notes:

¹ – Includes stockpile areas and associated disturbance areas.

² – Provided by NMCC

cy – Cubic yards

The growth media stockpile material will be consumed as part of the cover systems associated with reclamation of the site. After moving this material from the stockpiles to its location of use in reclamation, there will be a need to also reclaim the area disturbed at the stockpile location. There may be a need to



leave a minimum of 6 inches of growth media material in place to provide for seeding the disturbed stockpile area. The design criteria associated with reclamation of any remaining growth media stockpile material that may be present following closure of these facilities, and the disturbed areas associated with the growth media stockpiles are summarized in **Table E1**, and the planned approach for closure of these facilities is described below.

2.7.1 Growth Media Stockpile Reclamation and Closure Plan

The approach for closure of any residual growth media stockpile material that may be present following closure of the facilities described in Sections 2.1 through 2.6 above, and the disturbed areas associated with the growth media stockpiles includes the following:

- Grading remaining growth media material and disturbed areas to drain and re-contour surfaces to blend into the natural topography;
- Ripping of the remaining growth media and disturbed areas to a depth of between 18 and 24 inches;
- Leave a minimum of six inches of growth media material in place, as needed, within the stockpile footprints. The only area that may require a minimum of six inches of residual stockpiled growth media to remain within the stockpile footprint is GMSP-3 which is underlain by andesitic bedrock. The other two stockpile areas (GMSP-1 and GMSP-2) are underlain by alluvial materials (suitable growth media); and
- Seeding of remaining growth media and disturbed areas to reestablish vegetation using a seed mix approved by the BLM and MMD.

2.8 Ancillary Facilities and Structures

A miscellaneous group of ancillary facilities and structures will be present within the Copper Flat mine permit area including: access roads utilized during operations; HDPE pipelines and trenches; electrical power distribution system and components; storm water structures for drainage, diversion, and sediment control; equipment storage areas; and fencing. The Copper Flat project also includes several off-site facilities that are integral to the project, including an electrical substation located on 30 acres of land owned by the State of New Mexico, nine separate 5-acre mill-site claim sites associated with the well field and an approximate 8-mile long fresh water buried pipeline contained in approximately 53 acres, 45 of which are off the site boundary.

Reclamation of the disturbed areas associated with the ancillary facilities and structures will be accomplished by burying utility and structure foundations, removing and disposing of above-ground pipelines and power lines on-site, and removing all buildings erected by NMCC. Erosion and drainage control and revegetation of these areas will be provided as necessary. Surface disturbance at the five acre mill sites will be reclaimed. The buried fresh water pipeline will be left in place. No reclamation will be required as the pipeline corridor contains natural vegetation consistent with the surrounding environs. The electrical substation will be the property of the service provider. It is assumed that the substation will continue to provide local infrastructure power supply to the area well beyond the time of cessation of



operation of the mine. As such, no reclamation requirement is contemplated. The closure components and the closure activities for the ancillary facilities and structures are described below.

2.8.1 Components to Remain At Closure

The closure components and related engineering controls associated with the ancillary facilities and structures that will be used for post-closure purposes include:

- On-site power poles may be left in place to the extent possible as bird perching sites;
- Continued utilization of access roads for post closure access to the pit bottom and reclaimed facilities for reclamation monitoring and long-term access to public and private lands;
- Continued utilization of storm water control structures located along post-closure haul roads and access roads;
- The existing 115-kV transmission line and the electrical substation constructed on State land will be left in place. The local power utility owns these facilities and will be responsible for their continued operation and maintenance; and
- Preservation of existing water supply pipeline from four production wells located off-site on BLM land. The wells will remain in a condition suitable for other uses. All roads, power lines and foundations for the production wells are in place. No additional disturbance will occur during mining operations, with the exception of occasional minor disturbance that may occur during inspection and maintenance. Such disturbances will be repaired and reclaimed as they may be needed during operations. Surface structures and equipment will be removed and the well area will be left as it currently exists after closure of the mine.

2.8.2 Ancillary Facilities and Structures Reclamation and Closure Plan

The design criteria for the ancillary facilities and structures are summarized in **Table E1** and the approach for closure includes:

- Haul roads and access roads not needed for closure and post-closure access will be reclaimed. The compacted road material will be loosened by ripping to a depth of between 18 and 24 inches and revegetated using a seed mix approved by the BLM and MMD. Covering of haul roads with 6-inches of suitable cover material where unsuitable growth media exists after ripping and revegetated using a seed mix approved by the BLM and MMD;
- Removal of culverts not needed for post-closure storm water management and disposal of them in an approved manner;
- Removal of overhead lines and power poles and disconnect from the 115kV line owned by Tri-State Generation and Transmission;
- Removal of pumping stations and on-site electrical substation once they are no longer needed for post-closure water management;
- Removal of residual sediments and fluids from pipelines and disposal of materials in the TSF prior to reclamation of this facility, or at an approved location;
- Removal and disposal of the above-ground pipelines in the TSF prior to reclamation of this facility, or at a nearby approved construction and debris landfill;
- Capping all buried pipelines;
- Ripping HDPE liners within corridors;



- Backfilling of pipeline corridor trenches with clean fill, as necessary, in lifts and compacting each lift;
- Grading the backfilled areas to drain and recontour surfaces to blend into the natural topography;
- Covering of pipeline corridors with 6-inches of suitable cover material where unsuitable growth media exists after grading;
- Ripping of non-impacted disturbed areas to a depth of 18 to 24 inches; and
- Seeding of ripped and covered areas to reestablish vegetation using a seed mix approved by the BLM and MMD;

2.9 Post-Mining Land Use

The NMMA Rules (MMD 1996) defines Post-Mining Land Use (PMLU) as:

“a beneficial use or multiple uses which will be established on a permit area after completion of a mining project. The PMLU may involve active management of the land. The use shall be selected by the owner of the land and approved by the Director [of MMD]. The uses, which may be approved as PMLUs, may include agriculture, commercial or ecological uses that would ensure compliance with Federal, State or local laws, regulations and standards and which are feasible.” 19.10.1.7. P (5) NMAC

The major land uses in the vicinity of the project area are mining, grazing, wildlife, watershed and recreation, particularly on federal lands. This multiple land management strategy concurs with the BLM administrative directives and resource management plans. Land use in the project area will not change from pre-mining approved purposes and the project area will continue to support these approved uses following closure. Post-closure land uses at Copper Flat will conform to the previously defined BLM Caballo Planning Unit, the 1986 White Sands Resource Management Plan (BLM, 1986) and the Sierra County Comprehensive Land Use Plan.

Grazing land, wildlife habitat, recreation, and mining are the PMLUs most consistent with the surrounding land uses of the Copper Flat site. NMCC will reclaim all disturbed areas in the permit boundary to support the multiple uses of grazing, wildlife habitat, and mining consistent with current BLM land uses. At completion of mining activities, the site will be reclaimed to a native plant community similar to surrounding undisturbed areas. Reclamation will result in the development of an early-stage grass/shrub community that will provide a locally-important increase in landscape-level (plant community) diversity. Native vegetation established on reclaimed areas at Copper Flat will result in increased erosion protection, habitat improvement, forage production, and reduced net infiltration of water into the underlying materials relative to current conditions.

The open pit area will be reclaimed to the extent practicable to a wildlife habitat. A water conveyance channel will be constructed along the existing pit haul road to direct surface water flows to the pit lake. As a rule, catch benches left in pit walls and/or inaccessible benches will not be revegetated after mining for safety reasons. Safety protocols do not permit personnel or equipment to operate below a highwall, in areas



with no safe access, or at the rim of the open pit. These areas will be allowed to revegetate themselves through natural processes. The open pit walls and benches will become wildlife habitat providing abundant rock outcroppings and cliff habitat suitable for small mammals, reptiles and birds.



3.0 IMPLEMENTATION OF RECLAMATION AND CLOSURE PLAN

At completion of mining activities, Copper Flat will be reclaimed according to the reclamation and closure plan described in Section 2.0 and the site will be restored to conditions to meet post-mining land uses of wildlife habitat, grazing land, recreation, and mining consistent with the BLM land management plan (BLM, 1986). The focus of this section is to detail regrading operations, growth media placement and the revegetation plan including planting techniques and proposed seed mixes.

3.1 Growth Media Placement

The use of the term “growth media” herein is synonymous with the term “topdressing” as defined in the Mining Act regulations. The major guiding elements in developing the grading plans for the major facility areas and ancillary facilities at Copper Flat are to achieve positive drainage, facilitate constructability and the efficient conveyance of water and limit slope length and gradient, soil erosion and eliminate long-term maintenance requirements. Prior to cover placement, top surfaces and disturbed areas will require minor grading to fill rills, enable the construction of surface water control features and ensure that the final grade is between 1 and 5 percent for reclaimed waste storage units, and graded to blend into the natural topography for other disturbed areas. More extensive grading will be required on the slopes to achieve the desired slope configuration, smooth the bed materials and accommodate surface water control features.

Once facilities are regraded, cover materials will be hauled from growth media stockpiles and placed on the top surface and slopes using a variety of equipment including scrapers or haul trucks. Bulldozers and motor graders will be used to smooth the surfaces and facilitate access for cover placement and revegetation activities. Reclamation of EWRSP-1, EWRSP-2B, a portion of EWRSP-2A and the outslope of EWRSP-4 will occur during the pre-production phase of mine operations. Suitable growth media for these existing facilities will be excavated from TSF and/or WRSP-2 and -3 borrow areas and hauled directly as part of the plans to reclamation contemporaneously during the pre-production phase of mine operations (Section 2.1).

The cover system will be designed to provide erosion control, sustain vegetation and reduce net infiltration of stormwater through the underlying materials. The soil cover will be a monolithic store and release/evapotranspiration system. Where mine wastes are present (EWRSPs, WRSPs, and TSF) soil covers will be 36 inches thick, and other disturbed areas that do not have adequate growth media already in place will be covered with 6-inches of soil. The covers will be designed to be protective of groundwater, resist erosion and support vegetation. Growth media to construct the cover will be spread and graded with care taken to prevent compaction by limiting the number of passes.

NMCC will construct the soil covers in single lift. However, there may be occasions during cover construction when the subsurface and surface cover materials may be placed in separate operations. In these instances, NMCC will place coarser textured growth media materials in the surface lift, particularly on out slopes. Coarser textured soils materials are expected to have better performance related to their



ability to resist erosion and capture water (high infiltration capacity) during high intensity precipitation events that occur during the summer monsoons (Golder, 2013).

Growth media will be placed at a depth of at least 6 inches in all other disturbance areas. GMSP-1 and -2 are underlain by alluvial deposits that are suitable growth media. As such, the existing deposits underlying these stockpiles will simply be graded, scarified, and reseeded. Removal of growth media from GMSP-3 may expose bedrock that will require a minimum of 6 inches of stockpiled growth media to remain within the stockpile footprint prior to reseeded. **Table E6** provides an estimate of the volume of growth media required for reclamation.

Table E6: Estimated Reclamation Cover Requirements

Facility	Regraded Surface Area ¹ (acres)	Cover Thickness (ft)	Growth Media Requirement (cy)
EWRSP-1 ²	17.5	3	84,700
EWRSP-2A ^{2,3}	8.3	0	0
EWRSP-2B ^{2,3}	5.1	3	24,684
EWRSP-3	19.5	3	94,574
EWRSP-4 ²	22.6	3	109,481
WRSP-1	41.9	3	202,796
WRSP-2 and WRSP-3	171.8	3	831,512
TSF	564.4	3	2,731,696
Plant Area (excluding EWRSP-3)	78.9	0.5	63,646
Surface Impoundments ⁴	36.2	0.5	25,168
Open Pit ⁵	165.3	0	0
GMSP-1	29.3	0	0
GMSP-2	31.6	0	0
GMSP-3	14.1	0.5	11,374
Ancillary Facility Areas ⁶	19.7	0.5	15,891
Total	1,226.2	-	4,195,522

Notes:

¹ - Regraded areas based on reclamation and closure designs presented in Attachment E1.

² - EWRSP-1, EWRSP-2B, and a portion of EWRSP-2A will be reclaimed during the pre-production phase of mine operations. The outslope of EWRSP-4 will also be reclaimed during the pre-production phase of mine operations. The top surface of EWRSP-4 will be reclaimed following cessation of mining.

³ - The portion of the EWRSP-2A that lies within the footprint of proposed WRSP-1 and will be incorporated into this new stockpile. The portion of EWRSP-2A located outside of the OPSDA boundary will be relocated to the top of EWRSP-2B and the disturbed area will be ripped and seeded. EWRSP-2B includes 5.1 acres of waste rock stockpile that will get covered and 7.6 acres of disturbed area that will get ripped and seeded.

⁴ - Impacted Stormwater Impoundment A and the Process Water Reservoir cover requirements are already covered within the Plant Area and are excluded in the cover volume calculation.

⁵ - Open pit area (132.1 acres) and associated disturbed area around the pit perimeter (33.2 acres) that will get ripped and seeded.

⁶ - Includes ancillary facilities and structures not already included in one of the specific facilities listed within this table. Includes haul and access roads, electrical power distribution system; storm water and sediment control structures; equipment storage areas; pipeline corridors; pump stations; tanks; explosives magazine and associated access road; and fencing.



3.2 Revegetation Plan

The revegetation plan for the Copper Flat Project is designed to create a stable, self-sustaining plant community and will conform to the planned grazing and wildlife habitat PMLU for areas outside the open pit. Revegetation of the site will consist mainly of the establishment of grass, forb and shrub species characteristic of the desert grassland community. Plant species were chosen based on their ability to provide satisfactory cover, on their nutritional value and ability to support livestock production and wildlife habitat. General planting techniques and proposed seed mixtures are provided below.

3.2.1 Planting Techniques

In general, revegetation operations will follow immediately after cover material placement, or after grading and ripping for disturbed areas that don't require additional cover, and will be timed to take advantage of summer moisture to encourage the establishment of warm season grasses. Thus, to the extent practicable, soil placement and reclamation seeding will occur prior to the monsoon season of July, August and September.

Revegetation will be performed in a manner consistent with industry standards to promote erosional stability and support the post-mining land use. The general order agronomic practices for revegetation seeding are ripping or scarification, disking, seeding (drill, broadcast or hydroseed), mulching and crimping or tackifying. These practices are discussed in more detail below.

After placement, the cover will be scarified (ripped 8 to 12 inches) to break up compaction and roughen the surface. The ripping operation will be implemented on the contour for sloping areas to reduce the potential for early-stage soil erosion during vegetation establishment. The roughened surface is a transient condition that provides micro-sites for seedling establishment and to reduce concentrated overland flow and erosion. Prior to seeding, the seed bed will be prepared by disking or harrowing to a depth of approximately 6 inches. If soil amendments are required, disking to prepare the seedbed will take place after applying the amendments. Ripping and other seedbed preparation procedures will be conducted when surface and subsurface soil moisture conditions are dry to avoid compaction.

Specific seeding methods to be utilized at the site are dependent on many factors including the topography, surface conditions, seed mixture and equipment availability. Specialized rangeland drills are available for seeding native seed mixes. They are equipped with an agitator and depth bands to mix seed and ensure proper seeding depths. Alternatively, seed may be broadcast and covered using a drag or hydro-seeding may be used on steep, small areas where larger equipment cannot easily operate. In most cases, seed will be planted using a rangeland drill or similar equipment. Wherever possible, seeding will be done along the contour. When drill seeding is not practical due to steep slopes or wet soil conditions, broadcast seeding will be employed. For broadcast seeding, the drill seeding rate will be doubled and areas will be raked with a chain- or tire-harrow to lightly cover the seed and achieve good soil-seed contact.



Following seeding, certified weed-free mulch will be uniformly spread at a rate of about 2 tons/acre. Mulch will contain a minimum of viable seeds associated with the source (i.e., barley or wheat). Long-stem mulch will be given preference over shorter materials. The mulch will be then be crimped with a straight-disc harrow or similar equipment to fix it in place. On steep slopes, tackifier emulsion may be used to secure the mulch rather than crimping.

Weed control will be implemented only if necessary. Methods of weed control will be determined upon recommendations from the BLM and/or MMD.

3.2.2 Seed Mixtures

Table E7 provides the proposed interim seed mix (primarily associated with the seeding of the growth media stockpiles) and final seed mixes for the Copper Flat Project for the grazing and wildlife PMLUs. The seed mixtures include native warm and cool season grasses, perennial shrubs and forbs. **Table E8** provides the primary functions and attributes of each proposed plant species. The species selected for the reclamation seed mixtures have been successfully used in mine reclamation and range improvement projects in many parts of New Mexico and are readily available from seed suppliers. The seed mix is selected to provide early establishment of ground cover, erosion control and productivity while providing diversity in growth forms.

The seed mixes are designed for application prior to the summer rains and the seeding will be completed in early- to mid-June. The ratio of cool season to warm season grasses may be adjusted if the seeding is conducted after the summer rains. The overall target seed rate for final seeding is expected to vary, but will range from about 40 to 60 seeds per square foot. Interim seedings for growth media stockpiles and other temporary stabilization seedings target a seed density of 30 seeds per square foot. All seed mixes shall be certified as weed free.

NMCC may propose to adjust the seeding rates or species listed in **Table E7** based on seed availability or to accommodate variations in seeding methods (e.g., broadcast, drill, hydraulic) and field conditions. A list of alternate or substitute species is included in **Table E9**. Based on the performance on the interim seed mix and seeding associated with the contemporaneous reclamation of the existing stockpiles, the final seed mix may be modified with approval of the MMD.



Table E7: Interim and Final Reclamation Seed Mixes

Scientific Name	Common Name	PLS/ac ¹	
		Interim	Final
Grasses – Warm Season			
<i>Bothriochloa barbinodis</i>	Cane bluestem	0.15	0.20
<i>Bouteloua curtipendula</i>	Sideoats grama	1.00	1.10
<i>Bouteloua gracilis</i>	Blue grama	0.20	0.25
<i>Pleuraphis jamesii</i>	Galleta	0.75	1.10
<i>Leptochloa dubia</i>	Green sprangletop	0.15	0.20
<i>Seteria vulpiseta</i>	Plains bristlegrass	0.20	0.30
<i>Sporobolus cryptandrus</i>	Sand dropseed	0.03	0.04
Grasses – Cool, Intermediate Season			
<i>Achnatherum hymenoides</i>	Indian ricegrass	0.60	1.30
<i>Eragrostis intermedia</i>	Plains lovegrass	0.05	0.04
<i>Hesperostipa newmexicana</i>	NM feathergrass	0.70	0.50
Shrubs			
<i>Atriplex canescens</i>	Four-wing saltbush	0.30	1.75
<i>Ericamerica nauseosus</i>	Rubber rabbitbrush	0.10	0.35
<i>Fallugia paradoxa</i>	Apache plume	--	0.10
<i>Krascheninnikovia lanata</i>	Winterfat	0.15	0.70
Forbs			
<i>Dalea candida</i>	White prairie clover	0.10	0.40
<i>Linum lewisii</i>	Blue flax	0.15	0.35
<i>Ratibida colomnifera</i>	Prairie coneflower	--	0.10
<i>Sphaeralcea ambigua</i>	Desert globemallow	0.10	0.40
	Total	4.73	9.18

Notes:

¹- Rate is in pounds of pure live seed (PLS) per acre; Substitutions may change seeding rates.



Table E8: Functions and Attributes of the Primary Plant Species

Species	Character ¹	Attributes and Function
Cane beardgrass (<i>Bothriochloa barbinodis</i>)	N,P,W,G	Bunch grass providing ground cover and forage
Blue grama (<i>Bouteloua gracilis</i>)	N,P,W,G	Drought resistant sod grass providing ground cover and forage
Side-oats grama (<i>Bouteloua curtipendula</i>)	N,P,W,G	Drought tolerant bunchgrass providing ground cover and forage
Galleta (<i>Pleuraphis jamesii</i>)	N,P,W,G	Bunchgrass providing erosion control and early spring/late fall forage
Green sprangletop (<i>Leptochloa dubia</i>)	N,P,W,G	Erect bunchgrass; aggressive short-lived nurse plant with forage value
Plains lovegrass (<i>Eragrostis intermedia</i>)	N,P,I,G	Bunchgrass providing ground cover and early spring forage
Plains bristlegrass (<i>Setaria vulpiseta</i>)	N,P,W,G	Palatable bunchgrass with valuable seed for upland birds and small mammals
NM needlegrass (<i>Hesperostipa neomexicana</i>)	N,P,C,G	Persistent bunch grass providing ground cover and forage
Sand dropseed (<i>Sporobolus cryptandrus</i>)	N,P,W,G	Drought tolerant bunchgrass adapted to sandy sites
Indian ricegrass (<i>Achnatherum hymenoides</i>)	N,P,C,G	Tufted grass providing forage/seed to birds and small mammals
Apache plume (<i>Fallugia paradoxa</i>)	N,P,S	Mid-height shrub providing browse, cover and erosion control
Four-wing saltbush (<i>Atriplex canescens</i>)	N,P,S	Slightly evergreen shrub providing cover/forage for wildlife and livestock
Winterfat (<i>Krascheninnikovia lanata</i>)	N,P,HS	Low shrub providing nutritious winter browse
Rubber rabbitbush (<i>Ericamerica nauseosus</i>)	N,P,S	Mid-height shrub providing cover and erosion control
Desert globemallow (<i>Sphaeralcea ambigua</i>)	N,P,F	Persistent mid-height forb providing browse for deer and antelope
Prairie coneflower (<i>Ratibida colomnifera</i>)	N,P,F	Red and yellow flowered forb attracting pollinators
White prairie clover (<i>Dalea candida</i>)	N,P,F	Nitrogen-fixing forb with low water requirements providing forage and ground cover
Blue flax (<i>Linum lewisii</i>)	N,P,F	Persistent blue-flowered forb, nutritious seed for ground birds

Notes:

¹ - N = Native; P = Perennial; W = Warm season; C = Cool season; I = Intermediate season; G = Grass; S = Shrub; HS = Half shrub; F = Forb

**Table E9: Alternative or Substitute Plant Species for Seed Mixtures**

Scientific Name	Common Name
Grasses	
<i>Andropogon saccharoides</i>	Silver bluestem
<i>Aristida purpurea</i>	Purple three-awn
<i>Bouteloua eriopoda</i>	Black grama
<i>Eragrostis curvula</i>	Weeping lovegrass
<i>Digitaria californica</i>	Arizona cottontop
<i>Hesperostipa comata</i>	Needle and thread
<i>Heterotheca contortus</i>	Tanglehead
<i>Panicum obtusum</i>	Vine mesquite
<i>Pleuraphis mutica</i>	Tabosa
<i>Sporobolus contractus</i>	Spike dropseed
Shrubs	
<i>Calliandra eriophylla</i>	Fairyduster
<i>Isocoma tenuisecta</i>	Burroweed
<i>Lycium pallidum</i>	Wolfberry
<i>Nolina microcarpa</i>	Beargrass
Forbs	
<i>Baileya multiradiata</i>	Desert marigold
<i>Coreopsis lanceolata</i>	Lanceleaf tickseed
<i>Machaeranthera tanacetifolia</i>	Prairie aster
<i>Penstemon parryii</i>	Parry's penstemon



4.0 RECLAMATION SCHEDULE AND SEQUENCE

Table E10 presents the anticipated reclamation schedule and sequence for the Copper Flat Mine based on best available information and mine planning forecasts. The proposed schedule summarizes NMCC's understanding of the near-term mine operation and longer-term mine plan projections. More specifically, the schedule is based on the following considerations:

- Practical phasing of the reclamation projects to account for the anticipated labor, equipment and other resources that will be necessary to complete these projects based on current conditions;
- Sequential closure of facilities in a phased cost efficient manner; and
- Total annual acreages that will be reclaimed over this period.

The anticipated duration for reclamation presented in **Table E10** include earthwork and reseeding, but do not include vegetation success/O&M/monitoring that will be conducted throughout the post-closure monitoring period as described in Section 5. The reclamation schedule is based on the number of years from the time NMCC obtains the required Mine permits and approvals to begin operations. Contemporaneous reclamation of EWRSP-1, EWRSP-2, and portions of EWRSP-4 will begin during the initial mine pre-production period.



Table E10. Copper Flat Reclamation Schedule and Sequence

Reclamation Facility Area	Reclamation Schedule, Year Following Mine Permit Approval ¹	
	Start	Finish
<i>Existing Waste Rock Stockpiles</i>		
EWRSP-1, EWRSP-2A, and EWRSP-2B (Placement of EWRSP-2A Stockpile Material Outside OPSDA onto the top of EWRSP-2B, Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 2	Year 2
EWRSP-4 (Regrading, Cover Placement on Outcrops, Revegetation Outcrops, Revegetation of Top Surface,	Year 2	Year 2
EWRSP-4 (Cover Placement on Top Surface, Conveyance Channel Construction,	Year 14	Year 14
EWRSP-3 ² (Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 15	Year 16
<i>Proposed Waste Rock Stockpiles</i>		
WRSP-1 (Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 13	Year 16
WRSP-2 (Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 14	Year 16
WRSP-3 ³ (Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 10	Year 16
<i>Proposed Tailings Storage Facility</i>		
Tailings Storage Facility (Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 15	Year 21
Active Evaporation of Drain Down Waters	Year 12	Year 18
Construction of Evaporation Pond	Year 17	Year 18
Passive Evaporation of Drain Down Waters	Year 18	Year 38
Evaporation Pond Closure (Ripping Liner and Backfilling Pond, Regrading and Cover Placement, Revegetation)	Year 39	Year 39
<i>Proposed Plant Area Facility²</i>		
Plant Area (Building/Structure Demolition & Removal, Regrading and Cover Placement, Conveyance Channel Construction, Revegetation)	Year 15	Year 16



Table E10. Copper Flat Reclamation Schedule and Sequence (Continued)

Reclamation Facility Area	Reclamation Schedule, Year Following Mine Permit Approval ⁽¹⁾	
	Start	Finish
<i>Proposed Open Pit</i>		
Rapid Fill of Pit Lake	Year 13	Year 14
Pit Perimeter (Construction of Pit Perimeter Conveyance Channels Perimeter Berm/Fencing and Cover Placement, Construction of Haul Road Conveyance Channel, Revegetation)	Year 15	Year 15
<i>Ancillary Facilities⁴</i>		
Building/Structure/Pipeline Demolition, Removal, and Burial, Regrading and Cover Placement, Facility Revegetation	Year 16	Year 20

Notes:

- ¹ - The reclamation schedule is based on the number of years from the time NMCC obtains all of the required Mine permits and approvals to begin operations.
- ² - Reclamation of the plant area will also include reclamation of EWRSP-3 and the haul roads running through the plant area.
- ³ - Reclamation of the WRSP-3 may begin contemporaneous with mining operations.
- ⁴ - Includes ancillary facilities and structures not already included in one of the specific facilities listed within this table. Includes haul and access roads, electrical power distribution system; storm water and sediment control structures; equipment storage areas; pipeline corridors; pump stations; tanks; explosives magazine and associated access road; and fencing.



5.0 PERFORMANCE AND RECLAMATION STANDARDS AND REQUIREMENTS

5.1 Most Appropriate Technology and Best Management Practices

The reclamation and closure plan has been designed to protect human health and safety, the environment, wildlife and domestic animals using Most Appropriate Technology (MAT) and BMPs. MAT in mine operations is understood as the selection and application of the most suitable techniques, practices, or methods of operation that have been proven effective in achieving the intended purpose or performance standard while reducing impacts to the environment (prevent, reduce or control pollution). The selection of a MAT is typically accomplished in mine feasibility studies that evaluate mining technologies, processes and operating methods relative to site-specific conditions. The Copper Flat Project will be designed and operated using both MAT and BMPs based on site-specific technical and economic feasibility. Mining technologies, processes and operating methods proposed by NMCC are provided in Section 2.0 of Updated Mine Operations and Reclamation Plan.

BMPs are defined as any program, technology, process, siting criteria, operating method, measure or device, which controls, prevents, removes or reduces impacts to the environment. BMPs are currently accepted, effective and practical methods including structural or engineered control devices, systems and materials as well as operational or procedural practices used to prevent or reduce environmental impacts of ground disturbing activities.

NMCC will meet or exceed applicable state and federal reclamation requirements through application of MAT and BMPs. NMCC has designed its reclamation and closure plan to use the most appropriate technology for an open pit mine operation. Structural and operational BMPs will be used to stabilize the site, limit erosion, reduce sediment control fugitive dust, and prevent noxious weeds from existing and proposed facilities and disturbed areas during reclamation. These BMPs include:

- Surface stabilization measures – dust control, regrading, mulching, riprap, temporary and permanent revegetation/reclamation and placing growth media;
- Run-on and runoff control and conveyance measures – clean water diversions and swales, armored drainage channels, runoff diversions and berms;
- Sediment traps and barriers – check dams, grade stabilization structures, sediment detention, sediment/silt fence and straw bale barriers and sediment traps;
- Air quality – apply water to control dust on haul roads and other disturbance areas, interim seeding of growth media stockpiles and other surface disturbance areas
- Revegetation and noxious weeds – use of certified weed-free seed and mulch, cleaning heavy equipment before entering the mine area, noxious weed monitoring and treatment.

BMPs will be employed in appropriate sites during the reclamation phase of the Project and structures will be inspected periodically, with repairs performed as needed. NMCC will limit disturbance and preserve existing vegetation to the maximum extent possible. Additional details regarding structural and operational BMPs are included in Section 4.0 of the Updated Mine Operations and Reclamation Plan.



5.2 Contemporaneous Reclamation

Contemporaneous reclamation, mining-for-closure, or “designing for closure” will reduce erosion, provide early impact mitigation, limit costs and reduce final reclamation work. NMCC is committed to maximize this type of reclamation at the Copper Flat Mine Project and has designed mine facilities to employ contemporaneous reclamation, to the extent appropriate and practicable. Specific details regarding contemporaneous reclamation of the EWRSP-1, -2B and -4 and some outcrops of the proposed WRSP-3 are provided in Section 2.1 and 2.2 respectively. Other examples include, designing the waste rock stockpiles in such a way to facilitate their reclamation, building final lifts to accommodate the required reclamation slopes, and managing tails deposition while constructing the embankment to achieve the desired outcrop grade.

5.3 Protection Assurance

NMCC has designed a reclamation plan to assure protection of human health and safety, the environment, wildlife and domestic animals. Mine development and operation activities will also be implemented to assure protection of human health and safety, the environment, wildlife and domestic animals. Specific details of the protection assurances associated with signs, markers, and safeguarding [603.C (1)]; wildlife protection [603.C (2)]; cultural resources [603.C (3)]; hydrologic balance [603.C (4)]; stream diversions [603.C (5)]; impoundments [603.C (6)]; mass movement minimization [603.C (7)]; riparian and wetland areas [603.C (8)]; roads [603.C (9)]; subsidence control [603.C (10)]; and explosives [603.C (11)] are provided in Section 4.3 of NMCC’s Updated Mine Operations and Reclamation Plan.

5.4 Site Stabilization and Configuration

The permit area will be stabilized, to the extent practicable, to prevent future impact to the environment and protect air and water resources. The final surface configurations for the Project are presented in **Figure E4** and will be suitable to achieve the grazing, wildlife habitat, and mining PMLUs. All facilities, slopes, embankments and roads will be designed, constructed, maintained and reclaimed to achieve stable configurations within the industry’s and engineering “standard of care” design process. Specific details on the steps to be taken to stabilize and configure to minimize future impact to the environment and protect air and water resources are provided in Section 4.4 of NMCC’s Updated Mine Operations and Reclamation Plan.

5.5 Topdressing and Cover Materials

Topdressing, for the purposes of reclamation, refers to any suitable soil or geological material used as growth media or soil cover for establishing native vegetation on sites disturbed by mining activities. Reclamation suitability is based on the material’s ability to provide erosion control, sustain vegetation, and reduce net infiltration into the underlying materials.



As previously described, growth media as referred to within this Mine Reclamation and Closure Plan is equivalent to topdressing as defined in 19.10.1.7.T(1) NMAC. Growth media removal quantities have been estimated on the basis of the Supplemental Soils Investigation (Golder, 2013) and are discussed below. The majority of the cover materials required to support revegetation and reclamation efforts will be obtained from within the footprints of the proposed TSF, WRSP-2 and WRSP-3.

The proposed soil cover system for the Copper Flat Project is a monolithic store-and-release or evapotranspiration (ET) cover. A store-and-release cover system stores precipitation during wet periods and releases the moisture back to the atmosphere via evapotranspiration during dry periods, reducing net infiltration. Where mine wastes are present, soil covers will be 36 inches thick unless NMCC can demonstrate a thinner cover can be protective of groundwater (per NMAC 20.6.7.33.F), resist erosion and support vegetation. Other reclamation units including the plant site, roads, other ancillary facilities, and disturbed areas that do not contain adequate growth media will require a minimum of 6 inches of cover. GMSP-1 and 2 which are underlain by unconsolidated alluvial materials will not require additional cover. Additional information regarding cover material requirements for reclamation are provided in Section 3.1.

5.5.1 Suitability

The suitability of topdressing/cover materials is based on the material's ability to provide erosion control, sustain vegetation, and reduce net infiltration. The ability of the materials to meet these performance objectives is primarily related to their physical properties, specifically the texture and rock fragment content of the final cover system (Golder, 2013). Cover materials identified within the limits of the TSF and WRSP-2 and -3 were found to be generally suitable from a physical perspective, though many surficial soils are fine-textured (high clay and silt content) that could be present challenges with respect to revegetation and soil erodibility. In general, soils and underlying colluvial and alluvial materials in the permit area are considered suitable and have no chemical limitations (salinity, pH, macro and micronutrients, lime, or specific ion toxicity) for growth of native and adapted reclamation species.

Pursuant to Paragraph (2) of Subsection F of 20.6.7.33 NMAC, the proposed soil cover system must be designed to limit net-percolation by having the capacity to store at least 95 percent of the long-term average winter (December, January and February) precipitation or at least 35% of the long-term average summer (June, July and August) precipitation, whichever is greater as determined by utilizing field or laboratory test results or published estimates of available water capacity. Long-term average annual summer precipitation totals for the Hillsboro area are 5.43 inches (35% equals 1.90 inches), and winter precipitation totals 1.96 inches (95% equals 1.86 inches) (WRCC, 2016). Therefore in order to comply with the Copper Rule, the proposed 3-foot thick soil cover system must be able to store at least 1.90 inches of water.

The available water capacity (AWC) for the salvageable growth media within the limits of the TSF and WRSP-2 and -3 were estimated as part of the Supplemental Soils Investigation at Copper Flat (see Subsection 3.3.1 and Table 3, Golder, 2013). The AWC estimates were based on general relationships



between water retention and soil texture (Brady and Weil 2002, USDA NRCS 2005). These estimates show an average AWC of approximately 0.9 inches of water per 1 foot of soil for the salvageable growth media within the footprint of WRSP-2 and -3, with a range of between 0.6 and 1.3 inches of water per 1 foot of soil. The AWC estimates for the salvageable growth media within the footprint of the TSF show an average AWC of approximately 1.2 inches of water per 1 foot of soil, with a range of between 0.4 and 2.2 inches of water per 1 foot of soil.

The actual water retention of the salvaged soils will vary based on the types of soil materials that are placed in the growth media stockpiles. Cover materials with varying physical properties (e.g. texture and rock fragment content) will be blended during soil salvage. Given the range of materials identified as suitable cover, the proposed cover system at Copper Flat is expected to meet the storage requirements of the Copper Rule.

Additional sources of alternative sources and types of materials for use as reclamation cover may be identified as part of a growth media management plan to be developed as part of the CQA/CQC plan that will be submitted to the agencies within 180 days of submission of a notice of intent to implement the reclamation plan and in consideration of performance objectives for the soil cover system.

5.5.2 Salvage

Where salvageable soil and alluvial materials exists, either on undisturbed or reclaimed areas, NMCC will salvage as much material as can be safely and practicably recovered. Additional suitable soils and other suitable cover materials including unconsolidated subgrade materials, colluvium and overburden will be salvaged to meet the volumetric requirements for final cover construction at closure (Section 3.1). Suitable soil materials available for reclamation from the previously mined and disturbed areas are very limited. Efforts will be made to carefully recover and stockpile these materials during the pre-production phase of the Project.

As part of the proposed operations, NMCC will salvage suitable soils and near-surface alluvial materials from within the TSF, WRSP-2 and WRSP-3 footprints. These materials are part of the Santa Fe formation gravels and alluvial basin fill. NMCC will bulk salvage suitable topdressing materials. The deep coarse-textured alluvial materials will be mixed with the more fine-textured surficial soils as part of the bulk salvaging, loading and hauling operations and will create a suitable growth media for final reclamation (Golder, 2013).

Large diameter trees and shrubs will be grubbed prior to soil salvage. To the extent that it doesn't interfere with soil salvage operations, small diameter woody plants and herbaceous vegetation will be salvaged and incorporated in with the stockpiled growth media to maintain organic matter content of the cover materials. However, vegetation residues from creosote-dominated vegetation will not be salvaged and incorporated



in the growth media stockpiles due to creosote’s allelopathic properties that may prevent seedling germination during revegetation.

The estimated volumes of salvageable cover material available are shown in **Table E11**. These estimates are based on the test pit data collected during the Supplemental Soils Investigation by Golder (2013). The area with existing tailing deposits (about 60 acres) will not get salvaged and that approximately 87% of the materials with in the remaining TSF footprint area contains suitable growth media (Golder 2013). Assuming a 20-foot excavation, there is approximately 13,755,280 cy of suitable cover materials within the proposed TSF footprint.

The WRSP-2 and WRSP-3 footprint occurs on the back- and footslopes of Animas Peak and the soils and unconsolidated materials are shallower than those found within the TSF footprint. Test pit excavations in these locations encountered andesite bedrock and were restricted to 10 feet. Additionally, soils on the steep slopes of Animas Peak are expected to be thin and salvage may not be practical. Thus, cover estimates assume that only 50% of the proposed WRSP-2 and WRSP-3 disturbance footprints will be salvageable. Assuming a 10 foot excavation depth, there is approximately 1,385,853 cy of suitable cover materials available within the footprints of WRSP-2 and WRSP-3.

Surficial soil materials will salvaged in association with the construction of the plant, pipeline corridor, access roads and ancillary facilities. The salvaged growth media in these locations will be windrowed for local redistribution during final reclamation of the site.

Total available topdressing materials for the project are estimated at over 15 million cy (**Table E11**). These estimates surpass the required 4.2 million cy of cover required for the Copper Flat Reclamation and Closure Plan (**Table E6**). To obtain the necessary cover volume, a single 134-acre excavation to 20 feet will salvage sufficient materials. Additional sources of alternative sources of growth media for reclamation may be identified prior to construction activities and in consideration of performance objectives for the soil cover system.

Table E11: Estimated Available Topdressing Materials

Facility	Growth Media Salvage Area (acres)	Estimated Available Topdressing Materials (cy)
Tailings Storage Facility Area ¹	490	13,755,280
WRSP-2 and WRSP-3 Areas ²	86	1,385,853
Total	576	15,141,133

Note:

¹ - Surface area excludes 60 acres of existing tailing deposits and assumes that 87% of the salvage area has suitable material.

² - Surface area excludes steep slopes and existing waste rock pile, it is assumed that 50% of the WRSP footprint areas will be salvageable.



Oversight and coordination will be required to handle suitable growth media during salvage operations. Specific borrow areas will be developed within the TSF and WRSP footprints for growth media and engineering materials. Depending on the construction sequence, both engineering and growth media materials may come from a single borrow area. The procedures to segregate topdressing from engineering materials will be further defined in a materials handling plan once final engineering specifications are determined.

5.5.3 Stockpiling

Salvaged growth media will be stored in the three GMSPs shown in **Figure E4**. The design capacity of GMSPs is estimated at 4.5 million cy (Section 2.7). Approximately 4.2 million cy of growth media are required for reclamation (**Table E6**). This includes nearly 204,000 cy of topdressing required to reclaim the EWRSPs during the pre-production phase (Section 2.1) and 80,000 cy of topdressing to be stored in windrows around the perimeter of surface disturbances for the plant area and ancillary facilities (Section 3.1). For final reclamation of the TSF and WRSPs, it is estimated that the GMSPs need to contain a total of 3.92 million cy. Thus, the GMSPs at their design capacity will contain an additional 584,000 cy of growth media to offset any losses associated with the storage and redistribution of growth media.

GMSPs are located so as not to be disturbed or impacted by mining operations. The surfaces of the stockpile will be shaped after construction with overall slopes of 3H:1V or shallower to minimize soil loss. To further minimize erosion and the establishment of undesirable weeds, growth media stockpiles will be seeded with the interim seed mix listed in **Table E7**. Interim seeding will be conducted prior to growing season. Diversion ditches will be constructed upgradient of the stockpiles, where necessary, to prevent run-on erosion. Additionally, berms will be constructed around the crest of stockpiles, as needed, to prevent outslope erosion from overland flow. BMPs such as silt fences or staked straw bales will be used as necessary to capture sediment and reduce soil loss.

5.5.4 Re-Distribution

Details regarding growth media's redistribution and application on regraded areas ready for reclamation are discussed in the Reclamation Plan (Section 3).

5.5.5 Stabilization

Cover materials will be stabilized after redistribution with seedbed preparation including scarification and disking along the contour and by seeding and mulching operations as described in the Reclamation Plan (Section 3).

5.5.6 Soil Fertility

Native soils in the project area, like most semi-arid soils in the region, have inherently low fertility in the upper horizons. In particular, site soils have relatively thin and poorly developed A horizons with low nitrogen and phosphorus levels and low to moderate organic matter content (Section 6, BDR; Golder,



2013). Further, most semi-arid native plants have adapted to low to moderate soil fertility conditions and are relatively unresponsive to increased soil fertility compared to crop plants (Chapin 1980). Fertilizer additions have been shown to also have negative impacts in reclamation including increases in weedy annuals, shifts in species composition and decreases in drought, disease and pest resistance. Based on the performance of interim seeding and concurrent reclamation efforts, NMCC will evaluate potential soil amendment requirements for redistributed topdressing materials through soil testing relative to native plant nutrient requirements.

5.6 Erosion Control

Reclamation activities described in Section 2.0 will stabilize disturbed areas to a condition that protects against erosion. All disturbed areas will be regraded and shaped to a final contour that achieves positive drainage, reconstructs slopes with lengths and gradients that will provide long-term stability and seeded and mulched to establish a vegetative cover. Stormwater will be diverted away from facilities. Drainage channels will be designed to regulate the velocity of water and minimize the potential for channel erosion. BMPs for stormwater diversions, drainage and other water conveyance channels may include lining the channel with rock, riprap, vegetation or other geotechnical materials.

NMCC will routinely inspect and maintain all reclaimed areas, drainage channels, diversion structures, retention impoundments and auxiliary erosion control features in accordance with professionally recognized standards such as Natural Resources Conservation Service. Post-construction/reclamation inspection schedules will be developed to include provisions for periodic (annual or semi-annual) and extreme event monitoring as appropriate for individual facilities.

5.7 Revegetation Success

The MMD rules require reclamation to a self-sustaining ecosystem (SSE) appropriate for the life zone of the surrounding area following closure. Revegetation success will be determined in conformance with Section 19.10.603.G of the Mining Act rules.

To summarize the revegetation success standards specified in Section 603.G, to obtain the release of financial assurance, revegetated lands under the grazing and wildlife habitat PMLU will meet the following:

- Total herbaceous cover and productivity shall be equal to 90 percent of the reference area within a 90 percent statistical confidence.
- The diversity of plant life forms (woody plants, grasses and forbs) shall determine what is reasonable given the physical environment of the reclamation.
- Woody plant species shall be established to an approved density with an 80 percent statistical confidence, or
- Other reasonable, attainable standards approved by the Director.



To establish revegetation success standards, NMCC will work with the MMD and BLM to develop the appropriate, reasonable, and attainable success standards for the site. They may include use of a reference area to develop technical standards from the analysis of vegetation data collected in native plant communities, the performance of areas reclaimed in the pre-production phase, interpretation of the ecological site potential and the anticipated differences in community structure among the reference area and reclaimed lands, or other reasonable means. The revegetation success standards agreed to will not be applied to the open pit area.

At the time of monitoring for financial assurance release, vegetation in reclaimed areas will likely represent an early-seral stage, grass-shrub community whereas the reference area will be representative of a mature plant community. The selection of the reference area and establishment of any technical achievable revegetation success standards for reclaimed areas will be developed in coordination with the MMD and/or the BLM.

Vegetation sampling techniques and statistical protocols for data analysis and hypothesis testing will also be developed in consultation with the agencies. The limitations associated with sampling and statistical analyses of vegetation data from heterogeneous semi-arid plant communities (i.e., minor components are often not represented in monitoring data) will be considered in selecting these appropriate sampling and analysis protocols. Standard plant ecology field methods and agency technical guidance will be used to create a robust and defensible vegetation monitoring program.

Revegetation success will be determined by monitoring the vegetation parameters in the final two years of the financial assurance period (years 11 and 12 following final reclamation). Data collection will be performed using the same methods and techniques on both reference and reclaimed areas. Vegetation success monitoring will be conducted once per year in the early fall after the tenth growing season. Two years of achieving the revegetation criteria will be considered a demonstration of success of the revegetation program.

5.8 Perpetual Care

The Reclamation and Closure Plan for the Copper Flat facility is designed to meet, without perpetual care, all applicable environmental requirements for post-mining, reclaimed sites and support a SSE. Details regarding perpetual care are provided in Section 4.8 of the Updated Mine Operations and Reclamation Plan.



6.0 REFERENCES

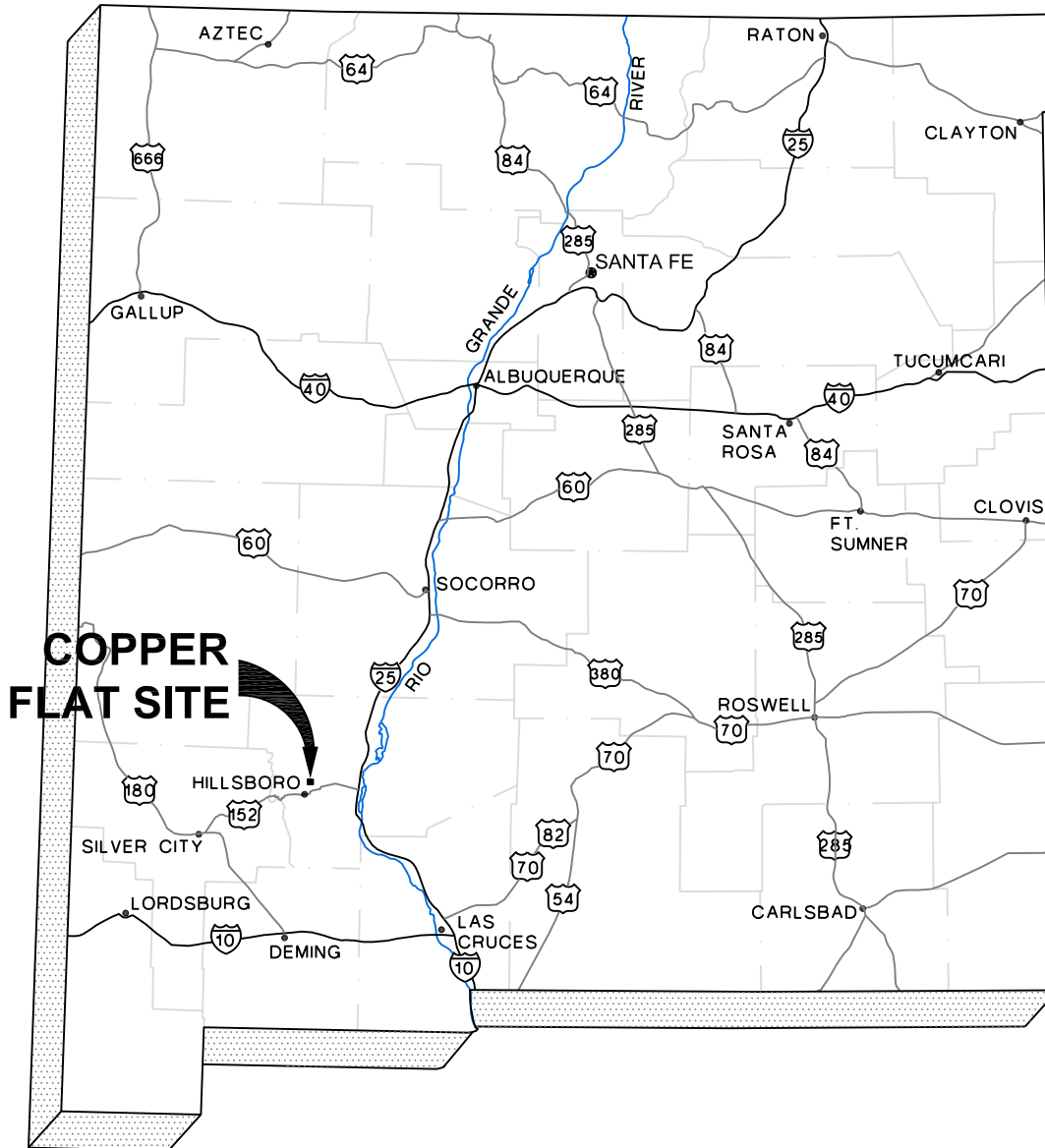
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FIGURES



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PROJECT

**COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN**

CONSULTANT



YYYY-MM-DD 2016-10-05

PREPARED CM

DESIGN TS

REVIEW TS

APPROVED TS

TITLE

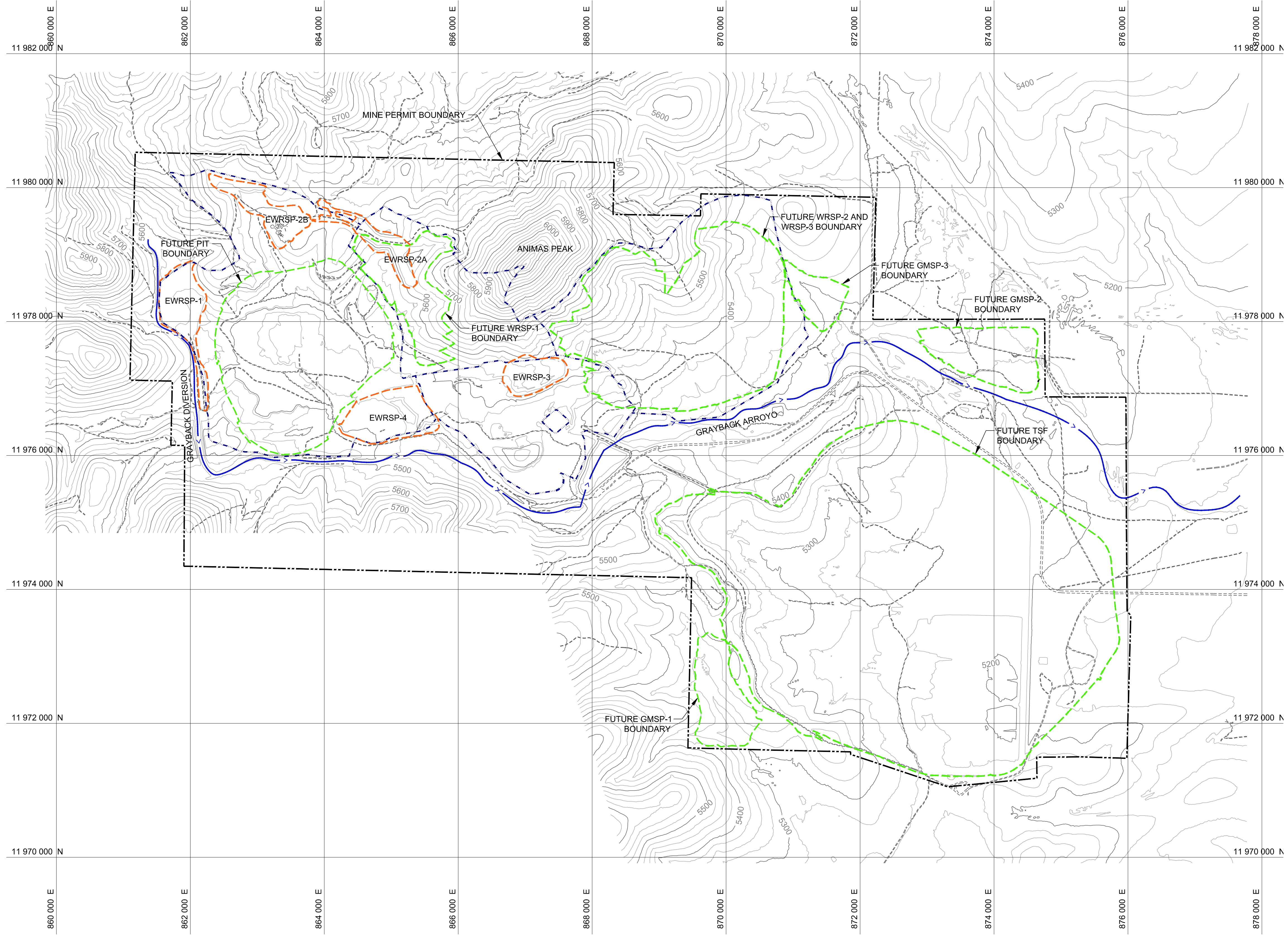
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PROJECT No.
1531453

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0300

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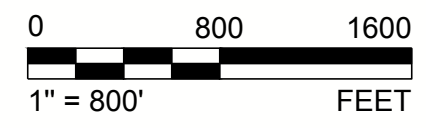
FIGURE
E1



- GENERAL LEGEND**
- 5400 — EXISTING GROUND CONTOURS (ft-MSL)
 - - - - MINE PERMIT BOUNDARY
 - - - - EXISTING FACILITY BOUNDARY (APPROXIMATE)
 - - - - PROPOSED FACILITY BOUNDARY
 - - - - WATERSHED BOUNDARY (BY OTHERS)
 - < — EXISTING DIVERSION OR ARROYO
 - - - - EXISTING ROAD
- WRSP WASTE ROCK STOCKPILE
 - EWRSP EXISTING WASTE ROCK STOCKPILE
 - TSF TAILING STORAGE FACILITY
 - GMSP GROWTH MEDIA STOCKPILE

- NOTES**
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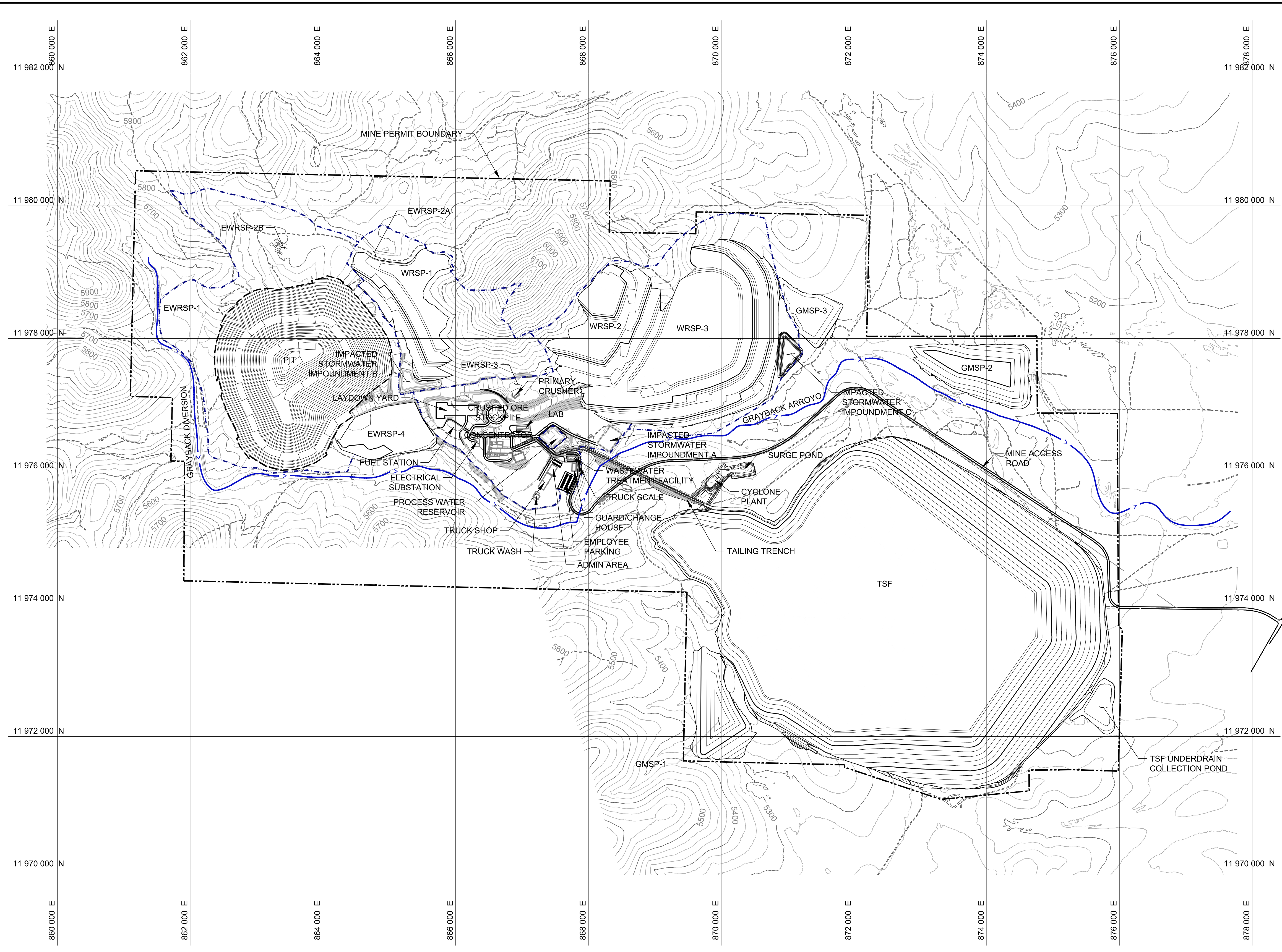
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
**GENERAL ARRANGEMENT
EXISTING SITE CONDITIONS**

PROJECT NO. 1531453	CONTROL 0500	REV. A	1 of 1	FIGURE E2
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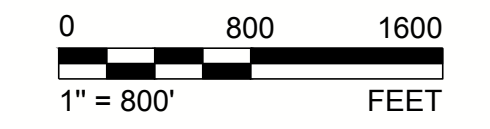


GENERAL LEGEND

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	5400	PROPOSED DESIGN CONTOURS (ft-MSL)
		MINE PERMIT BOUNDARY
		WATERSHED BOUNDARY (BY OTHERS)
		EXISTING DIVERSION OR ARROYO
		EXISTING ROAD
	WRSP	WASTE ROCK STOCKPILE
	EWRS	EXISTING WASTE ROCK STOCKPILE
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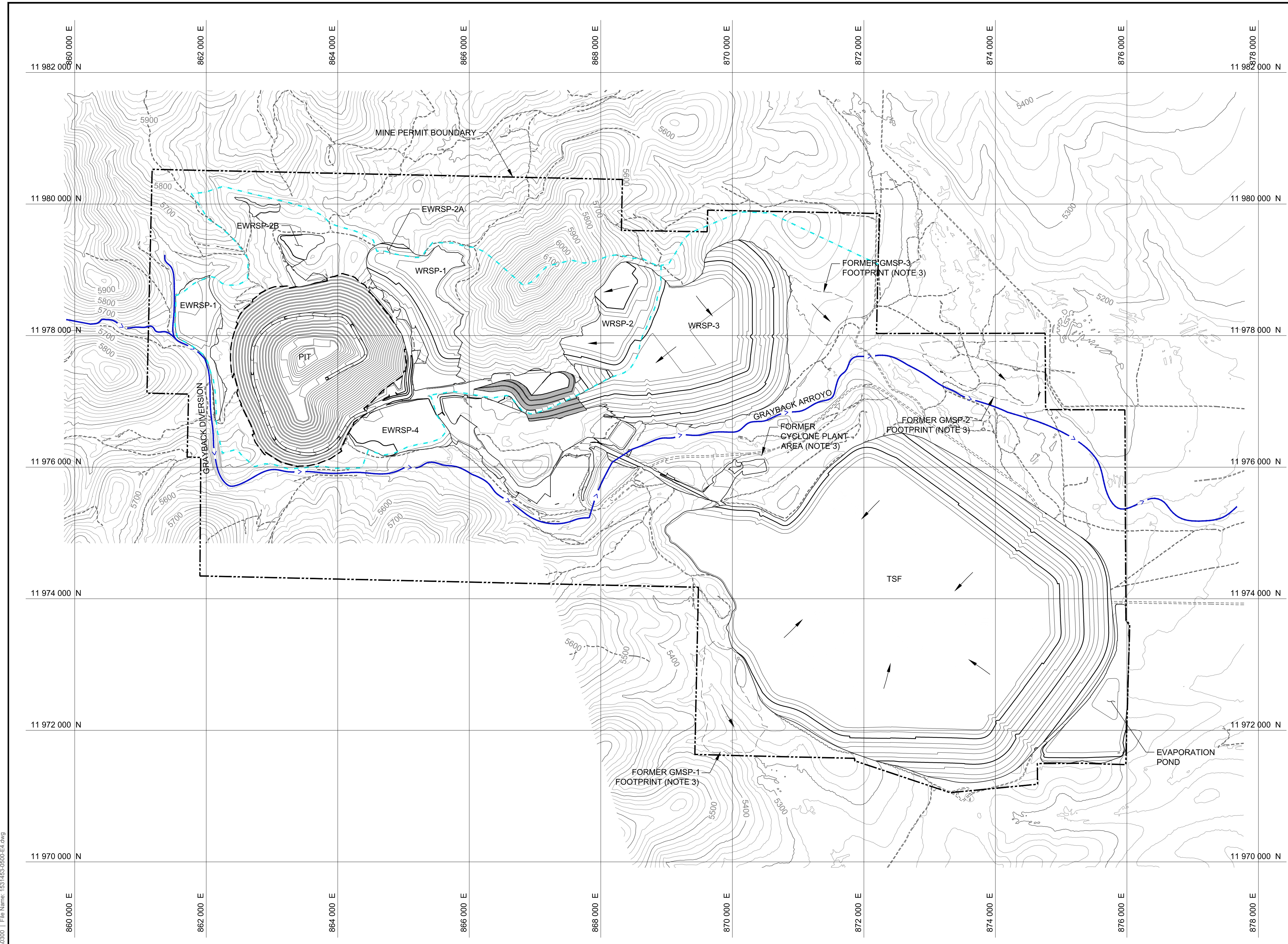
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TITLE
**GENERAL ARRANGEMENT
AT FINAL BUILDOUT**

PROJECT NO. 1531453 CONTROL 0500 REV. A 1 of 1 FIGURE E3

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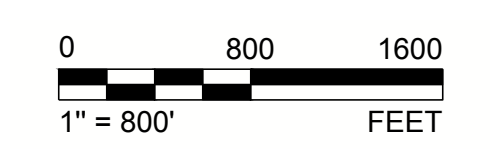


GENERAL LEGEND

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	5400	PROPOSED DESIGN CONTOURS (ft-MSL)
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		RECLAMATION WATERSHED BOUNDARY
		EXISTING DIVERSION OR ARROYO
		EXISTING ROAD
		GRADE INDICATOR
	WRSP	WASTE ROCK STOCKPILE
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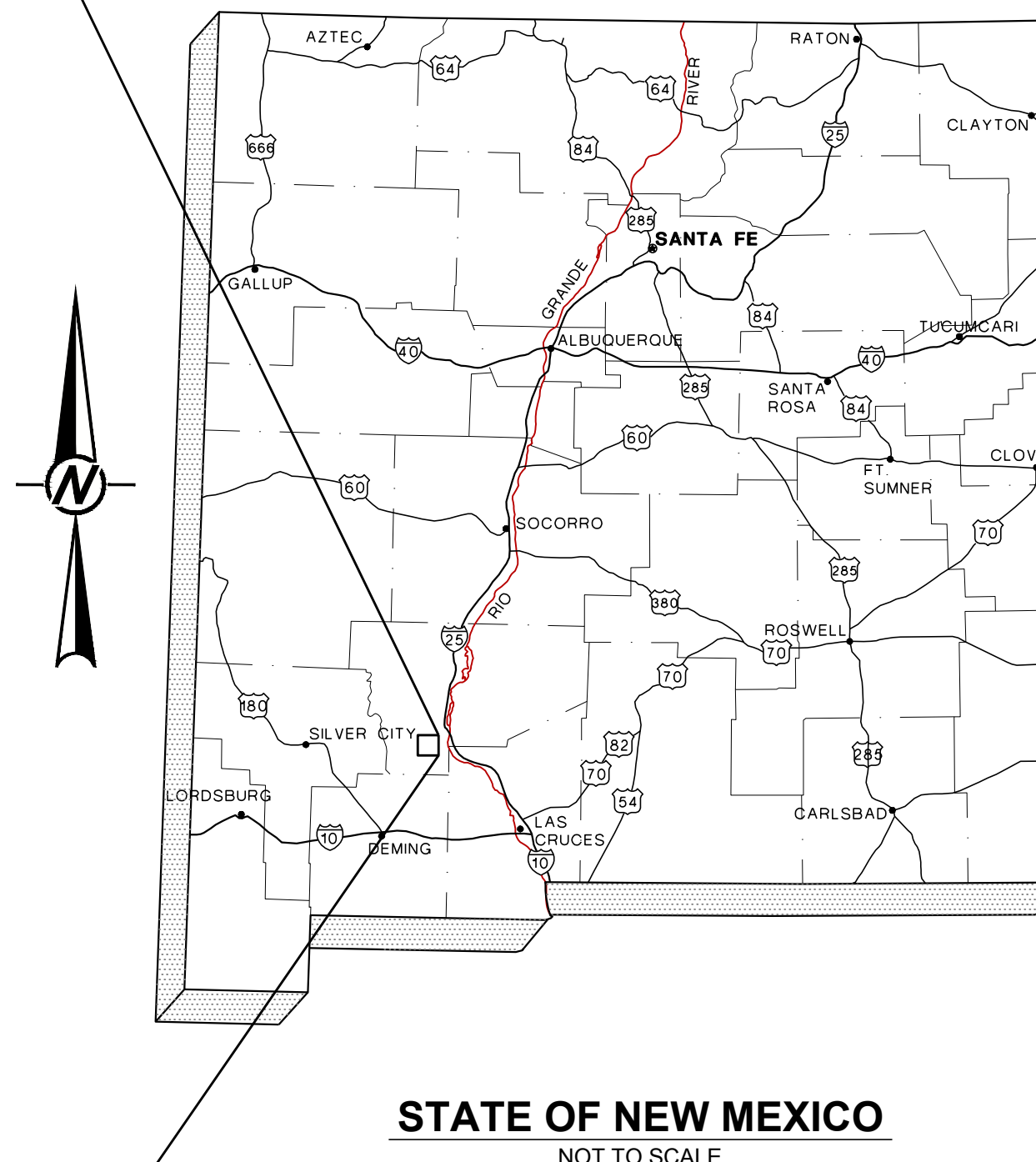
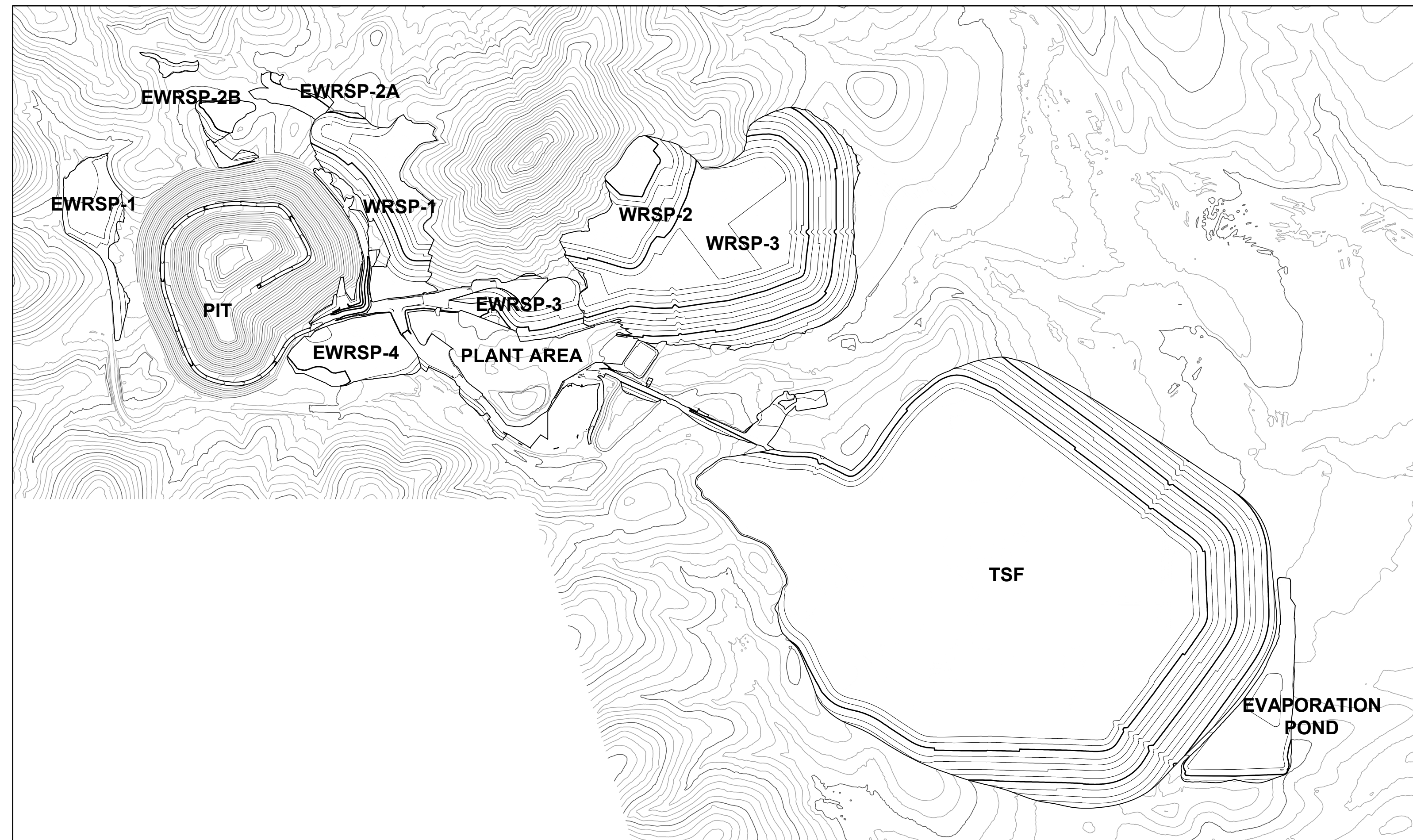
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**GENERAL ARRANGEMENT
 FINAL RECLAMATION TOPOGRAPHY**

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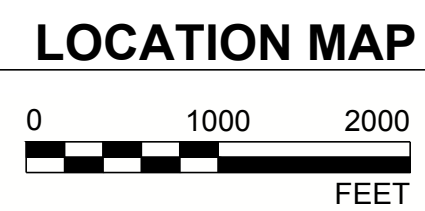
PROJECT NO.	CONTROL	REV.	1 of 1	FIGURE
1531453	0500	A		E4

ATTACHMENT E1
MINE RECLAMATION AND CLOSURE DESIGN PLAN DESIGNS

COPPER FLAT PROJECT MINE RECLAMATION AND CLOSURE PLAN SIERRA COUNTY, NEW MEXICO OCTOBER 2016



LIST OF DRAWINGS	
DRAWING NUMBER	DRAWING TITLE
G-001	TITLE SHEET
G-002	GENERAL ARRANGEMENT EXISTING SITE CONDITIONS
G-003	GENERAL ARRANGEMENT AT FINAL BUILDOUT
G-004	GENERAL ARRANGEMENT FINAL RECLAMATION TOPOGRAPHY
C-001	EWRSP-1 EXISTING FACILITY LAYOUT
C-002	EWRSP-1 REGRADE AND DRAINAGE PLAN
C-003	EWRSP-2B EXISTING FACILITY LAYOUT
C-004	EWRSP-2B REGRADE AND DRAINAGE PLAN
C-005	EWRSP-4 EXISTING FACILITY LAYOUT
C-006	EWRSP-4 REGRADE AND DRAINAGE PLAN
C-007	WRSP-1 AT FINAL BUILDOUT
C-008	WRSP-1 REGRADE AND DRAINAGE PLAN
C-009	WRSP-2 AND WRSP-3 AT FINAL BUILDOUT
C-010	WRSP-2 AND WRSP-3 REGRADE AND DRAINAGE PLAN
C-011	TSF AT FINAL BUILDOUT
C-012	TSF RECLAMATION AND DRAINAGE PLAN
C-013	PIT AT FINAL BUILDOUT
C-014	PIT RECLAMATION AND DRAINAGE PLAN
C-015	PLANT AREA AT FINAL BUILDOUT
C-016	PLANT AREA RECLAMATION AND DRAINAGE PLAN
C-017	EWRSP RECLAMATION SECTIONS
C-018	WRSP RECLAMATION SECTIONS
C-019	TSF, PIT AND PLANT RECLAMATION SECTIONS
C-020	TYPICAL SECTIONS AND DETAILS (1 OF 2)
C-021	TYPICAL SECTIONS AND DETAILS (2 OF 2)



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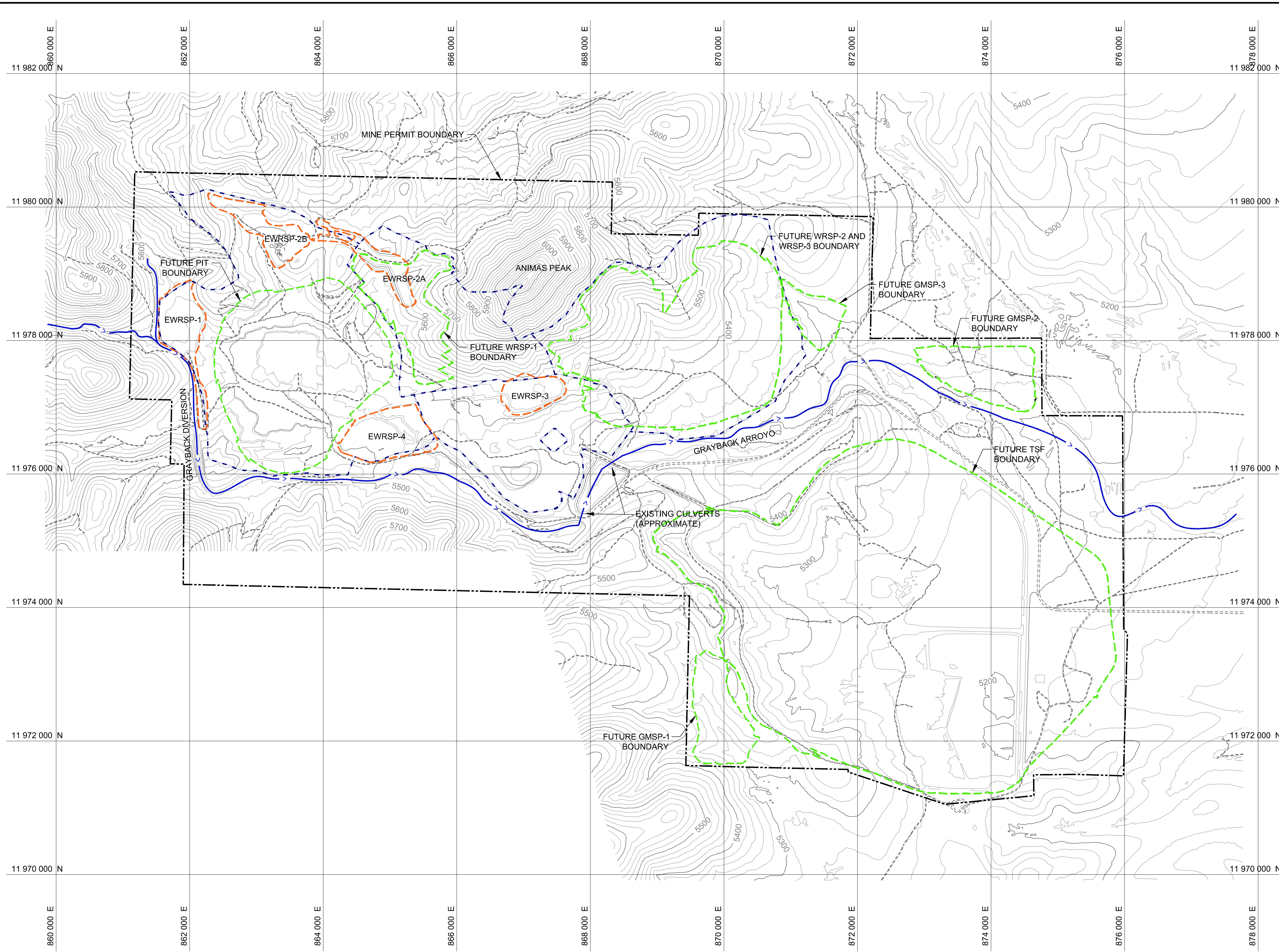
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TITLE
TITLE SHEET

PROJECT NO. 1531453 CONTROL 0300 REV. C 1 of 25 DRAWING **G-001**

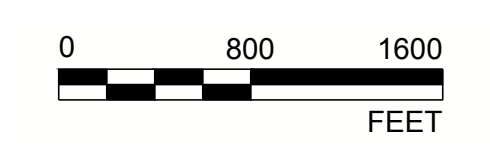
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GENERAL LEGEND

	EXISTING GROUND CONTOURS - 20 ft CONTOUR INTERVAL (ft-MSL)
	MINE PERMIT BOUNDARY
	EXISTING FACILITY BOUNDARY (APPROXIMATE)
	PROPOSED FACILITY BOUNDARY
	WATERSHED BOUNDARY (BY OTHERS)
	EXISTING DIVERSION OR ARROYO
	EXISTING ROAD
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TSF	TAILING STORAGE FACILITY
GMSP	GROWTH MEDIA STOCKPILE

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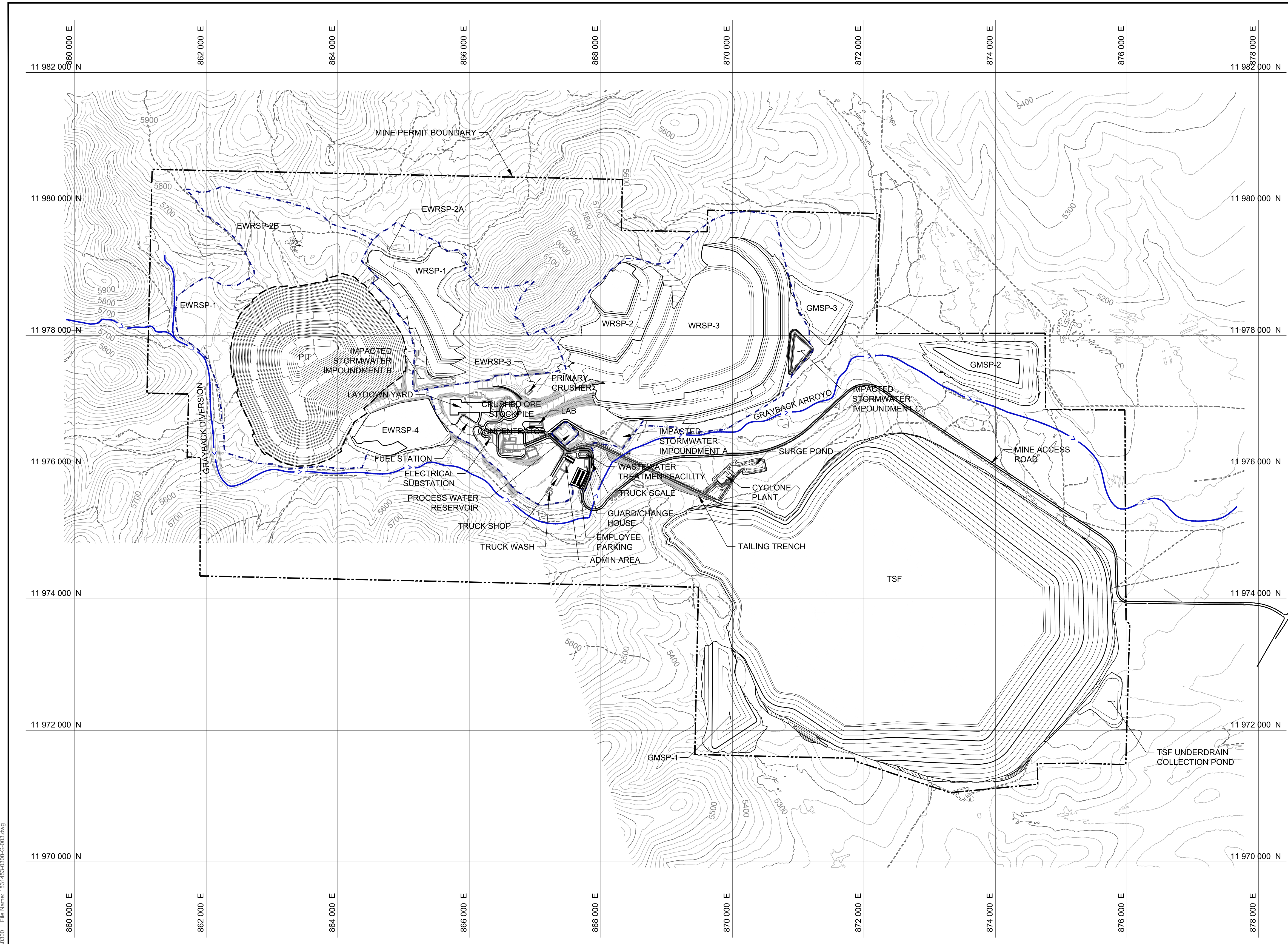
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EXISTING SITE CONDITIONS

PROJECT NO. 1531453 CONTROL 0300 REV. C 2 of 25 DRAWING G-002

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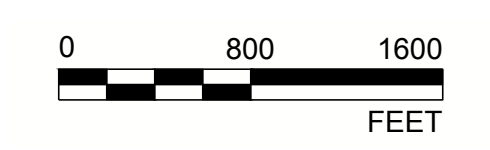
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GENERAL LEGEND

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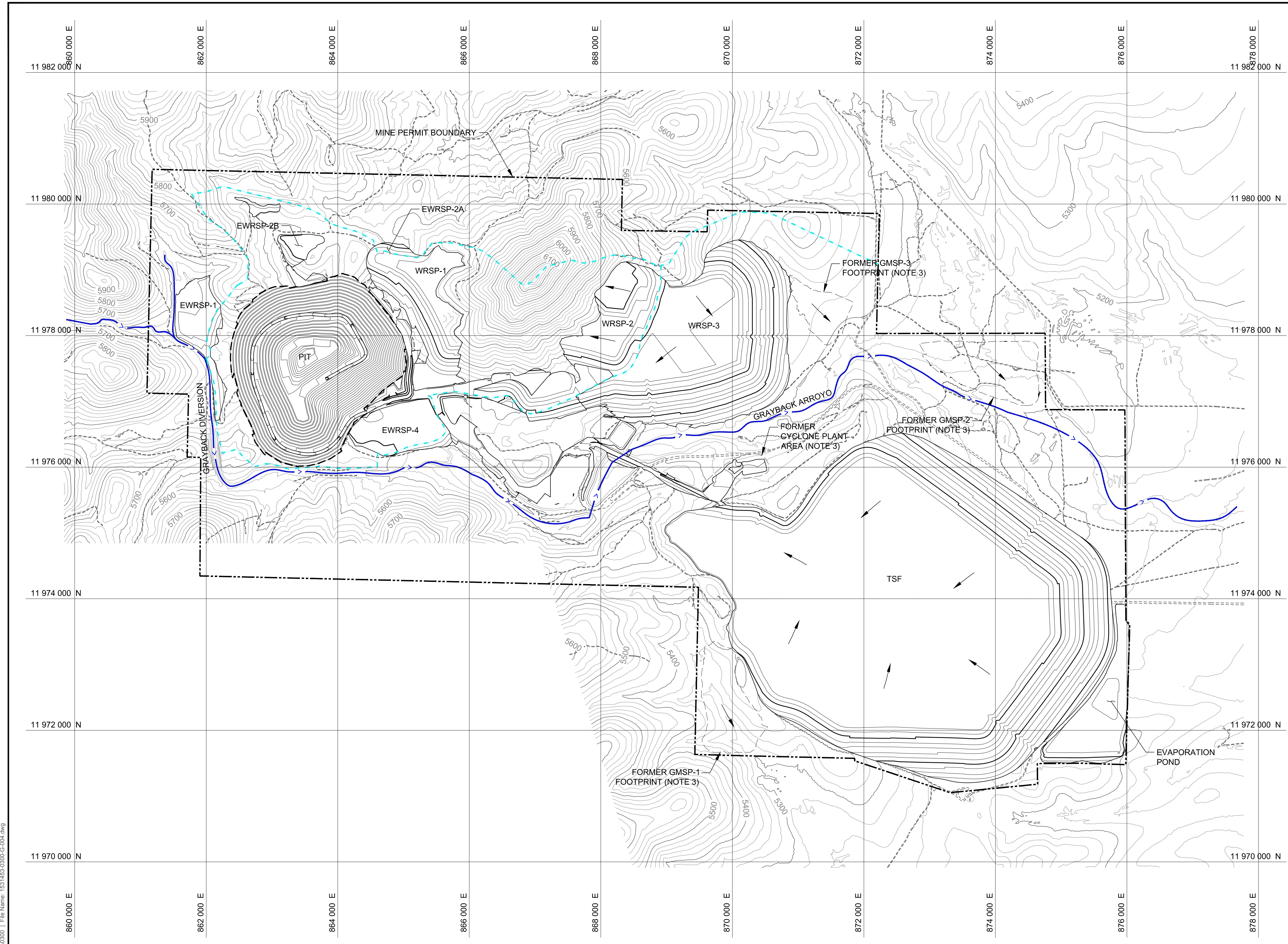
PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
GENERAL ARRANGEMENT
AT FINAL BUILDOUT

PROJECT NO. 1531453	CONTROL 0300	REV. C	3 of 25	DRAWING G-003
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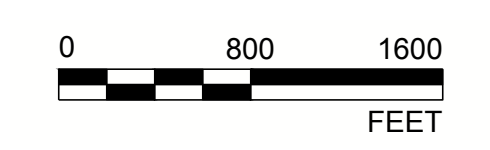
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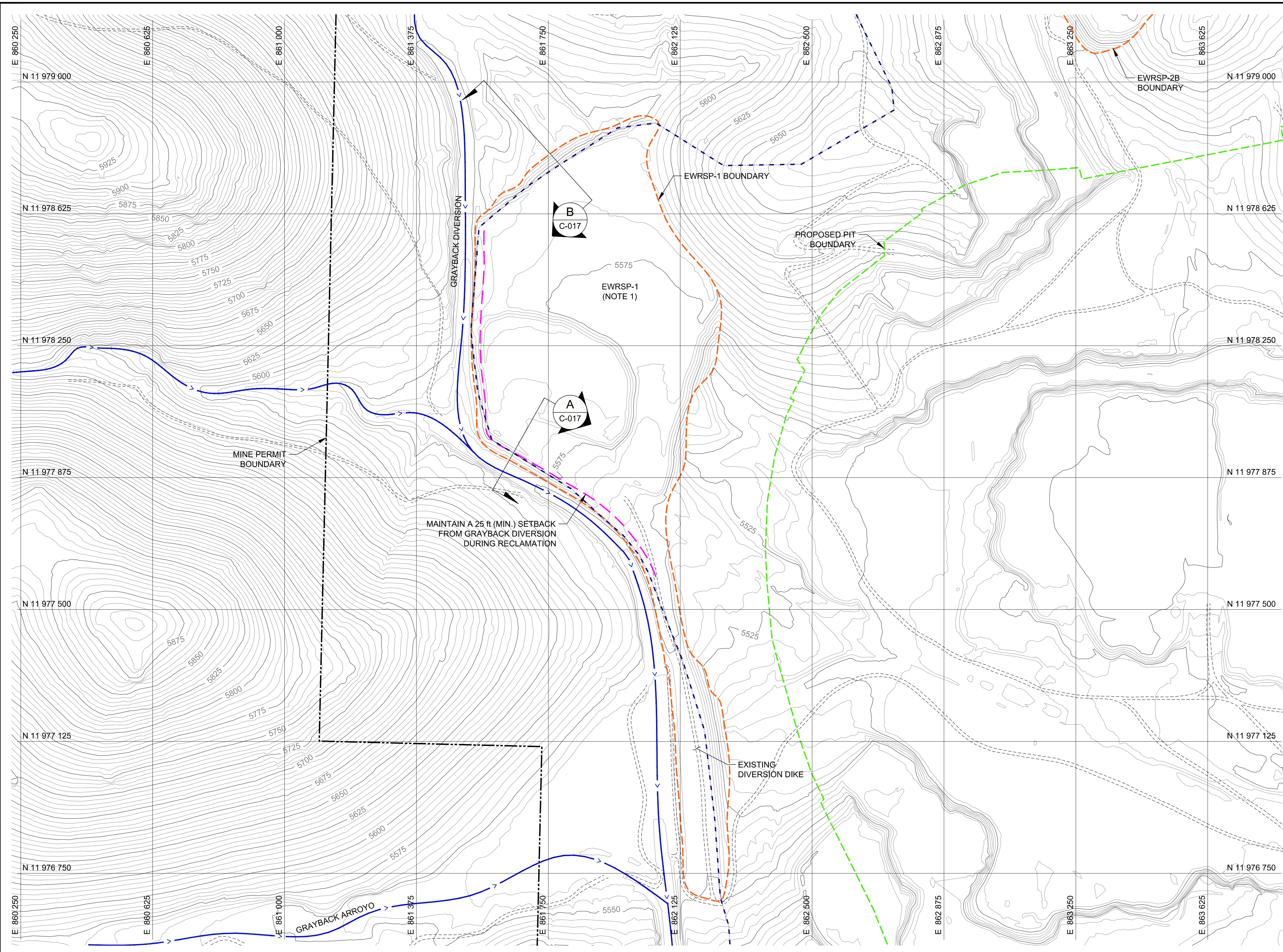
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TITLE
GENERAL ARRANGEMENT
FINAL RECLAMATION TOPOGRAPHY

PROJECT NO. 1531453	CONTROL 0300	REV. C	4 of 25	DRAWING G-004
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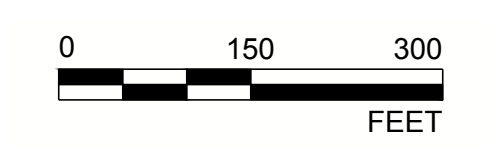
1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



LEGEND

	EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
	MINE PERMIT BOUNDARY
	EXISTING FACILITY BOUNDARY (APPROXIMATE)
	PROPOSED FACILITY BOUNDARY
	WATERSHED BOUNDARY (BY OTHERS)
	EXISTING DIVERSION OR ARROYO
	EXISTING ROAD
	PROPOSED SETBACK
	SECTION CALLOUT
	SECTION ID
	DRAWING LOCATION
	EXISTING WASTE ROCK STOCKPILE
	MINIMUM

NOTES
 1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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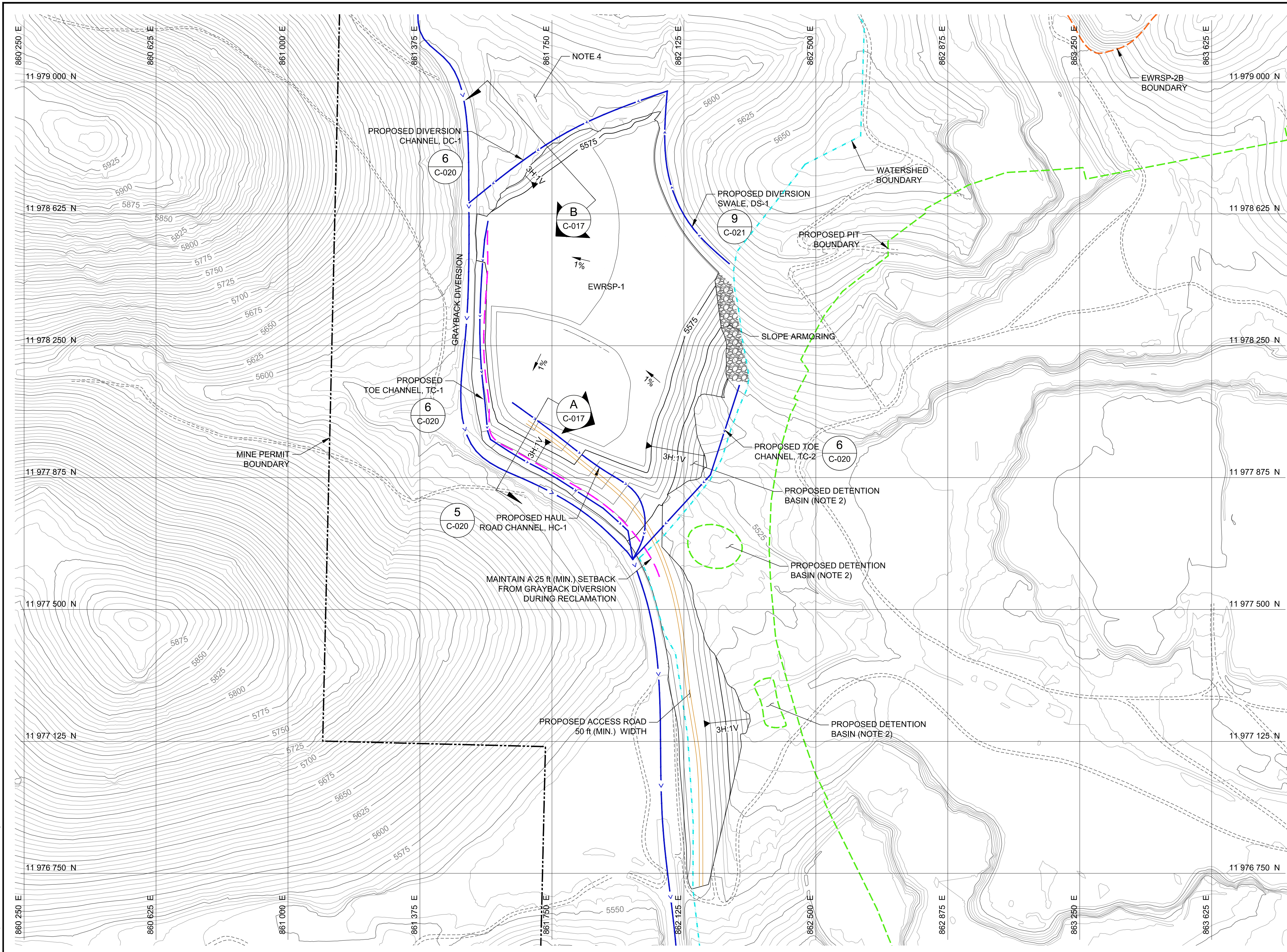
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PROJECT
 COPPER FLAT PROJECT
 MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-1 EXISTING FACILITY LAYOUT

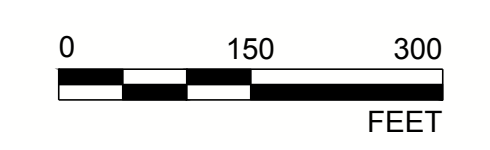
PROJECT NO. 1531453	CONTROL 0300	REV. C	5 of 25	DRAWING C-001
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LEGEND

- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 5)
- MINE PERMIT BOUNDARY
- PROPOSED FACILITY BOUNDARY
- EXISTING FACILITY BOUNDARY (APPROXIMATE)
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- EXISTING ROAD
- PROPOSED ROAD
- PROPOSED SETBACK
- 3H:1V SLOPE INDICATOR
- 1% GRADE INDICATOR
- 1 C-020 DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- A C-017 SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- EWRSP EXISTING WASTE ROCK STOCKPILE
- MIN. MINIMUM
- SLOPE ARMORING

- NOTES**
- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 - DETENTION BASINS SHALL BE CONSTRUCTED FOR EVAPORATION AND USE AS A WILDLIFE HABITAT.
 - RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMAC 20.6.7.33. C(3). A 1% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMAC 20.6.7.33. C(2). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 - PLACE FILL MATERIAL TO PROMOTE POSITIVE DRAINAGE TOWARD PROPOSED DIVERSION CHANNEL, DC-1, AND THE EXISTING GRAYBACK DIVERSION.
 - PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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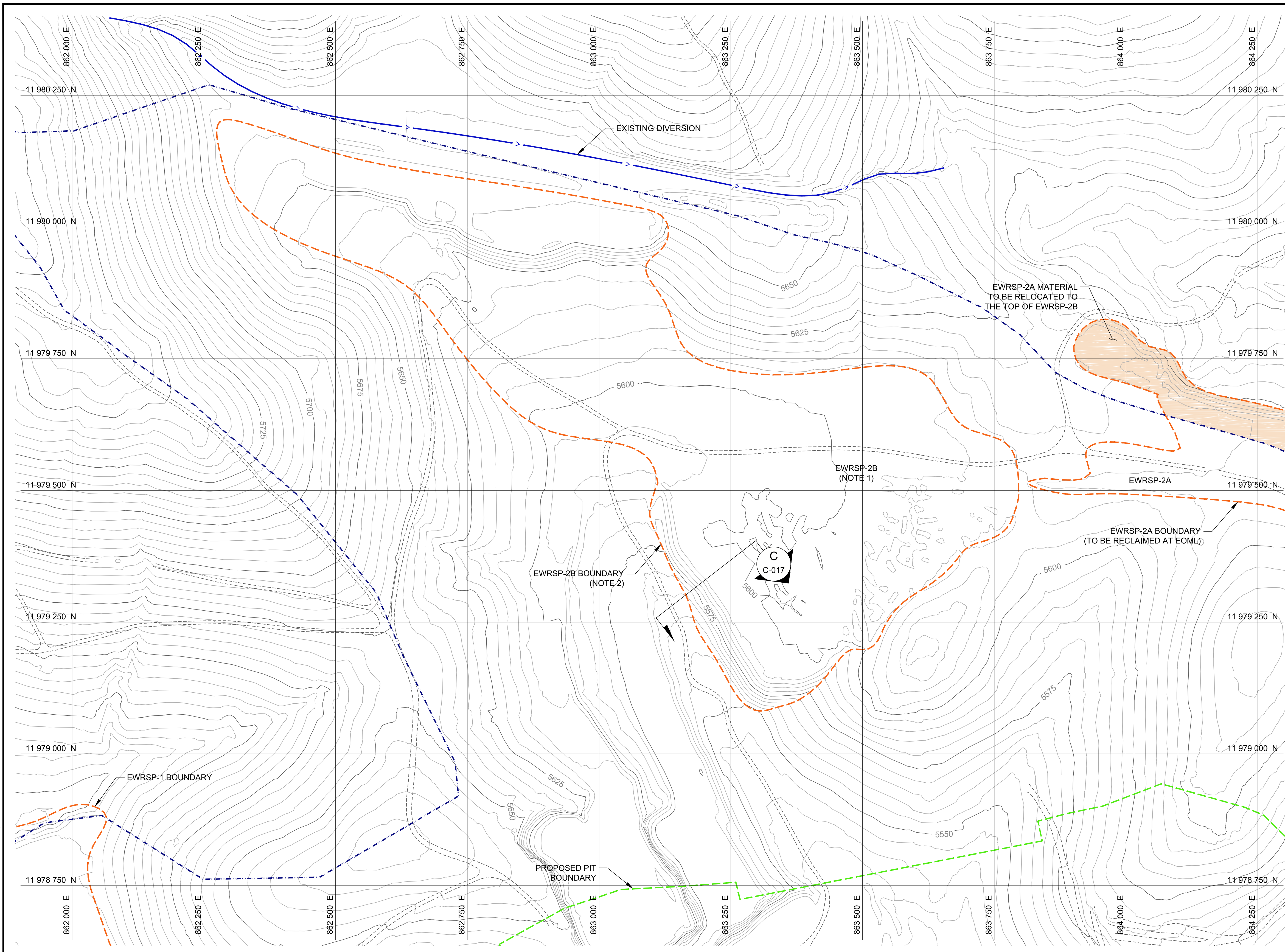
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-1 REGRADE AND DRAINAGE PLAN

PROJECT NO. 1531453 CONTROL 0300 REV. C 6 of 25 DRAWING C-002

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



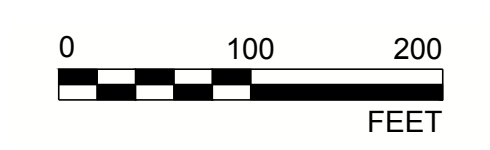
LEGEND

- 5400 — EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- - - - - EXISTING FACILITY BOUNDARY (APPROXIMATE)
- - - - - PROPOSED FACILITY BOUNDARY
- - - - - WATERSHED BOUNDARY (BY OTHERS)
- < — EXISTING DIVERSION OR ARROYO
- - - - - EXISTING ROAD

SECTION CALLOUT
SECTION ID
DRAWING LOCATION

EWRSP EXISTING WASTE ROCK STOCKPILE
EOML END OF MINE LIFE

- NOTES**
- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 - EXISTING EWRSP-2B FACILITY FOOTPRINT INCLUDES ASSOCIATED DISTURBED AREAS.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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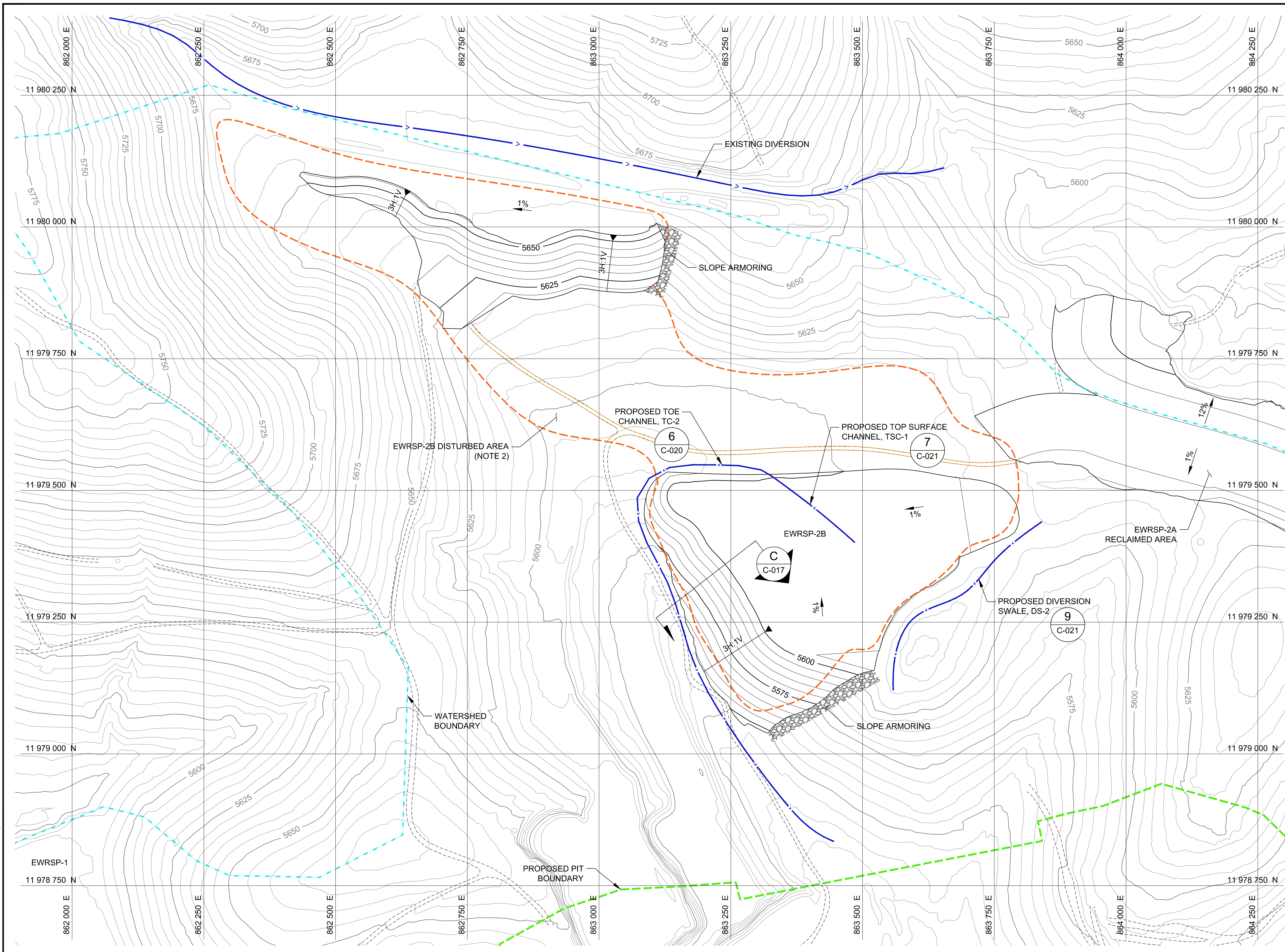
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-2B EXISTING FACILITY LAYOUT

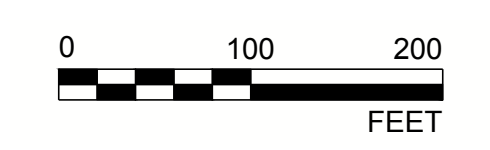
PROJECT NO. 1531453 CONTROL 0300 REV. C 7 of 25 DRAWING C-003

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A3S D



- LEGEND**
- 5400 — EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
 - 5400 — PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 4)
 - - - - - EXISTING FACILITY BOUNDARY (APPROXIMATE)
 - - - - - PROPOSED FACILITY BOUNDARY
 - - - - - RECLAMATION WATERSHED BOUNDARY
 - < — EXISTING DIVERSION OR ARROYO
 - < — PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
 - - - - - EXISTING ROAD
 - — — — — PROPOSED ROAD
 - ▲ 3H:1V — SLOPE INDICATOR
 - 1% — GRADE INDICATOR
 - ① 1 C-020 — DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
 - ⬠ A C-017 — SECTION CALLOUT
SECTION ID
DRAWING LOCATION
 - EWRS — EXISTING WASTE ROCK STOCKPILE
 - ▨ — SLOPE ARMORING

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. EWRSP-2B TOP SURFACE SHALL BE GRADED TO PROPOSED TOE CHANNEL, TC-2, FOR FLOW CONVEYANCE TOWARD PIT PERIMETER CHANNEL, PC-1. EWRSP-2B FACILITY FOOTPRINT INCLUDES ASSOCIATED DISTURBED AREAS TO BE GRADED, RIPPED AND VEGETATED.
 3. EWRSP-2B RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMCC 20.6.7.33. C(2). A 1% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMCC 20.6.7.33. C(2). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORWATER CONVEYANCE.
 4. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.



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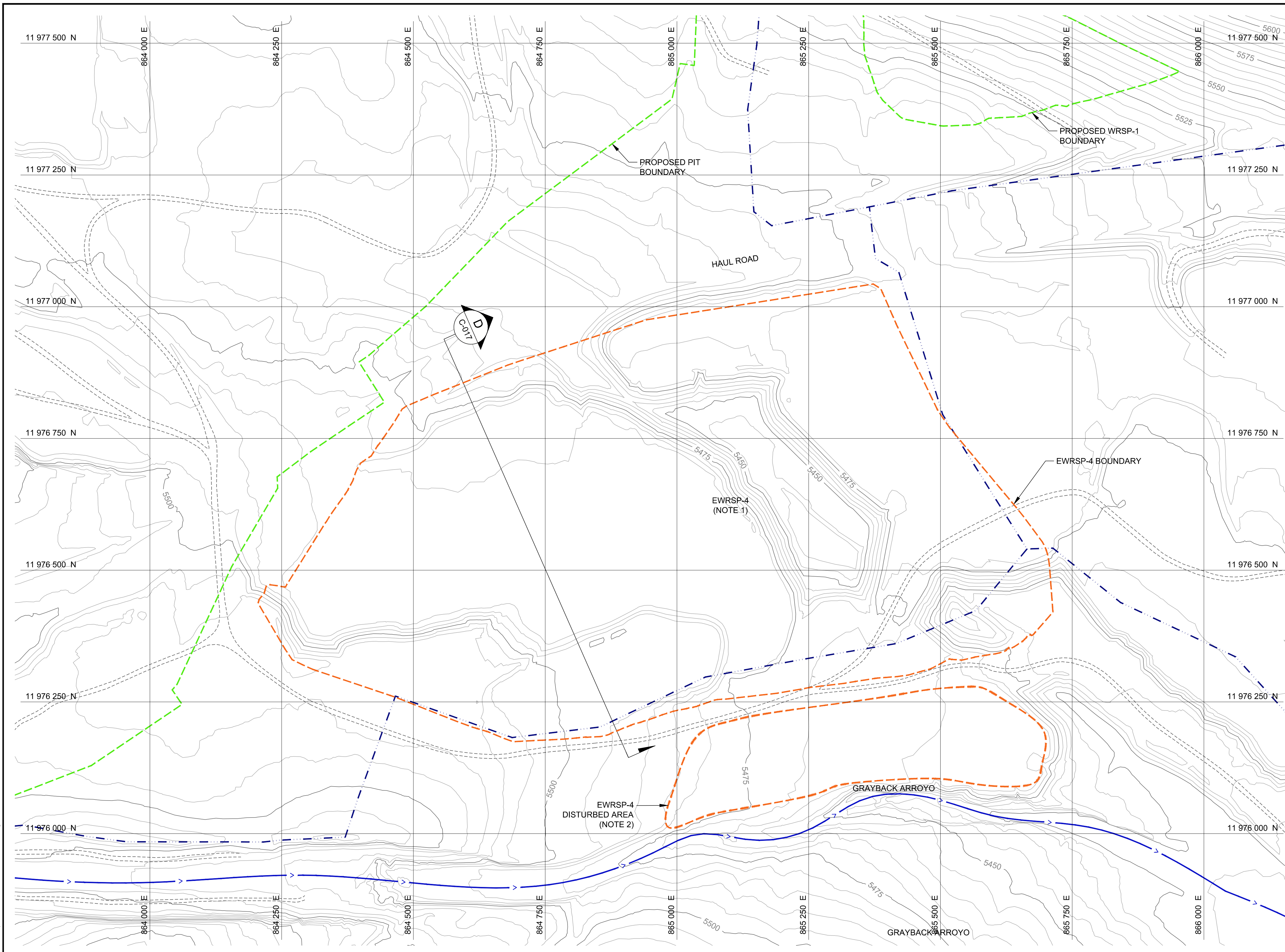
PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-2B REGRADE AND DRAINAGE PLAN

PROJECT NO. 1531453 CONTROL 0300 REV. C 8 of 25 DRAWING C-004

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI D



LEGEND

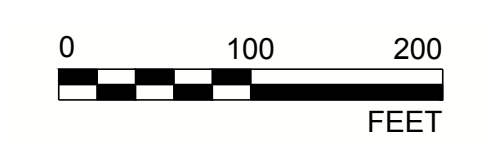
- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- EXISTING FACILITY BOUNDARY (APPROXIMATE)
- PROPOSED FACILITY BOUNDARY
- WATERSHED BOUNDARY (BY OTHERS)
- EXISTING DIVERSION OR ARROYO
- EXISTING ROAD

SECTION CALLOUT
SECTION ID: C-017
DRAWING LOCATION: [North Arrow]

- EWRSP EXISTING WASTE ROCK STOCKPILE
- WRSP WASTE ROCK STOCKPILE

NOTES

- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
- EXISTING EWRSP-4 FACILITY FOOTPRINT INCLUDES ASSOCIATED DISTURBED AREAS.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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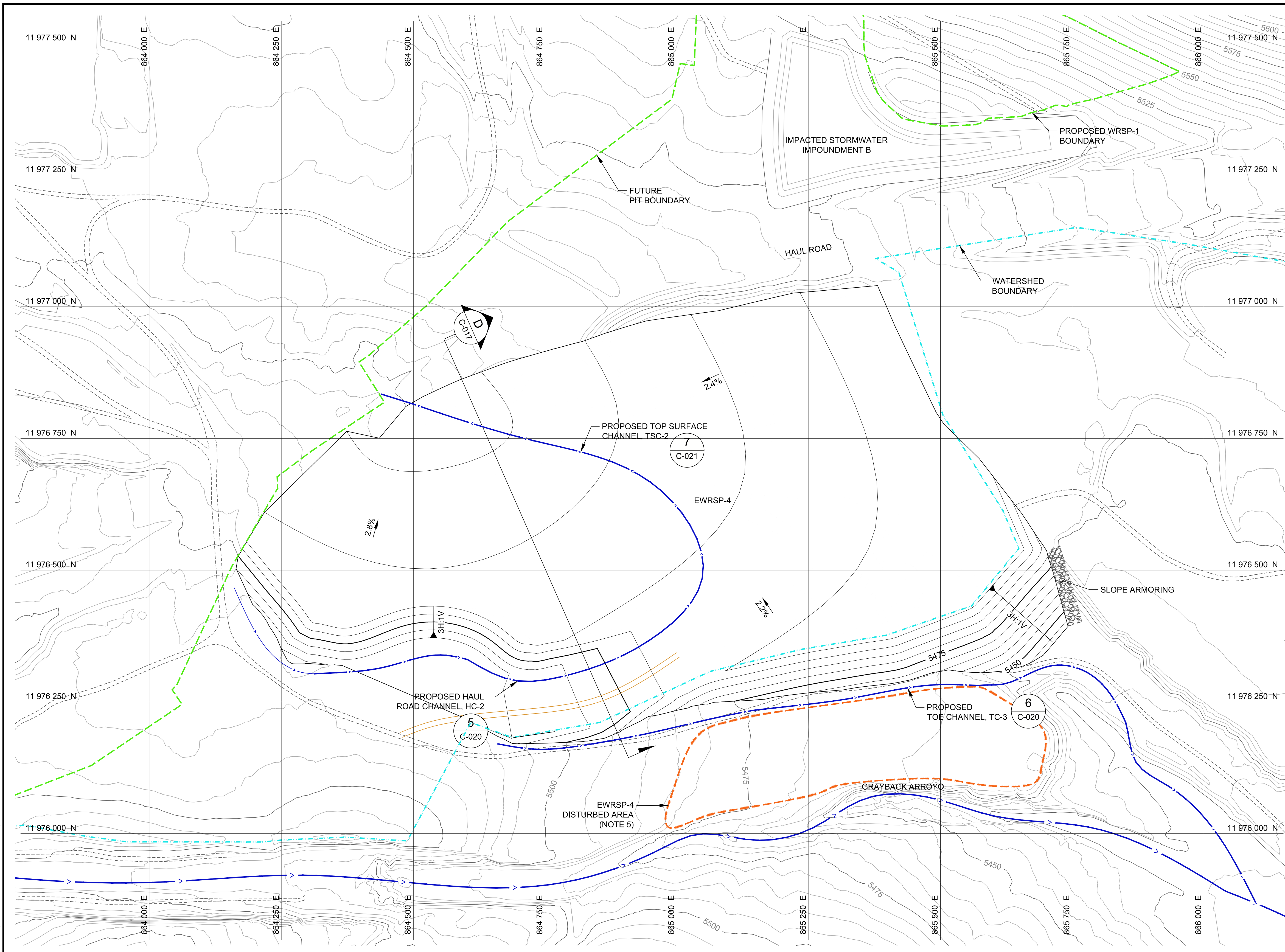
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-4 EXISTING FACILITY LAYOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 9 of 25 DRAWING C-005

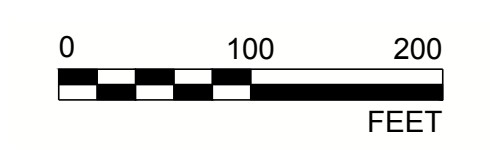
1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A3S D



LEGEND

- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 4)
- EXISTING FACILITY BOUNDARY (APPROXIMATE)
- PROPOSED FACILITY BOUNDARY
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- EXISTING ROAD
- PROPOSED ROAD
- 3H:1V SLOPE INDICATOR
- 1% GRADE INDICATOR
- 1
C-020 DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- A
C-017 SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP WASTE ROCK STOCKPILE
- EWRSP EXISTING WASTE ROCK STOCKPILE
- SLOPE ARMORING

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. EWRSP-4 TOP SURFACE SHALL BE GRADED TO A PROPOSED HAUL ROAD CHANNEL, HC-3, FOR FLOW CONVEYANCE TOWARD THE PIT.
 3. EWRSP-4 RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMCC 20.6.7.33. C(3). A 1% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMCC 20.6.7.33. C(2). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT, AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 4. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.
 5. EXISTING EWRSP-4 FACILITY FOOTPRINT INCLUDES ASSOCIATED DISTURBED AREAS.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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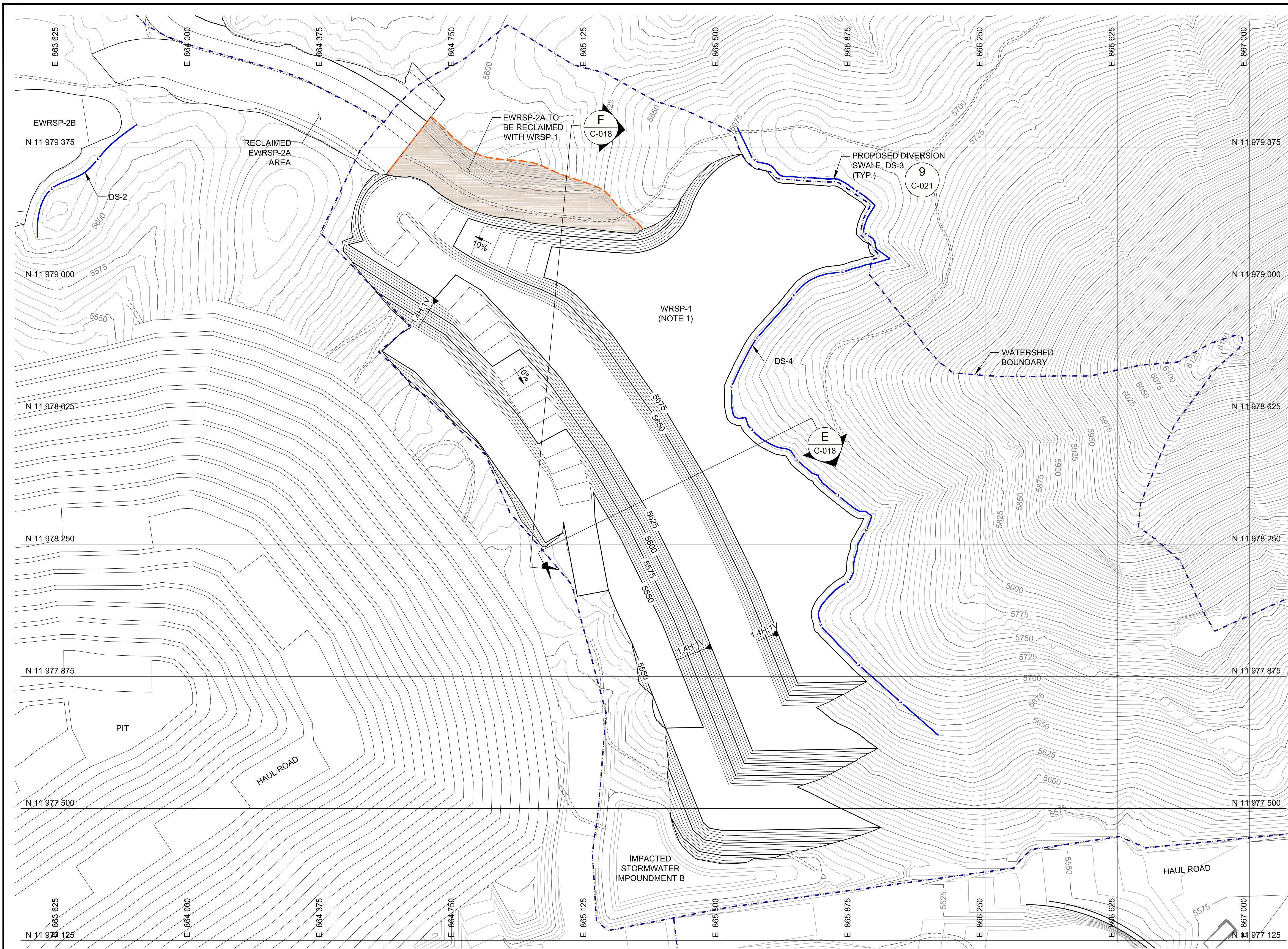
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PROJECT
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MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSP-4 REGRADE AND DRAINAGE PLAN

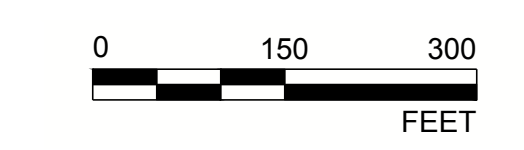
PROJECT NO. 1531453	CONTROL 0300	REV. C	10 of 25	DRAWING C-006
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1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM AANSI D



- LEGEND**
- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
 - 5400 DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 1)
 - EXISTING FACILITY BOUNDARY (APPROXIMATE)
 - WATERSHED BOUNDARY (BY OTHERS)
 - EXISTING DIVERSION OR ARROYO
 - EXISTING ROAD
 - 3H:1V SLOPE INDICATOR
 - 1% GRADE INDICATOR
 - SECTION CALLOUT
SECTION ID
DRAWING LOCATION
 - WRSP WASTE ROCK STOCKPILE
 - EWRSP EXISTING WASTE ROCK STOCKPILE
 - TYP. TYPICAL

NOTES
 1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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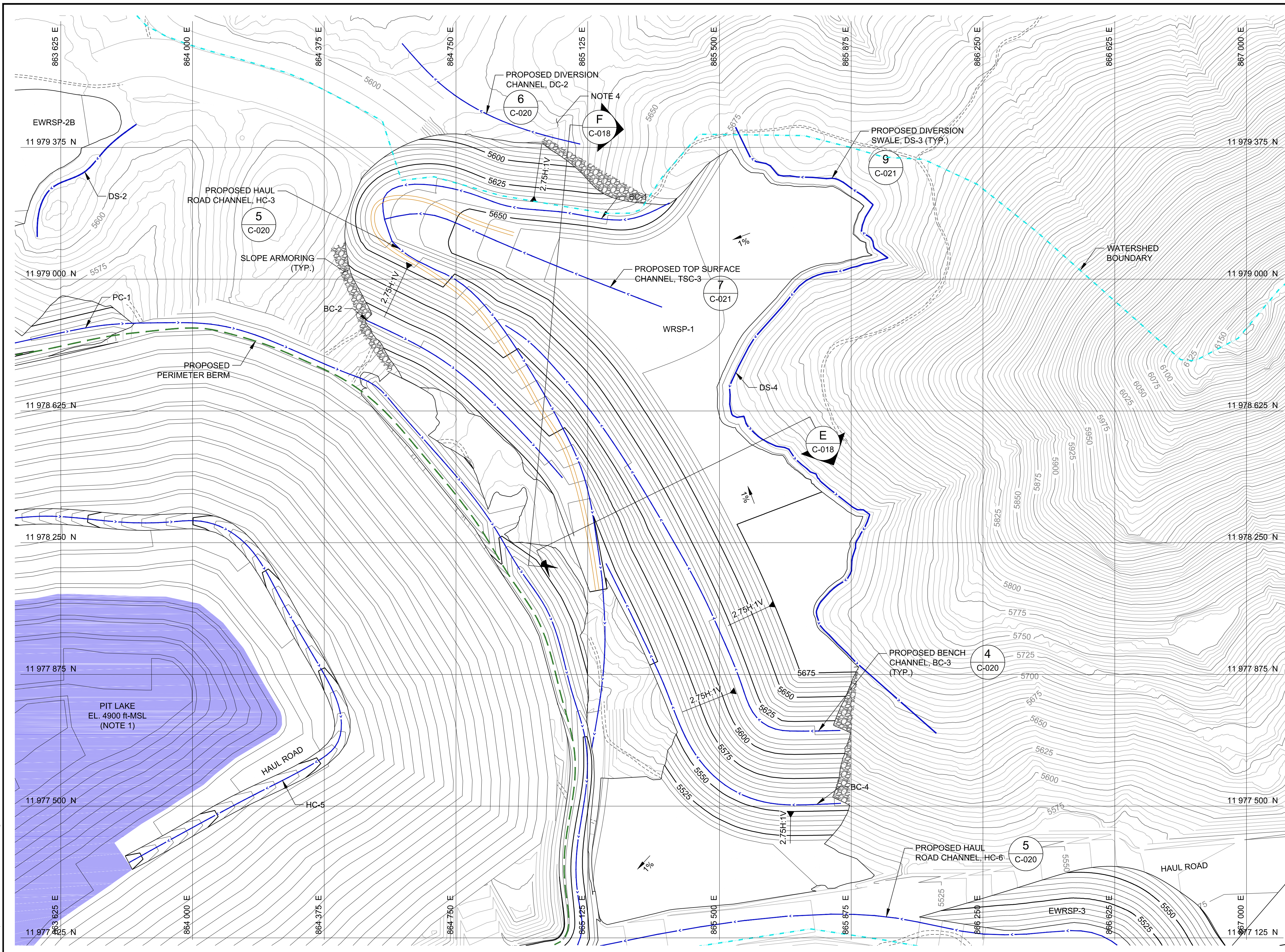
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PROJECT
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 MINE RECLAMATION AND CLOSURE PLAN

TITLE
WRSP-1 AT FINAL BUILDOUT

PROJECT NO. 1531453	CONTROL 0300	REV. C	11 of 25	DRAWING C-007
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1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



LEGEND

- 5400 — EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 — PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 5)
- - - - RECLAMATION WATERSHED BOUNDARY
- < — EXISTING DIVERSION OR ARROYO
- < — PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- - - - EXISTING ROAD
- — PROPOSED ROAD
- — PROPOSED PERIMETER BERM
- ▲ 3H:1V — SLOPE INDICATOR
- 1% — GRADE INDICATOR
- ① 1 C-020 — DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- ◀ A C-017 — SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP — WASTE ROCK STOCKPILE
- EWRS — EXISTING WASTE ROCK STOCKPILE
- TYP. — TYPICAL
- PIT LAKE EL. 4900 ft-MSL (NOTE 1)
- ▨ SLOPE ARMORING

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC. PIT LAKE ELEVATION PROVIDED BY NMCC.
 2. WRSP-1 TOP SURFACE SHALL BE GRADED TO A HAUL ROAD CHANNEL FOR FLOW CONVEYANCE TOWARD THE PIT.
 3. WRSP-1 RECLAMATION DESIGN SHALL UTILIZE 2.75H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMCC 20.6.7.33. C(3). A 1% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMCC 20.6.7.33. C(2). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 4. PLACE FILL MATERIAL TO PROMOTE DRAINAGE AWAY FROM THE RECLAIMED WRSP-1 TOE.
 5. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.

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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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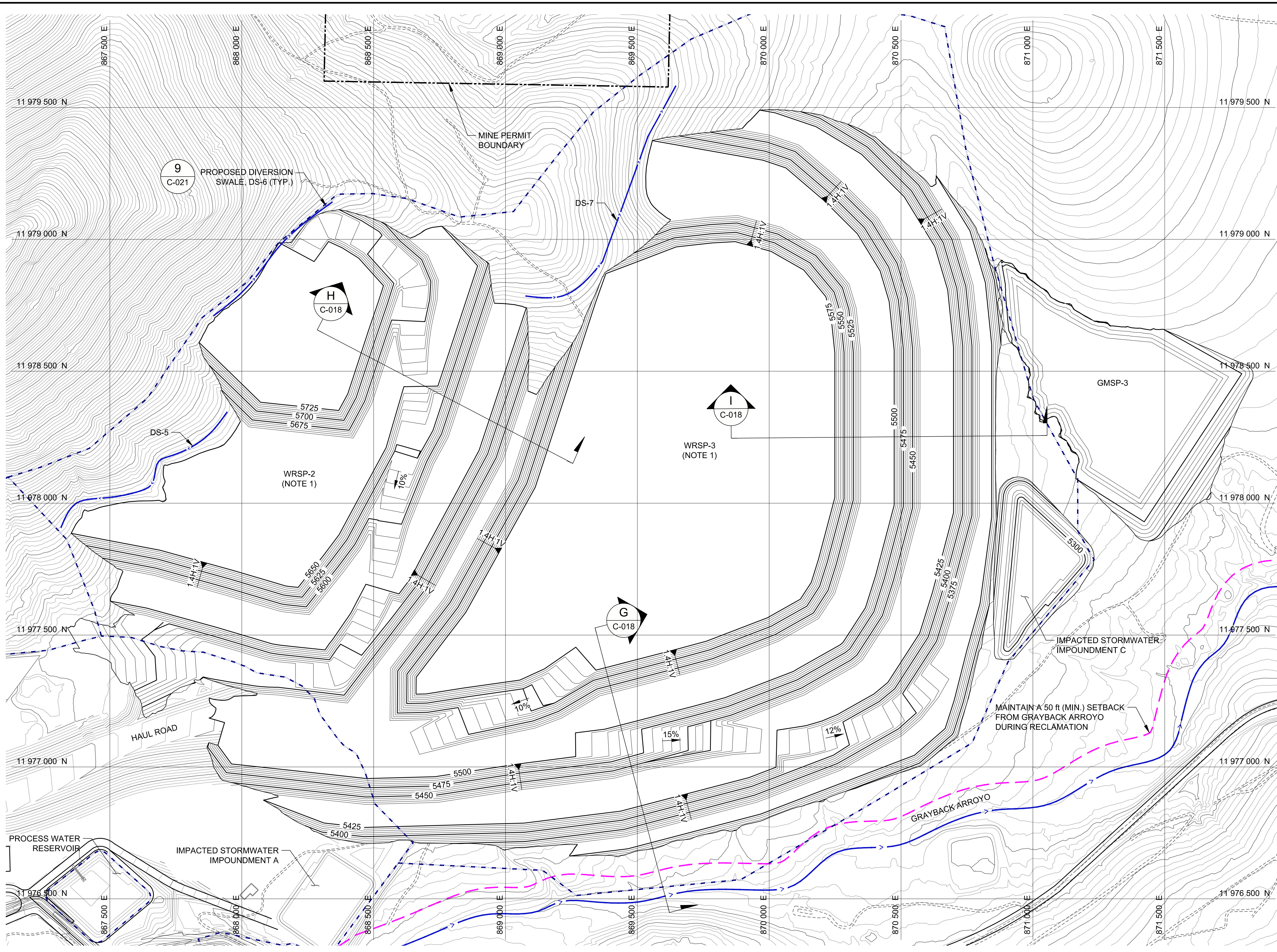
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PROJECT
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MINE RECLAMATION AND CLOSURE PLAN

TITLE
WRSP-1 REGRADE AND DRAINAGE PLAN

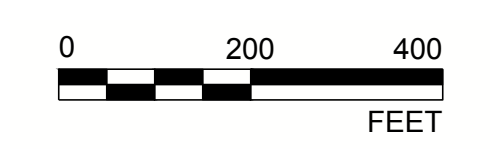
PROJECT NO. 1531453 CONTROL 0300 REV. C 12 of 25 DRAWING C-008

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A4S-D



- LEGEND**
- 5400 — EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
 - 5400 — PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL)
 - - - - MINE PERMIT BOUNDARY
 - - - - WATERSHED BOUNDARY (BY OTHERS)
 - < — EXISTING DIVERSION OR ARROYO
 - - - - EXISTING ROAD
 - — — — SETBACK
 - ▲ 3H:1V — SLOPE INDICATOR
 - ▶ 1% — GRADE INDICATOR
 - ⬅ A — SECTION CALLOUT
C-017 — SECTION ID
DRAWING LOCATION
 - WRSP — WASTE ROCK STOCKPILE
 - GMSP — GROWTH MEDIA STOCKPILE
 - MIN. — MINIMUM
 - TYP. — TYPICAL

NOTES
 1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.



Path: \\ussm\cadd\2015 Projects\1531453\PRODUCTION\03001_1 File Name: 1531453-0300-C-009.dwg

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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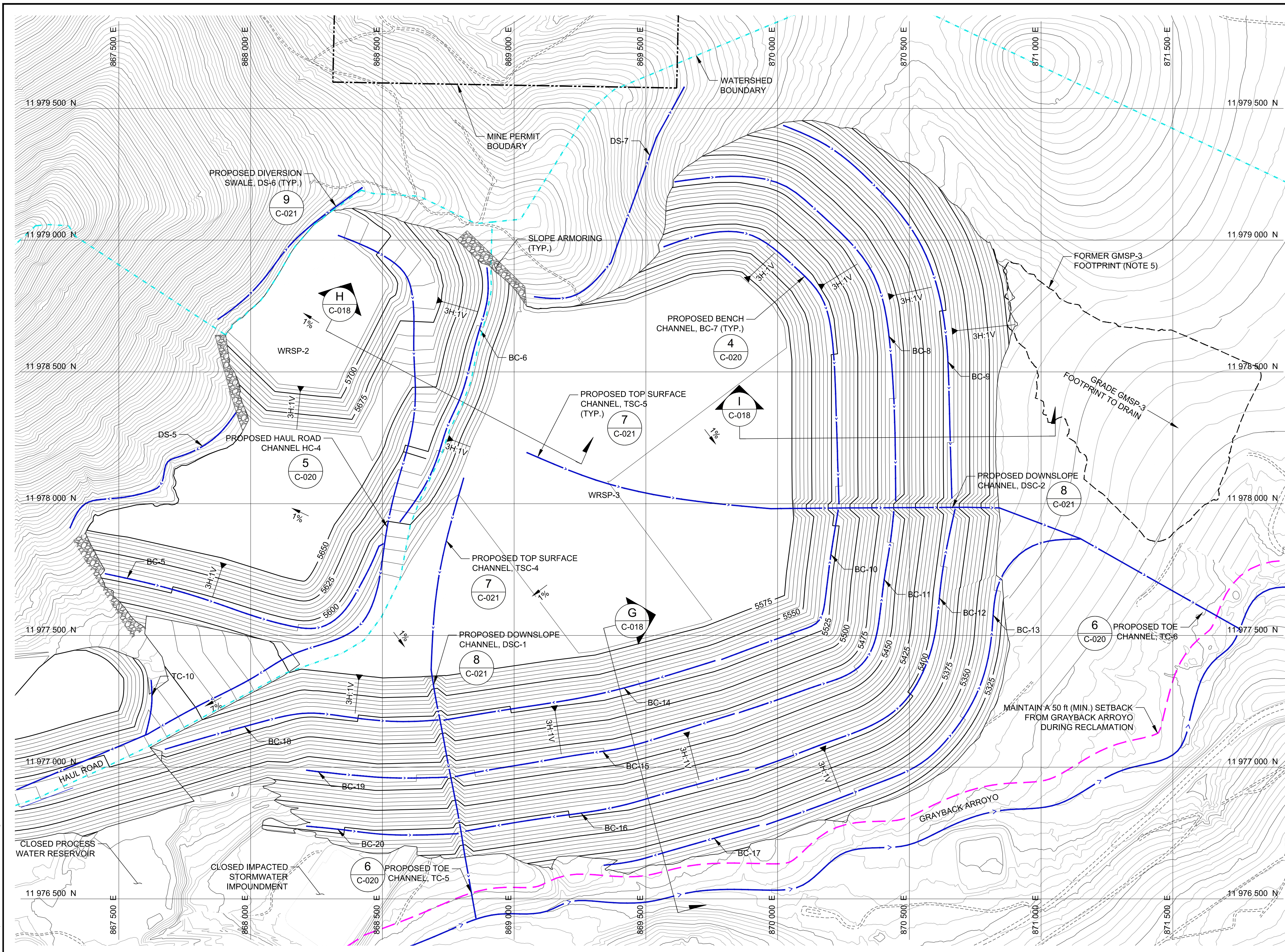
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 UNITED STATES OF AMERICA
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PROJECT
 COPPER FLAT PROJECT
 MINE RECLAMATION AND CLOSURE PLAN

TITLE
WRSP-2 AND WRSP-3 AT FINAL BUILDOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 13 of 25 DRAWING C-009

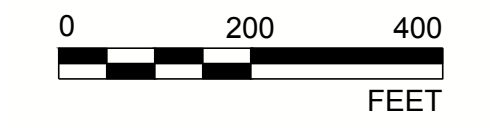
1 in = 1000 feet IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A NS D



LEGEND

- 5400 — EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 — PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 6)
- - - - MINE PERMIT BOUNDARY
- - - - RECLAMATION WATERSHED BOUNDARY
- < — EXISTING DIVERSION OR ARROYO
- < — PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- ===== EXISTING ROAD
- ===== PROPOSED ROAD
- — — — SETBACK
- ▲ 3H:1V — SLOPE INDICATOR
- 1% — GRADE INDICATOR
- ① C-020 — DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- ◀ A C-017 — SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP — WASTE ROCK STOCKPILE
- TYP. — TYPICAL
- ▨ — SLOPE ARMORING

- NOTES**
- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 - WRSP-2 AND WRSP-3 TOP SURFACE SHALL BE GRADED TOWARD THE SOUTH, WEST AND EAST, AND UTILIZE PLANNED HAUL ROADS WHERE PRACTICAL FOR STORMWATER CONVEYANCE.
 - WRSP-2 AND WRSP-3 RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMCC 20.6.7.33. C(3). A 1% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMCC 20.6.7.33. C(2). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 - IMPACTED STORMWATER IMPOUNDMENT A AND THE PROCESS WATER RESERVOIR TO BE RECLAIMED AS PART OF THE PLANT AREA RECLAMATION PLAN (SEE DRAWING C-016).
 - FOOTPRINT OF FORMER GROWTH MEDIA STOCKPILE 3 WILL BE GRADED TO APPROXIMATE SURROUNDING TOPOGRAPHY, RIPPED AND REVEGETATED.
 - PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.

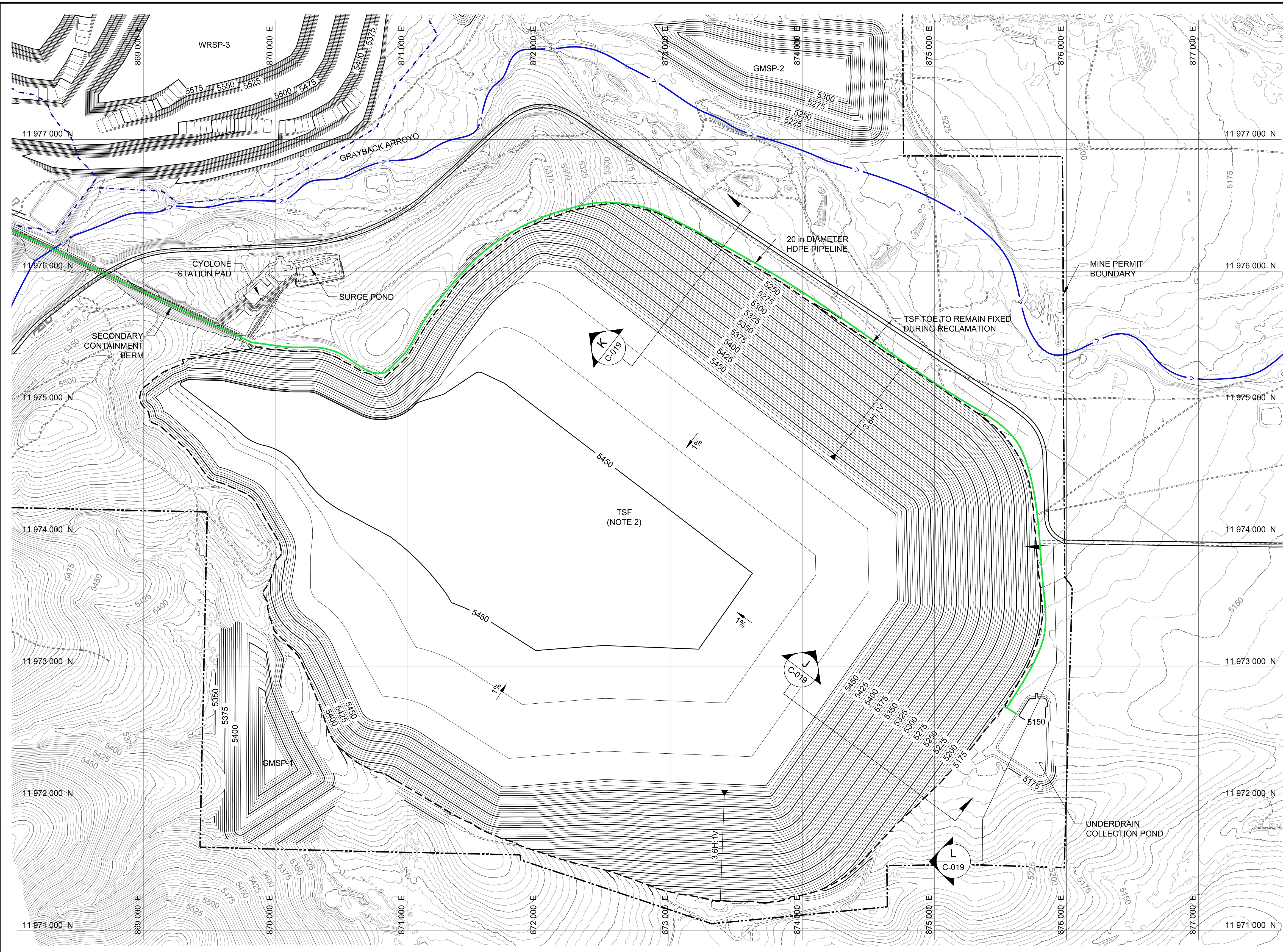


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<p>CONSULTANT</p> <p>Golder Associates</p>	<p>TUCSON OFFICE 4730 N. ORACLE ROAD, SUITE 210 TUCSON, ARIZONA UNITED STATES OF AMERICA [+1] (520) 888 8818 www.golder.com</p>	<p>PROJECT NO. 1531453</p> <p>CONTROL 0300</p> <p>REV. C</p>	<p>14 of 25</p> <p>DRAWING C-010</p>

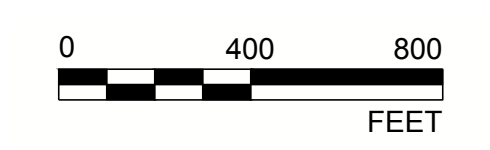
REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



- LEGEND**
- EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
 - PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTES 1 AND 2)
 - MINE PERMIT BOUNDARY
 - WATERSHED BOUNDARY (BY OTHERS)
 - EXISTING DIVERSION OR ARROYO
 - EXISTING ROAD
 - SLOPE INDICATOR
 - GRADE INDICATOR
 - SECTION CALLOUT
SECTION ID
DRAWING LOCATION
 - WRSP WASTE ROCK STOCKPILE
 - GMSP GROWTH MEDIA STOCKPILE
 - TSF TAILING STORAGE FACILITY

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. FINAL BUILDOUT TOPOGRAPHY FOR THE TSF WAS DEVELOPED BY GOLDER AND IS FROM THE 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN REPORT (GOLDER, 2015).



Path: \\ussm\cadd\2015 Projects\1531453\PRODUCTION\0300\1 File Name: 1531453-0300-C-011.dwg

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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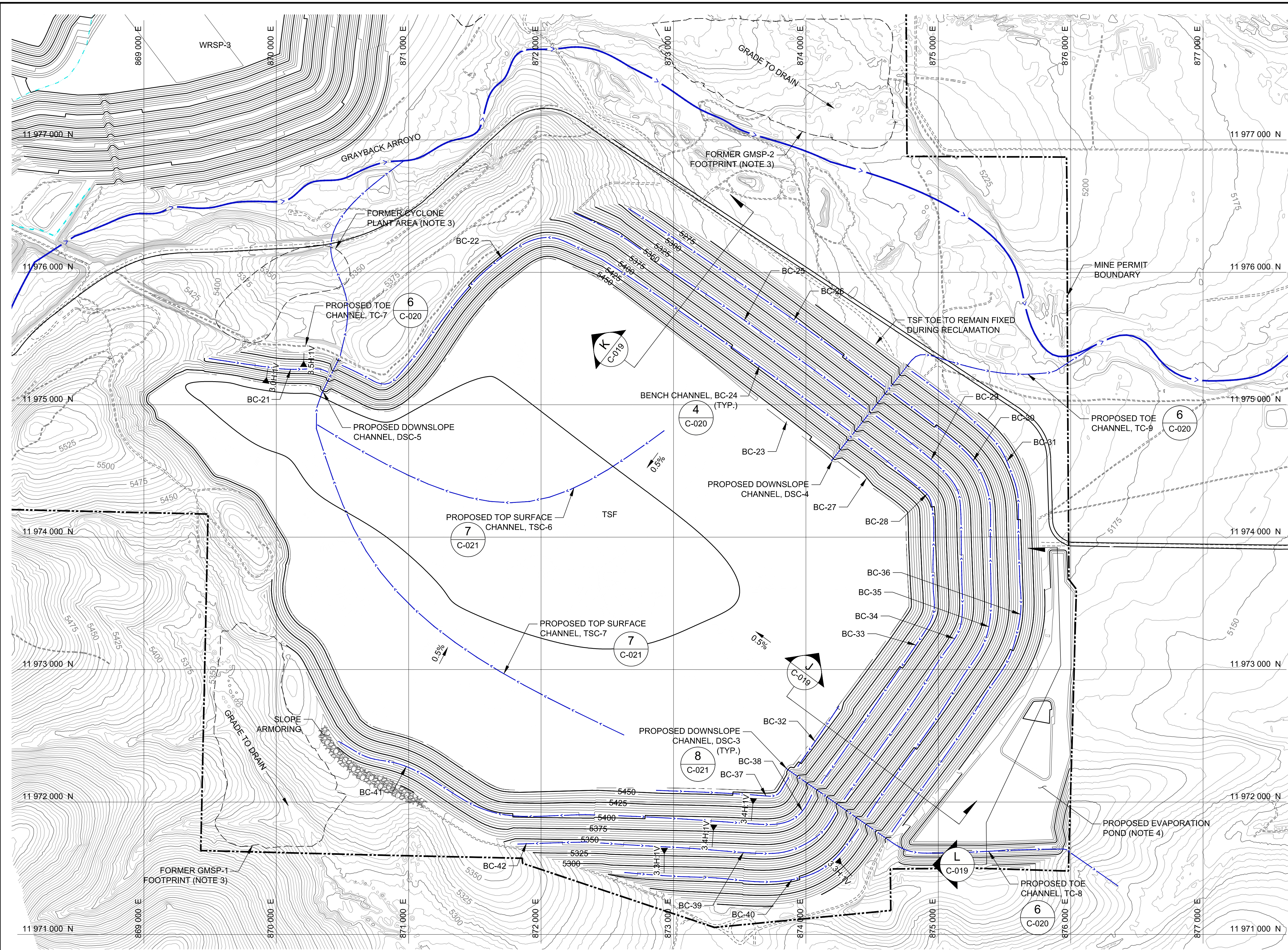
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
TSF AT FINAL BUILDOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 15 of 25 DRAWING C-011

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM AANSI D



LEGEND

- EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 5)
- MINE PERMIT BOUNDARY
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- EXISTING ROAD
- PROPOSED ROAD
- SETBACK
- 3H:1V SLOPE INDICATOR
- 1% GRADE INDICATOR
- 1
C-020
DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- A
C-017
SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP WASTE ROCK STOCKPILE
- TSF TAILING STORAGE FACILITY
- SLOPE ARMORING

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMAC 20.6.7.33. C(3). A 0.5% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMAC 20.6.7.33. C(1). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 3. FOOTPRINT OF FORMER GROWTH MEDIA STOCKPILES AND CYCLONE PLANT AREA WILL BE GRADED TO APPROXIMATE SURROUNDING TOPOGRAPHY, RIPPED AND REVEGETATED.
 4. EVAPORATION POND WILL BE CONSTRUCTED IN YEAR 5 FOLLOWING CESSATION OF MINING, NEAR THE END OF THE ACTIVE EVAPORATION PROGRAM. EVAPORATION POND WILL BE RECLAIMED AT THE END OF THE PASSIVE EVAPORATION PROGRAM.
 5. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.

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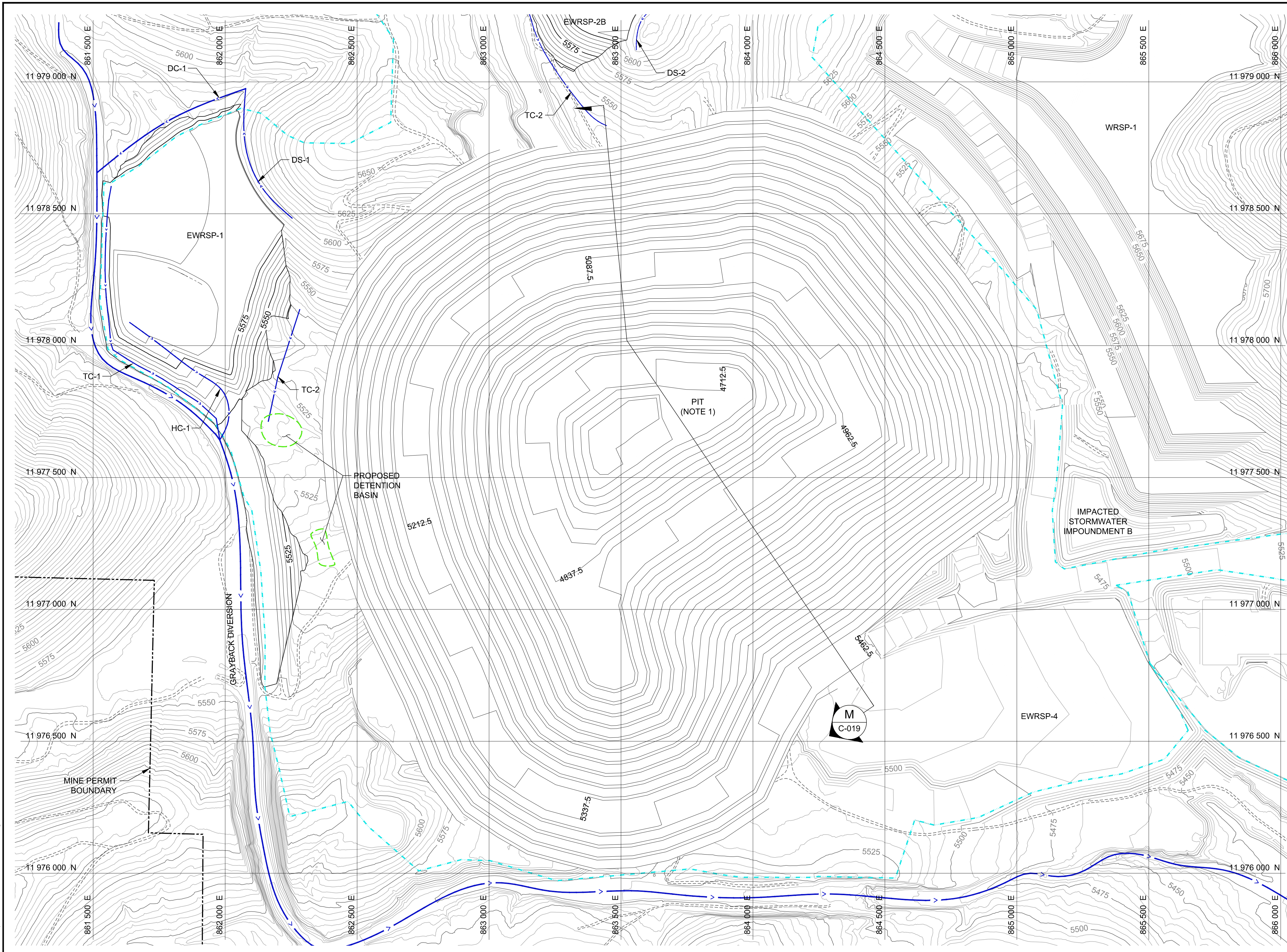
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PROJECT
COPPER FLAT PROJECT
MINE PERMIT CLOSURE DESIGN

TITLE
TSF RECLAMATION AND DRAINAGE PLAN

PROJECT NO. 1531453 CONTROL 0300 REV. C 16 of 25 DRAWING C-012

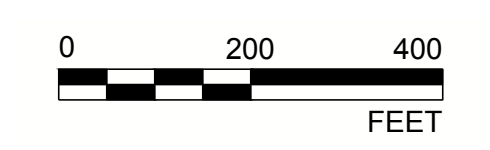
1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM AANSI D



LEGEND

- EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL)
- MINE PERMIT BOUNDARY
- EXISTING FACILITY BOUNDARY (APPROXIMATE)
- PROPOSED FACILITY BOUNDARY
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- EXISTING ROAD
- SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WASTE ROCK STOCKPILE
- EXISTING WASTE ROCK STOCKPILE

NOTES
 1. PLANNED END OF MINE LIFE TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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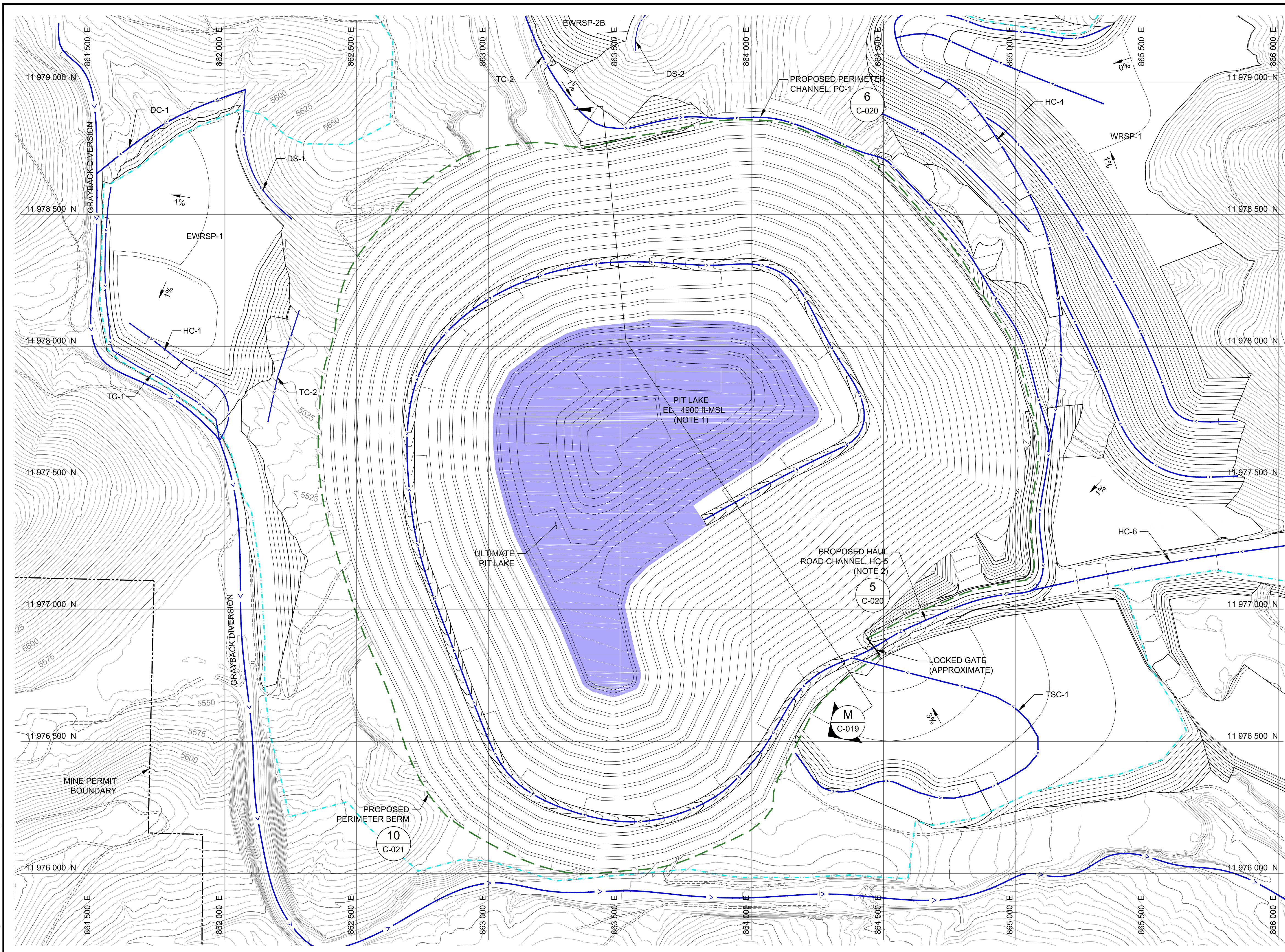
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PROJECT
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 MINE RECLAMATION AND CLOSURE PLAN

TITLE
PIT AT FINAL BUILDOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 17 of 25 DRAWING C-013

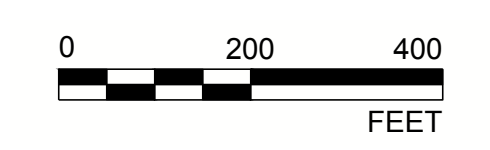
1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



LEGEND

- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 4)
- MINE PERMIT BOUNDARY
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- EXISTING ROAD
- PROPOSED PERIMETER BERM
- GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP WASTE ROCK STOCKPILE
- EWRSP EXISTING WASTE ROCK STOCKPILE
- PIT LAKE EL. 4900 ft-MSL (NOTE 1)

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC. PIT LAKE ELEVATION PROVIDED BY NMCC.
 2. THE PLANNED HAUL ROADS SHALL BE UTILIZED FOR STORMWATER CONVEYANCE TO THE PIT.
 3. ASSUMED 100 foot WIDE DISTURBANCE AREA AROUND PERIMETER OF THE PIT TO BE RECLAIMED.
 4. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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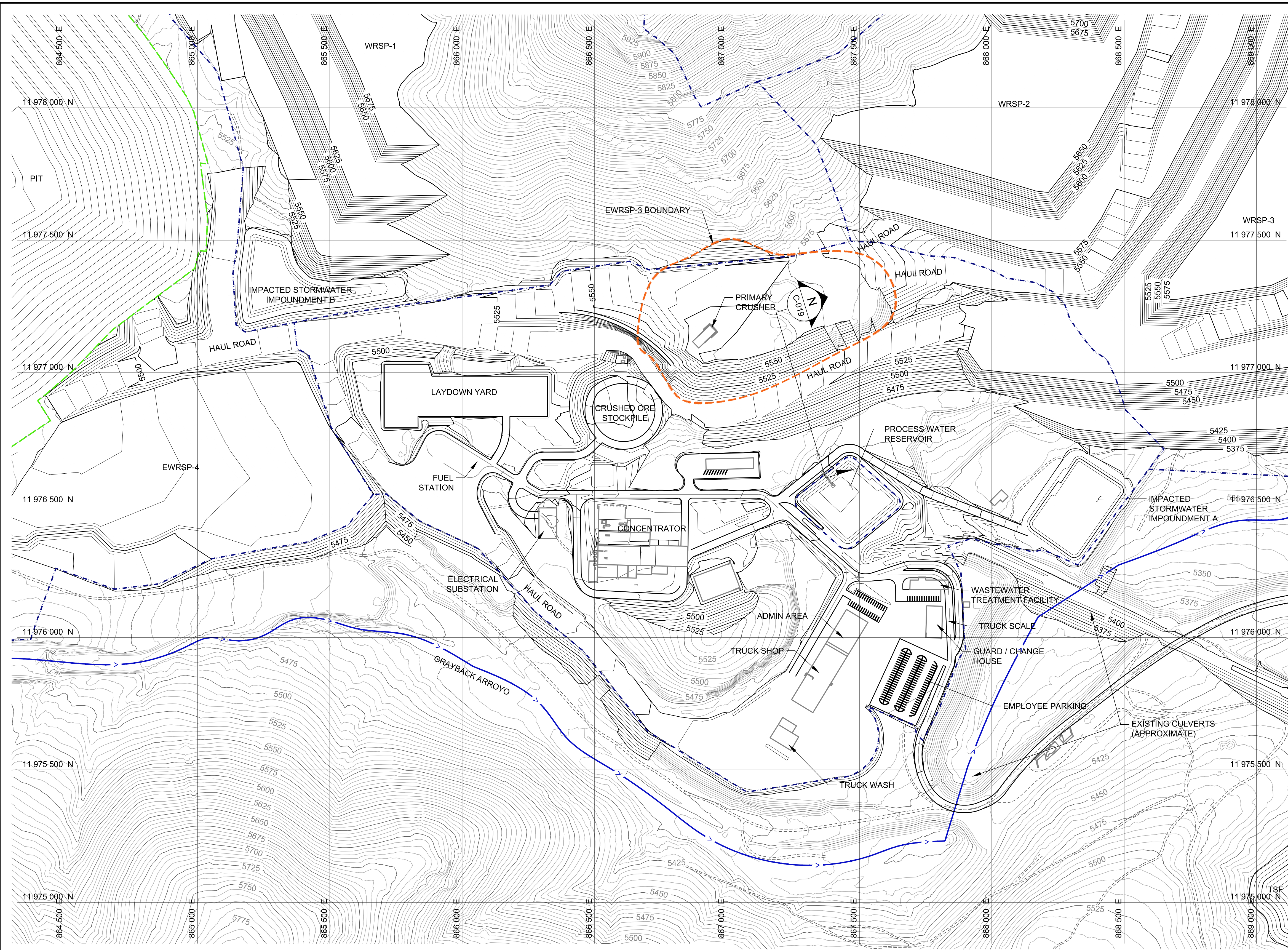
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PERMIT

TITLE
PIT RECLAMATION AND DRAINAGE PLAN

PROJECT NO. 1531453 CONTROL 0300 REV. C 18 of 25 DRAWING C-014



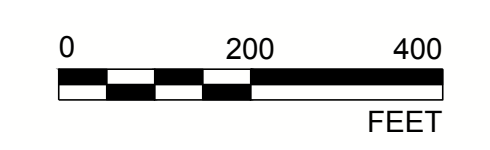
LEGEND

- EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 1)
- EXISTING FACILITY BOUNDARY (APPROXIMATE)
- PROPOSED FACILITY BOUNDARY
- WATERSHED BOUNDARY (BY OTHERS)
- EXISTING DIVERSION OR ARROYO
- EXISTING ROAD

SECTION CALLOUT
 SECTION ID
 DRAWING LOCATION

WRSP WASTE ROCK STOCKPILE
EWRSP EXISTING WASTE ROCK STOCKPILE

NOTES
 1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.



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C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2016-09-02	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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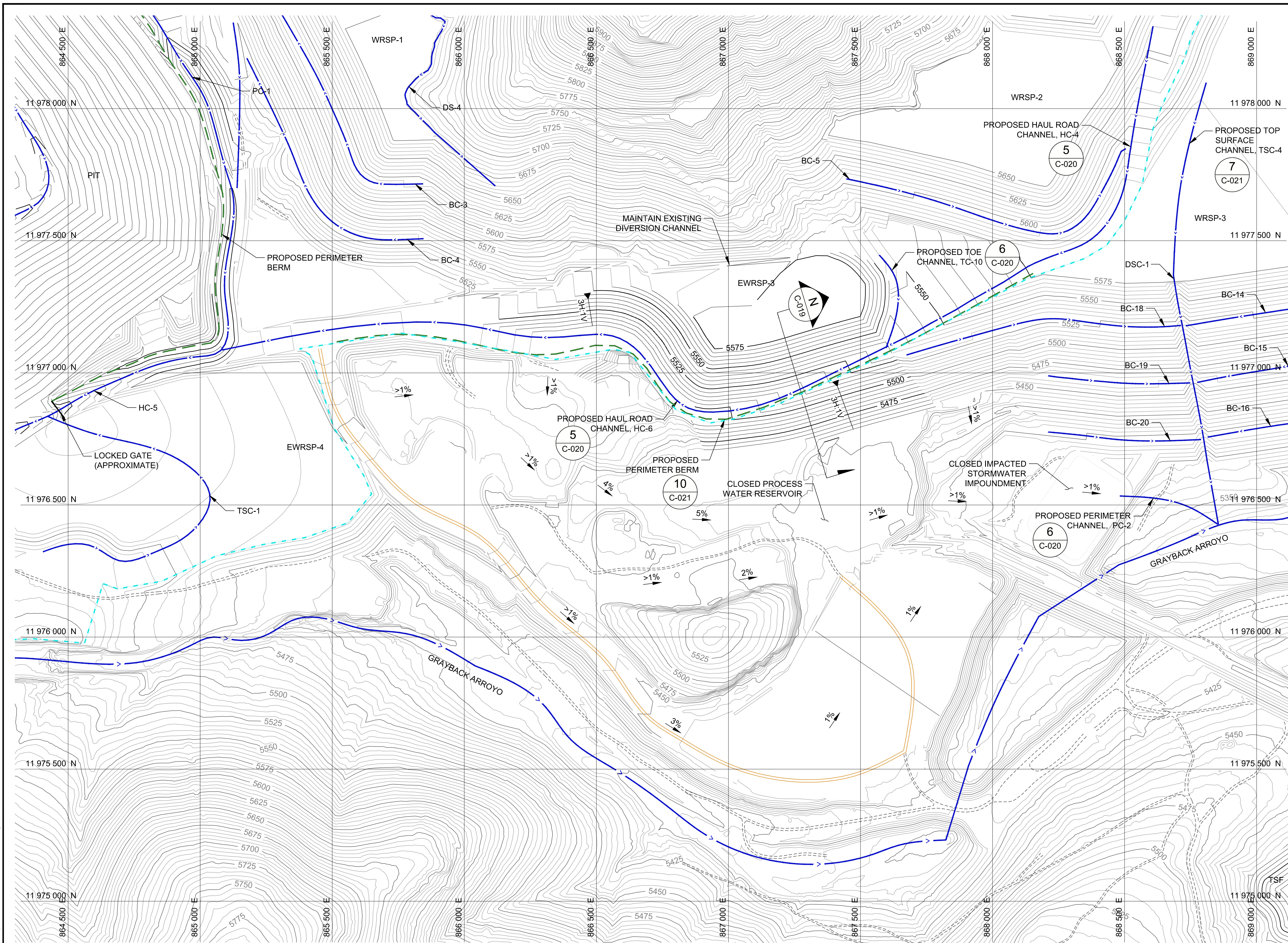
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PROJECT
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 MINE RECLAMATION AND CLOSURE PLAN

TITLE
PLANT AREA AT FINAL BUILDOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 19 of 25 DRAWING C-015

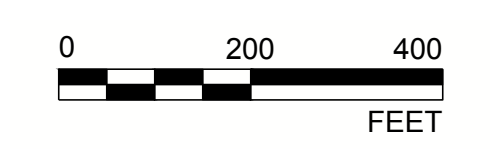
1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS D



LEGEND

- 7600 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 7580 PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 4)
- RECLAMATION WATERSHED BOUNDARY
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- EXISTING ROAD
- PROPOSED ROAD
- PROPOSED PERIMETER BERM
- 3H:1V SLOPE INDICATOR
- 1% GRADE INDICATOR
- DETAIL CALLOUT
DETAIL ID
DRAWING LOCATION
- SECTION CALLOUT
SECTION ID
DRAWING LOCATION
- WRSP WASTE ROCK STOCKPILE
- EWRSP EXISTING WASTE ROCK STOCKPILE

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. FINAL BUILDOUT TOPOGRAPHY FOR THE TSF WAS DEVELOPED BY GOLDER AND IS FROM THE 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN REPORT (GOLDER, 2015).
 3. PLANNED HAUL ROADS WILL BE UTILIZED FOR STORMWATER CONVEYANCE TO THE PIT.
 4. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.



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C	2016-10-14	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	TC
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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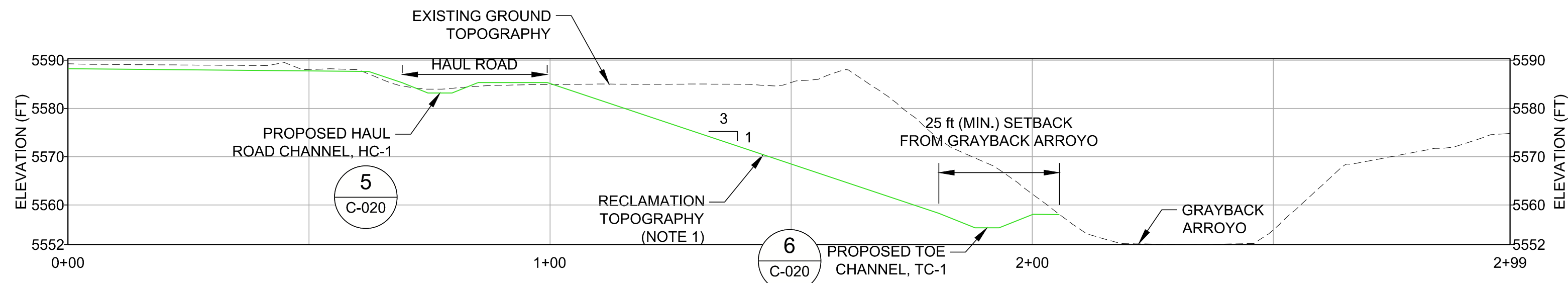
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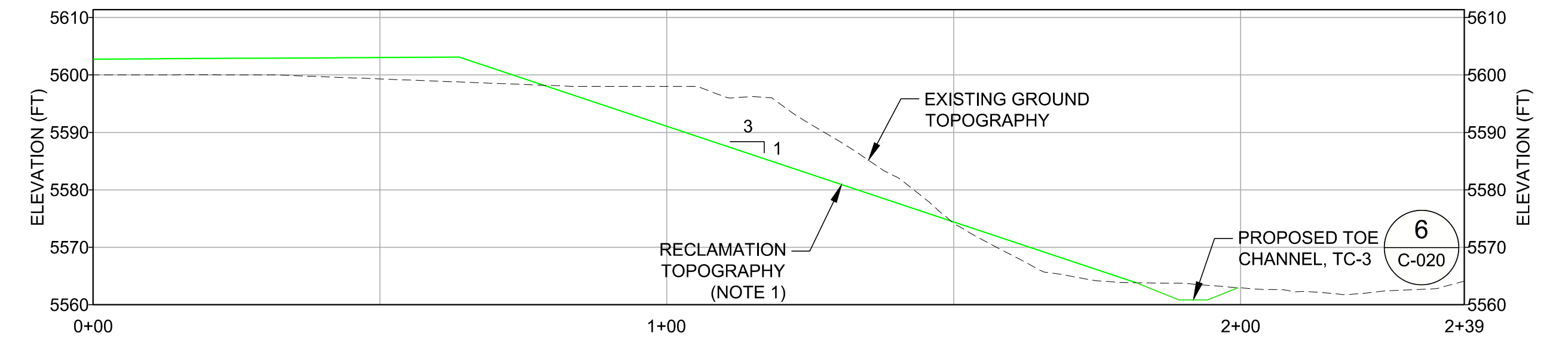
TITLE
PLANT AREA RECLAMATION AND DRAINAGE PLAN

PROJECT NO. 1531453 CONTROL 0300 REV. C 20 of 25 DRAWING C-016

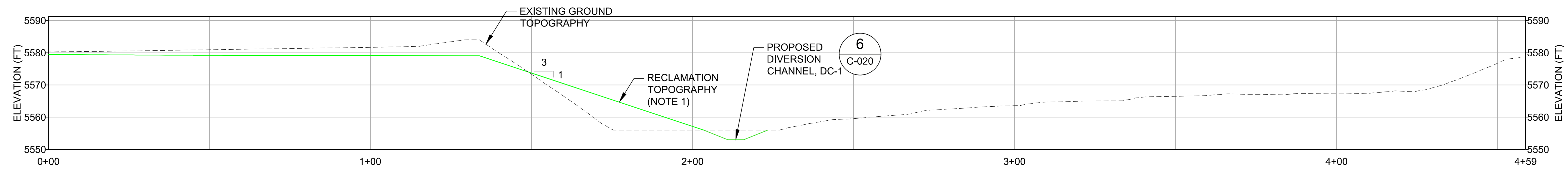
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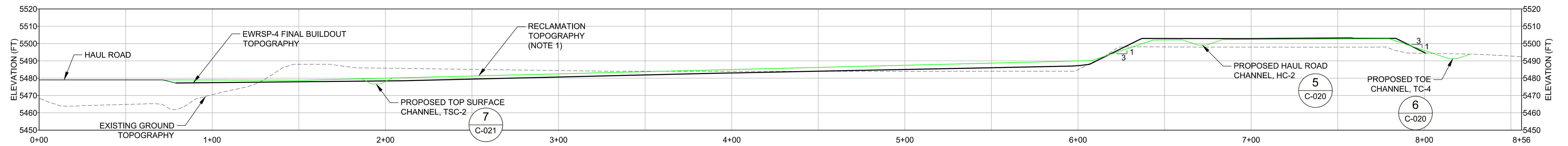
EWRSRSP-1 OUTSLOPE SECTION A
 (SEE DRAWING C-002)
 SCALE N.T.S. A C-017



EWRSRSP-2B OUTSLOPE SECTION C
 (SEE DRAWING C-004)
 SCALE N.T.S. C C-017



EWRSRSP-1 OUTSLOPE SECTION B
 (SEE DRAWING C-002)
 SCALE N.T.S. B C-017



EWRSRSP-4 SECTION D
 (SEE DRAWING C-006)
 SCALE N.T.S. D C-017

NOTES
 1. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.

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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-10-15	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
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A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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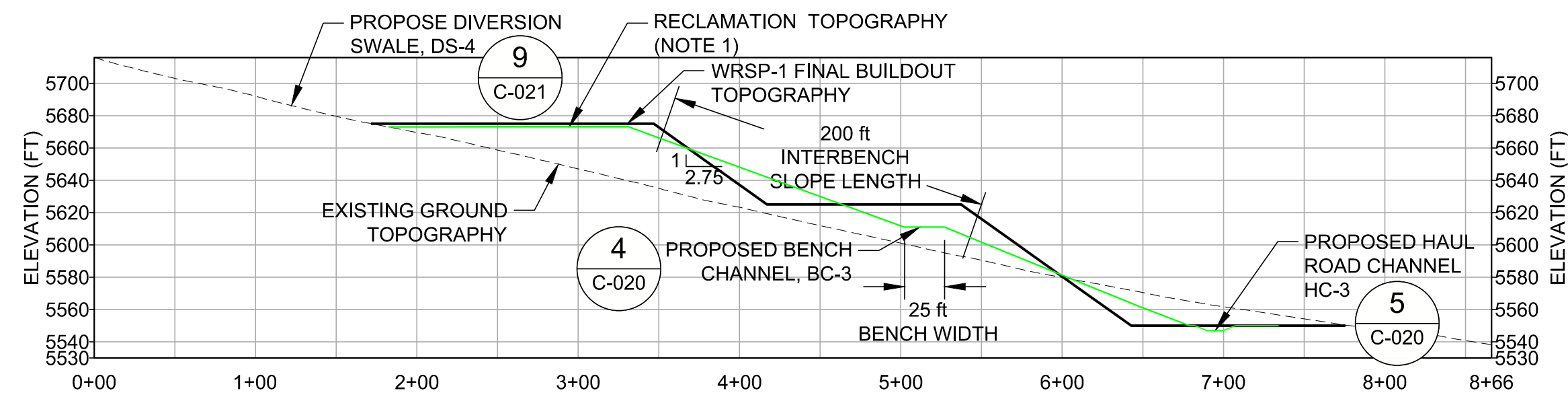
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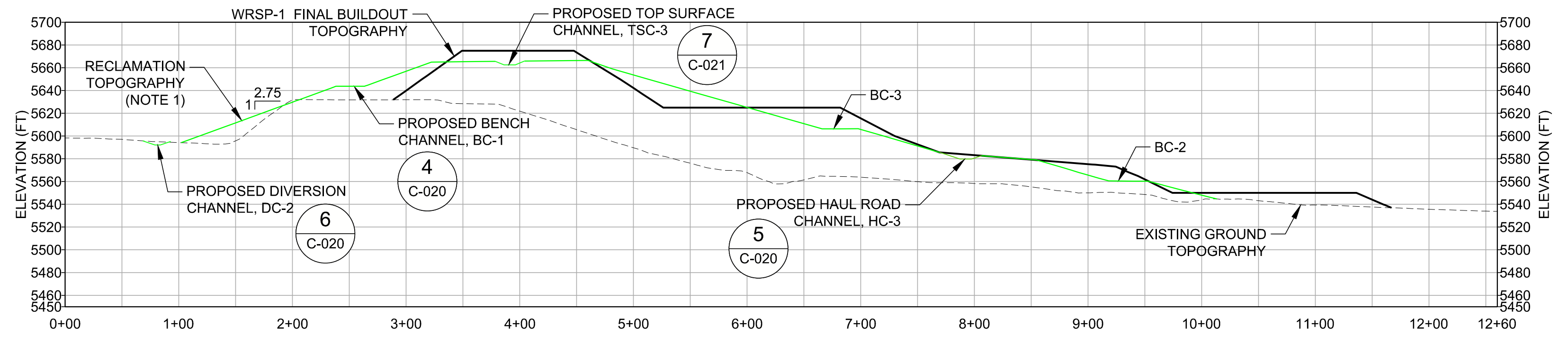
PROJECT
 COPPER FLAT PROJECT
 MINE RECLAMATION AND CLOSURE PLAN

TITLE
EWRSR RECLAMATION SECTIONS

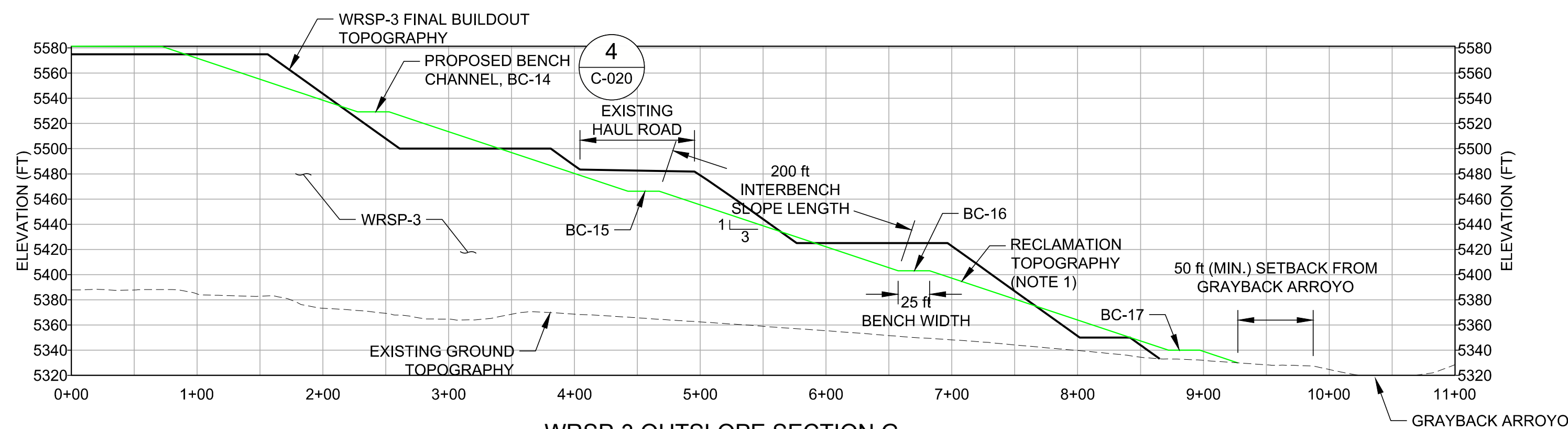
PROJECT NO. 1531453 CONTROL 0300 REV. C 21 of 25 DRAWING C-017



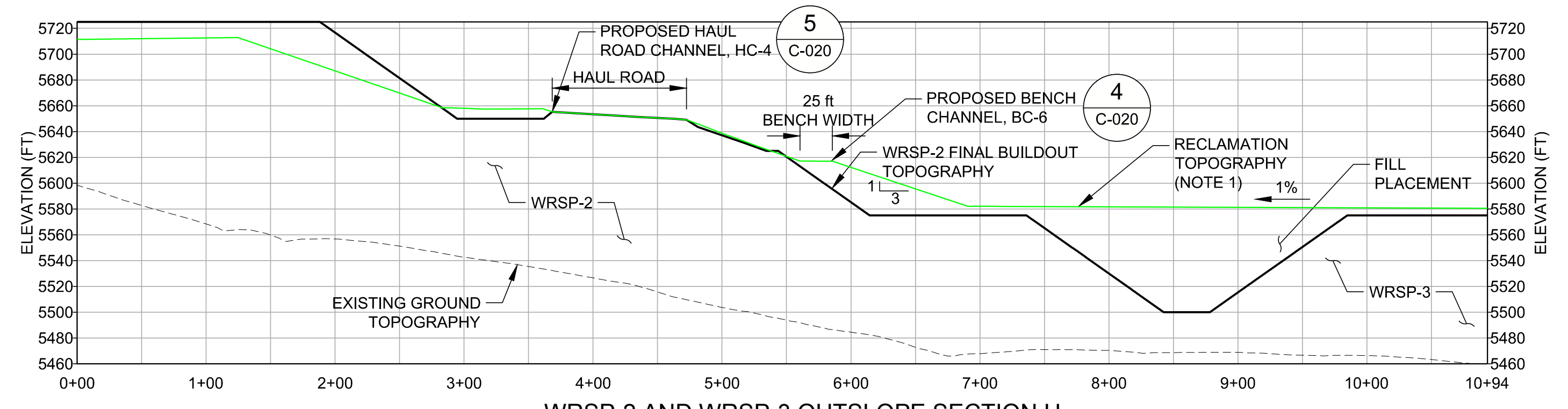
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C-018



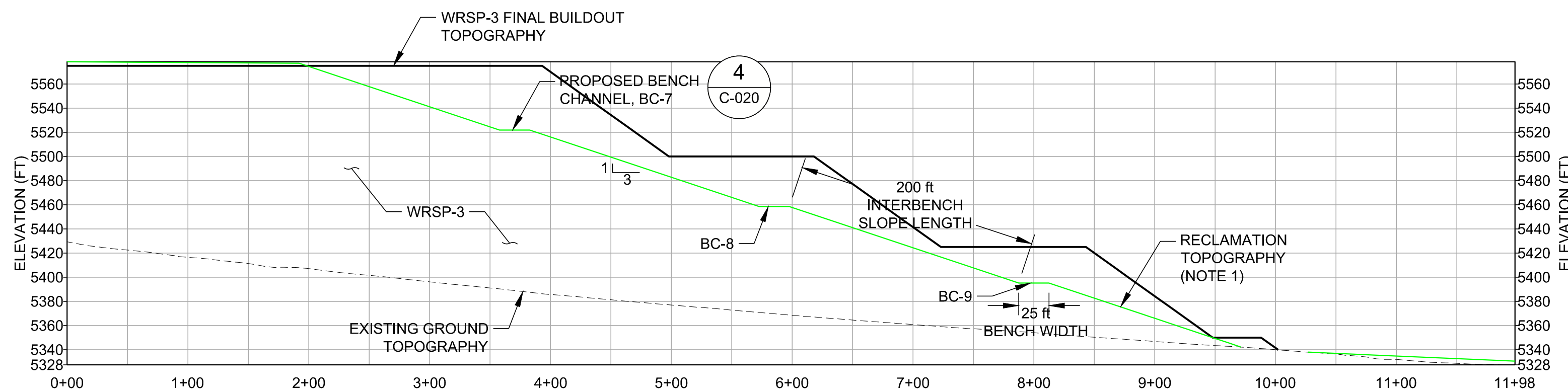
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WRSP-3 OUTSLOPE SECTION G
SCALE N.T.S. G (SEE DRAWING C-010)
C-018



WRSP-2 AND WRSP-3 OUTSLOPE SECTION H
SCALE N.T.S. H (SEE DRAWING C-010)
C-018



WRSP-3 OUTSLOPE SECTION I
SCALE N.T.S. I (SEE DRAWING C-010)
C-018

NOTES
1. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.

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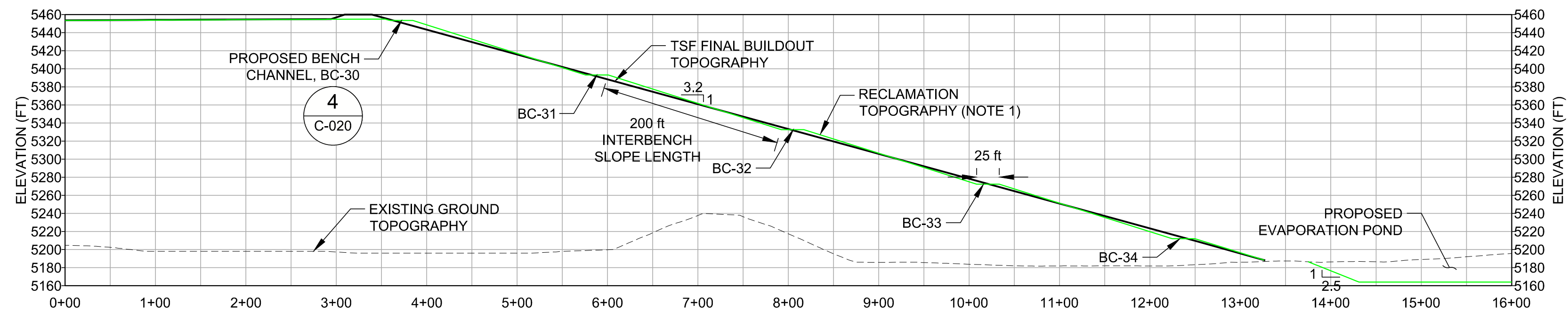


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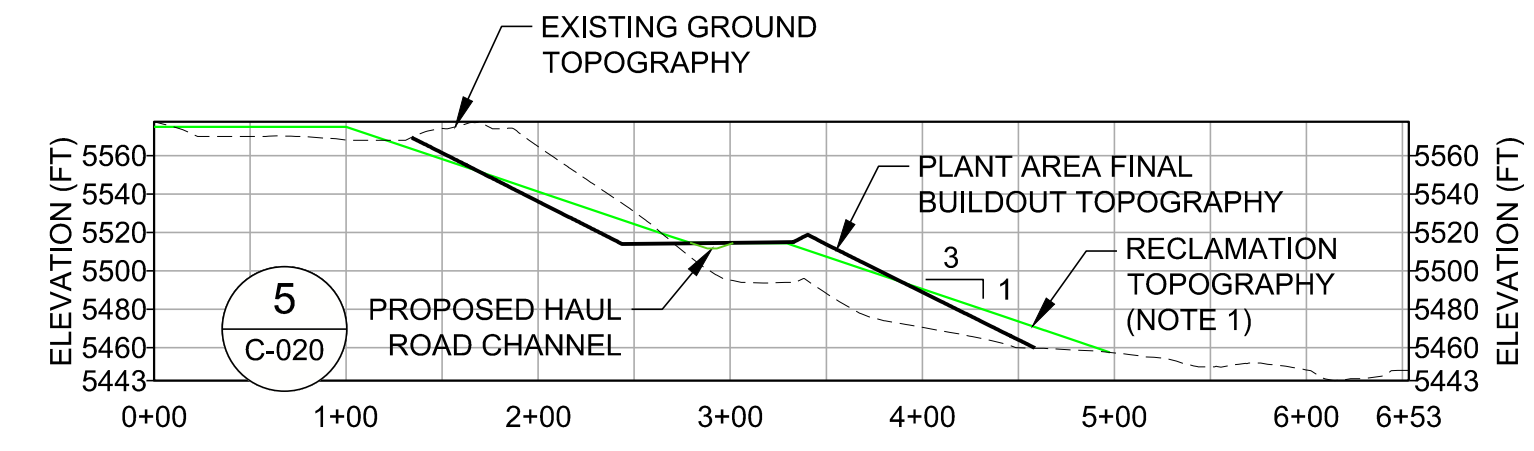
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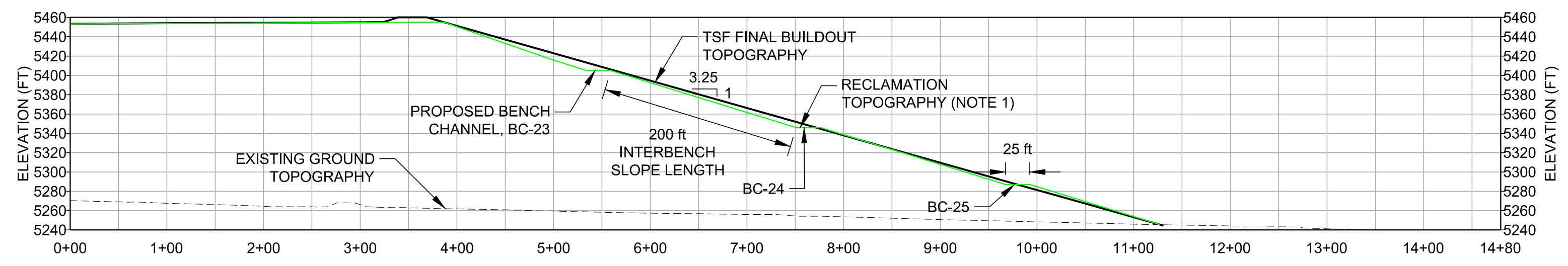
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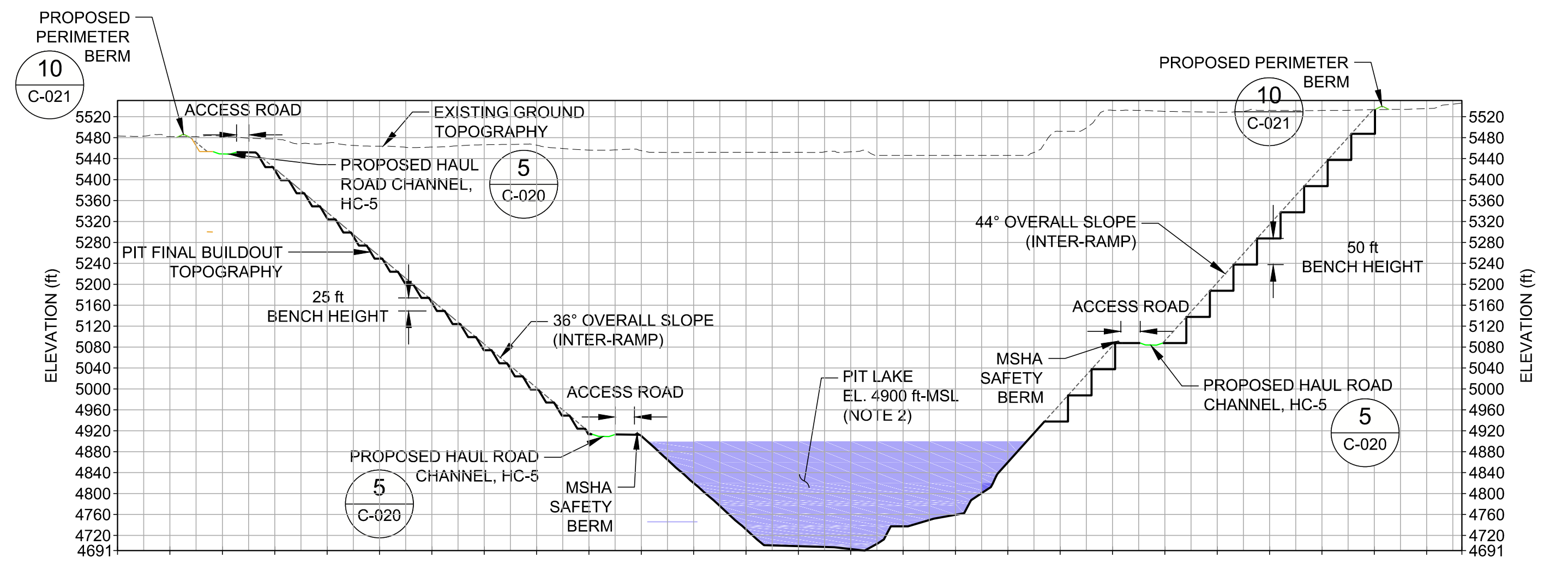
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PLANT AREA OUTSLOPE SECTION N
(SEE DRAWING C-016)
SCALE N.T.S. N
C-019

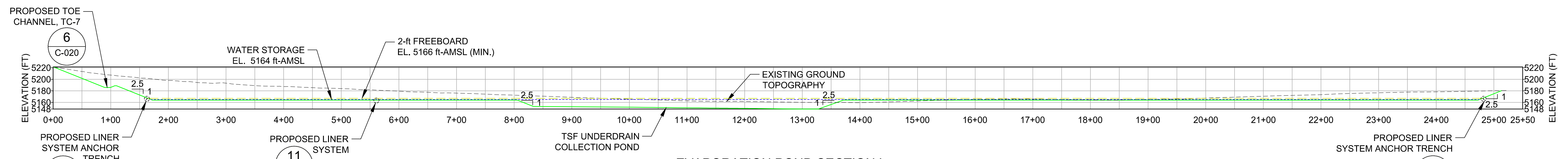


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(SEE DRAWING C-012)
SCALE N.T.S. K
C-019



PIT SLOPE SECTION M
(SEE DRAWING C-014)
SCALE N.T.S. M
C-019

- NOTES**
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 2. PIT LAKE ELEVATION PROVIDED BY NMCC.



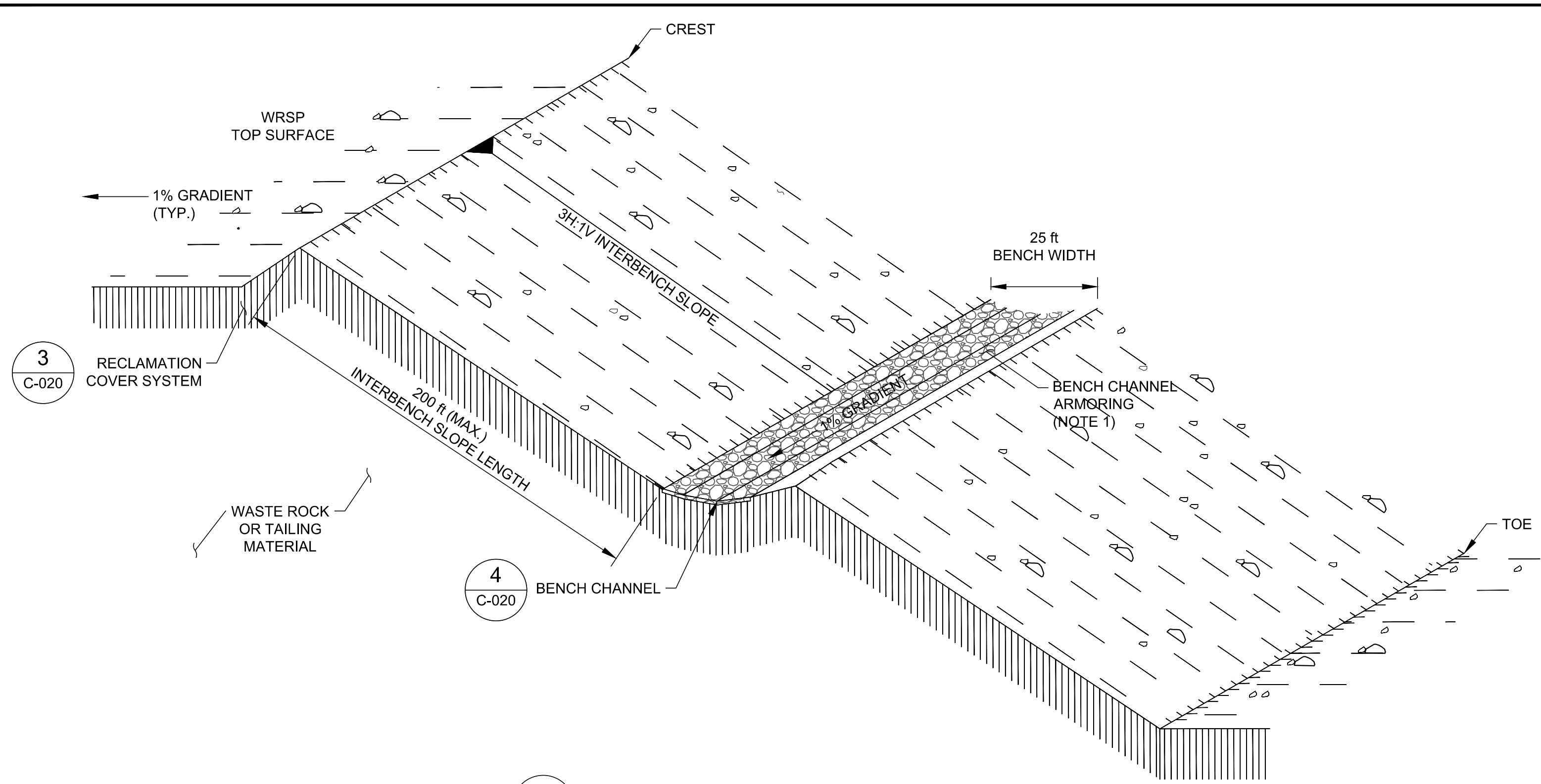
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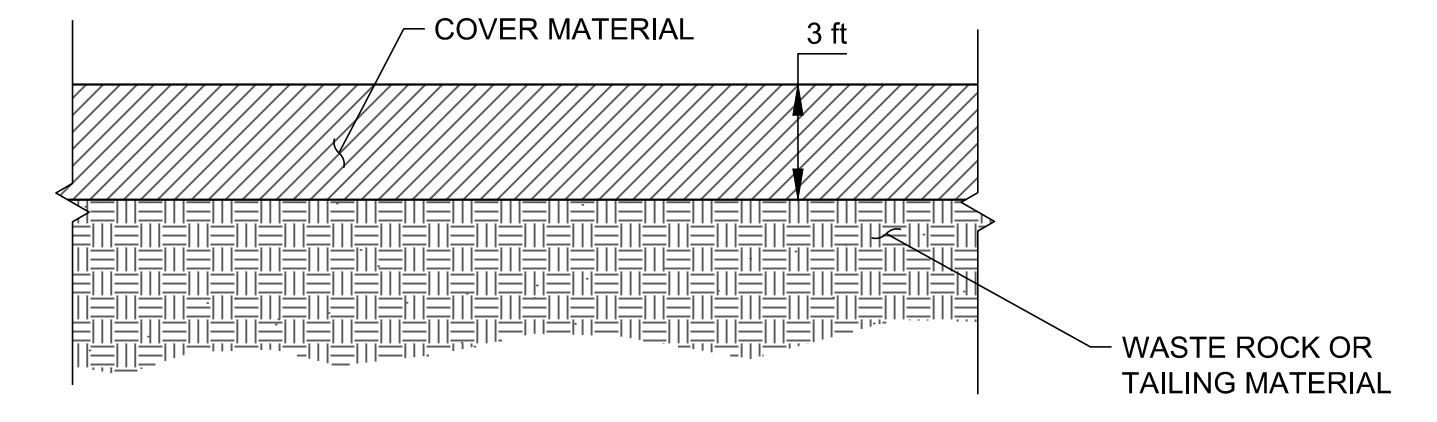
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CONSULTANT Golder Associates		TUCSON OFFICE 4730 N. ORACLE ROAD, SUITE 210 TUCSON, ARIZONA UNITED STATES OF AMERICA [+1] (520) 888 8818 www.golder.com	TITLE TSF, PIT AND PLANT RECLAMATION SECTIONS
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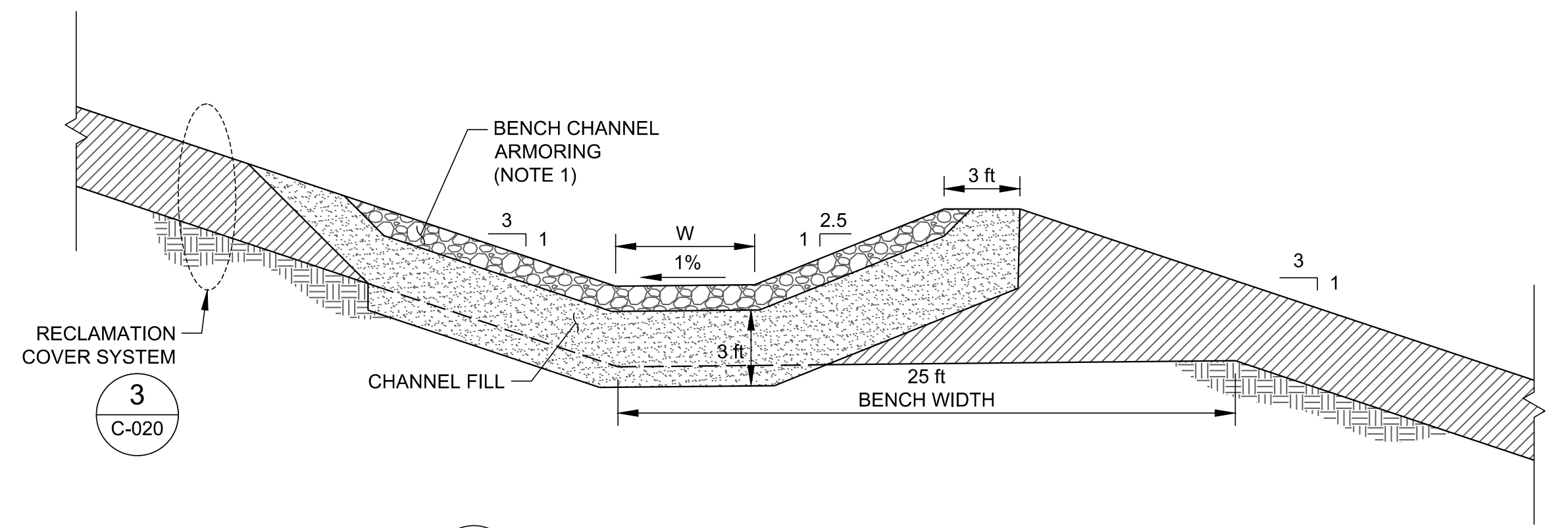
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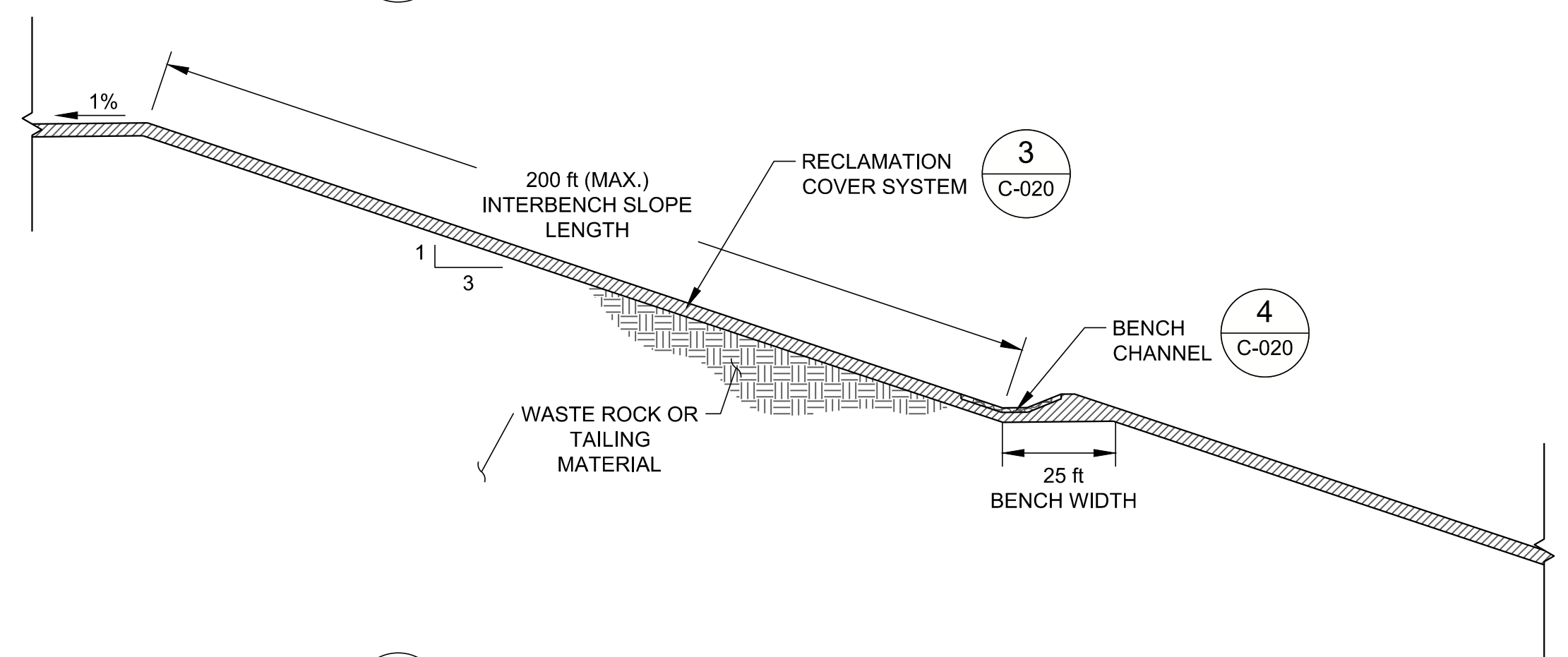
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C-020



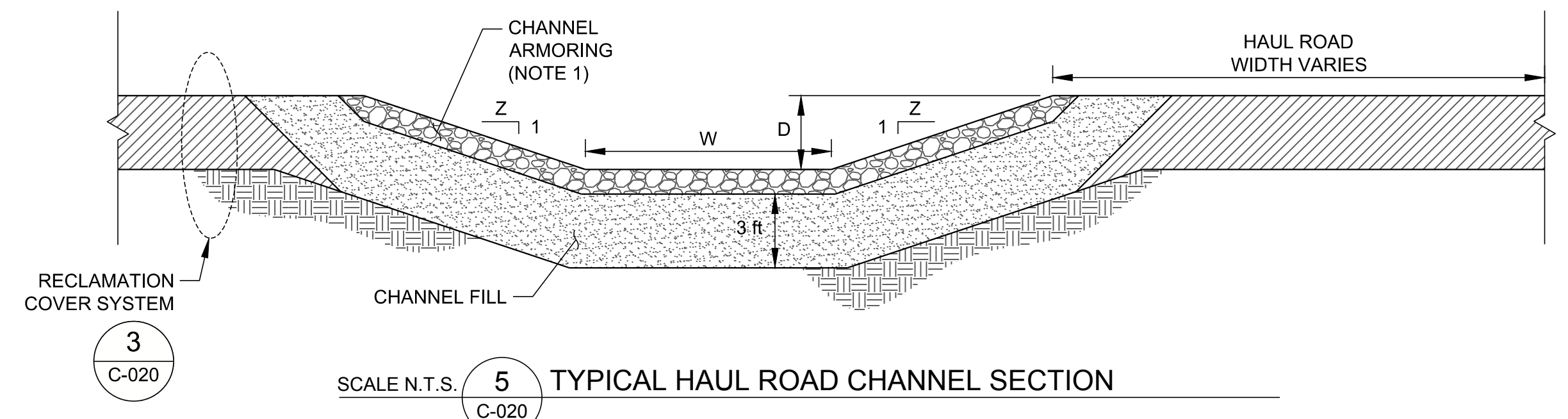
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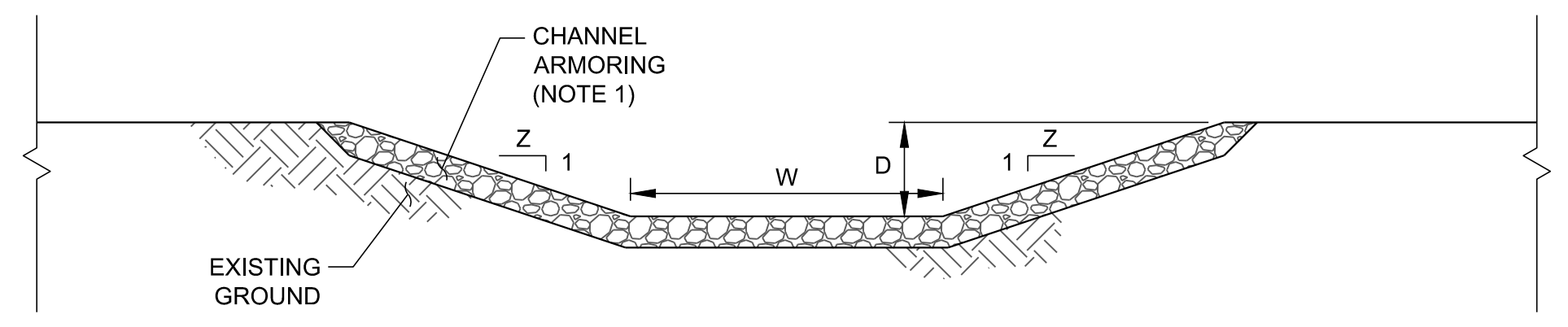
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SCALE N.T.S. 2 TYPICAL OUTSLOPE SECTION
C-020



SCALE N.T.S. 5 TYPICAL HAUL ROAD CHANNEL SECTION
C-020



SCALE N.T.S. 6 TYPICAL DIVERSION, TOE, AND PERIMETER CHANNEL
C-020

NOTES
1. SEE CHANNEL SCHEDULE FOR DIMENSION AND ARMORING REQUIREMENTS. MINIMUM RIPRAP THICKNESS SHALL BE 2 X D₅₀. MINIMUM BEDDING THICKNESS IS 6 in.

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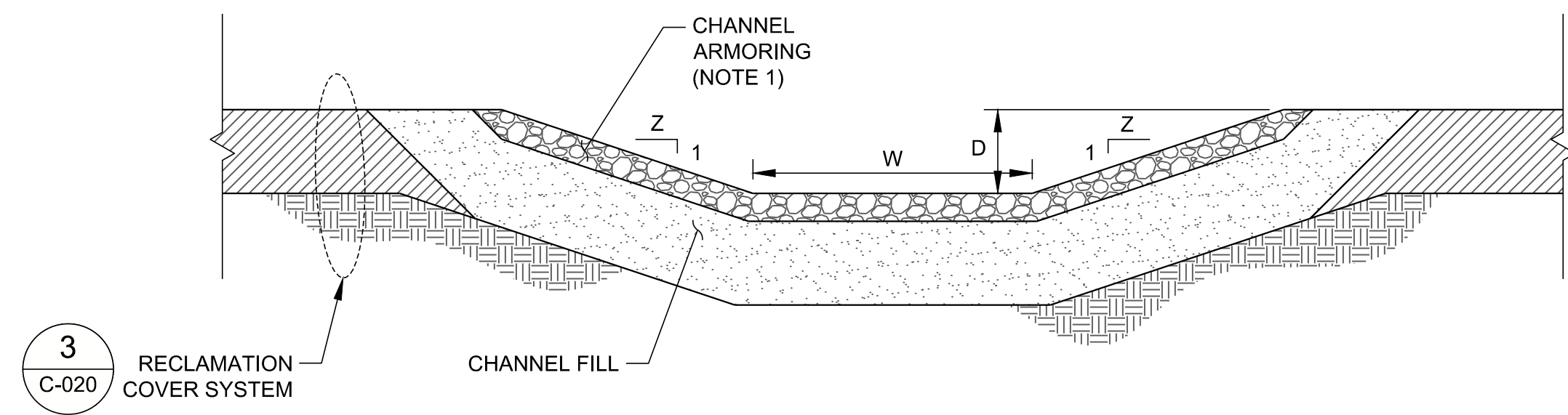
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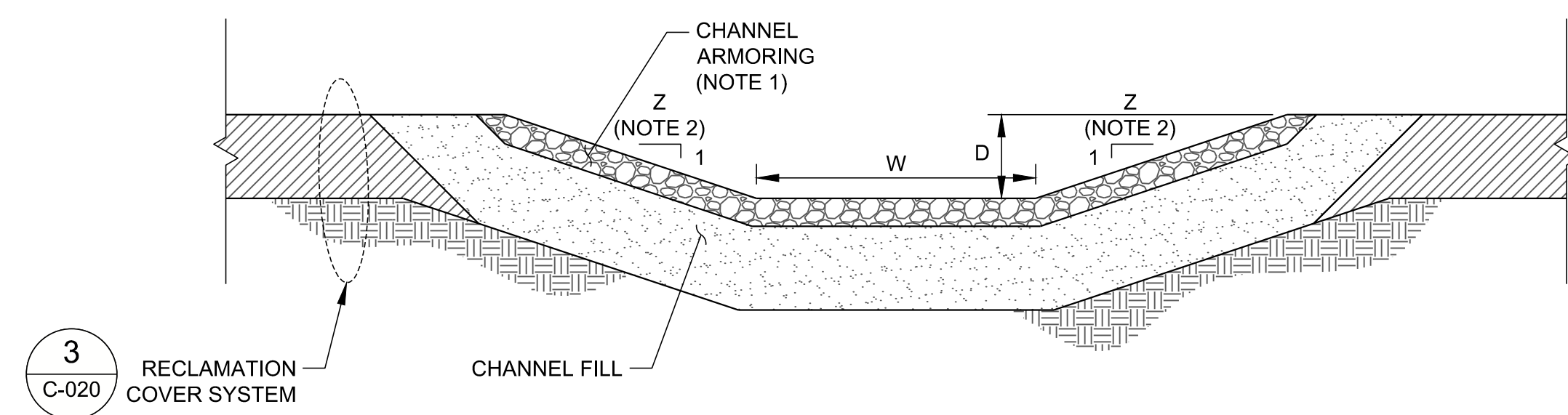
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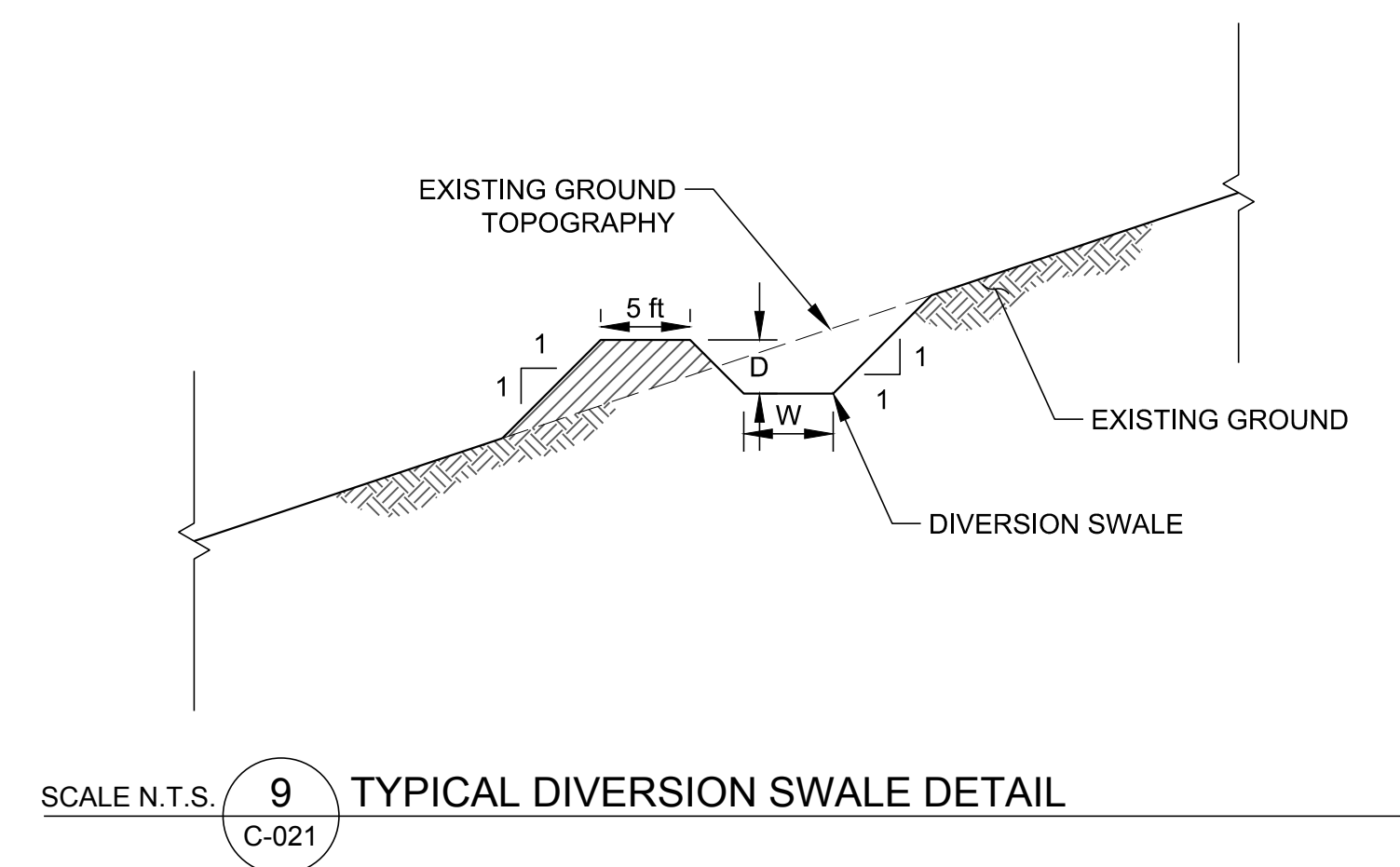
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SCALE N.T.S. **7** TYPICAL TOP SURFACE CHANNEL SECTION
C-021

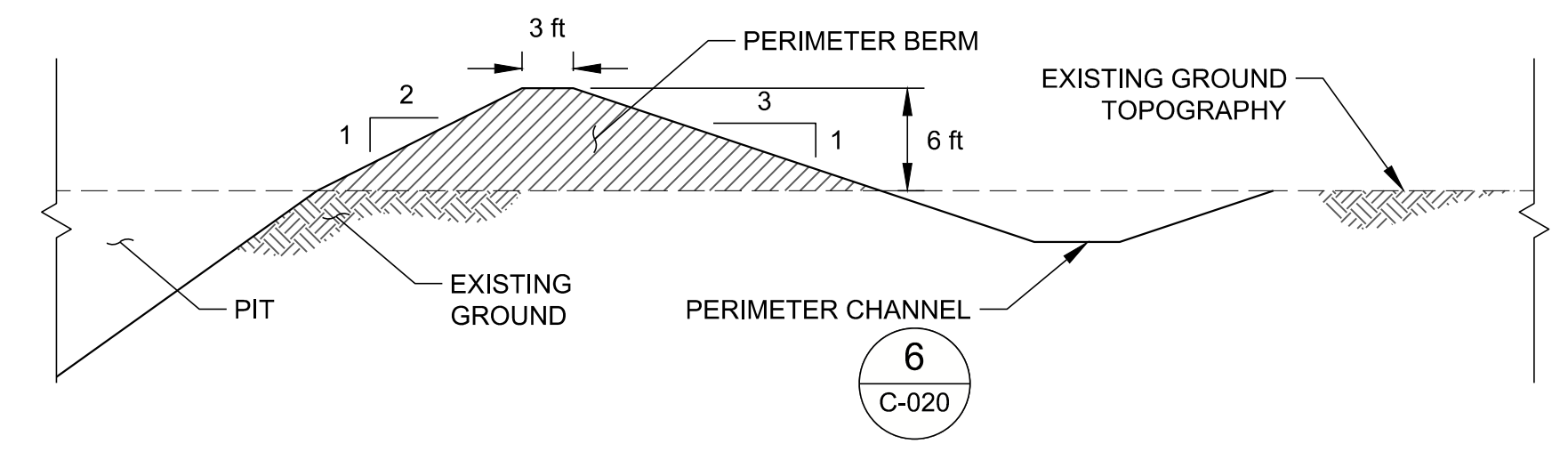


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C-021

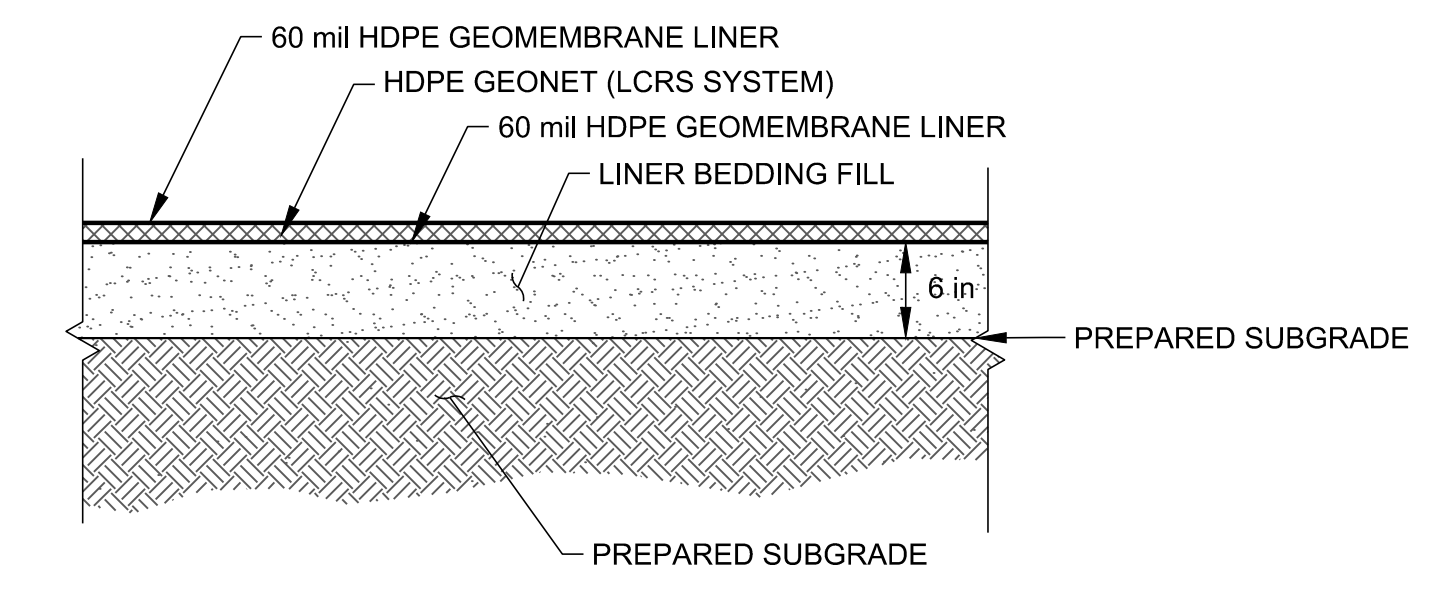


SCALE N.T.S. **9** TYPICAL DIVERSION SWALE DETAIL
C-021

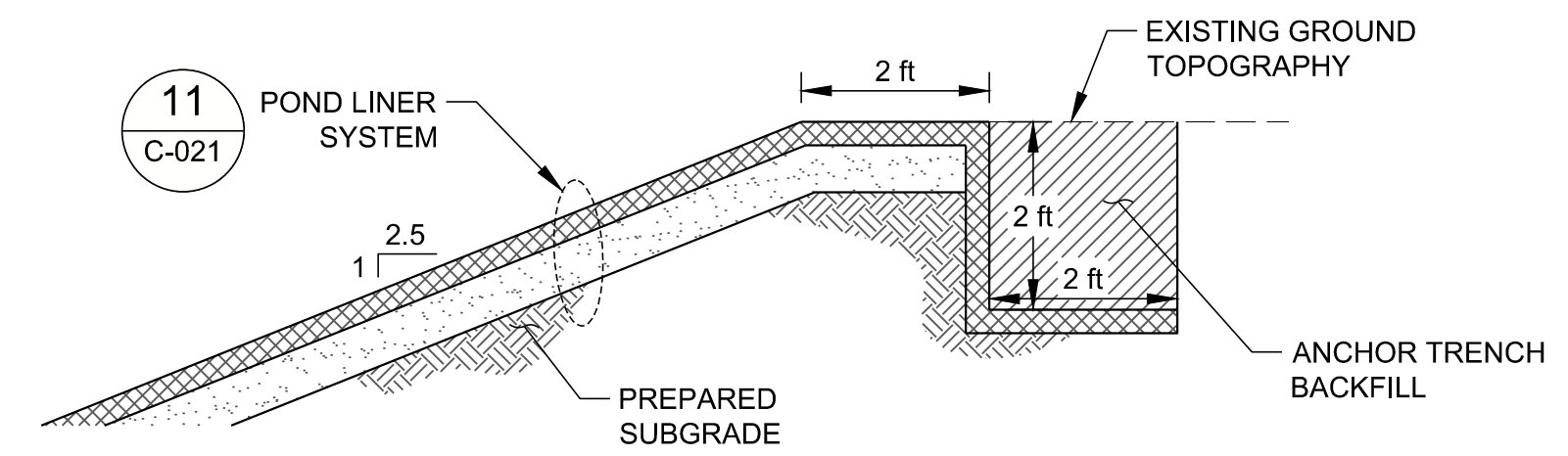
- NOTES**
- SEE CHANNEL SCHEDULE FOR DIMENSION AND ARMORING REQUIREMENTS. MINIMUM RIPRAP THICKNESS SHALL BE 2 X D_{50} . MINIMUM BEDDING THICKNESS IS 6 in.
 - DOWNSLOPE CHANNEL SIDESLOPES SHALL MAINTAIN A 3H:1V SLOPE PERPENDICULAR TO THE FLOW DIRECTION.



SCALE N.T.S. **10** PERIMETER BERM DETAIL
C-021



SCALE N.T.S. **11** EVAPORATION POND LINER SYSTEM
C-021



SCALE N.T.S. **12** EVAPORATION POND LINER SYSTEM ANCHOR TRENCH
C-021

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1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A3S-D



CALCULATIONS

Date: October 10, 2016
Project No.: 1531453
Subject: Channel Schedule
Project Short Title: COPPER FLAT MORP AND MINE PERMIT APPLICATION

Made by: HNL
Checked by: TLS
Reviewed by: TLS

CHANNEL SCHEDULE

Reach Designation ¹	Q _{design} (cfs)	Reach Design							Reports to
		Bottom Width (ft)	Bed Slope (%)	Left Side Slope (H:1V)	Right Side Slope (H:1V)	Normal Flow Depth (ft)	Average Velocity (fps)	Min. D50 (in)	
EWRS-1									
Diversion Channel, DC-1	100.0	10	0.5	3.0	3.0	2.1	3.0	3.0	Grayback Diversion
Diversion Swale, DS-1	12.0	10	0.5	3.0	3.0	0.6	1.6	3.0	Grayback Diversion
Toe Channel, TC-1	27.0	10	3.0	3.0	3.0	0.6	3.7	3.0	Grayback Diversion
Toe Channel, TC-2	17.0	10	0.5	3.0	3.0	0.8	1.8	3.0	Grayback Diversion
Haul Road Channel, HC-1	12.9	10	10.0	3.0	3.0	0.3	4.2	6.0	Grayback Diversion
EWRS-2B									
Top Surface Channel, TSC-1	81.2	10	1.0	3.0	3.0	0.3	1.0	3.0	Pit Perimeter Channel to Pit
Toe Channel, TC-3	125.6	10	2.9	3.0	3.0	1.5	3.6	12.0	Pit Perimeter Channel to Pit
Diversion Swale, DS-2	3.4	10	0.5	3.0	3.0	1.5	6.0	-	Pit Perimeter Channel to Pit
EWRS-4									
Top Surface Channel, TSC-2	86.0	10	2.5	3.0	3.0	1.2	5.1	3.0	Haul Road Channel HC-2 to Pit
Haul Road Channel, HC-2	20.0	10	6.7	3.0	3.0	0.4	4.4	3.0	Pit
Toe Channel, TC-4	13.0	10	7.4	3.0	3.0	0.3	3.9	3.0	Grayback Arroyo
WRSP-1									
Diversion Swale, DS-3	27.0	10	0.5	3.0	3.0	1.0	2.0	3.0	Off Site
Diversion Swale, DS-4	69.0	10	0.5	3.0	3.0	1.7	2.7	3.0	Natural Ground to Pit
Diversion Channel, DC-2	39.0	10	0.5	3.0	3.0	1.3	2.3	3.0	Off Site
Top Surface Channel-3	32.5	10	5.6	3.0	3.0	0.6	4.9	6.0	Pit Perimeter Channel to Pit
Bench Channels, BC-1 through BC-4	97.0	10	1.0	3.0	3.0	0.4	1.7	3.0	Pit Perimeter Channel to Pit
Haul Road Channel, HC-3	97.0	10	10.3	3.0	3.0	0.9	8.6	12.0	Pit Perimeter Channel to Pit
WRSP-2 and WRSP-3									
Diversion Swale, DS-5	112.0	10	0.5	3.0	3.0	2.2	3.1	3.0	Natural Ground to Pit
Diversion Swale, DS-6	52.0	10	0.5	3.0	3.0	1.5	2.5	3.0	Off Site
Diversion Swale, DS-7	26.0	10	0.5	3.0	3.0	1.0	2.0	3.0	Off Site
Haul Road Channel, HC-4	63.9	10	9.6	3.0	3.0	0.7	7.3	12.0	Pit
Top Surface Channel, TSC-4	42.0	10	1.0	3.0	3.0	1.1	3.0	3.0	Grayback Arroyo
Top Surface Channel, TSC-5	100.0	10	1.0	3.0	3.0	1.7	3.8	18.0	Grayback Arroyo
Downslope Channel, DSC-1	127.1	20	29.0	3.0	3.0	0.3	22.0	ACB	Grayback Arroyo
Downslope Channel, DSC-2	240.1	20	30.2	3.0	3.0	0.4	28.4	ACB	Grayback Arroyo
Toe Channel, TC-5	229.8	10	13.0	3.0	3.0	1.0	10.1	3.0	Grayback Arroyo
Toe Channel, TC-6	242.9	10	8.6	3.0	3.0	1.6	10.7	18.0	Grayback Arroyo
Bench Channels, BC-5 through BC-20	39.0	10	1.0	3.0	3.0	1.0	2.9	3.0	WRSP-2 to Pit, WRSP-3 to Grayback Arroyo
TSF									
Downslope Channel, DSC-3	181.8	20	27.6	3.0	3.0	0.4	24.9	ACB	Off Site
Downslope Channel, DSC-4	165.6	20	27.8	3.0	3.0	0.3	24.0	ACB	Grayback Arroyo
Downslope Channel, DSC-5	478.0	20	29.0	3.0	3.0	0.6	36.1	ACB	Grayback Arroyo
Top Surface Channel, TSC-6	243.0	10	0.5	3.0	3.0	2.8	3.7	3.0	DSC-5 to Grayback Arroyo
Top Surface Channel, TSC-7	236.4	10	0.5	3.0	3.0	2.8	3.7	3.0	DSC-5 to Grayback Arroyo
Bench Channels, BC-21 through BC-42	38.0	10	1.0	3.0	3.0	1.0	2.8	3.0	Off Site or Grayback Arroyo
Toe Channel, TC-7	487.7	15	0.5	3.0	3.0	4.0	4.5	3.0	Off Site
Toe Channel, TC-8	213.2	10	0.5	3.0	3.0	3.0	3.7	3.0	Grayback Arroyo
Toe Channel, TC-9	192.5	10	3.5	3.0	3.0	1.7	7.3	12.0	Grayback Arroyo
PLANT									
Perimeter Channel, PC-2	200.0	10	1.0	3.0	3.0	2.5	4.7	3.0	Grayback Arroyo
Toe Channel, TC-10	36.0	10	1.0	3.0	3.0	1.0	2.8	3.0	Pit
PIT									
Perimeter Channel, PC-1	294.0	10	2.0	3.0	3.0	2.5	6.7	3.0	Pit
Haul Road Channel, HC-5	984.4	10	10.0	3.0	3.0	1.8	36.7	ACB	Pit

Notes:

1 - See Mine Reclamation and Closure Plan drawing set for location of specific reach. Hydrology and Hydraulics calculation packet available upon request.

ACB - Articulated concrete block.

cfs - Cubic feet per second

ft - Feet

fps - Feet per second

in - Inch

Q_{design} - Design flows for channel determined from Hydrologic Modeling System (HEC-HMS) developed by the Hydrologic Engineering Center within the U.S. Army Corps of Engineers.

Min. D50 - median diameter or the medium value of the particle size distribution

Rip Rap size calculations based on the following criteria: U.S. Army Corps of Engineers (USACE, 1994) mild slope, <2% slopes; USACE steep slope, >2% to <20% slopes; Robinson method (1997), >20% to .40% slopes; ACB for all downslope channels.

**ATTACHMENT E2
TSF CLOSURE WATER MANAGEMENT PROGRAM**



DRAFT REPORT

ATTACHMENT E2

TSF POST-OPERATIONS WATER MANAGEMENT PLAN

COPPER FLAT MINE

Submitted To: Mr. Jeffrey Smith, PE
Chief Operating Officer
THEMAC Resources Group Ltd.
Copper Flat Mine Project
4253 Montgomery Blvd. NE, Suite 130
Albuquerque, NM 87109

Submitted By: Golder Associates Inc.
5200 Pasadena Avenue N.E., Suite C
Albuquerque, NM 87113 USA

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- 2 Copies – Mining and Minerals Division
- 2 Copies – New Mexico Environment Department
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October 7, 2016

1531453





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Figure E6 Passive Evaporation Water Management System Layout



1.0 INTRODUCTION

Golder Associates has been contracted by New Mexico Copper Corporation (NMCC) to prepare a Reclamation and Closure Plan for its Copper Flat project located near Hillsboro, New Mexico in Sierra County (**Figure E1**). The Reclamation and Closure Plan describes the designs and strategy for reclamation of all of the disturbed areas described in NMCC's Mine Operations and Reclamation Plan document to which this document is attached as Appendix E. As part of this Plan, Golder has developed the following plan for management of drain down and surface runoff waters from the Tailings Storage Facility (TSF) following cessation of operations. The following sections detail the sources of water to be managed, the performance objectives of the water management plan, and the associated plan for management of these waters, including designs for both active and passive evaporation water management systems.

1.1 Sources of Water to be Managed

The flow components that will need to be managed from the TSF at the end of operations include drainage water collected from the impoundment and dam underdrain collection systems, and surface water runoff from the embankment. As described in Section 6.5.2 of the TSF Feasibility Level Design Report (FLDR) [Golder, 2015], the maximum underdrain flow rate from the TSF at final buildout will be approximately 66 gallons per minute (gpm) from the impoundment underdrain and approximately 448 gpm from the dam underdrain. Surface water runoff estimates from the TSF are provided in the water balance calculations included in Appendix G of the FLDR. The estimated surface water runoff from the embankment ranges between 1.4 gpm in April and 86 gpm in July. These three flow components represent the maximum flows estimated to be managed. The flows will be reduced over time as drain down from the TSF decreases and the TSF gets reclaimed and covered. Following cover placement on the TSF embankment, surface water runoff will be routed to the nearest natural drainage.

1.2 Performance Objectives

The performance objective for the water management program is to collect and eliminate, using evaporation techniques, drain down waters and surface water runoff from uncovered slopes associated with the TSF following the cessation of mining and processing operations at Copper Flat. To meet the performance objectives the following strategy will be utilized:

- A short-term active evaporative water management system (AEWMS) will be utilized to capture and evaporate all tailing drain down waters and surface water runoff from the TSF embankment out slopes for the first five years following closure, or until flows reduce to a point to where they can be managed exclusively through passive evaporation.
- A long-term passive evaporative water management system (PEWMS) will be utilized to evaporate the residual tailing drain down waters beginning in year six and continuing for an estimated 20 years thereafter.



2.0 BACKGROUND

The purpose of the TSF water management program is to provide an environmentally sound and cost effective approach to manage, reduce, and eventually eliminate drain down waters and surface water runoff from uncovered slopes associated with the TSF following cessation of mining and processing operations at Copper Flat. The water management program assumes that tailings delivery to the TSF will terminate following the cessation of processing operations at Copper Flat.

2.1 TSF Facility

Golder completed a feasibility level design of the TSF in 2015 (Golder, 2015). The proposed TSF will include an HDPE-lined tailings impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam, and a water reclaim or recycle system to maximize water reuse. The TSF and associated ancillary facilities will cover approximately 564 acres in size at full capacity (**Figure E2**) and will be constructed using the centerline construction method. The TSF design will comply with the design and dam-safety guidelines of the New Mexico Office of the State Engineer Dam Safety Bureau. The tailings impoundment is designed to store 113 million tons of tailings produced over approximately 11 years, with tailings deposition occurring at an average rate of approximately 27,900 tons per day over the life of the mine.

The underdrain collection pond will be constructed along the southeastern corner of the TSF to contain drainage water from the TSF impoundment and dam underdrains, as well as runoff from the downstream face of the tailings dam. The pond is designed for full TSF operation, including dam construction. Flow to the underdrain collection pond will begin to decrease immediately once dam construction stops. The pond is sized to contain 24 hours of tailing drainage flow at maximum estimated drainage rates, runoff from the 100-year, 24-hour storm event of 3.73 inches (National Oceanic and Atmospheric Administration, 2006) incident on the downstream dam face, and an additional minimum 2-feet of freeboard. This pond will be utilized after mine operations cease as part of the TSF water management program.

At reclamation, the TSF outslope will, first, be graded down to interbench slopes of between 3.0H:1V and 3.5H:1V with 25 foot wide terrace benches on the outslope at maximum slope lengths of between 200 feet (3H:1V) and 250 feet (3.5:1V). Reclamation of the TSF includes grading of the TSF top surface to a final grade of between 1 and 5% to direct storm water to the back side of the TSF and out to Grayback Arroyo after cover placement. The top surfaces and outslopes of the TSF will be covered with 36 inches of growth media and seeded to reestablish vegetation. Surface water conveyance channels will be constructed on the top surface of the TSF and terrace benches to direct surface water off the covered TSF surfaces and ultimately to Grayback Arroyo and other natural drainages to the exterior of the mine.



2.2 Climate

The Copper Flat property is located within an arid, high desert area in the Basin and Range physiographic province subject to hot summers and relatively mild winters. Maximum summer temperatures can exceed 100 degrees Fahrenheit (°F) while the average maximum daily temperature during winter months is approximately 60 °F. Average annual rainfall is approximately 12.4 inches and the property receives snow periodically (**Table E1**).

Most rainfall occurs in July through September and is associated with high intensity, short duration, convective storms and moisture from the Gulf of Mexico. Winter precipitation is associated with west to east moving Pacific frontal storms. These storms typically produce less intense precipitation over a longer duration.

Evaporative demand in this region is high and annual evaporation far exceeds annual precipitation. Annual pan evaporation measured at Caballo Dam is approximately 107 inches, and ranges between approximately 3.5 inches in December to 14.8 inches in June (**Table E2**). Equivalent free water evaporation rates for the Copper Flat site are calculated based on these data and an applied pan coefficient of 0.7. Annual equivalent free water evaporation is estimated at approximately 74.9 inches, and ranges between approximately 2.4 inches in December to 10.4 inches in June (**Table E2**). The estimated amount of excess water evaporated from surface water bodies at the Copper Flat site is calculated as equivalent free water evaporation minus precipitation. The estimated annual excess evaporation for surface water bodies at Copper Flat is approximately 62.5 inches, and ranges between 1.6 inches in December to 9.7 inches in June (**Table E3**). Annual excess evaporation is equivalent to passive evaporation within the context of this plan.



3.0 TSF POST-OPERATIONS WATER MANAGEMENT PLAN

The FLDR for the TSF indicates that the maximum drain down flow rate will be approximately 448 gpm from the dam underdrain and 66 gpm from the impoundment underdrain. Due to the coarser nature of the tailings sands used to construct the TSF dam and the higher permeability of these materials, the TSF embankment will drain quickly in comparison to the impoundment. It is estimated that it will take approximately 2 to 3 years for the TSF embankment to drain sufficiently to begin reclamation. It is also anticipated that some reclamation of the impoundment can begin within 5 years of cessation of operations as the impoundment continues to drain and dry, allowing construction equipment to be utilized to commence cover placement.

The underdrain systems will continue to operate after cessation of mining and processing as drain-down of the TSF will continue to produce water for a number of years thereafter. This Plan assumes that drain-down will continue for 25 years. The actual amount of time required to do so is a function of porosity of tailings materials in the long-term and the volume of water remaining in the TSF. An “active” water management program (short-term AEWMS) will be implemented at the end of operations. Water captured in the TSF underdrain collection pond will be pumped back to the impoundment surface of the TSF. A forced or enhanced evaporation system will be installed within the impoundment to dispose of the water. The duration of operation of the “active” water management system will be determined by the volume of water that continues to drain from the impoundment. However, for planning purposes it has been assumed that there will be 5 years of operation of the active program followed by 20 years of “passive” drain-down water management. During the active evaporation phase, reclamation of the TSF will continue such that grading will have been completed and the cover will have been placed on the TSF. Toward the end of the active evaporation phase, an evaporation pond will be constructed that will incorporate the underdrain collection pond. Once complete, the “passive” evaporation phase (PEWMS) will be initiated.

After decommissioning of the active program and full reclamation of the TSF, water that may continue to drain from the TSF will be captured and evaporated in the new evaporation pond (**Figure E3**). Details of the short-term AEWMS and the PEWMS are provided in the following sections.

3.1 Short-Term Active Evaporative Water Management System (AEWMS)

The short-term AEWMS consists of forced evaporation and wetted surface evaporation of water collected from the TSF underdrain collection system and surface water runoff from the TSF embankment. The collected waters will be pumped to the top of the TSF into a designated ponding/spray area and actively and passively evaporated (**Figure E3**). The “active” or forced evaporation system consists of a 15 acre ponding/spray area to maximize the evaporation rate of the waters. Forced evaporation will be accomplished with a mechanical spray system designed to handle flows up to 380 gpm per unit. Spray evaporation estimates are based on a daily evaporation chart for Model 1210 evaporator systems provided by Minetek (2012). The typical spray pattern for these systems covers an area approximately 500 feet long



by 300 feet wide. A 4 unit spray system with a capacity to handle up to 1,520 gpm will be installed. Passive evaporation of the TSF drain down waters will also continue to occur at the same time within the TSF impoundment surface and on the ponded surface of the TSF underdrain collection pond.

3.1.1 AEWMS Estimates

Based on the daily evaporation chart provided by Minetek (2012), it is estimated that the four unit spray system will be capable of evaporating between approximately 520 gpm (December) and 1,018 gpm (June), with an average evaporation rate of approximately 789 gpm (**Table E4**). The residual water in the ponded area will either evaporate through surface evaporation or will drain into the underlying tailing material and back to the TSF underdrain collection system. The estimated amount of passive evaporation of the waters within the TSF underdrain collection pond ranges between approximately 4 gpm in December to approximately 25 gpm in June. The estimated monthly and annual evaporation potential of the short-term AEWMS is summarized in **Table E4**.

The short-term AEWMS will continue to operate until drain down reduces to a point to where the flows can be handled through passive evaporation within the evaporation pond discussed later, herein. Underdrain flows will diminish over time once tailing discharge to the top of the TSF is discontinued. Due to the high permeability of the tailings sands used to construct the TSF embankment, drain down of the TSF embankment will occur more rapidly than within TSF impoundment area, where the permeability of the slimes fraction of the tailings is much lower. Thus, the largest flow component (448 gpm) will drop off more rapidly than the flow component from the impoundment.

A conceptual drain down curve was developed for the Copper Flat TSF based on evaluation of measured drain down flow rates from four closed tailing storage facilities in Nevada and a conceptual tailings storage facility drain down curve developed by the Nevada Department of Environmental Protection (2011). These drain down curves are presented in terms of relative flows (drain down flow relative to the maximum flow at the beginning of closure) in **Figure E4**. The conceptual Copper Flat TSF drain down curve developed for the purpose of this water management plan is also presented in **Figure E4**. **Figure E5** presents the conceptual Copper Flat TSF drain down curve in terms of estimated flows in gpm over time following cessation of mining and processing operations at Copper Flat.

It is anticipated that the TSF embankment will be regraded, covered and revegetated within three years following cessation of operations. Once the embankment area has been reclaimed, surface water runoff from the outslope of the embankment will no longer need to be captured and will be directed to the nearest drainage. However, for water management planning purposes, the surface water runoff component is included throughout the 5 year AEWMS operational period.



3.1.1 AEWMS Water Balance

The maximum flows that will be encountered during the short-term AEWMS will occur during the first year of operation as the higher permeability sands within the TSF embankment drain. As shown on **Table E5**, the four unit spray system will provide excess evaporation potential under these maximum flow conditions. It is anticipated that the number of spray units and/or their hours of operation will be adjusted throughout the AEWMS program based on the actual flows encountered during the initial months of operation and as the TSF drain down flows decline. The AEWMS will be capable of providing even greater excess evaporation potential in the remaining four years of AEWMS operation as the TSF drain down flows decline over time. Although it is estimated that the AEWMS will operate for a period of 5 years, the duration of operation will be determined when TSF drain down reduces to a point to where the flows can be handled through passive evaporation within the evaporation pond. As described in Section 3.2, the maximum flow that can be handled with the PEWMS is approximately 70 gpm.

3.2 Long-Term Passive Evaporative Water Management System (PEWMS)

A long-term PEWMS will be utilized to evaporate residual tailing drain down waters after operation of the AEWMS is no longer required. Prior to the start of the PEWMS, a new HDPE-lined evaporation pond will be constructed to provide sufficient surface area to passively evaporate the residual drain down waters from the TSF. The following sections provide details of the evaporation pond construction and the evaporation schedule associated with the PEWMS.

3.2.1 Evaporation Pond Construction

The location of the evaporation pond is shown on **Figure E6**. The existing TSF underdrain collection pond will be incorporated into the new evaporation pond by removing the existing collection pond embankment and fusing new HDPE liner onto the remaining collection pond liner. The evaporation pond will essentially be an extension of the existing TSF underdrain collection pond and will incorporate all the design features of the underdrain collection pond. The evaporation pond will be double-lined with an 80-mil HDPE geomembrane top liner and a 60-mil HDPE geomembrane bottom liner. An HDPE geonet will be placed between the liners to serve as the evaporation pond leakage collection and recovery system (LCRS) and minimize the head on the lower pond liner. The existing TSF underdrain collection pond and LCRS pump will be utilized to recover any leakage through the upper geomembrane.

Drain down water from the TSF impoundment and dam will be conveyed to the evaporation pond through the existing underdrain collection system. The pond will cover an area of 22.3 acres and is sized to contain one foot of TSF drain down water for passive evaporation, and an additional minimum 2-feet of freeboard. At a stage of one foot, the surface area of the pond will cover 21.6 acres. The pond capacity is approximately 21.93 million gallons with 2 feet of dry freeboard below the crest of the pond (top of pond liner).



3.2.2 PEWMS Estimates

TSF drain down water will be allowed to fill to a maximum depth of one foot within the pond and will be maintained at this depth throughout the PEWMS operation for passive evaporation. At a depth of one foot, the surface area of the pond will cover 21.6 acres. Based on the surface water evaporation estimates provided in **Table E3**, it is estimated that the 21.6 acre water body will evaporate an average of approximately 135 acre-feet/year (average 84 gpm annually), and incident precipitation will amount to approximately 22 acre-feet/year (average 14 gpm annually). Excess evaporation (free water evaporation minus incident precipitation) from the 21.6 acre water body is estimated at approximately 113 acre-feet/year (average 70 gpm annually). This excess evaporation is referred to herein as passive evaporation.

3.2.3 PEWMS Water Balance

The TSF drain down estimates over the 25 year TSF water management program are summarized in **Table E6** along with the PEWMS water balance over the 20 year operational period. The maximum TSF drain down flow during the PEWMS operation is estimated at 67 gpm immediately following the AEWMS operation and will reduce to rates below 5 gpm after 16 years of PEWMS operation. The TSF water management plan assumes 20 years of PEWMS operation. The duration of the PEWMS operation will be dependent on when the volume of drain down from the TSF is reduced to a point to where the evaporation pond is no longer required. This point in time will be determined in collaboration with the Agencies and the evaporation pond will be reclaimed thereafter in accordance with the Reclamation and Closure Plan.



4.0 SUMMARY

The TSF closure water management plan includes both a short-term AEWMS and a long-term PEWMS to manage drain down waters and surface water runoff from the TSF following cessation of mining operations at Copper Flat. The TSF underdrain systems will continue to operate after cessation of operations, and the AEWMS program will commence thereafter. The AEWMS will include pumping captured water from the TSF underdrain collection pond to the impoundment surface of the TSF and use of forced or enhanced evaporation equipment to minimize or eliminate the water. The AEWMS will utilize a forced evaporation system at a dedicated spray area on top of the TSF. Forced evaporation of these waters will be accomplished with a 4 unit mechanical spray system with a capacity to handle up to 1,520 gpm. Additional passive evaporation of the TSF drain down waters will also occur on the ponded surface of the TSF underdrain collection pond.

A conceptual drain down curve was developed for the Copper Flat TSF based on evaluation of measured drain down flow rates from four closed tailing storage facilities in Nevada and a conceptual tailings storage facility drain down curve developed by the Nevada Department of Environmental Protection (2011). Based on this conceptual drain down curve and estimated surface water runoff from the TSF embankment provided in the FLDR (Golder, 2015) it is estimated that the AEWMS will operate for approximately 5 years. Although it is estimated that the AEWMS will operate this long, the duration of operation will be determined when the volume of TSF drain down water reduces to a point to where the flows can be handled through passive evaporation within a constructed evaporation pond. The maximum flow that can be handled with the PEWMS is approximately 70 gpm.

After decommissioning of the active program and reclamation of the TSF, water that may continue to drain from the TSF will be captured in a new 22.3 acre evaporation pond that will be constructed below the toe of the TSF within the mine permit area coincident with the underdrain pond. TSF drain down water will be allowed to fill to a maximum depth of one foot within the pond and will be maintained at this depth throughout the PEWMS program to be passively evaporated. At a depth of one foot, the surface area of the pond will cover 21.6 acres. Approximately 113 acre-feet/year (average 70 gpm annually) will passively evaporate from a 21.6 acre surface water body at Copper Flat. The maximum TSF drain down flow during the PEWMS operation is estimated at 67 gpm immediately following the AEWMS operation and will reduce to rates below 5 gpm over time. While the TSF water management plan assumes 20 years of PEWMS operation, the actual duration of the PEWMS operation will continue until the volume of drain down water from the TSF reduces to the point where the evaporation pond is no longer required. This point in time will be determined in collaboration with the Agencies and the evaporation pond will be reclaimed thereafter.



5.0 REFERENCES

- Golder. 2015. Feasibility Level Design, 30,000 TPD Tailings Storage Facility, Copper Flat Project, Sierra County, New Mexico. Submitted to New Mexico Copper Corporation THEMAC Resources Group Ltd. November 30, 2015.
- Minetek. 2012. Daily Evaporation Chart for Model 1210 Evaporator Systems for Chino, New Mexico Climate provided by Duane Thompson of Minetek to Todd Stein of Golder. June 28, 2012.
- Nevada Department of Environmental Protection. 2011. Tailings Facility Process Fluid Stabilization (PFS). May 18, 2011. https://ndep.nv.gov/bmrr/file/ifm_pfs_definitions.pdf
- New Mexico Office of the State Engineer Dam Safety Bureau. 2010. Rules and Regulations Governing Dam Design, Construction and Dam Safety. December 31, 2010.
- National Oceanic and Atmospheric Administration. 2006. Precipitation-Frequency Atlas of the United States, Atlas 14, Volume 1, Version 4.0, Semiarid Southwest (Arizona, Southeast California, Nevada, New Mexico, and Utah. G. M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley. National Weather Service, Silver Spring, Maryland. 2004, revised October 4, 2006.

TABLES

**Table E1. Average Monthly Temperatures and Precipitation Near the Copper Flat Project Area⁽¹⁾**

Month	Average Temperature		Average Precipitation
	° F		Inches
	Min	Max	Total
January	25.0	55.9	0.57
February	28.1	59.8	0.56
March	33.5	66.4	0.38
April	39.7	74.4	0.32
May	47.4	82.5	0.52
June	56.1	91.7	0.71
July	61.3	91.2	2.30
August	59.6	88.5	2.42
September	52.8	83.4	2.08
October	42.0	73.8	1.17
November	31.3	62.8	0.55
December	24.9	54.3	0.83
Annual	41.8	73.6	12.40

Notes:

⁽¹⁾ Data from the Western Regional Climate Center covering the period between 1893 and 2016 from Hillsboro, NM Climate Station #294009.

**Table E2. Average Monthly Pan Evaporation and Free Water Evaporation Near the Copper Flat Project Area**

Period	Average Pan Evaporation ⁽¹⁾	Estimated Average Free Water Evaporation ⁽²⁾
	inches	inches
	Total	Total
January	4.42	3.09
February	5.10	3.57
March	8.56	5.99
April	11.37	7.96
May	13.59	9.51
June	14.80	10.36
July	13.08	9.16
August	11.35	7.95
September	9.26	6.48
October	7.27	5.09
November	4.78	3.35
December	3.48	2.44
Annual	107.06	74.94

Notes:

- (1) Pan evaporation data from the Western Regional Climate Center for Caballo Dam covering the period between 1938 and 2005.
- (2) Estimated free water evaporation based on applying a pan coefficient of 0.7 to the measured pan evaporation values.

**Table E3. Estimated Amount of Excess Water Evaporated from Surface Water Bodies at the Copper Flat Site**

Month	Estimated Average Free Water Evaporation ⁽¹⁾	Average Precipitation ⁽²⁾	Excess Evaporation ⁽³⁾
	Inches	Inches	Inches
	Total	Total	Total
January	3.09	0.57	2.52
February	3.57	0.56	3.01
March	5.99	0.38	5.61
April	7.96	0.32	7.64
May	9.51	0.52	8.99
June	10.36	0.71	9.65
July	9.16	2.30	6.86
August	7.95	2.42	5.53
September	6.48	2.08	4.40
October	5.09	1.17	3.92
November	3.35	0.55	2.80
December	2.44	0.83	1.61
Annual	74.94	12.40	62.53

Notes:

- (1) Estimated free water evaporation based on applying a pan coefficient of 0.7 to the measured pan evaporation values. Pan evaporation data from the Western Regional Climate Center for Caballo Dam covering the period between 1938 and 2005.
- (2) Average monthly precipitation from the Western Regional Climate Center covering the period between 1893 and 2016 from Hillsboro, NM Climate Station #294009.
- (3) Excess evaporation equals estimated free water evaporation minus precipitation.

**Table E4. AEWMS Evaporation Potential at the Copper Flat Site**

Month	Estimated Passive Evaporation ⁽¹⁾		Estimated Active Evaporation ⁽²⁾		Total AEWMS Evaporation Potential ⁽³⁾	
	gallons	gpm	gallons	gpm	gallons	gpm
January	278,947	6.2	24,302,594	544	24,581,542	551
February	332,659	8.3	26,454,042	656	26,786,701	664
March	620,227	13.9	35,638,915	798	36,259,142	812
April	844,247	19.5	39,994,440	926	40,838,687	945
May	993,888	22.3	44,760,458	1,003	45,754,346	1,025
June	1,066,498	24.7	43,966,426	1,018	45,032,925	1,042
July	757,711	17.0	43,450,773	973	44,208,484	990
August	610,612	13.7	39,389,900	882	40,000,511	896
September	486,500	11.3	33,221,181	769	33,707,681	780
October	433,120	9.7	29,292,662	656	29,725,782	666
November	309,008	7.2	24,491,661	567	24,800,669	574
December	177,492	4.0	23,197,969	520	23,375,461	524
Annual	6,910,908	---	408,161,022	---	415,071,930	---

Notes:

- (1) Estimated passive evaporation from the TSF underdrain collection pond equals free water evaporation (surface area of 4.07 acres @ 2' of freeboard) minus incident precipitation.
- (2) Estimated active evaporation based on a daily evaporation chart for Model 1210 evaporator systems provided by Duane Thompson of Minetek on June 28, 2012. Four unit system assumed for AEWMS
- (3) Total AEWMS evaporation potential equals estimated passive evaporation plus estimated active evaporation.

**Table E5. Estimated AEWMS Program Water Balance for the First Year Following Closure**

Month	Surface Water Runoff ⁽¹⁾	TSF Drain Down ⁽¹⁾	Total Inflows to AEWMS ⁽³⁾	Total AEWMS Evaporation Potential ⁽⁴⁾	Excess Evaporation Potential ⁽⁵⁾
	gpm	gpm	gpm	gpm	gpm
January	7.3	514	521	551	29
February	7.7	500	508	664	157
March	2.3	486	488	812	324
April	1.3	472	473	945	472
May	5.8	458	464	1,025	561
June	12.4	444	456	1,042	586
July	88.3	430	518	990	472
August	46.8	416	463	896	433
September	35.2	402	437	780	343
October	7.6	388	395	666	270
November	6.9	374	381	574	193
December	16.5	360	376	524	147

Notes:

- (1) Surface water runoff from the TSF embankment prior to regrading and cover placement. Runoff estimates from the Feasibility Level Design, 30,000 TPD Tailings Storage Facility, Copper Flat Project, Sierra County, New Mexico (Golder, 2015).
- (2) Estimated TSF drain down from the conceptual Copper Flat TSF drain down curve developed herein.
- (3) Total inflows to the AEWMS equals surface water runoff plus TSF drain down.
- (4) Total AEWMS evaporation potential equals estimated passive evaporation plus estimated active evaporation.
- (5) Excess evaporation potential equals total AEWMS evaporation potential minus total inflows to the AEWMS.

**Table E6. Estimated PEWMS Program Water Balance Over 20 Year Operating Period**

Year ⁽¹⁾	TSF Drain Down ⁽²⁾	Passive Evaporation ⁽³⁾	Excess Evaporation Potential ⁽⁴⁾
	gpm	gpm	gpm
<i>Short-Term Active Evaporative Water Management System Operation</i>			
1	514	NA	NA
2	360	NA	NA
3	252	NA	NA
4	170	NA	NA
5	113	NA	NA
<i>Long-Term Passive Evaporative Water Management System Operation</i>			
6	67	70	3
7	51	70	18
8	44	70	26
9	39	70	31
10	33	70	36
11	28	70	42
12	23	70	47
13	21	70	49
14	18	70	52
15	12.9	70	57
16	11.3	70	58
17	10.3	70	60
18	9.8	70	60
19	9.3	70	61
20	7.2	70	63
21	5.1	70	65
22	4.1	70	66
23	3.9	70	66
24	3.8	70	66
25	3.6	70	66

Notes:

NA – Not applicable

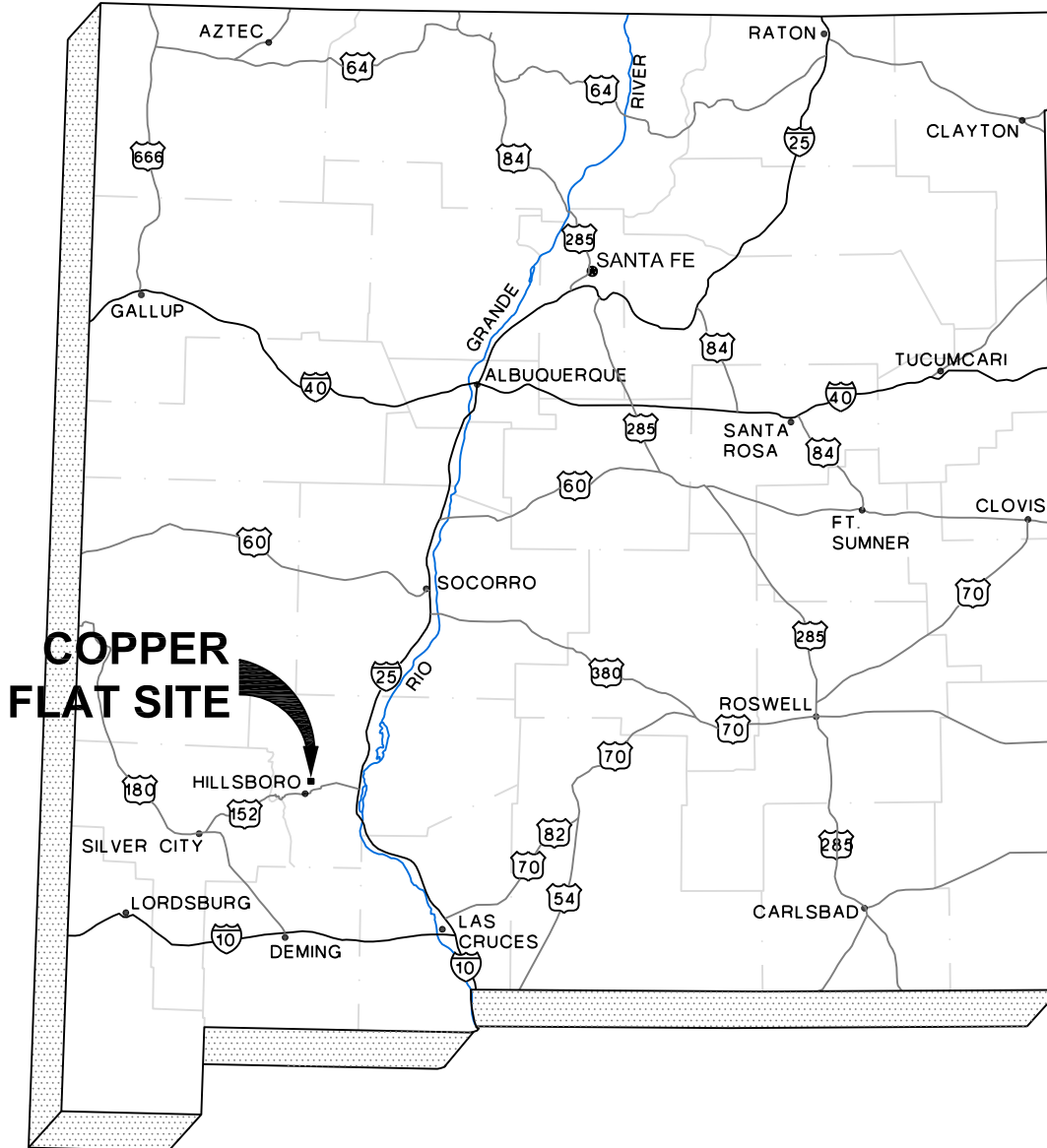
⁽¹⁾ Closure year. Cessation of operations = Year 1.⁽²⁾ Maximum estimated drain down flows for the year based on conceptual Copper Flat TSF drain down curve.⁽³⁾ Estimated passive evaporation from the 21.6 acre ponded surface within the evaporation pond.⁽⁴⁾ Excess evaporation potential from the 21.6 acre surface within the evaporation pond.

FIGURES



STATE OF NEW MEXICO

NOT TO SCALE



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NEW MEXICO COPPER CORPORATION

PROJECT

COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

CONSULTANT



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DESIGN	TS
REVIEW	TS
APPROVED	TS

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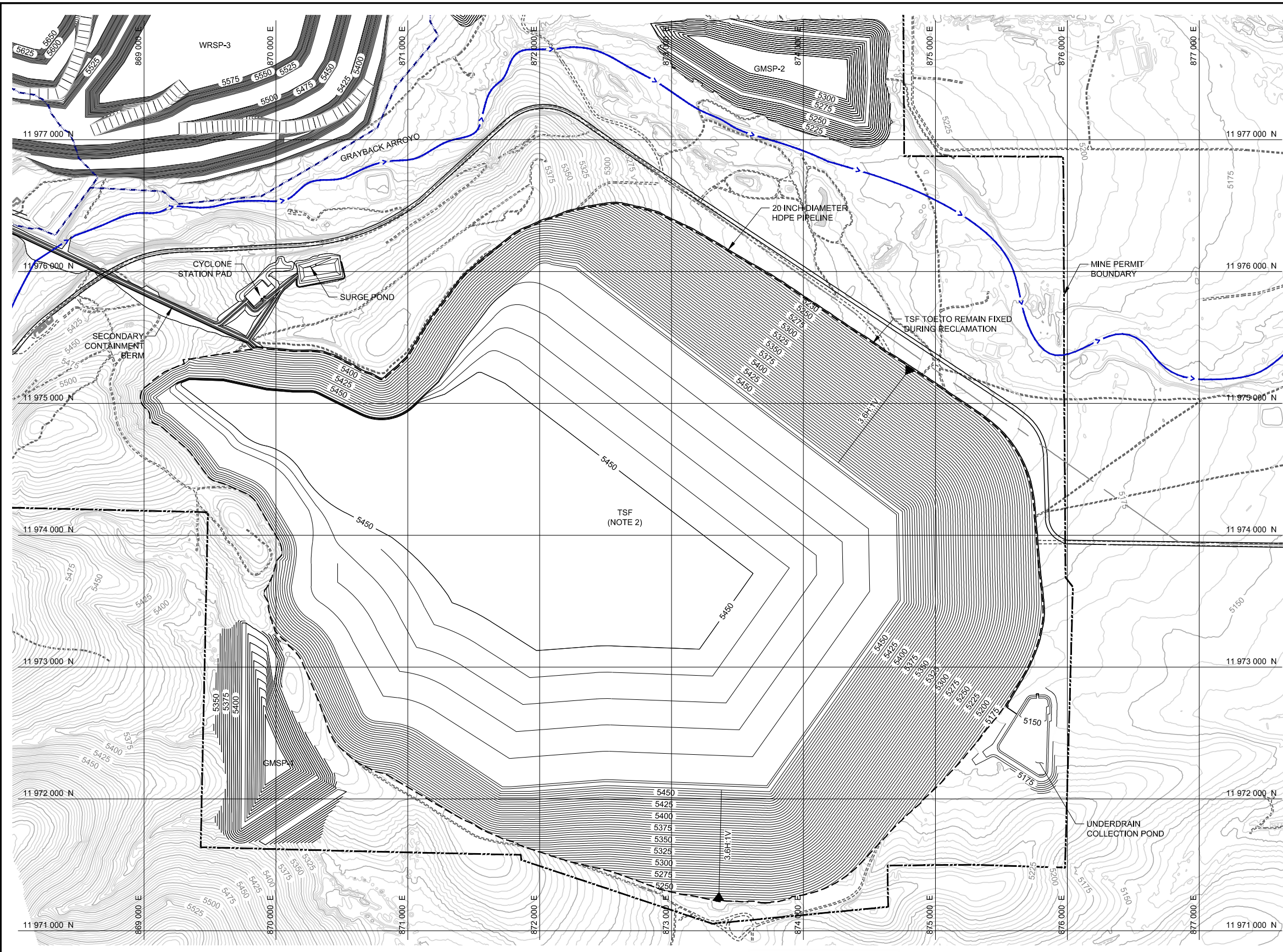
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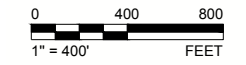
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- 5400 PROPOSED DESIGN CONTOURS (ft-MSL) (NOTES 1 AND 2)
- MINE PERMIT BOUNDARY
- WATERSHED BOUNDARY (BY OTHERS)
- EXISTING DIVERSION OR ARROYO
- EXISTING ROAD
- SLOPE INDICATOR
- 1% GRADE INDICATOR
- WRSP WASTE ROCK STOCKPILE
- GMSP GROWTH MEDIA STOCKPILE
- TSF TAILING STORAGE FACILITY
- AEWMS ACTIVE EVAPORATION WATER MANAGEMENT SYSTEM

NOTES

- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
- FINAL BUILDOUT TOPOGRAPHY FOR THE TSF WAS DEVELOPED BY GOLDER AND IS FROM THE 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN REPORT (GOLDER, 2015).

PRELIMINARY FOR AGENCY REVIEW



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REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-09-10	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	TS
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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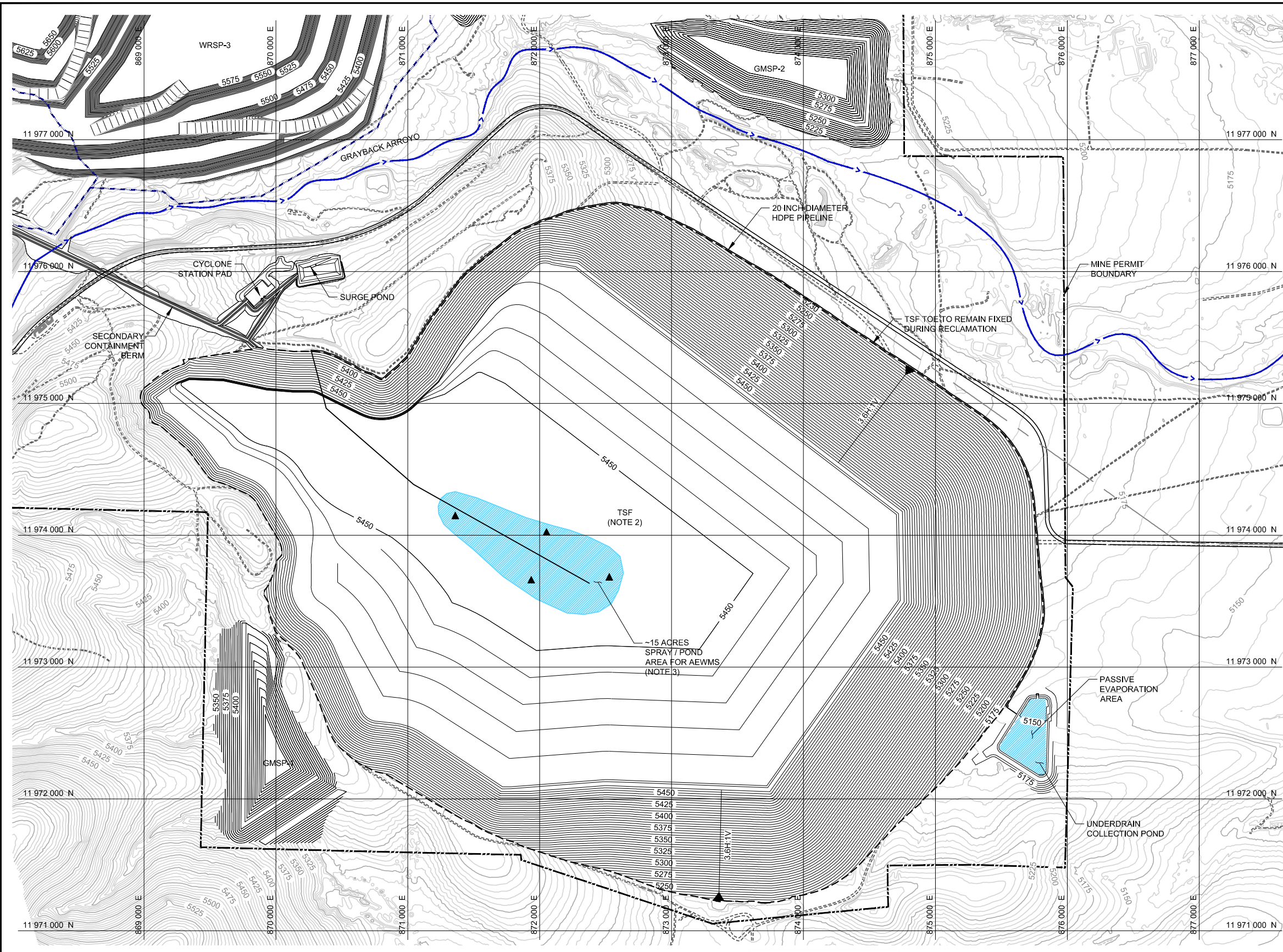
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PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
GENERAL ARRANGEMENT TSF AREA AT FINAL BUILT OUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 15 of 25 FIGURE E2

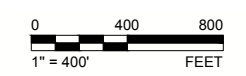
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 - MINE PERMIT BOUNDARY
 - WATERSHED BOUNDARY (BY OTHERS)
 - EXISTING DIVERSION OR ARROYO
 - EXISTING ROAD
 - SLOPE INDICATOR
 - GRADE INDICATOR
 - WASTE ROCK STOCKPILE
 - GROWTH MEDIA STOCKPILE
 - TAILING STORAGE FACILITY
 - ACTIVE EVAPORATION WATER MANAGEMENT SYSTEM
 - MECHANICAL SPRAY SYSTEM

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 2. FINAL BUILDOUT TOPOGRAPHY FOR THE TSF WAS DEVELOPED BY GOLDER AND IS FROM THE 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN REPORT (GOLDER, 2015).
 3. APPROXIMATE LOCATION OF SPRAY/POND. SIZE AND LOCATION MAY BE ADJUSTED TO MAXIMIZE EVAPORATION CAPACITY BASE. ON ACTUAL FLOWS TO BE MANAGED.

PRELIMINARY FOR AGENCY REVIEW



REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-09-10	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	TS
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

CLIENT

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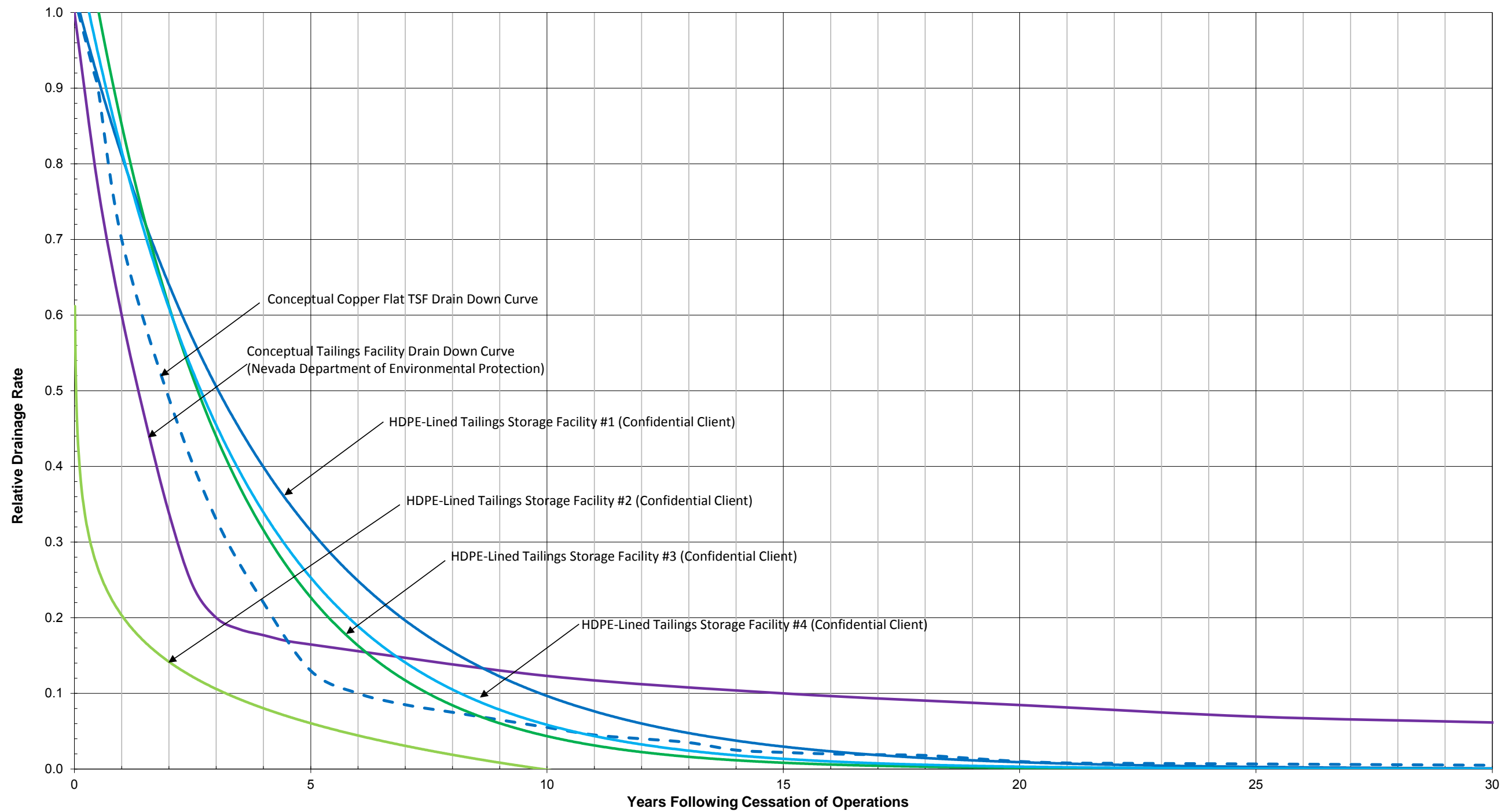
PROJECT
COPPER FLAT PROJECT
MINE RECLAMATION AND CLOSURE PLAN

TITLE
ACTIVE EVAPORATION WATER MANAGEMENT SYSTEM LAYOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 15 of 25 FIGURE E3

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1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI D



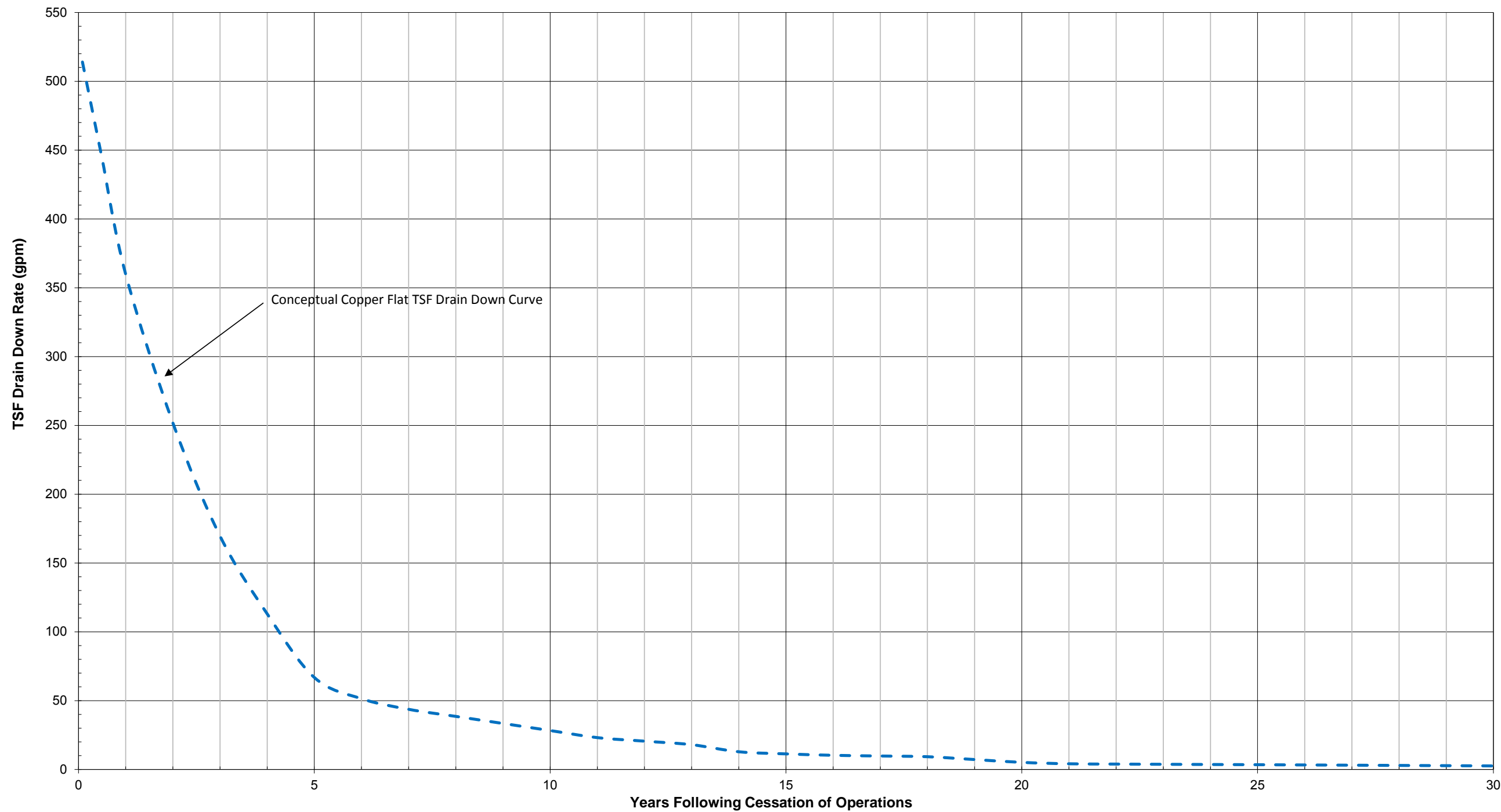
TITLE
Example Tailings Storage Facility Drain Down Curves

CLIENT/PROJ
**THEMAC Resources Group Ltd.
 Copper Flat Mine Project**

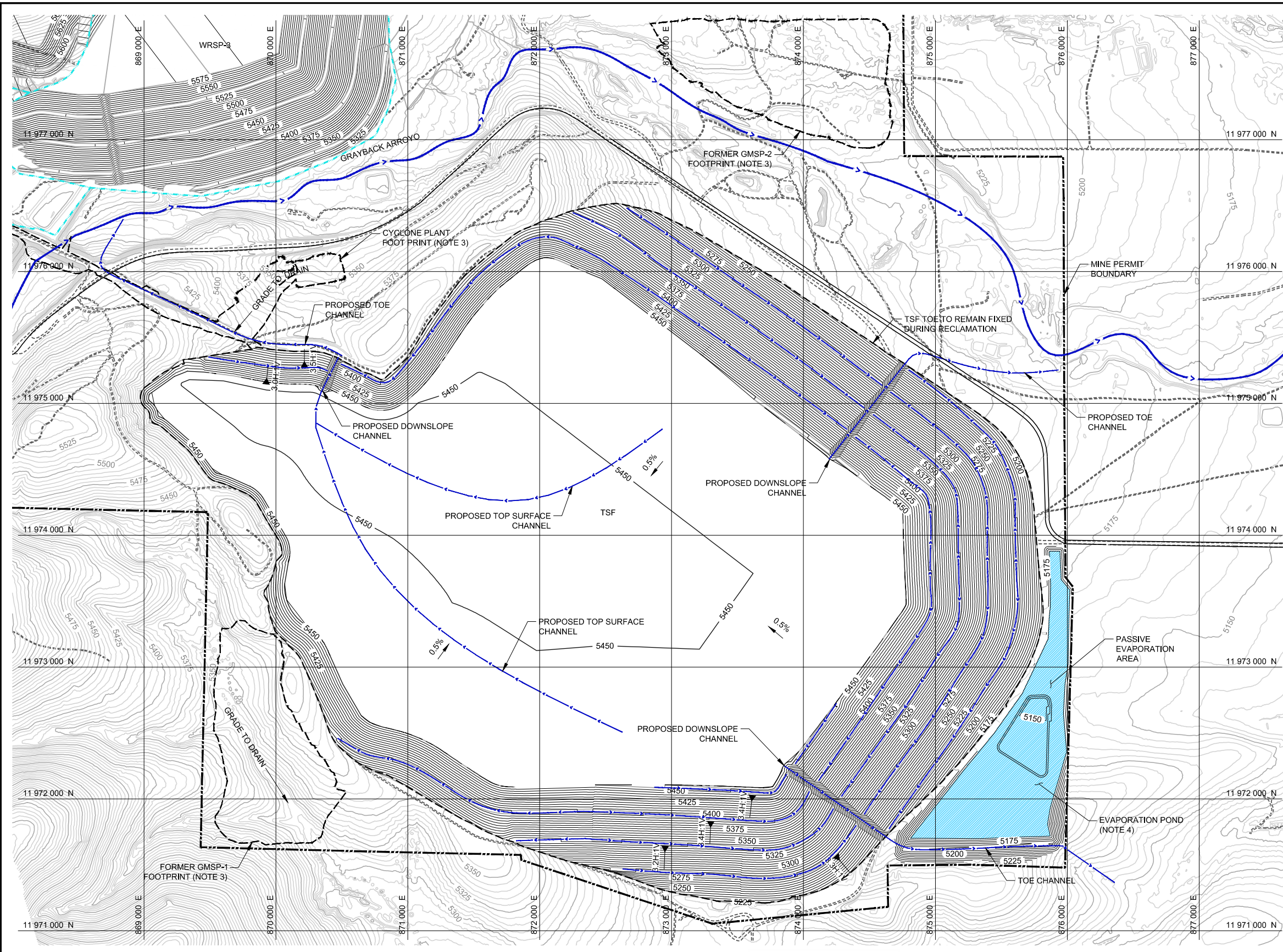
DRAWN	TS
CHECKED	TS
REVIEWED	TS

DATE	09/29/2016
SCALE	NA
FILE	Copper Flat Water Balance 09072016

JOB NO. 153-1453
FIGURE E4



TITLE			
Conceptual Copper Flat TSF Drain Down Curve			
CLIENT/PROJ	DRAWN	DATE	JOB NO.
THEMAC Resources Group Ltd. Copper Flat Mine Project	TS	09/29/2016	153-1453
	CHECKED	SCALE	FIGURE E5
	TS	NA	
	REVIEWED	FILE	
	TS	Copper Flat Water Balance 09072016	

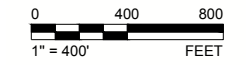


LEGEND

	EXISTING GROUND CONTOURS (ft-MSL)
	5400 PROPOSED DESIGN CONTOURS (ft-MSL) (NOTE 5)
	MINE PERMIT BOUNDARY
	RECLAMATION WATERSHED BOUNDARY
	EXISTING DIVERSION OR ARROYO
	PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
	EXISTING ROAD
	SLOPE INDICATOR
	GRADE INDICATOR
	WASTE ROCK STOCKPILE
	TAILING STORAGE FACILITY

- NOTES**
- FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC.
 - RECLAMATION DESIGN SHALL UTILIZE 3H:1V INTERBENCH REGRADE SLOPES AND 200 FOOT INTERBENCH SLOPE LENGTHS PER NMAC 20.6.7.33. C(3). A 0.5% (MIN.) TOP SURFACE REGRADE GRADIENT SHALL BE ESTABLISHED PER NMAC 20.6.7.33. C(1). A 25 FOOT BENCH WIDTH, 1% BENCH CROSS SLOPE GRADIENT AND 1% LONGITUDINAL BENCH SLOPE GRADIENT WILL BE UTILIZED FOR STORMWATER CONVEYANCE.
 - FOOTPRINT OF FORMER GROWTH MEDIA STOCKPILES WILL BE GRADED TO APPROXIMATE SURROUNDING TOPOGRAPHY, RIPPED AND REVEGETATED.
 - EVAPORATION POND WILL BE CONSTRUCTED IN YEAR 5 FOLLOWING CESSATION OF MINING. NEAR THE END OF THE ACTIVE EVAPORATION PROGRAM. EVAPORATION POND WILL BE RECLAIMED AT THE END OF THE PASSIVE EVAPORATION PROGRAM.
 - PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE. PRE-COVER PLACEMENT TOPOGRAPHY. THIS FACILITY WILL HAVE 3FT OF COVER SO FINAL RECLAIMED SURFACE WILL BE 3RT HIGHER THAN CONTOURS SHOWN.

PRELIMINARY FOR AGENCY REVIEW



REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2016-09-30	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	TS
B	2016-09-09	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2016-09-02	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

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PROJECT
COPPER FLAT PROJECT
MINE PERMIT CLOSURE DESIGN

TITLE
PASSIVE EVAPORATION WATER MANAGEMENT SYSTEM LAYOUT

PROJECT NO. 1531453 CONTROL 0300 REV. C 16 of 25 FIGURE E6

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