

**Appendix A**

**Feasibility Level Design, 30,000 TPD Tailings Storage Facility  
And  
Tailings Distribution and Water Reclaim Systems**

**Copper Flat Project**

**Sierra County, New Mexico**

**Golder Associates Inc.,**

**Revised, June 2016**

**Revised, November 2016**



FEASIBILITY LEVEL DESIGN  
REPORT

# FEASIBILITY LEVEL DESIGN, 30,000 TPD TAILINGS STORAGE FACILITY

COPPER FLAT PROJECT  
SIERRA COUNTY, NEW MEXICO

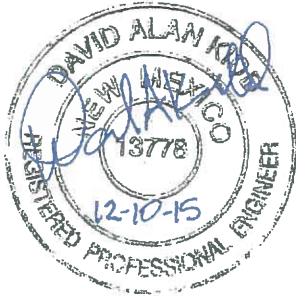
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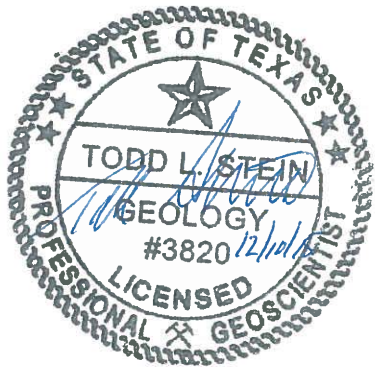
This report documents the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The design included herein was developed at a level consistent for agency review. Development of this report and associated TSF design was conducted under the oversight of the following Golder staff:



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12-10-2015

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30,000 TPD Tailings Storage Facility Report)

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## List of Acronyms

1D	one dimensional
3D	three dimensional
ASTM	American Society of Testing and Materials
cm/sec	centimeters per second
F	Fahrenheit
FEMA	Federal Emergency Management Agency
FOS	factor of safety
ft-bgs	feet below the ground surface
ft-msl	feet above mean sea level
gpm	gallon per minute
HDPE	high density polyethylene
HSA	hollow stem auger
kg	kilograms
LCRS	leakage collection and recover system
MAG	Mineral Advisory Group
MDE	Maximum Design Earthquake
mm	millimeter
NMCC	New Mexico Copper Corporation
NMDSB	Dam Safety Bureau
NMED	New Mexico Environment Department
NOAA	National Oceanic and Atmospheric Administration
OSE	New Mexico Office of the State Engineer
PCPE	polyethylene pipe
pcf	pounds per cubic foot
PGA	peak ground acceleration
PI	plasticity index
PMP	probable maximum precipitation
PSD	particle size distribution curve
psi	pound per square inch
SPT	standard penetration test
TPD	tons per day
TPH	tons per hour
TPY	tons per year
TSF	tailing storage facility
USCS	Universal Soil Classification System
wt%	percent by total weight
yd <sup>3</sup>	cubic yard





## 1.0 INTRODUCTION

### 1.1 Scope

Golder Associates Inc. (Golder) has been contracted by the New Mexico Copper Corporation (NMCC) and its parent company THEMAC Resources Group Ltd (THEMAC), to complete the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The TSF design presented herein has been completed in support of an overall Copper Flat project feasibility study as well as to support the various regulatory processes leading to permit approval of NMCC's project.

The TSF feasibility study report addresses geotechnical aspects of the project and presents the feasibility-level design of the TSF and tailings distribution and water reclaim systems. The individual components of the TSF feasibility study include: (1) the TSF design; (2) whole tailings delivery and distribution systems from the process plant and cyclone plant areas; (3) tailings delivery systems on the TSF; (4) underdrain collection system beneath the TSF; (5) TSF underdrain collection pond designs; (6) surge pond designs; (7) tailings reclaim water collection and delivery systems; and (8) systems for handling potential upset flow conditions (including secondary containment).

The location of the proposed facility is shown on Drawing 1 (Appendix J). Copper Flat is a proposed porphyry copper mining operation at a property that was briefly operated by Quintana Resources in 1981 and 1982. During the former mining operation, open pit pre-stripping was completed and a TSF with a design capacity of approximately 60 million tons was constructed and operated. Shortly after mining operations started, the mine was closed due to adverse economic conditions and depressed copper prices. In Drawing 2, which shows existing site conditions, the remains of the starter dam and splitter dike from the Quintana mining operation can be seen.

A new TSF will be constructed at Copper Flat in the same location as the former (old) Quintana Resources facility. The new TSF will extend approximately 1,000 feet to the east of the old starter dam (the tailings expansion area) as shown on Drawing 2.

A centerline construction method using cycloned tailings sand (cyclone underflow) for tailings dam construction will be utilized. A starter dam will be constructed using borrow material to provide initial storage capacity and to provide a location for initial discharge of tailings. The centerline approach allows construction of a stable, drained tailings dam using the cyclone underflow (i.e., tailings sand), while reducing the quantity of fill material required for dam construction. Tailings slimes (cyclone overflow) will be discharged into the interior of the TSF impoundment. The use of sand tailings for dam construction are such that the cyclone plant will be operated continually to produce the construction material.



The new TSF design will comply with the design and dam-safety guidelines and regulations of the New Mexico Office of the State Engineer (OSE) Dam Safety Bureau (NMDSB, 2010). Stormwater that cannot be diverted will be accommodated inside the impoundment by maintaining a dam crest elevation that provides adequate freeboard for containment of direct precipitation and run on.

The Mining and Environmental Compliance Section of the Ground Water Quality Bureau of the New Mexico Environment Department (NMED) will be the permitting authority for the groundwater discharge permit. NMED has provided guidance on anticipated design requirements for the TSF's liner system, which have been incorporated in this feasibility level design. Golder has also provided the feasibility level design of the tailings distribution and water reclaim systems.

Design drawings (Appendix J) to be read in conjunction with this report include the following:

Drawing 1	Title Sheet and Location Map
Drawing 2	Existing Conditions and Proposed Facilities Plan
Drawing 3	Site Exploration Plan and Geologic Cross Section Locations
Drawing 4	Geologic Cross Section A-A'
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## 1.2 Terminology/Definitions

In this report, the TSF is described as consisting of the impoundment and the tailings embankment or dam. The impoundment refers to the interior tailings storage area located upstream of the embankment or dam. Tailings with different gradations will be placed inside the impoundment and on the dam.



Whole tailings refer to the process tailings delivered to the cyclone plant from the flotation plant outlet. At the cyclone plant, the tailings will be separated into cyclone underflow and cyclone overflow. The cyclone underflow, which consists primarily of the coarse sand fraction of the tailings, will be used to construct the dam. The cyclone overflow, which consists primarily of the fine fraction of the tailings, will be placed in the impoundment. Each of these tailings products will be transported and discharged as a slurry, with varying concentrations of tailings solids suspended in process water.

The cyclone overflow discharged into the impoundment will form a surface that gently slopes away from the point of discharge. The beach refers to the area near the point of discharge where the coarsest particles in the cyclone overflow will tend to settle. The slimes refer to the finer fraction of the cyclone overflow, which will flow down the beach with the majority of the process water to the distal portion of the impoundment surface and settle in the vicinity of the free water pond.

The TSF will have two separate underdrain systems. The dam underdrain will underlie the dam and overlie the dam liner and collect drainage from the cyclone underflow. The impoundment underdrain will overlie the impoundment liner and will collect drainage from the tailings beach and slimes (cyclone overflow).



## 2.0 SITE DESCRIPTION

### 2.1 Topography

Existing surface conditions at Copper Flat are shown on Drawing 2. The starter dam, splitter dike, and approximately 1.2 million tons of tailings from the Quintana Resources operation remain on the property.

The TSF site consists of a broad, shallow basin located at the head of a natural drainage that discharges to Grayback Arroyo. Elevation ranges from 5,170 to 5,435 feet above mean sea level (ft-msl) within the proposed TSF footprint. Topography is gently sloping over most of the site with the steepest slopes located around the west and southwest periphery of the facility. Because the site is near the head of a natural basin, requirements to divert water from upstream catchment areas will be minimal. Surface water diversion is discussed in Section 6.6.

### 2.2 Climate

The property is located within an arid, high desert area in the Basin and Range physiographic province subject to hot summers and relatively mild winters. Maximum summer temperatures can exceed 100 degrees Fahrenheit (F) while the average maximum daily temperature during winter months is approximately 40 degrees F. Average annual rainfall is approximately 13 inches and the property receives snow periodically.

Most rainfall occurs in July through September and is associated with high intensity, short duration, convective storms and moisture from the Gulf of Mexico. Winter precipitation is associated with west to east moving Pacific frontal storms. These storms typically produce less intense precipitation over a longer duration.

Based on the National Oceanic Atmospheric Administration Atlas 14 (NOAA, 2006), the 100-year, 24-hour storm event is estimated to be approximately 3.73 inches. Hydrometeorological Report 55A (US Department of Commerce, 1988), which provides probable maximum precipitation (PMP) estimates for areas located between the continental divide and the 103<sup>rd</sup> meridian, indicates a 72-hour PMP depth of 26 inches.

### 2.3 TSF Area Subsurface Conditions

#### 2.3.1 Geology

The proposed TSF site is located in Sierra County, within the southern Basin and Range physiographic province (Parsons, 1995). The Basin and Range province is described as a broad, highly extended terrain that extends from Canada, through the western United States, and across much of Mexico (Parsons, 1995). The name is derived from the type of extensional block-faulting that left the characteristic pattern of alternating basins and ranges across the province.



The site lies near the eastern edge of the Black Range, on the Piedmont slopes of the Palomas Basin. The Black Range is a Late Cretaceous to early Tertiary (Laramide) volcanic-plutonic arc (Oniell et al, 2002). The Basin and Range province in this region is cut by the Rio Grande Rift zone. The property sits in the westernmost basin of the Rio Grande rift zone, which is made up of three parallel north-trending basins separated by intra-rift horsts (Chapin, 1971).

The Palomas Valley is a relatively narrow flood plain flanked on the east by the piedmont slopes of the Caballo Mountains and on the west by the long gentle slopes ascending to the base of the Hillsboro-Animas Hills (Hawley, 1965).

Detrital material derived from the erosion of the adjacent slopes fills the Rio Grande valley to a depth of up to 9,000 feet (Harley, 1934). Much of this material is gravel, which consists of boulders and pebbles of quartzite, limestone, granite, rhyolite, andesite, and basalt, derived from the surrounding rock complex. They are coarser along the valley sides, grading to finer material toward the middle of the basins. In the higher regions, as in the sides of the valley between Fairview and Hillsboro and at Hermosa and elsewhere, alluvial deposits of Quaternary age are comprised of boulders and pebbles in a finer matrix of the same material, loosely cemented into a firm hard conglomerate known as Palomas gravel (Harley, 1934). Younger sand and gravel deposits, which form some of the intermediate terraces, represent a filling of channels eroded in the older deposits of Santa Fe and Palomas age (Harley, 1934).

The principal fan in the district is that formed by the drainage toward the east out of Copper Flat, principally through Grayback Arroyo, but in part through Dutch Gulch. The basal part of the original fan is composed of fine to coarse fragments of rhyolite derived from the late flows that once covered the Animas Hills. The intermediate part of the fan is composed principally of andesite and latite fragments. The topmost portion of the fan is composed of basalt, andesite, and latite fragments (Harley, 1934).

The Animas Hills consists predominantly of andesite flows and breccias of Late Cretaceous age with minor interbeds of sandstone. The andesites and sandstone are intruded by a stock of quartz monzonite of Late Cretaceous age centered at Copper Flat, and by quartz latite dikes radial to the stock (Seegerstrom et al, 1975).

Upper Cenozoic basin fill in the Rio Grande rift is generally referred to as the Santa Fe Formation or Group. The Santa Fe Group (middle Pleistocene to uppermost Miocene), is comprised of multiple formations including the Camp Rice, Fort Hancock, Palomas, Sierra Ladrones, Arroyo Ojito, Ancho, Puye, and Alamosa Formations.

The Santa Fe consists of basal conglomerate and interbedded sand and clay beds. The cobbles and boulders are mainly andesite. Thickness is variable but generally in tens of feet in the subject area. Two facies in the study area include: 1) the piedmont facies consisting of brown, poorly sorted, weakly



stratified conglomerates and fan gravels and brown sandy silt, conglomerate and clay; and 2) the axial-river facies of cross-stratified sandstone, sand, pebble conglomerate, gravel, and clay lenses. The piedmont facies comprises the major portion of the Palomas Formation. It rests in angular unconformity on bedrock of varying lithologies along the major uplifts bordering the basins. However, in many central basin areas the piedmont facies appears to grade downward into coarse-grained deposits of the lower Santa Fe Group (Lozinsky, 1986).

### 2.3.2 Site Observations

The site geotechnical investigation program details are shown on Drawing 3. Drawings 4 through 9 contain TSF area geologic cross-sections developed on the basis of the recent drill holes and test pits, supplemented with subsurface information reported by Sergeant, Hauskins, and Beckwith (SHB, 1980).

Drill hole logs indicate that the foundation in the tailings area consists primarily of alluvial deposits that includes silt, sand and gravel underlain by clay. In the northwestern waste rock stockpile area, borings indicated the presence of gravelly silts and sands overlying conglomerate consisting primarily of andesite. The conglomerate is underlain by unweathered andesite.

Silts, sands, and gravels that occur in the proposed TSF area have been identified as piedmont alluvium and the older deposits of the Santa Fe Group (SHB, 1980) on which the piedmont alluvium was deposited. The Santa Fe Group is reported to consist of interfingering alluvial fan (gravel) and clay facies. Basalt flows are reported to occur in channels and arroyos cut into the piedmont and Santa Fe sediments. Basalt outcrops have been identified in an arroyo in the center of the TSF and locally around the site.

Drilling logs and geologic cross-sections indicate a high degree of variability in near surface materials both vertically and laterally, within the impoundment area and beneath the proposed dam. Silty/clayey horizons alternating with gravelly sand layers could potentially represent either the interfingering of the Santa Fe Group facies, or the more recent effects of local erosion and deposition.

In general, the interior of the impoundment is underlain by silty, clayey and gravelly sand, and cemented gravelly sand with a near surface layer of silty, wind-blown material. Eastward, toward the future dam site, interbedded clays and silts occur at depths typically greater than 20 feet; however, the composition of the foundation remains highly variable with interfingering, silty, sandy and clayey gravel units.

Groundwater is typically encountered at depths greater than 50 feet below ground surface (ft-bgs) in the vicinity of the TSF. A small zone of perched water has been identified in the vicinity of the old Quintana dam; however, recent drill holes completed in the perched water area to a depth of 50 ft-bgs did not intercept water. Groundwater and local perched water are not anticipated to impact the design and operation of the TSF.



## 3.0 SITE INVESTIGATIONS

### 3.1 Previous Site Studies

Portions of the TSF site were investigated by Sergeant, Hauskins, and Beckwith (SHB, 1980) prior to construction of the Quintana Resources TSF. The SHB investigation focused on the dam alignment for a 60 million ton facility, which does not coincide with the alignment currently proposed for the over 1 million ton facility. SHB coverage of the potential borrow areas in the impoundment interior was relatively extensive.

### 3.2 Site Geotechnical Investigation Program Description

Golder conducted a site geotechnical investigation between December 2012 and January 2013 to expand the coverage of the former SHB site investigation to include the new dam alignment. The field investigation consisted of 31 test pit excavations and 28 drill holes. Drawing 3 illustrates test pit and drill hole locations, and the location of geologic cross-sections developed to show the geology of the dam foundation.

Test pits were excavated with a Case CX210B or Terex 7606 hydraulic backhoe to depths up to 20 feet. Test pits were logged and photographed. Soil samples collected from test pits included bulk 5-gallon bucket and bag grab samples. Bulk samples are suitable for geotechnical soil classification, strength, consolidation, compaction and permeability testing. Bag samples are suitable for soil classification and moisture content testing. Test pit logs are contained in Appendix A.1.

Most of the drilling was completed with a track mounted CME 75 drill rig using hollow stem augers (HSA). The rig was equipped for conversion to diamond core drilling if bedrock was encountered. The drill hole target depth was 50 ft-bgs. Due to the presence of cemented gravels that were not amenable to drilling by either HSA or diamond core methods, down-hole percussion (Tubex) equipment was also used.

Standard penetration testing (SPT) was carried out at 5-foot intervals during HSA drilling. When percussion drilling was required, SPT test frequency was reduced to about 10 feet. Samples collected during drilling included bulk 5-gallon bucket cuttings samples from auger drilling, bagged Tubex rig cuttings samples, and bagged samples recovered from the SPT split-spoon sampler. Drill hole logs are contained in Appendix A.2.

### 3.3 Geotechnical Testing

Bulk and bag samples collected during site exploration were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for soil classification, compaction, permeability, consolidation, and strength testing. Laboratory reports for the various tests are contained in Appendix A.3. Soils were classified according to the Universal Soil Classification System (USCS). All geotechnical tests were completed in



accordance with applicable American Society of Testing and Materials (ASTM) standards. No undisturbed samples were collected during the site exploration. Geotechnical strength, consolidation, and permeability tests were conducted on remolded samples.

### **3.3.1 Soil Classification**

#### **3.3.1.1 Tailings Impoundment Interior**

Within the interior, central and western portions of the impoundment area, site soils consist predominantly of clayey sand with gravel (SC), well-graded silty sand with gravel (SW-SM) with lesser clayey and well-graded silty gravel (GC, GW-GM). Silty and clayey soils (CL-ML) also occur locally. The fine fraction (finer than 75 microns or the material passing the No. 200 standard sieve) in sandy samples from the impoundment interior ranges from 8 to 29 percent and averages 20 percent. Plasticity indices (PI) average 17 percent. The specific gravity of interior area soils ranged from 2.67 to 2.75.

Four composite samples of near surface sandy, gravelly soils were prepared to evaluate materials potentially available for the construction of processed (crushed and screened) drainage material. Each composite sample was composed of two to three 5-gallon bucket samples of near surface materials. Composite samples were initially crushed to 100 percent finer than 1 inch to simulate material that would be suitable for placement against a geomembrane liner. Three of the four composites were classified as clayey sand (SC), while the other was classified as well graded silty gravel (GW-GM). The fine fraction in the crushed composite samples ranged from 8 to 20 percent and the average PI was 17 percent. It is anticipated that screening will be required to reduce the fines content of this material if it is to be used for highly permeable, manufactured drain fill adjacent to critical drainage pipes.

#### **3.3.1.2 North Cell Tailings**

The north cell of the old Quintana TSF contains tailings from mining conducted in the early 1980s. Old tailings samples were classified as silty sand (SM) and low plasticity silt (ML). The sample classified as sand was located adjacent to the old dam and presumably, near the point of discharge. It contained a minus 75-micron fine fraction of 49 percent and was non plastic. The silty sample was obtained from the center of the north cell and had a minus 75-micron fraction of 71 percent and a PI of 5 percent. Differences in old tailings properties are presumed to be the result of tailings segregation on the former tailings beach. Moisture content in the old tailings samples ranged from 6.0 to 11.3 percent.

#### **3.3.1.3 Tailings Dam Footprint**

Soils encountered in the footprint of the proposed dam are highly variable. Clayey sand and gravel (SC,GC) generally occur at shallow depth with interbedded high and low plasticity clays (CH,CL) and silts (ML,MH) occurring at depths below 20 ft-bgs. Clay intercepts indicate that the clay occurs in





discontinuous lenses or in eastward dipping strata. The fine fraction in the sandy and gravelly soils ranges from 17 to 39 percent and the PI averages 17 percent.

### 3.3.2 Foundation Strength Testing

Foundation samples were subjected to consolidated undrained triaxial testing with pore pressure measurement to determine effective shear strength for use in supporting stability analyses. Confining pressures were selected to represent anticipated foundation pressures associated with tailings embankment construction. Granular foundation materials are assumed to be cohesionless. Strength data reported below represent the effective internal friction angle. Triaxial test reports are contained in Appendix A.3.

It is anticipated that the old tailings will be placed beneath the new TSF geomembrane liner as a liner bedding fill layer. A 12-inch by 12-inch direct shear test was conducted to evaluate the interface friction at the TSF liner interface. The direct shear set-up included a layer of compacted old tailings from Test Pit 10 in the center of the north cell, a textured 80-mil high density polyethylene (HDPE) geomembrane, and a layer of material from the drainage composite samples 1 through 4. Tailings underlying the liner were placed at optimum moisture content based on ASTM D698 compaction testing and maintained in an unsaturated state during shearing. The over-liner drainage materials were wetted prior to testing but maintained in an unsaturated state during the test. The test was conducted in a manner that allows the failure through the lower strength interface (i.e., liner against old tailings or liner against drain fill). Direct shear test results are contained in Appendix A.3.

Table 1 summarizes the strength test samples, test objectives and strength test results.

**Table 1: Summary of Strength Test Results**

Sample	Material, Objective, Test Method	Internal Friction Angle
TP-10-3-13	Old tailings, strength of liner bedding triaxial CU <sup>(1)</sup>	34 degrees
TP-10-3-13	Old tailings, liner interface shear strength, direct shear <sup>(2)</sup>	26.5 degrees
Composite 1-4	TSF Interior borrow for structural fill and drain material, triaxial CU	40 degrees
BH-16-0-8.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	28 degrees <sup>(3)</sup>
BH-25-0-12.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	43.5 degrees
BH-10-0-14.5	Clayey sand, structural fill and embankment foundation strength, triaxial CU	32 degrees

**Notes:**

<sup>(1)</sup> CU = consolidated, undrained

<sup>(2)</sup> Liner interface testing included old tailings and drain material in contact with textured, 80-mil HDPE geomembrane.

<sup>(3)</sup> Sample was 91 percent finer than ¾ inch. Represents a matrix shear strength and does not account for interlocking of the coarse fraction.

No undisturbed samples were collected during site exploration. Tests were conducted on remolded samples.



### **3.3.3 Drainage Material Permeability Testing**

Drainage material permeability testing was performed to support the design of the impoundment and dam underdrains. Testing was completed on composite samples 1 through 4, which represent the near surface materials that will be available for preparation of drainage material. Equal volumes of composite samples 1 through 4 were blended to prepare a sample for permeability testing. Samples were tested in a 10-inch diameter rigid wall cell with a 150-pound-per-square-inch (psi) load applied to simulate loading under field conditions. Permeability test reports are contained in Appendix A.3.

Two drainage materials are considered in the TSF design. These include crushed and screened materials with reduced fines content for the dam underdrain and primary drains in the impoundment interior (primary drain fill), and material obtained from selective borrowing of sandy soils within the TSF footprint (select native drain fill). Processing of the select native drain fill is anticipated to be limited to crushing to reduce the maximum particle size. No washing to reduce the fines content is anticipated to be required to produce suitable drainage materials.

#### **3.3.3.1 Primary Drain Fill**

A primary drain fill sample was prepared for testing the performance of the dam underdrain. The minus No. 40 standard sieve fraction of the sample was removed to simulate a prepared drainage material with low fines content that is filter compatible with the cyclone underflow sand. The hydraulic conductivity of the primary drain fill sample was  $9.1 \times 10^{-2}$  centimeters per second (cm/sec).

#### **3.3.3.2 Select Native Drain Fill**

The composite sample was tested without additional modification to estimate the permeability of the sandy site soils. As tested, the minus 75-micron fraction in the composite samples was 9 percent. The hydraulic conductivity of the select native drain fill sample was  $3.8 \times 10^{-5}$  cm/sec.

### **3.3.4 Foundation Sample Consolidation Testing**

Selected foundation samples were subjected to conventional one-dimensional consolidation testing to support estimation of settlement potential in the proposed dam foundation. Samples were selected to evaluate silty and clayey horizons where changes in loading conditions could result in additional consolidation. Samples were remolded to dry densities ranging from 88 to 95 pounds per cubic foot (pcf) at natural moisture content to reflect in-situ density estimated from standard penetration tests. Foundation sample consolidation test reports are contained in Appendix A.3. Foundation settlement potential is discussed in Section 11.0.



## 4.0 TAILINGS TESTING

### 4.1 Program Description

The TSF embankment proposed in this feasibility study is to be constructed by the centerline raise method using cyclone underflow. Whole tailings from the flotation plant will be routed to a cyclone plant where the tailings will be separated into underflow (sand) and overflow (slimes) fractions. The cyclone underflow will be routed to the dam centerline and used to construct the tailings dam. The overflow will be discharged into the impoundment interior.

Primary considerations for effective centerline sand dam construction include adequate drainage and compaction of the cyclone underflow sand. Drainage requirements are typically met by:

- Producing a relatively free draining sand material. This is usually achieved when the minus 75-micron fraction of the underflow does not exceed 20 percent by weight.
- Having the hydraulic conductivity of the material placed in the dam two orders of magnitude greater than the material placed in the impoundment.

These two conditions generally result in a well-drained structure. A fixed cyclone station maintains optimum and consistent conditions at all cyclones and facilitates meeting gradation objectives. A blanket drain will be placed beneath the embankment to facilitate collection of cyclone sand drainage and minimize saturated conditions at the base of the embankment.

Industry experience at operating mines utilizing cyclone sand for dam construction indicates that hydraulic placement and self-weight compaction is generally sufficient to minimize liquefaction potential in cyclone sand dams located in regions with low seismic risk, such as at Copper Flat. Where it is required, compaction to a relative density of 60 percent (equivalent to approximately 90 percent of ASTM D698 maximum dry density) will result in low potential for liquefaction under static and seismic loading conditions (CANMET, 1977). At Copper Flat, tailings placed on the dam crest will be compacted. Some compaction of tailings placed on the dam out-slope will occur as a result of dozer spreading operations.

An initial cyclone test was conducted on a 55-kilogram (kg) tailings sample to determine the quantity and quality of sand available for dam construction, and produce samples of future tailings products for geotechnical testing. The initial cyclone test sample was produced by Metcon Research in September 2011. The sample was run through a 4-inch cyclone at the FL Smidth-Krebs (Krebs) facility in Tucson, Arizona. On the basis of the initial 4-inch cyclone test, Krebs, utilizing proprietary software, predicted that a gMAX15U-20 (15-inch) cyclone could recover 46 percent of the whole tailings stream with a minus 75-micron fraction of less than 20 percent. The quantity and quality of the cyclone underflow predicted on the basis of the 4-inch cyclone test were consistent with industry guidelines for sand dam construction. The quantity of sand recovered met anticipated construction requirements.



An additional pilot scale metallurgical study was conducted by the Mineral Advisory Group (MAG). Approximately 255 kg of tailings solids (whole tailings) were provided by MAG in five sealed 55-gallon steel drums. Drums were delivered to Krebs in July 2012 and the tailings were passed through a gMAX15U-20 cyclone in the Krebs laboratory on August 4, 2012. The sand recovery was 41 percent and the minus 75-micron fraction was under 16 percent. Cyclone underflow, and cyclone overflow produced during the test, and residual whole tailings were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for testing.

Appendix B.1 contains Krebs analysis of the August 2012 cyclone run and the simulation (prediction) of full field scale cyclone performance. The cyclone plant is estimated to have a recovery of 45 percent with a minus 75-micron fraction of 18 percent.

## 4.2 Tailings Test Program Description

Table 2 contains a test matrix for the tailings products produced during the cyclone test. Gradation analyses were completed on the cyclone underflow, cyclone overflow and whole tailings. The cyclone overflow and whole tailings were subjected to flume testing to simulate discharge into the impoundment. Flume testing involves the discharge of tailings slurry at the field anticipated solids content into a 12-inch flume at low velocity. The tailings flow down and settle in the flume. Samples are collected at various flume locations to evaluate changes in gradation and solids content. Gradation and slurry consolidation tests were conducted on samples from the head and tail of the flume to evaluate the characteristics of tailings found on the beach and in the interior of the impoundment (slimes). Laboratory data sheets for tailings tests are contained in Appendix B.

**Table 2: Test Matrix for Cyclone Underflow, Cyclone Overflow and Whole Tailings**

Test Method	Underflow	Overflow	Whole Tailings
Gradation and Atterburg Limits (ASTM D4221, D4318)	1	2 <sup>(1)</sup>	2 <sup>(2)</sup>
Specific Gravity (ASTM D854)			1
Compaction Test (ASTM D-698)	1		
One-Dimensional Consolidation (ASTM D2435)	1		
Flume Test (flumes samples taken from head and tail of flume to evaluate segregation and settling) (specialty test)		1	1
Column Settling Test (single drain) (specialty test)		1	1
Slurry Consolidation Test (specialty test)		2 <sup>(1)</sup>	2 <sup>(2)</sup>
Permeability (ASTM D2434)	1		
Triaxial Shear Strength, Consolidated-Undrained (ASTM D2850)	1	1	

**Notes:**

- (1) Testing completed on head and tail section samples from the flume to simulate properties of the beach and slimes fraction of the cyclone overflow.
- (2) Testing completed on head and tail section samples from flume to simulate properties of the beach and slimes fraction of the whole tailings.



## 4.3 Test Results

### 4.3.1 Material Classification

Table 3 summarizes the characteristics of the tailings prior to and following cyclone separation. Gradation test results for the whole tailings cyclone feed, cyclone underflow and cyclone overflow are contained in Appendix B.2.

**Table 3: Summary of Tailings Properties, Pre- and Post-Cyclone Separation**

Material	P <sub>80</sub> (microns) <sup>1</sup>	Minus 75-micron Fraction (percent) <sup>2</sup>	USCS Classification
Whole Tailings (feed)	110	59	SM (silty sand)
Underflow (sand)	150	17.9	SM (silty sand)
Overflow (slimes)	5.7	90	ML (low plasticity silt)

**Notes:**

<sup>1</sup>P<sub>80</sub> is the particle size for which 80 percent of the material is finer

<sup>2</sup>The minus 75-micron fraction is the percentage of clay and silt sized particles

Based on the gradation test results, the whole tailings sample produced in the MAG metallurgical study is slightly coarser than the flotation tailings that will be produced during operations (P<sub>80</sub>=110 microns versus the design P<sub>80</sub> of 105 microns).

### 4.3.2 Cyclone Underflow Testing

Cyclone underflow sand will be delivered to the dam at a solids content of approximately 70 percent based on cyclone test results and cyclone performance predictions. Cyclone underflow sand discharged on the dam crest will be spread and compacted. Moisture density testing (ASTM D698) indicates a cyclone underflow maximum dry density of 97 pcf and an optimum moisture content of 16.8 percent. The moist weight of the compacted cyclone underflow will be on the order of 110 pcf. Underflow compaction test data are contained in Appendix B.2.

Cyclone underflow samples were subjected to one-dimensional consolidation and consolidated-undrained triaxial shear strength testing. A cyclone underflow sample compacted to within 5 percent of maximum dry density exhibited an effective internal friction angle of 40 degrees. Consolidation and triaxial test reports are contained in Appendix B.3.

### 4.3.3 Cyclone Overflow and Whole Tailings Testing

Flume tests were conducted on cyclone overflow and whole tailings in a 12-inch wide by 24-foot long test flume. The primary purpose of the flume tests is to provide data to support estimation of post deposition density. Samples were collected from the head and tail sections of the flume for gradation, settling and slurry consolidation testing. Test data were used to develop input parameters for one-dimension numerical consolidation modeling and impoundment filling rate studies. Flume test gradation, settling and



slurry consolidation test reports are contained in Appendix B.4. Tailings consolidation modeling is discussed in Section 5.0.

To support stability analyses, a sample of cyclone overflow collected from the head of the test flume (beach material) was subjected to consolidated-undrained triaxial shear strength testing. The sample was tested at a dry density of approximately 94 pcf and a moisture content of 27 percent. The measured effective internal friction angle was 37 degrees. The triaxial test report is contained in Appendix B.4.



## 5.0 TAILINGS CONSOLIDATION ANALYSES

### 5.1 Approach

Consolidation calculations were performed using the computer program FSConsol (GWP Software, 1999). FSConsol performs a one-dimensional (1D), large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967). For modeling purposes, the non-linear relationships used to express permeability and compressibility are those proposed by Abu-Hejleh and Znidarcic (1994 and 1996), and defined by Equations 5.1 and 5.2, which are used in the consolidation and desiccation numerical model.

$$k = C e^D \quad \text{Equation 5.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 5.2}$$

When using FSConsol model, Equation 5.1 remains the same; however, Equation 5.2 is rewritten by the modified power law form to represent compressibility, as shown in Equation 5.3.

$$e = A(\sigma')^B + M \quad \text{Equation 5.3}$$

Where:

e = void ratio of the tailings

$\sigma'$  = the effective confining stress

k = the hydraulic conductivity of the tailings

A, B, M (or Z), C, and D are material parameters determined from laboratory slurry consolidation and column settling tests

Five material parameters, *A*, *B*, *C*, *D*, and *M*, were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 and 2. Fitted parameters are shown in Tables 4 and 5. Data are based on slurry consolidation testing of cyclone overflow samples derived from the head and tail sections of the test flume discussed in Section 4.3.3.

**Table 4: Permeability Input Parameters for Cyclone Overflow**

Sample	C (centimeters per second)	D (dimensionless) <sup>1</sup>
Slimes	$1.380 \times 10^{-7}$	3.353
Overflow Beach	$1.523 \times 10^{-6}$	3.035

**Table 5: Compressibility Input Parameters for Cyclone Overflow**

Sample	A (1/kilopascals) <sup>B</sup>	B (dimensionless) <sup>1</sup>	M (dimensionless) <sup>1</sup>
Slimes	3.144	-0.1952	-0.1424
Overflow Beach	1.787	-0.2983	0.5224

**Note:**

<sup>1</sup> B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second units



The tailings impoundment was modeled by running analyses on single tailings columns or volumes filled with materials representing either the beach or slimes components of cyclone overflow. Under the proposed mining plan, the operator will be required to maximize use of the cyclone plant to generate sufficient sand to construct the dam. Therefore, whole tailings discharge will have little impact on the filling rate of the TSF or the characteristics of tailings placed in the TSF.

In each model run, the modeled component is assumed to represent 100 percent of the inflow to the model storage volume. Models were run until the modeled storage volume reached capacity, or the entire mass of tailings was input. The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities (adjusted for errors as discussed in Section 5.3) from the final void ratio profile of each tailings component to obtain an overall dry density that is weighted based on laboratory grain size distribution mass balance results, as discussed in Section 5.2.

## 5.2 Tailings Beach versus Tailings Slimes Split

Grain size distribution tests were performed by the Golder soils laboratory in the Denver, Colorado, on beach and slimes samples from laboratory flume tests and on a cyclone overflow head sample discharged into the test flume. From the grain size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the beach and slimes that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the beach and slimes samples should equate to the percentage of sand and fines in the original cyclone overflow head sample. Table 6 depicts the results of the laboratory gradation tests and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes) by weight or mass.

**Table 6: Laboratory Grain Size Distribution and Beach to Slimes Split Calculation**

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids Recovered	% Sand recovered from overflow	% Fines recovered from overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head Sample	9.63	90.37	100	9.63	90.37

## 5.3 Consolidation Modeling Results

FSConsol model output and a detailed description of modeling procedures and interpretation are contained in Appendix C. If 100 percent of the inflow into the TSF impoundment is represented by the tail section flume sample (i.e., the finest fraction of the tailings), FSConsol predicts that the final dry density of the slimes fraction over the modeled profile will average approximately 33.5 pcf. With 100 percent of the





inflow representative of tailings overflow beach materials, FSConsol predicts a final average tailings dry density of 78.6 pcf

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall bowl shaped geometry of the TSF, and the fact that the mass and volume quantities are larger towards the top than at the bottom of the TSF. The average dry density of each tailings component is corrected based on the average of the calculated percent error between 1D checks of the height of solids, the volume of solids, and the mass of solids. Based on the TSF geometry, calculated material properties, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1 to 14 percent with an average error ranging between 2.7 and 5.4 percent. Based on Golder's experience, 1D analyses resulting in less than a 15 percent average error reasonably depict expected consolidation in the field; therefore, the three-dimensional (3D) impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error calculated for the following checks: height of solids, volume of solids, and mass of solids.

In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components. After accounting for 3D characteristics of the impoundment geometry and errors, the predicted dry density of the beach and slimes components are 74.6 and 31.7 pcf, respectively.

#### 5.4 TSF Capacity

Based on model results and corrections based on error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow slimes and beach at the end of filling. The capacity of the TSF interior is 96.9 million cubic yards (yd<sup>3</sup>) with a crest elevation of 5,460 ft-msl and a maximum tailings surface elevation of 5,450 ft-msl. While the beach and slimes components are predicted to represent 60 and 40 percent, respectively, of the mass of cyclone overflow to be discharged into the TSF interior based on the mass balance as discussed in Section 5.2, these components will represent approximately 40 and 60 percent, respectively, of the storage volume. At the estimated densities predicted with FSConsol, the required storage capacity is 93.4 million yd<sup>3</sup>. This assumes near constant operation of the cyclone plant. With approximately 96.5 percent utilization of the cyclone plant, available storage capacity will equal required storage capacity.

Without consideration of managed deposition effects, there is a small excess in available storage volume. The estimated weighted dry density of tailings within the TSF impoundment is 48.4 pcf, which is low in comparison to copper industry experience. Managed deposition such as cycling discharge locations to promote desiccation and controlling the size of the free water pond can be expected to increase the post deposition dry density and reduce storage volume requirements. In addition, the FSConsol modeling runs for slimes inflow were run for a time period of 7.9 years. The slimes can reasonably be expected to



continue to consolidate and increase in dry density through the remainder of the 11.1-year mine life. Only a slight increase in the dry density of the slimes over the remainder of the mine life will result in an increase in storage capacity.

The inflow rate for cyclone overflow assumes near constant operation of the cyclone plant to produce sufficient sand to construct the dam to the elevation of 5,460 ft-msl. As such, the mill should not be operated unless the cyclone plant is operating, and maintenance of the cyclone plant should be performed concurrently with mill maintenance. The cyclone overflow distribution system has two operating legs that will facilitate near constant operation. During operations, filling rates and tailings post deposition dry density should be regularly monitored to evaluate consolidation characteristics. If the rate of consolidation is better than predicted, utilization of the cyclone plant could potentially be reduced, the final dam crest could be lowered, and a corresponding reduction in the amount of sand required to construct the dam could be realized.



## 6.0 FEASIBILITY LEVEL DESIGN

Table 6 summarizes key design criteria assumed in feasibility level design of the new TSF.

**Table 6: Feasibility Study Design Criteria**

Regional Design Factors	
Precipitation/ Evaporation	Based on NOAA weather data for Hillsboro and Caballo Dam, New Mexico
Design Storm Events	100 percent of the 72-hour general storm probable maximum precipitation (PMP), 26 inches
Stability FOS	Minimum 1.5 for static conditions and 1.1 for seismic loading conditions
Seismicity PGA	USGS MDE, 2475-year return period, 0.13 times gravitational acceleration (0.13g)
TSF Design Factors	
Storage Capacity	112 million tons (THEMAC)
Production/Delivery Schedule	1,333 tons per hour (TPH) net tailings to the TSF year 1-5, 1,222 TPH years 6 to 11.1, 125,000 tons per year (TPY) (THEMAC), post concentrate recovery
Mill utilization	92.5 percent (M3)
Operating Life	11.1 years (THEMAC)
Tailings Specific Gravity	2.64 (Golder test)
Tailings Solids Content (wt%)	29.2 percent solids by weight (whole tailings to cyclone plant). Tailings diluted in outlet sump as needed to optimize cyclone performance.
Production Rate	Varies, Net tailings to the TSF from 9,182 to 10,704 kiltons per year (25,156 to 29,326 tons per day) (NMCC)
Tailings post- deposition dry density	31.7 and 74.6 pounds per cubic foot (pcf) dry weight assumed for post-deposition cyclone overflow slimes and beach, respectively, 92 pcf dry weight for the cyclone underflow fraction. (Golder estimate)
Embankment Construction	Phase 1 earthen starter dam to an elevation of 5,250 ft-msl. Post Phase 1 peripheral earthen dam extension constructed to 10 feet above grade. Centerline raise construction using cyclone underflow sand. Cyclone underflow on dam crest compacted to minimum of 90 percent of American Society for Testing and Materials (ASTM) D698, relative density > 60 percent
Liner System	From bottom to top: Prepared foundation, 12-inch liner bedding fill, 80-mil HDPE geomembrane, overliner drainage collection layer with internal drainage pipe network beneath the tailings embankment and continuous beneath impoundment



<b>TSF Design Factors (cont.)</b>	
Earthworks Slopes (assumed)	Soil cut slopes = 1.5H:1V (1.5 horizontal to 1 vertical) Rock cut slopes = 1H:1V Fill slopes = 2H:1V Lined slopes = 3H:1V to 2.5H:1V max Embankment out-slope = 3H:1V nominal Starter Dam, 2.5H:1V inner, 2H:1V outer
Drainage/TSF Underdrain Collection Pond	Double-lined pond with LCRS to contain dam and impoundment under drainage and surface water runoff. Pond to be constructed as an OSE non-jurisdictional facility.
Collection Pond Reclaim	Submersible turbine pumps with 4,000-gallon per minute (gpm) capacity
Collection Pond Capacity	Normal inventory, 24 hours reserve capacity for underdrain for reclaim pump system upset, 100-year, 24-hour event (3.73 inches) stormwater storage capacity for runoff contributing areas
Tailings Management	Tailings routed through eighteen 15-inch cyclones at 83 TPH feed rate per cyclone. 45.2 to 45.6 percent underflow solids recovery (Krebs), 18.2 to 18.4 minus 200 fraction in underflow (Krebs). Cyclone overflow discharged from the dam crest into the impoundment interior.
Supernatant Reclaim	Floating barge with 12,978-gpm capacity
<b>TSF Water Storage and Stormwater Diversion Design Factors</b>	
Dam Safety Hazard Ranking	Significant, due to environment risks associated with a release of tailings (OSE)
TSF Pond Design Freeboard	As required to accommodate wave run-up and provide minimum freeboard for design storm
TSF Pond Required Stormwater Storage	Contain flows from 1.0 times the 72-hour PMP storm event plus normal inventory of supernatant water
Hydrology Runoff Curve Numbers	100 - Impounded tailings and lined areas 50 - Tailings embankment sand shell 92 - Native ground surfaces
Stormwater Diversion	Divert runoff from undeveloped areas inside ultimate footprint where feasible. Divert exterior area runoff where feasible.
Underdrain System	Continuous underdrain layer beneath dam and TSF interior. Collected water will be returned to the process via TSF underdrain collection pond reclaim pump system
TSF Water Pond Surface Area	40 percent of tailings impoundment interior or a maximum of 40 acres assumed for feasibility level water balance calculations
TSF Water Pond Surface Evaporation	75 percent of average Pan evaporation
Tailings Surface Evaporation	50 percent of average Pan evaporation

**Notes:**

FOS = factor of safety

MDE = maximum design earthquake

PGA = peak ground acceleration

HDPE = High density polyethylene



The following sections describe the various construction components of the TSF feasibility level design.

## 6.1 Earthworks

TSF construction activities will require borrowed structural fill for starter dam and toe berm construction, drain fill for constructing underdrain layers, and liner bedding fill material. Due to permit boundary and land ownership conditions, the majority of the construction materials must be derived from within the TSF footprint. Meeting fill, reclamation topdressing salvage, and reclamation cover material requirements for construction will necessitate stockpiling selected materials early in the life of the TSF because the borrow sources for these materials will be buried as the TSF footprint expands. Initial Phase 1 grading and liner installation will cover approximately 60 percent of the ultimate TSF footprint and much of the area available for borrowing construction material and reclamation cover.

The Phase 1 liner bedding fill material will be derived from the existing tailings produced during the Quintana Resources operation. All existing tailings lie within the Phase 1 construction footprint. Liner bedding fill needed for Phase 2 through Phase 5 construction will be derived from soil borrow areas.

Drain fill material will be produced by crushing and screening native soils and gravels. Drain fill material will be placed in contact with geomembrane liners. To meet drainage and liner compatibility requirements, a minus 1-inch gradation is assumed. The fine rejects (undersized) from drain fill material production will be suitable for liner bedding fill. Because Phase 1 liner bedding fill requirements will be met by using the existing tailings, the undersized materials produced in Phase 1 will be stockpiled for use as liner bedding fill in Phases 2 and 3. Liner bedding fill stockpiled in Phase 1 will be supplemented with additional material produced during construction. Approximately 100,000 cy<sup>3</sup> of undersized material will be stockpiled in Phase 1.

### 6.1.1 Site Grading

The TSF grading plan for Phases 1 through 5 is illustrated on Drawing 10. The topographic surface shown on the grading plan reflects the over-excavation/removal of borrow materials required for cover material stockpiling and TSF construction. The approximate construction limits by phase are also indicated on Drawing 10. Drawing 11 illustrates the Phase 1 grading plan and Phase 1 construction. Site grading will include removal of the old starter dam and splitter dike for use as structural fill in the new starter dam and toe berm. Additional structural fill and drainage material will be borrowed from within the TSF footprint. In general, gravelly sands suitable for drain material lie on the interior impoundment slopes to the west of the dam. Materials suitable for structural fill are exposed on the surface over most of the TSF footprint. Borrow areas developed during phased construction will extend across construction phase limits but will lie within the ultimate TSF footprint.



## 6.2 Toe Berm and Starter Dam

The Phase 1 toe berm and starter dam are illustrated on Drawing 11. The TSF at final build-out is shown on Drawing 12, and the toe berm and dam sections and details are shown on Drawings 13 and 14.

A temporary toe berm will be constructed around the downslope TSF periphery in Phase 1. The temporary toe berm will be removed and reconstructed as the liner is extended outward and downslope in Phases 2 through 4. In Phase 5, a permanent toe berm will be constructed.

The primary purpose of the temporary toe berm is to contain runoff and sediment from the dam face, and direct dam drainage to the underdrain collection system and then to the TSF underdrain collection pond. Both the temporary toe berms and permanent toe berm will be constructed with structural fill and a geomembrane “flap” draped over the perimeter berm. The temporary berm liner flap will be anchored in a temporary perimeter anchor trench on the top of the berm. To relocate the temporary toe berm, the geomembrane flap will be folded inward over sandbags to divert drainage away from the temporary berm while it is removed. Once the temporary toe berm is removed, the liner extension will be installed and the original liner will be laid back over the liner extension. The seam between the two liners will then be extrusion welded in accordance with industry standards. This method will ensure that the liner seam is located in the downgradient flow direction for tailing drainage. At final build-out, a permanent toe berm will be constructed to contain runoff and sediment, buttress the dam toe and establish the limit for reclamation cover placement. The return water pipeline from the TSF underdrain collection pond will also run along the upstream side of the toe berm and above the geomembrane liner. The return water pipeline will be relocated in conjunction with the reconstruction of the temporary toe berms during Phases 2 through 4. In Phase 5, the return water pipeline will be placed in its final location along the upstream side of the permanent toe berm and above the geomembrane liner.

The Phase 1 starter dam will be constructed to an elevation of 5,250 ft-msl, with a 2.5H:1V inner slope and a 2H:1V outer slope. The purpose of the starter dam is to provide initial containment of tailing material, and to aid in tailings distribution from the dam crest. In the early stages of the operation, impounded water may periodically come in contact with the upstream face of the starter dam. The upstream face of the Phase 1 starter dam will be lined with an extension of the TSF geomembrane liner. The purpose of the liner extension is to prevent tailing drainage into the starter dam fill. The liner extension is illustrated on Drawing 13.

The starter dam will be constructed over the impoundment liner and underdrain collection systems. In Phases 2 through 5, starter dam extensions will be constructed to a height of 10 feet over the liner surface along the south, west and north boundaries.



### 6.3 TSF Liner System

Liner system details are shown on Drawing 14. The liner will consist of an 80-mil HPDE liner placed on a 12-inch thick liner bedding fill layer. In Phase 1, the liner bedding fill will consist of a minimum of 12 inches of tailings recovered from the north cell of the old starter dam. After Phase 1, liner bedding fill will consist of a 12-inch layer of crushed and screened native material, or selected local soil.

### 6.4 Tailings Drainage

Drainage from future tailings will be collected in two separate underdrain systems and transported to the TSF underdrain collection pond. Drainage from the TSF impoundment interior will be collected in a continuous underdrain (impoundment underdrain) constructed over the geomembrane liner. A separate blanket drain will underlie the tailings dam (dam underdrain). The layout of the underdrain systems is shown on Drawing 15. Underdrain details are shown on Drawings 16 and 17.

#### 6.4.1 Drain Description

The impoundment underdrain system will consist of a system of primary 10-inch diameter drainage pipes placed in drainage channels, and a system of 4-inch diameter lateral drain pipes that cover the remainder of the TSF interior floor. Two types of drain fill will be used for the impoundment underdrain. These include primary drain fill placed as an envelope around primary drain pipes, and a continuous minimum 18-inch thick layer of selected native drain material that covers the impoundment liner and contains the lateral pipe network. The primary drain fill will be produced by processing native gravelly sand to reduce its content of fine sand, silt and clay sized particles. The native drain fill material will consist of selected site soils (gravelly sand).

Scour protection will be placed at points of cyclone overflow discharge to protect the impoundment underdrain system. The scour protection will consist of locally derived coarse material cover over the underdrain, or the incorporation of energy dissipation measures on discharge spigots. Scour protection details are provided in Drawing 29. The specific number and type of scour protection required will be determined based on estimated cyclone overflow discharge volumes and flow velocities.

The dam underdrain system will consist of a minimum 18-inch thick layer of primary drain fill material and a network of 4-inch diameter internal drainage pipes.

##### 6.4.1.1 Impoundment Underdrain

Pipes in the impoundment and dam underdrains will be placed at a spacing that maintains minimum hydraulic head on the geomembrane liner and reduces the potential for leakage through the geomembrane liner. Pipe spacing is a function of the rate at which tailings drainage reports to the underdrain and the hydraulic conductivity of the drain fill.



Tailings placed in contact with the impoundment underdrain can be expected to rapidly consolidate and form a low permeability layer over the drain fill. Slurry consolidation tests indicate that the cyclone overflow that will be deposited in the impoundment interior will exhibit post-consolidation hydraulic conductivities ranging from  $5 \times 10^{-7}$  cm/sec for material deposited on the tailings beach, to  $5 \times 10^{-8}$  cm/sec for cyclone overflow slimes. Assuming a unit hydraulic gradient and an average tailings beach and slimes hydraulic conductivity of  $2.75 \times 10^{-7}$  cm/sec ( $9 \times 10^{-9}$  feet/second), the rate of drainage through the tailings and into the drain layer will be on the order of  $7.8 \times 10^{-4}$  feet/day/ft<sup>2</sup>. At final build-out with an impoundment floor area of 321 acres, total drainage collected in the impoundment underdrain will be on the order of 66 gallons per minute (gpm).

The drain pipe spacing is set to maintain a drain layer water depth and liner head that is less than the drain layer thickness of 1.5 feet. Because drainage into the impoundment underdrain will occur at a very low rate, the drain layer fill hydraulic conductivity can be relatively low and still maintain drainage and low liner head. Using the mound equation (Masada, 1988) and a hydraulic conductivity of  $3.8 \times 10^{-5}$  cm/sec for the native material drain fill layer, a spacing of 35 feet between impoundment lateral drain pipes will result in a maximum liner head of 1.33 feet. The spacing calculation assumes a 1 percent grade between drain pipes, which is the minimum grade on the TSF floor. Steeper slopes between drain pipes will reduce the head on the liner. Drainage mound and pipe spacing calculations are contained in Appendix D.1.

#### 6.4.1.2 Dam Underdrain

The dam underdrain constructed beneath the cyclone underflow sand fill will be subject to different conditions. The sand will be relatively permeable and the drainage rate will be variable because sand deposition locations will change frequently. The water in the cyclone underflow will be delivered to the dam at an average flow rate of approximately 1,042 gpm. In order to determine dam underdrain pipe spacing, the following assumptions were used to estimate drain inflow rates:

- Approximately 42 percent of the water deposited with the cyclone overflow will be retained in the tailings pore space. The remainder (approximately 58 percent) will either drain through the sand dam and report to the drain, or be lost to evaporation. Approximately 15 percent of the underflow water is assumed to be lost to evaporation. The resulting maximum flow of approximately 448 gpm will report to the dam underdrain.
- Dam construction is assumed to occur over an area of approximately 100 by 600 feet (60,000 square ft<sup>2</sup>).
- The slope between drain pipes is one percent.
- The dam underdrain fill (primary drain fill) will be a relatively clean fill, with approximately 20 percent finer than the No. 4 standard sieve, produced by crushing and/or screening of native gravelly sand. It is assumed to have a hydraulic conductivity of  $1 \times 10^{-1}$  cm/sec. A permeability test of prepared drain fill material with 50 percent finer than the No. 4 sieve exhibited a hydraulic conductivity of  $9.1 \times 10^{-2}$  cm/sec.





Based on these assumptions, the rate of application of cyclone drainage to the dam underdrain will be approximately  $5.42 \times 10^{-4}$  cm/sec (1.5 feet/day/ft<sup>2</sup>). Using the mound equation, a pipe spacing of 45 feet will result in a maximum head of 1.41 feet on the geomembrane liner. Drain layer thickness will be a minimum of 1.5 feet.

## 6.4.2 Drain Fill and Tailings Filter Compatibility

### 6.4.2.1 General Requirements for Drain Fill

Drain fill materials shall meet hydraulic conductivity and stability requirements. The dam underdrain fill (primary drain fill) shall be capable of retaining the cyclone underflow sand while allowing the transfer of drainage, i.e., without clogging. Inside the impoundment, the primary drain fill shall also be compatible with the underdrain pipe slot size and shall be capable of retaining the select native drain fill material while allowing tailing drainage to pass.

The select native drain fill layer shall meet a number of conditions, including:

- It shall be erosion resistant to control the potential for scour while temporarily exposed on impoundment slopes.
- It shall be compatible with the drain pipe slot size.
- It shall be capable of retaining the cyclone overflow slimes.
- It shall be retained by the primary drain fill.

### 6.4.2.2 Dam Underdrain Fill/Primary Drain Fill

Figure 3 shows the particle size distribution curve (PSD) for the cyclone underflow that will be used to construct the TSF embankment and will be in contact with the dam underdrain. The cyclone underflow filter envelope defines the range of drain fill gradations that are filter compatible with the cyclone underflow, i.e., materials that will restrict the migration of the cyclone underflow sand into the drain fill. It is assumed that this material will be prepared on site by reducing (screening out) a portion of the fine fraction of the native gravelly sand. A drain material with a minus 150-micron (No. 100 standard sieve) fraction of less than 10 percent and a minus 75-micron (No. 200 standard sieve) fraction of less than 5 percent is anticipated.

Figure 3 shows the estimated average PSD for the native soil composite samples following removal of the fine fraction with approximately 10 percent finer 425 microns (approximately 10 percent passing the No. 40 sieve). The modified gradation falls within the filter envelope for the cyclone underflow.

Type N-12 dual wall perforated, corrugated pipes have 3-millimeter (mm) wide slots for pipe diameters up to 10 inches. For broadly graded drain fill, Federal Emergency Management Agency (FEMA) (2007) recommends that the ratio of the D<sub>85</sub> particle size (the particle size for which 85 percent is finer) to the drain pipe slot width be greater than 4. To satisfy this condition, a D<sub>85</sub> of at least 12 mm is recommended.



Gravelly sand composite samples that were collected to represent materials available for drain construction meet the  $D_{85}$  requirement. When screened to remove fine particles, the  $D_{85}$  particle size will increase and the material will continue to meet the  $D_{85}$  size recommendation.

The primary drain fill that will be placed as an envelope around the primary drain pipes in the TSF interior will meet the same  $D_{85}$  size requirements for compatibility with the drain pipe slot size. The gravelly sand processed for the dam underdrain will also meet the requirements for primary drain fill.

For field production of the dam drain fill and primary drain fill during TSF construction, the minus 0.19 inch fraction (material finer than the No. 4 sieve) will be screened out to produce a granular drainage material. Table 7 presents the specification for the primary drain fill.

**Table 7: Primary Drain Fill Specification**

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	100-40
No. 4	0-20
No. 100	<10
No. 200	<5

Based on review of native soil gradation test results, a primary drain fill recovery rate of 40 to 60 percent is estimated during processing of available gravelly-sand soils. Fine rejects will be suitable for liner bedding fill material.

#### 6.4.2.3 Select Native Drain Fill

The cyclone overflow slimes represent the finest material that will be discharged into the impoundment and placed in contact with the impoundment underdrain. Figure 4 shows the PSD of slimes and the gradation of the filter envelope required to retain the slimes. The SD of the composite gravelly sand samples that are anticipated to be used for the select native material drain fill are also shown. The PSD intercept the slimes filter band in the critical  $D_{15}$  range. The select native drain fills will be capable of retaining the slimes. Some local clogging could be anticipated because the native material fines content is higher than required; however, the material is expected to function as needed because the drainage rate into the filter layer will be very low. Table 8 presents the gradation specifications for the select native drain fill.

**Table 8: Select Native Drain Fill Specification**

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	90-60
No. 4	80-40
No. 10	70-30
No. 100	10-30
No. 200	>10

If the native soils are crushed to minus 1 inch, the recovery of select native drain fill should be 100 percent during field processing for construction.

#### **6.4.2.4 Primary and Select Native Drain Fill Compatibility**

The primary drain fill shall be capable of retaining the select native drain fill, while allowing tailing drainage to pass from the select native fill into the primary drains. Figure 5 shows the filter envelope for the select native drain fill and the estimated PSD for the primary drain. The primary drain fill in the critical  $D_{15}$  to  $D_{50}$  range of the PSD falls within the select native drain material filter envelope and the two drain materials are expected to be filter compactible, i.e., the select native drain fill will not migrate into the primary drain fill.

#### **6.4.3 Drain Piping**

The primary drainage pipes in the impoundment underdrain will be 10-inch diameter Type N-12 dual wall perforated, corrugated polyethylene pipe (PCPE). The 10-inch PCPE pipe will be plain-end and joined with soil tight split couplings.

The lateral drain piping in the impoundment and dam underdrain will be 4-inch diameter Type N-12 PCPE pipe with plain ends and soil tight split couplings.

#### **6.4.4 Pipe Placement**

The primary drain pipes will be placed in a constructed channel or drainage swale that is a minimum of 16 inches deep and inside an envelope of primary drain fill as shown in Drawings 16 and 17. After the primary drain fill is placed, the lateral drain pipes will be placed on the liner surface and oriented to drain into the primary drain pipe channel. In the primary pipe channel, lateral pipes will lie in select native fill and run parallel to the primary drain fill envelope for a length of at least 4 feet. The select native drain material fill will be placed over and around the primary drain fill and lateral drainage pipes. The final cover of primary and select native drain fill placed over the primary pipes will be a minimum of 24 inches thick. Pipe placement is such that collapse or damage of an individual lateral drain pipe will not result in the transfer of tailings or select native drain fill into the primary drain pipes. The cover of select native drain fill



over the primary drain fill will prevent the transfer of soil or tailings from the lateral to primary pipe network.

Dam underdrain pipes will be placed within the drain fill and oriented to discharge outside the toe of the dam in the dam underdrain and runoff collection channel. As the dam is raised, the dam underdrain and runoff collection channel will be relocated and the dam underdrain pipes will be extended to the new perimeter collection channel.

#### 6.4.5 Drain Pipe Deflection

Deflection analyses were conducted to evaluate the performance of the underdrain pipes under the loads imposed by a tailings cover of approximately 240 feet. The Type N-12, dual walled PCPE pipes used for the underdrain collection system are considered to be flexible and can resist damage by distorting sufficiently to shed overburden loads to the surrounding underdrain fill.

As pressure on the top of the pipe is increased through an increase in tailings height, an increasing proportion of the vertical pressure on the pipe is transferred to the surrounding fill. This process is commonly called bridging. Therefore, the key parameter in assessing deflection of the underdrain collection system is the stiffness (modulus) of the fill in contact with the pipe. This bridging phenomenon was first accurately modeled by Burns and Richards in the paper *Attenuation of Stress for Buried Cylinders* (Burns and Richards, 1964). Golder has analyzed the pipe stresses and deformations based on the work of Burns and Richards and Hoeg (1968), with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression.

Golder analyzed the worst-case scenario with an entire column of tailings underflow cyclone sand of maximum height placed over the underdrain collection system. Based on Golder laboratory test results for the tailings underflow cyclone sand, a friction angle of 39 degrees and soil density of 120 pcf was used to model the tailings properties used in the analysis. The supporting fill was assumed to consist of the select native drain fill material with a similar stress versus strain relationship for a silty sand compacted to 80 percent of maximum dry density. The primary drain fill used beneath the dam and in the impoundment primary drains will have a reduced fines content compared to the select native drain fill, and will exhibit a strength that is at least that of the select native drain fill. Modeling deflection based on the select native drain fill is conservative.

Drain fill properties, along with pipe dimension and properties, were used to determine the maximum deformation that may be expected for the pipes. The maximum vertical pipe deflections are estimated to be between 11 and 14 percent. Golder's observations have been that pipe deflections greater than 15 to 20 percent often result in plastic deformation of the pipe at the springline. A vertical deformation of 15 percent is assumed as the maximum limiting deflection for flexible pipe. Therefore, the estimated



deformations of 11 to 14 percent are within the acceptable performance criteria established by Golder. Detailed data worksheets and calculations are provided in Appendix D.2.

#### **6.4.6 Drainage Outlet Works**

Tailing drainage from the impoundment underdrain will be routed beneath the tailings starter dam and cyclone sand dam to the TSF underdrain collection pond. Drainage outlet works details are shown on Drawings 16 and 17. The primary drain pipe network in the impoundment underdrain will be reduced to three pipes to transmit drainage to the outlet works. Three primary drainage pipes will be routed into a 12-inch diameter Type N-12 PCPE manifold at the upstream toe of the starter dam, which will in turn be connected to a 14-inch diameter, Schedule 80 carbon steel drain pipe. The steel drain pipe will be routed from the upstream toe of the starter dam to the TSF underdrain collection pond in a 42-inch deep by 42-inch wide ditch that will be backfilled with concrete.

The steel outlet pipe will pass through a valve vault. The valve vault will consist of a 72-inch diameter prefabricated concrete manhole base unit placed on an 18-inch thick reinforced concrete foundation mat. The manhole base unit will be fabricated with inlet and outlet openings for the steel drain pipe. Prefabricated manhole riser sections will be used to extend the valve vault vertically to a maximum height of 100 feet, to an elevation of approximately 5,300 ft-msl.

A 14-inch diameter, hydraulically actuated knife gate valve will be installed on the steel outlet pipe in the valve vault. The hydraulic actuator lines will be 100 feet long and will be routed up through the riser sections to the top surface of the dam for connection to a portable hydraulic power pack. As the manhole riser sections are added, the outlet valve hydraulic lines will be secured to the man-way ladder steps cast into the manhole riser sections. This arrangement will enable valve operation without manhole entry.

The purpose of the outlet valve is to prevent the drainage of excess water into the TSF underdrain collection pond in the early stages of impoundment operation (i.e., before the impoundment underdrain is covered with cyclone overflow). As positioned, the valve will be upstream of the main body of the dam, and its use will not result in pressurization of the underdrain pipe inside the dam. Once the underdrain is covered with cyclone overflow, the flow into the impoundment underdrain will be limited by the low hydraulic conductivity of the tailings slimes overlying the drain. When the dam reaches a height of 100 feet (approximately 5,300 ft-msl), the outlet valve will be fully opened and the valve vault will be backfilled with cement grout and granular fill materials.

### **6.5 TSF Underdrain Collection Pond**

#### **6.5.1 Pond Description**

The location of the TSF underdrain collection pond is shown on Drawings 2, 10, 11, and 12. TSF underdrain collection pond details and sections are shown on Drawings 13, 18, 19 and 20. Figure 6



illustrates total TSF underdrain collection pond capacity, the maximum operating water required to preserve upset and stormwater storage capacity, and the maximum stormwater storage level.

The TSF underdrain collection pond will be double-lined with minimum 60-mil HDPE geomembrane liners. An HDPE geonet will be placed between the liners to serve as the collection pond leakage collection and recovery system (LCRS) and minimize the head on the lower pond liner. The pond will be fitted with a primary drain material filled sump and LCRS pump to recover any leakage through the upper geomembrane.

TSF underdrain collection pond reclaim pumps will be submersible, vertical turbine pumps supported in a reinforced concrete sump and headwall structure. The sump will allow the water level in the pond to be drained to the pond floor level and no dead storage will be required. The use of submersible turbine pumps mounted in a concrete sump will eliminate the potential for liner damage associated with a barge mounted pump coming to rest on the pond floor. The reinforced concrete sump is shown in cross section on Drawing 19.

Impoundment underdrain flows will be transported to the pond via a buried steel pipe. Runoff and dam underdrainage will be routed to the pond via an HDPE lined open ditch constructed at the toe of the dam.

### **6.5.2 TSF Underdrain Collection Pond Sizing**

The TSF underdrain collection pond will contain drainage water from the TSF impoundment and dam underdrains, as well as runoff from the downstream face of the tailings dam. The pond is sized to contain 24 hours of tailing drainage flow at maximum estimated drainage rates, runoff from the 100-year, 24-hour storm event of 3.73 inches (National Oceanic and Atmospheric Administration [NOAA] 2006) incident on the downstream dam face, and an additional minimum 2-feet of freeboard. Underdrain flow rate calculations and runoff estimates are contained in Appendix E.

Underdrain flow rate estimates are based on the assumption that materials representative of the consolidated, cyclone overflow will be in contact with the impoundment underdrain and will control the rate of tailing drainage reporting to the TSF underdrain collection pond. The hydraulic conductivity of materials representative of beach and slimes samples are  $5.0 \times 10^{-7}$  and  $5.0 \times 10^{-8}$  cm/sec, respectively. If it is assumed that the more permeable beach-like material cover 60 percent of the impoundment underdrain at final build-out, the maximum underdrain flow rate will be on the order of 66 gpm.

Approximately 1,042 gpm of water will be delivered to the dam in cyclone underflow with a moisture content of approximately 30 percent. An estimated 42 percent of the water will be permanently bound or entrained within the pore space of the sand fill, and an additional 15 percent is estimated to be lost to evaporation. The remaining 28 percent (448 gpm) is assumed to report to the dam underdrain and TSF underdrain collection pond.



A storage allowance is provided for potential inflows associated with the free water pond coming in direct contact with the impoundment underdrain system. In this case, the drainage rate will be controlled by the hydraulic conductivity of the select native drain fill material that will cover the impoundment floor and drain pipe network. Permeability testing of a representative sample of select native drain fill indicated a hydraulic conductivity of  $3.8 \times 10^{-5}$  cm/sec. If it is assumed that a 20-acre area of impoundment drain will be inundated to an average depth of 2.5 feet, the estimated drainage rate will be on the order of 1,220 gpm.

The maximum contribution of stormwater runoff to the TSF underdrain collection pond will be from the combination of dam out-slope area and exposed toe area liner and underdrain occurring in Phase 4. The 100-year, 24-hour storm event incident on this area is estimated to produce a runoff volume of 3.94 million gallons.

Table 9 summarizes the TSF underdrain collection pond storage capacity requirements. The pond capacity is approximately 12.24 million gallons with 2 feet of dry freeboard below the crest of the pond (top of pond liner). The pond has the capacity to store up to approximately 5.8 million gallons of process water for facilitating process water make-up or storage of extra water during wet periods. A maximum water surface elevation of 5,157 should be maintained in order to provide sufficient storage for stormwater associated with the 100-year, 24-hour storm event and a coincident 24-hour upset period.

**Table 9: TSF Underdrain Collection Pond Storage Capacity Requirements**

Source	Type of Inflow	Volume (gal)
Dam Face Runoff	Storm Event Runoff	3,942,528
Dam Underdrainage	24-hour upset volume	645,206
Impoundment Underdrainage	24-hour upset volume	95,074
Free water pond direct drainage	24-hour upset volume	1,754,857
<b>Total</b>		<b>6,437,666</b>

## 6.6 Surface Water Management

### 6.6.1 Control of Impoundment Runon

The TSF will be required to contain inflows and direct precipitation associated with the 72-hour PMP of 26 inches. Diversion ditches constructed for impoundment runon control have been sized to carry the peak discharge associated with the prescribed PMP event using a rainfall intensity versus time distribution defined in Hydrometeorological Report 55A (US Department of Commerce, 1998). Runoff estimation and ditch sizing calculations are contained in Appendix F.1.

Diversion ditches will be constructed to divert runon away from the impoundment where possible. Peripheral catchment and runoff contributory areas are limited because the TSF lies in the head of a hydrologic catchment area. The Phase 1 grading plan (Drawing 11) indicates the location of diversion ditches. The Phase 1 ditches are located outside the Phase 2 construction area and will be functional



during Phases 1 and 2. In Phase 3, a permanent diversion ditch will be constructed on the west periphery of the TSF as shown on Drawing 11. Table 10 summarizes peak discharge estimates for the Phase 1 and 3 diversion ditches.

**Table 10: Summary of Impoundment Runoff Diversion Ditch Capacity and Size Requirements**

Phase/Location	Peak Discharge (cfs)	Ditch Width (ft)	Ditch Depth (ft) <sup>(3)</sup>
Phase 1, TSF northeast	525	10 <sup>(1)</sup>	5.5
Phase 1, TSF southwest	340	10 <sup>(2)</sup>	5.1
Phase 3/ TSF southwest periphery	205	10 <sup>(2)</sup>	4.2

**Notes**

<sup>(1)</sup> 2H:1V side slopes assumes on the downslope side, slope on upstream side varies

<sup>(2)</sup> 2H:1V slopes

<sup>(3)</sup> Depth at the lowest channel slope, includes 1 foot of freeboard.

### 6.6.2 Dam and TSF Underdrain Collection Pond Surface Water Management

Surface water management facilities other than impoundment diversions are designed to contain and transport flows associated with the appropriate 100-year storm event. Hydrologic calculations for the dam and TSF underdrain collection pond surface water management facilities are contained in Appendix F.2.

Runoff from the downstream face of the dam will be routed to the TSF underdrain collection pond. The time of concentration ( $T_c$ ) for the toe ditch catchment area is estimated to be three hours. The dam underdrain and runoff collection channel at the toe of the dam has been sized to carry the peak discharge associated with the 100-year, 3-hour storm. The 100-year, 3-hour storm will produce the peak 100-year storm runoff of 71 cubic feet per second. The flow depth at peak discharge is estimated to be a maximum of 0.5 feet. The perimeter toe berm height will be 3 feet (temporary berms) to 4 feet (permanent berms) high and will provide 2.5 to 3.5 feet of dry freeboard in the toe area dam underdrain and runoff collection channel.

### 6.7 Cyclone Plant Area

Excavation and site preparation will be required for the cyclone plant pad, the pump equipment pad and the surge pond. The cyclone plant general arrangement plan and site grading plan are shown on Drawing 21. The purpose of the cyclone plant is to separate whole tailings into sand and slimes fractions. Its' design and purpose are described in more detail in Section 7.0.

Surge pond cross sections and details are shown on Drawing 22. The purpose of the surge pond is to contain discharges (tailings, process, and reclaim water) from various processing locations under upset conditions, due to a pipe failure or shutdown of the cyclone plant. Upset flows from the cyclone plant will discharge by gravity to the surge pond within a secondary containment ditch lined with a minimum 60-mil HDPE geomembrane liner placed over 6 inches of liner bedding fill. Further details of the secondary containment ditch are provided below in Section 7.4.





## 7.0 TAILINGS DELIVERY AND DISTRIBUTION SYSTEM DESIGN

### 7.1 General System Description

The tailings delivery and distribution system design consists of pipeline system that delivers whole tailings from the processing plant to the tailings storage facility. Whole tailings will be separated into fine material and sand material in the cyclone plant. The sand fraction will be transported to the TSF and used for dam construction while fine material will be deposited into the TSF. The tailings surge system is designed for tailings management in case of unanticipated shutdown of any of the tailings stations or surges or overflows from station sumps. Return or reclaim water will be collected from the TSF surface pond and TSF underdrain water collection pond and transported back to the process plant. A general process flow diagram for the tailing delivery and distribution system is provided on Drawing 23.

Process equipment for the tailings delivery and distribution system will be located in four main stations as listed below:

- Cyclone Station: including the cyclone cluster, slurry pumps, slurry transfer sumps, gland seal water system, and electrical equipment;
- Surge Discharge System: including the surge pond evacuation pumps and lined secondary containment ditches;
- TSF Return Water Pond Barge Station: including a floating barge and barge mounted vertical turbine pumps and electrical equipment; and
- TSF Underdrain Collection Pond Pump Station: including vertical turbine pumps in a permanent structure and electrical equipment.

Tailings distribution will include whole tailings transport from the process area to the cyclone station and sand and fine tailings transport to the TSF. Return water will include tailing drainage water and TSF return water transported to the process plant. The major pipelines are listed below, and their interactions are shown in the overall system process flow diagram on Drawing 23.

- Cyclone Feed Line
- Cyclone Overflow Line
- Cyclone Underflow Line
- Cyclone Whole Tailings Bypass Line
- TSF Return Water Line
- TSF Underdrain Collection Return Water Line
- Main Surge Discharge Line

The major pipelines will be installed within secondary containment ditches lined with a minimum 60-mil HDPE geomembrane liner placed over six inches of liner bedding fill. The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23



NMAC, and will include secondary containment. Further details of the secondary containment ditches are provided below in Section 7.4.

The arrangement of the major components of the tailings delivery and distribution system is shown on Drawings 21 and 24. Drawings 25 and 26 present the tailings delivery and distribution system plan and profile. Whole tailings produced at the flotation plant will be transported via a 30-inch HDPE DR17 pipeline to the cyclone plant at the northwest side of the TSF. The cyclone plant will separate the sands fraction, which represents approximately 45 percent of the whole tailings stream, from the slimes fraction, which represents approximately 55 percent of the whole tailings stream. The sands fraction or the “underflow” of the cyclone plant will produce an underflow slurry which will be transported to the TSF in a separate 12-inch HDPE DR9 pipeline and discharged on the dam. The cyclone underflow sand placed on the dam crest will be then be spread, graded and compacted, as necessary to push sand down the dam out-slope and continually build the dam.

The cyclone overflow (slimes) from the cyclone plant will be routed to the TSF in a separate 30-inch HDPE DR17 pipeline and discharged into the impoundment. The cyclone overflow water will be returned to the process water reservoir at a rate of up to 13,000 gpm. The cyclone plant will operate continuously to produce the sand material needed for continuous construction of the dam. In the event of upset conditions when the cyclone plant is not in operation, whole tailings will be discharged via gravity into the surge pond through a lined secondary containment ditch (see Section 7.4).

## 7.2 Tailings Delivery

### 7.2.1 Underflow Sand

The cyclone underflow pipeline will deliver sand to the top of the embankment for tailings dam construction. Two underflow pipelines will be used. The east leg will be routed around the north side of the TSF, and the south leg will be routed around the south side of the TSF as shown on Drawing 24. Each leg is sized to transport 100 percent of the cyclone underflow at up to 45.6 percent sand recovery. This allows for 100 percent availability of sand delivery to the dam.

Cyclone underflow will be discharged through 4-inch spigots placed every 333 feet. Each spigot will include one 4-inch manual pinch valve. The underflow pipelines will also have in-line knife-gate isolation valves every 2,000 feet to allow for isolation and relocation of the pipe as the dam rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pick-up truck.

The north and south cyclone underflow pipelines will be operated independently. When one is in operation, the other can be serviced or broken down and relocated. Cyclone underflow pipes will be flanged each 500 feet to facilitate breakdown and relocation.



## 7.2.2 Cyclone Overflow

Two cyclone overflow delivery pipelines, one leg to the north side and one leg to the south side of the TSF, will transport the cyclone overflow to the TSF interior (impoundment interior) as shown on Drawing 24. The cyclone overflow will be discharged via spigots placed every 667 feet. Each spigot will include a manual pinch valve. Each pipe is sized to carry 100 percent of the cyclone overflow to permit pipeline relocation without interrupting operation as the TSF elevation rises. One leg will remain active while the other is serviced or relocated.

The cyclone overflow pipelines will also have knife-gate isolation valves placed every 2,000 feet to allow for isolation and relocation of the pipe as the impoundment rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pickup truck. The cyclone overflow delivery pipelines will be flanged every 500 feet to allow for breaking down and relocating the pipe.

## 7.3 Deposition Management

### 7.3.1 Dam Construction

Figure 7 illustrates height versus capacity and surface area relationships and rate of rise for the tailings impoundment. Near continuous operation of the cyclone plant will be required to produce sufficient sand to construct the dam to the ultimate elevation of 5,460 ft-msl. The difference in elevation between the dam crest and the head of the cyclone overflow impoundment beach will be maintained at 10 feet. Maintenance of this elevation difference will place the transition between the cyclone underflow sand dam fill and the interior cyclone overflow a distance of approximately 30 feet upstream of the inside dam crest. The elevation differential of 10 feet will be maintained in order to maintain adequate freeboard for stormwater storage. Maintaining this elevation differential also allows for maximum storage capacity for cyclone overflow, and thus maximum production of sand needed for dam construction.

In the early stages of operation, there will be more sand available than needed to maintain the elevation differential between the dam crest and the head of the beach. Excess sand will be pushed down the out-slope of the dam and used to construct the dam base. In the later stages of operation, the sand previously used to construct the dam base will reduce the sand requirements for raising the dam crest, and facilitate maintaining the crest to beach elevation differential.

### 7.3.2 Cyclone Overflow Discharge

The storage volume for the cyclone overflow will be maximized through managing deposition and practicing sub-area, thin lift deposition. Discharge spigot locations will be frequently cycled so that a thin lift of tailings is placed on the tailings beach. Exposure to evaporation prior to burial with a subsequent lift of tailings will allow the tailings to desiccate and consolidate. The degree to which thin lifts can be placed



and consolidation can occur under managed deposition is primarily a function of rate of tailings rise. It is also influenced by tailings properties, climatic conditions, surface water management, and operator effort.

#### 7.4 Management of Upset Flows

Potential upset flows from the process area, cyclone plant, and TSF will be controlled through a series of secondary containment ditches, the surge pond, and the TSF underdrain collection pond (see Section 6.5). The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23 NMAC. The secondary containment ditches will run from the process area to the TSF (the main ditch), from the main ditch to the cyclone area, and from the cyclone area to the surge pond. The secondary containment ditches are designed to contain and transport flows via gravity that are related to potential upset conditions and direct precipitation onto the ditches associated with the 25-year 24-hour storm event (2.88 inches). Maximum upset flow conditions would be associated with overtopping of the process water reservoir (as estimated by M3, the design contractor for the process water reservoir). This maximum upset flow was assumed to be 18,000 gpm over a 30-minute period, at which point the process area pumps would be shut down. The secondary containment ditches are designed for these maximum upset flows, direct precipitation, and an additional 2 feet of freeboard. The main ditch is designed to flow to the TSF by gravity for the first six years. After year six, gravity flow to the TSF is no longer possible because of the increased height of the TSF and upset flows will then discharge to the surge pond via gravity in a lined ditch through year 11.1. The alignment of the secondary containment ditches is shown on Drawings 2, 3, 10, 11, 12, 21, and 24 through 26. Details of the secondary containment ditches are provided in Drawing 29.

Surge pond cross sections and details are shown on Drawing 22. The surge pond liner system will consist of a liner bedding fill layer overlain with a minimum 60-mil HDPE geomembrane liner. The surge pond is located at an elevation of 5,340 feet and is sized for a surge retention time of half an hour with an additional reserve capacity of over one million gallons. The pond is sized for the retention of approximately 1,610,000 gallons of slurry with an additional 2 feet of freeboard. The use of the surge pond will be intermittent and temporary and the pond will be empty under normal operating conditions. The pond will be equipped with dedicated hard-wired pumps that will automatically evacuate its contents. Emergency power for the pumps will be provided by the emergency diesel power generation system located on-site in the event of a power outage. The process facility control room will be equipped with emergency alarms that notify the operator of an upset condition allowing the operator to make necessary adjustments in the process, as needed. The pumps at the surge pond will be automatically activated upon the pond reaching a predetermined level. Water and solids collected from the surge pond will be discharged through a 12-inch HDPE DR17 pipeline to the top of the TSF. The solids handling pump is designed to evacuate the surge pond within 12 hours.



## 8.0 WATER RECLAIM SYSTEM DESIGN

The water reclaim system is a significant part of NMCC's water conservation program. It will provide approximately 75 percent of the water used in the process in the form of recycled water. The purpose of the water reclaim system is to recycle supernatant water stored in the TSF and water captured in the underdrain collection gallery and stored in the TSF underdrain collection pond. The TSF water reclaim system will recover water released as the cyclone overflow consolidates within the TSF. The underdrain water collection system will recover water from the bottom of the tailings impoundment through the TSF underdrains into the TSF underdrain collection pond. The underdrain pond will also store water captured by the dam underdrains from the downstream side of the dam as sand is deposited and compacted as well as precipitation run-off water from the face of the dam. All of this water will be transported from the TSF and TSF underdrain collection pond to the process water reservoir located at the plant via a 20-inch HDPE variable DR return water pipeline. Water from the TSF underdrain collection pond may also be pumped back directly to the TSF. The major components of the water reclaim system are shown on Drawing 27. Water reclaim system details are shown on Drawing 28.

The TSF reclaim system will be a barge-mounted pump station in the impoundment equipped with four pumps (three operating and one spare) with a design flow from this station of 13,000 gpm. This is equal to the maximum design rate at which water will be delivered to the TSF from the cyclone plant during normal operation.

The TSF underdrain collection pond system will be a pump station of two pumps (one operating and one spare) installed in a sump within the pond. The design flow from this station will be 4,000 gpm. This is the maximum design flow of the TSF underdrain collection gallery. It is anticipated that up 4000 gpm of flow will be captured by the underdrains in the initial stages of operation of the TSF. This flow rate will become less over time as the tailings impoundment fills and the underdrains become overlain with tailings materials.

The water reclaim barge in the TSF impoundment will be accessed from a ramp constructed over the impoundment liner as shown on Drawing 28. The ramp will be approximately 35 feet wide and initially constructed to a height of 10 feet above the impoundment floor. As the tailings level rises during operations, the position of the barge will migrate up the ramp northwestward along the reclaim pipeline alignment. In each construction phase, ramp construction will be completed with borrowed structural fill material.



## 9.0 WATER BALANCE

Water balance calculations are included in Appendix G. Figure 8 summarizes the results of a water balance analysis of the proposed TSF for average rainfall conditions. The water balance model incorporates water input from slurry water inflow, direct precipitation on the impoundment surface and runoff from un-diverted upgradient areas. On-site meteorological data collection at Copper Flat was initiated in August 2010. Due to the short duration of record keeping and recording gaps, NOAA data from Hillsboro and Caballo Dam, New Mexico, were used in conjunction with Copper Flat data to estimate monthly precipitation and evaporation rates. The ratio of site evaporation to Hillsboro evaporation for months where data are available from both sites was used to estimate site evaporation for months where data were not collected at the site.

Water balance model losses include entrainment of water within the tailings solids, evaporation from the TSF supernatant pool, evaporation from the exposed tailings beach, and evaporation of water from the dam. Entrainment represents the most significant water loss and is calculated on the basis of the estimated final post-deposition dry density of the cyclone underflow and cyclone overflow. An average post-deposition dry density of 57 pcf is assumed for the cyclone overflow. Over the life of the facility, approximately up to 49 million tons of cyclone underflow will be produced assuming near constant operation of the cyclone plant and mill.

The water balance model does not identify reclaim rates from specific locations because operation of the impoundment will impact where water accumulates. Water that is not lost to evaporation or bound within the tailings is assumed to be recovered from either the impoundment free water pond or the TSF underdrain collection pond.

The impoundment underdrain will be equipped with a shutoff valve at its inlet during the initial years of operation so that when the water level in the TSF underdrain collection pond exceeds the normal operating level, the underdrain will be closed to utilize the TSF supernatant pool for storage and the TSF undergrain collection pond will be pumped down.

A total process water inflow of 12,978 gpm is estimated based on average operating conditions. This inflow includes water contained in the cyclone overflow slurry and water delivered to the dam with the cyclone underflow. As shown in Figure 8, the estimated process water reclaim rate averages 9,215 gpm. The average make-up water requirement, calculated as the difference between the process water delivered and the water reclaimed, is estimated to be 3,169 gpm or approximately 152 gallons per ton of tailings placed in the TSF. The maximum estimated make-up water rate is 3,676 gpm.

The water balance examines water reclaim rates for average rainfall conditions. If the site experiences precipitation that is less than or exceeds average conditions, water reclaim rates and make-up water



requirements can also be expected to vary. The water reclaim system is capable of recovering water at a rate adequate to account for all water in the tailings slurry discharged from the flotation plant. Maximum reclamation of water following storm events can temporarily reduce demand on external water sources. The water balance does not consider additions from the open pit or waste rock stockpile stormwater ponds. Water available from other on-site sources is not expected to be significant.



## 10.0 TAILINGS DAM STABILITY ANALYSIS

### 10.1 Methods

Slope stability analyses were performed in support of the feasibility design of the Copper Flat TSF with an ultimate crest elevation of 5,435 ft-msl. The analyses were performed using limit-equilibrium slope stability software and Spencer's (Spencer, 1967) method of slices to compute the theoretical factors of safety (FOS) for various potential failure surfaces. Material properties used in the stability calculations for native soil and tailings materials were based on laboratory geotechnical testing performed in Golder's in Denver, Colorado laboratory and discussed previously in Sections 3.3 and 4.3.

Slope stability analyses were conducted to determine the FOS against failure for the critical stability section along the highest embankment section and most adverse subsurface topography on the downstream slope. The critical factors of safety assumed for stability analysis are 1.5 for static conditions and 1.1 for pseudo-static conditions, according to NMAC 19.25.12.11.12. Stability analyses were performed for static (steady-state) and dynamic (pseudo-static) loading conditions. Steady-state loading conditions represent the long-term stability of the TSF and pseudo-static loading conditions represent the stability of the TSF during the design earthquake loading event.

The computer package SLIDE™ (Version 6.021) was used to conduct the stability analyses (Rocscience, 2013). An arcuate (circular) failure mode was used to analyze the critical section with the shear surface failing through the tailings and/or foundation materials. A block failure mode was used to analyze the critical section with a potential failure at the liner interface. Both methods are based on the principle of limit equilibrium, i.e., the method calculates the shear strengths that would be required to maintain equilibrium, and then calculates the FOS by dividing the available shear strength by the shear strength required to maintain stability.

Analyses were limited to the investigation of global failures that can affect the full height of the embankment, are deeper seated, and may result in lowering the embankment crest and loss of containment. Local stability analyses associated with shallow slope failures were not investigated.

A pseudo-static analysis approach was used for the seismic loading case. With this method, a lateral force is added to a potential failure mass, with magnitude equal to some fraction of the weight of the sliding mass. The fraction is defined in the form of a pseudo-static coefficient and is expressed as a percentage of gravity. Selection of the pseudo-static coefficient is discussed below. Stability analysis supporting data and computer-generated outputs are contained in Appendix H.





## 10.2 Seismic Design Criteria

According to the regulations set by the NMDSB, the TSF can be classified as having a significant hazard potential. Dams assigned the significant hazard potential classification are those where failure results in no probable loss of human life but can cause economic loss, environmental damage, and/or disruption of lifeline facilities. The NMDSB requires that structures such as the Copper Flat TSF be designed to withstand the seismic loading from the Maximum Design Earthquake (MDE) with a 2 percent probability of exceedance in 50 years (approximately 2,475-year return frequency). The peak ground acceleration (PGA) for the Copper Flat property was obtained using the US Seismic “Design Maps” Web Application developed by the United States Geological Survey (USGS) Geologic Hazards Science Center (USGS, 2011). Considering the 2009 National Earthquake Hazards Reduction Program provisions for a Site Class C and a site location of 32.96° North latitude and 107.5° West longitude, the resulting PGA for the 2,475-year return MDE is approximately 0.13 times gravitational acceleration (0.13g).

The method developed by Hynes-Griffin and Franklin (1984) and Jansen (1985) was used to evaluate pseudo-static loading conditions, as outlined by the NMDSB guidelines (2010). This method recommends that the pseudo-static coefficient selected for analysis must be at least 50 percent of the predicted PGA, but not less than 0.05g, and the FOS under pseudo-static analysis should be 1.1 or greater. A coefficient of 0.087g, corresponding to two-thirds of the design PGA, was conservatively used for the analyses.

The results of the previous seismic liquefaction potential evaluation of the Copper Flat TSF by SHB (1980) indicated that the probability of liquefaction affecting the Quintana TSF was extremely remote based on the seismic hazard potential of the site and empirical data derived from case histories of tailings dams and natural, saturated, loose sandy deposits subject to earthquake-induced ground motions. Golder anticipates that a drained response to seismic loading will dominate the pore water conditions in the TSF during and following its active life based on the material permeabilities, boundary drainage conditions, and construction practices. Given the seismic hazard potential of the site and the proposed TSF construction and operating practices, the liquefaction potential of the new TSF is also considered low.

## 10.3 Tailing Drainage and Phreatic Conditions

The primary source of tailing drainage is drain-down water associated with the centerline-constructed cycloned underflow tailings portion of the embankment, with a minor contribution from consolidation and drainage of the impounded cyclone overflow upstream of the embankment crest. The drainage control system consists of a continuous granular fill blanket drain beneath the cyclone underflow sand embankment and toe area, with lateral underdrain pipes spaced at 45-foot centers across the embankment footprint. Lateral drain pipes will connect to an open channel, which gravity drains to the HDPE-lined TSF underdrain collection pond.



The phreatic surface was assigned to be coincident with the drainage control system at the base of the final embankment, based on performance of similar structures. Because of the large difference in the hydraulic conductivity of the cyclone underflow and cyclone overflow, a near vertical phreatic surface is assumed at the dam/beach interface. This condition will result in the main body of the dam being well-drained and unsaturated.

#### 10.4 Pore Pressure Conditions and Liquefaction Potential

The response of tailings material to loading can be either drained or undrained, and is associated with the development of pore water pressures. For stability analyses, a phreatic surface was assumed at the beach material surface level upstream of cycloned underflow tailings embankment and an undrained response was evaluated. Undrained analyses were performed by applying an undrained strength to the cyclone overflow on and beneath the beach.

Susceptibility to liquefaction potential is assumed to be limited to the saturated beach areas upstream of the dam. Steady-state (residual) undrained strength was applied to the tailings beach upstream of the cyclone sand dam fill to evaluate post-liquefaction stability.

The risk of static and seismic liquefaction triggering will likely be low if appropriate control over embankment construction and the phreatic-surface elevation is exercised, and the cyclone underflow sand behaves as anticipated. Sand placed on the dam crest will be spread and compacted. Operating experience at mine sites utilizing cyclone underflow fill indicates that self-weight compaction of sand on the tailings out-slope is typically adequate for minimizing liquefaction potential. At Copper Flat, some compaction will be realized on the dam out-slope as a result of dozer spreading operations.

#### 10.5 Material Properties

The material properties used in the stability analyses are based on a review of the properties used in previous site studies (SHB 1980), new data derived from testing of samples collected during the site geotechnical investigation, and from tests conducted on tailings characterization study samples. The components of the slope stability model include:

- Foundation Materials
- Liner Interface Zone
- Cyclone Underflow sand
- Cyclone overflow (beach material)
- Structural Fill

Table 11 summarizes the strength parameters used in the preliminary slope stability analyses.

**Table 11: Summary of Properties Used in Stability Analyses**

Component	Unit Weight (pcf)	Drained Strength		Undrained Strength ( $S_u/\sigma'_v$ )
		Cohesion (psf)	$\phi'$ (degrees)	
Foundation Materials	120	150	29	NA
Liner Interface Zone	120	0	26.5	NA
Cyclone Underflow	113	0	39	NA
Cyclone overflow	108	NA	NA	0.05
Structural Fill	120	0	29	NA

**Notes:**

pcf = pounds per cubic foot

psf = pounds per square foot

 $\phi'$  = Effective stress friction angle $S_u/\sigma'_v$  = Residual undrained shear strength normalized by the effective overburden stress

NA = Not applicable

**10.5.1 Foundation Materials**

The foundation underlying the proposed TSF embankment has been characterized as an alluvial deposit comprised of predominately silty and clayey sands and gravels. Foundation material strength test results ranged from 28 to 32 degrees (effective friction angle). Based on the results of consolidated-undrained triaxial tests completed by Golder, an effective stress friction angle ( $\phi'$ ) of 29 degrees and effective cohesion of 150 pounds per square foot were applied for the shear strength of the foundation materials.

**10.5.2 Liner Interface Zone**

The shear strength of the cycloned underflow tailings/geomembrane liner/liner bedding interface will be the controlling factor for possible sliding block-type failure surfaces along the base of the TSF embankment. The interface shear strength was evaluated in a direct shear test performed by Golder. Old (Quintana) tailings, and drain fill materials were placed in contact with a sample of textured, 80-mil HDPE geomembrane. The composition of the test sample is representative of the Phase 1 interface, when the old tailings will be utilized for liner bedding fill. The use of coarser, higher strength materials (crushed and screened native gravelly sand) is anticipated in later construction phases. The direct shear test indicated an interface friction of 26.5 degrees. This strength was assigned to the interface zone comprised of liner bedding, geomembrane and drainage materials.

**10.5.3 Cyclone Underflow**

The sloping, cyclone underflow embankment was considered to be fully drained and cohesionless because of the anticipated low phreatic-surface elevation and the high permeability of the cyclone underflow in the embankment, and the effect of the drainage control system. Mittal and Morganstern (1975) evaluated the  $\phi'$  for cycloned copper tailings sands. The effects of particle crushing and sand dilatancy were most pronounced at stresses up to 40 psi, while at higher stresses  $\phi'$  remained relatively constant and approximately equal to 34 degrees. Volpe (1975) reported  $\phi'$  values for copper sands and slimes between 33 to 37 degrees. A consolidated-undrained triaxial test conducted by Golder on a



cyclone underflow sample indicated an internal friction angle of 40 degrees. An effective friction angle of 39 degrees was assumed in stability analyses.

#### **10.5.4 Cyclone Overflow**

Cyclone overflow in the impoundment is assumed to exhibit a drained response for most loading conditions except for the conditions occurring after static or seismic liquefaction. However; for conservatism, an undrained shear strength behavior was assumed for the cyclone overflow upstream of the embankment.

Undrained shear strength implicitly accounts for the effects of shear-induced pore pressures. Based on experience from similar mining projects and published data (Mittal and Morganstern 1975, Vick 1990), the peak undrained shear strength normalized by the effective overburden stress ( $S_u/\sigma'_{vo}$ ) was estimated to be 0.20 for the impoundment area extending upstream from the embankment crest.

Residual undrained shear strengths were used in a static analysis to evaluate embankment stability following liquefaction in the beach area. Similar to that for peak shear strength, the residual undrained shear strength normalized by the effective overburden stress ( $S_u/\sigma'_{vo}$ ) was estimated to be 0.05 for the tailings upstream of the cyclone underflow dam.

#### **10.5.5 Structural Fill**

Strength parameters for the starter dam structural fill are based on testing of representative materials recovered from the TSF borrow areas during the site exploration. Foundation material strength test results ranged from 28 to 32 degrees. The material was conservatively classified as cohesionless with a  $\phi'$  of 29 degrees with a moist unit weight of 120 pcf.

#### **10.5.6 Liner Material**

The liner interface direct shear test was conducted with a sample of textured geomembrane. The use of un-textured geomembrane may be feasible, but testing of interface strength with un-textured liner has not been performed. To evaluate the potential for use of smooth liner, the interface strength was varied to find the interface friction angle required to meet the minimum pseudostatic FOS of 1.1. This analysis was performed for the block failure mode.

#### **10.5.7 Fissured Clay Foundation Analysis**

Stability analyses presented in Section 10.6 for the maximum embankment section address the embankment, the cyclone underflow beach and the liner interface zone on a maximum height embankment section. Additional sections corresponding to geology sections B-B' and D-D' were evaluated to determine the effects of clay foundation soils on the stability of the TSF.



Drill holes in the vicinity of these sections intercepted a clay layer that appears to be dipping to the eastward based on the first high plasticity clay intercepts identified during drilling. At several locations in the TSF expansion area, the top of this clay layer exhibited characteristics of a softened clay, with locally high moisture content and corrected SPT blow counts, in the range of 12 to 25, that were lower than those in overlying and underlying soils.

Stark and Eid (1977) state that fissured clays in first time slides (or first time slope failures) may exhibit a mobilized shear strength that is lower than the strength of fully softened clay, and suggest the use of the average of the fully softened and residual (large strain) clay strength in evaluating stability. Clay shear strength was estimated based on an empirical method presented by Mesri and Shahien (2003) that relates shear strength under varying normal stress to plasticity index. The method provides a non-linear shear strength envelope used to estimate the fully softened, residual and average strength of hard, fissured clays. The resulting shear strength versus normal stress envelope is used for clay strength input in the slope stability model.

The highest plasticity index ( $I_p$ ) of 42 percent in Copper Flat samples, which was associated with a softened clay sample recovered from drill hole BH-18 at a depth of 43 feet, was assumed for estimating the strength of the high plasticity clay layers. In section B-B', clay interbedded clay and granular soil layers were modeled. In Section D-D', the clay layer was assumed to extend from the first high plasticity clay intercept to the base of the model section.

## 10.6 Stability Analysis Results

The results of stability analyses for static and pseudo-static loading conditions are summarized in Table 12.

**Table 12: Calculated Slope Stability Factors of Safety**

Failure Mode	Method	Static FOS (Global)	Pseudostatic FOS (Global)
Maximum Section Circular	Spencer	2.53	1.92
Maximum Section Block	Spencer	2.24	1.69
Maximum Section Circular, Post Liquefaction	Spencer	2.53	NA
Maximum Section Block, min required interface strength = 13.6 degrees	Spencer	1.53	1.1
Fissured Clay Section B-B' Circular	Spencer	1.56	1.12
Fissured Clay Section B-B' Block	Spencer	2.50	1.90
Fissured Clay Section D-D' Circular	Spencer	1.53	1.13
Fissured Clay Section D-D' Block	Spencer	2.48	1.87

**Notes:**

FOS = factor of safety

NA = not applicable

Min = minimum



The conservative assumptions applied in the stability analyses suggest that the Copper Flat TSF will be stable. All factors of safety meet or exceed the minimum NMDSB requirements of 1.1 and 1.5 for static and pseudostatic conditions. The residual strength analysis suggests if liquefaction of saturated tailings upstream of the dam occurs, the embankment will remain stable. The evaluation of the sensitivity of pseudostatic stability to the friction angle of the liner interface indicates a relatively low interface friction angle is required to maintain stability, and that the required interface strength is likely to be achievable with an un-textured geomembrane liner.



## 11.0 TAILINGS DAM FOUNDATION SETTLEMENT POTENTIAL

### 11.1 Analysis Approach

The TSF will consist of an earthen starter dam constructed to a height of approximately 50 feet with the remainder of the dam constructed with sand recovered from the cyclone plant. A geotechnical investigation was performed in the embankment footprint, which included standard penetration testing and sample collection from the surface to a depth of 50 feet. Drilling indicated that in general, the tailings embankment foundation consists primarily of alluvial deposits that include silt, sand and gravel, which are underlain by clay.

Representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Selected samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because settlement of the embankment will be adequately mitigated by continuous fill placement during ongoing embankment construction. Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc., for the analysis of settlement and consolidation under foundations and embankments.

A detailed description of the settlement potential investigation, settlement calculations and supporting information are contained in Appendix I.1. Drill holes and the location of cross-sections used to evaluate subsurface conditions are shown on Drawing 3. Drawings 5 and 7 present geologic cross sections B-B' and D-D', respectively, which were developed to evaluate settlement perpendicular to the dam axis. The cross-sections also include information derived from the former geotechnical study conducted on behalf of Quintana by Sargent Hauskins and Beckwith (SHB, 1980). Drill hole logs are contained in Appendix A.2.

A differential settlement and geomembrane strain analysis was subsequently conducted by Golder and is included in Appendix I.2. Cross sections were developed to intercept the various geologic materials underlying the TSF site. The engineering properties of the foundation materials were derived from the 1980 Sargent, Hauskins and Beckwith (SHB) geotechnical study, the geotechnical investigation conducted as part of the TSF design report and experience with similar foundation materials.

### 11.2 Settlement Potential Analysis Results

Laboratory consolidation testing was conducted on remolded specimens of the fine fraction of samples recovered from the embankment foundation. As such, the settlement prediction does not account for the presence of the coarse fraction in the foundation soils, and associated inter-particle contact and support of



foundation loads. Settlement predictions based on the laboratory consolidation tests are therefore conservative.

Results of the settlement potential analysis are shown graphically on geologic sections B-B' and D-D'. The maximum calculated settlement beneath the embankment is approximately 2.1 feet in the area of the maximum dam (and tailings beach) foundation loads. Settlement decreases at a relatively uniform rate as the weight of post-construction loading decreases towards the outer toe of the embankment.

Settlement prediction based on the laboratory consolidation testing of the fine fraction of foundation samples is conservative. SPT testing conducted during drilling showed the foundation strata to generally be very dense to hard. On the basis of SPT test results, actual post-construction consolidation settlement of less than 1 foot is anticipated.

Dam construction will be more or less continuous during the life of the facility. The effects of foundation settlement include the potential for the loss of dry freeboard for stormwater storage. The potential loss of freeboard can be mitigated by elevating the dam crest with managed/targeted placement of cyclone underflow sand.

The analyses did not indicate the potential for differential settlement that could impact the integrity of the TSF geomembrane liner. Sections B-B' and D-D' indicate predicted settlement varies uniformly across areas subject to changing foundation loads.

The impoundment underdrain will pass beneath the dam in a steel pipe placed in a ditch backfilled with concrete near section F-F' (Drawing 9). The settlement will not adversely impact the impoundment underdrain outlet pipe. There is adequate grade and elevation change along the outlet pipe alignment to accommodate predicted settlement.

A basalt outcrop identified by SHB (SHB, 1980) may lie beneath or in the vicinity of the impoundment underdrain pipe inlet near the upstream toe of the dam. The outcrop occurred in an area that was disturbed during Quintana dam construction activities, and was not observed during the recent site exploration. If the inlet to the underdrain pipe bears on basalt, local differential settlement could occur along the pipe alignment, which could induce stress on the outlet pipe. If, during construction, a basalt outcrop is identified at the location of the inlet, an alignment change may be warranted to avoid the pipe bearing on basalt.

It should be noted that the settlement potential investigation was performed for a previously completed design study, and evaluated an embankment geometry that differs from that presented in this report. The new embankment is higher and the depth of embankment fill overlying the foundation is greater for this 30,000 tons per day design; however, the original analyses assumed a higher, more conservative embankment moist unit weight of 130 pcf. Tailings testing completed after the settlement potential study





was conducted indicates a post embankment fill placement moist unit weight of approximately 113 pcf. The foundation loads imposed by the higher embankment fill, when corrected for the moist unit weight determined by laboratory testing, are lower than those used in the settlement potential analysis. Therefore, the results of the settlement investigation presented above are conservative relative to the current design. As part of future detailed engineering studies, settlement calculations will be updated for final design conditions; however, the conclusions are anticipated to be consistent with those presented herein.

The results of the differential settlement and geomembrane strain analysis indicates that, in general, settlement potential across the TSF is predicted to be limited. As such, the potential for tearing of the HDPE liner due to potential differential settlement within the entire area of the TSF is considered to be low. The maximum settlement is estimated to be 0.72 feet, while the maximum tensile strain on the HDPE liner due to differential settlement is estimated to be 0.02 percent. The allowable tensile strain on an 80 mil HDPE geomembrane liner is 10 percent and the predicted tensile strain is well within acceptable conditions. Therefore, Golder does not expect tearing of the HDPE liner due to differential settlement to be an issue.



## 12.0 USE OF THIS REPORT

This feasibility level design report has been prepared by Golder exclusively for the use of THEMAC and NMCC. No third-party engineer or consultant shall be entitled to rely on any of the information, conclusions, or opinions contained in this report without the written approval of Golder, THEMAC or NMCC.

The conclusions and recommendations in this report have been prepared in a manner consistent with the level of care and skill ordinarily exercised by engineering professionals practicing under similar conditions, subject to the time limits and financial and physical constraints imposed on or otherwise applicable to the work.



## 13.0 REFERENCES

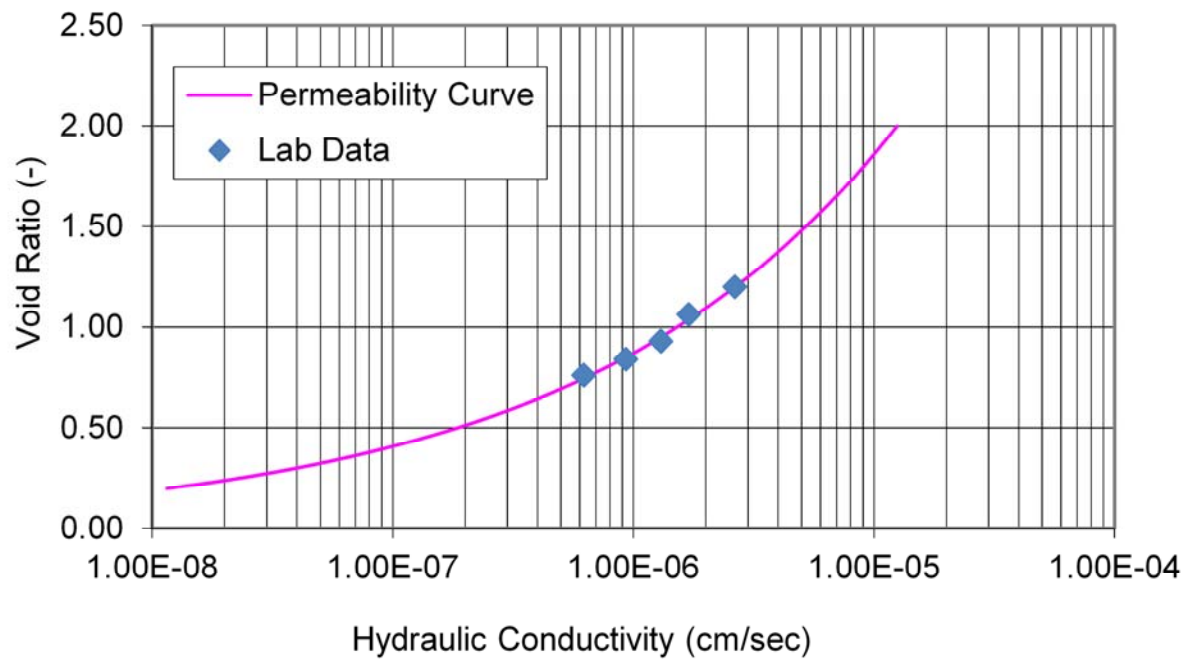
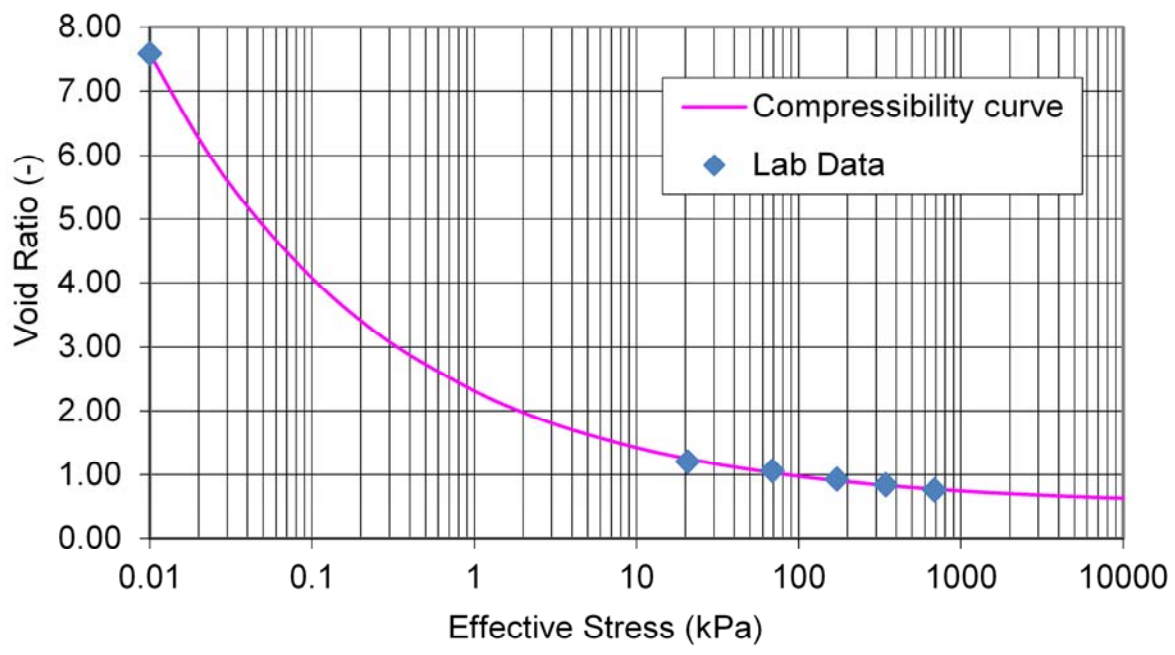
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## FIGURES

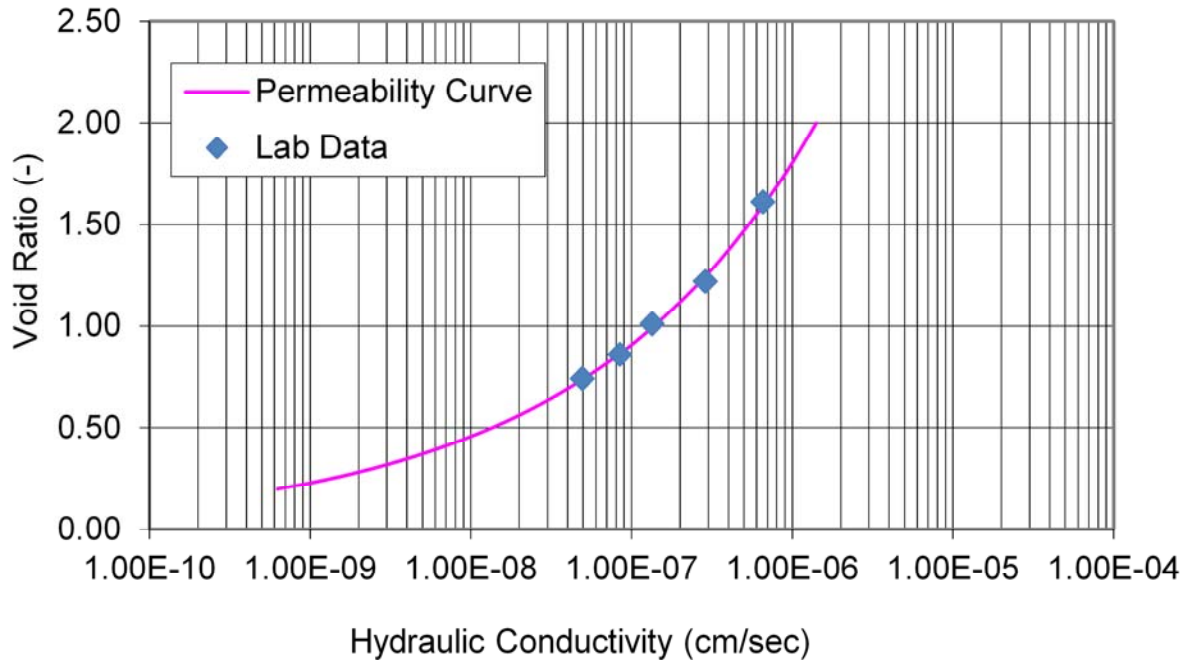
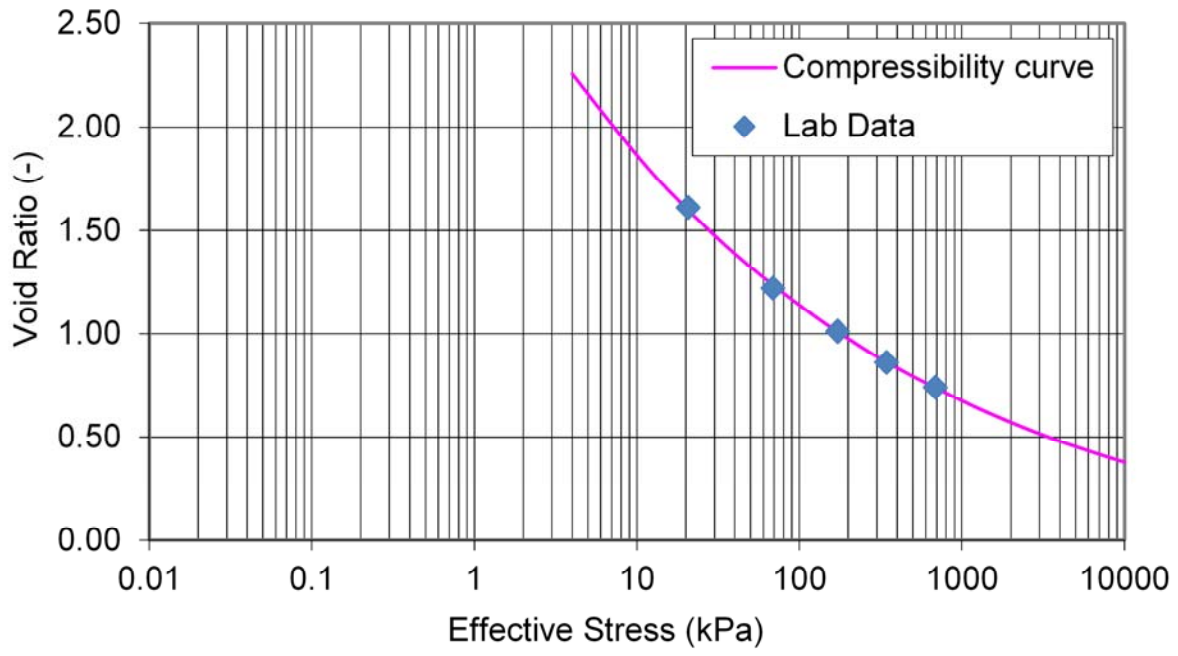
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<p>PROJECT</p> <p><b>THEMAC</b> RESOURCES</p> <p><small>Environmentally Responsible. Community-Minded. Local Opportunities.</small></p>	<p>NEW MEXICO COPPER CORPORATION</p>	<p>DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>
<p>TITLE</p> <p><b>COMPRESSIBILITY AND PERMEABILITY VS. VOID RATIO THICKENED OVERFLOW- BEACH</b></p>		
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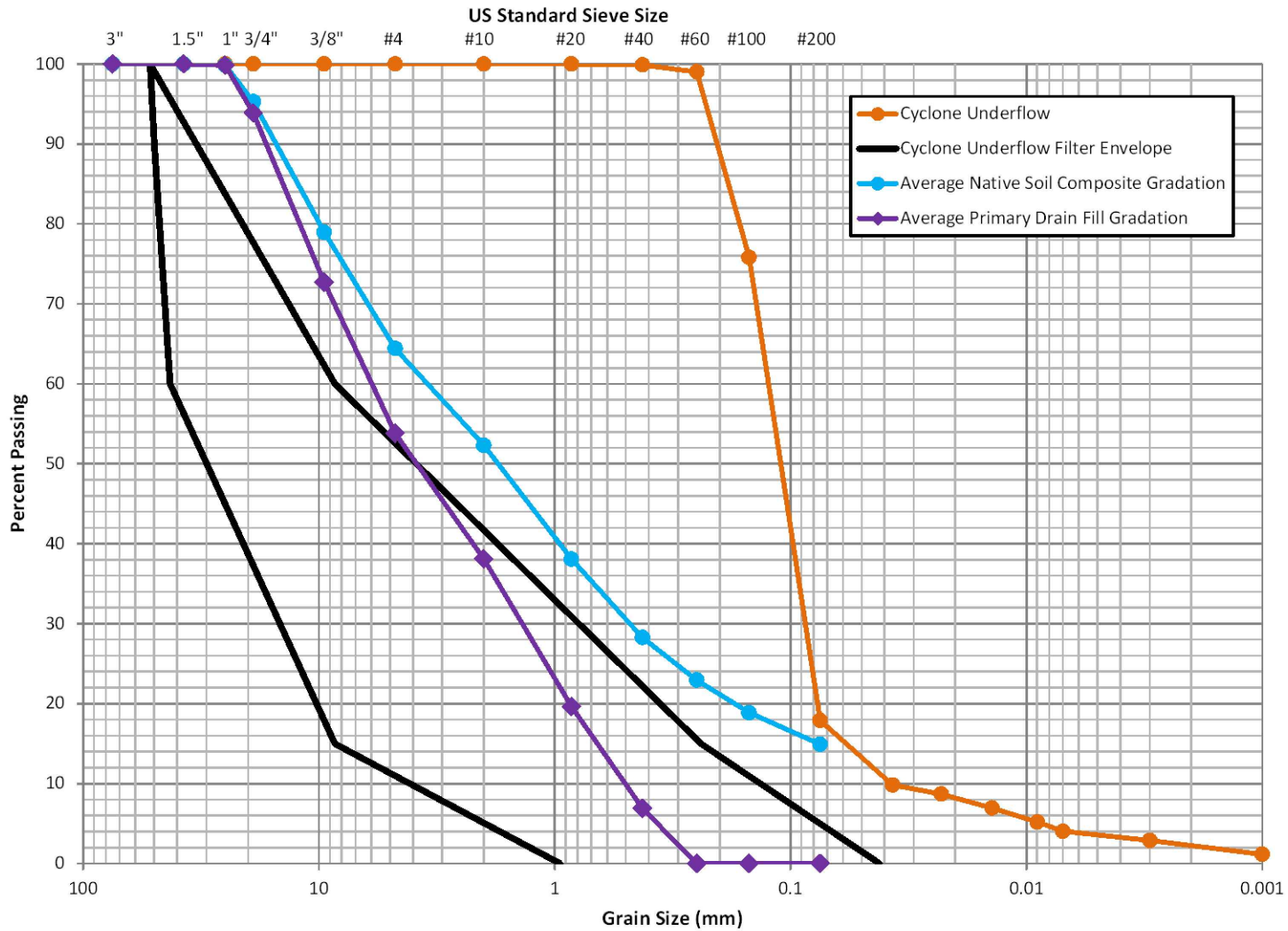


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<p>PROJECT</p> <p><b>THEMAC</b> RESOURCES</p> <p><small>Environmentally Responsible. Community-Minded. Local Opportunities.</small></p>	<p>NEW MEXICO COPPER CORPORATION</p>	<p>DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>
<p>TITLE</p> <p><b>COMPRESSIBILITY AND PERMEABILITY VS VOID THICKENED OVERFLOW SLIMES</b></p>		
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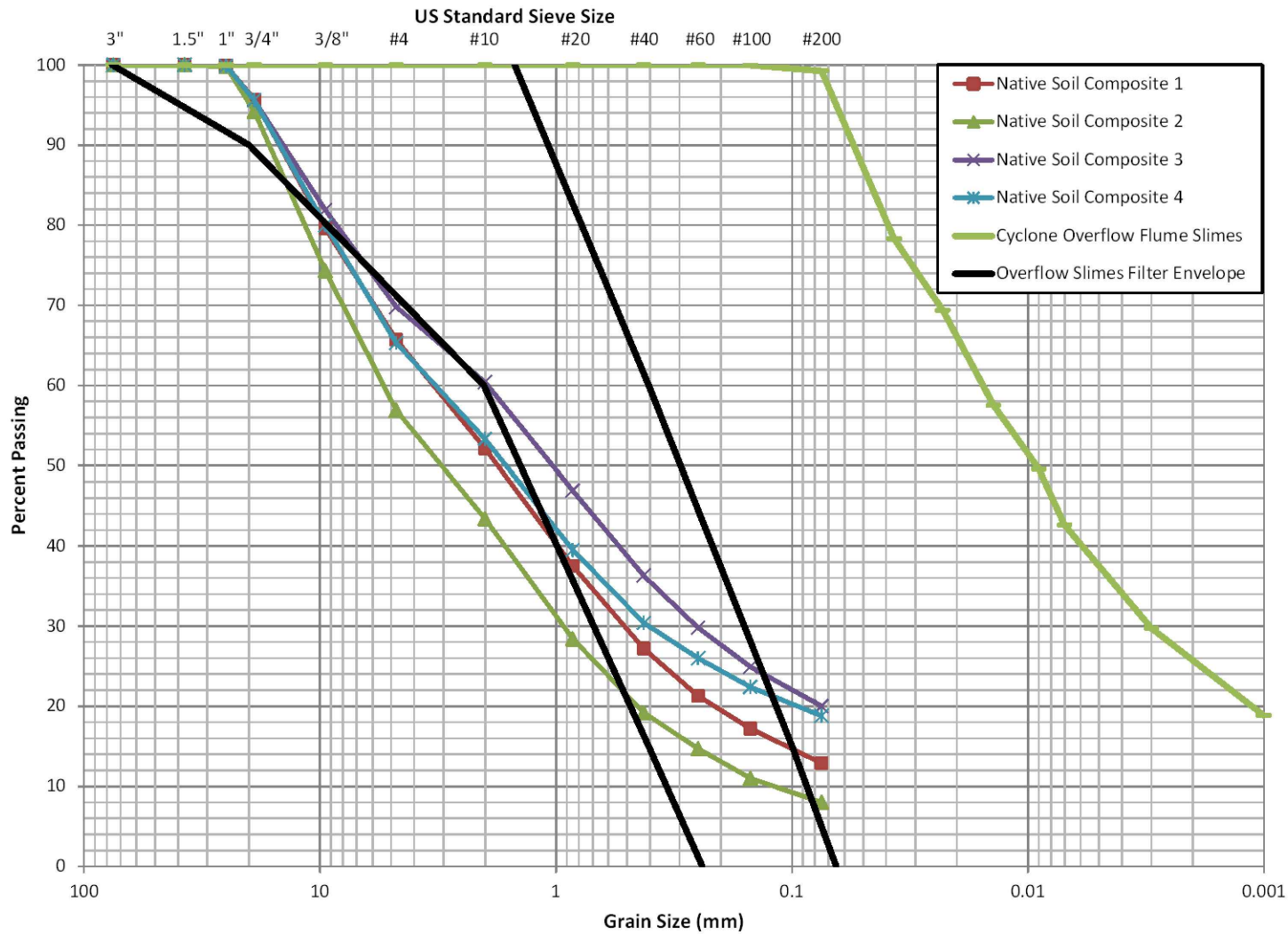
PROJECT  
**THEMAC** RESOURCES  
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NEW MEXICO COPPER CORPORATION  
 DRAFT COPPER FLAT PROJECT  
 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**CYCLONE UNDERFLOW PARTICLE SIZE DISTRIBUTION AND FILTER ENVELOPE**

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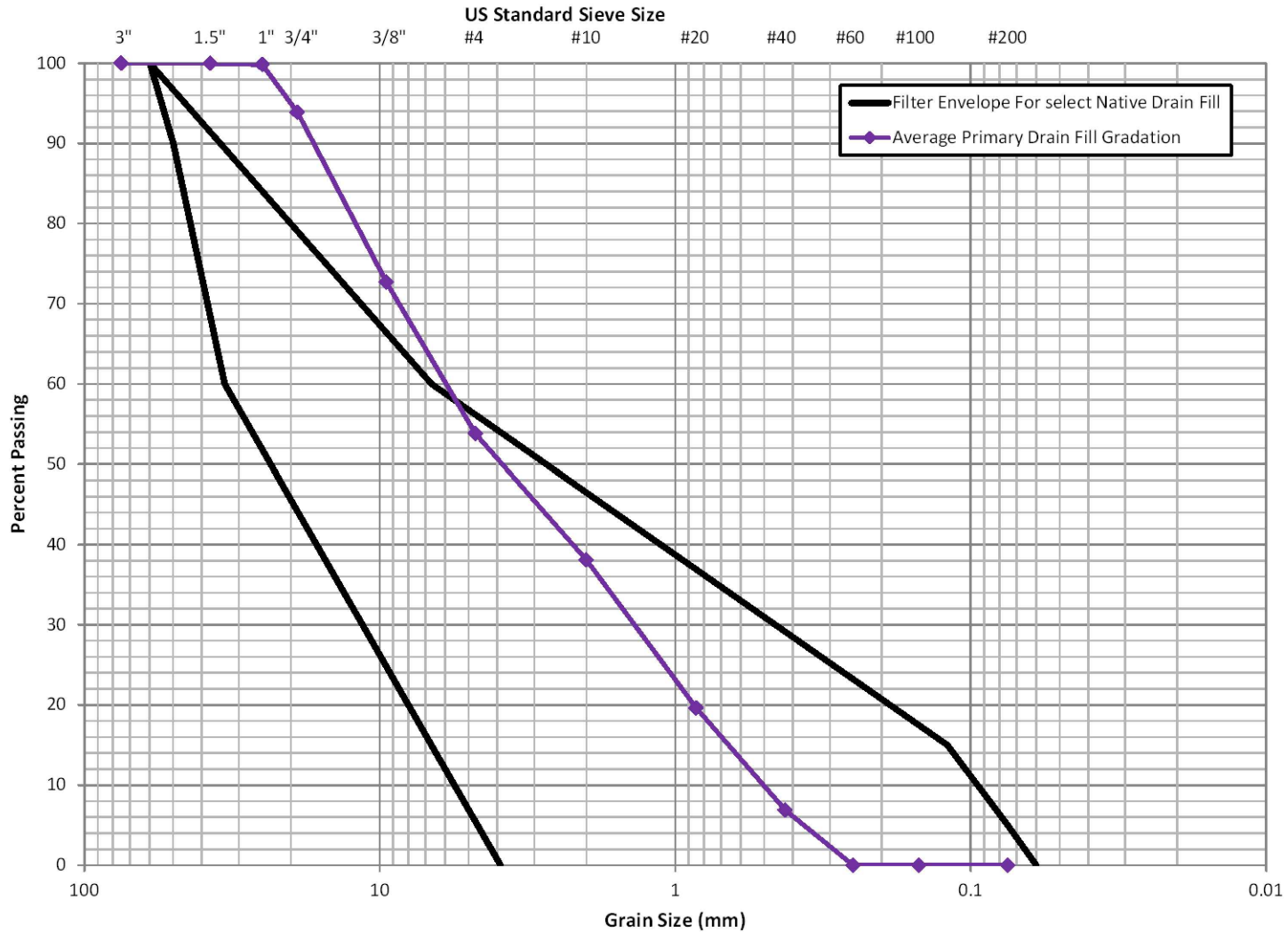





PROJECT  
**THEMAC** RESOURCES  
 NEW MEXICO COPPER CORPORATION  
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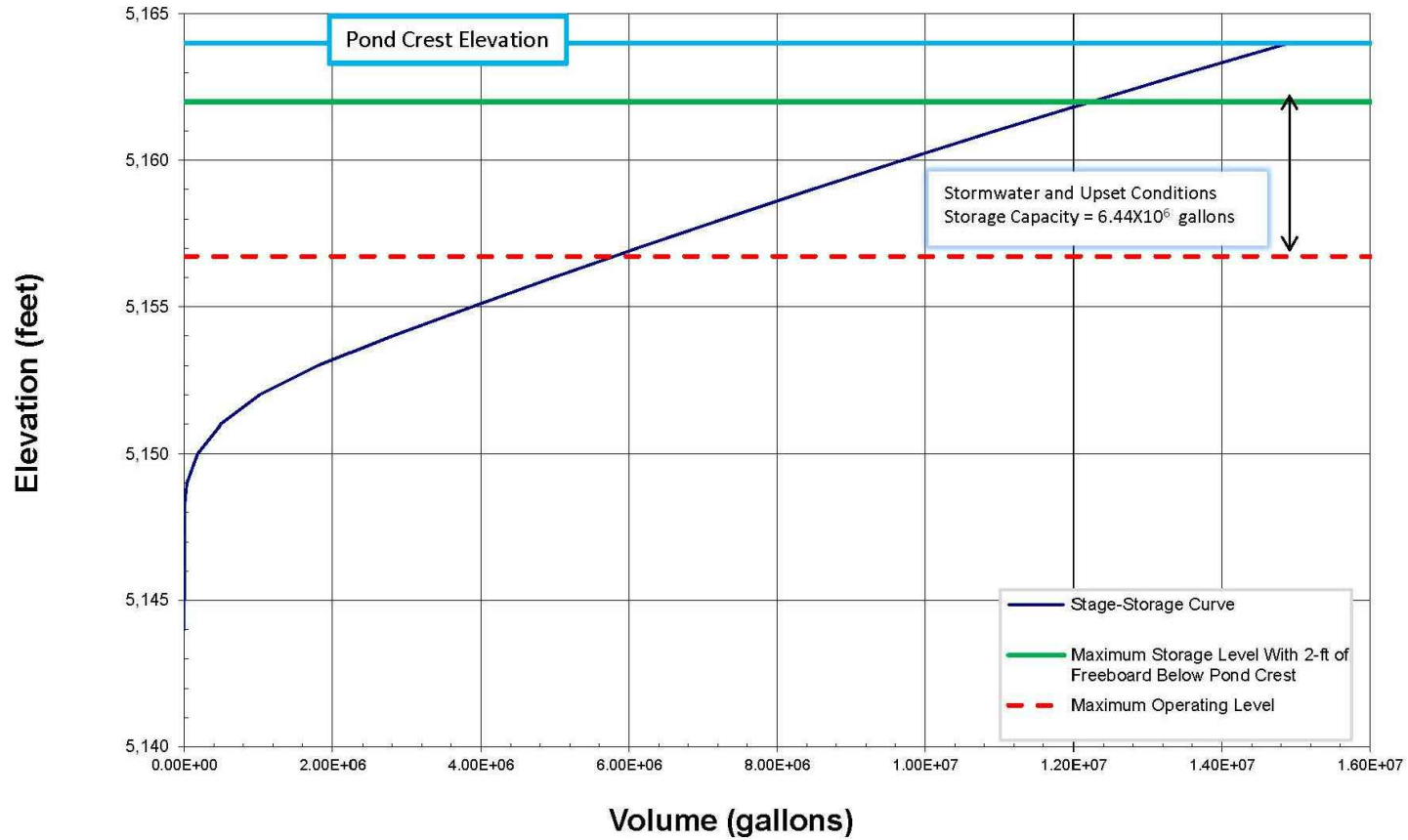
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 FACILITY FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO




TITLE  
**CYCLONE OVERFLOW SLIMES  
 PARTICLE SIZE DISTRIBUTION  
 AND FILTER ENVELOPE**

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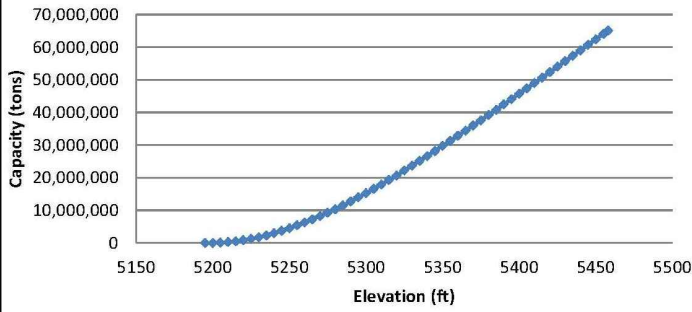
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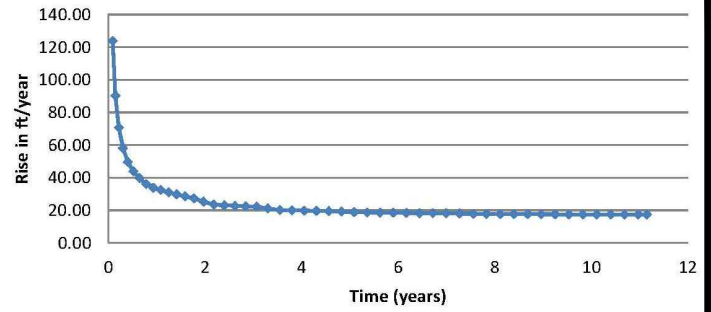
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CHECK	GM	2013-07-12	EBID		
REVIEW	DAK	2013-07-12	Exhibit 4		

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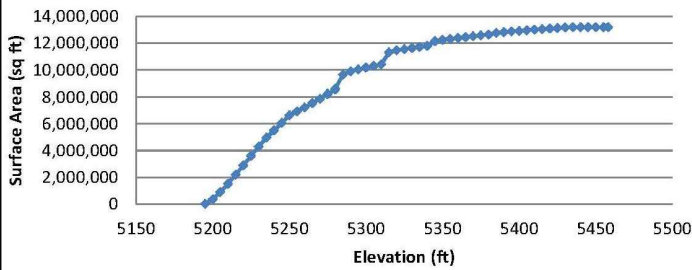
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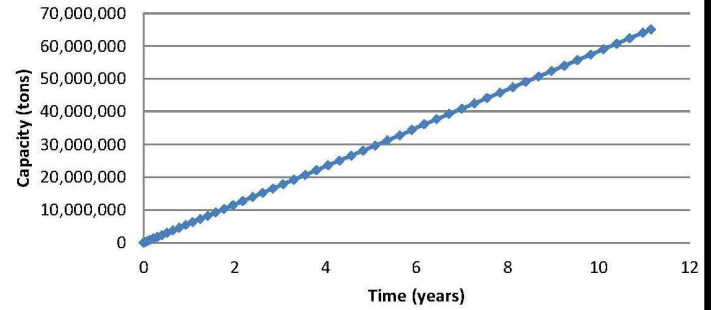
**Rate of Rise (ft/yr)**



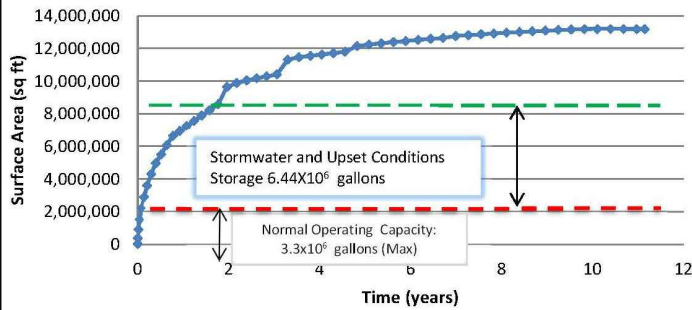
**Impoundment Surface Area vs. Elevation**



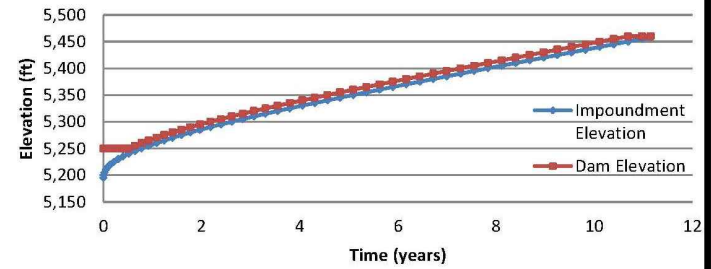
**Impoundment Cumulative Storage**



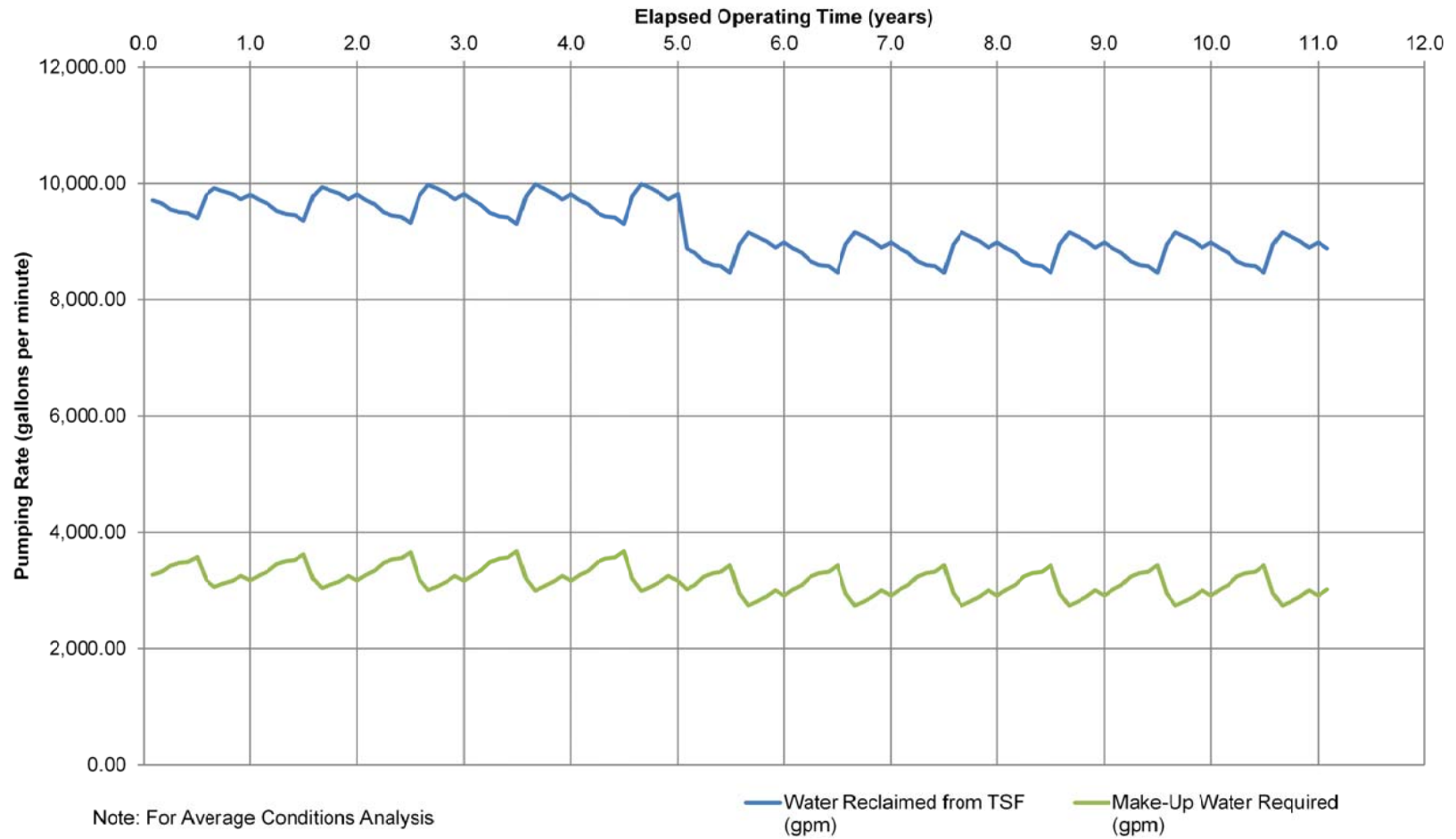
**Impoundment Surface Area vs. Time**






**Dam & Impoundment Elevation vs. Time**



<p>Environmentally Responsible. Community-Minded. Local Opportunities.</p>	<p>PROJECT</p> <p><b>THE MAC</b> RESOURCES</p> <p>NEW MEXICO COPPER CORPORATION</p>		<p>DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>		
	<p>TITLE</p> <p><b>TAILINGS STORAGE FACILITIES ELEVATION, SURFACE AREA, CAPACITY AND RATE OF RISE RELATIONSHIPS</b></p>				
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	CADD	JHR	2013-07-11	FIGURE	
	CHECK	GM	2013-07-12		
	REVIEW	DAK	2013-07-12		



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DESIGN	DMW	2013-04-30	FIGURE		
CADD	JHR	2013-07-11			
CHECK	GM	2013-07-12			
REVIEW	DAK	2013-07-12			

**APPENDIX A  
SITE EXPLORATION**

**APPENDIX A.1  
TEST PIT LOGS**



# TEST PIT LOG: TP-1

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977739.46 E: 867983.49

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 2 ft.	GM	sandy SILTY GRAVEL, medium, sub-angular, 35% coarse sub-rounded sand, 20% medium plasticity fines, 30% sub-angular cobbles (3-10"); pinkish gray (7.5YR 6/2), weak CaCO <sub>3</sub> cementation; non-cohesive, dry, dense.
2 - 4 ft.	GP	GRAVEL, medium, and SAND fine to coarse, poorly graded, 10% non-plastic fines; 25% angular to sub-angular cobbles (3-10"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as cemented masses and disseminated; non-cohesive, dry, very dense.
4 - 8 ft.	SP	Friable/weathering rock (andesite); gravelly SAND, coarse, sub-rounded, 20% fine to medium sub-angular gravels; 10% non-plastic fines; 50% angular to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO <sub>3</sub> coating rock fragments and fractures.



**Samples:**  
None.

**Special Notes:**  
Archaeologist present during excavation.  
Pit located in road; top 2 ft. of original surface removed for road cut.  
Native hillslope has ~70% surface rocks; with weathered/friable boulders exposed along road cut.  
Test pit location immediately adjacent to existing waste rock disposal facility.





# TEST PIT LOG: TP-2

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11976945.00 E: 869820.78

**Date:** 12/21/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 15% fine to coarse sub-rounded to sub-angular gravel, 40% low plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 4/3), blocky; cohesive, slightly moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded to sub-angular gravel, 35% low plasticity fines; 5% sub-rounded cobbles (3-6"), pinkish white (7.5YR 8/2), moderate CaCO <sub>3</sub> cementation, blocky to platy; non-cohesive, dry, dense.
2 - 6 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded to sub-angular and SAND, fine to coarse, sub-rounded, 15% low to no plasticity fines; 20% sub-angular to sub-rounded cobbles and boulders (3-15"), pinkish gray (7.5YR 7/2), strong RXN with HCl, dry, dense.
6 - 7 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plasticity fines; 25% sub-angular cobbles and boulders (3-15"), light brown (7.5YR 6/3), strong RXN with HCl; non-cohesive, dry, very dense.
7 - 9 ft.	GW	Friable/weathering rock (andesite); GRAVEL, fine to coarse, angular, 10% coarse sand, 5% non-plastic fines; 50% angular cobbles and boulders (3- 15"), weak RXN with HCl.



### Samples:

2-6 ft., bag  
6-7 ft., bag

### Special Notes:

7 to 9 feet moderately strong rock, slightly weathered. Can slowly excavate with excavator (hard digging).



# TEST PIT LOG: TP-3

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977450.61 E: 869805.07

**Date:** 12/21/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM/ML	gravelly sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded to sub-angular gravels, 20% fine to coarse sub-rounded sand; 20% sub-angular cobbles and boulders (3-15");\, brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 20% low to non-plastic fines: 15% sub-angular cobbles, pinkish white (7.5YR 8/2), moderate CaCO <sub>3</sub> cementation; non-cohesive, dry, dense.
2 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 25% sub-angular cobbles and boulders (3-20"), pink (7.5YR 7/3), strong RXN with HCl; non-cohesive, dry, dense.
7 - 9 ft.	GC	CLAYEY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% medium plasticity fines; 30% sub-angular cobbles and boulders (3-20"); brown (7.5YR 5/3), CaCO <sub>3</sub> disseminated and as coatings on rocks; non-cohesive, dry, dense.
9 - 11 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 10% low to non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/4), CaCO <sub>3</sub> as cemented masses, disseminated and coatings on rocks; non-cohesive, dry, very dense.



**Samples:**  
BMI samples all layers (bag samples)

**Special Notes:**  
Refusal at 11 ft., hit andesite. Fracturing andesite above.



# TEST PIT LOG: TP-5

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977574.94 E: 868955.59

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse sub-rounded, 20% fine to coarse sub-angular gravel, 25% low plasticity fines; 25% sub-angular cobbles (3-8"), yellowish brown (10YR 5/4), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 20% low plasticity fines; 5% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses, weakly cemented in places, blocky; non-cohesive, dry, compact.
3 - 7 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 10% non-plastic fines; 5% sub-angular cobbles, light brown (7.5YR 6/3), moderate CaCO <sub>3</sub> cementation in places (large plates excavated), clay fingering at 3 to 4 ft.; non-cohesive, dry, very dense.



### Samples:

1-3 ft. bag, bucket  
3-7 ft. bag, bucket  
BMI samples (0-1ft, 1-3ft, 3-7ft)

### Special Notes:

Refusal at 7 ft., hit bedrock (andesite).



# TEST PIT LOG: TP-6

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11978206.72 E: 871778.25

**Date:** 1/4/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 25% fine to coarse poorly graded gravel, 20% non-plastic fines; 35% sub-rounded to sub-angular cobbles, brown (10YR 4/3), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 25% fine to coarse sub-rounded sand, 35% low plasticity fines; 15% sub-rounded to sub-angular cobbles (3-5"), very pale brown (10YR 7/3), blocky, CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry, compact.
3 - 5 ft.	CL/GC	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded gravel, 25% fine to coarse sub-rounded sand; 5% sub-rounded to sub-angular cobbles (3-5"), strong brown (7.5YR 5/6), blocky, clay fingering, CaCO <sub>3</sub> as masses and moderately cementation in places, large blocky plates excavated; non-cohesive, dry, stiff.
5 - 7 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-rounded, poorly graded, 20% fine to coarse sub-rounded sand, 15% medium plasticity fines; 15% sub-angular cobbles (3-7"), light brown (7.5YR 6/4); non-cohesive, dry, dense.
7 - 13 ft.	SM	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 25% med. plasticity fines; 45% sub-angular to angular cobbles and boulders (3-12"); light brown (7.5YR 6/4), CaCO <sub>3</sub> coatings on rocks; non-cohesive, dry, very dense.



### Samples:

- 1-3 ft. bag
- 3-5 ft. bag
- 5-7 ft. bag, bucket
- 7-13 ft. bag, bucket

### Special Notes:

At 7 feet excavator broke through large boulder.



# TEST PIT LOG: TP-7

Checked GM 1/29/2013

**Client:** THEMAC **Date:** 12/17/2012  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974576.20 E: 872338.77

**Lithology:**

Depth	USCS	Description
0 - 1.5 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% medium to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1.5 - 4 ft.	CH	CLAY and SAND, high plasticity, 45% medium to coarse poorly graded subrounded sand, 5% fine to coarse poorly graded gravels; reddish brown (5YR 4/4), blocky, some CaCO <sub>3</sub> masses and disseminated; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 20% low plasticity fines; trace sub-angular cobbles (3-5"), pinkish gray (7.5YR 7/2), moderate CaCO <sub>3</sub> cementation; non-cohesive, dry dense.
6 - 8 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, 10% non-plastic fines; 5% sub-angular cobbles (3-5"), pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> masses and disseminated; non-cohesive, dry, very dense.
8 - 10 ft.	GP	GRAVEL, fine, poorly graded, and SAND, poorly graded, fine to coarse, 5% non-plastic fines; trace cobbles (3-4"), brown (7.5YR 5/3), CaCO <sub>3</sub> coating on rocks; non-cohesive, dry, very dense.
10 - 12 ft.	GP-GM	SILTY GRAVEL, fine to coarse, poorly graded, and SAND fine to coarse, sub-rounded, 10% low plasticity fines; 10% sub-angular cobbles (4-5"), brown (7.5YR 5/3), CaCO <sub>3</sub> coating on rock fragments; non-cohesive, dry, dense.



**Samples:**  
 0-1.5 ft. bag } 0-4ft. Bucket  
 1.5-4 ft. bag }  
 4-6 ft. bag  
 6-10 ft. bag, bucket  
 10-12 ft. bag, bucket  
 BMI bag samples all layers

**Special Notes:**



# TEST PIT LOG: TP-8

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974106.85 E: 872357.94

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy SILTY CLAY, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace cobbles (3-6"), brown (7.5YR 4/2), disseminated CaCO <sub>3</sub> , platy; cohesive, moist, firm.
2 - 5 ft.	CI	sandy gravelly SILTY CLAY, high plasticity, 30% fine to coarse sand, 20% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), CaCO <sub>3</sub> masses and weakly cemented in places, platy; cohesive, dry, stiff.
5 - 7 ft.	SC/CI	CLAYEY SAND, fine to coarse, sub-rounded, 45% medium plasticity fines, 5% fine to coarse poorly graded gravels; dark brown (7.5YR 3/4), CaCO <sub>3</sub> masses, platy; cohesive, dry, stiff.
7 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse poorly graded gravels; trace cobbles (3-4"), reddish brown (5YR 5/4), moderate CaCO <sub>3</sub> cementation, large plates excavated; cohesive, dry, hard.
13 - 16 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 5% sub-rounded cobbles (3-4"), light brown (7.5YR 6/3); CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, compact to dense.



### Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-7 ft. bag
- 7-13 ft. bag
- 13-16 ft. bag

### Special Notes:



# TEST PIT LOG: TP-9

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974362.19 E: 873288.54

**Date:** 12/17/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 45% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, firm.
2 - 6 ft.	SP	TAILINGS. poorly graded SAND, medium, sub-rounded, 5% non-plastic fines; pale yellow (2.5Y 7/3), no RXN with HCl; non-cohesive, dry, loose. Tailing thickness in pit is tapered east to west: lower depth 6 ft. (east) and 4 ft. (west).
6 - 8 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine poorly graded gravel, 15% medium plasticity fines; 5% sub-angular cobbles (3-4"); very pale brown (10YR 7/3), CaCO <sub>3</sub> masses; non-cohesive, dry, dense.
8 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 5% angular cobbles (3-6"), pale brown (10YR 7/3), CaCO <sub>3</sub> coatings on coarse fragments and disseminated; non-cohesive, dry, dense.
10 - 11 ft.	GM	sandy SILTY GRAVEL, poorly graded, fine, sub-rounded, 35% fine to coarse sand, 15% medium plasticity fines; 10% angular cobbles (3-4"), pinkish gray (7.5YR 6/2), moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GP-GM	sandy SILTY GRAVEL, poorly graded, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 10% medium plasticity fines; pinkish gray (7.5YR 6/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.



**Samples:**  
 6-8 ft. bag } 6-10 ft. bucket  
 8-10 ft. bag }  
 10-11 ft. bag } 10-14 ft. bucket  
 11-14 ft. bag }

BMI bag samples: 6-8 ft., 8-10ft, 10-11 ft.

**Special Notes:**  
 Reclaimed area on tailing dam.



# TEST PIT LOG: TP-10

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974364.43 E: 873777.16

**Date:** 12/17/2012

**Lithology:**

Depth	USCS	Description
0 - 0.5 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% medium sub-angular gravel; trace sub-angular cobbles (3-6"), dark yellowish brown (10YR 4/4), friable, strong RXN with HCl; cohesive, slightly moist, soft.
0.5 - 3 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 40% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), dark brown (10YR 3/3), blocky, disseminated CaCO <sub>3</sub> ; cohesive, dry, firm.
3 - 6 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 7/4), platy, no RXN with HCl; non-cohesive, dry, compact.
6 - 12 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 8/4), no RXN with HCl; non-cohesive, dry, loose.
12 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 25% medium plasticity fines, 25% fine to coarse sub-rounded gravels; 15% sub-rounded cobbles (3-10"), pale brown (10YR 6/3), CaCO <sub>3</sub> masses and coatings on rock fragments, mixing with tailing at horizon contact; non-cohesive, dry, dense.



**Samples:**

- 0.5-3 ft. bag
- 3-6 ft. bag, bucket
- 6-12 ft. bag, bucket
- 12-13 ft. bag, bucket

**Special Notes:**

Reclaimed area on tailing dam.  
 Stop at 13 feet due to limit of backhoe, but appear to be on top of a layer of more gravels.





# TEST PIT LOG: TP-11

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974375.04 E: 874235.34

**Date:** 12/17/2012

### Lithology:

Depth	USCS	Description
0 - 0.83 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-10"), brown (10YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
0.83 - 5 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; light gray (2.5Y 7/2), platy, weak RXN with HCl; non-cohesive, dry, compact.
5 - 11 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; brownish yellow (10YR 6/8), no RXN with HCl; non-cohesive, dry, loose to compact.
11 - 13 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; grayish brown (2.5Y 5/2), no RXN with HCl; non-cohesive, dry, loose.



### Samples:

- 0-1 ft. bag
- 1-5 ft. bag, bucket
- 5-11 ft. bag, bucket
- 11-13 ft. bag, bucket

### Special Notes:

Reclaimed area on tailing dam.  
Stop at 13 feet due to limit of backhoe.



# TEST PIT LOG: TP-12

Checked GM 1/29/2013

Client: THEMAC  
 Project: Copper Flat  
 Project No.: 103-92557  
 Location: Sierra County, NM  
 NAD 83: N: 11974885.17 E: 875173.88

Date: 1/2/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	ML	SILT and SAND, low plasticity, 45% poorly graded fine sand, 5% medium gravel; trace cobbles (4-6"), strong brown (7.5YR 4/6), blocky; cohesive, dry, firm.
1 - 3 ft.	CL	sandy SILTY CLAY, medium plasticity, 35% fine to coarse sub-rounded sand, 5% medium gravel; trace sub-rounded to sub angular cobbles (4-6"), pink (7.5 YR 7/3), clay fingering, CaCO <sub>3</sub> masses and cemented in places, blocky; cohesive, dry, stiff.
3 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 40% sub-rounded to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, dense.
7 - 8 ft.	SM/GM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded; 20% low to medium plasticity fines; 10% cobbles (3-10"), pink (7.5YR 7/4), blocky, strong CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.
8 - 11 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% non-plastic fines; 5% cobbles (3-10"), pale brown (10YR 6/3), weak to moderate SiO <sub>2</sub> cementation, CaCO <sub>3</sub> trace masses and disseminated; non-cohesive, dry, dense.
11 - 13 ft.	SM	SILTY SAND, fine to coarse, sub-rounded and GRAVEL, fine to coarse, sub-rounded; 20% low plasticity fines; trace cobbles (3-4"), yellowish brown (10YR 5/4), platy, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, CaCO <sub>3</sub> visible in pores and disseminated; non-cohesive, dry,
13 - 15 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% low to non-plastic fines; 15% cobbles (3-10"), yellowish brown (10YR 5/4), CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, dense.



**Samples:**

BMI bag samples: 0-1 ft., 1-3 ft., 3-7 ft., 8-11 ft., 11-13 ft.

**Special Notes:**

Surface reworked by wind, small dunes around shrubs (*Flourensia cernua*, tarbush and *Prosopis glandulosa*, honey mesquite).



# TEST PIT LOG: TP-13

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974438.98 E: 875447.89

**Date:** 1/2/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	SILT, non-plastic, 10% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1 - 3 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light yellowish brown (10YR 6/4), blocky, clay fingering, CaCO <sub>3</sub> as masses and weakly cemented in places; cohesive, dry, soft.
3 - 5 ft.	CL	sandy SILTY CLAY, low to medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; light brown (7.5YR 6/4), blocky, CaCO <sub>3</sub> as masses and disseminated; cohesive, dry, firm.
5 - 8 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light brown (7.5YR 6/4), strong angular blocky, CaCO <sub>3</sub> along pores, weak RXN with HCl; cohesive, dry, stiff.
8 - 10 ft.	SM	SILTY SAND, fine to coarse, subrounded, and GRAVEL, fine to coarse, sub-rounded, 15% low plasticity fines; 15% sub-rounded to sub-angular cobbles and boulders (3-15"), brown (7.5YR 5/4), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
10 - 18 ft.	GW	sandy GRAVEL, fine to coarse, sub-rounded, 25% fine to coarse sub-rounded sand, 10% non-plastic fines; 30% sub-rounded to sub-angular cobbles and boulders (3-20"), dark yellowish brown (10YR 4/4), weak RXN with HCl; non-cohesive, dry, very, dense.



### Samples:

- 5-8 ft. bag, bucket
- 5-10 ft. bag, bucket
- 10-18 ft. bag, bucket

### Special Notes:



# TEST PIT LOG: TP-14

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974366.80 E: 874917.98

**Date:** 1/2/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	CI	gravelly sandy SILTY CLAY, high plasticity, 20% fine to coarse sub-angular gravel, 15% fine to coarse sub-rounded sand; 20% sub-angular cobbles (3-12"); dark reddish brown (5YR 3/4), blocky, no RXN with HCl; cohesive, dry, firm.
1 - 4 ft.	CL	gravelly sandy SILTY CLAY, moderate plasticity, 25% fine to coarse sub-angular gravel, 25% fine to coarse sub-rounded sand; 15% sub-angular cobbles (3-12"); pink (7.5YR 7/3), blocky, CaCO <sub>3</sub> masses and weak cementation in places, strong RXN with HCl clay fingering; cohesive, dry, firm.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 15% fine to coarse poorly graded gravels; trace cobbles (3"), pinkish white (7.5YR 8/2), strong CaCO <sub>3</sub> cementation (large plates excavated); non-cohesive, dry, dense.
7 - 12 ft.	ML	sandy SILT, non-plastic, 20% fine to medium poorly graded sand, trace fine poorly graded gravels; brown (7.5YR 5/4), platy, CaCO <sub>3</sub> lining pores and some masses, weak RXN with HCl; cohesive, dry, soft.
12 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 15% non-plastic fines; 15% sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16.5 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sand, 15% non-plastic fines; trace sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), weak SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rocks, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**

- 0-1 ft. bag
- 1-4 ft. bag
- 4-7 ft. bag
- 7-12 ft. bag
- 12-14 ft. bag
- 14-16.5 ft. bag

**Special Notes:**

Offset pit location approximately 25 feet to the west to keep disturbance on tracked road.



# TEST PIT LOG: TP-15

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973832.93 E: 874871.54

**Date:** 12/20/2012

**Lithology:**

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse sub-angular gravels; 5% sub-angular cobbles (3-10"), pink (7.5 YR 7/4), blocky, strong RXN with HCl; cohesive, dry, firm.
4 - 8 ft.	CI	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to medium poorly graded gravel; light reddish brown (5YR 6/4), angular blocky (breaking to fine aggregates), CaCO <sub>3</sub> nodules and masses; cohesive, dry, firm.
8 - 10 ft.	CH	sandy CLAY, high plasticity, 15% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/4), angular blocky (breaking to gravel sized aggregates), weak RXN with HCl, clay pressure faces; cohesive, moist, firm.
10 - 20 ft.	CH	CLAY, high plasticity, 5% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/3), angular blocky (rock structure), weak RXN with HCl, clay pressure faces; cohesive, moist, stiff.



**Samples:**

8-10 ft. bag, bucket  
 10-20 ft. bag, bucket

**Special Notes:**

Excavator leaves slick sidewalls at 8+ feet. Clays formed in place (not illuvial) from weathering primary minerals.



# TEST PIT LOG: TP-16

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973732.56 E: 875481.72

**Date:** 12/20/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; 10% sub-angular cobbles (3-6"), brown (7.5YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 4 ft.	ML	sandy CLAYEY SILT, low plasticity, 20% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; trace sub-angular cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, stiff.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 20% fine poorly graded sub-angular gravels; brown (7.5YR 5/3), friable, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
7 - 10 ft.	ML	sandy SILT, non-plastic, 30% fine to coarse sub-angular sand, 5% fine poorly graded sub-angular gravels; light brown (7.5YR 6/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl, thin layer (<1ft) of moderate cementation; non-cohesive, dry, hard.
10 - 17 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 20% sub-rounded cobbles (3-12"); brown (7.5YR 5/3), weak to strong CaCO <sub>3</sub> cementation (stratified), weak to strong RXN with HCl; non-cohesive, dry, very dense.



**Samples:**

- 0-2 ft. bag
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-10 ft. bag, bucket
- BMI bag samples same as above

**Special Notes:**

10 to 17 foot interval has varying degrees of CaCO<sub>3</sub> cementation (none to strong), but grouped together due to particle size similarities.



# TEST PIT LOG: TP-17

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973205.01 E: 873937.67

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	sandy SILT, low plasticity, 40% fine to coarse sub-rounded sand, trace fine poorly graded gravels; trace sub-angular cobbles (3-6"), yellowish brown (10YR 5/4), platy, weak RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	CH	sandy CLAY, high plasticity, 35% fine to coarse poorly graded sand, 5% fine to coarse poorly graded gravels; trace cobbles (3"), reddish brown (5YR 4/4), blocky, disseminated CaCO <sub>3</sub> and masses, strong RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), pink (7.5YR 7/3), blocky, moderate CaCO <sub>3</sub> cementation and masses, strong RXN with HCl; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, poorly graded, 25% low plasticity fines, 25% fine poorly graded gravels; trace cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> masses and coating rock fragments, strong RXN with HCl; cohesive, dry, firm.
6 - 14 ft.	GW	sandy GRAVEL, fine to coarse, poorly graded, 40% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), stratified, thickly bedded, some weak CaCO <sub>3</sub> cementation at 8 feet moderate cementation at 14 feet, CaCO <sub>3</sub> masses, strong RXN with HCl; non-cohesive, dry, dense



### Samples:

0-2 ft. bag  
2-4 ft. bag  
4-6 ft. bag  
6-14 ft. bag  
BMI bag samples: 0-2 ft., 2-4 ft., 4-6 ft., 6-10 ft.

### Special Notes:

Disturbed surface, pit located in depression on tailing dam (cow lay-down area). Salt cedar (*Tamarix chinensis*) and seep willow (*Baccharis salicina*) stand.



# TEST PIT LOG: TP-18

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973182.90 E: 874892.73

**Date:** 12/20/2012

**Lithology:**

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 2 ft.	GM/SM	sandy SILTY GRAVEL, poorly graded, fine to medium, sub-angular, 25% fine to medium poorly graded sub-rounded sand, 45% low plasticity fines; trace sub-angular cobbles (3-4"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; trace sub-angular cobbles (3-4"), light brown (7.5YR 6/3), blocky, moderate CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 30% low plasticity fines; pink (7.5YR 7/3), blocky, moderate to weak CaCO <sub>3</sub> cementation in places, disseminated, and coatings on rocks; non-cohesive, dry, dense.
5 - 7 ft.	SM/GM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; reddish brown (5YR 5/3), large plates excavated, moderate SiO <sub>2</sub> cementation, strong reaction with HCl; non-cohesive, dry, dense.
7 - 9 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded gravel, 30% low plasticity fines; reddish brown (5YR 5/4), large plates excavated, strong SiO <sub>2</sub> cementation, weak reaction with HCl; non-cohesive, dry, very dense.
9 - 15 ft.	SP	SAND, poorly graded, medium, sub-rounded, 5% fine poorly graded gravel, 5% non-plastic fines; reddish brown (5YR 5/4), large plates excavated, moderate SiO <sub>2</sub> cementation, weak reaction with HCl; non-cohesive, dry, very dense.



**Samples:**

7-9 ft. bag, bucket  
 9-15 ft. bag, bucket

**Special Notes:**





# TEST PIT LOG: TP-19

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973045.18 E: 875451.64

**Date:** 12/19/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	GC	Disturbed/FILL. sandy CLAYEY GRAVEL, fine to coarse, sub-rounded, 45% high plasticity fines, 20% fine to coarse sub-rounded sand; 15% sub-rounded cobbles (3-6"), reddish brown (5YR 4/4), blocky, no RXN with HCl; cohesive, moist, firm.
2 - 3 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, poorly graded, sub-rounded, 40% high plasticity fines, 30% fine to coarse sub-rounded sand; 5% sub-rounded cobbles (3-6"), yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, dry, hard.
3 - 5 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% non-plastic fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), strong CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, subangular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO <sub>3</sub> as masses, disseminated and weak cementation in places; non-cohesive, dry, dense.
10 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% medium poorly graded gravel, 25% low to medium plasticity fines; trace cobbles (3-10"), white (7.5YR 8/1), blocky, CaCO <sub>3</sub> as masses, disseminated and moderate cementation in places; non-cohesive, dry, dense.
11 - 14 ft.	GM-GC	sandy GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 25% low to medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO <sub>3</sub> as masses, disseminated and strong cementation in places; non-cohesive, dry, dense.



**Samples:**

None.

**Special Notes:**

stratified gravels and sands, thickly bedded, from 5 to 14 feet.



# TEST PIT LOG: TP-20

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972549.21 E: 875734.91

**Date:** 12/19/2012

**Lithology:**

Depth	USCS	Description
0 - 0.5 ft.	ML	sandy SILT, non-plastic, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; brown (7.5YR 4/2), friable, no RXN with HCl; cohesive, dry, soft.
0.5 - 2 ft.	CH	CLAY and SAND, high plasticity, 40% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC-SP	gravelly CLAYEY SAND, fine to coarse, poorly graded, 30% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 10% sub-rounded cobbles (3-7"), brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated; strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% medium plasticity fines, 15% fine to coarse poorly graded sub-rounded gravels; trace sub-rounded cobbles (3"), white (7.5YR 8/1), blocky, weakly cemented (CaCO <sub>3</sub> ); non-cohesive, dry, very dense.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry, dense.
7 - 11 ft.	GP	GRAVEL, fine to coarse, poorly graded, and SAND, fine to coarse, sub-round SAND, 5% non-plastic fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), weakly cemented (CaCO <sub>3</sub> ); non-cohesive, dry, dense.
11 - 18.5 ft.	SW-SM	SILTY SAND fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 15% non-plastic fines; 10% sub-rounded cobbles (3-6"), grayish brown (10YR 5/2), blocky, CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments; non-cohesive, dry, dense to compact.



**Samples:**  
 0-2 ft. bag } 0-4 ft. bucket  
 2-4 ft. bag }  
 4-5 ft. bag } 4-7 ft. bucket  
 5-7 ft. bag }  
 7-11 ft. bag, bucket  
 11-18.5 ft. bag, bucket

**Special Notes:**  
 Surface disturbed.



# TEST PIT LOG: TP-21

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972274.72 E: 875755.12

**Date:** 12/19/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 45% fine to coarse sub-rounded sand, 5% coarse poorly graded sub-angular, gravels; 5% sub-angular cobbles (3-4"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC/SM	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% medium plasticity fines, 30% medium poorly graded sub-angular gravels; trace cobbles (3"), white (7.5YR 8/1), blocky, weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 25% fine to medium poorly graded sub-angular gravels; trace cobbles (3"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded sub-angular gravels, 25% medium plasticity fines; 5% sub-rounded cobbles (3-5"), brown (7.5YR 4/3), disseminated CaCO <sub>3</sub> , masses and coatings on rock fragments, strong RXN with HCl; cohesive, dry, stiff.
7 - 11 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 20% medium plasticity fines; 10% sub-angular cobbles (3-6"), brown (7.5YR 5/4), weak RXN with HCl, moderate SiO <sub>2</sub> cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, sub-angular, 40% fine to coarse sub-rounded sand, 15% low plasticity fines; 5% sub-rounded cobbles (3-6"); brown (7.5YR 5/4), weak RXN with HCl, weak SiO <sub>2</sub> cementation; non-cohesive, dry, dense.
14 - 18 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 20% low plasticity fines; trace cobbles (3-4"); light brown (7.5YR 6/4), large plates excavated, weak RXN with HCl, weak SiO <sub>2</sub> cementation; non-cohesive, dry, dense.



**Samples:**

- 7-11 ft. bag, bucket
- 11-14 ft. bag, bucket
- 14-18 ft. bag, bucket
- BMI bag samples: 7-11 ft., 11-14 ft., 14-18 ft.

**Special Notes:**

CaCO<sub>3</sub> masses and cementation confined to upper layers (2-7 ft.). Disseminated CaCO<sub>3</sub> and coatings on coarse fragments at 7+ feet. SiO<sub>2</sub> cementation/conglomerate at 7-18 feet.



# TEST PIT LOG: TP-22

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972597.58 E: 875051.00

**Date:** 12/20/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse sub-angular gravel; 5% cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% low to medium plasticity fines, 25% fine to coarse sub-angular gravel; 5% cobbles (3-5"), pinkish white (7.5YR 8/2), weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 10% non-plastic fines; 10% cobbles (3-12"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
5 - 8 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sub-rounded sand, 10% non-plastic fines; 20% cobbles and boulders (3-15"), brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
8 - 11 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 25% medium plasticity fines; 20% cobbles and boulders (3-15"), reddish brown (5YR 4/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
11 - 13 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 25% medium plasticity fines; trace cobbles (3-4"), reddish brown (5YR 5/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, 10% fine to medium poorly graded gravels, 10% low plasticity fines; yellowish red (5YR 5/6), weak RXN to HCl, moderate cementation (SiO <sub>2</sub> ), large plates excavated; non-cohesive, dry, very dense.



**Samples:**

None.

**Special Notes:**

Stratified gravels at 5 to 11 feet, thickly bedded.  
 Disturbed surface; A horizon has been removed.



# TEST PIT LOG: TP-23

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972621.41 E: 874709.99

**Date:** 12/20/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 35% fine to medium poorly graded sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; trace sub-angular cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	ML	CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/3), blocky, weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, very stiff.
3 - 5 ft.	ML-SM	CLAYEY SILT and SAND, low to medium plasticity, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; light brown (7.5YR 6/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 8 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low to non-plastic fines, 5% fine to medium poorly sub-angular graded gravel; pink (7.5YR 7/4), moderate CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
8 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 25% low plasticity fines, 25% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/4), strong CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.
11 - 12 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 30% low plasticity fines, 30% fine to coarse sub-rounded sand; 5% cobbles (3-5"), brown (7.5YR 5/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.



**Samples:**

None.

**Special Notes:**

very hard digging at 10 feet. Refusal at 12 feet.



# TEST PIT LOG: TP-24

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972670.75 E: 873699.23

**Date:** 12/18/2012

**Lithology:**

Depth	USCS	Description
0 - 3 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse poorly graded sub-rounded gravel; 20% cobbles (4-10"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded sand, 30% medium plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; 5% sub-angular cobbles (3-5"); brown (7.5YR 5/4), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
5 - 10 ft.	SM/SC	SILTY SAND, fine to coarse, well graded, sub-rounded, 25% medium plasticity fines, 10% fine to medium poorly graded sub-angular gravel; trace cobbles (3-5"); light brown (7.5YR 6/3), large blocky plates excavated, CaCO <sub>3</sub> as masses and weakly cemented (SiO <sub>2</sub> /CaCO <sub>3</sub> ) in places, strong RXN with HCl; non-cohesive, dry, dense.
10 - 14 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; 15% sub-angular cobbles; brown (7.5YR 5/3), CaCO <sub>3</sub> coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16 ft.	GP-GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, 30% fine to coarse well graded sand, 10% low plasticity fines; 20% sub-angular cobbles (3-10"), brown (7.5YR 5/3), CaCO <sub>3</sub> coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**  
 0-3 ft. bag  
 3-5 ft. bag, bucket  
 5-10 ft. bag, bucket  
 10-14 ft. bag, bucket  
 14-16 ft. bag, bucket  
 BMI samples all layers (bag samples)

**Special Notes:**  
 Disturbed surface.



# TEST PIT LOG: TP-25

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972408.78 E: 872794.31

**Date:** 12/13/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded sub-rounded gravel, 20% medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky; cohesive, dry, firm.
2 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravel, 10% low plasticity fines; 20% cobbles and boulders (3-15"), light brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry,
5 - 6 ft.	GP	sandy GRAVEL, fine to coarse, poorly graded, sub-angular, 25% fine to coarse sub-rounded graded sand, 5% non-plastic fines; 15% cobbles and boulders (3-12"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.
6 - 7 ft.	GP	sandy GRAVEL, fine to coarse, poorly grades, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 15% cobbles (3-10"), light brown (7.5YR 6/3), large blocky plates excavated, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-6 ft. bag
- 6-7 ft. bag

BMI sample: 2-5 ft.

### Special Notes:

Pit located in old borrow area. Hard to dig with backhoe at 5+ feet.



# TEST PIT LOG: TP-26

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973262.64 E: 872782.60

**Date:** 12/13/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	FILL. gravelly SILTY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse sub-rounded gravel; 5% sub-angular cobbles (3-4"), brown (10YR 5/3), friable; cohesive, dry, soft.
1 - 3 ft.	CL/SM	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 20% fine to coarse sub-rounded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), blocky, CaCO <sub>3</sub> as masses and disseminated; cohesive, dry, stiff.
3 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-rounded graded gravel, 15% low plasticity fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), large blocky plates excavated, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-angular gravel, 15% low plasticity fines; 5% sub-angular cobbles (3-5"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.



### Samples:

- 0-3 ft. bag
- 3-4 ft. bag
- 4-5 ft. bag

### Special Notes:

Pit located in old borrow area. Hard to dig with backhoe; refusal at 5 feet.





# TEST PIT LOG: TP-27

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972823.18 E: 871169.18

**Date:** 12/19/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-5"), dark brown (10YR 3/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse poorly graded sub-angular gravel; 10% sub-angular cobbles and boulders (3-20"); brown (7.5YR 4/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
3 - 7 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-angular, 45% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% sub-angular cobbles (3-10"), pinkish gray (7.5YR 7/2), CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl, moderate SiO <sub>2</sub> cementation at 5 feet; non-cohesive, dry, dense.
7 - 13 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), pinkish gray (7.5YR 6/2), moderate SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.
13 - 14 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 10% sub-angular cobbles (3-10"), pinkish gray (7.5YR 6/2), strong SiO <sub>2</sub> cementation (conglomerate), CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag, bucket
- 2-3 ft. bag, bucket
- 3-7 ft. bag, bucket
- 7-13 ft. bag, bucket
- 13-14 ft. bag
- BMI samples all layers (bag samples)

### Special Notes:

Hard digging due to oversize at 7 feet and cemented conglomerate at 13 feet.



# TEST PIT LOG: TP-28

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973129.48 E: 871528.89

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-rounded gravel; 10% sub-angular cobbles (3-8"), brown (7.5YR 4/2), blocky, weak RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM/CL	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 30% fine to coarse sub-rounded gravel; trace sub-angular cobbles (3-8"), pinkish gray (7.5YR 6/2), blocky, weak CaCO <sub>3</sub> cementation in places and masses strong RXN with HCl; cohesive, dry, firm.
4 - 6 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-rounded and SAND, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; trace cobbles (3-8"), brown (7.5YR 5/2), CaCO <sub>3</sub> coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
6 - 9 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), light brown (7.5YR 6/4), moderate SiO <sub>2</sub> cementation, very weak RXN with HCl; non-cohesive, dry, very
9 - 14.5 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 20% sub-angular cobbles (3-8"), light brown (7.5YR 6/4), strong SiO <sub>2</sub> cementation (conglomerate), very weak RXN with HCl; non-cohesive, dry, very dense.



**Samples:**  
None.

**Special Notes:**  
Difficult to excavate at 9 feet. Archaeologist present during excavation.



# TEST PIT LOG: TP-29

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974098.42 E: 871178.07

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-12"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
1 - 2 ft.	CI	gravelly sandy SILTY CLAY, medium to high plasticity, 25% fine to coarse sub-angular gravel; 20% medium to coarse poorly graded sub-rounded sand; 5% cobbles (3-12"), reddish brown (5YR 4/4), blocky, disseminated CaCO <sub>3</sub> , strong RXN with HCl; cohesive, dry, stiff.
2 - 4 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse sub-angular gravels; 5% cobbles (3-6"), pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, very stiff.
4 - 7 ft.	SP	gravelly SAND, fine to coarse, poorly graded, sub-rounded, 25% fine to coarse poorly graded gravels, 5% non-plastic fines; trace sub-angular cobbles (3-6"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, compact.
7 - 12 ft.	SW-GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 10% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), strong SiO <sub>2</sub> cementation (conglomerate) at 11 feet, strong RXN with HCl, CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments; non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag, bucket
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-12 ft. bag, bucket

### Special Notes:

Refusal at 12 feet due to cementation and oversized.



# TEST PIT LOG: TP-30

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974680.06 E: 871571.56

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-10"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
2 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% fine to coarse poorly graded sub-angular gravel, 15% low plasticity fines; 10% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO <sub>3</sub> and masses, strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SW	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine poorly graded sub-angular gravels, 5% non-plastic fines; pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> as masses and coatings on gravels, moderate SiO <sub>2</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 12 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 30% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO <sub>3</sub> and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**  
None.

**Special Notes:**



# TEST PIT LOG: TP-31

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11975597.72 E: 872172.62

**Date:** 1/3/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	CL	gravelly sandy CLAYEY SILT, medium to high plasticity, 30% fine to coarse sub-rounded sand, 25% fine to coarse sub-angular gravel; 10% sub-angular to sub-rounded cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 30% medium plasticity fines; 15% cobbles (3-9"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> masses, disseminated and weak cementation in places, clay fingering, strong RXN with HCl; cohesive, dry, stiff.
2 - 5 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, weak SiO <sub>2</sub> cementation, weak RXN with HCl, CaCO <sub>3</sub> coatings on rocks; non-cohesive, dry, dense.
5 - 8 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 5% non-plastic fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, mod. SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, very dense.
8 - 13 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), brown (7.5YR 5/4), platy, weak SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), platy, strong SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, very dense.



**Samples:**  
 1-2 ft. bag  
 2-5 ft. bag  
 5-8 ft. bag  
 8-13 ft. bag  
 13-16 ft. bag  
 BMI bag samples all layers

**Special Notes:**  
 Surface disturbed- placer mining location. Pit located in drainage.



# TEST PIT LOG: TP-32

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11975136.61 E: 872876.80

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	CL-ML	sandy CLAYEY SILT, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 25% medium plasticity fines; 20% cobbles (3-10"), pinkish gray (7.5YR 7/2), blocky, CaCO <sub>3</sub> masses, coatings on rock fragments and weak cementation in places, stron RXN with HCl; non-cohesive, dry, compact.
3 - 5 ft.	SM	SILTY SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 15% low plasticity fines; 5% cobbles (3-4"), light brown (7.5YR 6/3), weak SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN to HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, poorly graded, sub-rounded sand, 10% low plasticity fines; 20% cobbles and boulders (3-12"), pinkish gray (7.5YR 6/2), weak SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rock fragments, weak RXN to HCl; non-cohesive, dry, very dense.
10 - 14 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% low plasticity fines; 15% cobbles (3-8"), light brown (7.5YR 6/3), large platy blocks excavated, strong SiO <sub>2</sub> cementation, CaCO <sub>3</sub> masses, weak RXN to HCl; non-cohesive, dry, very dense.



### Samples:

3-5 ft. bag, bucket  
5-10 ft. bag, bucket  
10-14 ft. bag, bucket

### Special Notes:

Hard

**APPENDIX A.2  
DRILL HOLE LOGS**



# REPORT OF BOREHOLE: SOIL KEY

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Copper Flat  
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N, E  
 CHECKED: DP DATE: 2/21/13 ELEVATION: ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer, Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
5										<p><u>Blows Per Six Inches</u>            Number of sample hammer blows required to drive the sampler six inches, or recorded number of blows to drive the sampler the specified distance (e.g. 50/4" = 50 hammer blows to drive the sampler four inches).</p> <p><u>Blows Per Foot</u>            Number of sample hammer blows required to drive the sampler twelve inches. Resolved using the final twelve inches of the sample or the amount of penetration upon sample refusal (50 blow counts).</p>				
15			SS1	4	11	17 / 18				<p><u>Sample Types</u>            Standard Penetration Test - Full penetration with 17 of 18 inches recovered.</p>				
20			SS2	7	R	7 / 10				<p>Standard Penetration Test - Refusal at 10 inches, 7 of 10 inches recovered.</p> <p>Auger Bag/Bulk Sample - Bulk grab from auger cuttings to become a collective bag sample over an interval.</p>				
25														
30														
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT.GPJ TEMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:7/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



## UNIFIED SOIL CLASSIFICATION (ASTM D 2487-00)

MATERIAL TYPES	CRITERIA FOR ASSIGNING SOIL GROUP NAMES AND GROUP SYMBOLS USING LABORATORY TESTS			GROUP SYMBOL	SOIL GROUP NAMES & LEGEND	
COARSE-GRAINED SOILS >50% RETAINED ON NO. 200 SIEVE	GRAVELS >50% OF COARSE FRACTION RETAINED ON NO. 4. SIEVE	CLEAN GRAVELS <5% FINES	$C_u > 4$ AND $1 < C_c < 3$	GW	WELL-GRADED GRAVEL	If soil contains >15% sand, add "with sand"
			$C_u > 4$ AND/OR $1 > C_c > 3$	GP	POORLY-GRADED GRAVEL	
		GRAVELS WITH FINES >12% FINES	FINES CLASSIFY AS ML OR CL	GM	SILTY GRAVEL	
			FINES CLASSIFY AS CL OR CH	GC	CLAYEY GRAVEL	
	SANDS >50% OF COARSE FRACTION PASSES ON NO. 4. SIEVE	CLEAN SANDS <5% FINES	$C_u > 6$ AND $1 < C_c < 3$	SW	WELL-GRADED SAND	If soil contains >15% gravel, add "with gravel"
			$C_u > 6$ AND/OR $1 > C_c > 3$	SP	POORLY-GRADED SAND	
SANDS AND FINES >12% FINES		FINES CLASSIFY AS ML OR MH	SM	SILTY SAND		
		FINES CLASSIFY AS CL OR CH	SC	CLAYEY SAND		
FINE-GRAINED SOILS >50% PASSES NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT <50		CL	LEAN CLAY	If soil contains coarse-grained soil from 15% to 29%, add "with sand" or "with gravel" for whichever type is prominent, or for >30%, add "sandy" or "gravelly"	
			ML	SILT		
			OL	ORGANIC CLAY OR SILT		
	SILTS AND CLAYS LIQUID LIMIT >50		CH	FAT CLAY		
			MH	ELASTIC SILT		
			OH	ORGANIC CLAY OR SILT		
HIGHLY ORGANIC SOILS	PRIMARILY ORGANIC MATTER, DARK IN COLOR, AND ORGANIC ODOR	PT	PEAT			

$$C_u = \frac{D_{60}}{D_{10}} \quad C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$$

Gravels or sands with 5% to 12% fines require dual symbols (GW-GM, GW-GC, GP-GM, GP-GC, SW-SM, SW-SC, SP-SM, SP-SC) and add "with clay" or "with silt" to group name. If fines classify as CL-ML for GM or SM, use dual symbol GC-GM or SC-SM.

### DESCRIPTIVE TERMINOLOGY FOR PERCENTAGES (ASTM D 2488-00)

DESCRIPTIVE TERMS	RANGE OF PROPORTION
TRACE	0 - 5%
FEW	5 - 10%
LITTLE	15 - 25%
SOME	30 - 45%
MOSTLY	50 - 100%

### LABORATORY TEST ABBREVIATIONS

<b>AL</b> Atterberg Limits	<b>HY</b> Hydrometer	<b>SG</b> Specific Gravity
<b>CI</b> Chloride Content	<b>PT</b> Proctor	<b>SP</b> Swell Potential
<b>CO</b> Consolidation	<b>pH</b> Soil pH	<b>UC</b> Unconfined Compression
<b>CP</b> Collapse Potential	<b>RS</b> Restivity	<b>UU</b> Triaxial Unconsolidated, Undrained
<b>CU</b> Triaxial Consolidated Undrained	<b>RV</b> R-Value	
<b>DD</b> Dry Density	<b>SA</b> Sieve Analysis	
<b>DS</b> Direct Shear	<b>SC</b> Soluble Sulfate Content	

### CRITERIA FOR DESCRIBING MOISTURE CONDITION (ASTM D 2488-00)

DRY	Absence of moisture, dusty, dry to the touch
MOIST	Damp but no visible water
WET	Visible free water, usually soil is below water table

### RELATIVE DENSITY / CONSISTENCY ESTIMATE USING STANDARD PENETRATION TEST (SPT) VALUES

COHESIONLESS SOILS (GRAVEL, SAND, NONPLASTIC SILT)			COHESIVE SOILS (PLASTIC SILT, CLAY)		
DENSITY	N <sub>i</sub> (BLOWS /FOOT)*	RELATIVE DENSITY (%)	CONSISTENCY	N <sub>i</sub> (BLOWS /FOOT)*	COMPRESSIVE STRENGTH (TSF)
VERY LOOSE	0 - 4	0 - 15	VERY SOFT	0 - 2	0 - 0.25
LOOSE	4 - 10	15 - 35	SOFT	2 - 4	0.25 - 0.50
COMPACT	10 - 30	35 - 65	FIRM	4 - 8	0.50 - 1.0
DENSE	30 - 50	65 - 85	STIFF	8 - 15	1.0 - 2.0
VERY DENSE	OVER 50	> 85	VERY STIFF	15 - 30	2.0 - 4.0
			HARD	OVER 30	OVER 4.0

\*Refer to ASTM D 1586-99 for a definition of N. Values shown are based on N values corrected for overburden pressures (N<sub>i</sub>). N values may be affected by a number of factors including material size, depth, drilling method, and borehole disturbance. N values are only an approximate guide for consistency of cohesive soil.

### COMPONENT DEFINITIONS BY GRADATION

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. to 12 in.
GRAVEL	3 in. to No. 4 (4.76 mm)
COARSE GRAVEL	3 in. to 3/4 in.
FINE GRAVEL	3/4 in. to No. 4 (4.76 mm)
SAND	No. 4 (4.76 mm) to No. 200 (0.074 mm)
COARSE SAND	No. 4 (4.76 mm) to No. 10 (2.0 mm)
MEDIUM SAND	No. 10 (2.0 mm) to No. 40 (0.42 mm)
FINE SAND	No. 40 (0.42 mm) to No. 200 (0.074 mm)
SILT AND CLAY	Smaller than No. 200 (0.074 mm)
SILT	0.074 mm to 0.005 mm
CLAY	Less than 0.005 mm

### GENERAL NOTES

Report of Borehole logs present material classifications, test data, and observations from subsurface explorations at the subject site as reported by the field geologist, engineer, or scientist. In some cases, the classifications may be made based on laboratory test data when available. It should be noted that the investigation methods only recover a small part of the subsurface materials at the exploration location. Therefore, actual conditions between borings and sampled intervals may differ from those presented on the Report of Borehole logs.

This key and Report of Borehole logs must be read together with the attached report. The information presented on the logs and in this key provide only a basis for an evaluation of the subsurface conditions. Any evaluation of the conditions reported on the Report of Borehole logs must be performed by Professional Engineers or Geologists.



# REPORT OF BOREHOLE: BH-01

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/17/12  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-1 43.5 feet west  
 XY COORDINATES: N 11,973,679, E 871,432  
 ELEVATION: 5,298.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Compact, light gray, SILTY GRAVEL (GM) with sand, dry, some cobbles.				
5			SS1		18 17 32	49	18 / 18			Becomes dense, gray.				
				Bulk										
10			SS2		4 40 45	85	18 / 18		GM	Becomes very dense.				
				Bulk										
15			SS3		13 32 50	82	18 / 18							
				Bag										
	5280.5 17.5			Bulk						Hard, brown, GRAVELLY SILT (ML), dry, some cobbles.				
20			SS4		26 50/5"	R	11 / 11							
				Bulk										
25			SS5		45 30/2"	R	8 / 8		ML					
				Bag										
30			SS6		40 40/3"	R	3 / 6							
				Bag										
	5267.0 31.0			SS7	10 10/0"	R	0 / 0			Refusal at 31'. Backfilled with cuttings. No groundwater encountered in boring.				
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-02

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 12/18/12

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-2 2 feet east

XY COORDINATES: N 11,973,762, E 870,444

ELEVATION: 5,361.8 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: Hollow Stem Auger

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0										Compact, light gray, CLAYEY GRAVEL WITH SAND (GC), dry, some cobbles.				
5358.3	3.5		SS1		17 16 23	39	18 / 18		GC	Dense, light gray, SILTY SAND (SM), little gravel, dry.				
			B/Bag						SM	Becomes light brown, some gravel, occasional cobbles.				
5352.3			SS2		16 29 50	79	6 / 18		GW	6" lens of very dense, gray, GRAVEL (GW), dry.				
5359.8	10.0		Bulk						ML	Hard, brown, SANDY SILT and gravel (ML), dry, occasional cobble.				
			SS3		17 50/6"	R	12 / 12							
			Bulk											
5343.3	18.5		SS4		26 47 50/3"	R	15 / 15		SM	Very dense, brown and gray, SILTY SAND (SM), some gravel, dry.				
			Bulk											
5340.3	21.5									Refusal at 21.5'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

EBID

Exhibit 13



# REPORT OF BOREHOLE: BH-03

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-3 5 feet south  
 LOGGED: CMT DATE: 1/25/13 XY COORDINATES: N 11,977,597, E 868,520  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,468.6 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, gray, WELL-GRADED SAND WITH GRAVEL (SW), trace fines, dry.				
5									SW					
8.0	5460.6			SS1 Bulk	50/0"	R	0 / 0			Slightly weathered, dark gray, strong rock.	Becomes rock.			
15.0	5453.6			Core						Refusal at 8'. Bottom of borehole at 15'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-04

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/5/13 XY COORDINATES: N 11,977,281, E 870,076  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,355.3 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					SW-SM	Compact, gray/light gray, SILTY SAND (SM), some gravel, cobbles, dry, cementation.				
5351.3 4.0			SS1		14 23 19	42	18 / 18			Dense, gray, SANDY SILT (SM), dry, some cementation.				
				Bag					ML					
5346.3 9.0			SS2 Core		25/0"	R	0 / 0			Weathered, dark gray, fragmented ROCK, trace cementation, trace silty sand.	Becomes rock. Switch to Diamond Coring.			
				Core						Becomes moderate cementation				
				Core						Become strong cementation. [CONGLOMERATE]				
5337.3 18.0				Core						Refusal of Hollow Stem Auger at 9'. Core from 9' to 18'. Bottom of borehole at 18'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-05

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/7/13 XY COORDINATES: N 11,977,243, E 869,166  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,385.6 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Compact, gray/ light gray, SILTY SAND and gravel (SW-SM), dry, trace cementation, cobbles.				
5	5379.6 6.0			Bulk	12 24 41	65	18 / 18		SW-SM	Becomes very dense.				
10				Core						Weathered, dark gray, fragmented ROCK, some sandy silt, strongly cemented.	Become rock. Switch to Diamond coring.			
15	5372.6 13.0									Refusal of Hollow Stem Auger at 6'. Diamond coring from 6-13'. Bottom of borehole at 13'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-06

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/25/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,597, E 870,667  
 ELEVATION: 5,308.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag							Very dense, brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk						SW-SM	Becomes gray.				
5300.1 8.0			SS1 Bulk		50/0"	R	0 / 0			Slightly weathered, dark gray, strong ROCK.	Becomes rock.			
			Core								Switch to Diamond Coring.			
5283.1 25.0										Diamond coring from 18-25'. Bottom of borehole at 25'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-07

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,181, E 871,532  
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0	5371.5 0.5		Bag Bag						SM	Loose, brown, SILTY SAND (SM), little gravel, dry.				
									GP	Compact, light gray, SANDY GRAVEL (GP), dry.				
5	5367.0 5.0		SS1 Bag		14 28 33	61	18 / 18			Lens of brown/white, SILTY SAND, trace gravel, dry. Becomes very dense. Becomes well-graded gravel.				
									SW-SM	Very dense, gray, SILTY SAND and gravel (SW-SM), dry.				
10			SS2 Bag		12 39 50/5"	R	17 / 17							
15	5357.0 15.0		SS3 Bag Bag		11 37 50/2"	R	14 / 14							
										Very dense, gray, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
20			SS4 Bag		25 30/2"	R	10 / 10							
25	5347.5 24.5		SS5 Bag		23 32/3"	R	9 / 9							
										Very dense, brown, SILTY SAND and gravel (SW-SM), dry, few cobbles.				
30	5342.0 30.0		SS6 Core		36/5"	R	5 / 5							
			SS7 Core		50/0"	R	0 / 0				Weathered, dark gray, fractured ROCK, strong cementation.	Becomes Rock. Switch to Diamond Coring.		
			SS8 Core		20 50/0"	R	6 / 6							
35														
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-07

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,181, E 871,532  
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40										Weathered, dark gray, fractured ROCK, strong cementation. (continued)				
45														
50	5324.0 48.0									Refusal of Hollow Stem Auger at 30'. Diamond coring from 30-48'. Bottom of borehole at 48'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-08

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,703, E 873,489  
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Dense, brown, SILTY SAND (SW-SM), some gravel, slightly cohesive, trace cementation, trace clayey silt, dry.				
5														
10			SS1	18 19 23	42	18 / 18			SW-SM					
			Bulk							Becomes very dense.				
15			SS2	50 53 30	83	18 / 18								
			Bulk											
20	5200.0 18.0		Bulk							Hard/very dense, brown, SANDY SILT and gravel (ML), some clayey silt (low plasticity), dry, trace cementation.				
25			SS3	50/5"	R	5 / 5			ML					
			Bulk											
30	5190.0 28.0		Bag						SW-SM	Very dense, gray/brown, SILTY SAND and gravel (SW-SM), some clayey silt, dry.				
35	5185.0 33.0		SS4	50/5"	R	5 / 5			SW	Very dense, gray, SAND and gravel (SW), dry.				
			Bulk											
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-08

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,703, E 873,489  
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5175.0 43.0								SW	Very dense, gray, SAND and gravel (SW), dry. <i>(continued)</i>				
45									GW	Very dense, gray, GRAVEL and sand (GW), trace clayey silt, dry.				
50	5168.0 50.0		SS5		50/0"	R	0/0							
55														
60														
65														
70														
75														
80										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-09

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/21/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-9 2 feet east  
 XY COORDINATES: N 11,972,261, E 875,052  
 ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, brown, SANDY SILT (ML), trace gravel, dry.				
			SS1		20 34 40	74	18 / 18		ML					
	5170.7 6.0			Bulk										
			SS2	Bag(2)	20/0"	R	0 / 1			Dense to very dense, light gray/brown, SAND and gravel (SW), dry.				
			SS3		20 25 24	49	18 / 18		SW					
	5165.7 11.0			Bag(2)										
				Bag						Hard, light brown, SILT (ML), trace gravel, dry, slightly cohesive, trace cementation.				
			SS4		16 25 27	52	18 / 18			Becomes light reddish brown, little gravel.				
				Bulk					ML					
			SS5		4 14 24	38	18 / 18							
	5156.7 20.0			Bulk						Hard, reddish brown, CLAYEY SILT (MH), dry, cohesive.				
									MH					
			SS6		22 14 19	33	18 / 18							
	5150.7 26.0			Bulk										
				B/Bag						Hard, red SILTY CLAY (CL-ML), dry, cohesive, moderate plasticity.				
			SS7		16 15 19	34	18 / 18		CL-ML					
				B/Bag										
			SS8		6 4 13	17	18 / 18							
	5141.7 35.0			Bulk						Hard, red, CLAY (CL), dry.				
									CL					
			SS9		14 21 31	52	18 / 18							
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-09

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/21/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-9 2 feet east  
 XY COORDINATES: N 11,972,261, E 875,052  
 ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk					CL	Hard, red, CLAY (CL), dry. <i>(continued)</i>				
	5133.2 43.5		SS10		13 20 32	52	18 / 18		CL-ML	Hard, red, SILTY CLAY (CL-ML), dry.				
45	5131.7 45.0			Bulk					CH	Hard, red, CLAY, dry.				
			SS11		9 15 24	39	18 / 18							
50	5126.7 50.0									Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-10

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/12/13 XY COORDINATES: N 11,972,513, E 874,813  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				B/Bag						Compact, light gray, CLAYEY SAND WITH GRAVEL (SC), dry, few cobbles.				
5			SS1	X	17 6 5	11	18 / 18							
				B/Bag					SC					
10			SS2	X	9 12 18	30	18 / 18							
				B/Bag										
15	5167.5 14.5		SS3	X	20 16 14	30	18 / 18							
				Bag					MH	Very stiff, red/light brown, CLAYEY SILT (MH), dry, little gravel.				
20	5163.0 19.0		SS4	X	8 15 20	35	18 / 18							
				B/Bag						Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity.				
25			SS5	X	3 10 17	27	18 / 18							
				B/Bag										
30			SS6	X	10 19 41	60	18 / 18							
				B/Bag					CL					
35			SS7	X	8 11 17	28	18 / 18			Becomes moderate plasticity				
				B/Bag						Becomes slightly moist, moderate plasticity.				
40			SS8	X	4 6 19	25	18 / 18							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-10

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/12/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,513, E 874,813  
 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			B/Bag							Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity. <i>(continued)</i>				
5138.5 43.5			SS9		4 7 14	21	18 / 18		CL					
45										Very stiff, red, CLAY (CH), slightly moist, high plasticity.				
5132.0 50.0			SS10		7 10 16	26	18 / 18		CH					
50										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-11

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/3/13 XY COORDINATES: N 11,972,894, E 874,891  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff-very stiff, brown, SILT (ML), trace sand and gravel, dry.				
5178.0 2.5			Bulk											
			SS1		9 13 27	40	18 / 18		SW	Dense, white/light gray, GRAVELLY SAND (SW), dry.				
			Bulk											
5172.0 8.5			SS2		12 27 23	50	18 / 18		SW-SM	Very dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk											
5167.0 13.5			SS3		5 11 16	27	18 / 18		MH	Very stiff, brown, CLAYEY SILT (MH), dry, low plasticity.				
			Bulk											
5162.0 18.5			SS4		7 12 23	35	18 / 18		CL-ML	Hard, reddish brown, SILTY CLAY (CL-ML), white and black inclusions, dry.				
			B/Bag							Becomes red, trace gravel, moderate plasticity, slightly moist.				
			SS5		24 16 23	39	18 / 18							
			B/Bag											
			SS6		7 15 25	40	18 / 18							
			B/Bag											
5147.0 33.5			SS7		11 13 18	31	18 / 18		CH	Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity.				
			Bulk											
			SS8		12 18 30	48	18 / 18							
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-11

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/3/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,894, E 874,891  
 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9	X	11 13 18	31	18 / 18		CH	Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity. (continued)				
50	5130.5 50.0		SS10	X	11 12 21	33	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-12

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,981, E 875,148  
 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff, brown, SANDY SILT (ML) and gravel, dry.				
5175.5	4.0		SS1		5 12 15	27	18 / 18		GW	Compact, light gray, GRAVEL AND SAND (GW), dry.				
5174.5	5.0		Bulk						GW-GM	Compact, gray/light brown, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
			SS2		39 43 50	93	17 / 17		GW-GM	Becomes very dense, trace cobbles.				
			Bulk											
5164.5	15.0		SS3		13 33 50	83	16 / 16		ML	Hard, brown, SANDY SILT and gravel (ML), dry.				
			Bulk Bag											
5160.0	19.5		SS4		7 18 24	42	18 / 18		ML	Hard, red, SILTY CLAY (CL-ML), trace gravel, dry, low plasticity.				
			B/Bag											
			SS5		7 10 20	30	18 / 18		CL-ML					
			B/Bag											
			SS6		12 15 26	41	18 / 18		CL-ML					
			B/Bag											
5146.0	33.5		SS7		12 14 24	38	18 / 18		CH	Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity.				
			Bulk											
			SS8		12 16 26	42	18 / 18		CH					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-12

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,981, E 875,148  
 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity. <i>(continued)</i>				
45			SS9		10 11 18	29	18 / 18		CH	Becomes very stiff-hard.				
50	5129.5 50.0		SS10		10 12 20	32	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-13

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/17/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,776, E 875,471  
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, light gray, SANDY SILT and gravel (ML), dry, slightly cohesive.				
5			SS1	12 11 13	24	18 / 18								
				Bulk										
10			SS2	28 28 33	61	18 / 18			ML	Becomes hard, light brown, trace cementation.				
				Bag										
15			SS3	13 29 35	64	18 / 18								
				Bag										
20			SS4	21 30 31	61	18 / 18								
				Bulk										
25	5146.3 23.5		SS5	28 50/3"	R					Very dense, brown, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines.				
				Bulk										
30			SS6	21 50/4"	R					Lens of reddish brown clayey silt encountered at 34'.				
				Bulk										
35			SS7	27 49 50/2"	R									
				Bulk										
40	5131.3 38.5		SS8	13 24 33	57	18 / 18			CL-ML	Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-13

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/17/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,776, E 875,471  
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bag						Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel. (continued)				
45			SS9	Bulk	9 14	28	18 / 18		CL-ML	Becomes very stiff, red, dry.				
50	5119.3 50.5		SS10		25 32 28	60	18 / 18				Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-14

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,766, E 875,868  
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, brown, SILT (ML), some gravel, dry.				
5			SS1	23 44 30	74	18 / 18			ML	Becomes hard, brown, some gravel, cementation at 4-5 ft.				
10	5149.7 8.5		SS2	15 19 23	42	18 / 18			GW-GM	Dense, light brown/gray, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines, few cobbles.				
15			SS3	22 20 25	45	18 / 18			GW					
20			SS4	17 50/3"	R	9 / 9			GW	Becomes very dense, light brown/gray, GRAVEL and silty sand.				
25	5134.7 23.5		SS5	18 35 50/4"	R	16 / 16			ML	Hard, light gray, SANDY SILT and gravel (ML), dry.				
30	5129.7 28.5		SS6	25 50/6"	R	12 / 18			SW-SM	Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry.				
35			SS7	33 32 50/5"	R	17 / 17								
40			SS8	16 50/5"	R	11 / 11								

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-14

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,766, E 875,868  
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9 Bulk		50/1"	R	1 / 1		SW-SM	Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry. (continued)				
50	5108.2 50.0		SS10		31 29 44	73	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-15

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,609, E 875,724  
 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, light gray/light brown, SILT (ML), little gravel, dry.				
5			SS1	9 15 26	41				ML					
8.0	5168.4			Bulk										
10	5166.4		SS2	38 50/6"	R	12 / 12			GW	Very dense, brown, SANDY GRAVEL (GW), dry.				
12.0	5164.4			Bag					SW	Very dense, gray, SAND and gravel (SW), dry.				
15			SS3	17 23 50	73				GW	Very dense, gray, SANDY GRAVEL (GW), dry.				
17.5	5158.9			Bulk										
20	5154.9		SS4	30 50/3"	R	6 / 9			SW	Very dense, light gray, SAND and gravel (SW), dry.				
21.5	5154.9			Bag(2)										
25			SS5	28 50/4"	R	10 / 10			ML	Hard, light gray, SANDY SILT and gravel (ML), dry.				
27.5	5148.9			Bag(2)										
30	5144.9		SS6	22 50/6"	R	11 / 12			SW	Very dense, gray, SAND and gravel (SW), dry.				
31.5	5144.9			Bag(2)										
35			SS7	200"	R				SW-SM	Very dense, light gray, SILTY SAND (SW-SM), some gravel, dry.				
36.5	5139.9			Bag(2)										
40									SW	Very dense, light gray, SAND and gravel (SW), dry.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-15

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,609, E 875,724  
 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5134.9 41.5		SS8 Bag(2)		50/2"	R			SW	Very dense, light gray, SAND and gravel (SW), dry. (continued)				
45									ML	Very dense, gray/brown, SANDY SILT (ML), and gravel, dry, slightly cohesive.				
50	5126.4 50.0								MH	Hard, reddish brown, CLAYEY SILT (MH), some gravel, slightly moist, slightly cohesive.				
55	5122.1 54.3		SS9		24 54 38/3"	R				Bottom of borehole at 54.25'. Backfilled with cuttings. No groundwater encountered in boring.				
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-16

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north  
 XY COORDINATES: N 11,973,973, E 875,187  
 ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown, CLAYEY GRAVEL with sand (GC), dry.				
5			SS1	X	9 32 30	62	18 / 18		GC					
	5183.2 8.5			Bulk										
10			SS2	X	26 18 19	37	18 / 18		GP-GM	Very dense, gray, GRAVEL and sandy silt (GP-GM), dry.				
	5178.2 13.5			Bulk										
15			SS3	B/Bulk	50/3"	R	3 / 3		GW	Very dense, light brown, GRAVEL and silty sand (GW), dry, trace CaCO3.				
20			SS4	B/Bulk	26 50/4"	R	10 / 10							
25			SS5	B/Bulk	48 50/3"	R	9 / 9							
30	5162.7 29.0		SS6	Bulk	21 50/3"	R	9 / 9		SC	Very dense, gray, CLAYEY SAND with gravel, trace CaCO3, dry, some cementation.				
35	5157.7 34.0		SS7	Bulk	13 8 12	20	18 / 18		MH	Very stiff, light reddish brown, CLAYEY SILT (MH), dry.				
40	5153.2 38.5		SS8	X	14 9 11	20	18 / 18		CL-ML	Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-16

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north  
 XY COORDINATES: N 11,973,973, E 875,187  
 ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist. <i>(continued)</i>				
45			SS9		17 11 13	24	18 / 18		CL-ML					
				Bulk										
50	5141.7 50.0		SS10		4 2 4	6	18 / 18			Becomes firm, caliche and little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-17

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/5/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,131, E 875,734  
 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, gray/light brown, SANDY SILT and gravel (ML), dry.				
5			SS1	10 13 12	25	18 / 18		ML						
10	5177.5 8.5		SS2	4 45 50/4"	R	12 / 16				Very dense, gray, SANDY GRAVEL (GW), dry. Becomes brown/gray.				
20			SS3	7 28 39	67	18 / 18		GW						
25	5163.0 23.0									Very dense, light brown, SAND and gravel (SW), dry. Becomes light gray/ light brown.				
30	5158.0 28.0		SS4	24 50 50/4"	R	16 / 16		SW-SM		Very dense, brown, SILTY SAND and gravel (SW-SM), dry, slightly cohesive fines.				
35	5154.0 32.0									Very dense, white/gray CALICHE, dry.				
40			SS5	25/0"	R	0 / 0								

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-17

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/5/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,131, E 875,734  
 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40										Very dense, white/gray CALICHE, dry. <i>(continued)</i>				
45										very dense, white/gray CALICHE, dry, little gravel, seam of light brown silt at 44-45'				
50	5136.0 50.0		SS6		25/0"	R	0 / 0			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-18

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-18 2 feet east  
 XY COORDINATES: N 11,974,701, E 874,701  
 ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					ML	Stiff, light brown, SANDY SILT (ML), some gravel, dry.				
5205.3 2.0				Bulk										
				SS1	9 23 20	43	18 / 18		GC	Dense, light brown, CLAYEY GRAVEL with sand (GC), dry.	Switch to Air Rotary drilling at 6'.			
5199.3 8.0				Bulk										
				SS2										
				Bulk										
5189.3 18.0				SS2	50/4"	R			GW	Very dense, light brown/gray, GRAVEL and sand (GW), dry, little CaCO3.				
				Bulk										
5184.3 23.0				SS3	24 31 31	62	18 / 18		ML	Hard, light reddish white, SANDY SILT (ML), some gravel, dry, some CaCO3.				
				Bulk										
5173.8 33.5				SS4	13 8 12	20	18 / 18		CL	Hard, light reddish brown, sandy LEAN CLAY (CL), trace gravel, dry.				
				Bulk										
5168.8 38.5				SS5	38 50/3"	R	9 / 9		CL-ML	Hard, reddish brown, CLAY (CL-ML), some silty clay, little gravel, moderate plasticity, dry.				
				Bulk										
				SS6	11 21 24	45	18 / 18		CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist.				
				Bulk										

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-18

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-18 2 feet east  
 LOGGED: CMT DATE: 1/23/13 XY COORDINATES: N 11,974,701, E 874,701  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5163.8 43.5		Bulk						CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist. <i>(continued)</i>				
45			SS7 Bag		5 6 9	15	18 / 18		CH	Stiff-hard, reddish brown, CLAY (CH), high plasticity, slightly moist, blocky.				
50	5157.3 50.0		SS8		10 19 28	47	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-19

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/10/13 XY COORDINATES: N 11,974,564, E 875,328  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0										Soft, brown, SANDY SILT (ML), little gravel, dry.				
5193.4 3.0			SS1	11 21 22	43	18 / 18		ML		Dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk					SW-SM						
5187.4 9.0			SS2	19 38 50/5"	R	17 / 17				Very dense, brown/gray, SANDY GRAVEL (GW), dry.				
			Bulk											
			Bag							Becomes dark gray.				
			Bag(2)							Becomes brown/gray.				
			SS3	27 40 23	63	18 / 18		GW						
5174.4 22.0			B/Bag							Very dense, light gray/white, CALICHE, little gravel, dry.				
			SS4 B/Bag	50/0"	R	0 / 0								
			SS5 Bulk	25/0"	R	0 / 0				Seam of brown sandy silt at 41' - 42', dry.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-19

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/10/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,564, E 875,328  
 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5153.4 43.0			Bulk						Very dense, light gray/white, CALICHE, little gravel, dry. <i>(continued)</i>				
45				Bag					CL-ML	Very stiff, red, SILTY CLAY (CL-ML), slightly moist, cohesive				
50	5146.4 50.0			SS6	5 13 14	27	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-20

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to this location  
 XY COORDINATES: N 11,975,241, E 871,714  
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)							Compact, brown, SILTY SAND (SM), little gravel, dry.				
5			Bag						SM	Becomes compact to dense, light gray, slightly cohesive, trace clayey silt.				
8.0	5284.0		SS1		16 29 31	60	18 / 18			Very dense, gray, GRAVEL (GW), some silty sand, dry, trace clayey silt lenses.				
10			Bulk						GW					
18.0	5274.0		SS2		50/3"	R	3 / 3			Very dense, light brown/gray, SILTY SAND and gravel, (SW-SM) dry, slightly cohesive, some light reddish brown clayey silt.				
20			Bulk											
30			SS3		50/3"	R	3 / 3							
30			Bulk						SW-SM					
40	5252.0		SS4		15 50/3"	R	9 / 9							
40			Bulk											

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-20

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to this location  
 XY COORDINATES: N 11,975,241, E 871,714  
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0			Bulk										
45									SM	Very dense, light brown, SILTY SAND (SM), some gravel, slightly moist, slightly cohesive.				
50	5242.0 50.0		SS5	X	21 36 50/2"	R	14 / 14				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-21

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N 11,975,236, E 874,685  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,214.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown/gray, WELL-GRADED SAND with silt and gravel (SW-SM), dry.				
21			SS1		21	83	18 / 18		SW-SM					
36				Bulk	36									
47					47									
5196.2	18.0		SS2		25	68	18 / 18		SW-SM	Very dense, light brown/reddish white, SILTY SAND and gravel (SW-SM), trace clayey silt, cementation, dry.				
30			SS3		50/0*	R	0 / 0							
5184.2	30.0		Bag(2)							Hard, light reddish brown, CLAYEY SILT and silty clay (MH), trace gravel, dry.				
5177.7	36.5		SS4		15	65	18 / 18		MH					
										Bottom of borehole at 36.5'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-22

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/21/13 XY COORDINATES: N 11,974,757, E 873,750  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag Bulk						Loose, dark brown, SANDY SILTY CLAY (CL-ML), some gravel, dry. Becomes very stiff, yellowish/orange, trace gravel, dry (old tailings).				
5			SS1	16 8 8	16	18 / 18			CL-ML	Becomes orange/brown, slightly moist, cohesive, (old tailings).				
10			SS2	5 4 6	10	18 / 18			CL-ML	Becomes stiff, greenish-gray, moist, cohesive, (old tailings).				
15	5217.5 14.5		SS3	9 10 50/3"	R	15 / 15			GW	Very dense, light gray-gray, GRAVEL and sand (GW), dry, CaCO3 inclusions.				
25	5210.0 22.0		SS4	23 50/3"	R	9 / 9			ML	Hard, light gray/yellowish, SILT (ML), some gravel, slightly moist, slightly cohesive.				
30	5202.0 30.0			7 11 13	24	18 / 18			CL-ML	Becomes brown, SANDY SILT, some clayey silt, trace gravel, moist, cohesive fines.		10.4		
35	5197.0 35.0								CL	Very stiff, reddish brown, SILTY CLAY (CL-ML), moist, cohesive.				
40										Hard, reddish brown, CLAY (CL), dry, moderate plasticity.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-22

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/21/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,757, E 873,750  
 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5188.5 43.5		SS6		20 45 50/3"	R	15 / 15		CL	Hard, reddish brown, CLAY (CL), dry, moderate plasticity. <i>(continued)</i>				
45	5186.0 46.0		Bulk						SM	Hard, gray/brown, SILTY SAND (SM), slightly moist.				
50	5182.0 50.0		SS7		10 17 50/5"	R	17 / 17		CL-ML	Hard, brown, SILTY CLAY (SM), slightly moist, little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-23

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest  
 XY COORDINATES: N 11,974,757, E 873,750  
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
	5228.0 2.0			Bag					SP	Loose, brown, SAND and gravel (SP), dry.				
				SS						Very stiff, yellowish, SILT (ML), trace gravel, dry (old tailings).				
				Bulk						Becomes slightly cohesive.				
5									ML					
	5221.0 5220.6 9.5			SS1	13 38 36	74	18 / 18							
				Bag					GM	Very dense, gray/brown, GRAVEL and silt (GM), dry, slight cementation.				
										Hard, white, SANDY SILT (ML), little gravel, dry.				
									ML					
10														
	5216.0 14.0			SS2	13 16 14	30	18 / 30							
				Bulk						Dense, gray, SILTY SAND and gravel (SM), dry, CaCO3 inclusions, slight cementation.				
15														
				SS3	16 18 50/2"	R	14 / 14							
				Bag(2)						Becomes very dense.				
25														
				Bulk					SM	Becomes slightly moist.				
30														
				SS4	50/6"	R	6 / 4							
				Bulk						Becomes brown, some blocky and cohesive clayey silt (low plasticity), slightly moist.				
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-23

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest  
 XY COORDINATES: N 11,974,757, E 873,750  
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45									SM	Dense, gray, SILTY SAND and gravel (SM), dry, CaCO3 inclusions, slight cementation. <i>(continued)</i>				
48.0	5182.0													
49.5	5180.5		SS5		50/4"	R	4 / 5		ML	Very dense, brown, SANDY SILT (ML), wet, little gravel.				
50										Bottom of borehole at 49.5'. Backfilled with cuttings and bentonite. Groundwater encountered in boring at 45'.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations





# REPORT OF BOREHOLE: BH-24

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,975,910, E 872,908  
 ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						SP	Dense, gray/brown, SAND and gravel (SP), dry, trace CaCO <sub>3</sub> .				
2.5	5264.6			Bulk										
5										Very dense, gray, WELL-GRADED SAND with silt and gravel (SW-SM), dry.				
10				SS1 Bulk	50/4"	R	4 / 4							
15									SW-SM					
20				SS2 Bulk	50/3"	R	3 / 3							
25														
28.0	5239.1			SS3 Bulk	41 50/3"	R	9 / 9			Hard/very dense, brown, SANDY SILT and gravel (ML), trace clayey silt, dry.				
30									ML					
33.0	5234.1			Bulk						Very dense, gray/brown, SILTY SAND (SM), some gravel, trace clayey silt, dry.				
35									SM					
40	5227.1			SS4	44 50/3"	R	9 / 9							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-24

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,975,910, E 872,908  
 ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0			Bulk										
45									ML					
50	5217.1 50.0		SS5		50/1"	R	1 / 1							
55														
60														
65														
70														
75														
80														

Hard, light reddish brown, SANDY SILT (ML), some gravel, trace clayey silt, dry, slightly cohesive.

Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.



# REPORT OF BOREHOLE: BH-25

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,726, E 874,570  
 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMPT (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Dense to very dense, gray/brown, CLAYEY GRAVEL with sand (GC), well graded, dry.				
5			SS1		22 28	57	18 / 18		GC					
10			SS2		39 26 16	42	18 / 18		GC					
12.5	5199.7			Bulk						Hard, brown, SANDY SILT and gravel (ML), dry, slightly cohesive fines, trace CaCO3.				
15			SS3		18 21 20	41	18 / 18		ML					
20			SS4		15 15 33	48	18 / 18		ML					
22.0	5190.2			Bulk						Very dense, light reddish brown, CLAYEY SAND (SC), dry, trace gravel, trace CaCO3.				
25			SS5		10 27 31	58	18 / 18		SC					
30			SS6		5 32 35	57	18 / 18		SC					
35	5177.2	35.0		Bulk	6 9 16	25	18 / 18		CL-ML	Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive.				
40			SS8		8 8	25	18 / 18		CL-ML					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:L:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-25

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,726, E 874,570  
 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			Bulk		17					Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive. (continued)				
45	5167.2 45.0		SS9 Bulk		11 10 25	35	18 / 18		CL-ML		Hard, red, CLAY (CH), dry, sand seam at 44' to 44.5'.			
50	5161.7 50.5		SS10		10 18 26	44	18 / 18		CH		Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-26

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-26 2 feet south  
 XY COORDINATES: N 11,971,618, E 872,048  
 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
	5310.0 2.0			Bag(2) Bulk					GW	Dense, brown, GRAVEL (GW), some sand, dry.				
									SW	Very dense, gray/brown, SAND and gravel (SW), dry, sheen.				
	5304.0 8.0			SS1 Bulk	27 50/3"	R	9 / 9		GW	Very dense, dark gray, GRAVEL (GW), some sand, dry.				
									SW					
	5297.0 15.0			Bag(2)					GW	Very dense, brown/dark gray, SAND and gravel (SW), dry.				
									SW	Becomes gray/light brown, trace CaCO3.				
				SS2 Bulk	50/4"	R	4 / 4							
									SW					
				SS3 SS	25 50/2"	R	8 / 8							
									SW					
	5279.0 33.0			Bag(2)					SW-SM	Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt.				
				SS4 Bulk	21 50/5"	R	11 / 4							
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-26

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-26 2 feet south  
 XY COORDINATES: N 11,971,618, E 872,048  
 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45														
50	5262.0 50.0		SS5	50/2"	R	2 / 5			SW-SM	Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt. <i>(continued)</i>				
55											Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-27

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,983, E 874,918  
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Dense, brown, SILTY SAND and gravel (50%) (SM), dry, trace CaCO3, some cementation.				
5									SM					
10				SS1	17 16 15	31	18 / 18							
12.0	5186.1			Bulk										
15										Hard, light brown/white-gray, SANDY SILT (ML), some gravel, dry, some CaCO3, cementation.				
20				SS2	30 58/6'	R	12 / 12		ML					
23.0	5175.1			Bulk										
25										Very dense, brown, SAND and gravel (SW), dry.				
26.0	5172.1			SS3	20 24 33	57	18 / 18		CH	Hard, reddish brown, CLAY (CH), dry.				
27.5	5170.6			Bulk						Very stiff, light reddish brown, CLAYEY SILT (MH), some clay, dry, trace gravel.				
30														
35				SS4	10 7 13	20	18 / 18		MH					
40				Bulk										
40	5159.6			SS5	12 24 30	54	18 / 18		CL-ML	Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO3 inclusions.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-27

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,983, E 874,918  
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO3 inclusions. <i>(continued)</i>				
45			SS6		10 12 17	29	18 / 18							
			Bulk											
50	5148.1 50.0		SS7		14 20 27	47	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations





# REPORT OF BOREHOLE: BH-28

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-28 2 feet east  
 XY COORDINATES: N 11,975,241, E 870,785  
 ELEVATION: 5,388.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					GW	Dense, light gray, SANDY GRAVEL (GW), dry, few cobbles.				
5	5383.0 5.0		SS1	SS1	9 13 18	31			GP	Dense, light gray, SANDY GRAVEL (GP), dry.				
10	5378.0 10.0 5377.0 11.0		SS2	SS2	22 32 50	82			GW	Becomes very dense, white inclusions, sheen, dry.				
15	5374.5 13.5		SS3	SS3	30 30/3"	R			SM	Very dense, light gray, SANDY GRAVEL (GW), dry, few cobbles. Very dense, light brown and gray, gravelly SILTY SAND (SM), dry.				
20	5369.0 19.0		SS4	SS4	50/4"	R			SW	Very dense, light brown and gray, gravelly SAND (SW), sheen, dry.				
21	5367.0 21.0 5366.0 22.0		SS5	SS5	50/0"	R			GW	Seam of very dense, light gray, SANDY GRAVEL (GW), dry. Becomes light brown.				
22									SW	Very dense, light brown and gray, gravelly SAND, (SW) sheen, dry.				
22										Refusal at 22'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations

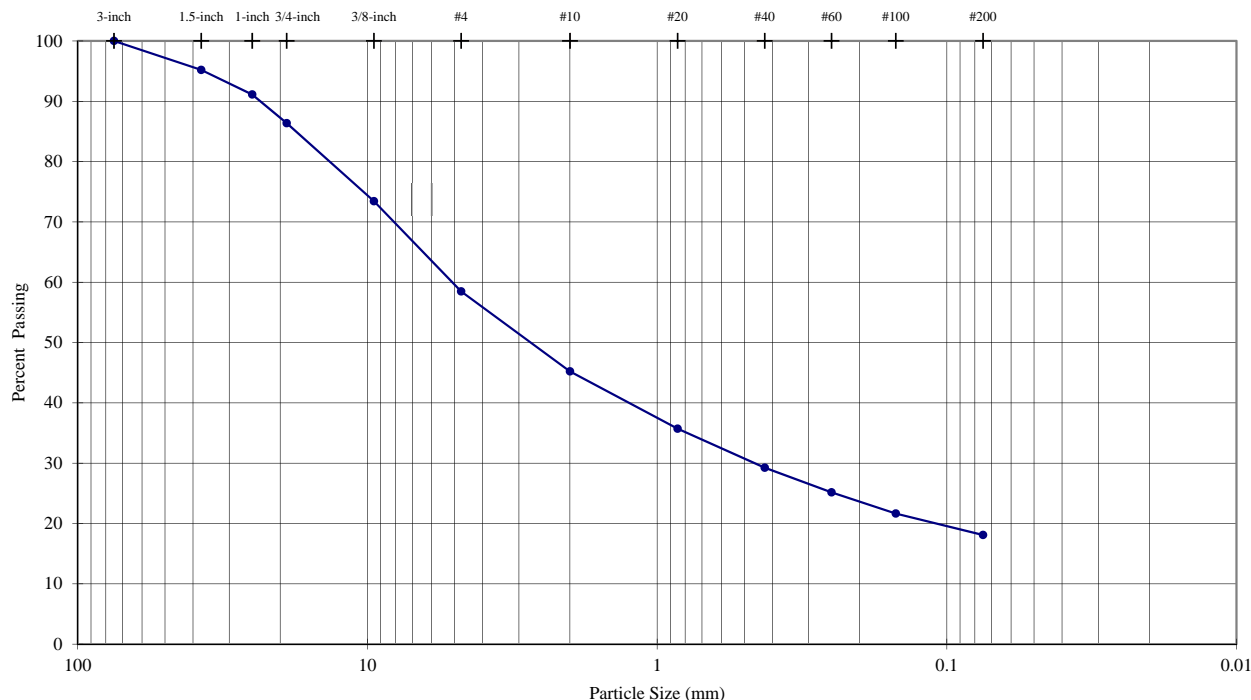
**APPENDIX A.3  
GEOTECHNICAL TEST RESULTS**

**APPENDIX A.3.1**  
**GRADATION MOISTURE/DENSITY TEST REPORTS**

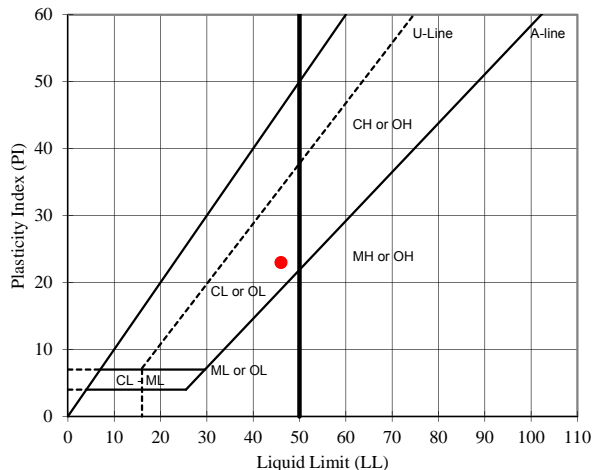
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-2**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-3.5**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>		
1.5-inch	37.5	<b>95.2</b>	Coarse Gravel	<b>13.63</b>
1-inch	25.0	<b>91.1</b>		
3/4-inch	19.0	<b>86.4</b>		
3/8-inch	9.5	<b>73.4</b>	Fine Gravel	<b>27.89</b>
#4	4.8	<b>58.5</b>	Coarse Sand	<b>13.26</b>
#10	2.00	<b>45.2</b>		
#20	0.85	<b>35.7</b>		
#40	0.43	<b>29.2</b>	Medium Sand	<b>15.98</b>
#60	0.25	<b>25.2</b>	Fine Sand	<b>11.15</b>
#100	0.15	<b>21.7</b>		
#200	0.075	<b>18.1</b>		
			Silt or Clay Fines	<b>18.10</b>



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, olive brown, dry

LL	PL	PI
<b>46</b>	<b>23</b>	<b>23</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**GC**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

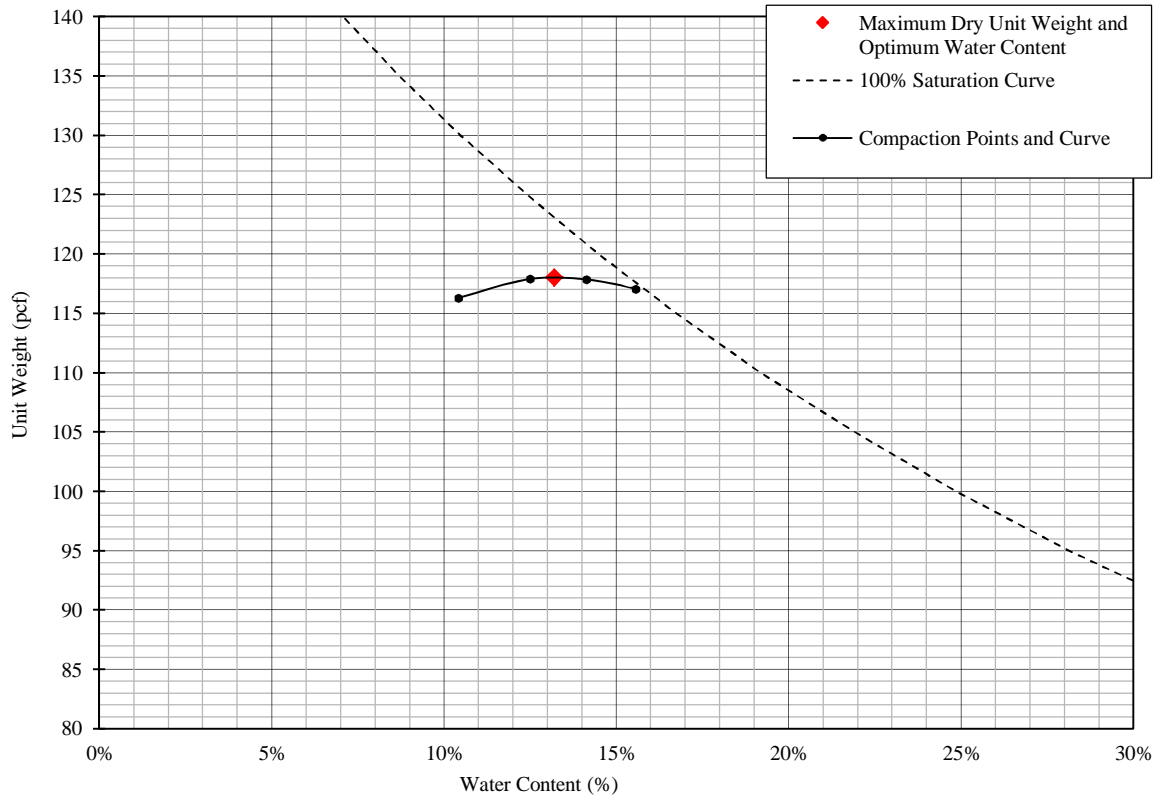
TECH MGC  
 DATE 2/22/2013  
 REVIEW MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method C

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-2**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-3.5**



% Test Fraction Passing 3/4-inch Sieve	<b>87%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.67</b>

Maximum Dry Unit Weight (pcf)	<b>118.0</b>
Optimum Water Content (%)	<b>13.2</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>122.4</b>
Corrected Optimum Water Content (%)	<b>11.5</b>

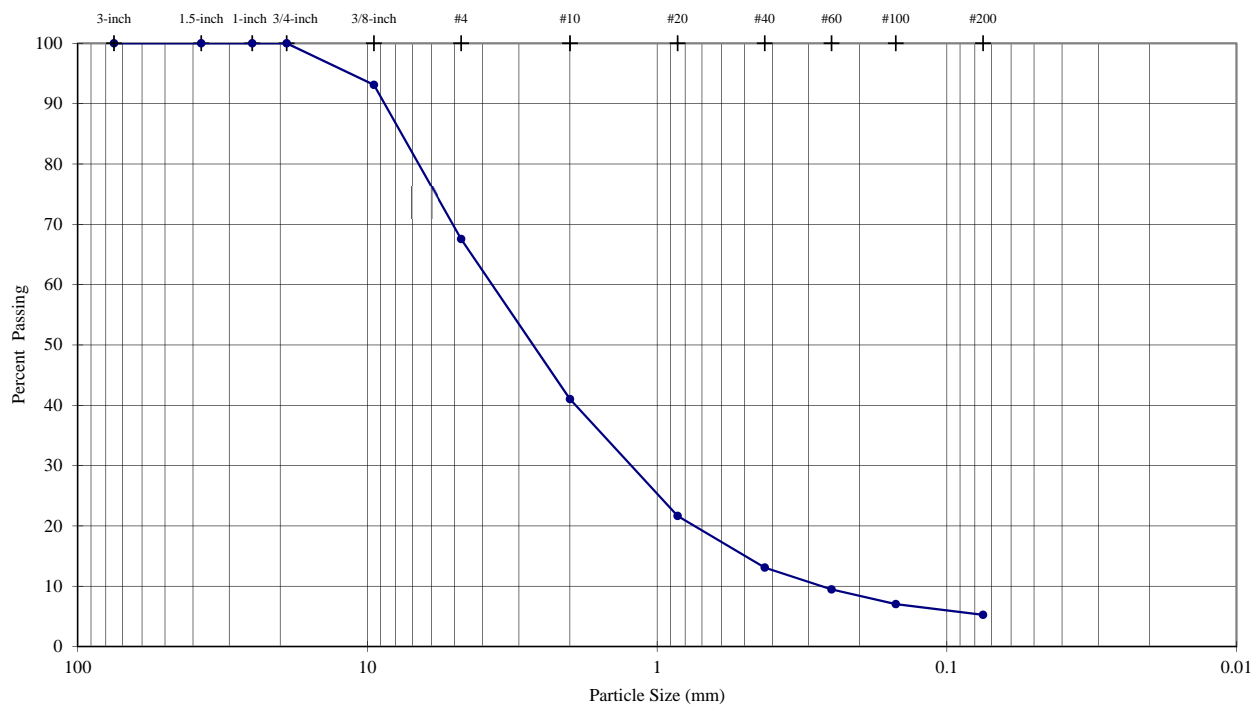
USCS Description (ASTM D 2487): Clayey gravel with sand, olive brown, dry

USCS GC

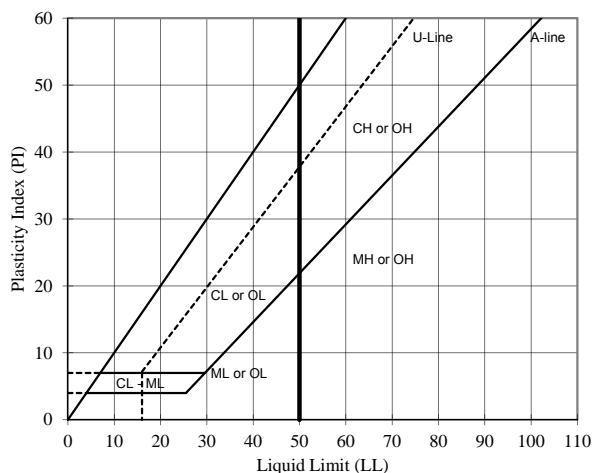
TECH	MGC
DATE	2-26-13
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-3** DEPTH (ft): **0-8**  
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	32.44
3/8-inch	9.5	93.1		
#4	4.8	67.6	Coarse Sand	26.53
#10	2.00	41.0		
#20	0.85	21.6		
#40	0.43	13.1	Medium Sand	27.93
#60	0.25	9.5		
#100	0.15	7.0	Fine Sand	7.83
#200	0.075	5.3		
			Silt or Clay Fines	5.26



Visual Description (Golder Procedure):  
**Gravelly SAND, some fines, greenish gray, dry**

LL	PL	PI
--	--	--

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 --

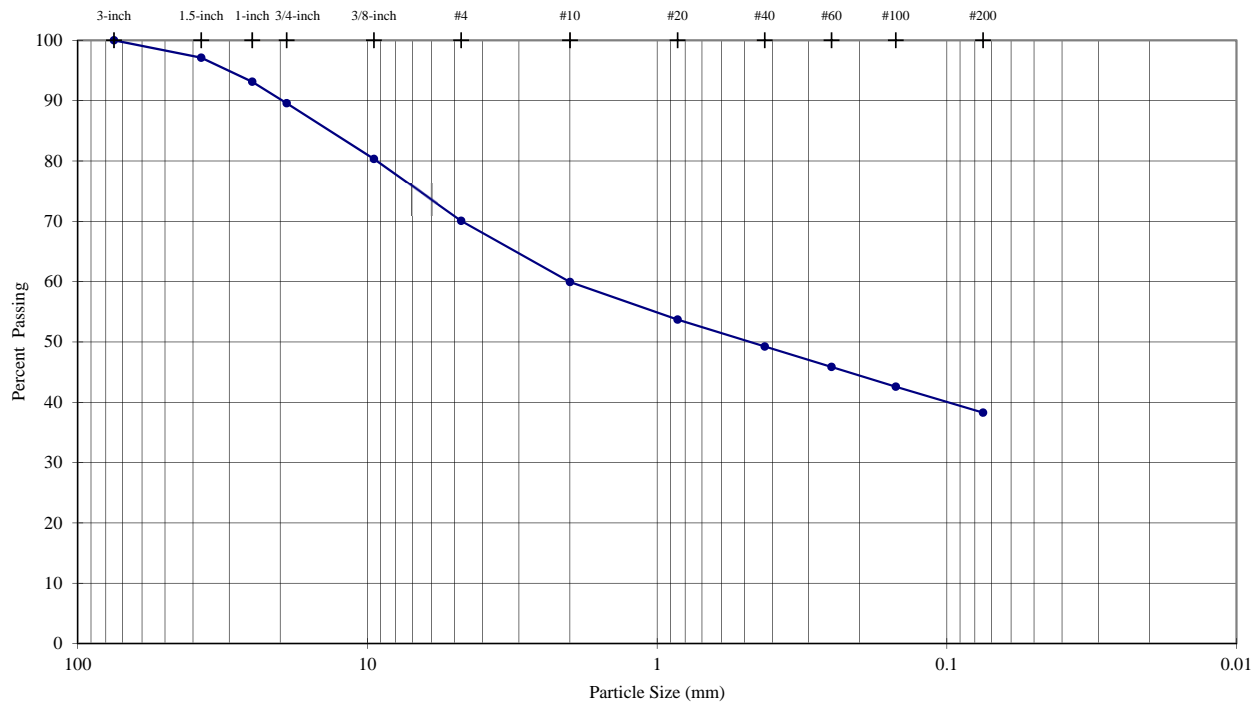
Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

TECH MGC  
 DATE 2/22/2013  
 REVIEW MB

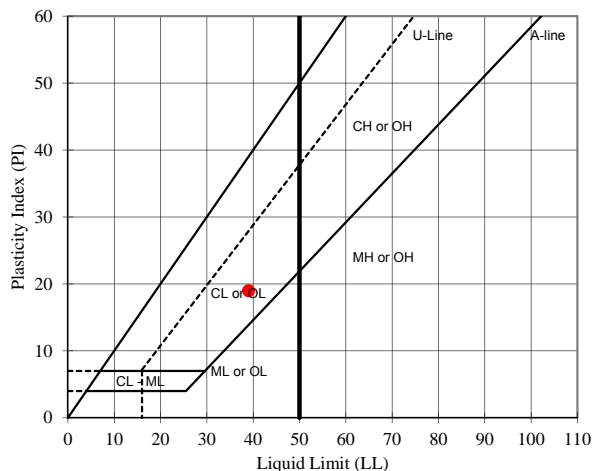
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10** DEPTH (ft): **0-14.5**  
 TYPE: **Pail/Bag**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	10.43
1.5-inch	37.5	97.1		
1-inch	25.0	93.1		
3/4-inch	19.0	89.6	Fine Gravel	19.50
3/8-inch	9.5	80.3		
#4	4.8	70.1	Coarse Sand	10.14
#10	2.00	59.9		
#20	0.85	53.7		
#40	0.43	49.2	Medium Sand	10.69
#60	0.25	45.9		
#100	0.15	42.6	Fine Sand	10.96
#200	0.075	38.3		
			Silt or Clay Fines	38.28



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, light yellowish brown, dry

LL	PL	PI
39	20	19

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

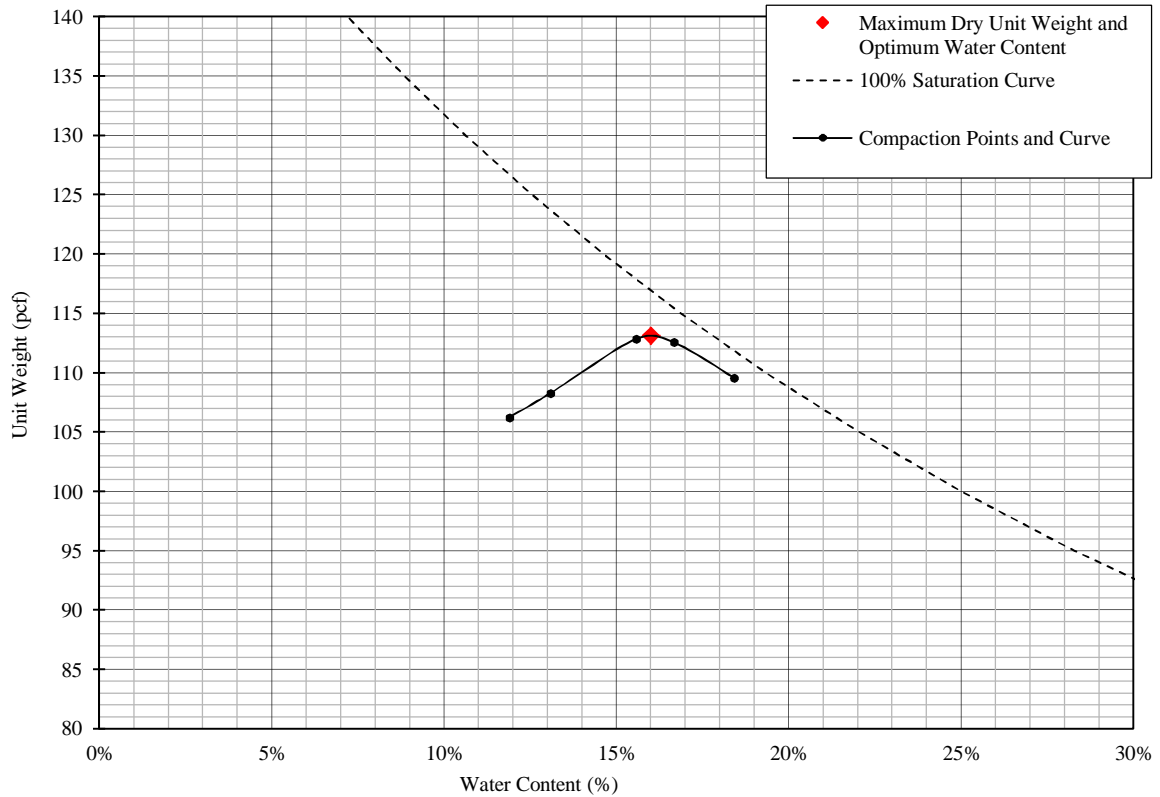
TECH: AMS/MGC  
 DATE: 2/25/2013  
 REVIEW: MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer    Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-14.5**



% Test Fraction Passing 3/8-inch Sieve	<b>81%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.68</b>

Maximum Dry Unit Weight (pcf)	<b>113.1</b>
Optimum Water Content (%)	<b>16.0</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>120.0</b>
Corrected Optimum Water Content (%)	<b>13.2</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, light yellowish brown, dry

USCS    SC

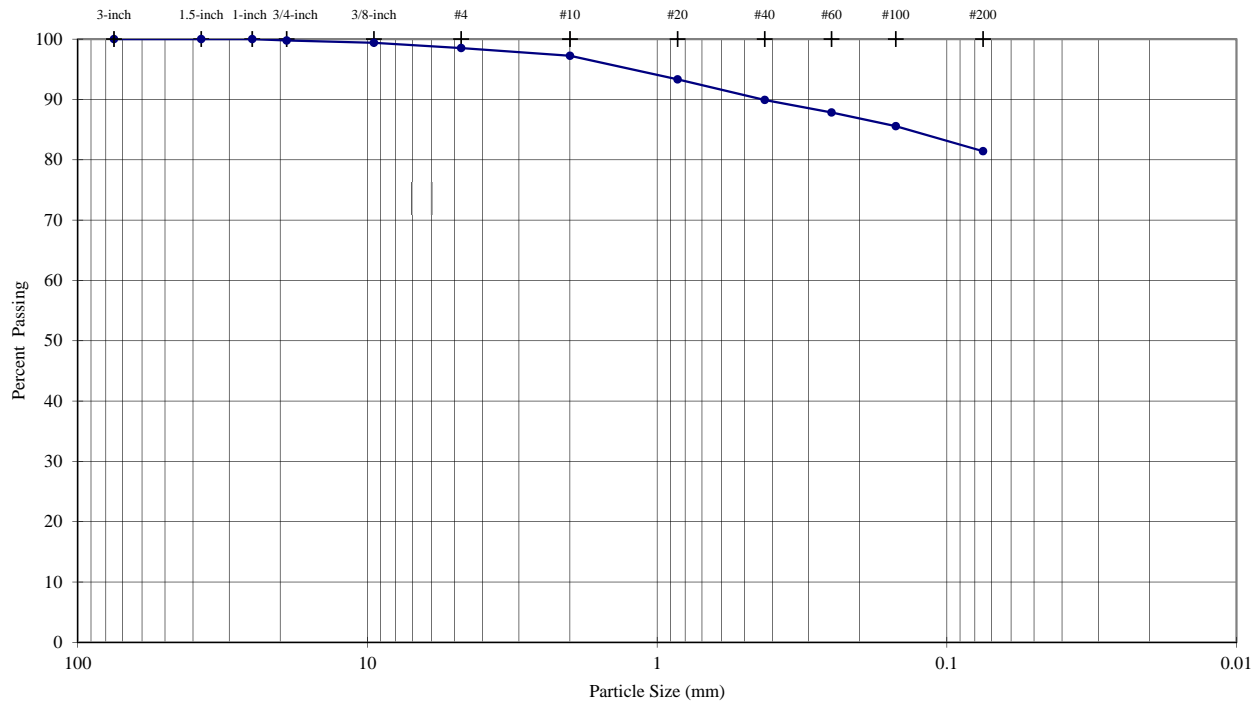
TECH	AMS
DATE	2-27-13
REVIEW	MB



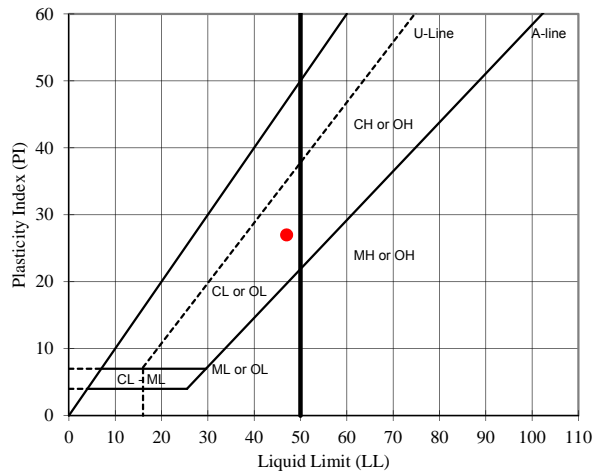
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10**  
 TYPE: **Pail**

DEPTH (ft): **19-33**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.23
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.8	Fine Gravel	1.25
3/8-inch	9.5	99.4		
#4	4.8	98.5	Coarse Sand	1.28
#10	2.00	97.2		
#20	0.85	93.3	Medium Sand	7.32
#40	0.43	89.9		
#60	0.25	87.8	Fine Sand	8.49
#100	0.15	85.6		
#200	0.075	81.4		
			Silt or Clay Fines	81.42



USCS Description (ASTM D 2487):

Lean clay with sand, yellowish red, dry

LL	PL	PI
47	20	27

As-Received Moisture Content (%)

14.4

USCS Group Symbol

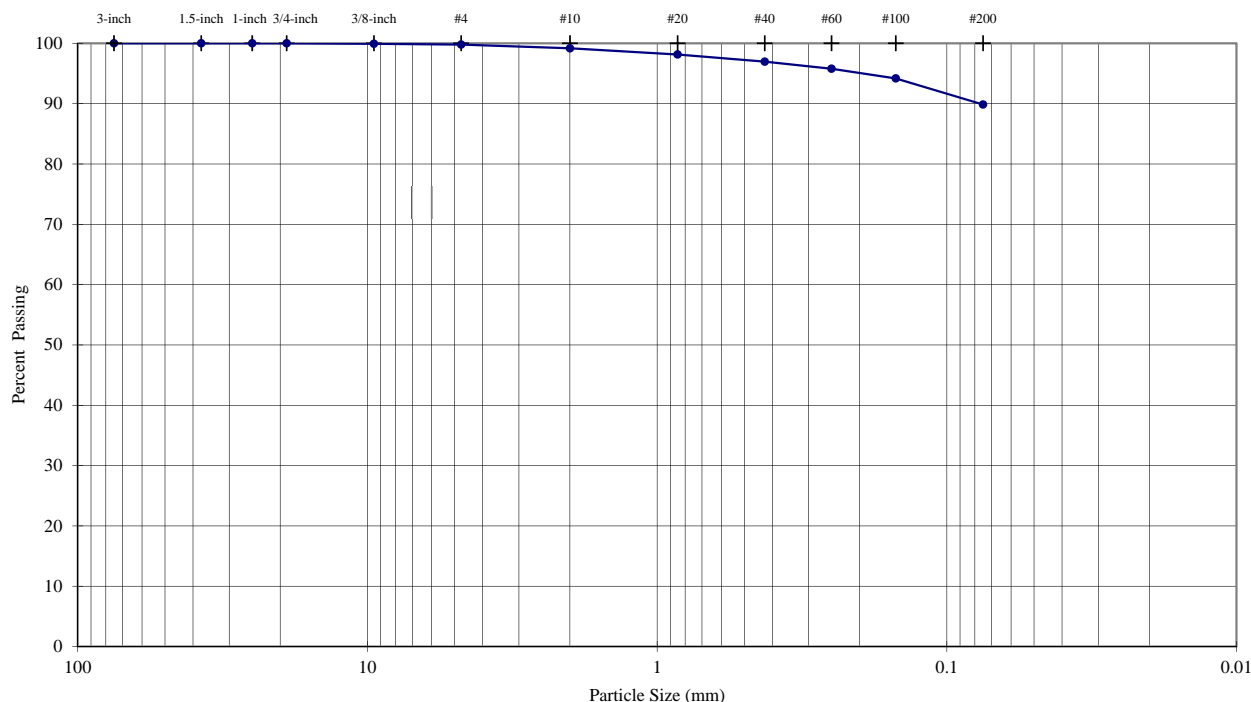
CL

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

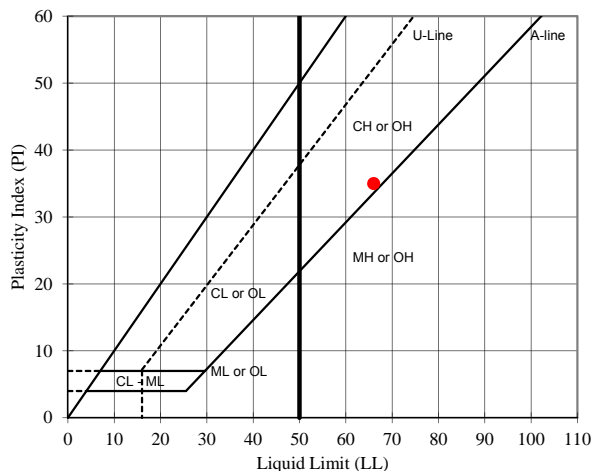
TECH	AMS
DATE	2/25/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-12** DEPTH (ft): **33.5-48.5**  
 TYPE: **Pail**



Sieve Analysis (Initial Separation on No. 4 Sieve)	Particle Size		Description	Percentage	
	Sieve	(mm)			% Passing
	3-inch	75.0	100.0	Coarse Gravel	0.00
	1.5-inch	37.5	100.0		
	1-inch	25.0	100.0		
	3/4-inch	19.0	100.0	Fine Gravel	0.21
	3/8-inch	9.5	99.9		
	#4	4.8	99.8	Coarse Sand	0.62
	#10	2.00	99.2		
	#20	0.85	98.1		
	#40	0.43	97.0	Medium Sand	2.21
	#60	0.25	95.8		
	#100	0.15	94.2	Fine Sand	7.10
	#200	0.075	89.9		
				Silt or Clay Fines	89.86



USCS Description (ASTM D 2487):  
 Fat clay, dark red, moist

LL	PL	PI
66	31	35

As-Received Moisture Content (%)  
 --

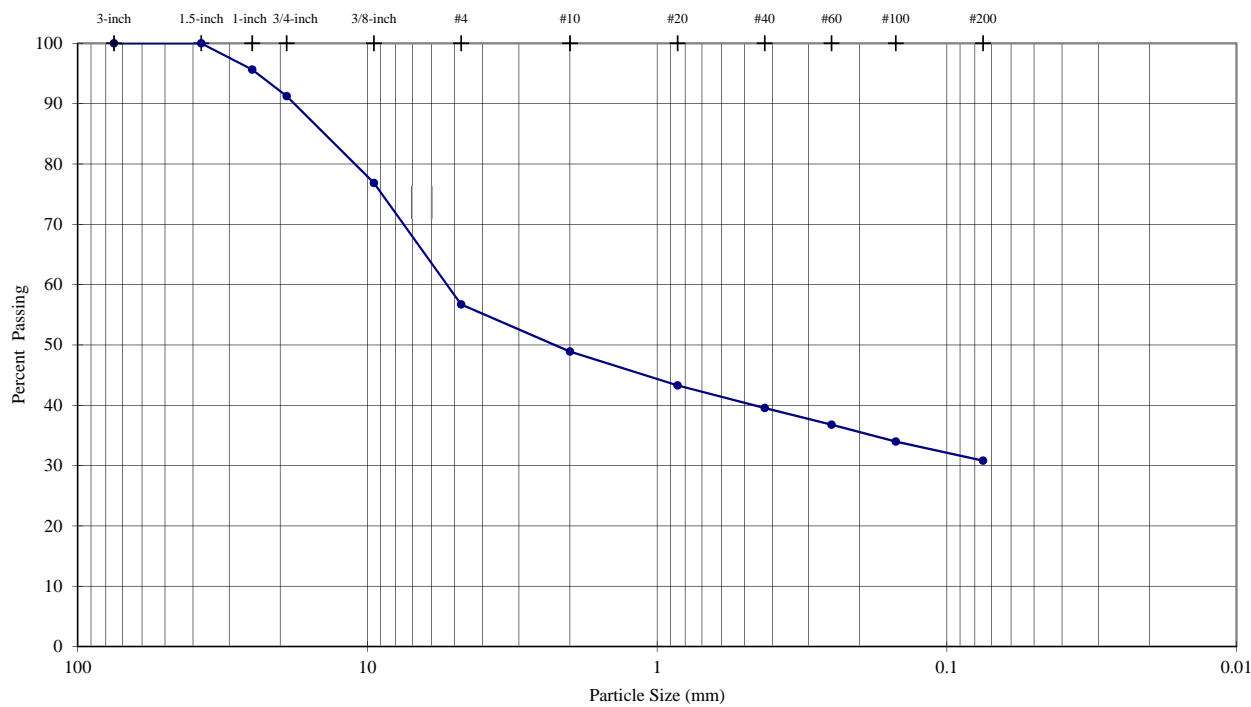
USCS Group Symbol  
 CH

Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

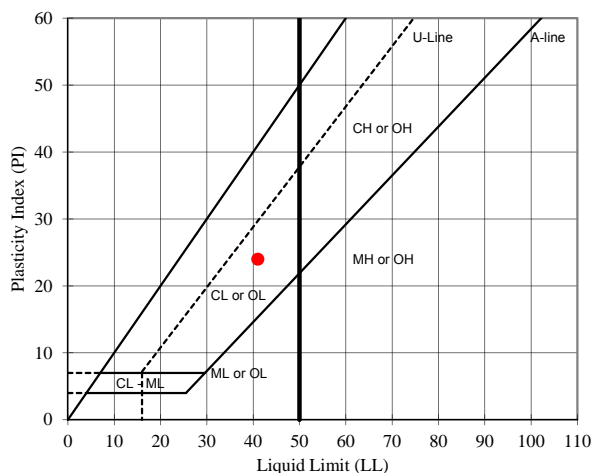
TECH AM  
 DATE 2/26/2013  
 REVIEW MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-16** DEPTH (ft): **0-8.5**  
 TYPE: **Pail**



Sieve Analysis (Initial Separation on No. 4 Sieve)	Particle Size		Description	Percentage
	Sieve	(mm)		
	3-inch	75.0	100.0	Coarse Gravel
	1.5-inch	37.5	100.0	
	1-inch	25.0	95.7	Coarse Gravel
	3/4-inch	19.0	91.2	
	3/8-inch	9.5	76.9	Fine Gravel
	#4	4.8	56.7	
	#10	2.00	48.9	Coarse Sand
	#20	0.85	43.3	
	#40	0.43	39.5	Medium Sand
	#60	0.25	36.8	
	#100	0.15	34.0	Fine Sand
	#200	0.075	30.8	
				Silt or Clay Fines
				30.81



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, reddish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 GC

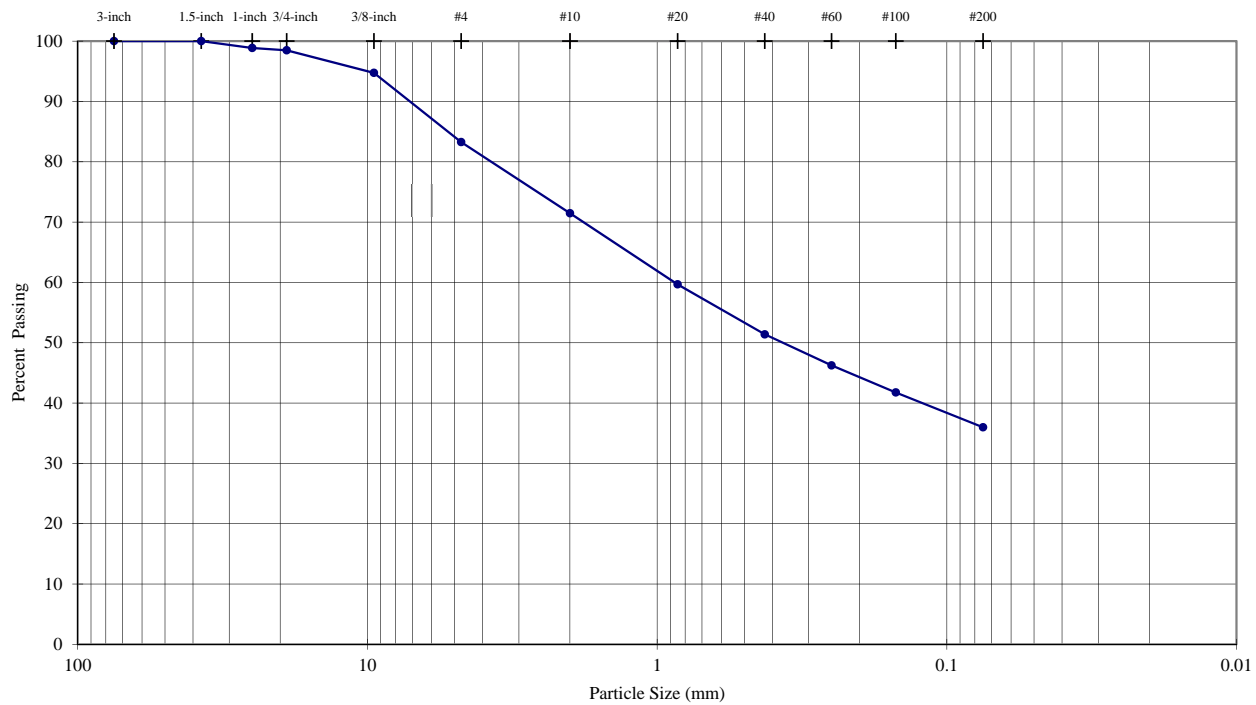
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/27/2013
REVIEW	MB

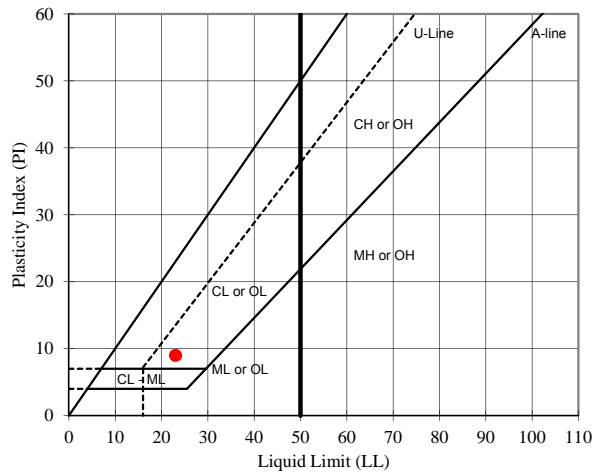
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-16**  
 TYPE: **Pail**

DEPTH (ft): **29-34**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	1.52
1.5-inch	37.5	100.0		
1-inch	25.0	98.9		
3/4-inch	19.0	98.5	Fine Gravel	15.22
3/8-inch	9.5	94.7		
#4	4.8	83.3	Coarse Sand	11.79
#10	2.00	71.5		
#20	0.85	59.7	Medium Sand	20.08
#40	0.43	51.4		
#60	0.25	46.2	Fine Sand	15.41
#100	0.15	41.8		
#200	0.075	36.0	Silt or Clay Fines	35.98



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, yellowish brown, dry

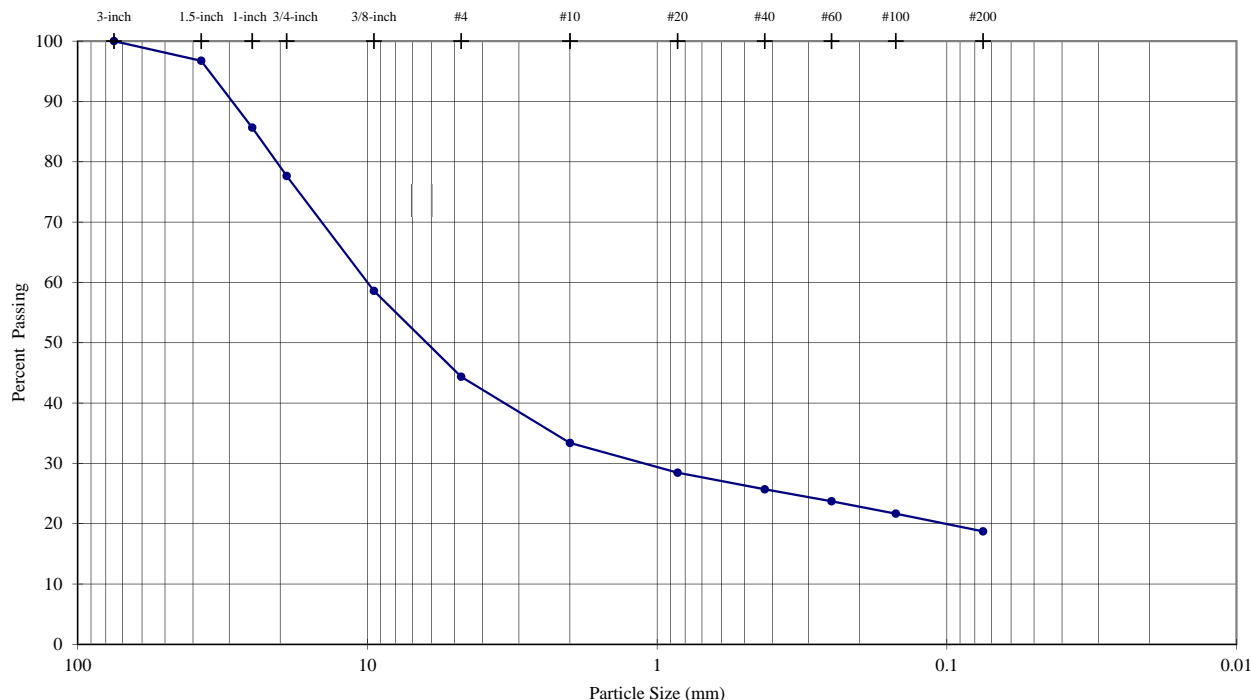
LL	PL	PI
23	14	9
As-Received Moisture Content (%)		
--		
USCS Group Symbol		
SC		

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

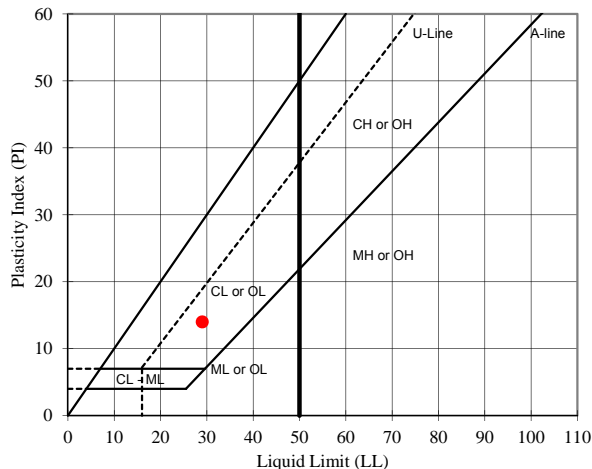
TECH: **EH**  
 DATE: **2/26/2013**  
 REVIEW: **MB**

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18** DEPTH (ft): **2-8**  
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	22.35
1.5-inch	37.5	96.7		
1-inch	25.0	85.7		
3/4-inch	19.0	77.6	Fine Gravel	33.28
3/8-inch	9.5	58.6		
#4	4.8	44.4	Coarse Sand	10.98
#10	2.00	33.4		
#20	0.85	28.5	Medium Sand	7.68
#40	0.43	25.7		
#60	0.25	23.7	Fine Sand	6.98
#100	0.15	21.7		
#200	0.075	18.7	Silt or Clay Fines	18.73



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, yellowish brown, dry

LL	PL	PI
29	15	14

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 GC

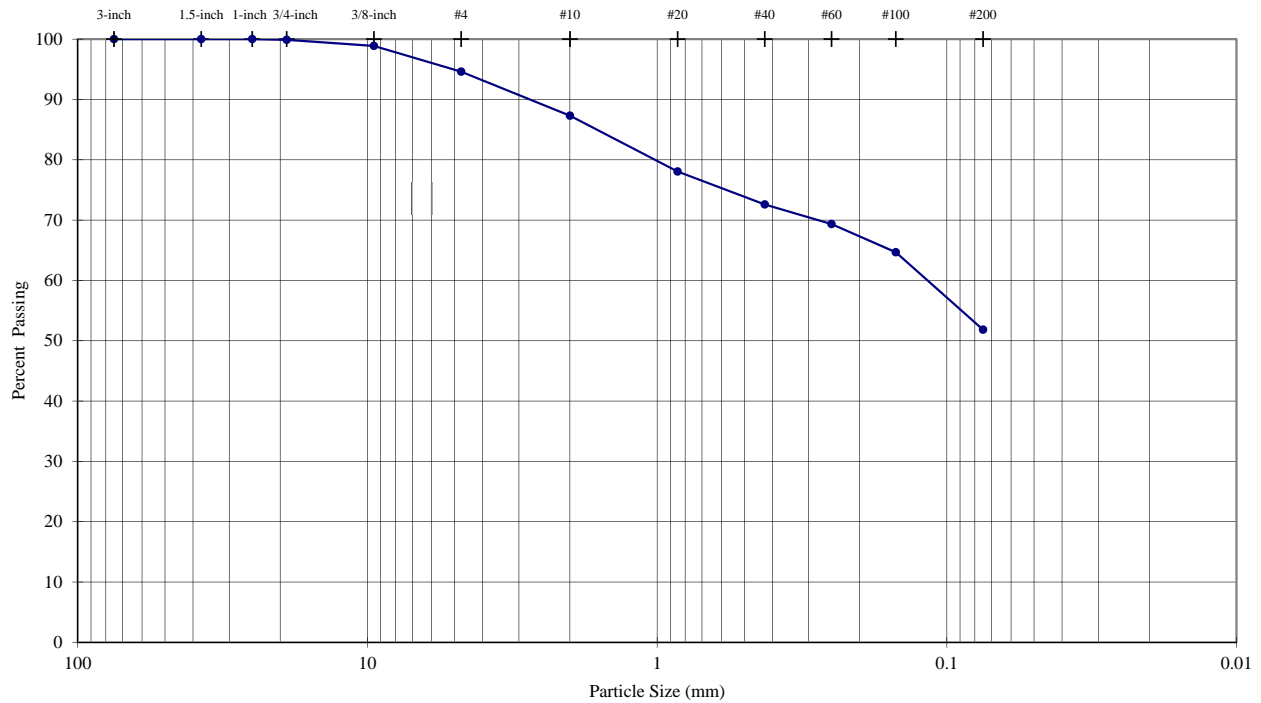
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AMS  
 DATE 2/28/2013  
 REVIEW MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

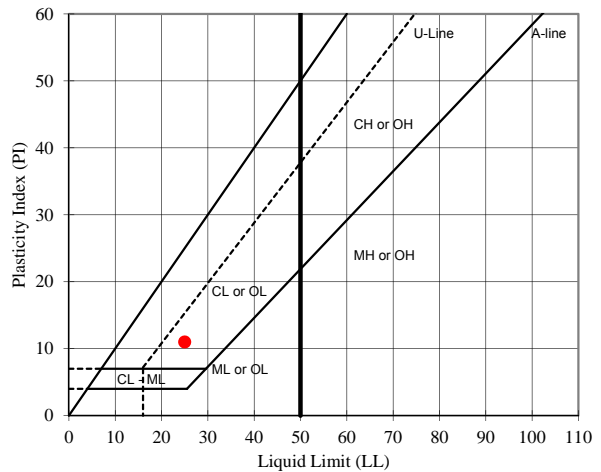
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18**  
 TYPE: **Pail**

DEPTH (ft): **23-33.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>0.12</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>100.0</b>		
3/4-inch	19.0	<b>99.9</b>	Fine Gravel	<b>5.27</b>
3/8-inch	9.5	<b>98.9</b>		
#4	4.8	<b>94.6</b>	Coarse Sand	<b>7.30</b>
#10	2.00	<b>87.3</b>		
#20	0.85	<b>78.1</b>	Medium Sand	<b>14.72</b>
#40	0.43	<b>72.6</b>		
#60	0.25	<b>69.3</b>	Fine Sand	<b>20.76</b>
#100	0.15	<b>64.7</b>		
#200	0.075	<b>51.8</b>		
			Silt or Clay Fines	<b>51.84</b>

Sieve Analysis (Initial Separation on No. 4 Sieve)



USCS Description (ASTM D 2487):  
 Sandy lean clay, reddish brown, moist

LL	PL	PI
<b>25</b>	<b>14</b>	<b>11</b>

As-Received Moisture Content (%)

**#DIV/0!**

USCS Group Symbol

**CL**

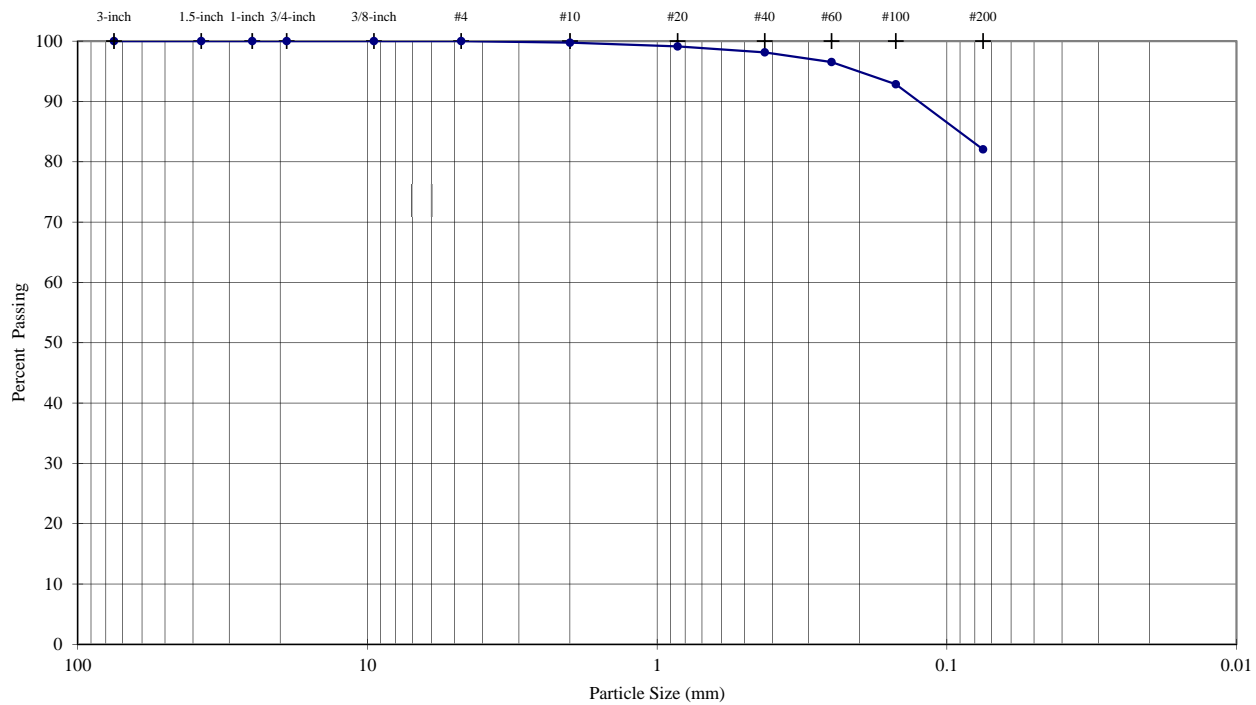
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH **AMS**  
 DATE **2/25/2013**  
 REVIEW **MB**

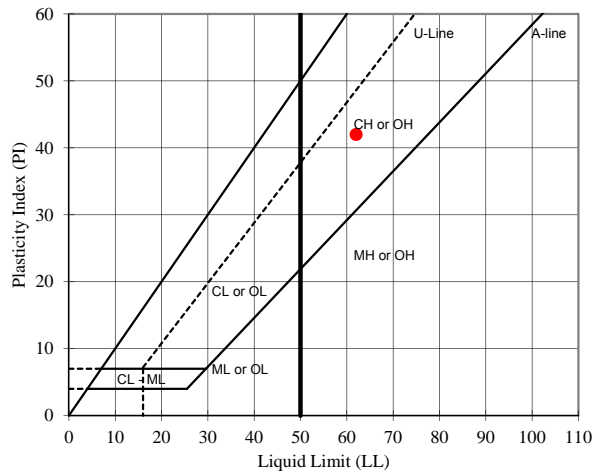
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18**  
 TYPE: **Bag**

DEPTH (ft): **43.5-48.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>0.00</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>100.0</b>		
3/4-inch	19.0	<b>100.0</b>		
3/8-inch	9.5	<b>100.0</b>	Fine Gravel	<b>0.00</b>
#4	4.8	<b>100.0</b>	Coarse Sand	<b>0.25</b>
#10	2.00	<b>99.7</b>		
#20	0.85	<b>99.1</b>		
#40	0.43	<b>98.1</b>	Medium Sand	<b>1.61</b>
#60	0.25	<b>96.5</b>		
#100	0.15	<b>92.9</b>	Fine Sand	<b>16.07</b>
#200	0.075	<b>82.1</b>		
			Silt or Clay Fines	<b>82.06</b>



USCS Description (ASTM D 2487):  
 Fat clay with sand, dark red, wet

LL	PL	PI
<b>62</b>	<b>20</b>	<b>42</b>

As-Received Moisture Content (%)  
**29.6**

USCS Group Symbol  
**CH**

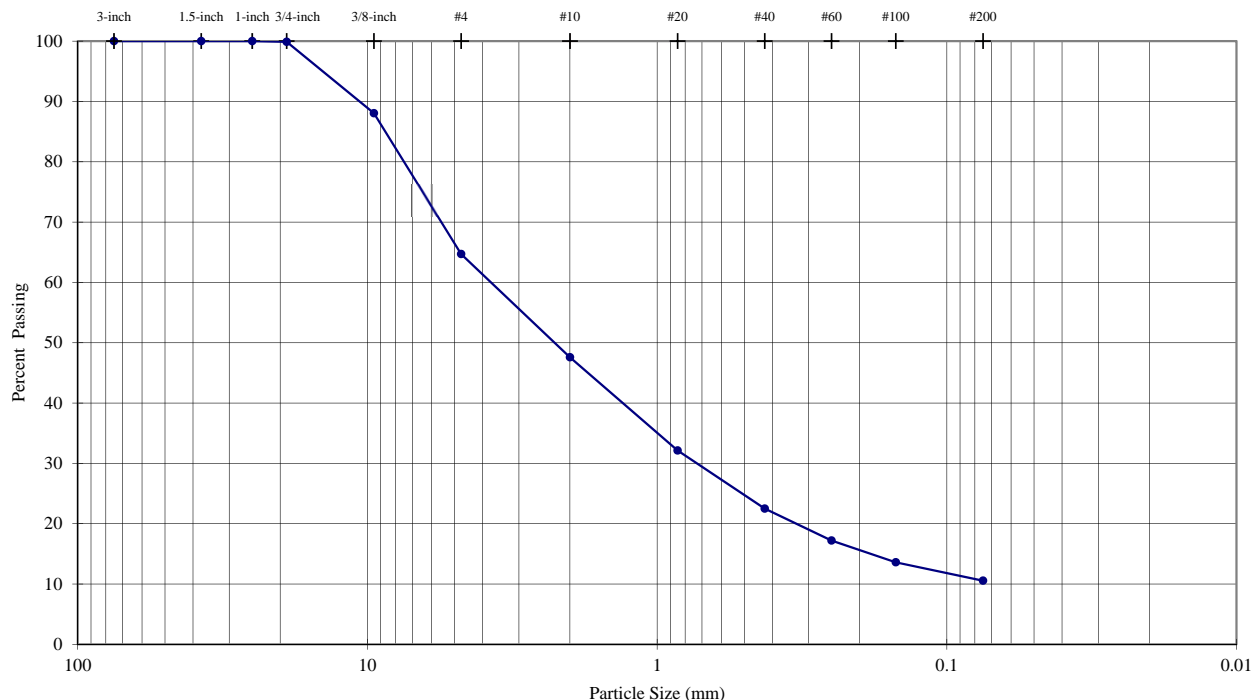
Notes: 0g of particles up to 4.8mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the wet method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AMS  
 DATE 2/25/2013  
 REVIEW MB

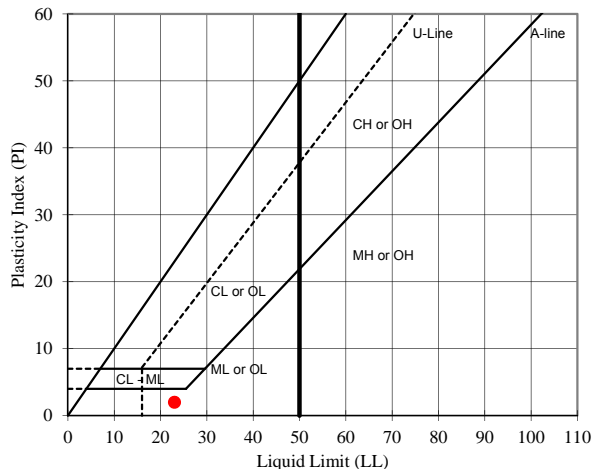
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-21**  
 TYPE: **Pail**

DEPTH (ft): **0-18**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.12
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9		
3/8-inch	9.5	88.1	Fine Gravel	35.18
#4	4.8	64.7	Coarse Sand	17.11
#10	2.00	47.6		
#20	0.85	32.1		
#40	0.43	22.5	Medium Sand	25.09
#60	0.25	17.2		
#100	0.15	13.6	Fine Sand	11.95
#200	0.075	10.6		
			Silt or Clay Fines	10.55



USCS Description (ASTM D 2487):

Well-graded sand with silt and gravel, yellowish red, dry

LL	PL	PI
23	21	2

As-Received Moisture Content (%)

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USCS Group Symbol

SW-SM

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/26/2013
REVIEW	MB

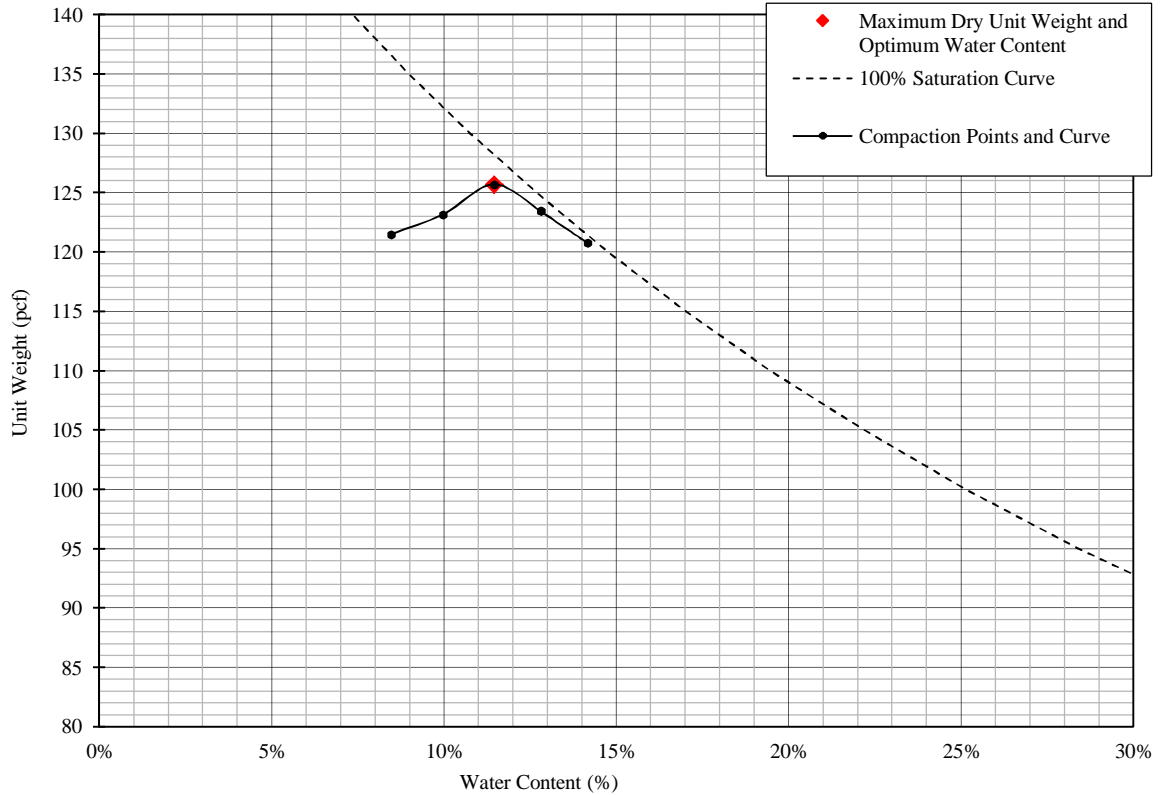


## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-21**  
 TYPE: **Pail**

DEPTH (ft): **0-18**



% Test Fraction Passing 3/8-inch Sieve	<b>88%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.69</b>

Maximum Dry Unit Weight (pcf)	<b>125.7</b>
Optimum Water Content (%)	<b>11.4</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>129.2</b>
Corrected Optimum Water Content (%)	<b>10.1</b>

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, yellowish red, dry

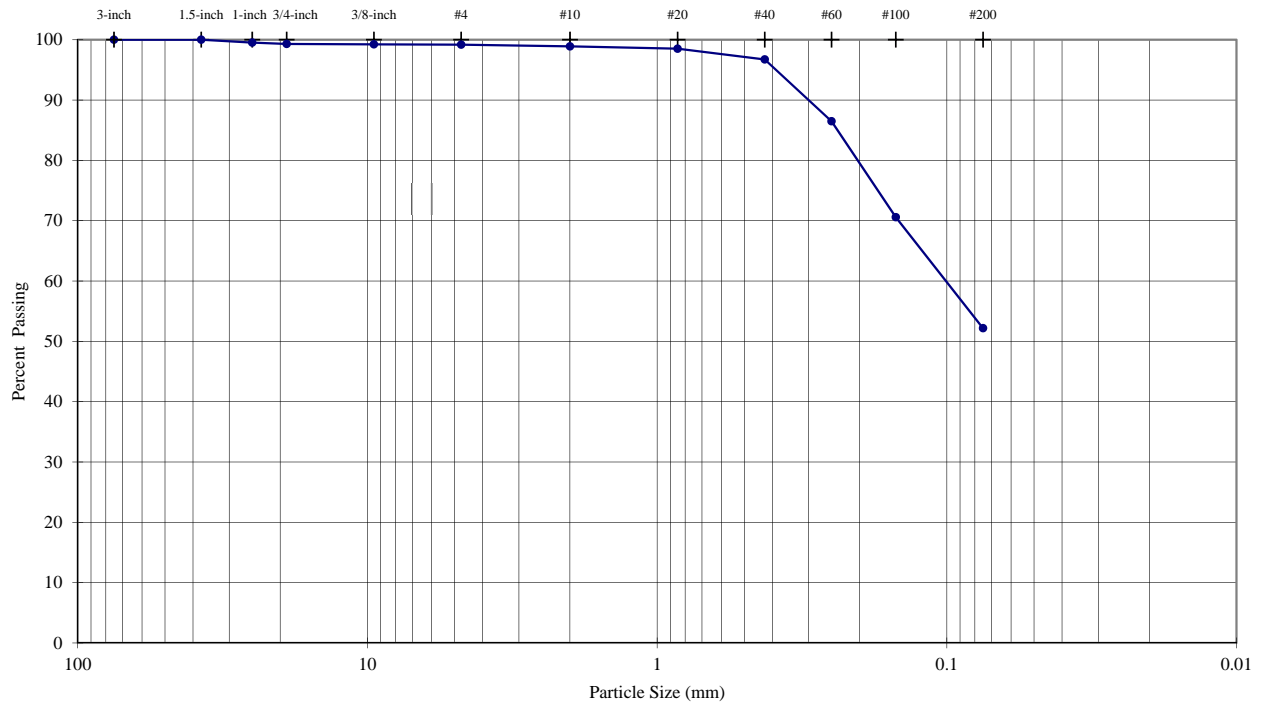
USCS SW-SM

TECH	AMS
DATE	2-27-13
REVIEW	MB

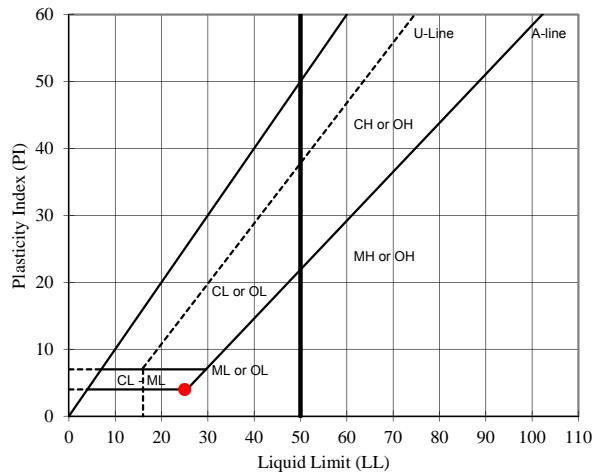
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
 ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-22**  
 TYPE: **Pail**

DEPTH (ft): **5-8.5**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.70
1.5-inch	37.5	100.0		
1-inch	25.0	99.5		
3/4-inch	19.0	99.3	Fine Gravel	0.12
3/8-inch	9.5	99.2		
#4	4.8	99.2	Coarse Sand	0.28
#10	2.00	98.9		
#20	0.85	98.5	Medium Sand	2.17
#40	0.43	96.7		
#60	0.25	86.5	Fine Sand	44.55
#100	0.15	70.6		
#200	0.075	52.2		
			Silt or Clay Fines	52.18



USCS Description (ASTM D 2487):  
**Sandy silty clay, brownish yellow, moist**

LL	PL	PI
25	21	4

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**CL-ML**

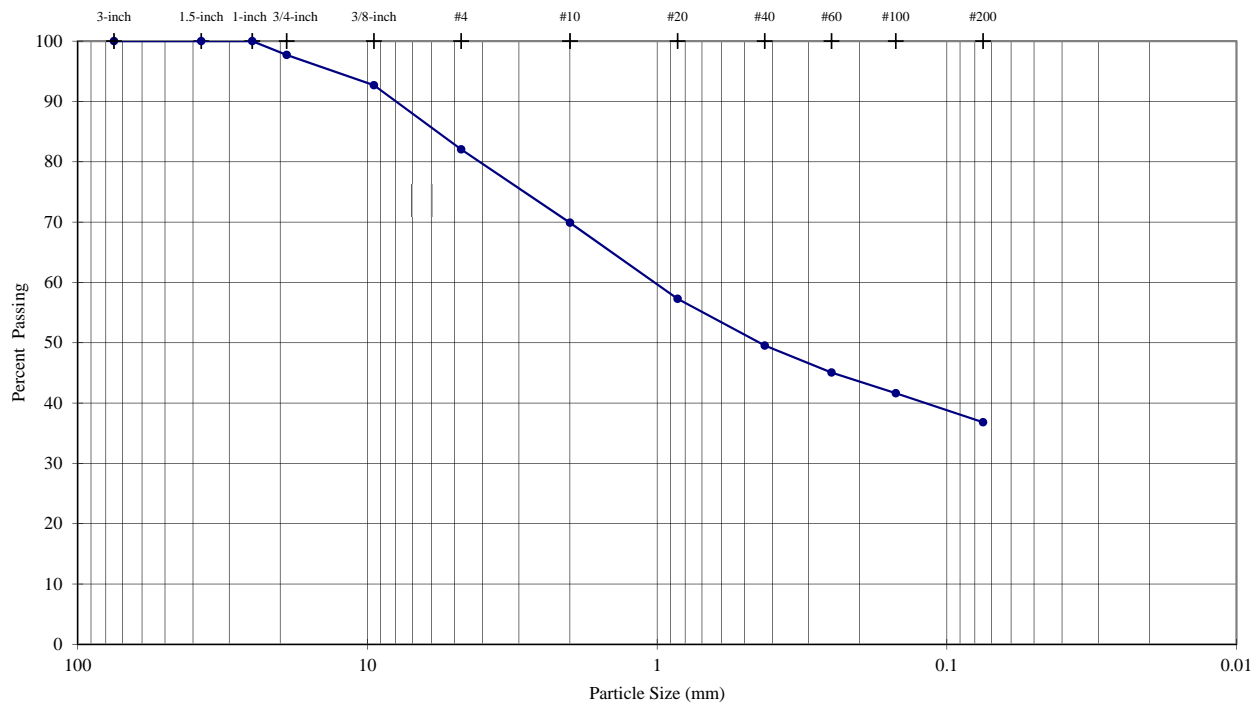
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH: **EH**  
 DATE: **2/26/2013**  
 REVIEW: **MB**

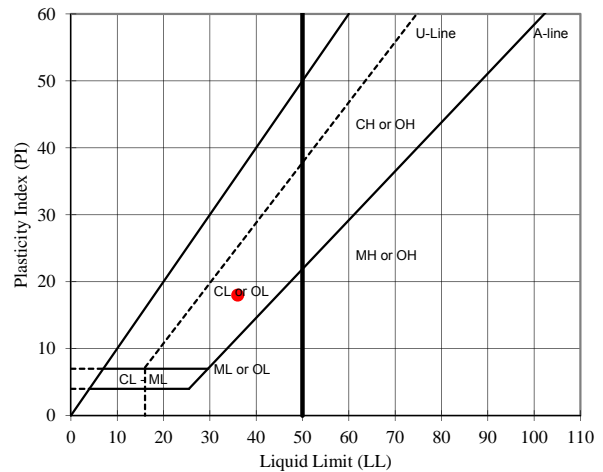
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-22**  
 TYPE: **Bag**

DEPTH (ft): **28-30**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	2.29
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	97.7		
3/8-inch	9.5	92.7	Fine Gravel	15.65
#4	4.8	82.1	Coarse Sand	12.16
#10	2.00	69.9		
#20	0.85	57.3		
#40	0.43	49.5	Medium Sand	20.36
#60	0.25	45.1		
#100	0.15	41.6	Fine Sand	12.71
#200	0.075	36.8		
			Silt or Clay Fines	36.82



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, reddish brown, moist

LL	PL	PI
36	18	18

As-Received Moisture Content (%)  
**10.4**

USCS Group Symbol  
**SC**

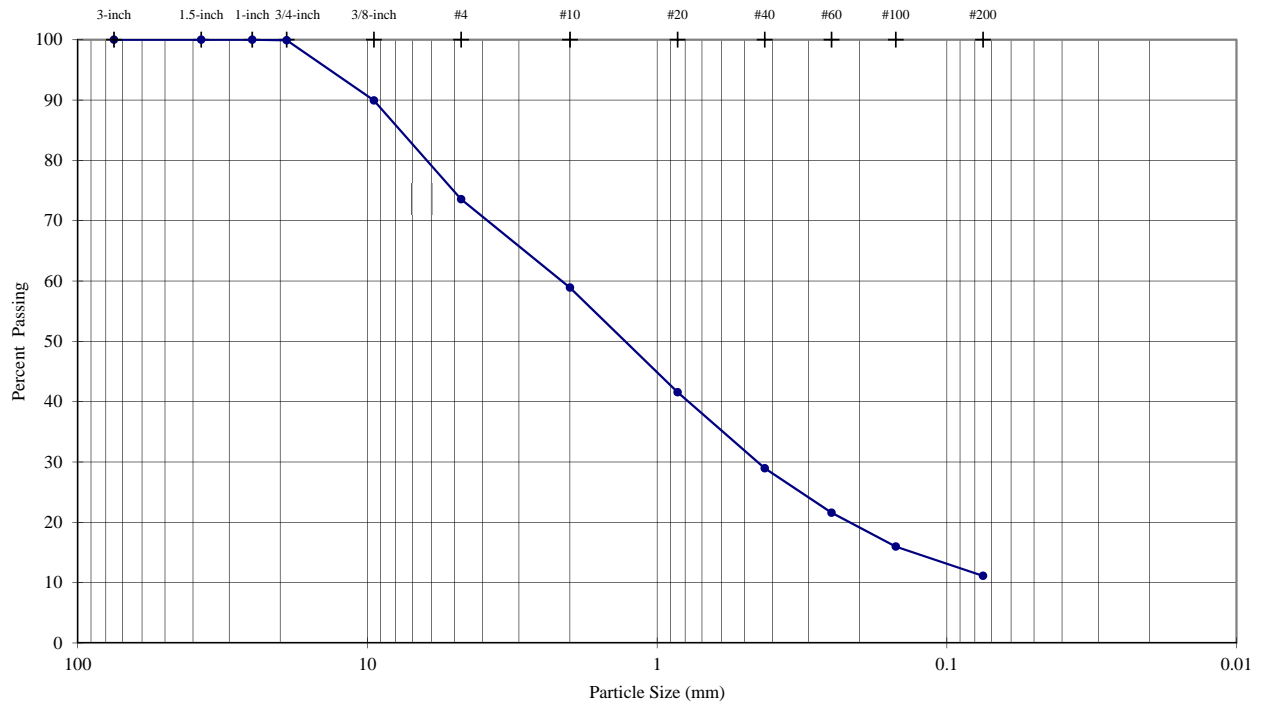
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH: AMS  
 DATE: 2/25/2013  
 REVIEW: MB

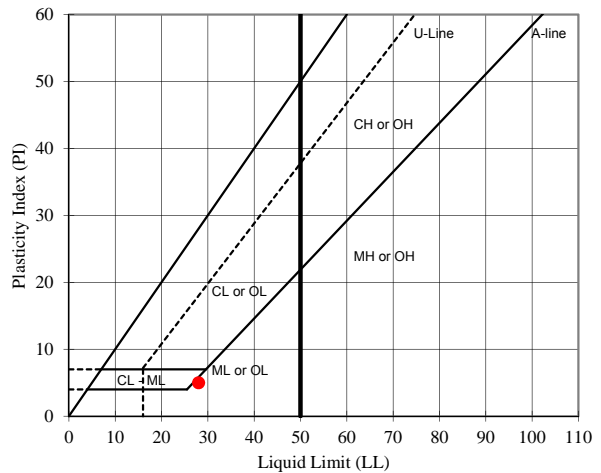
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-24**  
 TYPE: **Pail**

DEPTH (ft): **2.5-18**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.09
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	26.35
3/8-inch	9.5	89.9		
#4	4.8	73.6	Coarse Sand	14.65
#10	2.0	58.9		
#20	0.85	41.6	Medium Sand	29.96
#40	0.43	29.0		
#60	0.25	21.6	Fine Sand	17.83
#100	0.15	16.0		
#200	0.075	11.1		
			Silt or Clay Fines	11.12



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, weak red, dry

LL	PL	PI
28	23	5

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SW-SM

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

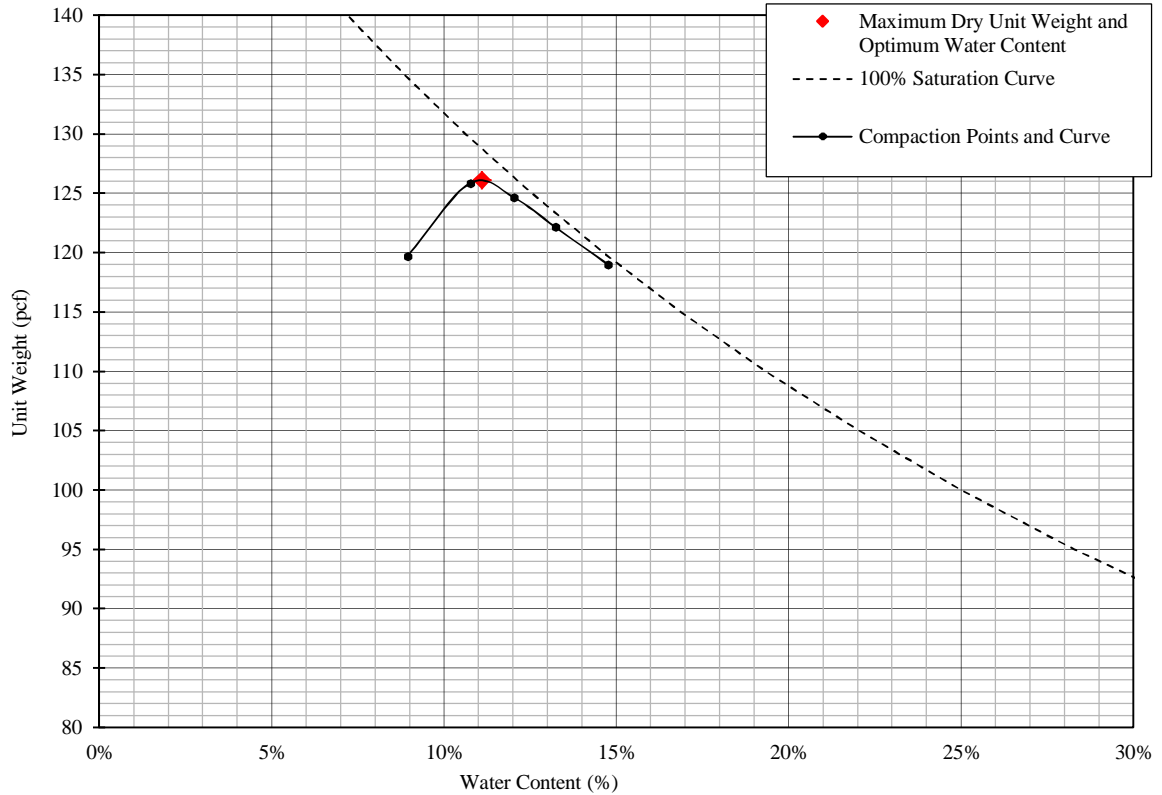
TECH	AMS
DATE	2/27/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer    Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-24**  
 TYPE: **Pail**

DEPTH (ft): **2.5-18**



% Test Fraction Passing 3/8-inch Sieve	<b>90%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.68</b>

Maximum Dry Unit Weight (pcf)	<b>126.1</b>
Optimum Water Content (%)	<b>11.1</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>128.9</b>
Corrected Optimum Water Content (%)	<b>10.0</b>

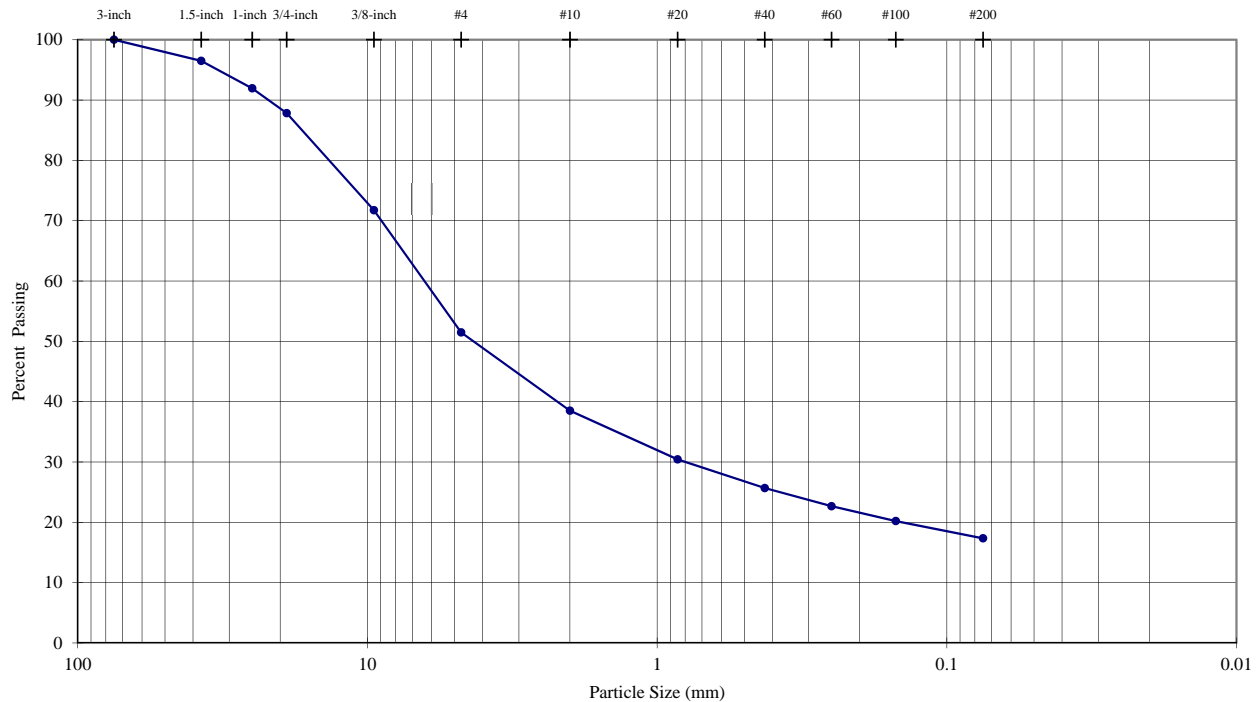
USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, weak red, dry

USCS: SW-SM

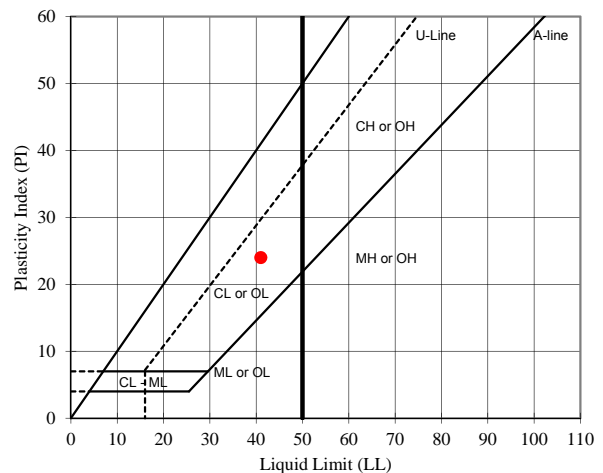
TECH	AMS
DATE	2-27-13
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
 ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-25** DEPTH (ft): **0-12.5**  
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	12.17
1.5-inch	37.5	96.5		
1-inch	25.0	91.9		
3/4-inch	19.0	87.8	Fine Gravel	36.36
3/8-inch	9.5	71.7		
#4	4.8	51.5	Coarse Sand	12.97
#10	2.00	38.5		
#20	0.85	30.4	Medium Sand	12.83
#40	0.43	25.7		
#60	0.25	22.7	Fine Sand	8.33
#100	0.15	20.2		
#200	0.075	17.3		
			Silt or Clay Fines	17.34



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, dark yellowish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 GC

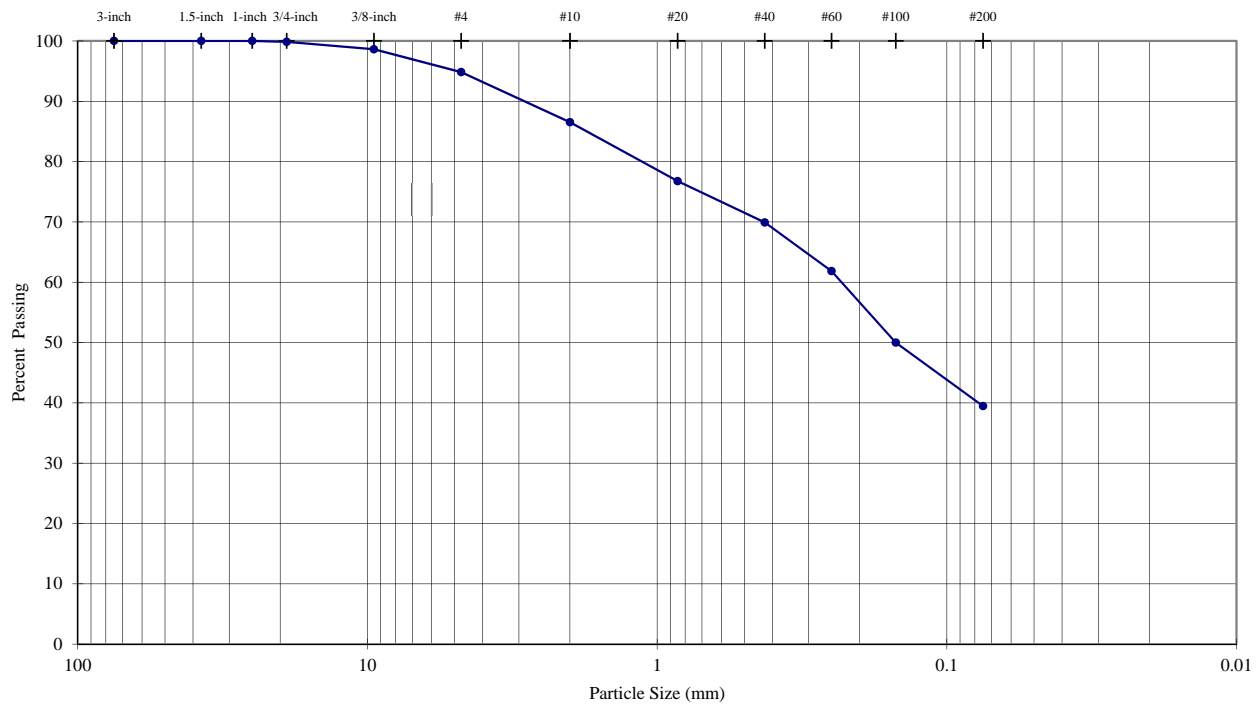
Notes: Og of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH: AMS  
 DATE: 2/27/2013  
 REVIEW: MB

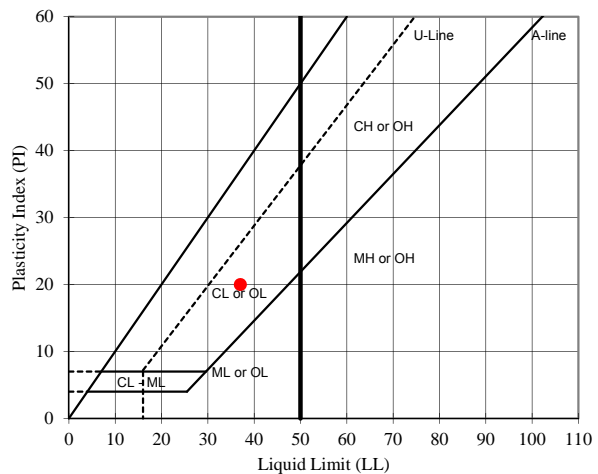
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-25**  
 TYPE: **Pail**

DEPTH (ft): **22-34**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.16
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.8	Fine Gravel	5.01
3/8-inch	9.5	98.6		
#4	4.8	94.8	Coarse Sand	8.31
#10	2.00	86.5		
#20	0.85	76.8		
#40	0.43	69.9	Medium Sand	16.62
#60	0.25	61.8		
#100	0.15	50.0	Fine Sand	30.43
#200	0.075	39.5		
Silt or Clay Fines				39.47



USCS Description (ASTM D 2487):  
 Clayey sand, reddish brown, moist

LL	PL	PI
37	17	20

As-Received Moisture Content (%)  
**#DIV/0!**

USCS Group Symbol  
**SC**

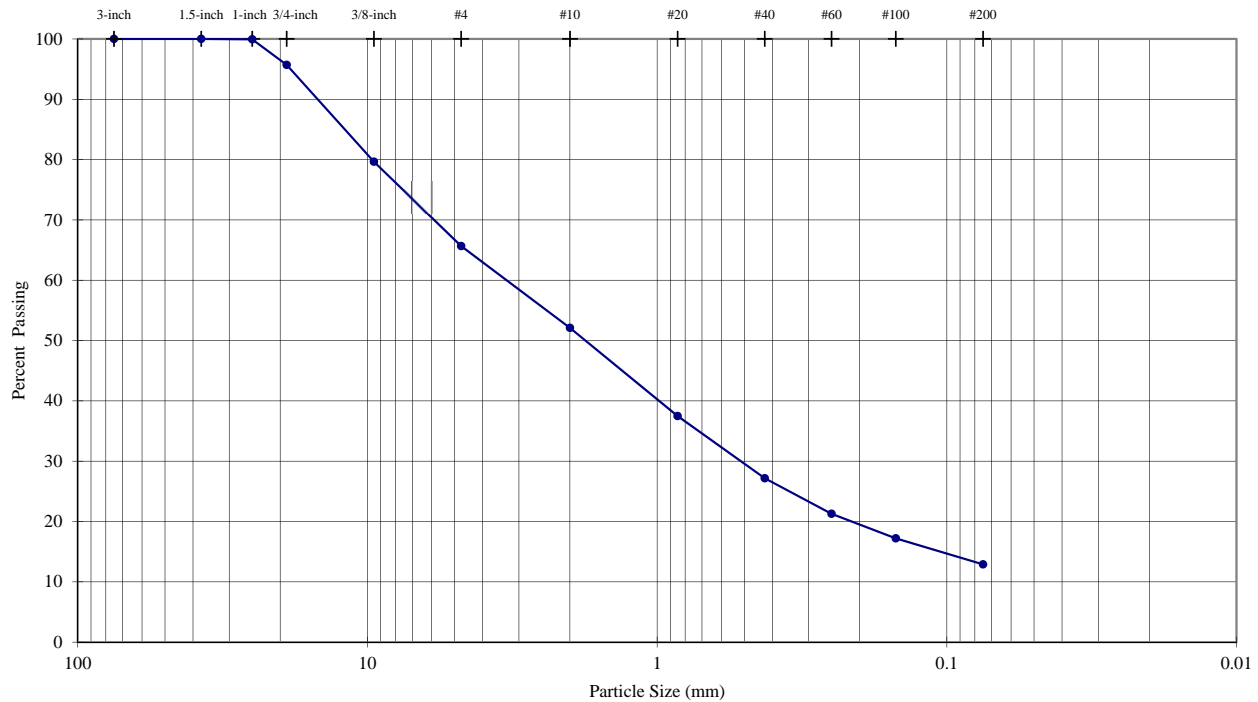
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/26/2013
REVIEW	MB

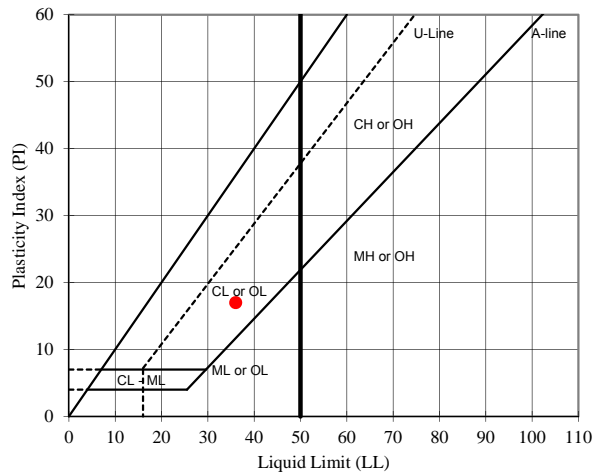
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #1**  
 TYPE: **Pails**

DEPTH (ft): **5-13**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>4.30</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>99.9</b>		
3/4-inch	19.0	<b>95.7</b>	Fine Gravel	<b>30.03</b>
3/8-inch	9.5	<b>79.6</b>		
#4	4.8	<b>65.7</b>	Coarse Sand	<b>13.57</b>
#10	2.00	<b>52.1</b>		
#20	0.85	<b>37.5</b>	Medium Sand	<b>24.93</b>
#40	0.43	<b>27.2</b>		
#60	0.25	<b>21.3</b>	Fine Sand	<b>14.27</b>
#100	0.15	<b>17.2</b>		
#200	0.075	<b>12.9</b>	Silt or Clay Fines	<b>12.90</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
<b>36</b>	<b>19</b>	<b>17</b>	<b>2.75</b>

As-Received Moisture Content (%) USCS Group Symbol

-- **SC**

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH **AMS**  
 DATE **3/4/2013**  
 REVIEW **MB**

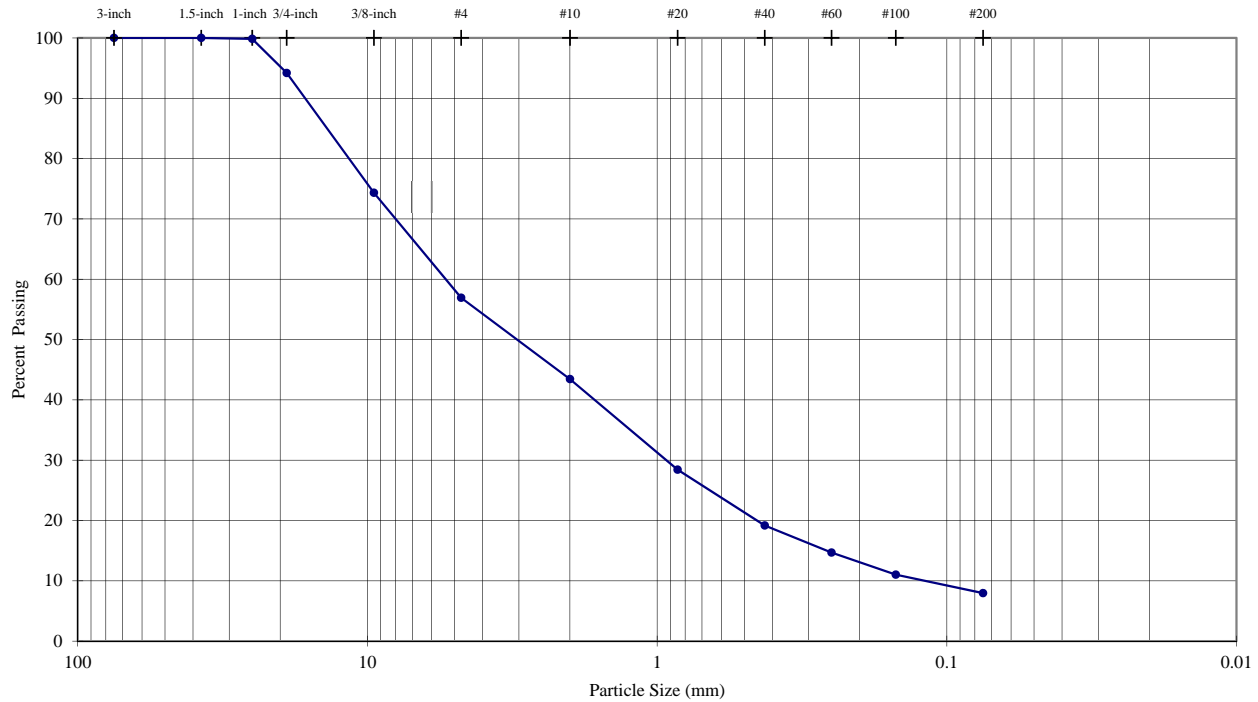


**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

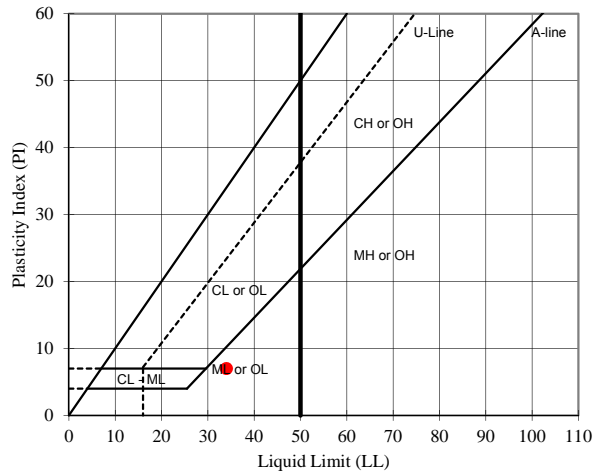
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #2**  
 TYPE: **Pails**

DEPTH (ft): **5-14**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	5.81
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	94.2	Fine Gravel	37.27
3/8-inch	9.5	74.3		
#4	4.8	56.9	Coarse Sand	13.48
#10	2.00	43.4		
#20	0.85	28.4	Medium Sand	24.28
#40	0.43	19.2		
#60	0.25	14.7	Fine Sand	11.20
#100	0.15	11.0		
#200	0.075	8.0		
			Silt or Clay Fines	7.96



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, brown, dry

LL	PL	PI	SpG
34	27	7	2.74

As-Received Moisture Content (%)

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USCS Group Symbol

GW-GM

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

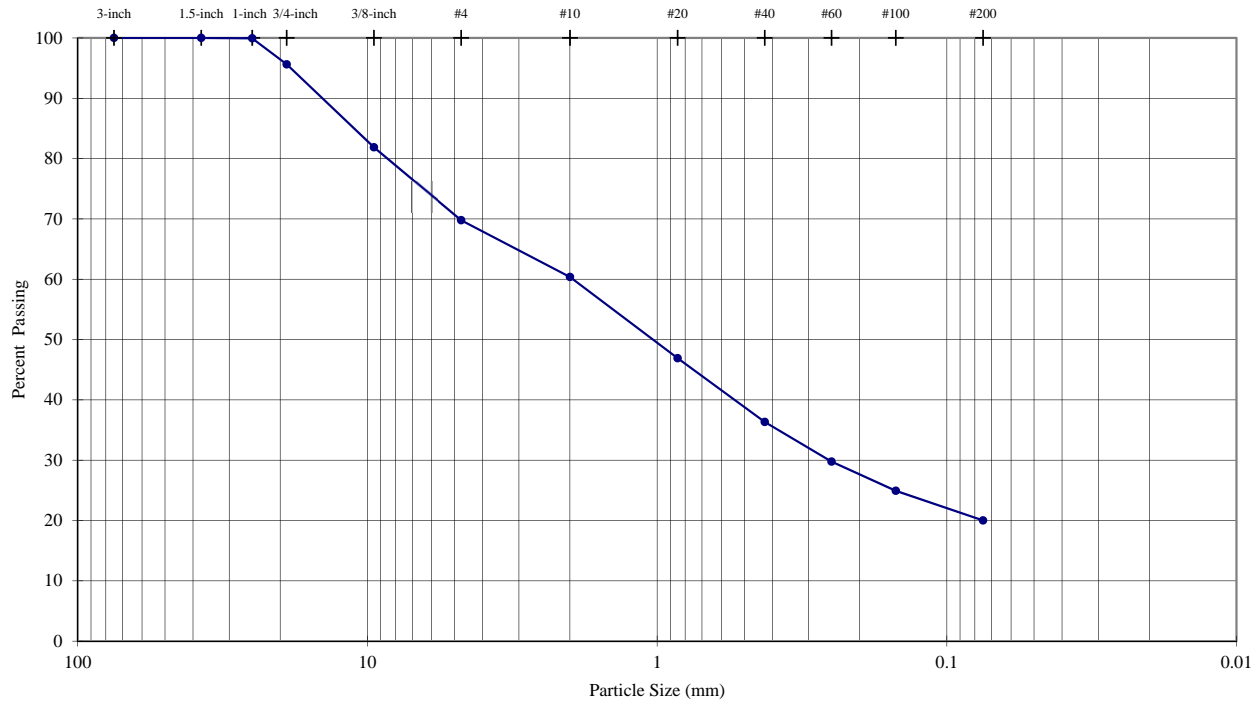
TECH	AMS
DATE	3/4/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

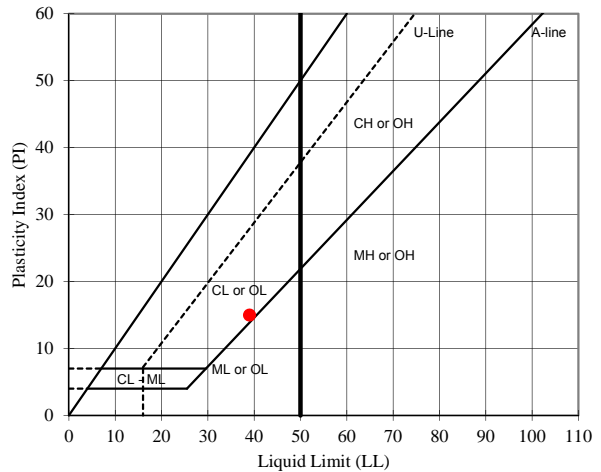
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #3**  
 TYPE: **Pails**

DEPTH (ft): **2-3**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	4.38
1.5-inch	37.5	100.0		
1-inch	25.0	99.9		
3/4-inch	19.0	95.6	Fine Gravel	25.84
3/8-inch	9.5	81.9		
#4	4.8	69.8	Coarse Sand	9.40
#10	2.00	60.4		
#20	0.85	46.9	Medium Sand	24.04
#40	0.43	36.3		
#60	0.25	29.8	Fine Sand	16.32
#100	0.15	24.9		
#200	0.075	20.0		
			Silt or Clay Fines	20.01



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
39	24	15	2.67

As-Received Moisture Content (%)

--

USCS Group Symbol

SC

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

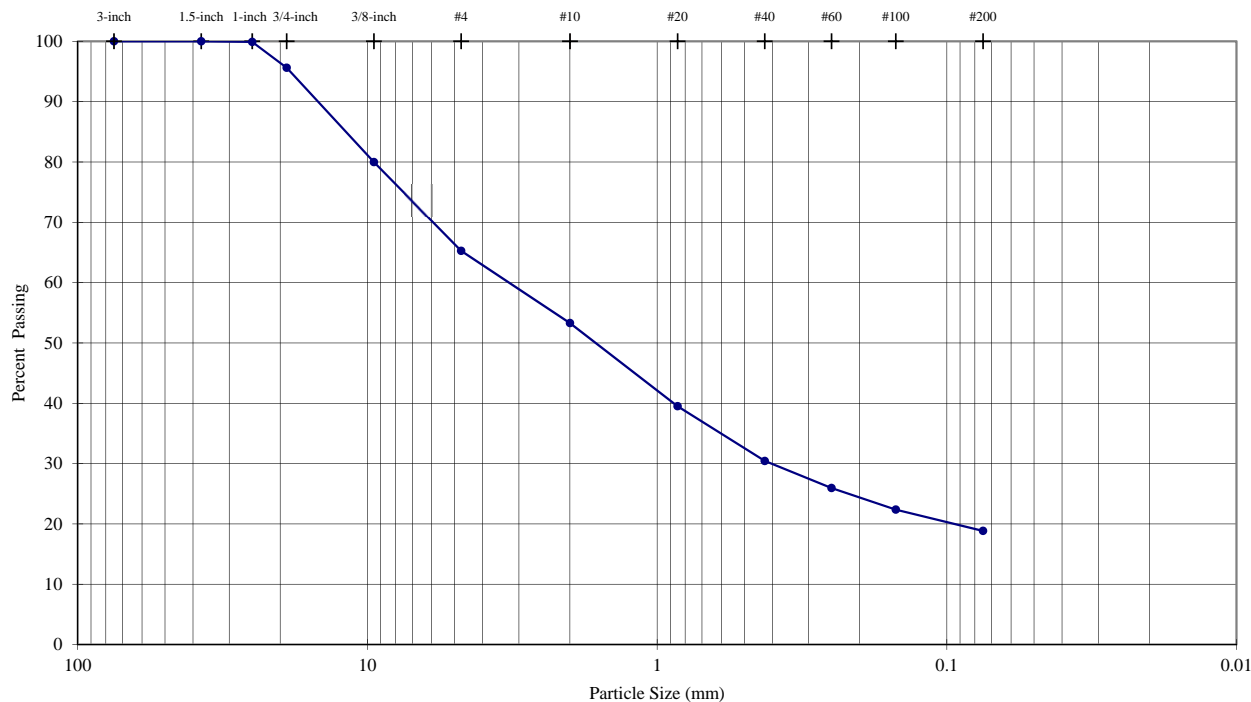
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**

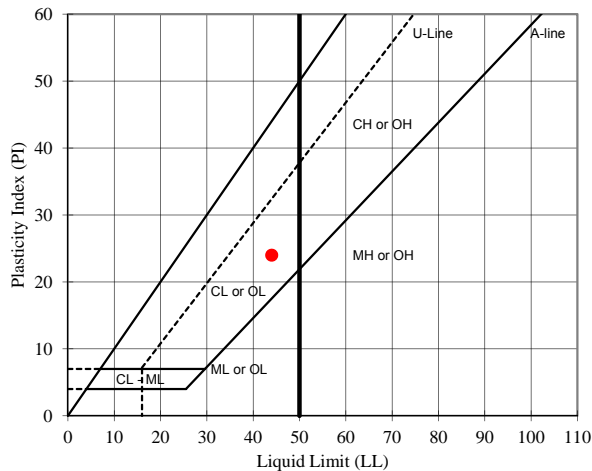
SAMPLE ID: **Composite #4**

DEPTH (ft): **0-10**

TYPE: **Pails**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>4.38</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>99.9</b>		
3/4-inch	19.0	<b>95.6</b>	Fine Gravel	<b>30.35</b>
3/8-inch	9.5	<b>80.0</b>		
#4	4.8	<b>65.3</b>	Coarse Sand	<b>11.97</b>
#10	2.00	<b>53.3</b>		
#20	0.85	<b>39.5</b>	Medium Sand	<b>22.86</b>
#40	0.43	<b>30.4</b>		
#60	0.25	<b>26.0</b>	Fine Sand	<b>11.60</b>
#100	0.15	<b>22.4</b>		
#200	0.075	<b>18.8</b>	Silt or Clay Fines	<b>18.84</b>



USCS Description (ASTM D 2487):  
Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
<b>44</b>	<b>20</b>	<b>24</b>	<b>2.70</b>

As-Received Moisture Content (%)  
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USCS Group Symbol  
**SC**

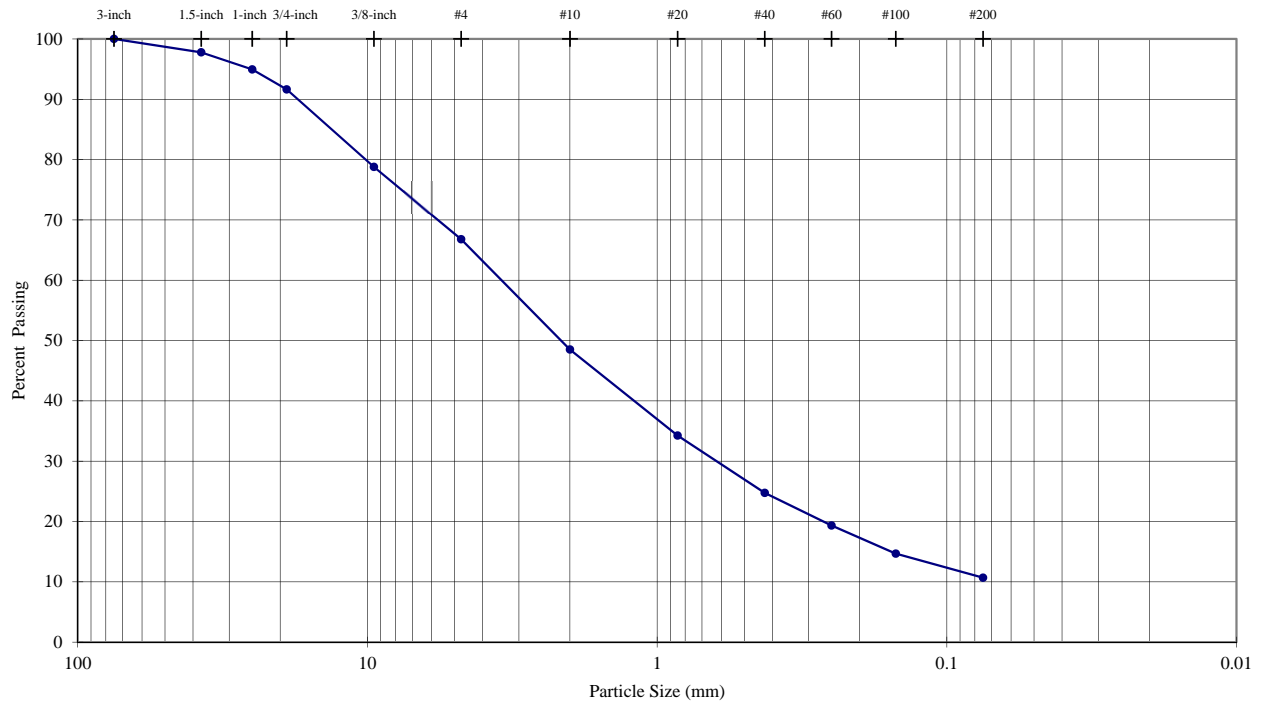
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
Sample prepared for Atterberg Limits testing by the dry method  
Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

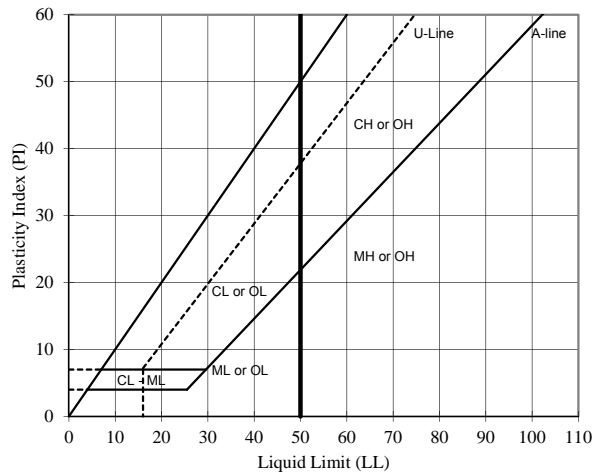
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-5**  
 TYPE: **Pail**

DEPTH (ft): **3-7**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>8.37</b>
1.5-inch	37.5	<b>97.8</b>		
1-inch	25.0	<b>94.9</b>		
3/4-inch	19.0	<b>91.6</b>	Fine Gravel	<b>24.84</b>
3/8-inch	9.5	<b>78.8</b>		
#4	4.8	<b>66.8</b>	Coarse Sand	<b>18.28</b>
#10	2.00	<b>48.5</b>		
#20	0.85	<b>34.2</b>		
#40	0.43	<b>24.7</b>	Medium Sand	<b>23.77</b>
#60	0.25	<b>19.3</b>		
#100	0.15	<b>14.7</b>	Fine Sand	<b>14.07</b>
#200	0.075	<b>10.7</b>		
			Silt or Clay Fines	<b>10.68</b>



Visual Description (Golder Procedure):  
 gravelly SAND, some non-plastic fines, yellowish brown, dry

LL	PL	PI
#VALUE!	#DIV/0!	#VALUE!

As-Received Moisture Content (%)  
 --

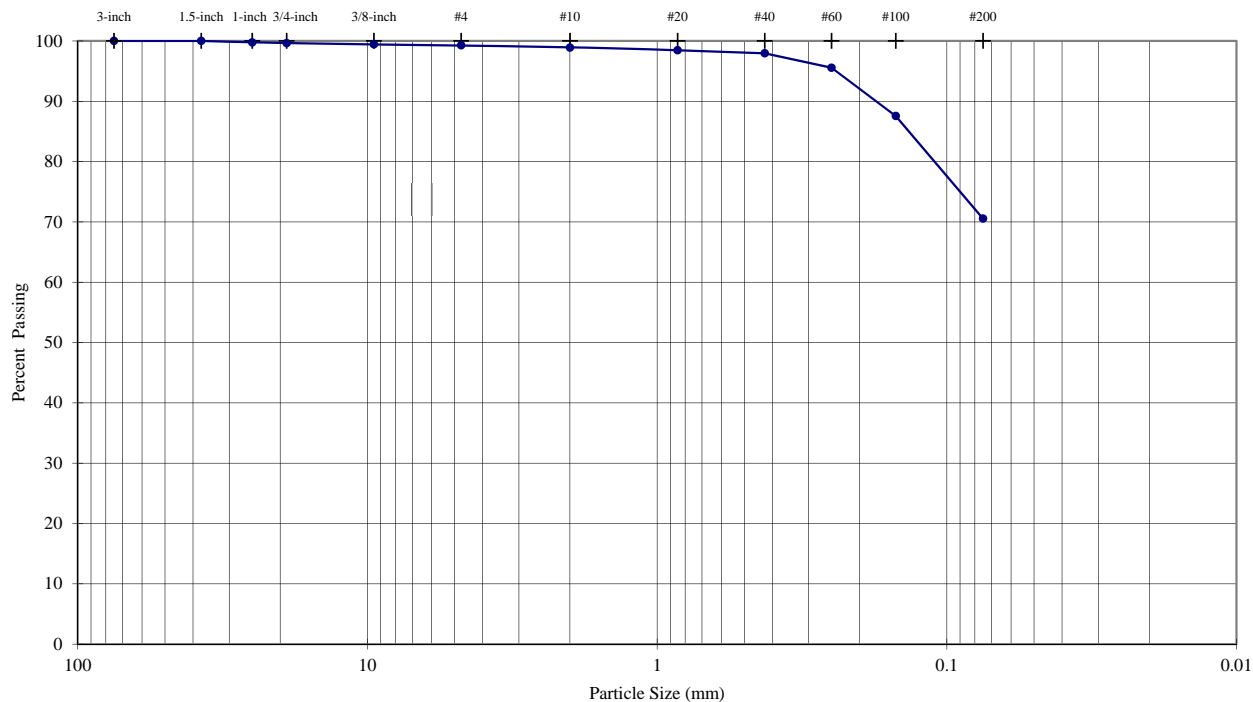
USCS Group Symbol  
 --

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

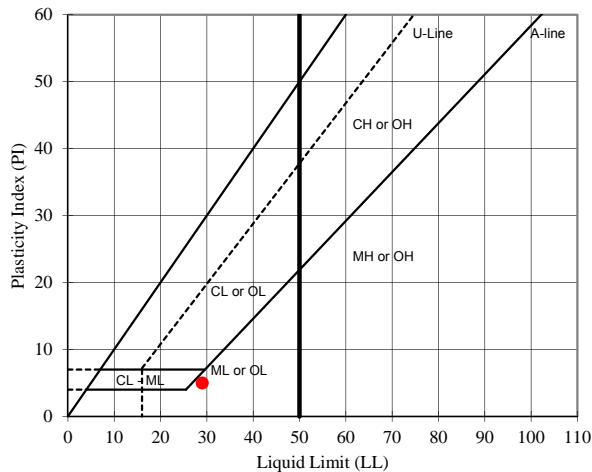
TECH	AMS
DATE	2/27/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-10**  
 TYPE: **Pail**  
 DEPTH (ft): **3-5(12)**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.37
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	99.6	Fine Gravel	0.36
3/8-inch	9.5	99.4		
#4	4.8	99.3	Coarse Sand	0.38
#10	2.00	98.9		
#20	0.85	98.5		
#40	0.43	97.9	Medium Sand	0.95
#60	0.25	95.6		
#100	0.15	87.6	Fine Sand	27.39
#200	0.075	70.6		
Silt or Clay Fines				70.55



USCS Description (ASTM D 2487):  
 Silt with sand, yellowish brown, dry

LL	PL	PI
29	24	5

As-Received Moisture Content (%)  
 --

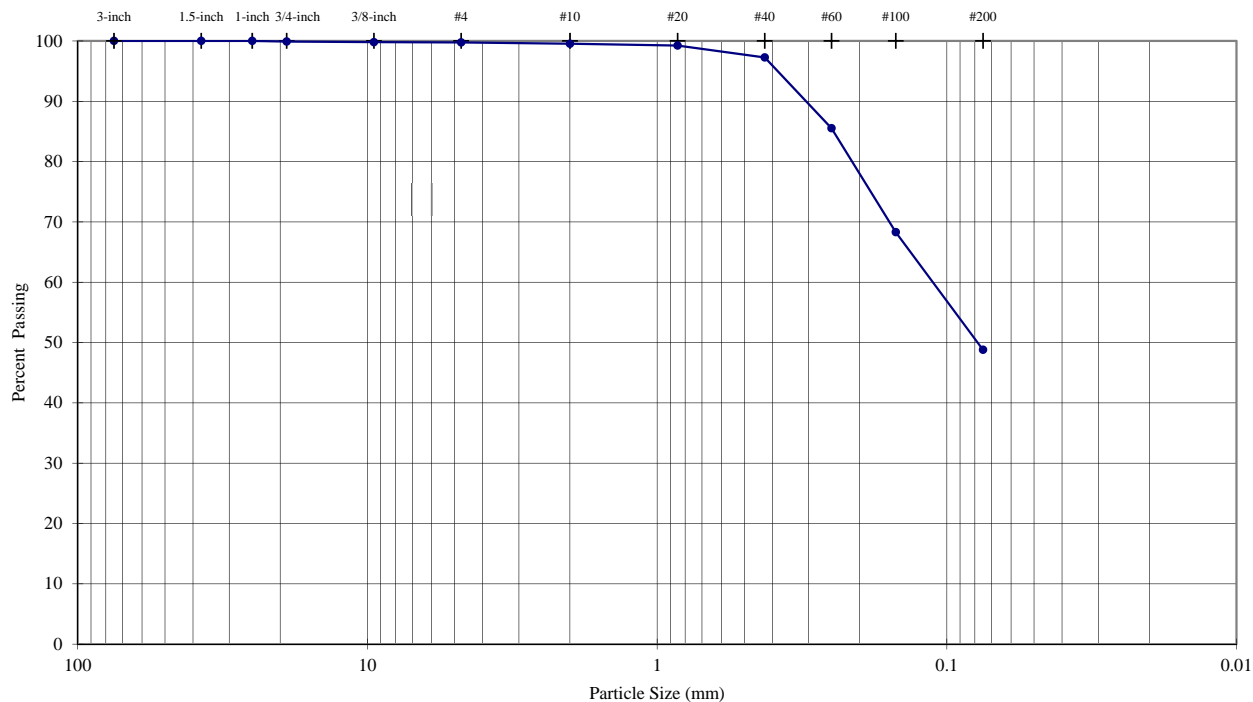
USCS Group Symbol  
 ML

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

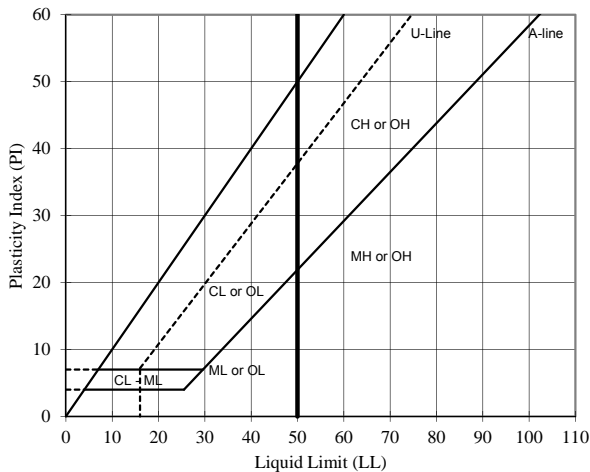
TECH AMS  
 DATE 3/1/2013  
 REVIEW MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-11** DEPTH (ft): **3-11**  
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.11
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	0.14
3/8-inch	9.5	99.8		
#4	4.8	99.8	Coarse Sand	0.22
#10	2.00	99.5		
#20	0.85	99.2		
#40	0.43	97.3	Medium Sand	2.27
#60	0.25	85.5		
#100	0.15	68.3	Fine Sand	48.47
#200	0.075	48.8		
			Silt or Clay Fines	48.80



USCS Description (ASTM D 2487):  
**Silty sand, brownish yellow, moist**

LL	PL	PI	SpG
NP	NP	NP	2.74

As-Received Moisture Content (%) USCS Group Symbol  
-- SM

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

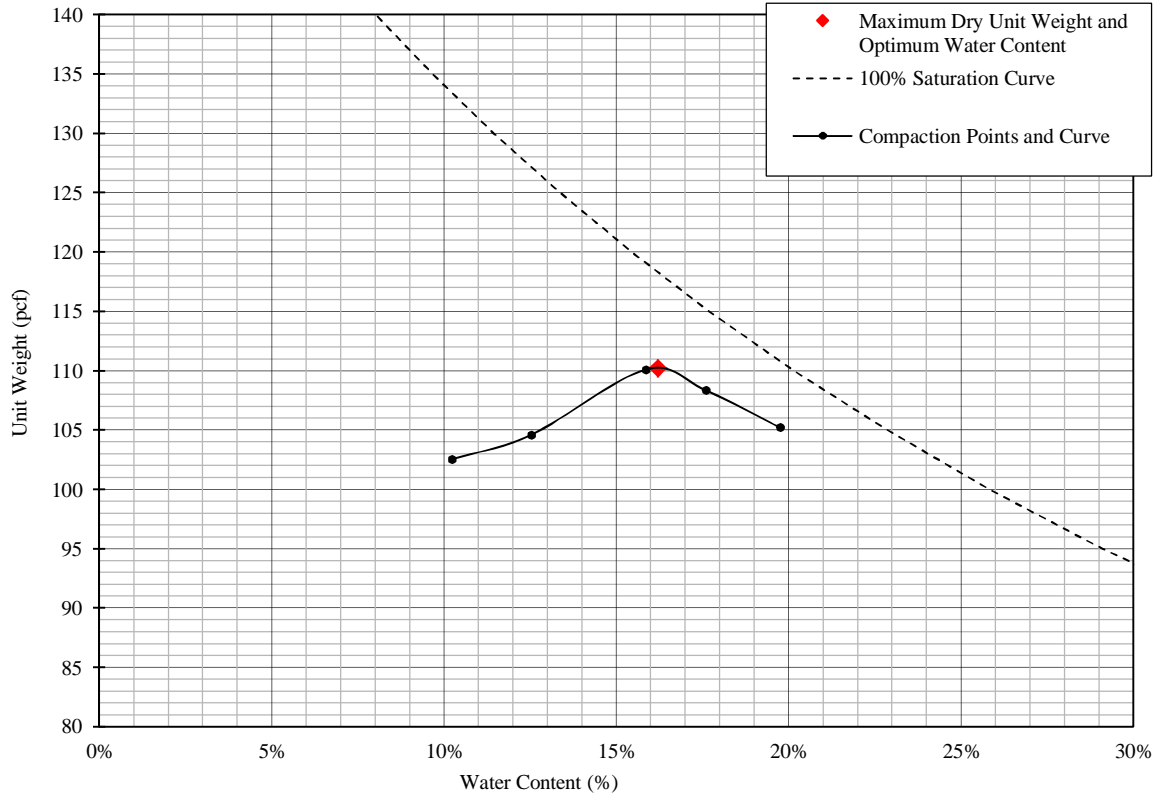
TECH	AM
DATE	2/20/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-11**  
 TYPE: **Pail**

DEPTH (ft): **3-11**



% Test Fraction Passing #4 Sieve	<b>100%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM D854)	<b>2.74</b>

Maximum Dry Unit Weight (pcf)	<b>110.2</b>
Optimum Water Content (%)	<b>16.2</b>

USCS Description (ASTM D 2487): Silty sand, brownish yellow, moist

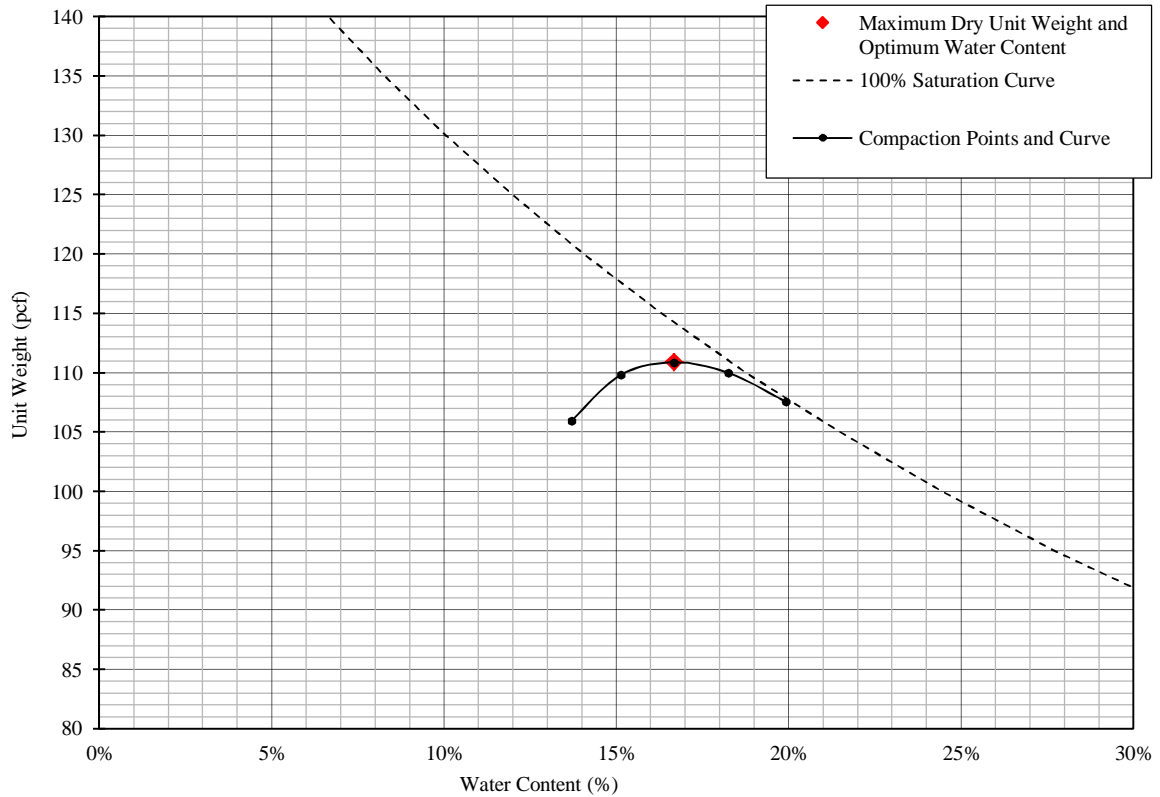
USCS SM

TECH	EH
DATE	2-22-2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20**      DEPTH (ft): **0-4**  
 TYPE: **Pail/Bag**



% Test Fraction Passing #4 Sieve	<b>82%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.64</b>

Maximum Dry Unit Weight (pcf)	<b>110.9</b>
Optimum Water Content (%)	<b>16.7</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>116.9</b>
Corrected Optimum Water Content (%)	<b>13.9</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, strong brown, wet

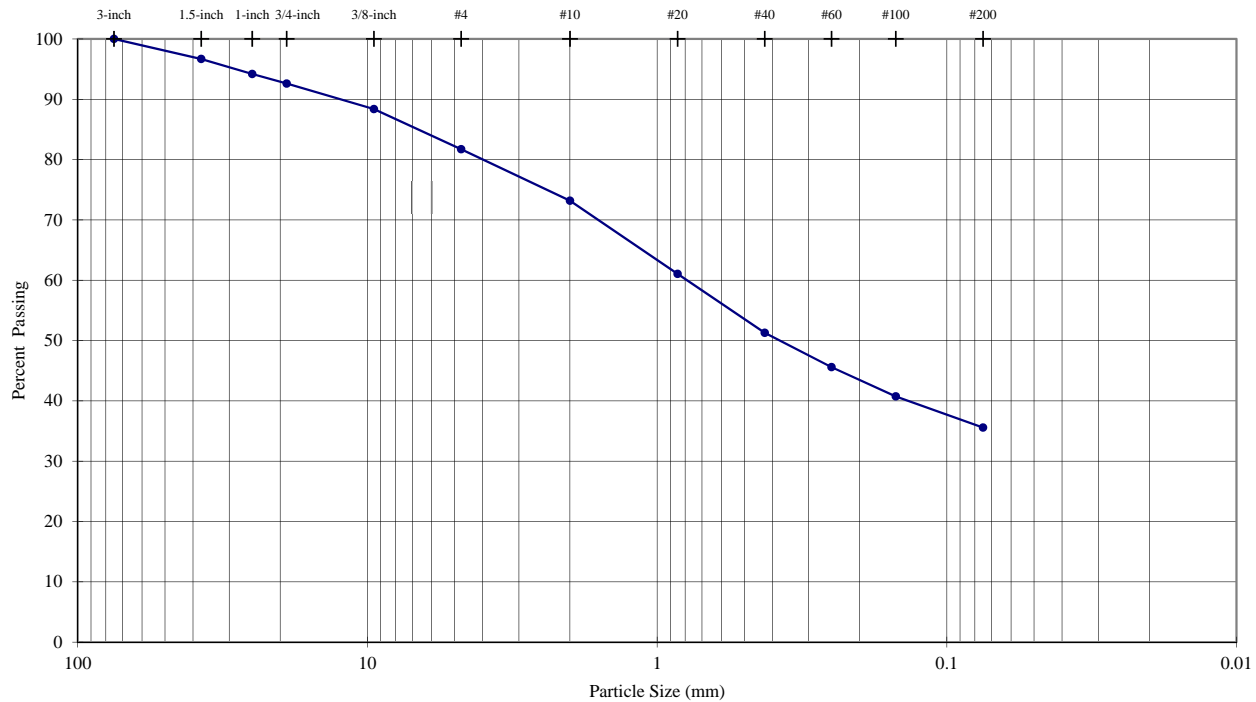
USCS      SC

TECH	AM
DATE	2-27-13
REVIEW	MB

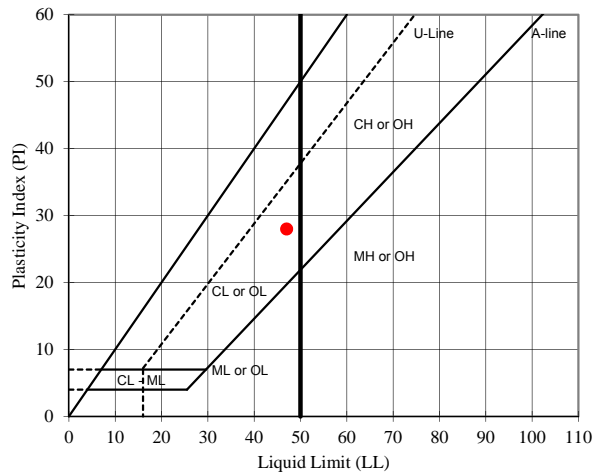


**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20** DEPTH (ft): **0-4**  
 TYPE: **Pail/Bag**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	7.40
1.5-inch	37.5	96.7		
1-inch	25.0	94.2		
3/4-inch	19.0	92.6	Fine Gravel	10.89
3/8-inch	9.5	88.4		
#4	4.8	81.7	Coarse Sand	8.53
#10	2.00	73.2		
#20	0.85	61.0	Medium Sand	21.90
#40	0.43	51.3		
#60	0.25	45.6	Fine Sand	15.70
#100	0.15	40.7		
#200	0.075	35.6		
Silt or Clay Fines				35.57



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, strong brown, wet

LL	PL	PI
47	19	28

As-Received Moisture Content (%)  
 --

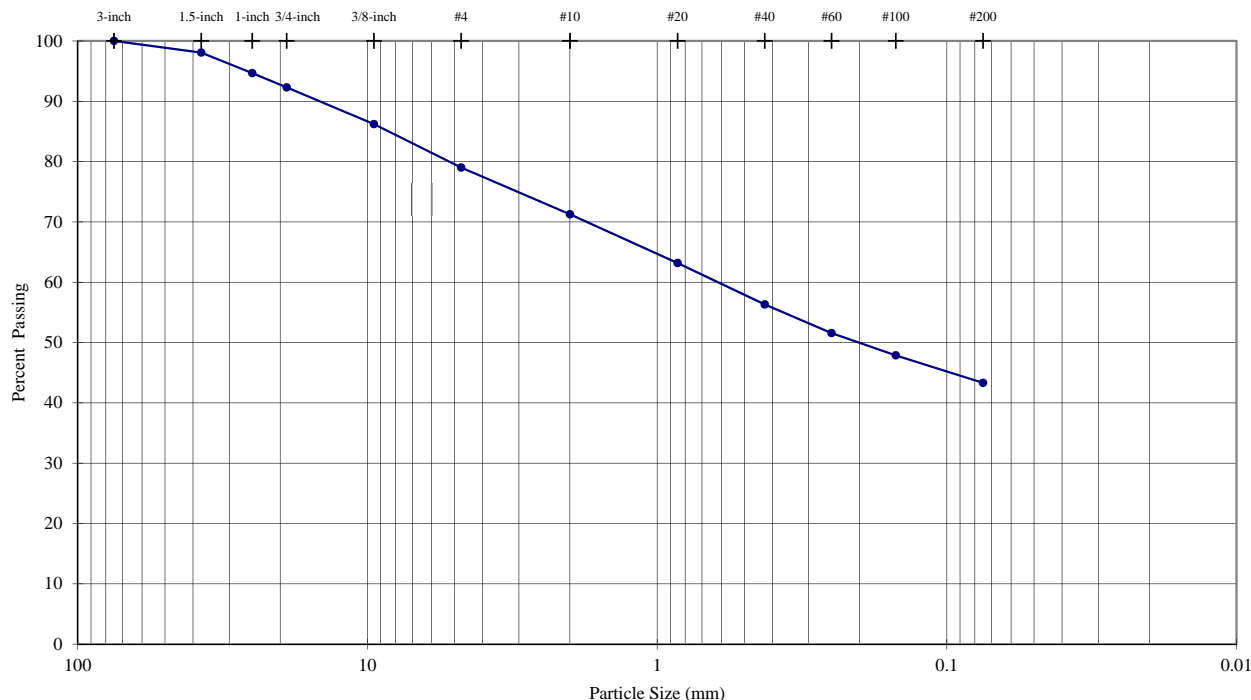
USCS Group Symbol  
 SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

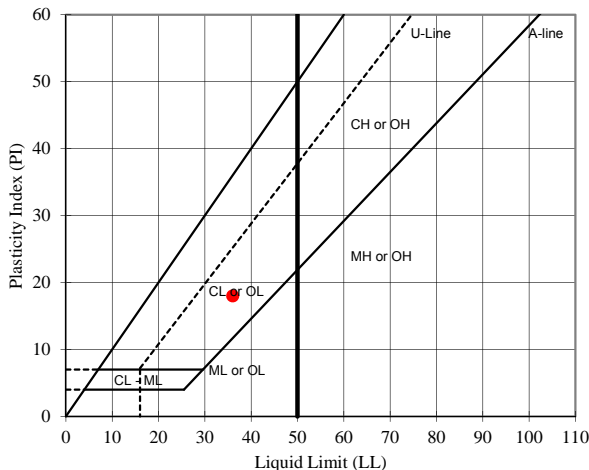
TECH	AMS
DATE	2/25/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20** DEPTH (ft): **4-7**  
 TYPE: **Pail/Bag**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>7.71</b>
1.5-inch	37.5	<b>98.1</b>		
1-inch	25.0	<b>94.7</b>		
3/4-inch	19.0	<b>92.3</b>	Fine Gravel	<b>13.28</b>
3/8-inch	9.5	<b>86.2</b>		
#4	4.8	<b>79.0</b>	Coarse Sand	<b>7.75</b>
#10	2.00	<b>71.3</b>		
#20	0.85	<b>63.2</b>	Medium Sand	<b>14.95</b>
#40	0.43	<b>56.3</b>		
#60	0.25	<b>51.6</b>	Fine Sand	<b>12.99</b>
#100	0.15	<b>47.9</b>		
#200	0.075	<b>43.3</b>	Silt or Clay Fines	<b>43.32</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI
<b>36</b>	<b>18</b>	<b>18</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SC**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

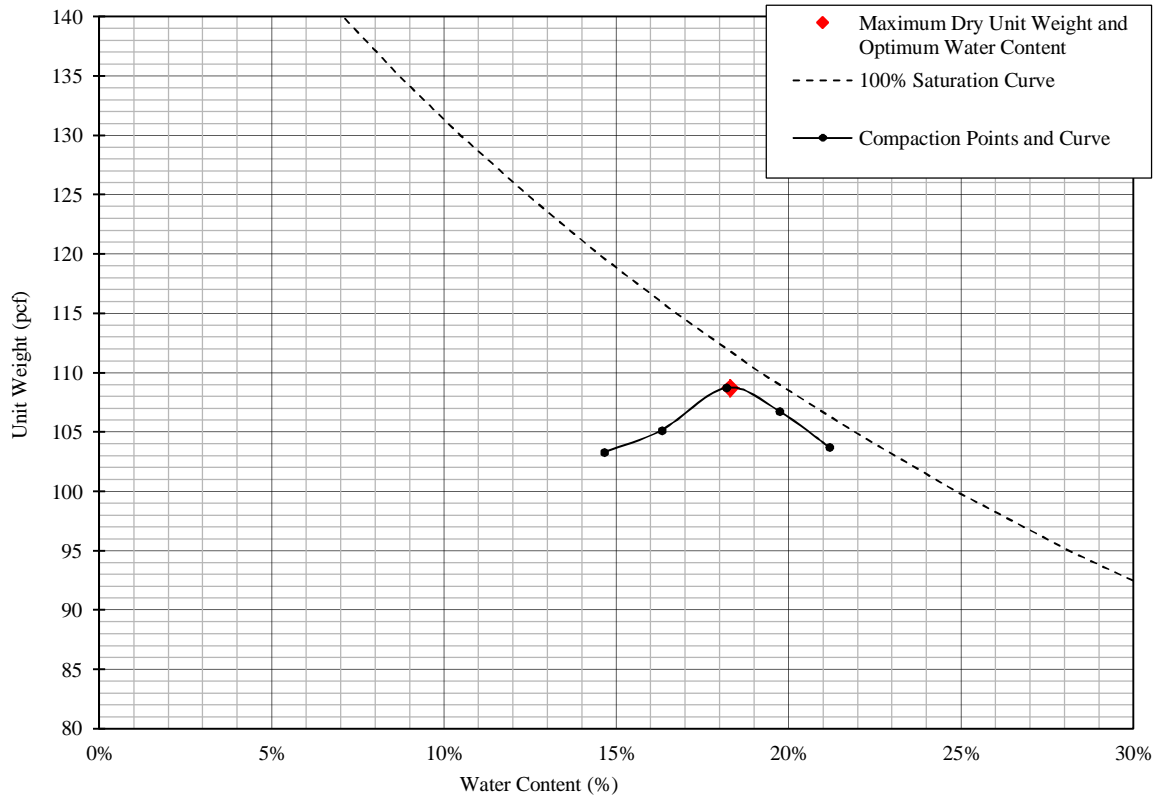
TECH	AMS
DATE	2/25/2013
REVIEW	MB

# LABORATORY COMPACTION CHARACTERISTICS OF SOIL

## ASTM D698 - Method A

**Manual Rammer      Moist Preparation**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20** DEPTH (ft): **4-7**  
 TYPE: **Pail/Bag**



% Test Fraction Passing #4 Sieve	<b>79%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.67</b>

Maximum Dry Unit Weight (pcf)	<b>108.7</b>
Optimum Water Content (%)	<b>18.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>116.1</b>
Corrected Optimum Water Content (%)	<b>14.8</b>

**USCS Description (ASTM D 2487):** Clayey sand with gravel, brown, dry

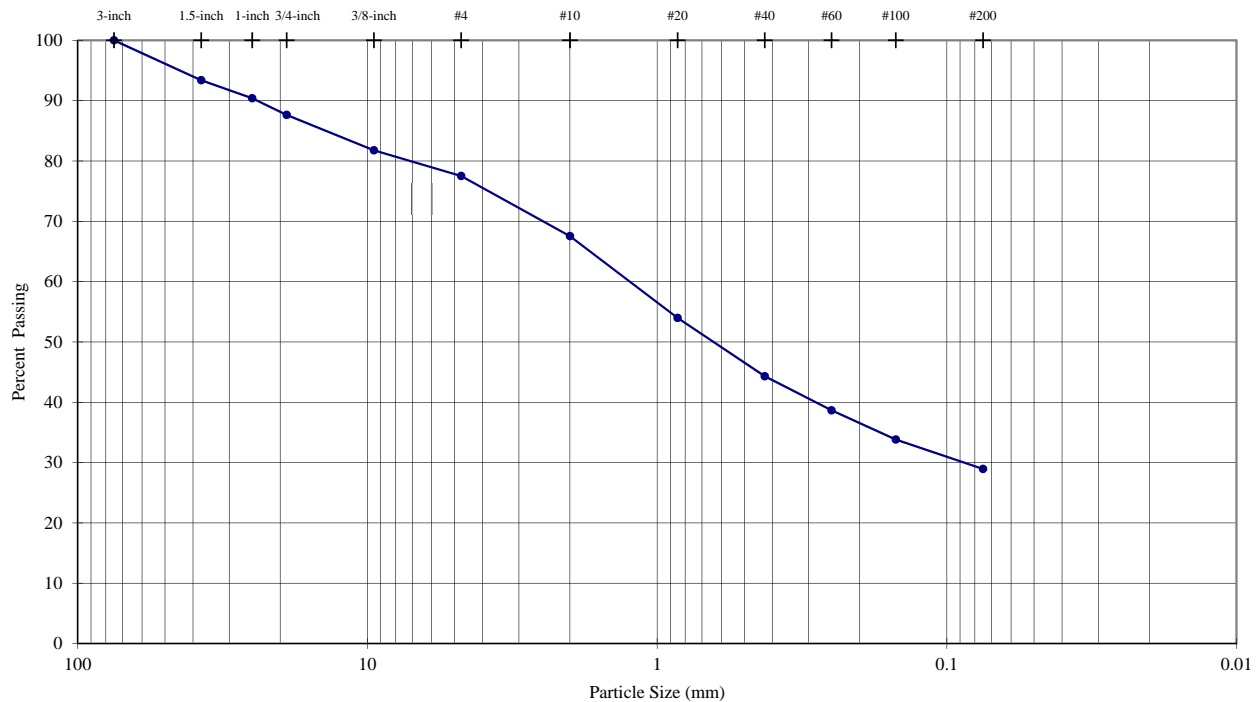
USCS	SC
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TECH	AMS
DATE	2-28-13
REVIEW	MB

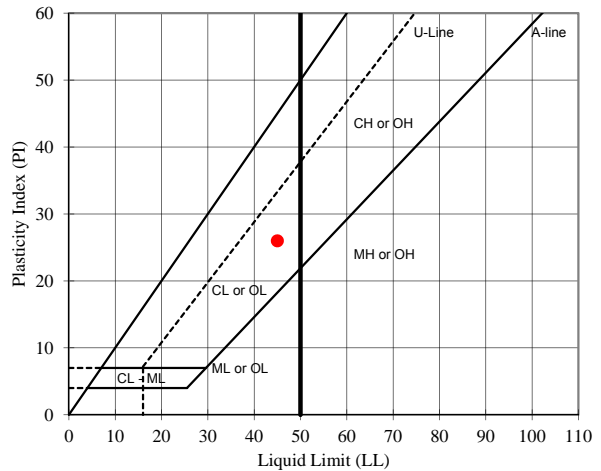
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-24**  
 TYPE: **Pail**

DEPTH (ft): **3-5**



Sieve	Particle Size	% Passing	Description	Percentage
	(mm)			
3-inch	75.0	100.0	Coarse Gravel	12.37
1.5-inch	37.5	93.4		
1-inch	25.0	90.4		
3/4-inch	19.0	87.6	Fine Gravel	10.12
3/8-inch	9.5	81.8		
#4	4.8	77.5	Coarse Sand	9.97
#10	2.00	67.5		
#20	0.85	54.0		
#40	0.43	44.3	Medium Sand	23.23
#60	0.25	38.7		
#100	0.15	33.8	Fine Sand	15.37
#200	0.075	28.9		
			Silt or Clay Fines	28.94



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, yellowish brown, dry

LL	PL	PI
45	19	26

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SC

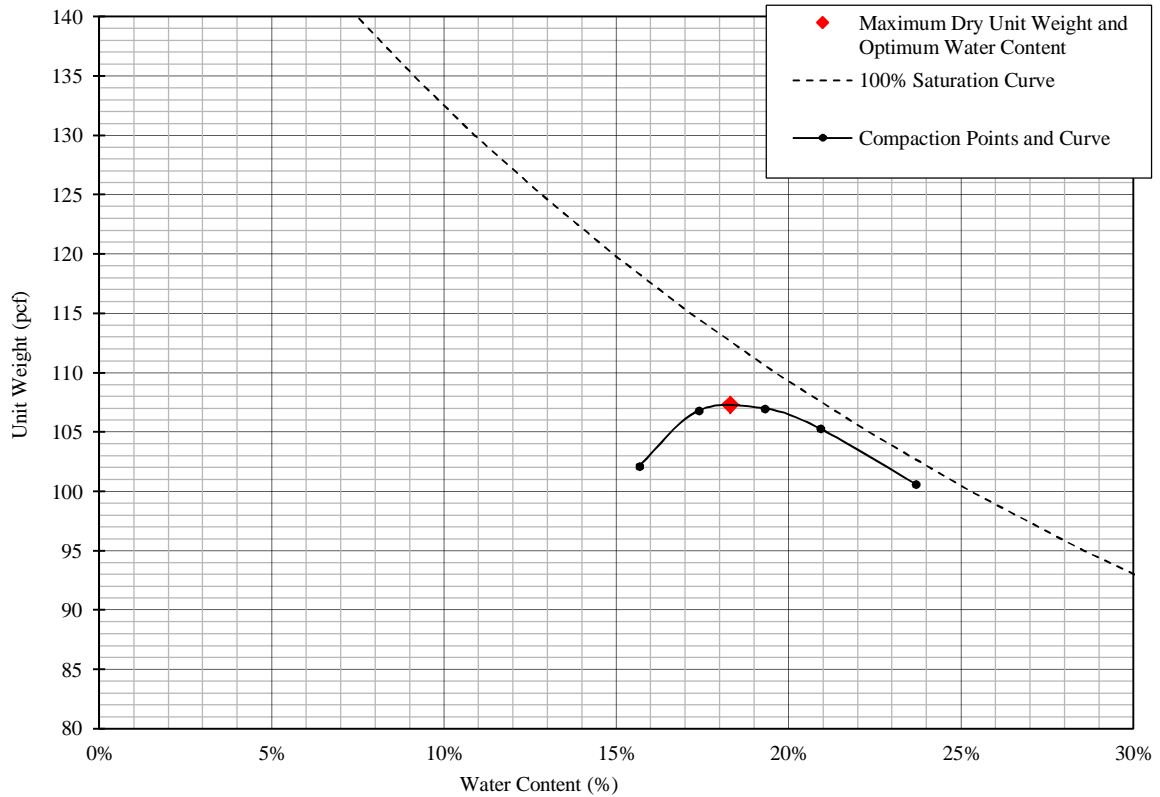
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM
DATE	2/20/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-24**      DEPTH (ft): **3-5**  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>79%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>107.3</b>
Optimum Water Content (%)	<b>18.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>114.7</b>
Corrected Optimum Water Content (%)	<b>14.4</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, yellowish brown, dry

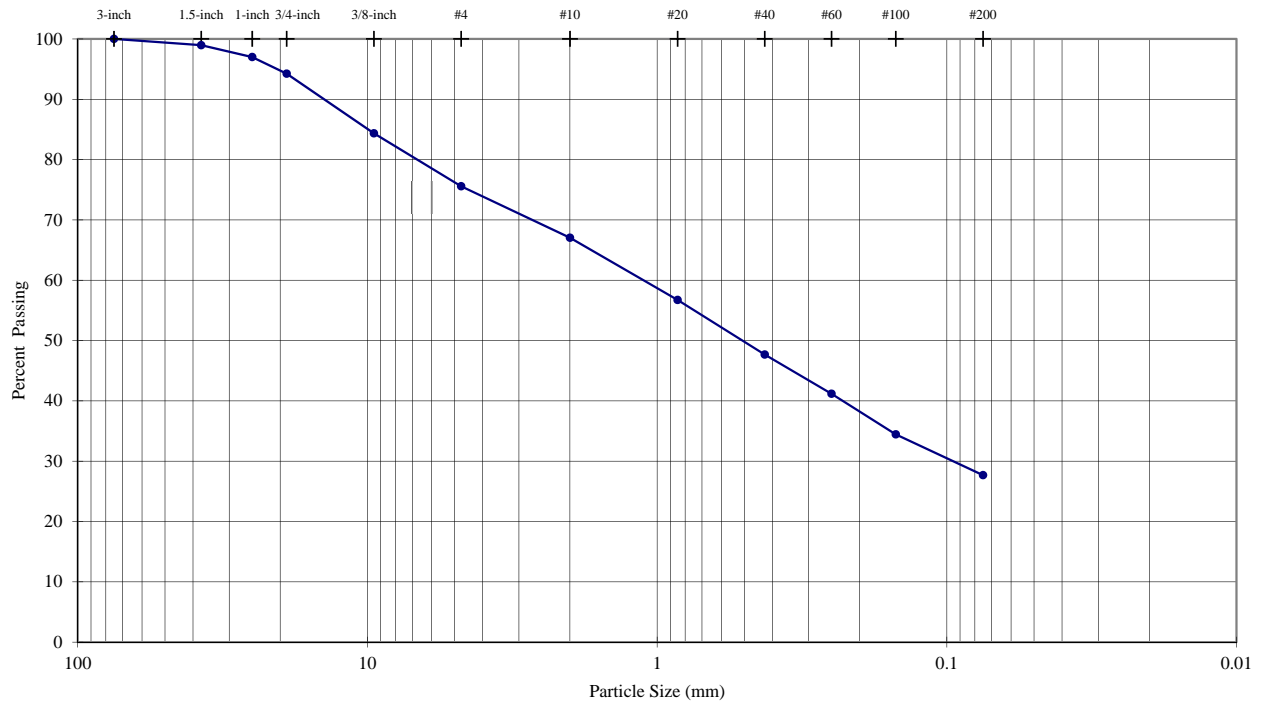
USCS      SC

TECH	EH
DATE	2-22-2013
REVIEW	MB

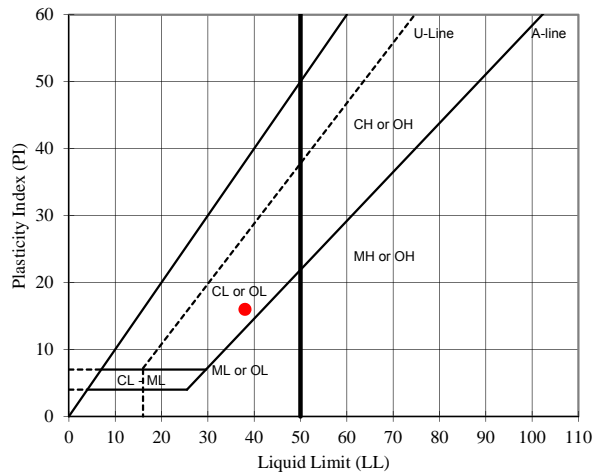
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Pail**

DEPTH (ft): **0-2**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>5.76</b>
1.5-inch	37.5	<b>99.0</b>		
1-inch	25.0	<b>97.0</b>		
3/4-inch	19.0	<b>94.2</b>	Fine Gravel	<b>18.67</b>
3/8-inch	9.5	<b>84.4</b>		
#4	4.8	<b>75.6</b>	Coarse Sand	<b>8.53</b>
#10	2.00	<b>67.0</b>		
#20	0.85	<b>56.7</b>	Medium Sand	<b>19.37</b>
#40	0.43	<b>47.7</b>		
#60	0.25	<b>41.2</b>	Fine Sand	<b>19.99</b>
#100	0.15	<b>34.4</b>		
#200	0.075	<b>27.7</b>	Silt or Clay Fines	<b>27.69</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, dark yellowish brown, dry

LL	PL	PI
<b>38</b>	<b>22</b>	<b>16</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SC**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

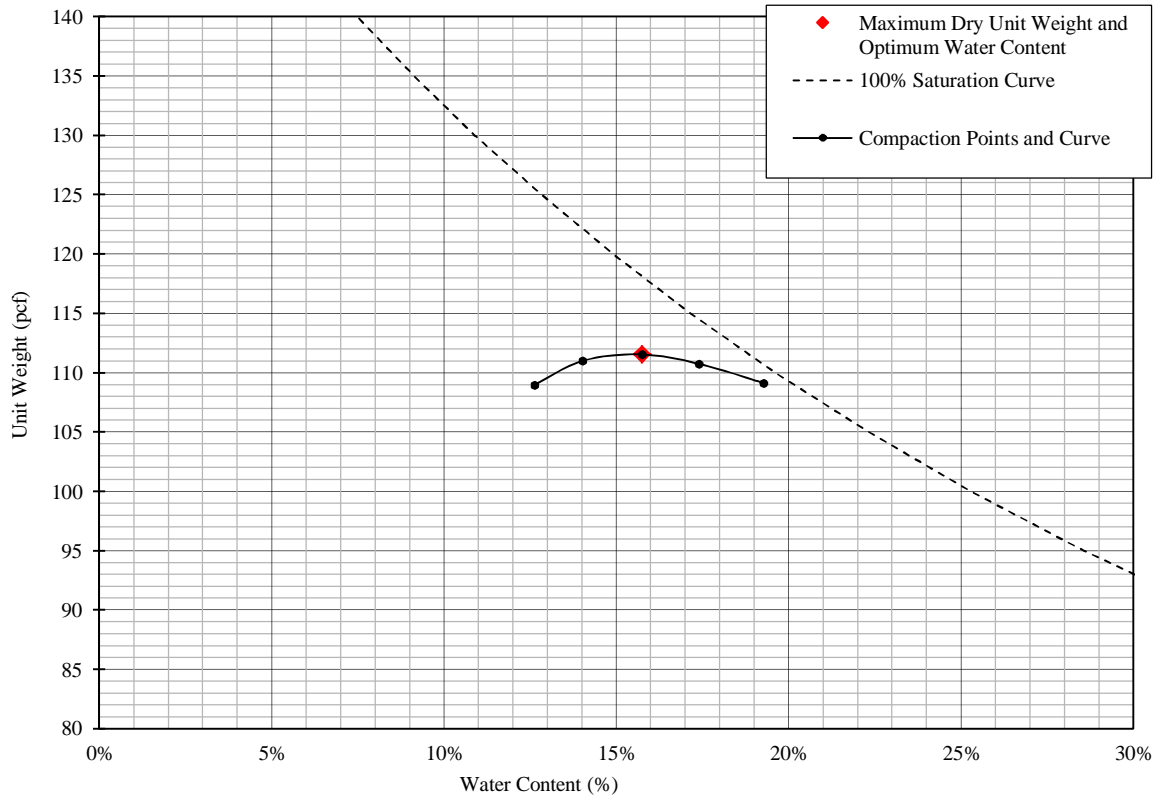
TECH	MGC
DATE	2/22/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Pail**

DEPTH (ft): **0-2**



% Test Fraction Passing #4 Sieve	<b>76%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>111.5</b>
Optimum Water Content (%)	<b>15.7</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>119.9</b>
Corrected Optimum Water Content (%)	<b>12.2</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, dark yellowish brown, dry

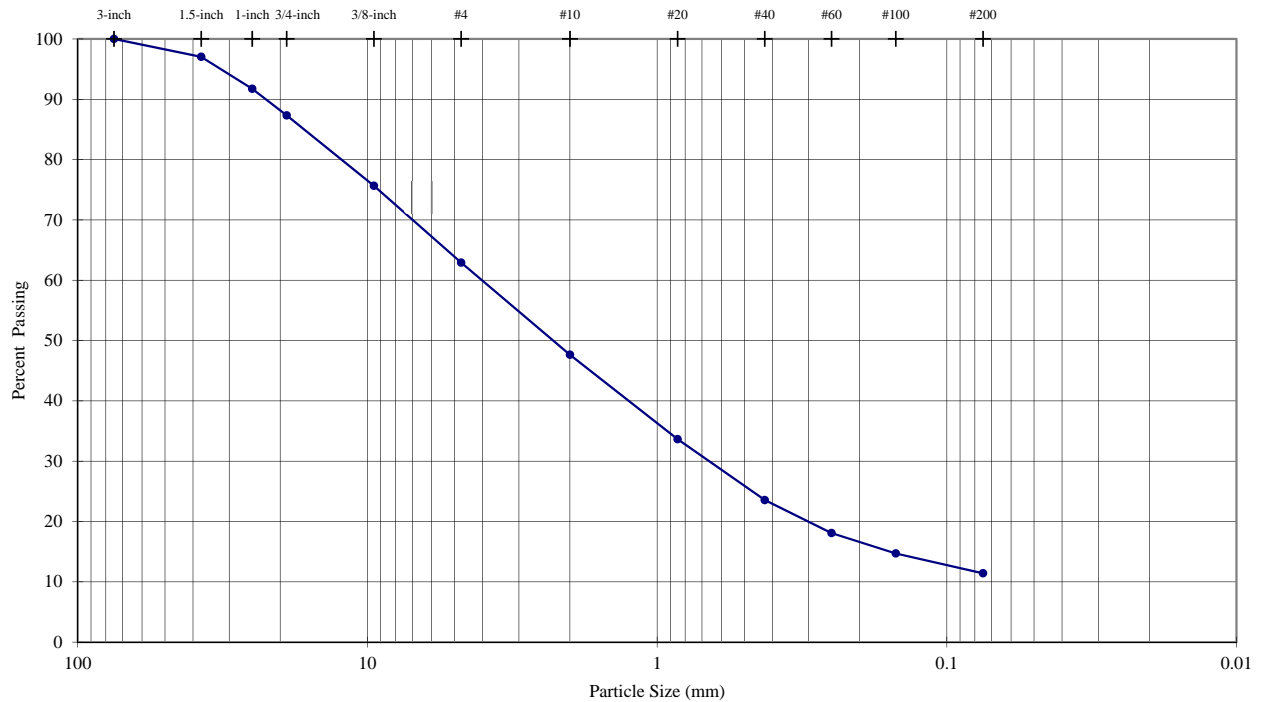
USCS SC

TECH	MGC/AMS
DATE	2-26-2013
REVIEW	MB

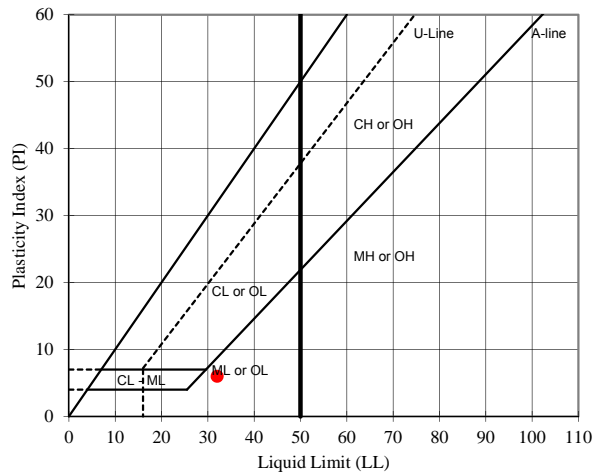
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Bag/Pail**

DEPTH (ft): **3-7**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>12.68</b>
1.5-inch	37.5	<b>97.0</b>		
1-inch	25.0	<b>91.7</b>		
3/4-inch	19.0	<b>87.3</b>	Fine Gravel	<b>24.40</b>
3/8-inch	9.5	<b>75.6</b>		
#4	4.8	<b>62.9</b>	Coarse Sand	<b>15.27</b>
#10	2.00	<b>47.6</b>		
#20	0.85	<b>33.7</b>	Medium Sand	<b>24.09</b>
#40	0.43	<b>23.6</b>		
#60	0.25	<b>18.1</b>	Fine Sand	<b>12.14</b>
#100	0.15	<b>14.7</b>		
#200	0.075	<b>11.4</b>	Silt or Clay Fines	<b>11.42</b>



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, dark yellowish brown, dry

LL	PL	PI
<b>32</b>	<b>26</b>	<b>6</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SW-SM**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	EH
DATE	2/20/2013
REVIEW	MB

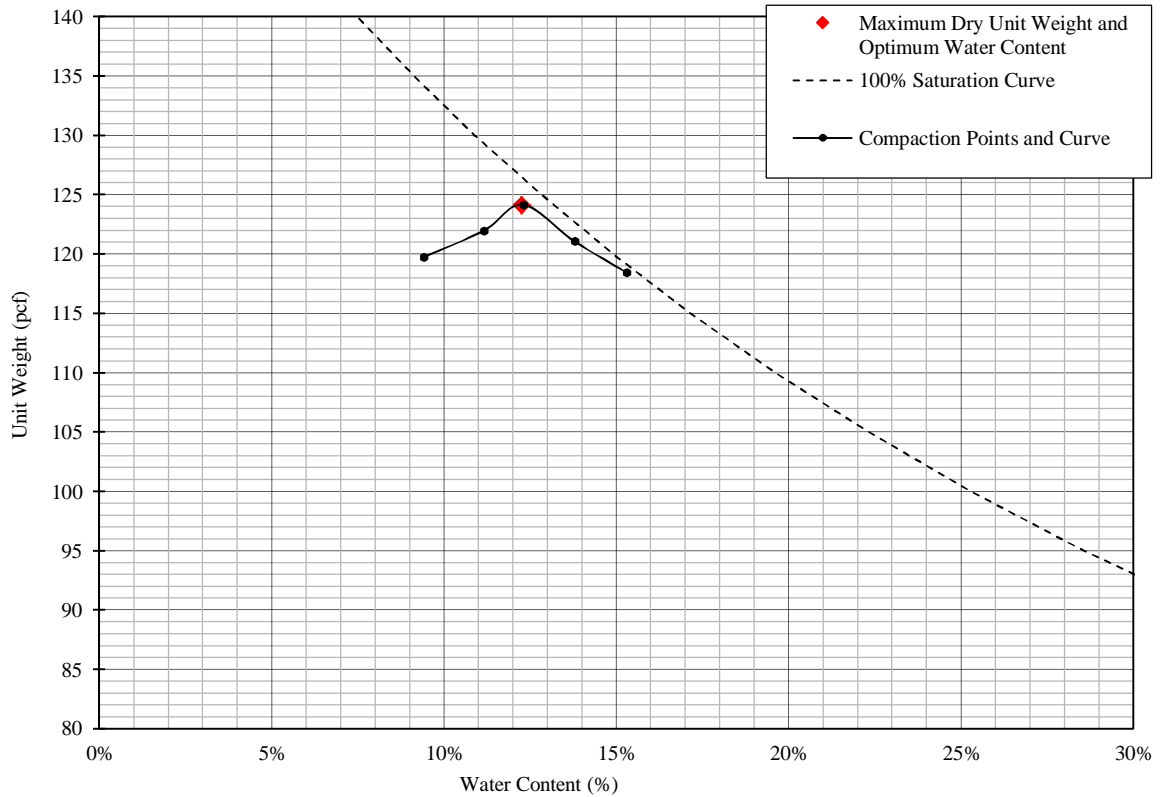


## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Bag/Pail**

DEPTH (ft): **3-7**



% Test Fraction Passing 3/8-inch Sieve	<b>76%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>124.1</b>
Optimum Water Content (%)	<b>12.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>130.5</b>
Corrected Optimum Water Content (%)	<b>9.6</b>

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, dark yellowish brown, dry

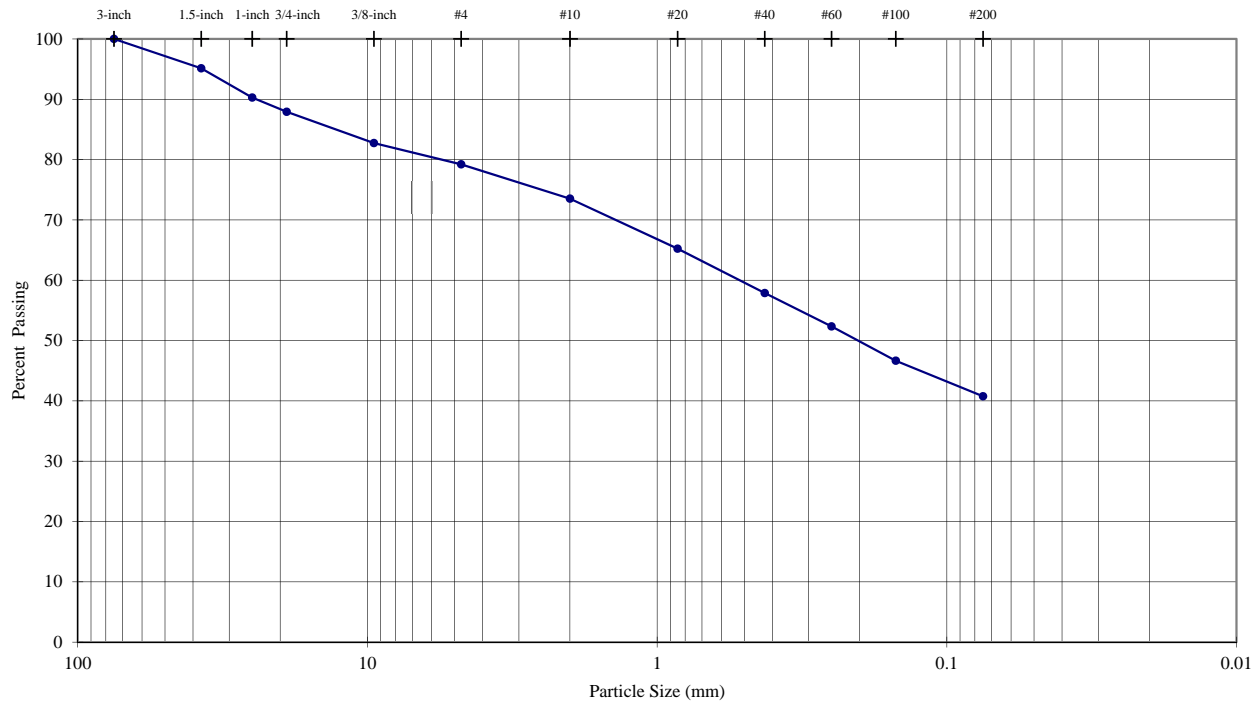
USCS SW-SM

TECH	EH
DATE	3-8-2013
REVIEW	MB

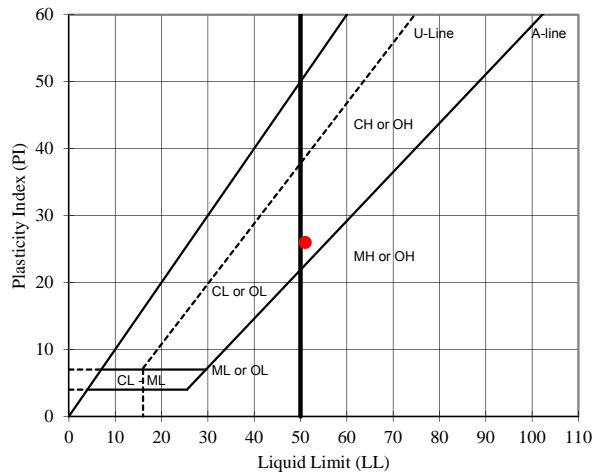
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-29**  
 TYPE: **Pail**

DEPTH (ft): **0-2**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>12.08</b>
1.5-inch	37.5	<b>95.1</b>		
1-inch	25.0	<b>90.3</b>		
3/4-inch	19.0	<b>87.9</b>	Fine Gravel	<b>8.71</b>
3/8-inch	9.5	<b>82.7</b>		
#4	4.8	<b>79.2</b>	Coarse Sand	<b>5.68</b>
#10	2.00	<b>73.5</b>		
#20	0.85	<b>65.2</b>		
#40	0.43	<b>57.9</b>	Medium Sand	<b>15.66</b>
#60	0.25	<b>52.3</b>		
#100	0.15	<b>46.7</b>	Fine Sand	<b>17.10</b>
#200	0.075	<b>40.8</b>		
Silt or Clay Fines				<b>40.76</b>



USCS Description (ASTM D 2487):  
**Clayey sand with gravel, strong brown, dry**

LL	PL	PI
<b>51</b>	<b>25</b>	<b>26</b>

As-Received Moisture Content (%)  
**--**

USCS Group Symbol  
**SC**

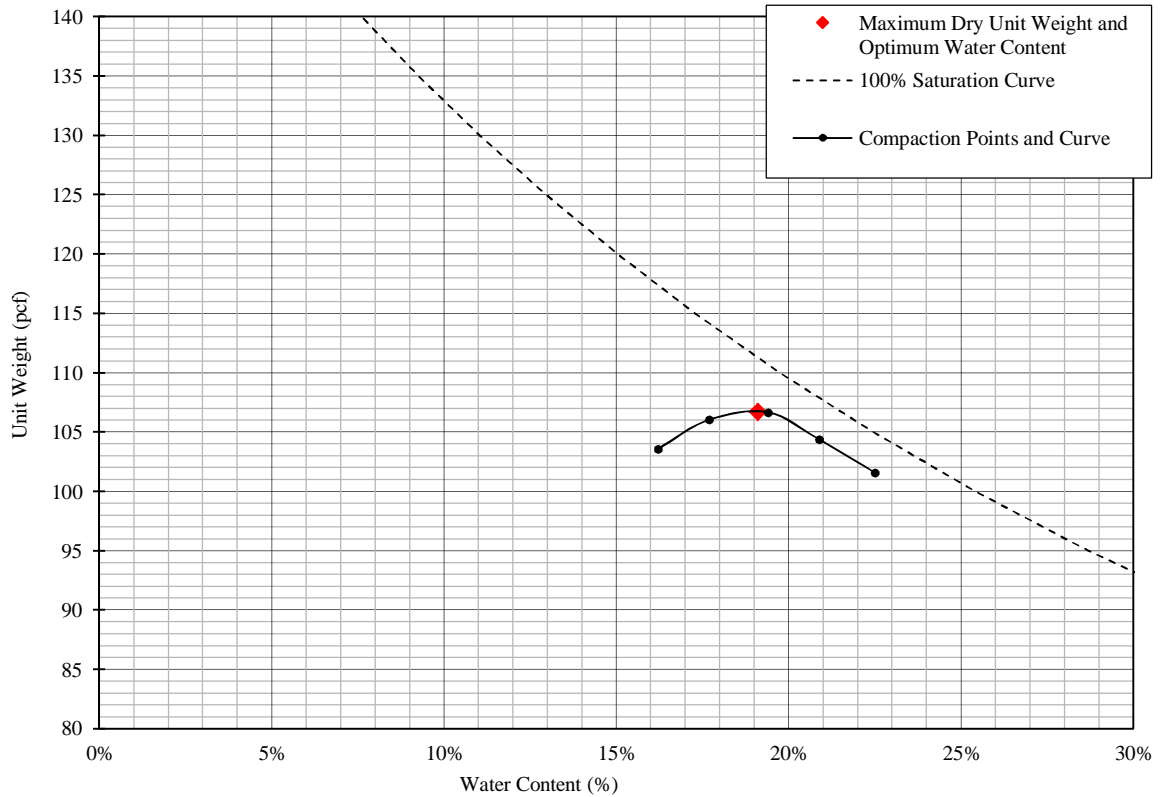
Notes: 0g of particles up to plus 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM
DATE	2/21/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-29**      DEPTH (ft): **0-2**  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>80%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.71</b>

Maximum Dry Unit Weight (pcf)	<b>106.7</b>
Optimum Water Content (%)	<b>19.1</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>114.4</b>
Corrected Optimum Water Content (%)	<b>15.5</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, strong brown, dry

USCS      SC

TECH	AM
DATE	2-25-2013
REVIEW	MB

**APPENDIX A.3.2  
TRIAxIAL TEST REPORTS**

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.517 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.355 lb  
 Moisture Content = 15.8%  
 Wet Unit Weight = 126.6 pcf  
 Dry Unit Weight = 109.3 pcf  
 Void Ratio = 0.51  
 Percent Saturation = 83%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.506 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.332 lb  
 Moisture Content = 16.0%  
 Wet Unit Weight = 126.4 pcf  
 Dry Unit Weight = 108.9 pcf  
 Void Ratio = 0.51  
 Percent Saturation = 83%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.506 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.377 lb  
 Moisture Content = 15.3%  
 Wet Unit Weight = 126.4 pcf  
 Dry Unit Weight = 109.6 pcf  
 Void Ratio = 0.50  
 Percent Saturation = 81%

**After Consolidation**  
 Length = 9.173 in  
 Diameter = 3.814 in  
 Area = 11.424 in<sup>2</sup> (Method B)  
 Volume = 104.789 in<sup>3</sup>  
 Moisture Content = 13.5%  
 Wet Unit Weight = 137.7 pcf  
 Dry Unit Weight = 121.3 pcf  
 Void Ratio = 0.36  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.094 in  
 Diameter = 3.821 in  
 Area = 11.465 in<sup>2</sup> (Method B)  
 Volume = 104.264 in<sup>3</sup>  
 Moisture Content = 13.4%  
 Wet Unit Weight = 137.8 pcf  
 Dry Unit Weight = 121.5 pcf  
 Void Ratio = 0.35  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.057 in  
 Diameter = 3.799 in  
 Area = 11.333 in<sup>2</sup> (Method B)  
 Volume = 102.647 in<sup>3</sup>  
 Moisture Content = 12.3%  
 Wet Unit Weight = 139.5 pcf  
 Dry Unit Weight = 124.2 pcf  
 Void Ratio = 0.32  
 Percent Saturation = 100%

B Parameter = 0.97  
 Shear Rate = 0.071% /min.  
 t<sub>50</sub> = 5.6 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 1.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.027% /min.  
 t<sub>50</sub> = 14.5 min.  
 Strain at Failure = 5.0%

Cell Pressure = 75 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 25 psi

Cell Pressure = 100 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 50 psi

Cell Pressure = 150 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey sand with gravel, yellowish brown, moist  
 Atterberg limits: LL = -- PL = -- PI = -- (-- indicates test was not performed)  
 Percent finer: 3/4 in. = -- No. 4 = -- No. 200 = -- (-- indicates test was not performed)  
 Specimen type: 

	Intact	X
	Cuttings	
	Wet	
	(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	
	Corrected	

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

X
---

 Entire specimen  
 Saturation method: 

X
---

 Dry  
 Failure criterion: 

--

 (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub>

5
---

 % strain  
 Membrane effect: 

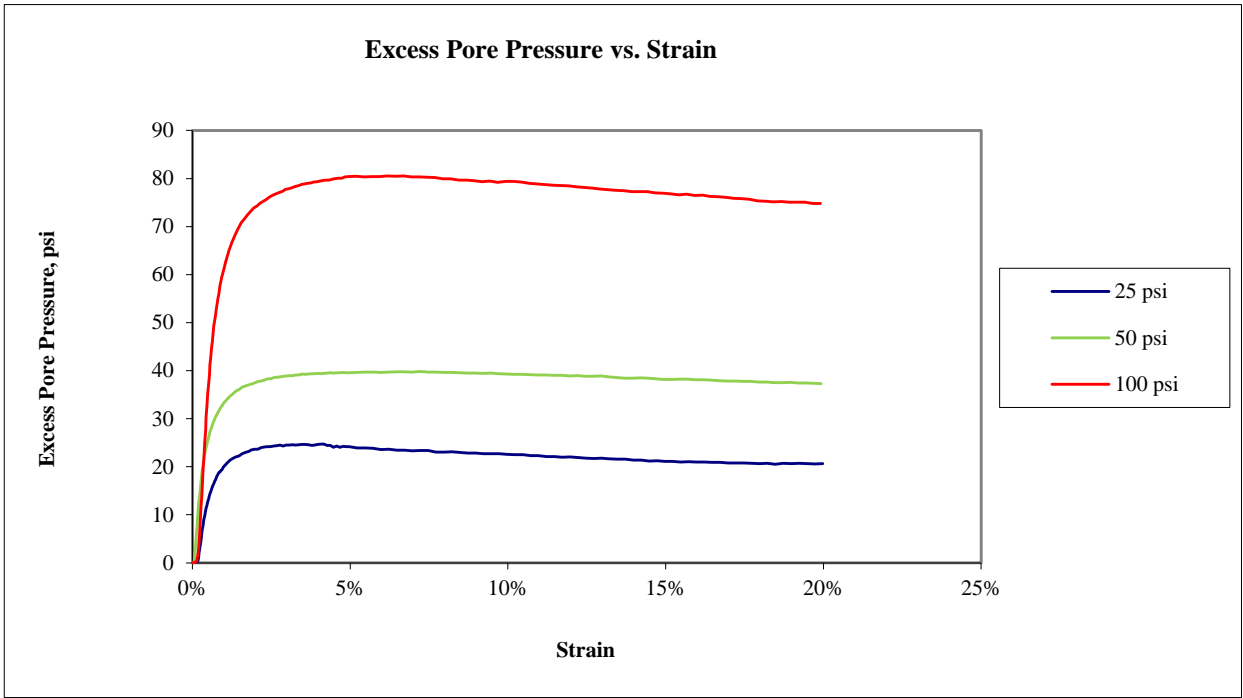
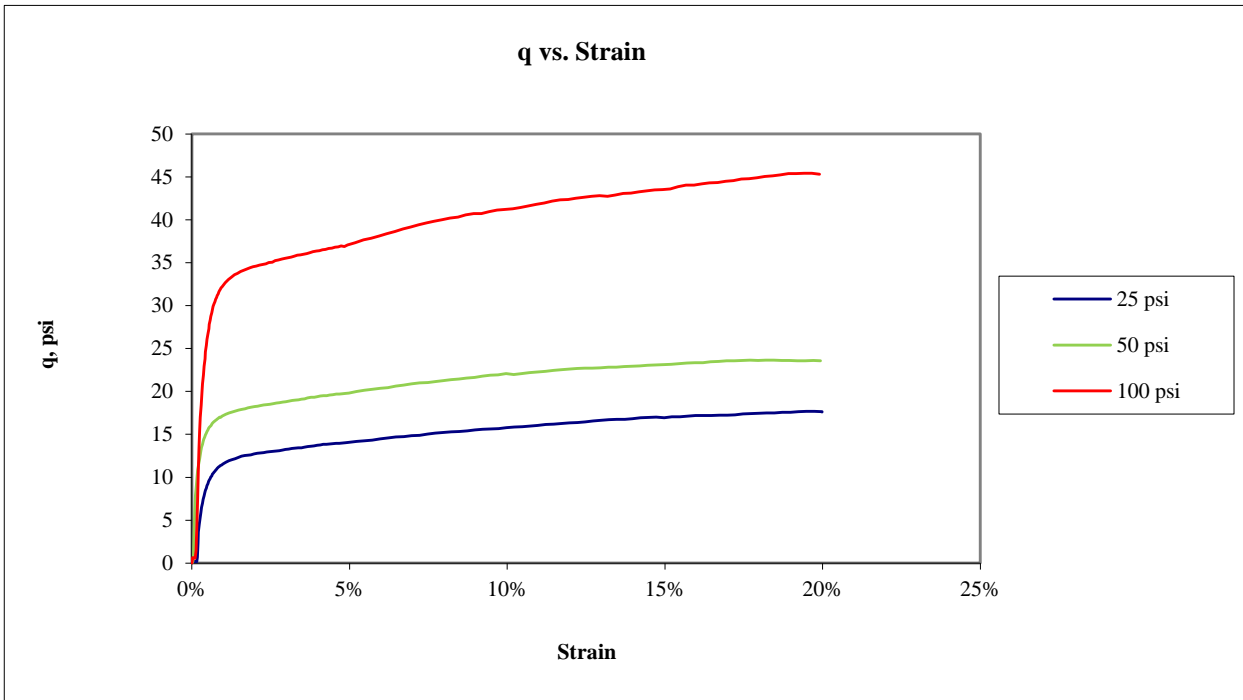
X
---

 Corrected 

--

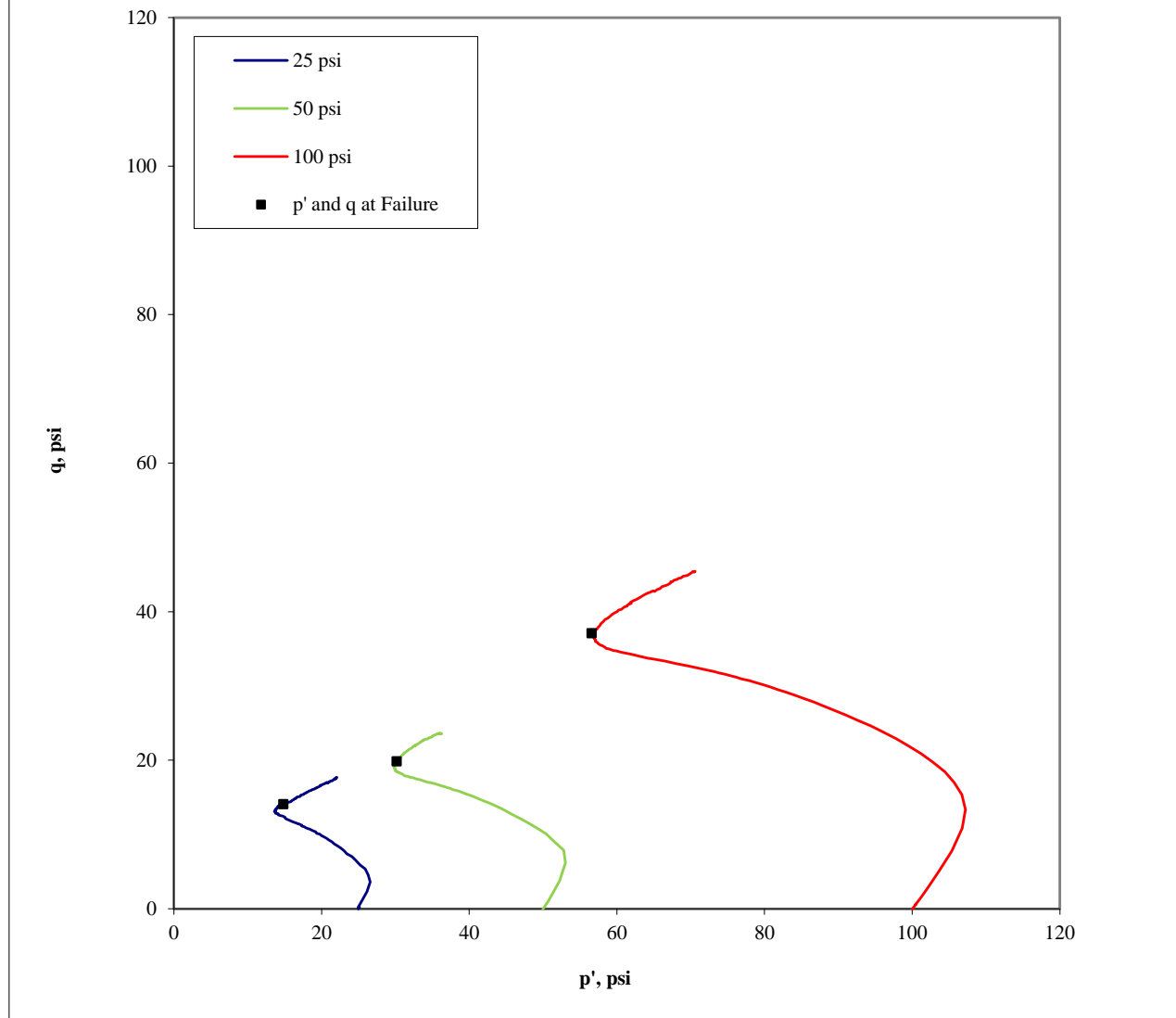
 Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

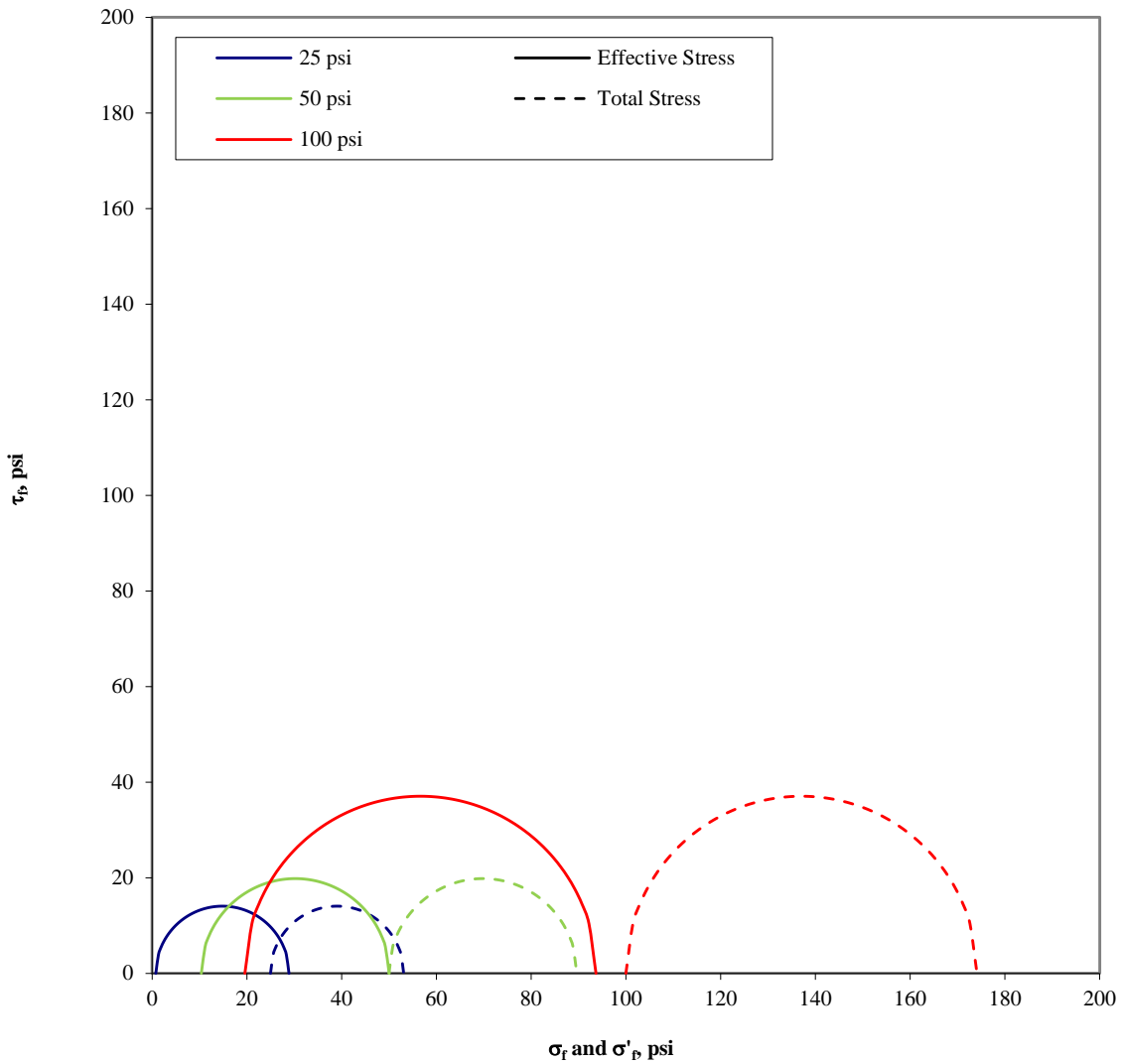
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	39.1	14.8	14.1
50	69.8	30.2	19.8
100	137.0	56.6	37.0

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>Composite 1-4</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>5/3/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	28.9	0.8	53.1	25.0
50	50.0	10.4	89.6	50.0
100	93.7	19.6	174.1	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM</b>			
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>Composite 1-4</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Date: <b>5/3/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>4</b>





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 3

**Initial**  
 Length = 5.751 in  
 Diameter = 2.882 in  
 Wet Mass = 2.743 lb  
 Area = 6.523 in<sup>2</sup>  
 Volume = 37.516 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.371 lb  
 Moisture Content = 15.7%  
 Wet Unit Weight = 126.3 pcf  
 Dry Unit Weight = 109.2 pcf  
 Void Ratio = 0.56  
 Percent Saturation = 76%

**Initial**  
 Length = 5.785 in  
 Diameter = 2.812 in  
 Wet Mass = 2.734 lb  
 Area = 6.210 in<sup>2</sup>  
 Volume = 35.927 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.367 lb  
 Moisture Content = 15.5%  
 Wet Unit Weight = 131.5 pcf  
 Dry Unit Weight = 113.8 pcf  
 Void Ratio = 0.50  
 Percent Saturation = 85%

**Initial**  
 Length = 5.764 in  
 Diameter = 2.877 in  
 Wet Mass = 2.724 lb  
 Area = 6.501 in<sup>2</sup>  
 Volume = 37.471 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.348 lb  
 Moisture Content = 16.0%  
 Wet Unit Weight = 125.6 pcf  
 Dry Unit Weight = 108.3 pcf  
 Void Ratio = 0.58  
 Percent Saturation = 76%

**After Consolidation**  
 Length = 5.697 in  
 Diameter = 2.772 in  
 Area = 6.035 in<sup>2</sup> (Method B)  
 Volume = 34.380 in<sup>3</sup>  
 Moisture Content = 15.8%  
 Wet Unit Weight = 138.0 pcf  
 Dry Unit Weight = 119.2 pcf  
 Void Ratio = 0.43  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.692 in  
 Diameter = 2.760 in  
 Area = 5.984 in<sup>2</sup> (Method B)  
 Volume = 34.058 in<sup>3</sup>  
 Moisture Content = 15.4%  
 Wet Unit Weight = 138.6 pcf  
 Dry Unit Weight = 120.1 pcf  
 Void Ratio = 0.42  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.579 in  
 Diameter = 2.731 in  
 Area = 5.859 in<sup>2</sup> (Method B)  
 Volume = 32.690 in<sup>3</sup>  
 Moisture Content = 13.7%  
 Wet Unit Weight = 141.2 pcf  
 Dry Unit Weight = 124.1 pcf  
 Void Ratio = 0.38  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 2.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.8 min.  
 Strain at Failure = 5.0%

Cell Pressure = 75 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 25 psi

Cell Pressure = 110 psi  
 Back Pressure = 60 psi  
 Confining Pressure = 50 psi

Cell Pressure = 150 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, reddish brown, moist  
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)  
 Percent finer: 3/4 in. = 91% No. 4 = 57% No. 200 = 31% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

Intact	<input checked="" type="checkbox"/>
Cuttings	<input type="checkbox"/>
Wet	<input type="checkbox"/>
(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	<input type="checkbox"/>
Corrected	<input type="checkbox"/>

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

Entire specimen	<input type="checkbox"/>
Dry	<input type="checkbox"/>

  
 Saturation method: 

Wet	<input checked="" type="checkbox"/>
Dry	<input type="checkbox"/>

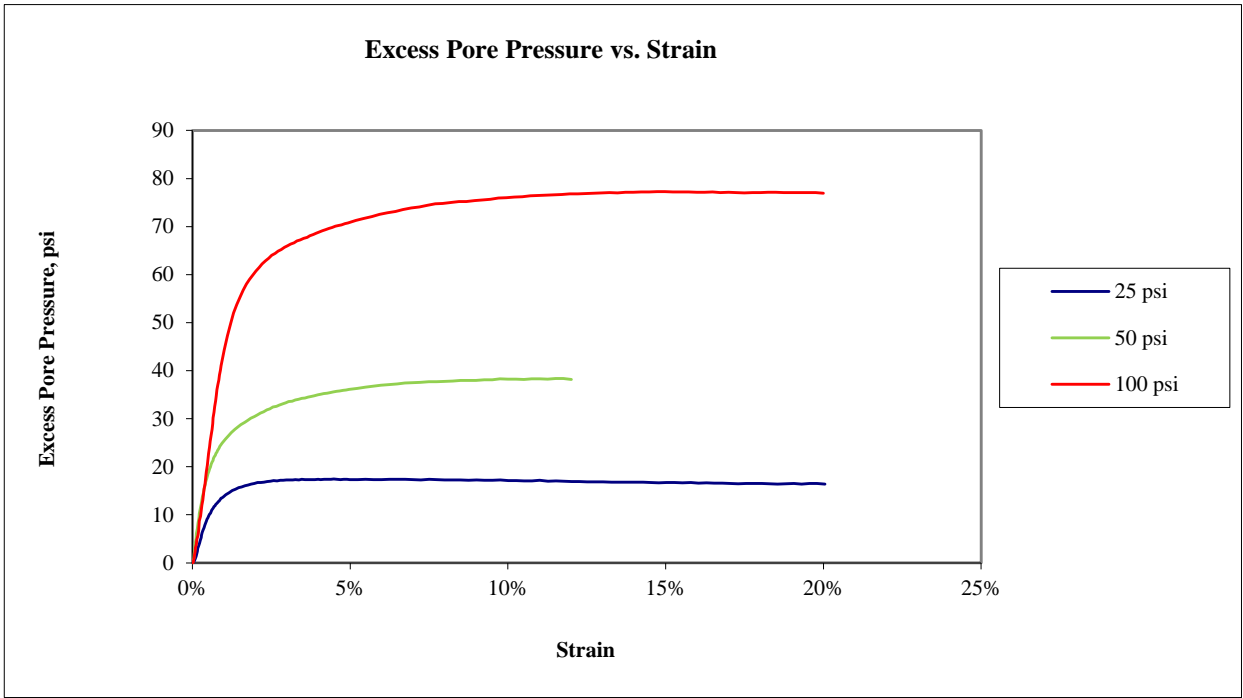
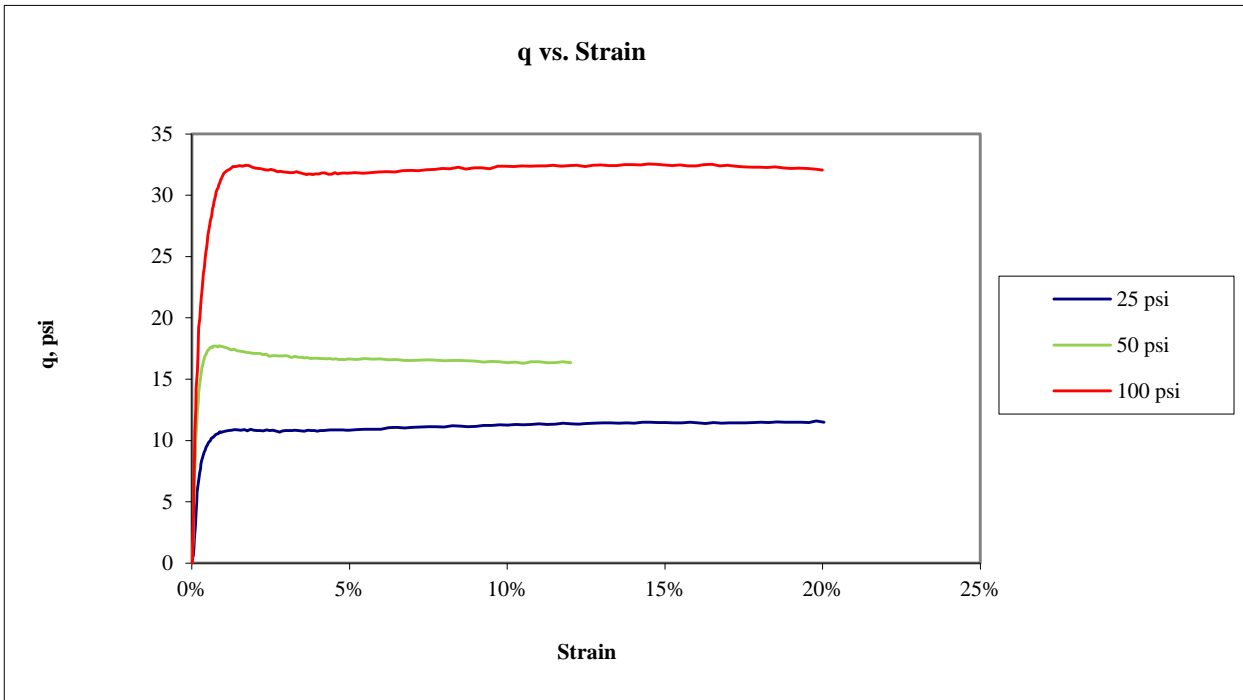
  
 Failure criterion: 

(σ <sub>1</sub> -σ <sub>3</sub> ) <sub>max</sub>	<input type="checkbox"/>
5 % strain	<input checked="" type="checkbox"/>

  
 Membrane effect: 

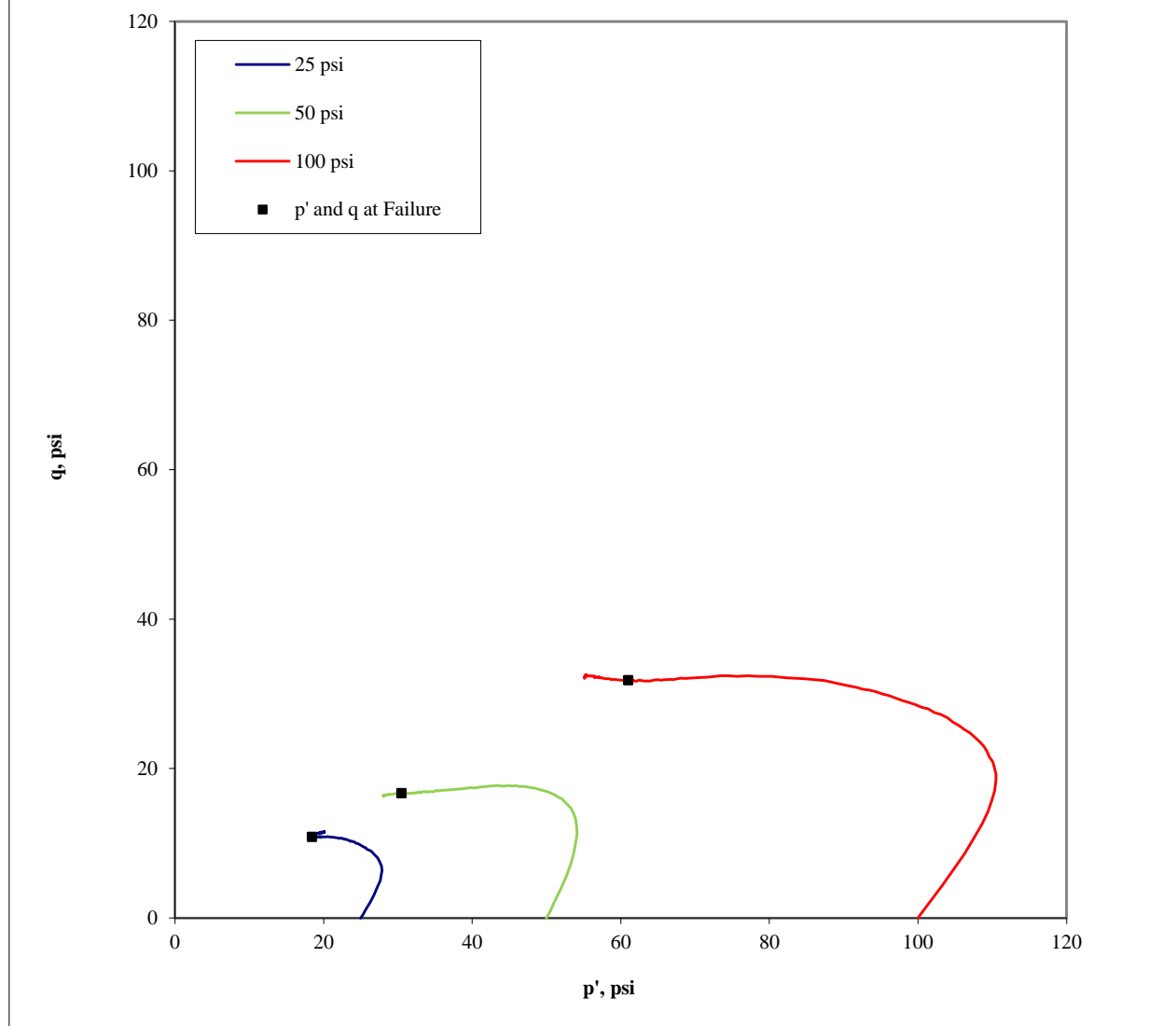
Corrected	<input checked="" type="checkbox"/>
Not Corrected	<input type="checkbox"/>

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

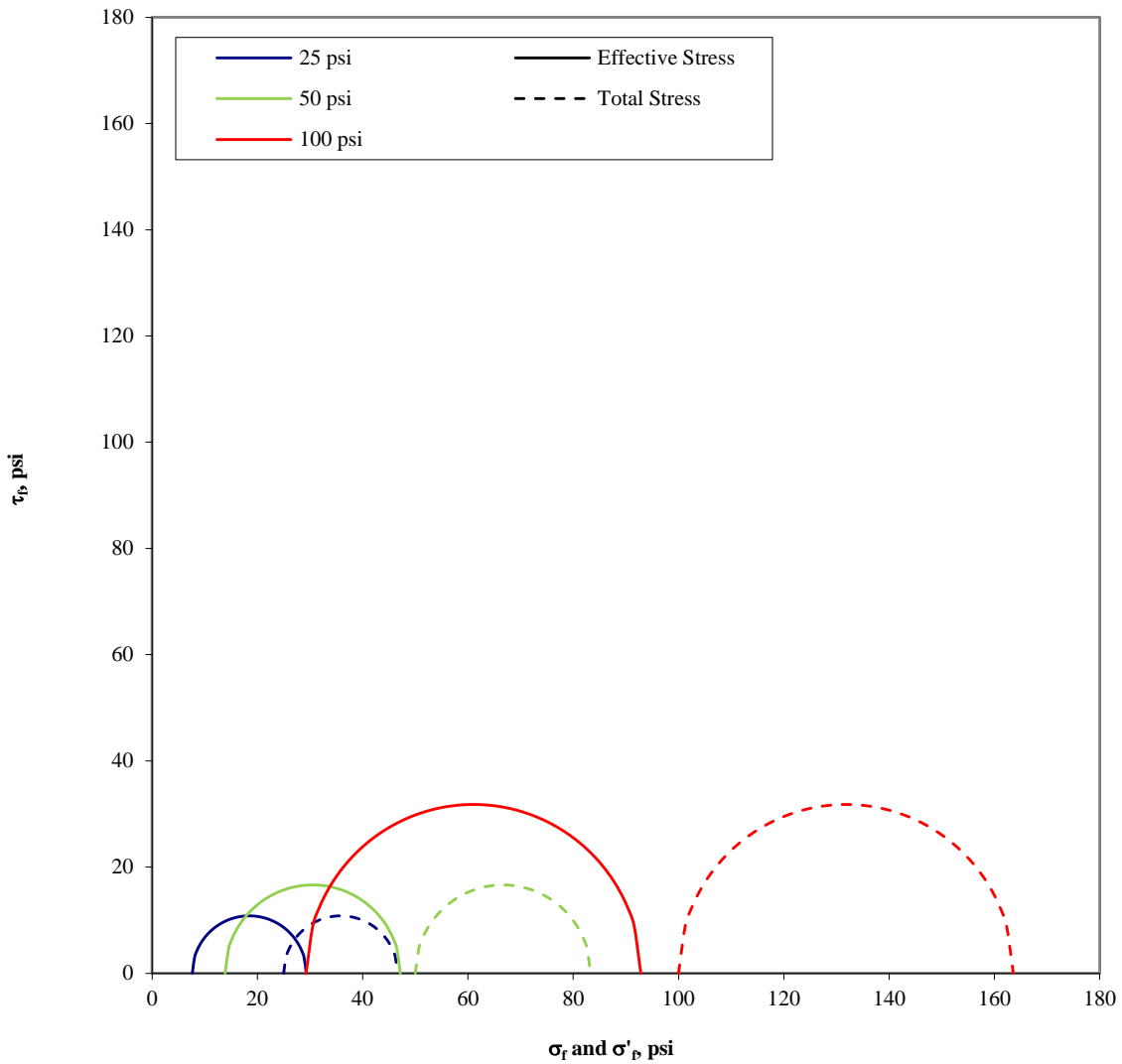
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	35.8	18.5	10.8
50	66.6	30.5	16.6
100	131.8	61.1	31.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>BH-16 @ 0.0 - 8.5 ft</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>3/26/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	29.3	7.6	46.7	25.0
50	47.2	13.9	83.3	50.0
100	92.8	29.3	163.6	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 4



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7	

Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 3

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.589 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.443 lb  
 Moisture Content = 15.4%  
 Wet Unit Weight = 127.6 pcf  
 Dry Unit Weight = 110.6 pcf  
 Void Ratio = 0.54  
 Percent Saturation = 78%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.601 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.421 lb  
 Moisture Content = 15.9%  
 Wet Unit Weight = 127.8 pcf  
 Dry Unit Weight = 110.3 pcf  
 Void Ratio = 0.55  
 Percent Saturation = 79%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.587 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.416 lb  
 Moisture Content = 15.8%  
 Wet Unit Weight = 127.6 pcf  
 Dry Unit Weight = 110.2 pcf  
 Void Ratio = 0.55  
 Percent Saturation = 79%

**After Consolidation**  
 Length = 9.195 in  
 Diameter = 3.788 in  
 Area = 11.267 in<sup>2</sup> (Method B)  
 Volume = 103.604 in<sup>3</sup>  
 Moisture Content = 13.7%  
 Wet Unit Weight = 141.2 pcf  
 Dry Unit Weight = 124.1 pcf  
 Void Ratio = 0.38  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.131 in  
 Diameter = 3.765 in  
 Area = 11.133 in<sup>2</sup> (Method B)  
 Volume = 101.651 in<sup>3</sup>  
 Moisture Content = 12.9%  
 Wet Unit Weight = 142.4 pcf  
 Dry Unit Weight = 126.2 pcf  
 Void Ratio = 0.35  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.054 in  
 Diameter = 3.749 in  
 Area = 11.037 in<sup>2</sup> (Method B)  
 Volume = 99.932 in<sup>3</sup>  
 Moisture Content = 12.1%  
 Wet Unit Weight = 143.7 pcf  
 Dry Unit Weight = 128.2 pcf  
 Void Ratio = 0.33  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.010% /min.  
 t<sub>50</sub> = 38.0 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.033% /min.  
 t<sub>50</sub> = 12.0 min.  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.047% /min.  
 t<sub>50</sub> = 8.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 65 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 25 psi

Cell Pressure = 90 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 50 psi

Cell Pressure = 145 psi  
 Back Pressure = 45 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, yellowish brown, moist  
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)  
 Percent finer: 3/4 in. = 88% No. 4 = 51% No. 200 = 17% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

	Intact	<input checked="" type="checkbox"/>	Reconstituted
	Cuttings	<input type="checkbox"/>	Entire specimen
	Wet	<input type="checkbox"/>	Dry
	(σ' <sub>1</sub> /σ' <sub>3</sub> ) <sub>max</sub>	<input type="checkbox"/>	(σ' <sub>1</sub> -σ' <sub>3</sub> ) <sub>max</sub> <input type="text" value="5"/> % strain
	Corrected	<input type="checkbox"/>	Not Corrected

 Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

<input checked="" type="checkbox"/>
-------------------------------------

  
 Saturation method: 

<input checked="" type="checkbox"/>
-------------------------------------

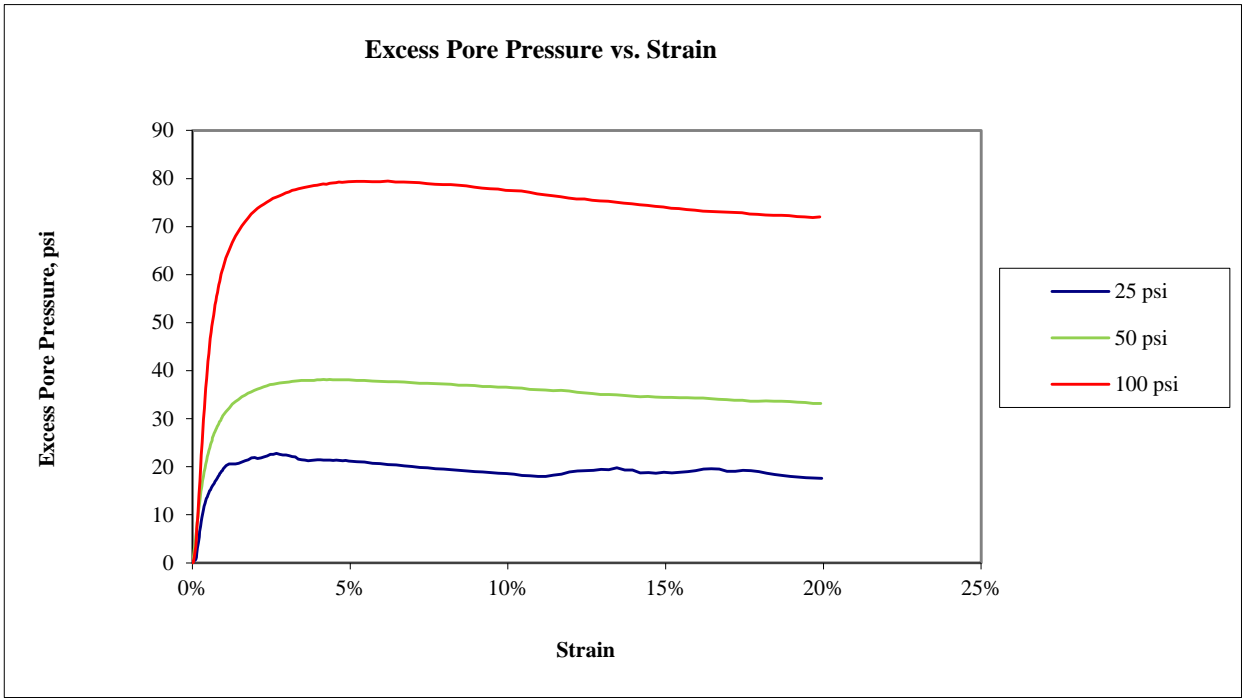
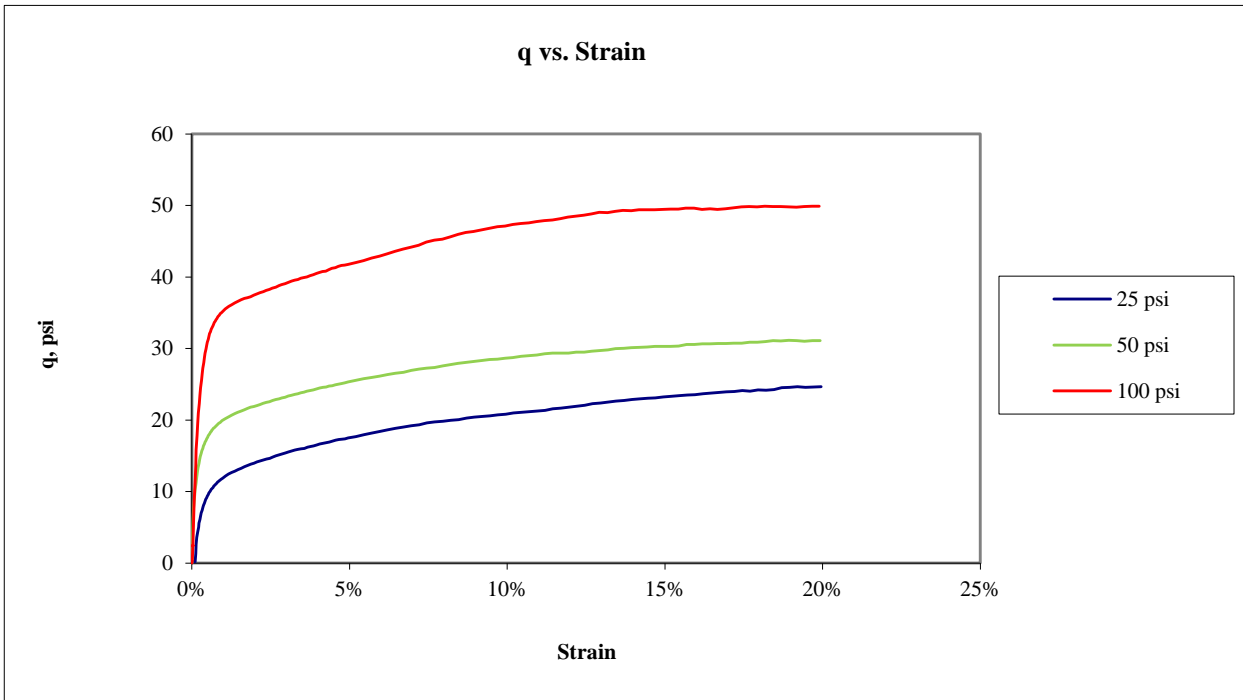
  
 Failure criterion: 

<input type="checkbox"/>
--------------------------

  
 Membrane effect: 

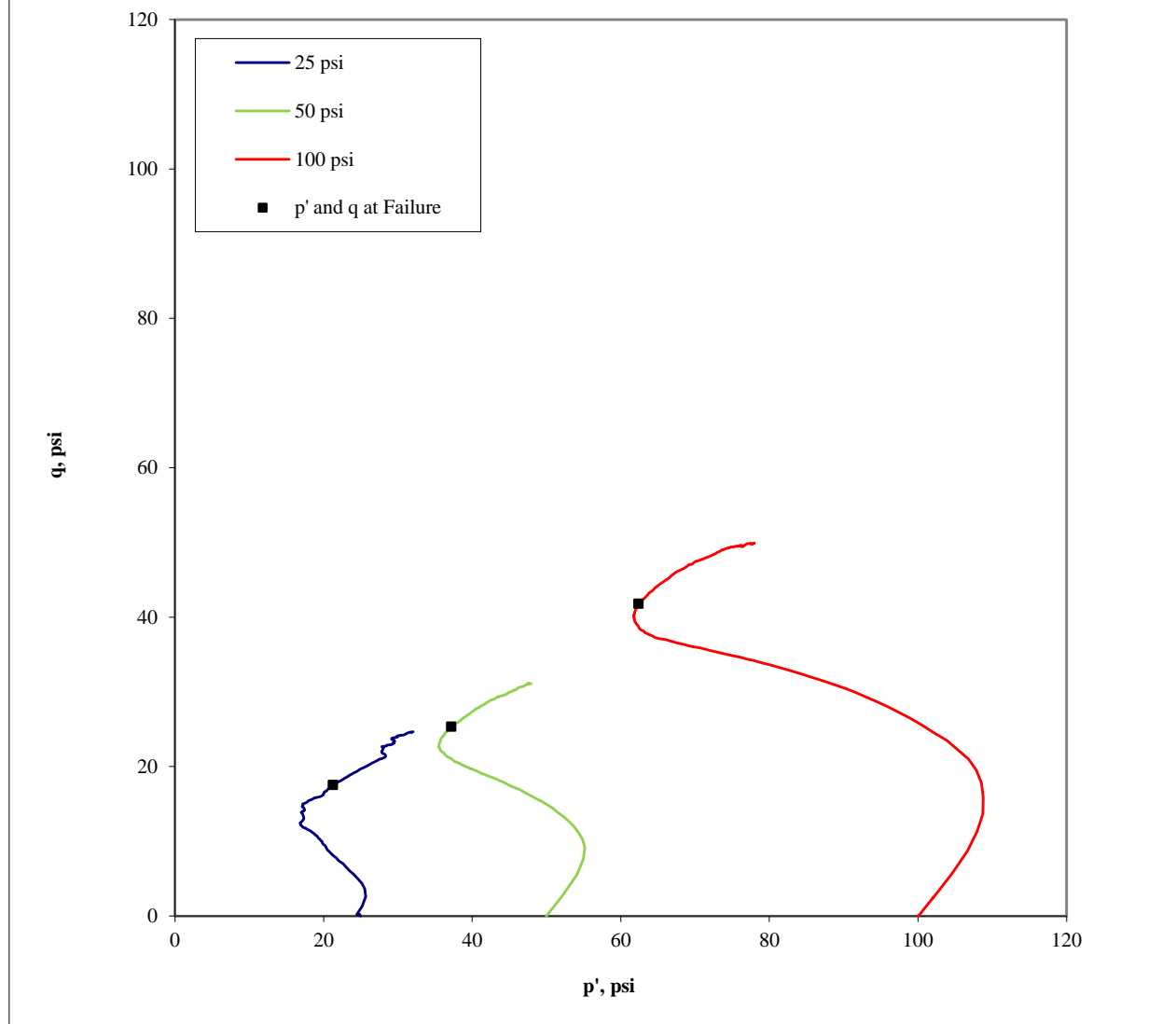
<input checked="" type="checkbox"/>
-------------------------------------

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

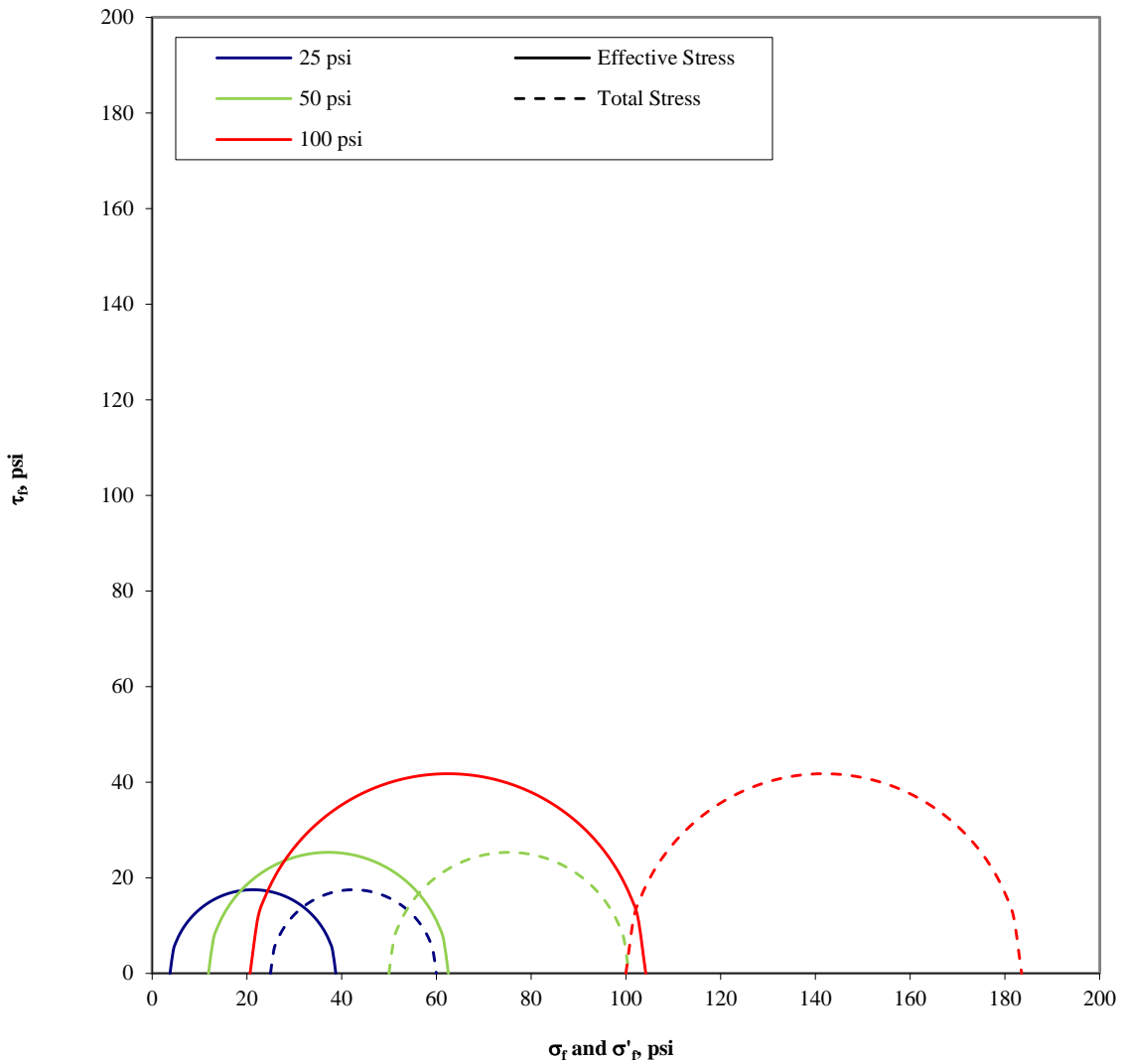
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	42.5	21.3	17.5
50	75.3	37.2	25.3
100	141.8	62.4	41.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>BH-25 @ 0.0 - 12.5 ft.</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>5/1/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	38.8	3.8	60.0	25.0
50	62.5	11.9	100.6	50.0
100	104.2	20.7	183.5	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 4



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 3

**Initial**  
 Length = 5.786 in  
 Diameter = 2.886 in  
 Wet Mass = 2.610 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.850 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.256 lb  
 Moisture Content = 15.7%  
 Wet Unit Weight = 119.2 pcf  
 Dry Unit Weight = 103.0 pcf  
 Void Ratio = 0.66  
 Percent Saturation = 65%

**Initial**  
 Length = 5.784 in  
 Diameter = 2.886 in  
 Wet Mass = 2.611 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.836 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.247 lb  
 Moisture Content = 16.2%  
 Wet Unit Weight = 119.2 pcf  
 Dry Unit Weight = 102.6 pcf  
 Void Ratio = 0.66  
 Percent Saturation = 67%

**Initial**  
 Length = 5.796 in  
 Diameter = 2.886 in  
 Wet Mass = 2.607 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.915 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.240 lb  
 Moisture Content = 16.4%  
 Wet Unit Weight = 118.8 pcf  
 Dry Unit Weight = 102.1 pcf  
 Void Ratio = 0.67  
 Percent Saturation = 67%

**After Consolidation**  
 Length = 5.747 in  
 Diameter = 2.842 in  
 Area = 6.345 in<sup>2</sup> (Method B)  
 Volume = 36.467 in<sup>3</sup>  
 Moisture Content = 21.8%  
 Wet Unit Weight = 130.2 pcf  
 Dry Unit Weight = 106.9 pcf  
 Void Ratio = 0.60  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.724 in  
 Diameter = 2.855 in  
 Area = 6.400 in<sup>2</sup> (Method B)  
 Volume = 36.631 in<sup>3</sup>  
 Moisture Content = 22.3%  
 Wet Unit Weight = 129.6 pcf  
 Dry Unit Weight = 106.0 pcf  
 Void Ratio = 0.61  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.667 in  
 Diameter = 2.884 in  
 Area = 6.532 in<sup>2</sup> (Method B)  
 Volume = 37.016 in<sup>3</sup>  
 Moisture Content = 23.1%  
 Wet Unit Weight = 128.7 pcf  
 Dry Unit Weight = 104.6 pcf  
 Void Ratio = 0.63  
 Percent Saturation = 100%

B Parameter = 0.96  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.5 min.  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.6 min.  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 95 psi  
 Back Pressure = 70 psi  
 Confining Pressure = 25 psi

Cell Pressure = 130 psi  
 Back Pressure = 80 psi  
 Confining Pressure = 50 psi

Cell Pressure = 190 psi  
 Back Pressure = 90 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Silty sand, brownish yellow, moist  
 Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 49% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

Intact	X
Cuttings	X

 Reconstituted Remold targets: 104.7 pcf (dry) at 16.2% moisture  
 Moisture from: 

Wet	
Dry	

 Entire specimen  
 Saturation method: 

X	Wet
	Dry

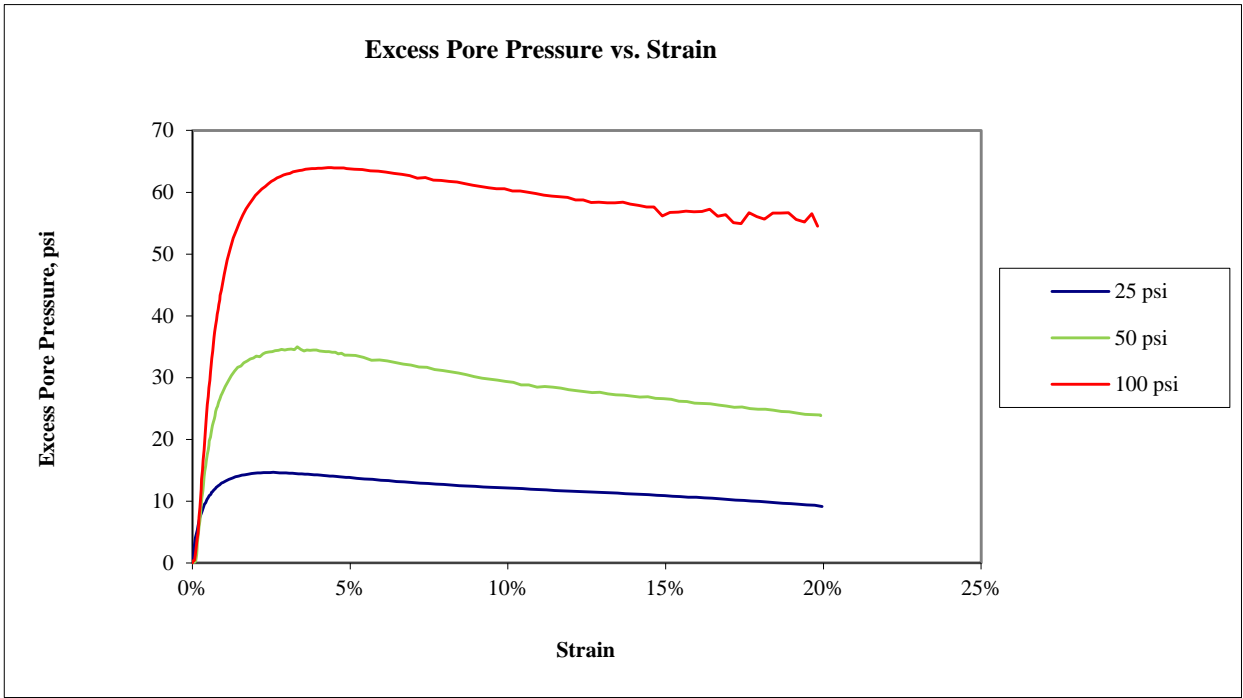
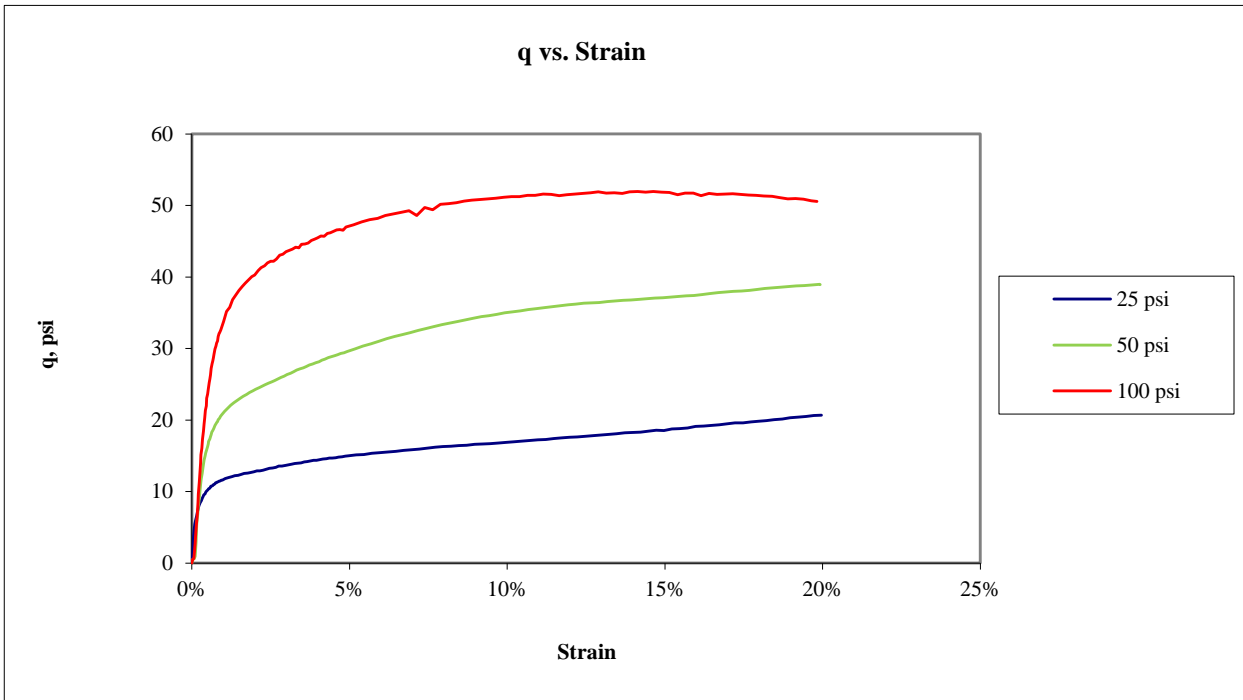
  
 Failure criterion: 

(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	(σ <sub>1</sub> -σ <sub>3</sub> ) <sub>max</sub>	5
--	--	---

 % strain  
 Membrane effect: 

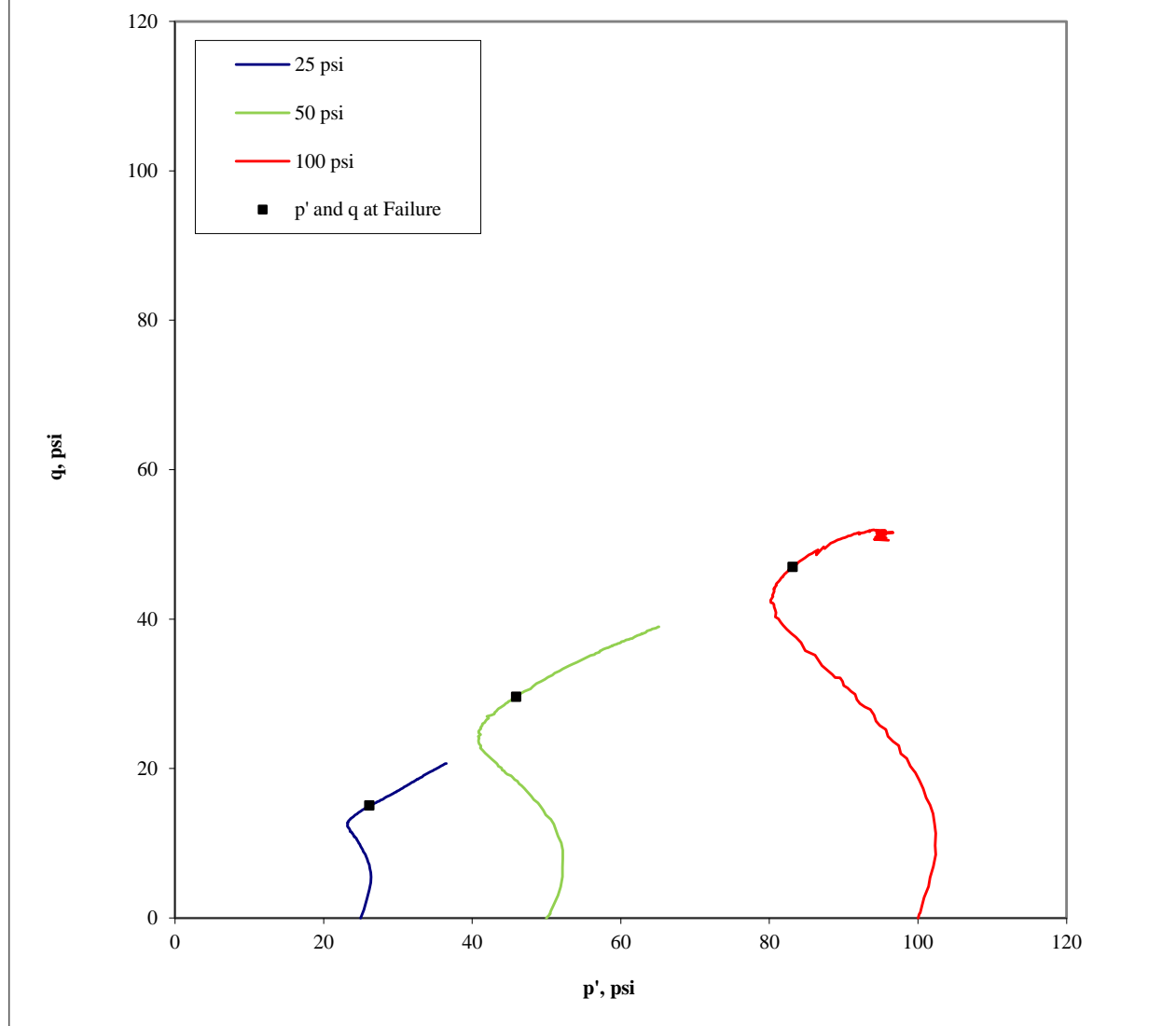
X	Corrected	Not Corrected
---	-----------	---------------

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

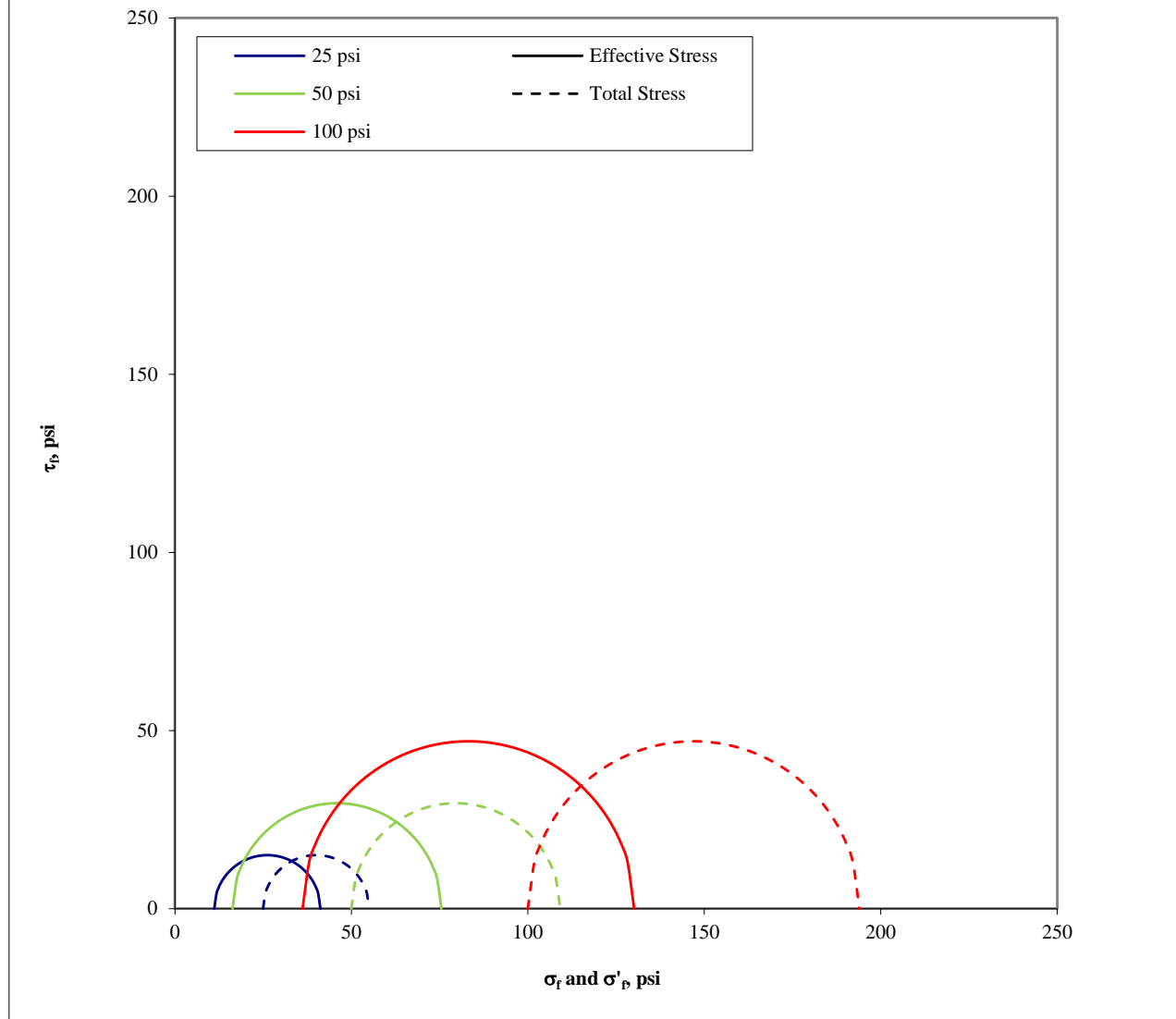
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	40.0	26.2	15.0
50	79.6	45.9	29.6
100	147.0	83.2	47.0

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>TP-10 @ 3 - 12 ft</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>4/8/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	41.2	11.2	55.0	25.0
50	75.5	16.4	109.2	50.0
100	130.1	36.2	193.9	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM</b>			
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>TP-10 @ 3 - 12 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Date: <b>4/8/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>4</b>



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT			
		SPECIMEN PHOTOGRAPH - 25 psi			
<b>Sample:</b>	TP-10 @ 3 - 12 ft	<b>Technician:</b>	RJM	<b>Reviewed:</b>	CCS
		<b>Date:</b>	4/8/2013	<b>Job Number:</b>	103-92557.006
				<b>Figure:</b>	5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7



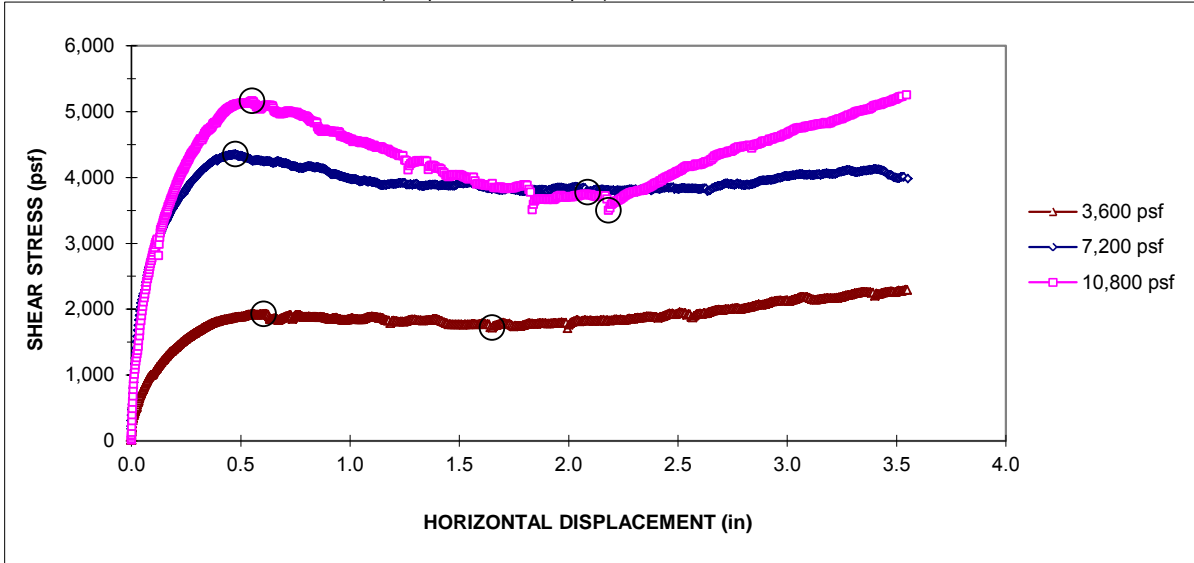
### DIRECT SHEAR TEST RESULTS

ASTM D5321

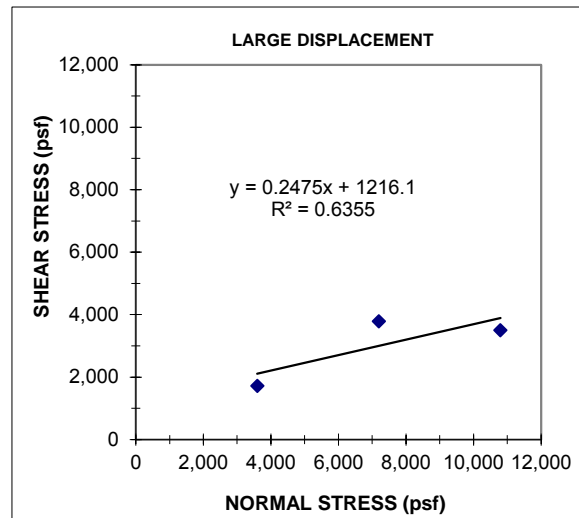
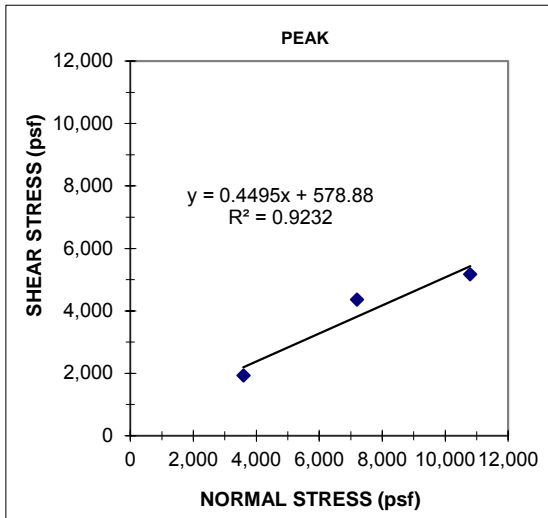
PROJECT NAME: Copper Flat Tailings Design Study

SAMPLE NUMBER: Tailings / 80-mil Textured Geomembrane / Drain Rock

**APPARTUS:** 12.0 inch by 12.0 inch shear box; air bladder used to apply normal loads.  
**INTERFACE TESTED:** Top: Tailings (remolded to 104.7 pcf @ 15.7% moisture content)  
 Bottom: GSE 80-mil HDPE double-sided textured geomembrane  
**TEST CONDITIONS:** Drain Rock inundated; consolidated overnight at normal load; Floating geomembrane  
**SHEAR RATE:** 0.04 in/min  
**SUBSTRATE:** Drain Rock (Composites 1-4, scalped)



Normal Stress (psf)	Shear Stress		Peak		Large Displacement	
	Peak <sup>1</sup> (psf)	Lg. Displ. (psf)	Friction Angle	Adhesion <sup>2</sup> (psf)	Friction Angle	Adhesion <sup>2</sup> (psf)
3,600	1,927	1,717	24.2	578.9	13.9	1216.1
7,200	4,354	3,778				
10,800	5,164	3,500				



**Observations After Test**

Peak and residual shear strengths were chosen based on testing observations. Shear stresses measured at greater horizontal displacements than those chosen for the large-displacement shear strength appeared to have been affected by gravel particles pushing against the end of the bottom shear box.

3,600 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface

7,200 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface

10,800 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface

Drain Gravel interfered with shear boxes at approximately 2.2 inches displacement and may have affected

Tech: PRH

Review: CCS

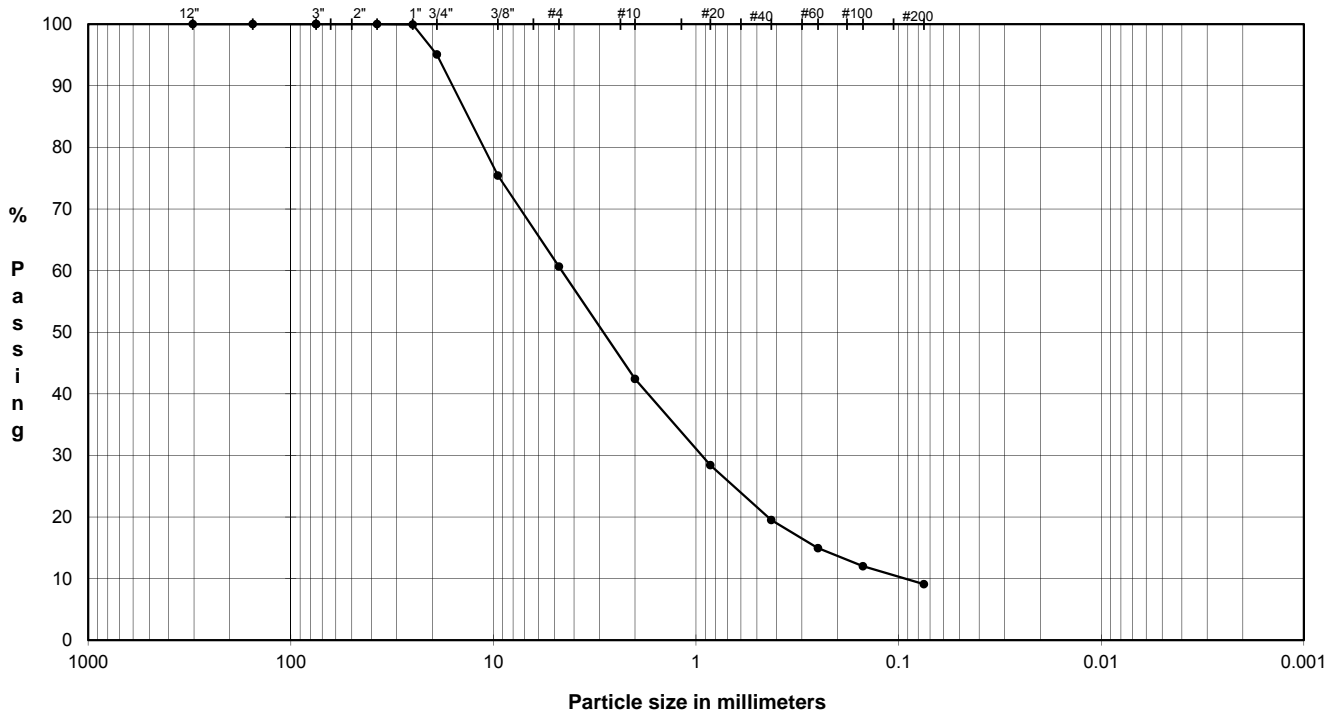
(1) Peak shear stresses for 3,600 psf, 7,200 psf, and 10,800 psf normal stresses were chosen at 0.606, 0.476, and 0.552 inches horizontal displacements, respectively.

(2) Interface shear parameters are based on the "best-fit" line per ASTM D5321. Interpretation of the test data by a qualified professional for the specific application is required.

**APPENDIX A.3.3  
PERMEABILITY TEST REPORTS**

**PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4** Depth (ft) **--**  
 TYPE: **Pail**

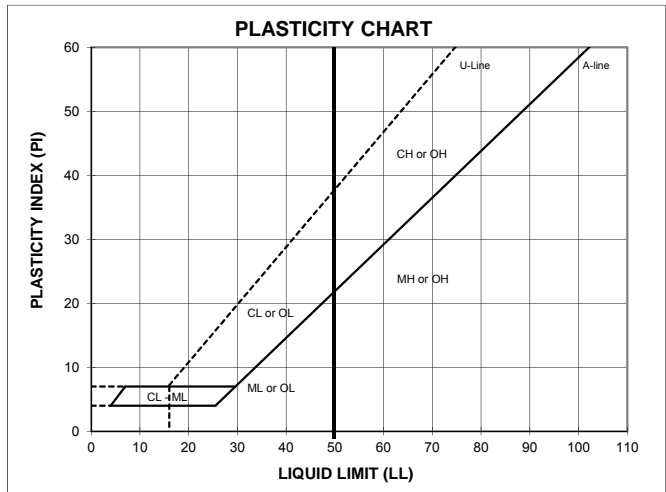


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**PRE-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2		<b>100.0</b>
3.0"	75.0		<b>100.0</b>
3.0"	75.0		<b>100.0</b>
1.5"	37.5		<b>100.0</b>
1.0"	25.0		<b>99.9</b>
0.75"	19.0	Coarse Gravel	<b>4.93</b>
0.375"	9.5		<b>75.4</b>
#4	4.8	Fine Gravel	<b>34.42</b>
#10	2.0	Coarse Sand	<b>18.25</b>
#20	0.9		<b>28.4</b>
#40	0.4	Medium Sand	<b>22.89</b>
#60	0.3		<b>14.9</b>
#100	0.2		<b>12.0</b>
#200	0.1	Fine Sand	<b>10.42</b>
		Fines	<b>9.09</b>



**ATTERBERG LIMITS**

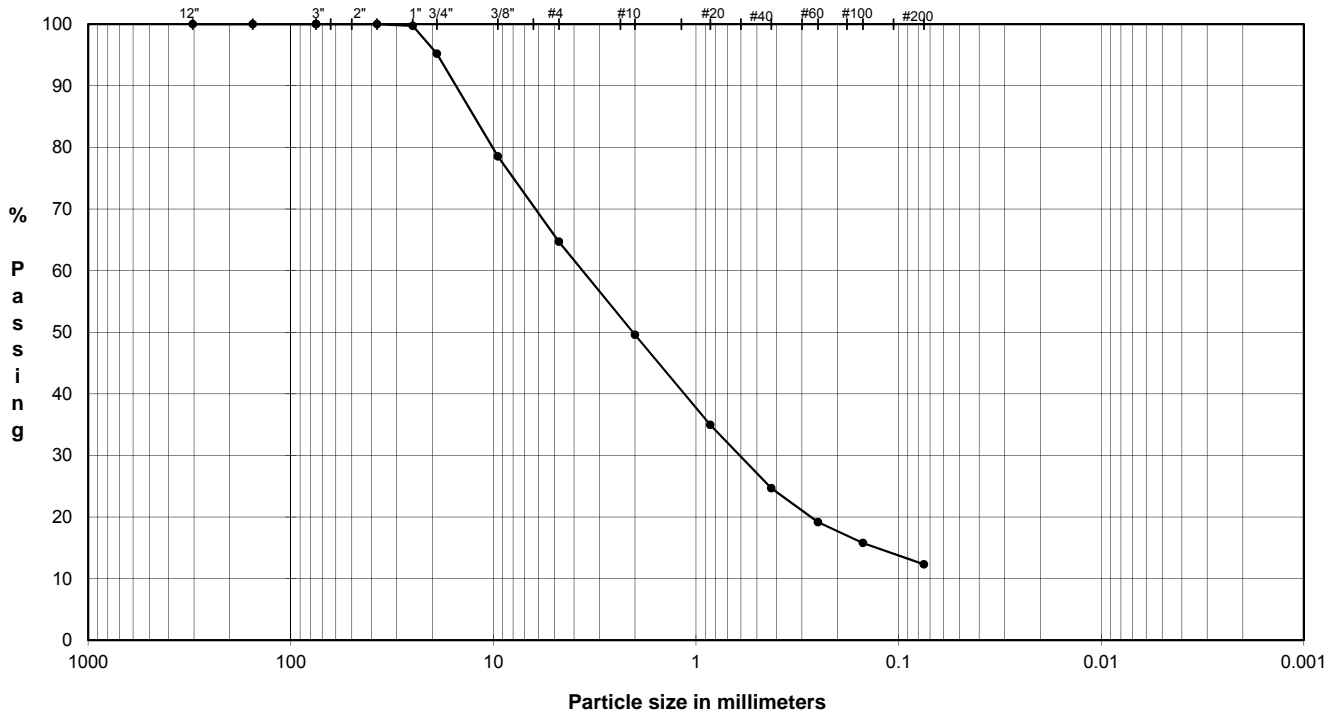
$M_c$	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown,  
 (Golder Procedure): dry  
 USCS: --

TECH: EH  
 DATE: 4/10/13  
 REVIEW: MB

**POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4** Depth (ft) **--**  
 TYPE: **Pail**

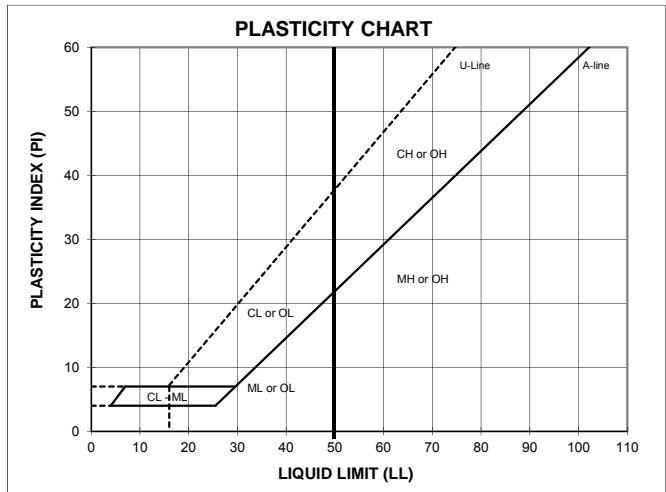


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**POST-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2	Cobbles	<b>0.00</b>
3.0"	75.0		
3.0"	75.0		
1.5"	37.5	Coarse Gravel	<b>4.79</b>
1.0"	25.0		
0.75"	19.0		
0.375"	9.5	Fine Gravel	<b>30.52</b>
#4	4.8		
#10	2.0	Coarse Sand	<b>15.12</b>
#20	0.9		
#40	0.4	Medium Sand	<b>24.90</b>
#60	0.3		
#100	0.2	Fine Sand	<b>12.36</b>
#200	0.1		
Fines			<b>12.31</b>



**ATTERBERG LIMITS**

$M_c$	LL	PL	PI	SG
--	--	--	--	--

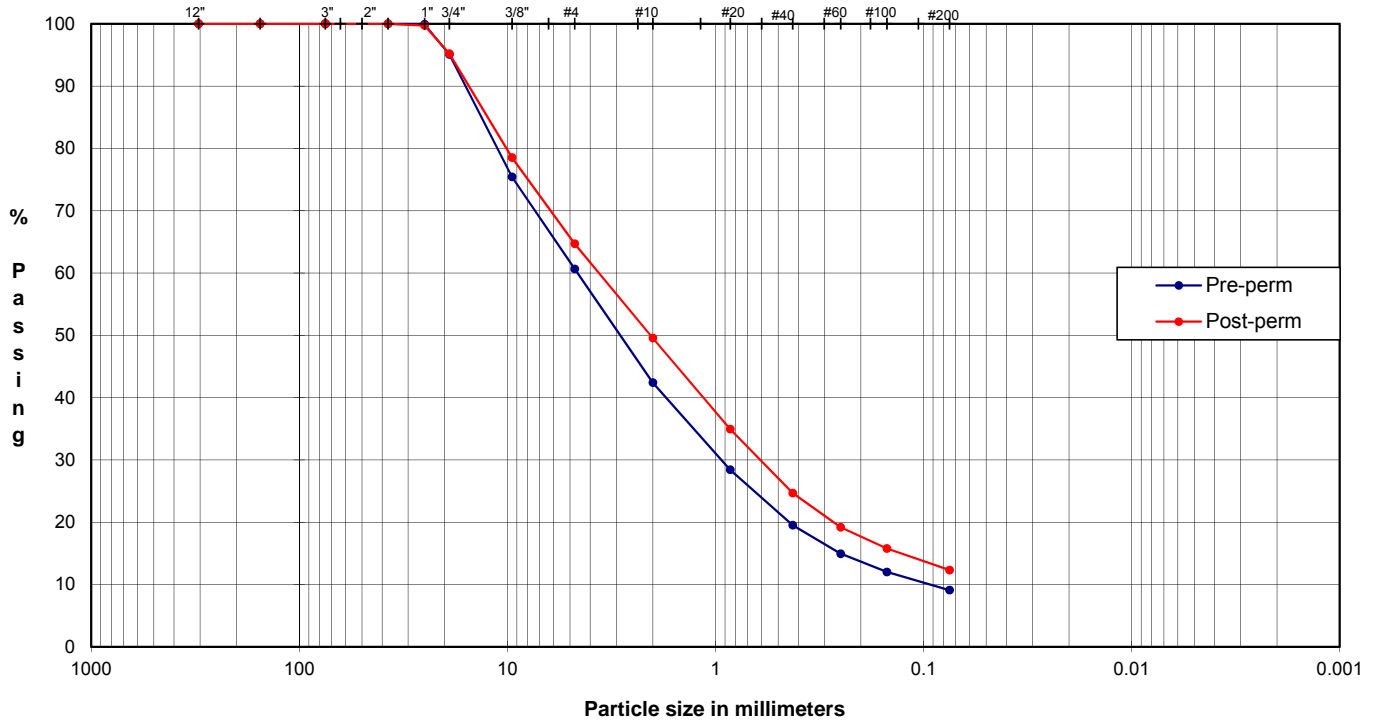
Visual Description: silty clayey SAND and GRAVEL, yellowish brown,  
 (Golder Procedure): dry  
 USCS: --

TECH: EH  
 DATE: 4/16/2013  
 REVIEW: MB

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4**  
 TYPE: **Pail**

Depth (ft) --



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			

#### PRE-PERM

#### POST-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles <b>0.00</b>
6.0"	154.2	100.0	
3.0"	75.0	100.0	
3.0"	75.0	100.0	
1.5"	37.5	100.0	
1.0"	25.0	99.9	
0.75"	19.0	95.1	Coarse Gravel <b>4.93</b>
0.375"	9.5	75.4	Fine Gravel <b>34.42</b>
#4	4.8	60.7	
#10	2.0	42.4	Coarse Sand <b>18.25</b>
#20	0.9	28.4	Medium Sand <b>22.89</b>
#40	0.4	19.5	
#60	0.3	14.9	
#100	0.2	12.0	Fine Sand <b>10.42</b>
#200	0.1	9.1	
Fines			<b>9.09</b>

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles <b>0.00</b>
6.0"	154.2	100.0	
3.0"	75.0	100.0	
3.0"	75.0	100.0	
1.5"	37.5	100.0	
1.0"	25.0	99.7	
0.75"	19.0	95.2	Coarse Gravel <b>4.79</b>
0.375"	9.5	78.5	Fine Gravel <b>30.52</b>
#4	4.8	64.7	
#10	2.0	49.6	Coarse Sand <b>15.12</b>
#20	0.9	34.9	Medium Sand <b>24.90</b>
#40	0.4	24.7	
#60	0.3	19.2	
#100	0.2	15.8	Fine Sand <b>12.36</b>
#200	0.1	12.3	
Fines			<b>12.31</b>

#### ATTERBERG LIMITS

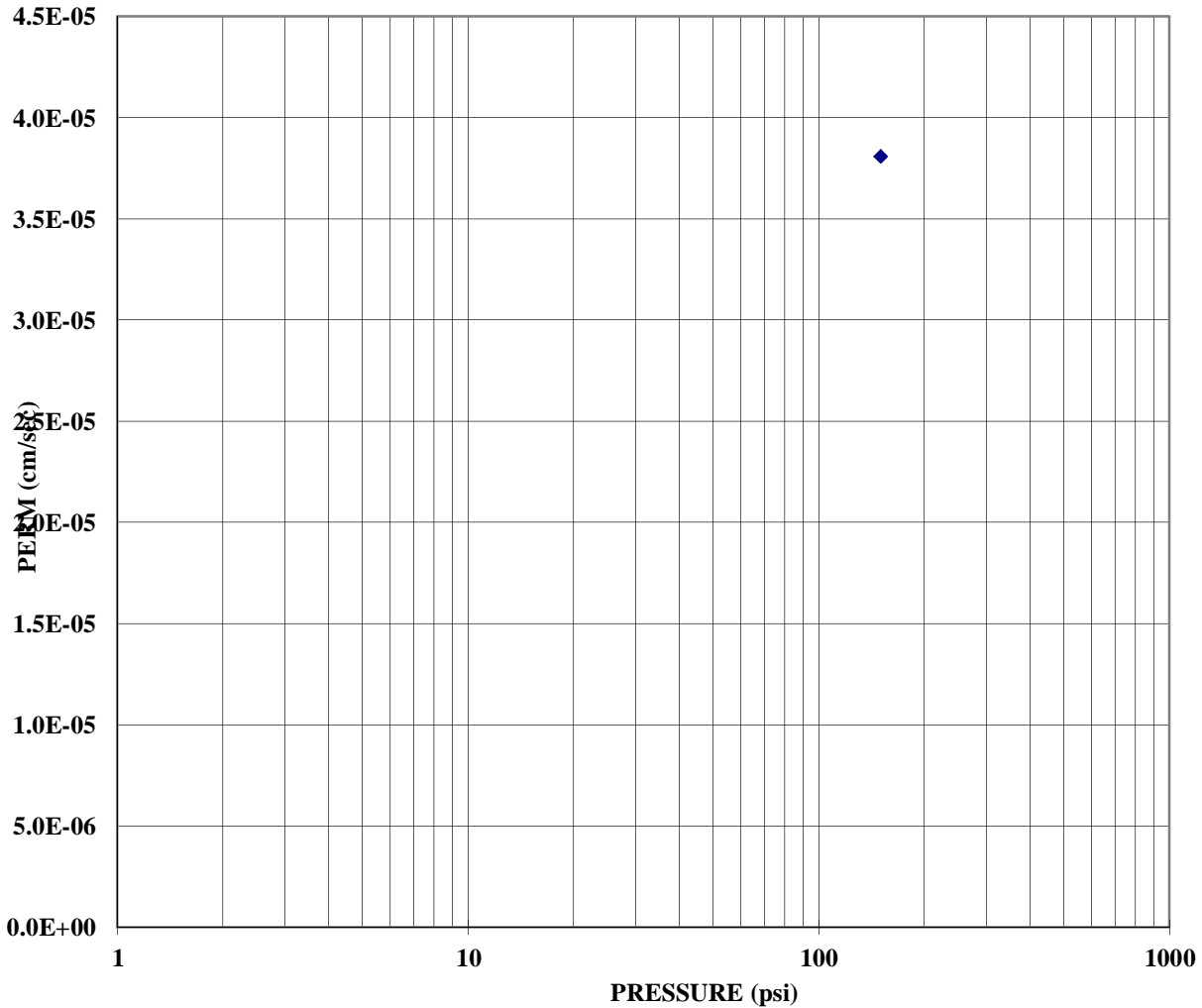
$M_L$	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown, (Golder Procedure): dry

USCS: --

TECH: EH  
 DATE: 4/10/13  
 REVIEW: MB

ONE-DIMENSIONAL CONSOLIDATION X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Repor



SAMPLE #: Composite 1-4

Visual Description: silty clayey SAND and  
 (Golder Procedure): GRAVEL, yellowish brown, dry

DATE 4/11/2013

TECH MGC

REVIEW MB

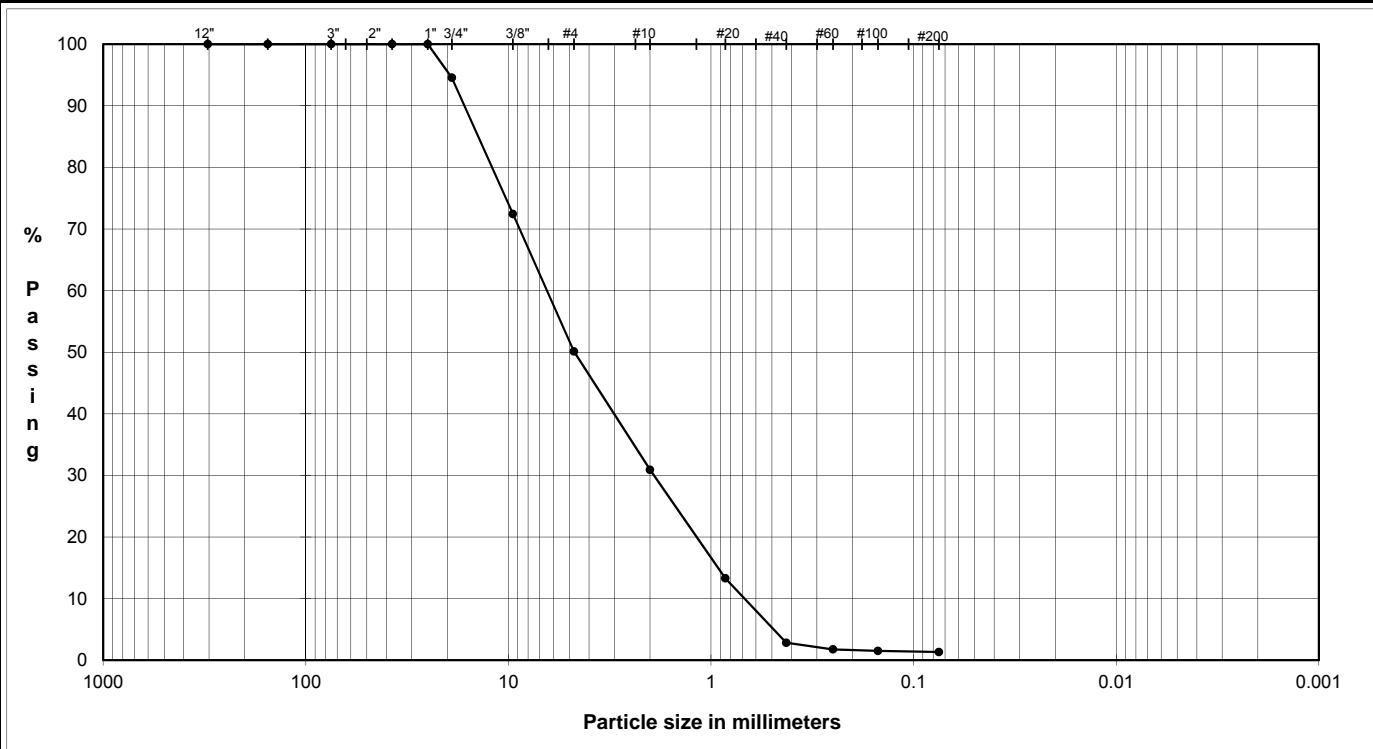
103-92557.006  
 Copper Flat Tailings Design Study

GOLDER ASSOCIATES INC.  
 LAKEWOOD, COLORADO



**PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED**      Depth (ft)    --  
 TYPE: **Pail**

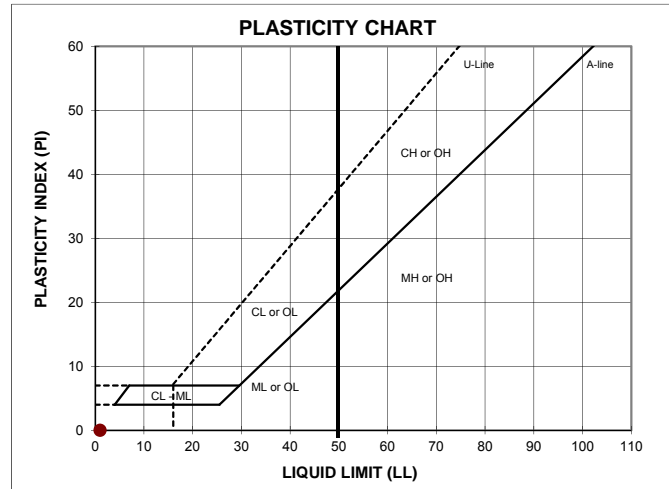


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**PRE-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		
6.0"	154.2		
3.0"	75.0	Cobbles	0.00
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	5.45
0.375"	9.5		
#4	4.8	Fine Gravel	44.42
#10	2.0	Coarse Sand	19.22
#20	0.9		
#40	0.4	Medium Sand	28.06
#60	0.3		
#100	0.2		
#200	0.1	Fine Sand	1.51
		Fines	1.33



**ATTERBERG LIMITS**

$M_c$	LL	PL	PI	SG
--	--	--	--	--

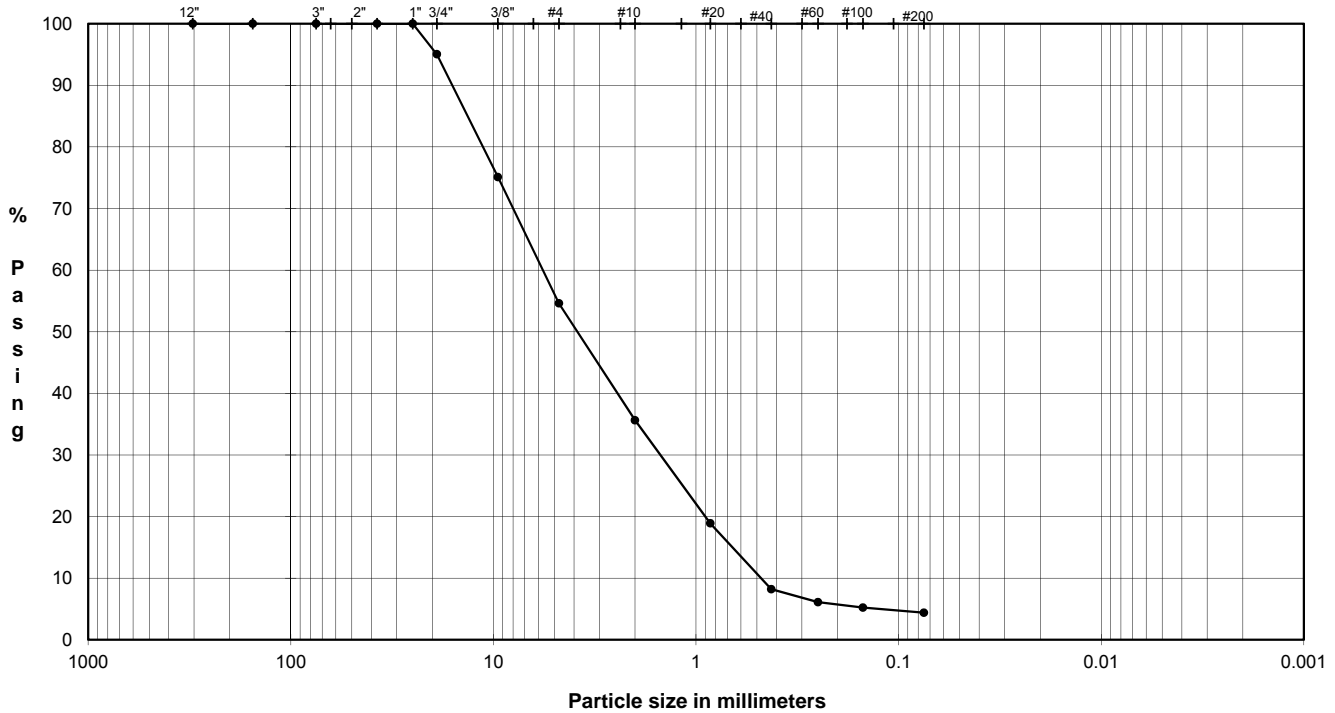
DESCRIPTION: SAND and GRAVEL, yellowish brown

USCS: --

TECH: AM  
 DATE: 4/22/13  
 REVIEW: PRH

**POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) **--**  
 TYPE: **Pail**

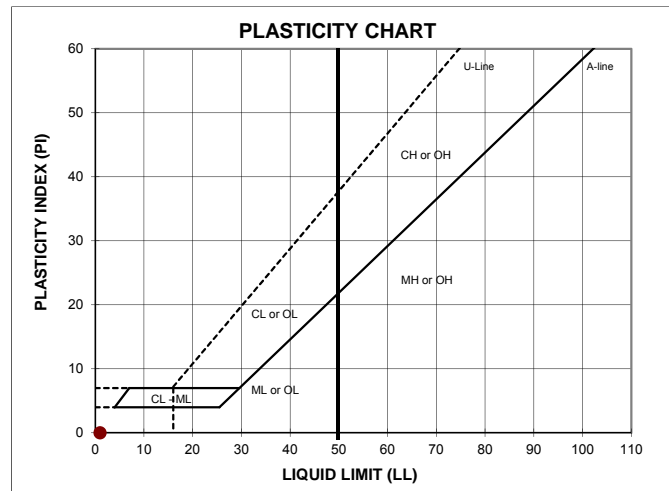


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**POST-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2		<b>100.0</b>
3.0"	75.0	Cobbles	<b>0.00</b>
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	<b>4.96</b>
0.375"	9.5		<b>75.1</b>
#4	4.8	Fine Gravel	<b>40.43</b>
#10	2.0	Coarse Sand	<b>18.97</b>
#20	0.9		<b>18.9</b>
#40	0.4	Medium Sand	<b>27.43</b>
#60	0.3		<b>6.1</b>
#100	0.2		<b>5.2</b>
#200	0.1	Fine Sand	<b>3.82</b>
		Fines	<b>4.39</b>



**ATTERBERG LIMITS**

M <sub>c</sub>	LL	PL	PI	SG
--	--	--	--	--

DESCRIPTION: SAND and GRAVEL, yellowish brown

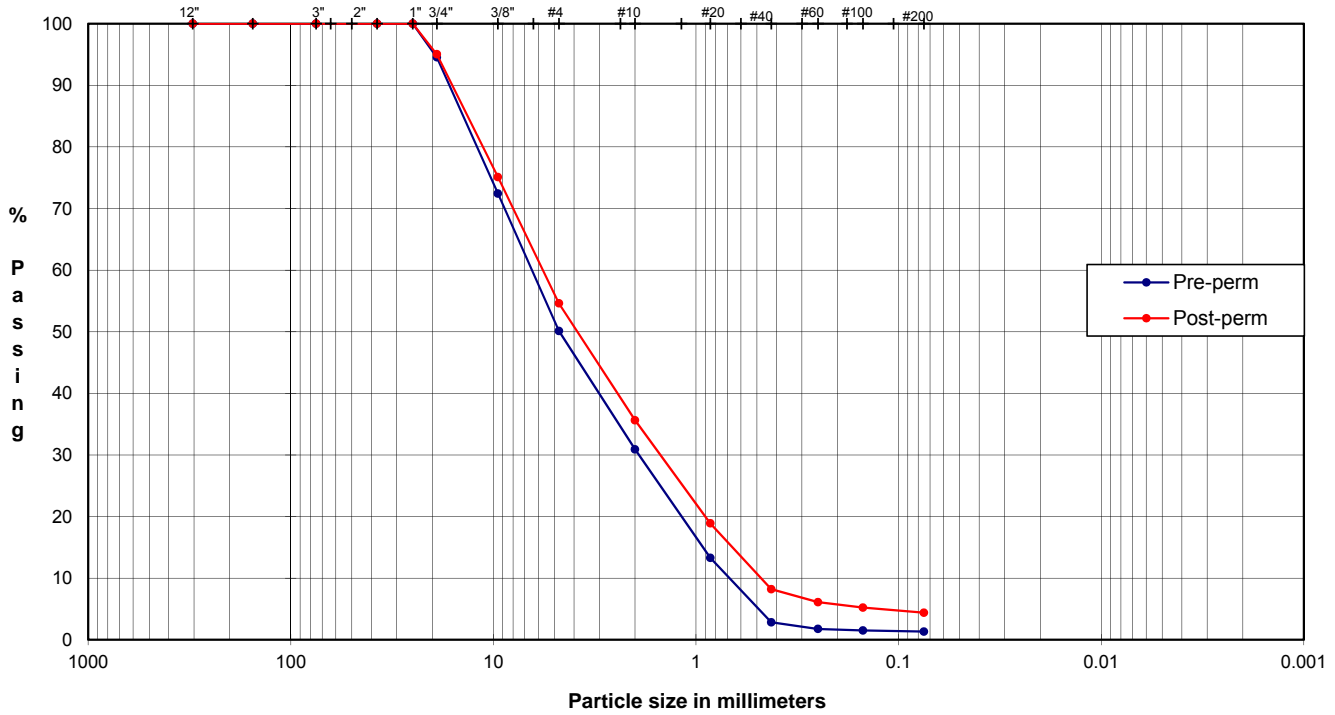
USCS: --

TECH: EH  
 DATE: 5/1/2013  
 REVIEW: PRH



**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED**      Depth (ft)    --  
 TYPE: **Pail**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

**PRE-PERM**

**POST-PERM**

U.S. Standard Sieves Sizes and Numbers	Particle Size		Classification	Percentage
	(mm)	% Passing		
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	100.0		
1.0"	25.0	100.0	Coarse Gravel	5.45
0.75"	19.0	94.5		
0.375"	9.5	72.4	Fine Gravel	44.42
#4	4.8	50.1		
#10	2.0	30.9	Coarse Sand	19.22
#20	0.9	13.3	Medium Sand	28.06
#40	0.4	2.8		
#60	0.3	1.8		
#100	0.2	1.5	Fine Sand	1.51
#200	0.1	1.3		
			Fines	1.33

U.S. Standard Sieves Sizes and Numbers	Particle Size		Classification	Percentage
	(mm)	% Passing		
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	100.0		
1.0"	25.0	100.0	Coarse Gravel	4.96
0.75"	19.0	95.0		
0.375"	9.5	75.1	Fine Gravel	40.43
#4	4.8	54.6		
#10	2.0	35.6	Coarse Sand	18.97
#20	0.9	18.9	Medium Sand	27.43
#40	0.4	8.2		
#60	0.3	6.1		
#100	0.2	5.2	Fine Sand	3.82
#200	0.1	4.4		
			Fines	4.39

**ATTERBERG LIMITS**

M <sub>c</sub>	LL	PL	PI	SG
--	--	--	--	--

DESCRIPTION: SAND and GRAVEL, yellowish brown  
 USCS: --

TECH: AM  
 DATE: 4/22/13  
 REVIEW: PRH

**Copper Flat Tailing Design Study**  
**Table 1 - Rigid Wall Compression**  
**Falling Head Permeability - 10 inch diameter cell**

**Project Title:** Copper Flat Tailings Design Study  
**Project Number:** 103-92557.006  
**Dates Tested:** 4/26/2013 To: 4/29/2013  
**Boring:** --  
**Sample:** Comp 1-4 SCALPED  
**Depth (ft):** --

**Sample Setup**

Initial Sample Height, in	9.194
Mold Diameter, in	10.00
Sample Area, in <sup>2</sup>	78.54
Wet Sample Weight, g	19,998.9
Wet Sample Weight, lb	44.10
Dry Sample Weight, g	19,416.3
Dry Sample Weight, lb	42.81

**Initial Sample:**

**Moisture Determination**

Tare	PGC
Wet Weight and Tare, g	663.26
Dry Weight and Tare, g	647.65
Tare Weight, g	127.37
Moisture Content, %	3.0

**Initial Sample Density and Void Ratio**

Specific Gravity <sup>1</sup>	2.70
Initial Sample Volume, ft <sup>3</sup>	0.418
Initial Wet Density, lb/ft <sup>3</sup>	105.5
Initial Dry Density, lb/ft <sup>3</sup>	102.5
Initial Void Ratio	0.64

**Final Sample Density and Void Ratio**

Final Sample Height, in	8.330
Final Sample Volume, ft <sup>3</sup>	0.379
Final Dry Density, lb/ft <sup>3</sup>	113.1
Final Void Ratio	0.49

Load (psi)	Height (in)	Dry Density (pcf)	Void Ratio	Flow Rate (ml/sec)	Gradient	Permeability (cm/sec)	Porosity
150	8.330	113.1	0.49	10.62	0.23	9.1E-02	0.33

**NOTES:** <sup>1</sup>Specific Gravity = Assumed Value

### CONSTANT-HEAD PERMEABILITY (RIGID-WALL)

JOB NUMBER: 103-92557.006  
 JOB NAME: Copper Flat Tailings Design Study  
 DATE TESTED: 04/26/13

BORING NUMBER: --  
 SAMPLE NUMBER: Comp 1-4 SCALPED  
 SAMPLE DEPTH: --

**Initial Moisture Content**

Tare:	PGC
Wet Weight & Tare, g:	663.26
Dry Weight & Tare, g:	647.65
Tare Weight, g:	127.37
Moisture, %:	<b>3.0</b>

**Final Moisture Content**

Tare:	J11
Wet Weight & Tare, g:	662.43
Dry Weight & Tare, g:	607.20
Tare Weight, g:	82.33
Moisture, %:	<b>10.5</b>

**Initial Height Determination (Inches)**

(Height from mold rim to plate on sample)	
1.	3.447
2.	3.442
3.	3.404
4.	3.379
5.	3.404
6.	3.365
Average	3.406
Cell Height	12.600
Sample Height <sup>1</sup>	9.194

**Density**

Wet Weight:	19,998.90	g
Dry Weight:	19,416.35	g
Diameter:	10.000	in      25.40 cm
Area:	78.540	in <sup>2</sup> 506.71 cm <sup>2</sup>
Initial Height:	9.194	in      23.35 cm
Final Height <sup>2</sup> :	8.330	in      21.16 cm
Initial Volume:	0.418	ft <sup>3</sup> 11,832.48 cm <sup>3</sup>
Final Volume:	0.379	ft <sup>3</sup> 10,721.13 cm <sup>3</sup>
Initial Wet Density:	105.6	pcf
Final Wet Density:	116.5	pcf
Initial Dry Density:	102.5	pcf
Final Dry Density:	113.1	pcf

TRIAL	TIME	OUTFLOW	BURETTE	Q	i	k
	seconds	ml	(mm)	(cm <sup>3</sup> /sec)		cm/sec
01	240	1704.9	35	7.1	0.165	8.48E-02
02	240	1704.5	35	7.1	0.165	8.47E-02
03	240	1705.2	35	7.1	0.165	8.48E-02
04	240	2513.2	48	10.5	0.227	9.11E-02
05	240	2508.93	48	10.5	0.227	9.09E-02
06	240	2507.75	48	10.4	0.227	9.09E-02
07	240	3436.38	62	14.3	0.293	9.64E-02
08	240	3434.01	62	14.3	0.293	9.64E-02
09	240	3435.08	62	14.3	0.293	9.64E-02
10						
<b>Average</b>	240.0	2549.996	48.3	10.6	0.228	9.07E-02

**NOTES:** <sup>1</sup>Sample Height = Cell Height - Average distance to rim of mold

<sup>2</sup>Final Height = Initial Height - Displacement of Final Reading *Before* Running Perm Test

**CONSTANT-HEAD PERMEABILITY  
(RIGID-WALL)**

JOB NUMBER: 103-92557.006  
 JOB NAME: Copper Flat Tailings Design Study  
 DATE TESTED: 04/26/13

BORING NUMBER: --  
 SAMPLE NUMBER: Comp 1-4 SCALPED  
 SAMPLE DEPTH: --

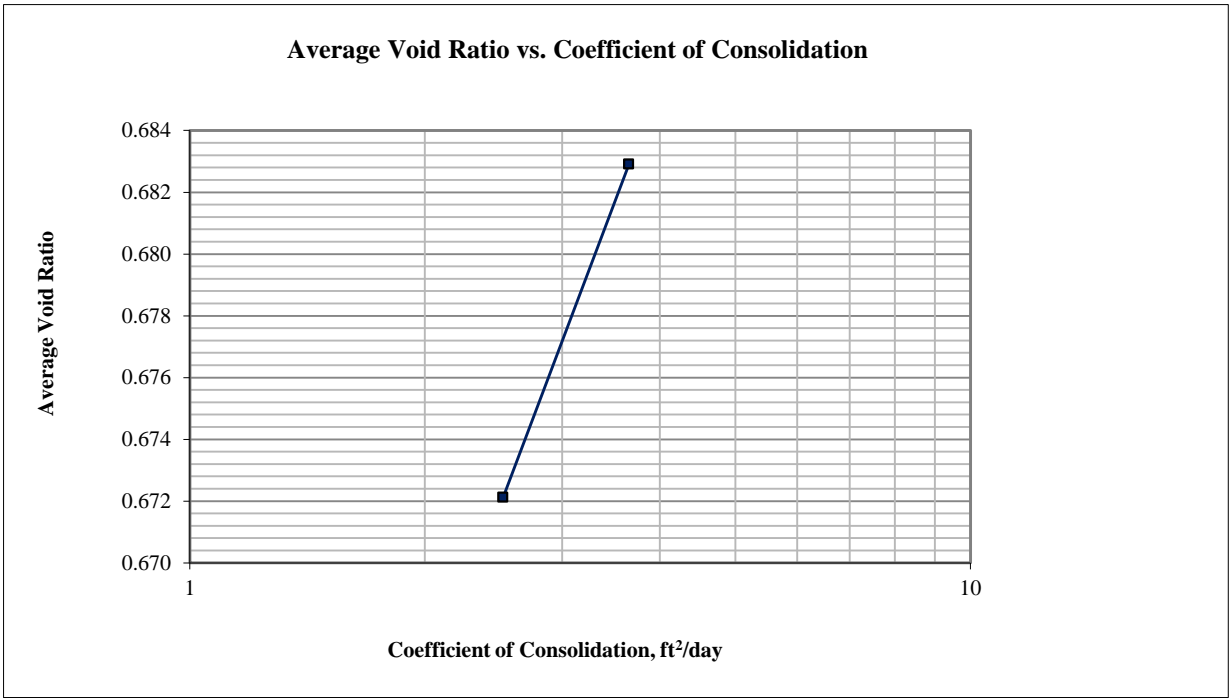
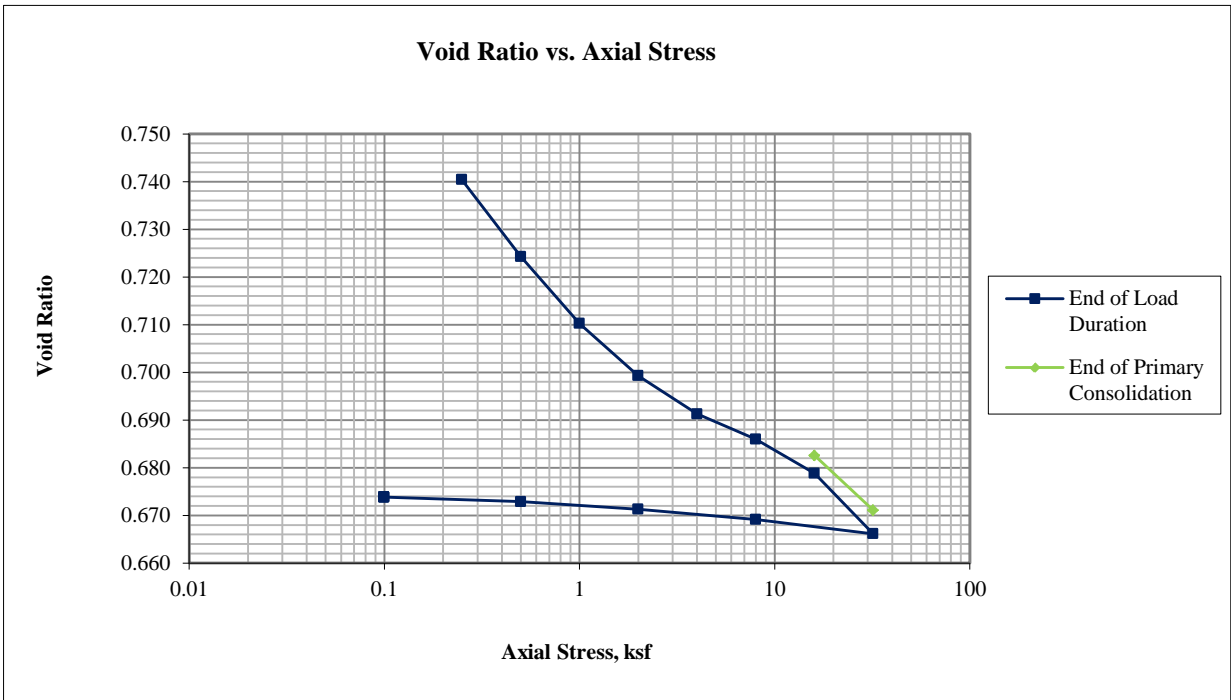
DATE	TIME	LOAD (psi)	DIAL (Left)	DIAL (Right)	AVERAGE	DISPLACEMENT
INITIAL	10:10 AM	0	0.005	0.004	0.005	--
SATURATE	10:25 AM	0	0.021	0.044	0.033	0.028
LOAD	12:40 PM	150	0.775	0.742	0.759	0.754
	8:55 AM	150	0.880	0.844	0.862	0.858
PERM	7:55 AM	150	0.886	0.850	0.868	0.864
END PERM	12:10 PM	150	0.887	0.851	0.869	0.865

**APPENDIX A.3.4  
CONSOLIDATION TEST REPORTS**

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.994 in		0.960 in	USCS description (ASTM D2487):	Lean clay with sand, yellowish red, moist
Diameter =	2.499 in		2.499 in	Atterberg Limits (ASTM D4318):	LL = 47 PL = 20 PI = 27
Area =	4.905 in <sup>2</sup>		4.905 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 99% No. 200 = 81%
Volume =	4.875 in <sup>3</sup>		4.709 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	14.4%		4.8%	Remold Targets:	95.0 pcf (dry) at 15.0% moisture
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.6%
Height of Solids =	0.5641 in		0.5641 in	Trimming Procedure:	Specimen remolded in ring
Void Ratio =	0.762		0.702	Inundation:	<input checked="" type="checkbox"/> Not inundated <input type="checkbox"/> Inundated
Degree of Saturation =	51.1%		18.5%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.308 lb		0.282 lb	Apparatus:	Frame No. 1 (Wykeham Farrance 24251)
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	109.3 pcf		103.6 pcf	Final Differential Height:	-0.0158 in
Dry Unit Weight =	95.5 pcf		98.9 pcf	Estimated Preconsolidation Stress:	Not Computed

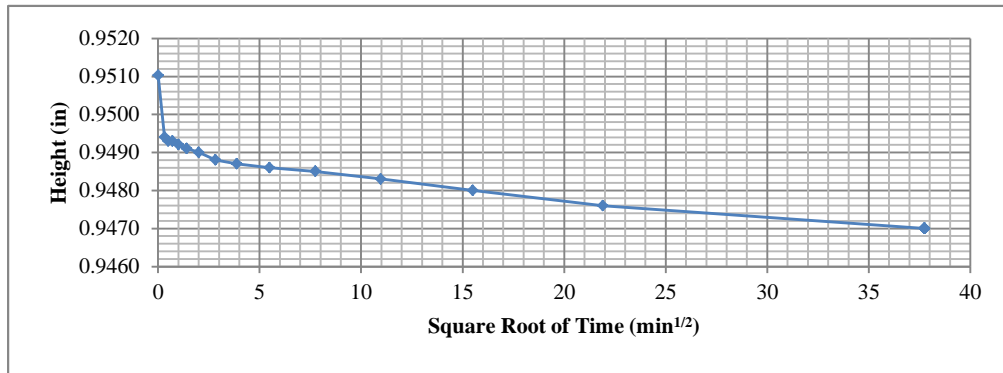
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-10 @ 19-33 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

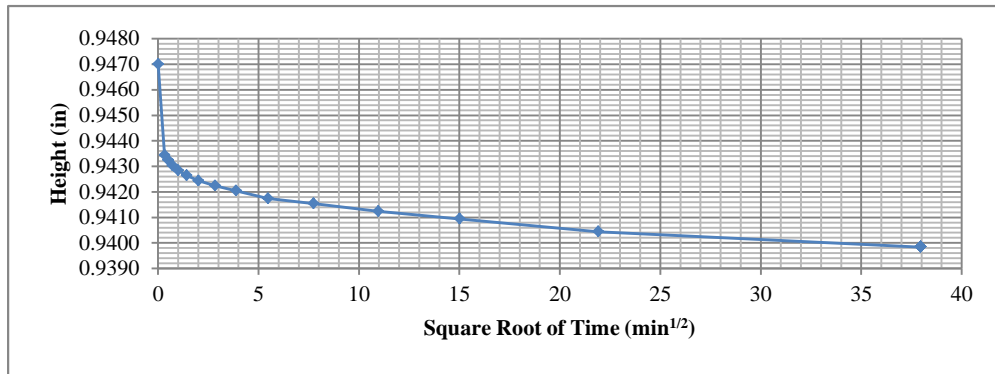


<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-10 @ 19-33 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

16.00 ksf



32.00 ksf



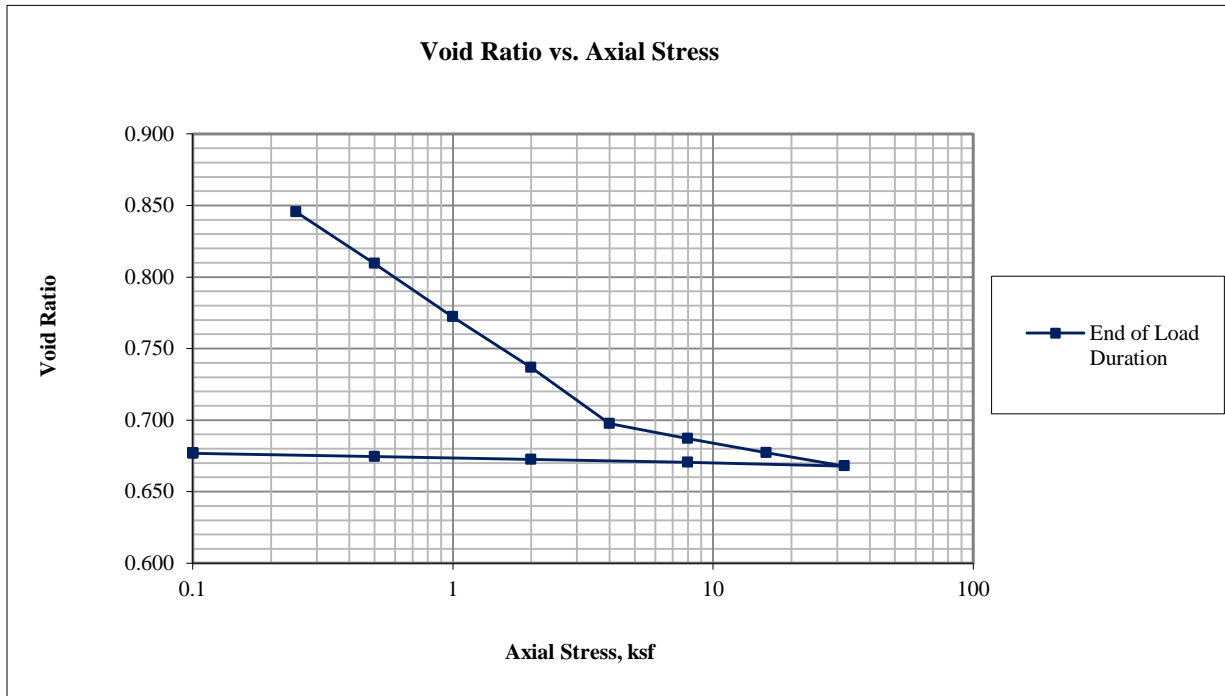
<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-10 @ 19-33 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>	



	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in <sup>2</sup>		4.893 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in <sup>3</sup>		4.428 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-12 @ 33.5-48.5 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1



<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-12 @ 33.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

	<b>Initial</b>		<b>Final</b>
Height =	0.993 in	0.887	in
Diameter =	2.500 in	2.500	in
Area =	4.909 in <sup>2</sup>	4.909	in <sup>2</sup>
Volume =	4.874 in <sup>3</sup>	4.354	in <sup>3</sup>
Water Content =	14.1%	1.7%	
Specific Gravity =	2.70 (Assumed)	2.70	(Assumed)
Height of Solids =	0.5636 in	0.5636	in
Void Ratio =	0.762	0.574	
Degree of Saturation =	49.8%	8.0%	
Wet Mass =	0.307 lb	0.274	lb
Dry Mass =	0.269 lb	0.269	lb
Wet Unit Weight =	108.9 pcf	108.7	pcf
Dry Unit Weight =	95.5 pcf	106.9	pcf

**Notes**

USCS description (ASTM D2487): Clayey sand with gravel, yellowish brown, dry

Atterberg Limits (ASTM D4318): LL = 23 PL = 14 PI = 9

Percent Finer (ASTM D422): 3/4 in. = 98% No. 4 = 83% No. 200 = 36%

Specimen Type:  Intact  Reconstituted

Remold Targets: 95.0 pcf (dry) at 15.0% moisture

Water Content of Trimmings (ASTM D2216): 14.7%

Trimming Procedure: Specimen remolded in ring

Inundation:  Not inundated  Inundated

Test Method:  A  B

Apparatus: Frame No. 1 (Wykeham Farrance 24251)

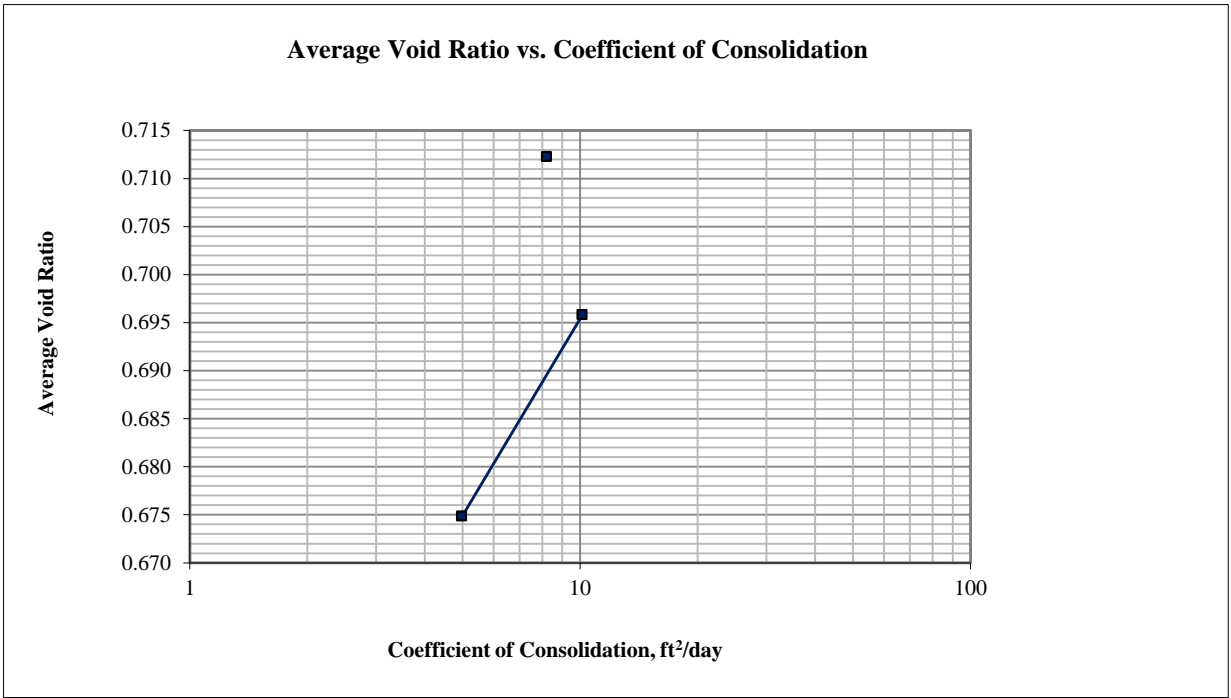
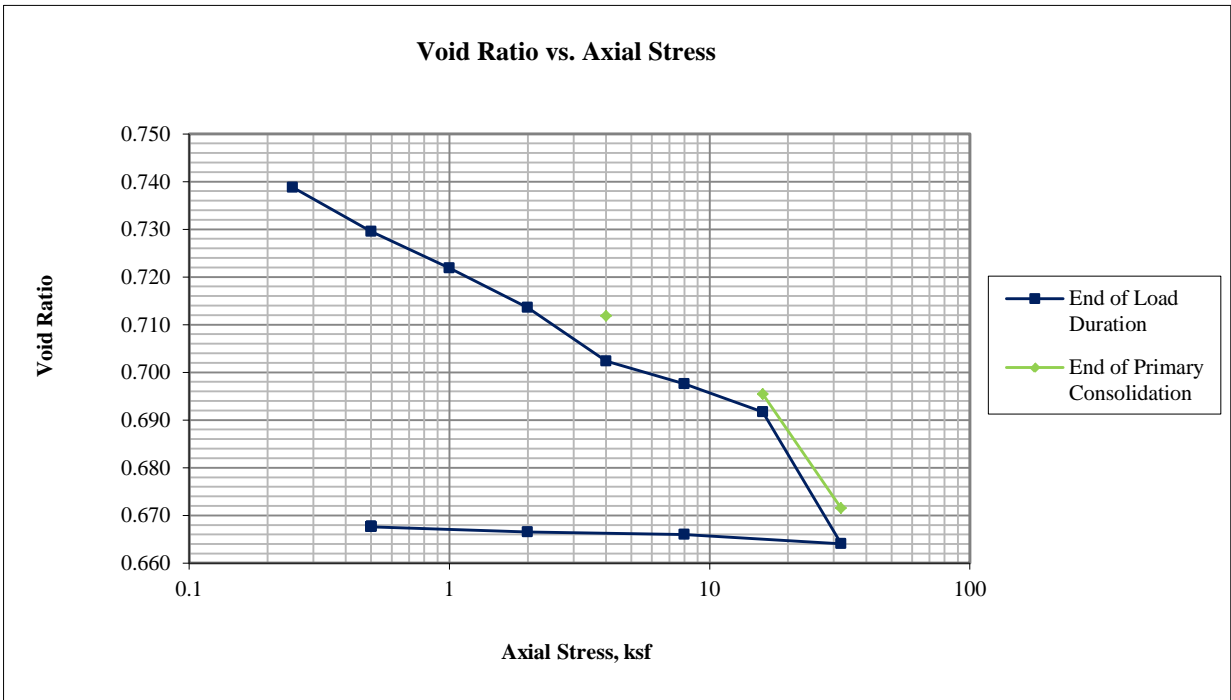
Final Water Content Specimen:  Entire  Partial

Final Differential Height: 0.0528 in

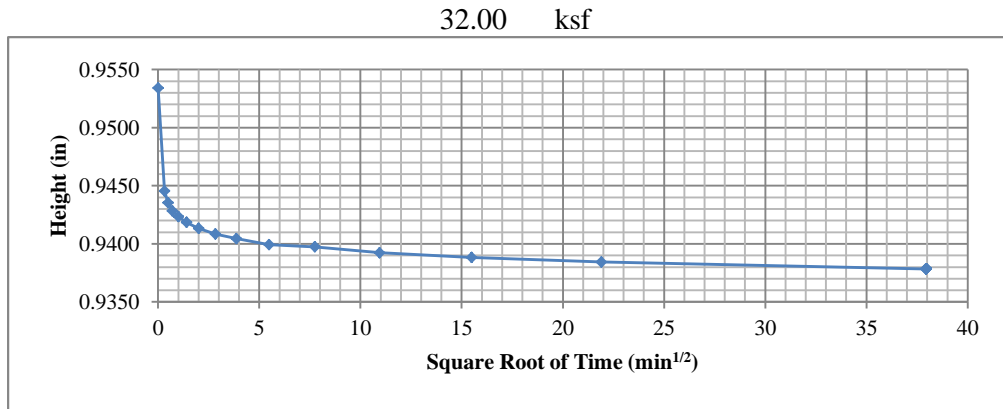
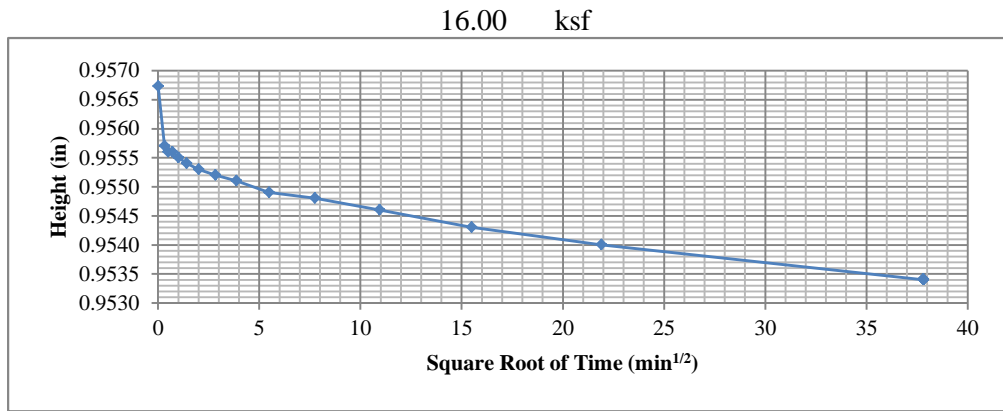
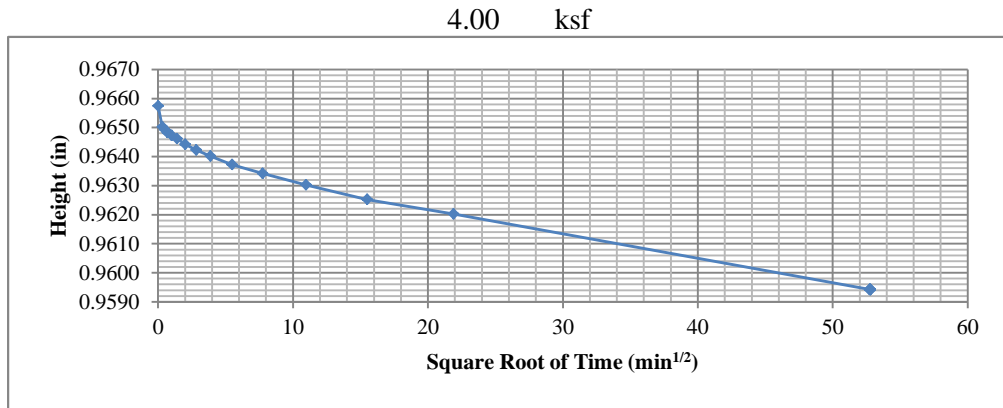
Estimated Preconsolidation Stress: 16.4 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 29-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-16 @ 29-34 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

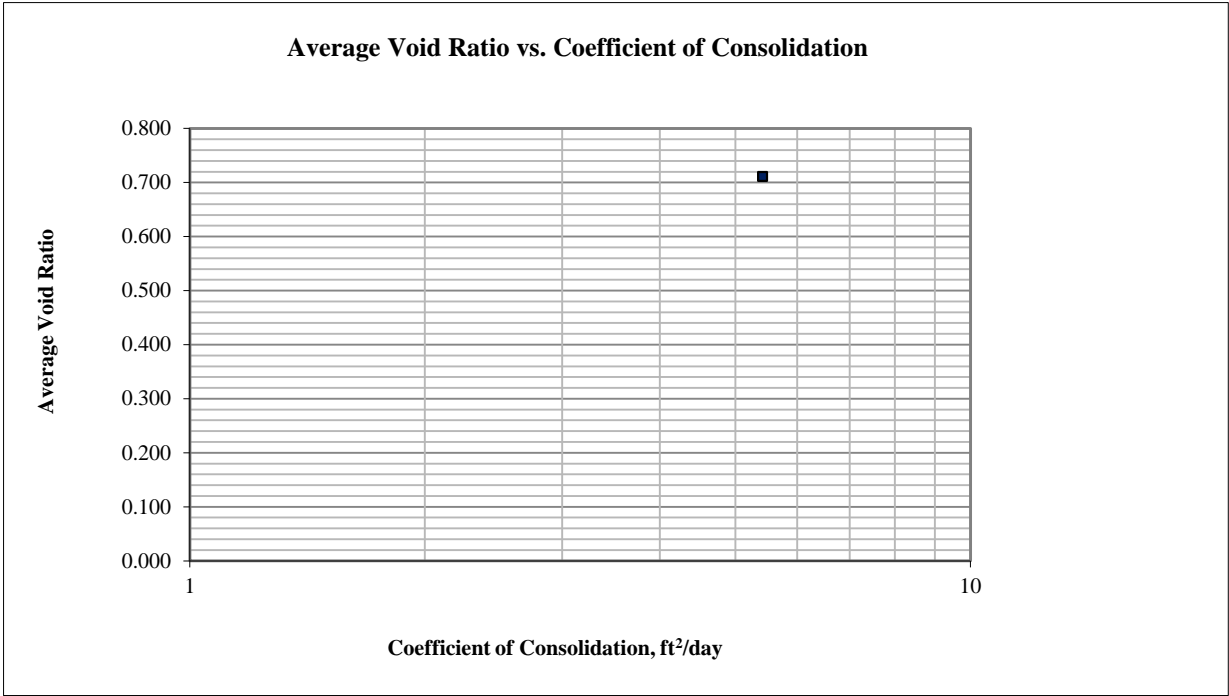
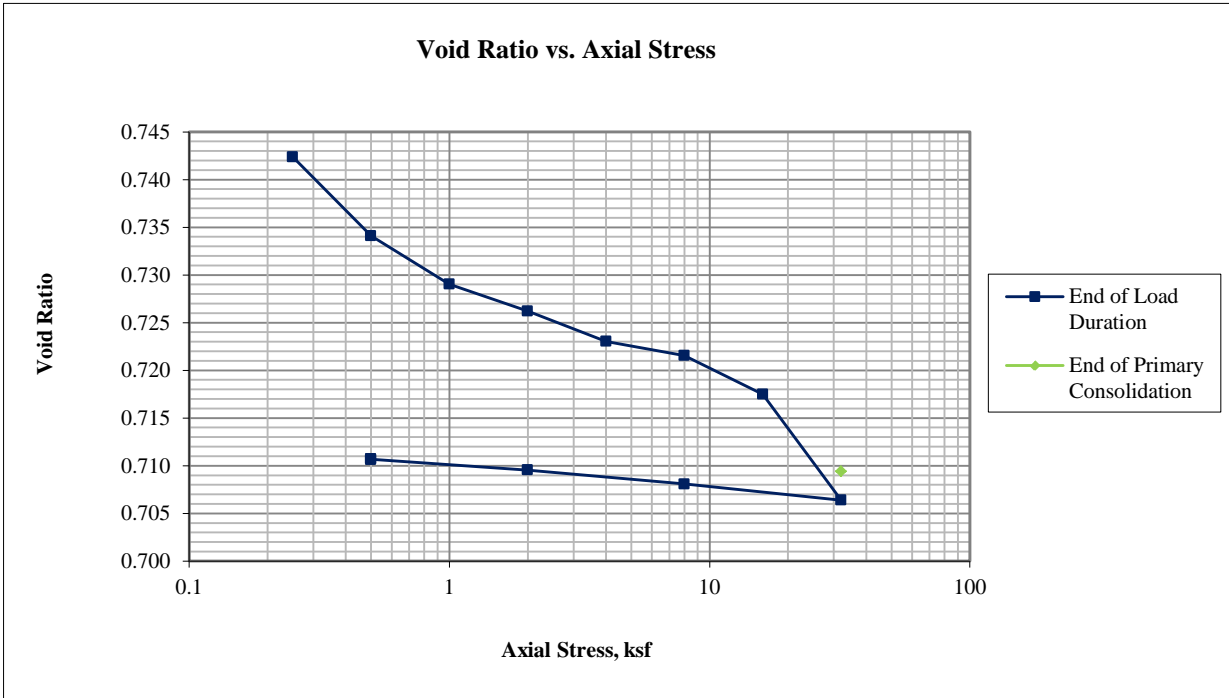


<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 29-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.997 in		0.960 in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 25 PL = 14 PI = 11
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 95% No. 200 = 52%
Volume =	4.886 in <sup>3</sup>		4.705 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	14.5%		2.7%	Remold Targets:	95.0 pcf (dry) at 15.0% moisture
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%
Height of Solids =	0.5638 in		0.5638 in	Trimming Procedure:	Specimen remolded in ring
Void Ratio =	0.768		0.703	Inundation:	<input checked="" type="checkbox"/> Not inundated <input type="checkbox"/> Inundated
Degree of Saturation =	51.1%		10.4%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.308 lb		0.276 lb	Apparatus:	Frame No. 4 (ELE C-320A)
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	109.0 pcf		101.5 pcf	Final Differential Height:	0.0045 in
Dry Unit Weight =	95.1 pcf		98.8 pcf	Estimated Preconsolidation Stress:	14.7 ksf

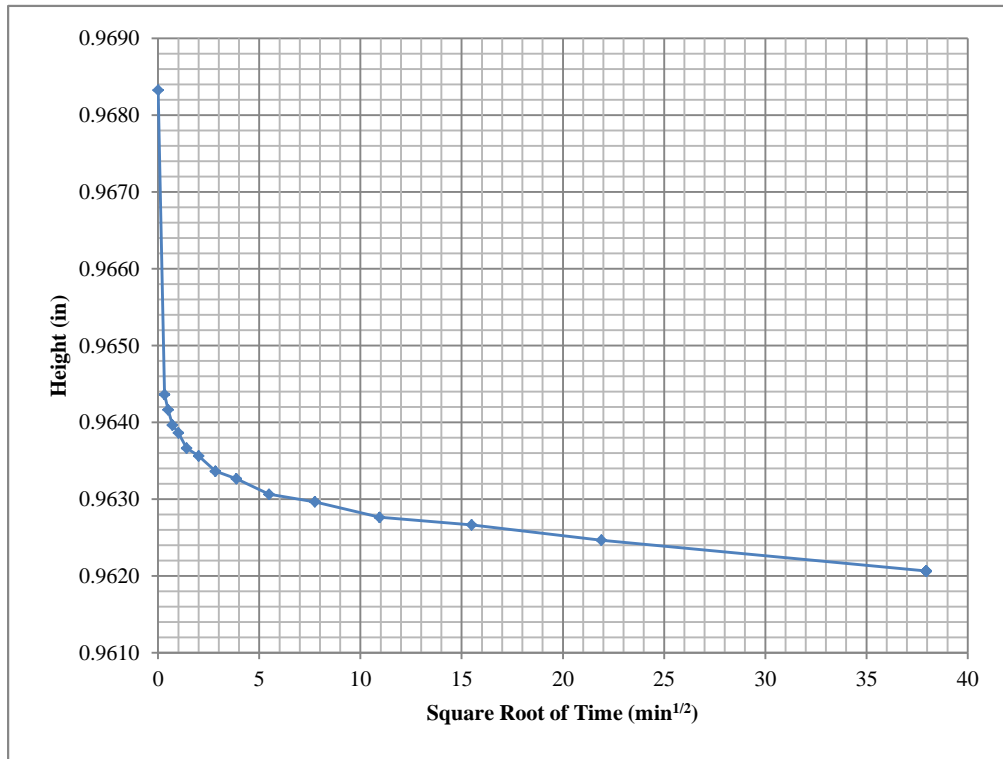
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-18 @ 23-33.5 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

32.00 ksf



<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-18 @ 23-33.5 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>	

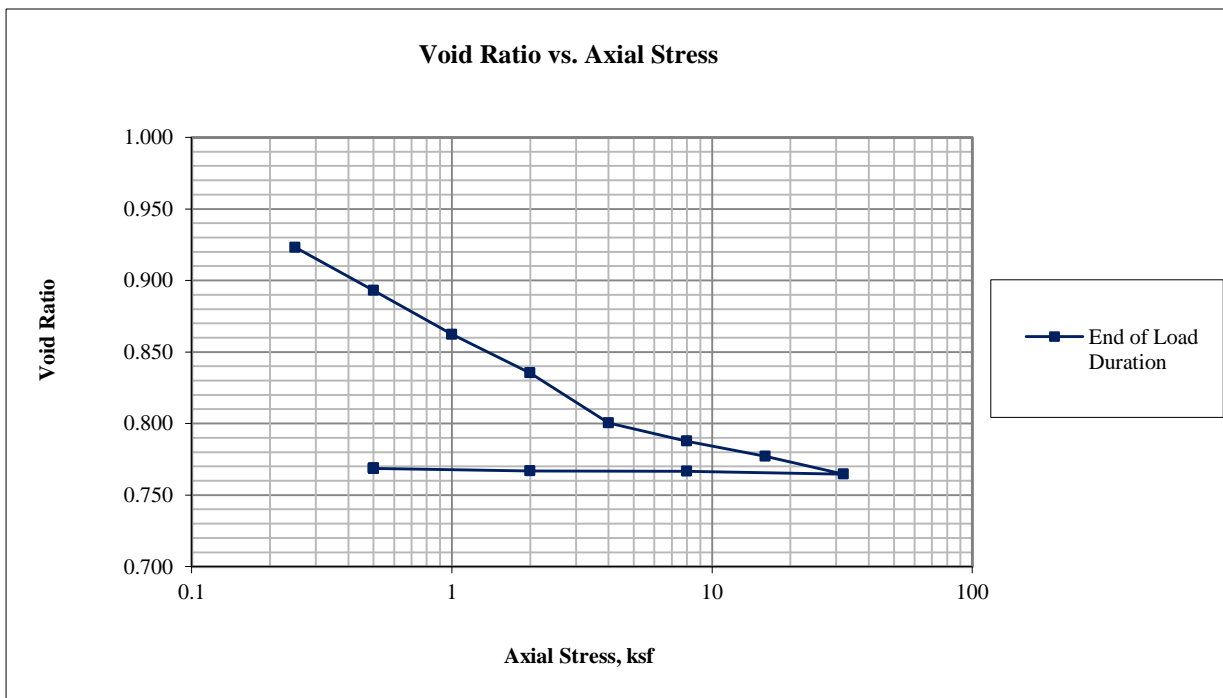


	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.994 in		0.924 in	USCS description (ASTM D2487):	Fat clay with sand, dark red, wet		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 62	PL = 20	PI = 42
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 82%
Volume =	4.871 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	28.9%		9.0%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	29.1%		
Height of Solids =	0.5085 in		0.5085 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.955		0.817	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	81.7%		29.7%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.243 lb		0.243 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.9 pcf		100.9 pcf	Final Differential Height:	-0.0246 in		
Dry Unit Weight =	86.1 pcf		92.6 pcf	Estimated Preconsolidation Stress:	-- ksf		

-- indicates test was not performed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 43.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

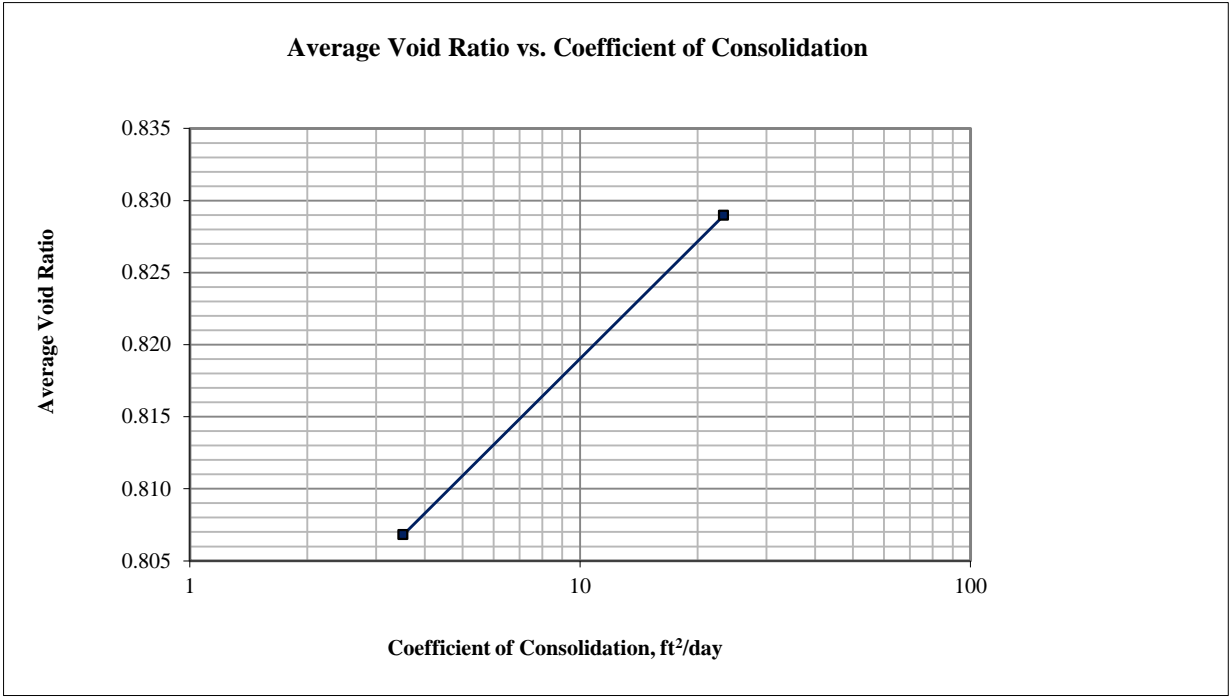
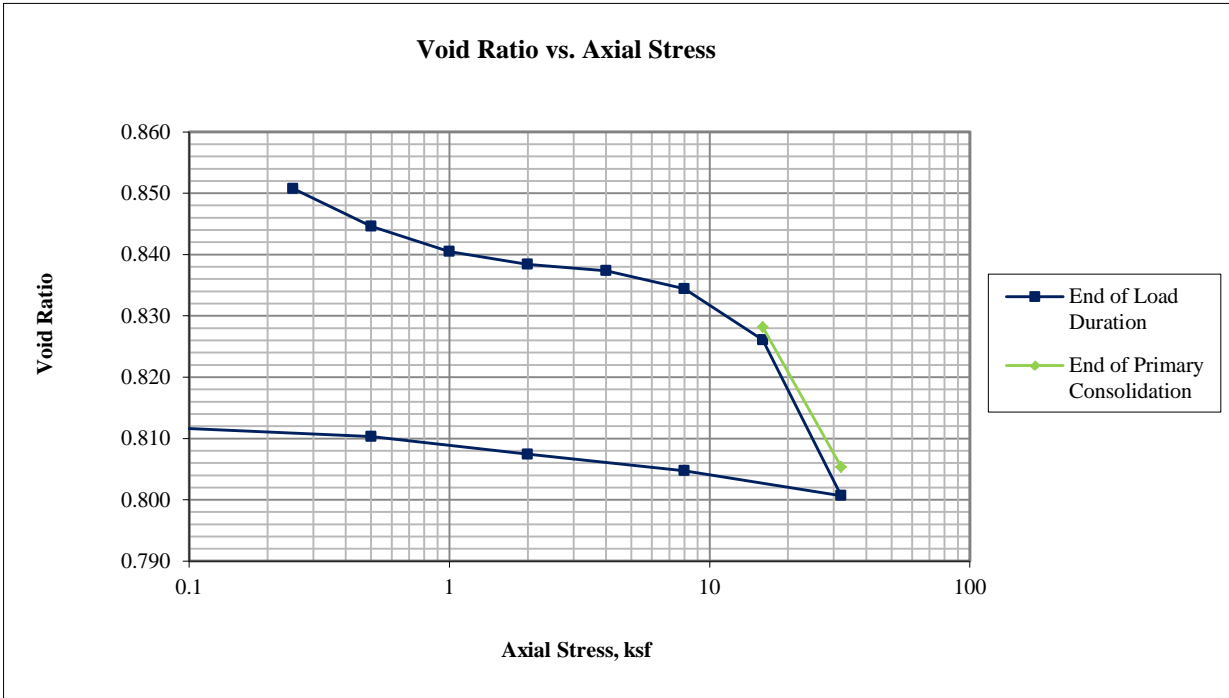


<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 43.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	1.000 in		0.981 in	USCS description (ASTM D2487):	Sandy silty clay, brownish yellow, moist		
Diameter =	2.497 in		2.497 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 21	PI = 4
Area =	4.897 in <sup>2</sup>		4.897 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 99%	No. 4 = 99%	No. 200 = 52%
Volume =	4.897 in <sup>3</sup>		4.804 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	9.7%		1.1%	Remold Targets:	90.0 pcf (dry) at	10.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.4%		
Height of Solids =	0.5363 in		0.5363 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.865		0.829	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	30.3%		3.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.281 lb		0.259 lb	Apparatus:	Frame No. 6	(ELE C-320A)	
Dry Mass =	0.256 lb		0.256 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	99.0 pcf		93.0 pcf	Final Differential Height:	-0.0094 in		
Dry Unit Weight =	90.2 pcf		92.0 pcf	Estimated Preconsolidation Stress:	13.0 ksf		

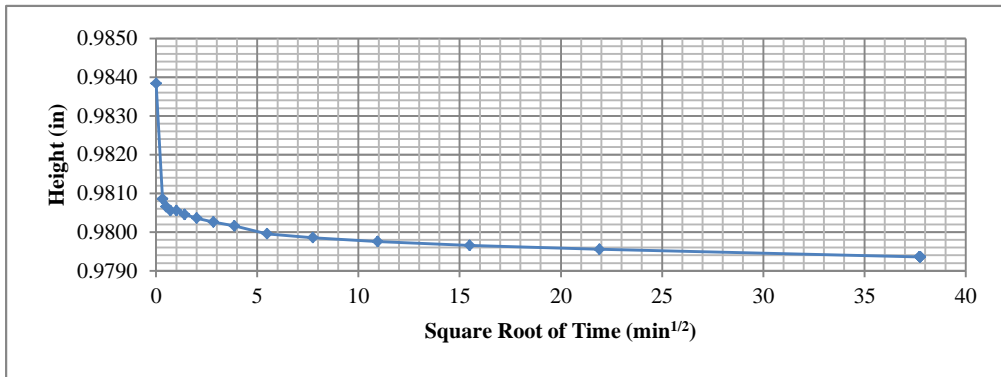
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>			<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study						
<b>Sample:</b>  BH-22 @ 0-8.5 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1	

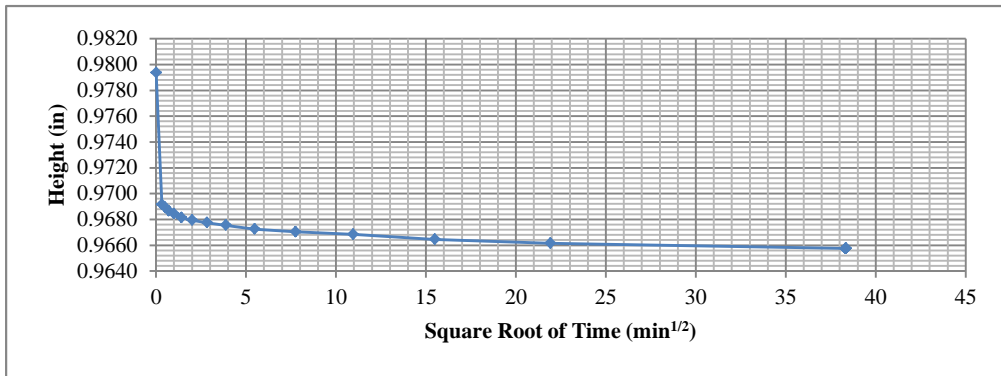


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-22 @ 0-8.5 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/11/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

16.00 ksf



32.00 ksf

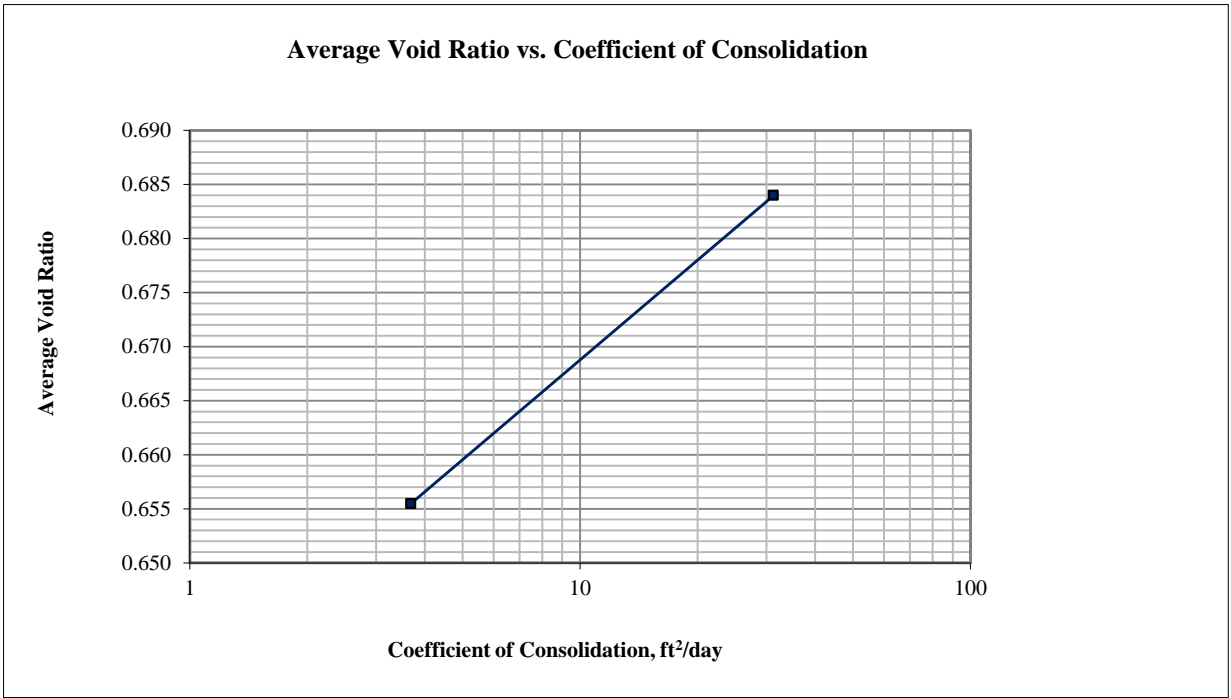
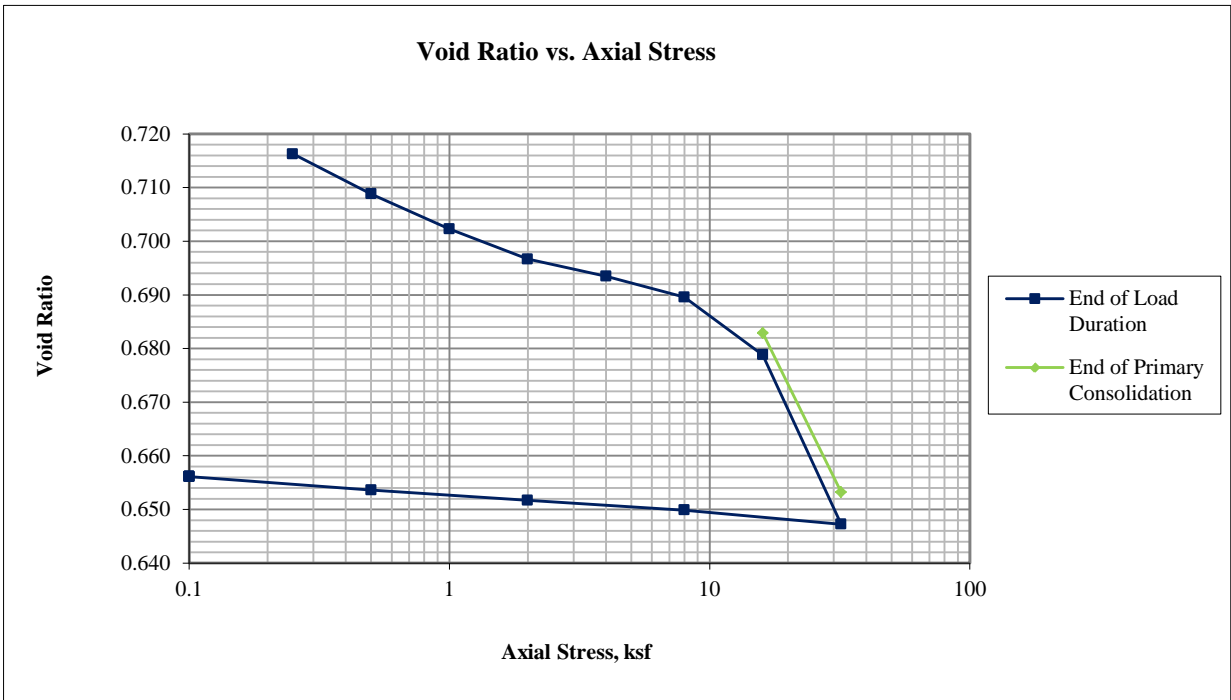


<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-22 @ 0-8.5 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>	

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36	PL = 18	PI = 18
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 82%	No. 200 = 37%
Volume =	4.886 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%		
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in		
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf		

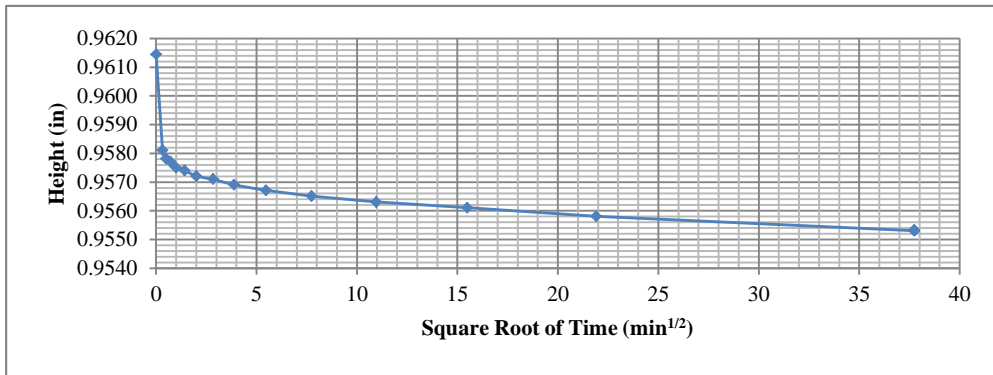
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-22 @ 28-30 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

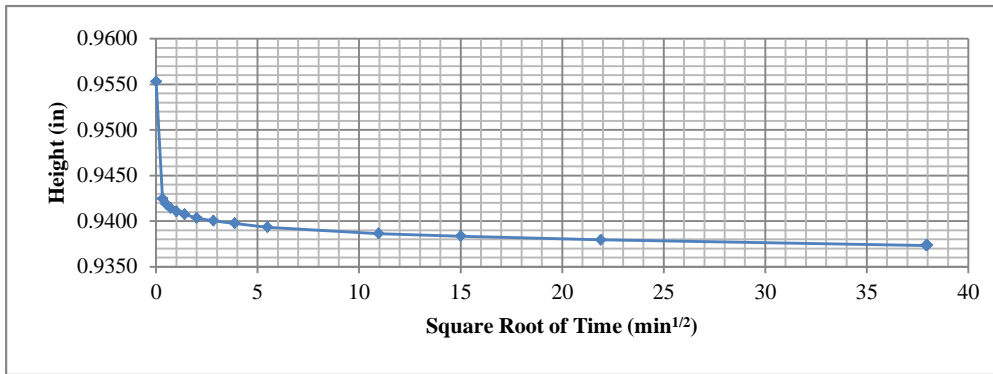


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-22 @ 28-30 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/11/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

16.00 ksf



32.00 ksf



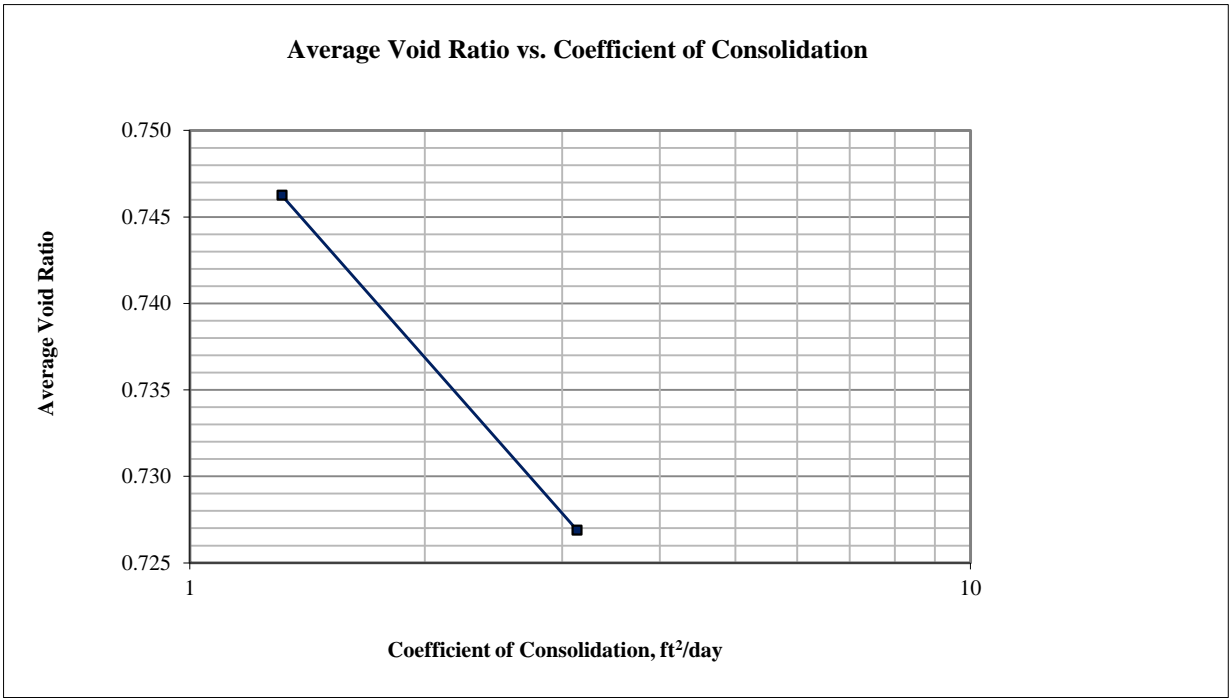
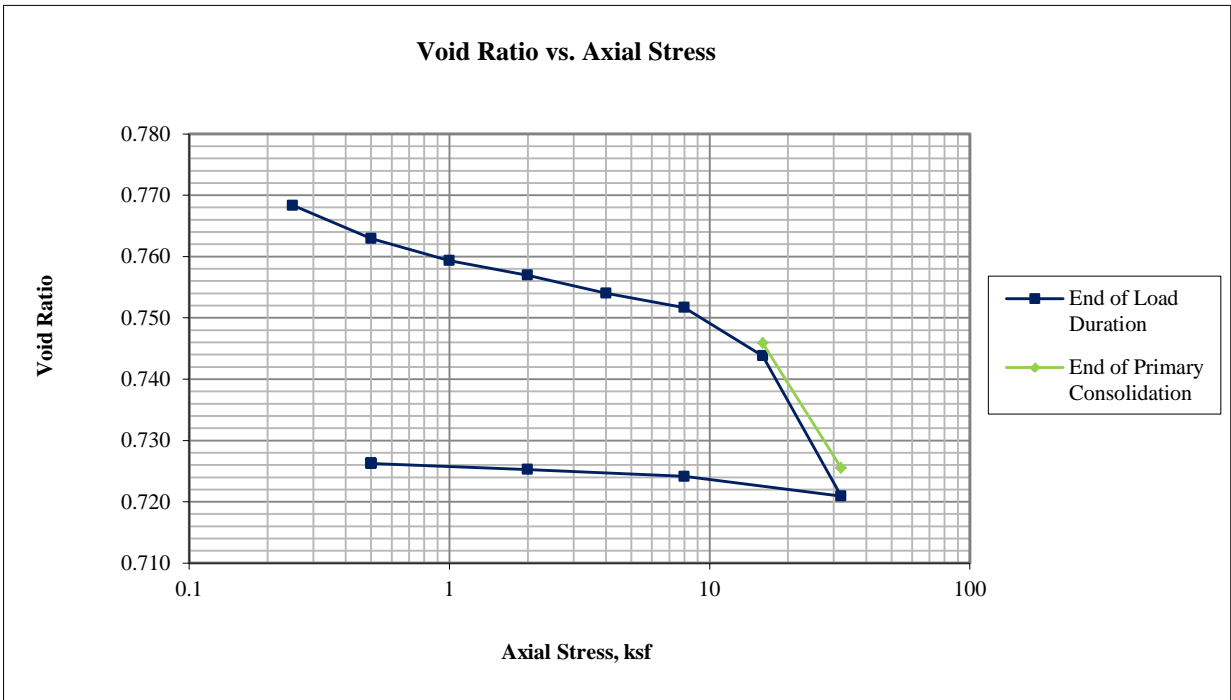
<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3	



	<b>Initial</b>		<b>Final</b>	<b>Notes</b>				
Height =	1.000 in		0.973 in	USCS description (ASTM D2487):	Clayey sand, reddish brown, moist			
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 37	PL = 17	PI = 20	
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 39%	
Volume =	4.901 in <sup>3</sup>		4.769 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted		
Water Content =	9.8%		4.2%	Remold Targets:	93.0 pcf (dry) at	10.0% moisture		
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.9%			
Height of Solids =	0.5558 in		0.5558 in	Trimming Procedure:	Specimen remolded in ring			
Void Ratio =	0.799		0.751	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated		
Degree of Saturation =	33.0%		15.1%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B		
Wet Mass =	0.291 lb		0.276 lb	Apparatus:	Frame No. 6	(ELE C-320A)		
Dry Mass =	0.265 lb		0.265 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial		
Wet Unit Weight =	102.7 pcf		100.1 pcf	Final Differential Height:	-0.0136 in			
Dry Unit Weight =	93.5 pcf		96.1 pcf	Estimated Preconsolidation Stress:	13.8 ksf			

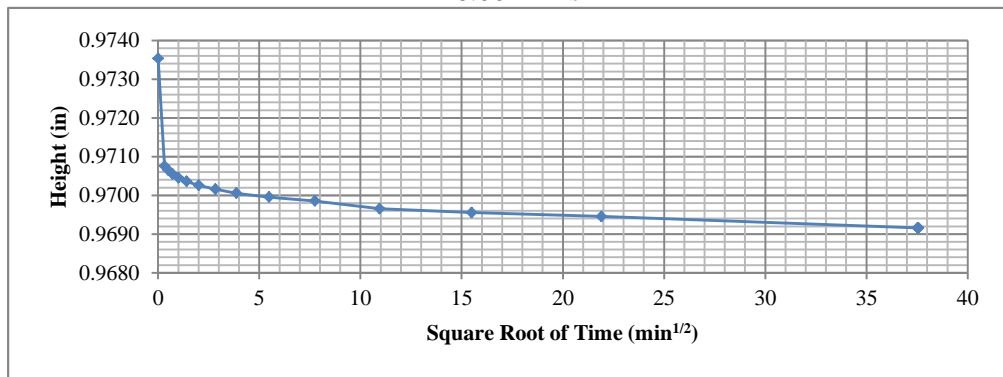
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 22-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

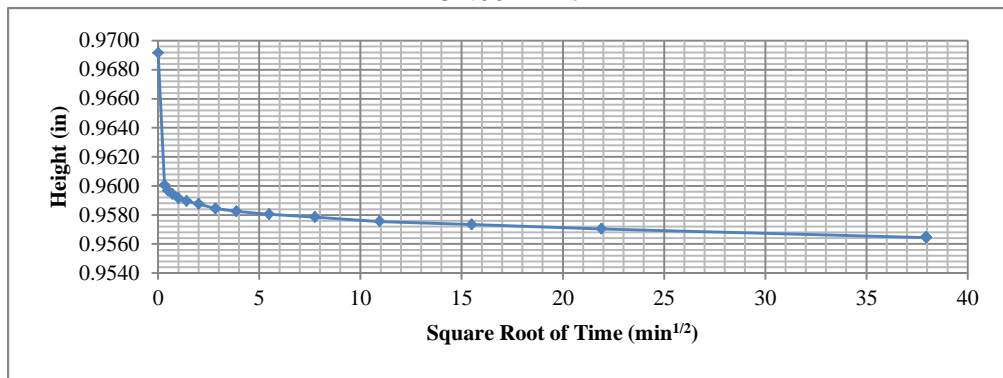


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-25 @ 22-34 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

16.00 ksf



32.00 ksf



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-25 @ 22-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3	

**APPENDIX B  
TAILINGS TEST RESULTS**

**APPENDIX B.1  
CYCLONE TEST RESULTS,  
FULL SCALE CYCLONE PERFORMANCE SIMULATION**

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1222 STPH; 55.5% -200 mesh

U/F = 18.2% -200 mesh; 45.2% recovery

Number, Model Krebs Cyclones: 15 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 13-14 PSI  
 Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1222.00	669.86	552.15
STPH Liquids	2977.31	2740.68	236.63
STPH Slurry	4199.31	3410.53	788.78
Wt Solids	29.10	19.64	70.00
S.G. Slurry	1.221	1.139	1.773
Vol% Solids	13.41	8.44	46.82
GPM Slurry	13734.13	11956.71	1777.42
M3/Hr. Slurry	3119.34	2715.64	403.69

Ref: 72.3 4.5 38.9\*

Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	22.0	0.00	0.00	0.0	3.98	3.98	22.0	100.0
100	149.0	18.50	16.70	204.1	0.23	0.23	1.6	40.66	36.68	202.5	99.2
150	104.0	30.60	12.10	147.9	2.69	2.46	16.5	64.46	23.79	131.4	88.9
200	74.0	44.50	13.90	169.9	13.74	11.05	74.0	81.81	17.36	95.8	56.4
270	53.0	56.40	11.90	145.4	29.20	15.46	103.5	89.40	7.59	41.9	28.8
325	45.0	61.10	4.70	57.4	35.93	6.73	45.0	91.64	2.24	12.4	21.6
400	37.0	63.70	2.60	31.8	39.89	3.97	26.6	92.58	0.94	5.2	16.4
-400	-37.0	100.00	36.30	443.6	100.00	60.11	402.6	100.00	7.42	40.9	9.2
<b>TOTAL</b>				<b>1222.00</b>			<b>669.86</b>			<b>552.15</b>	<b>45.2</b>

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1333 STPH; 55.5% -200 mesh

U/F = 18.4% -200 mesh; 45.6% recovery

Number, Model Krebs Cyclones: 16 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 14 PSI  
 Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1333.00	724.53	608.47
STPH Liquids	3247.76	2986.98	260.77
STPH Slurry	4580.76	3711.51	869.24
Wt Solids	29.10	19.52	70.00
S.G. Slurry	1.221	1.138	1.773
Vol% Solids	13.41	8.39	46.82
GPM Slurry	14981.66	13022.93	1958.73
M3/Hr. Slurry	3402.68	2957.81	444.87

Ref: 71.4 4.5 38.9\*

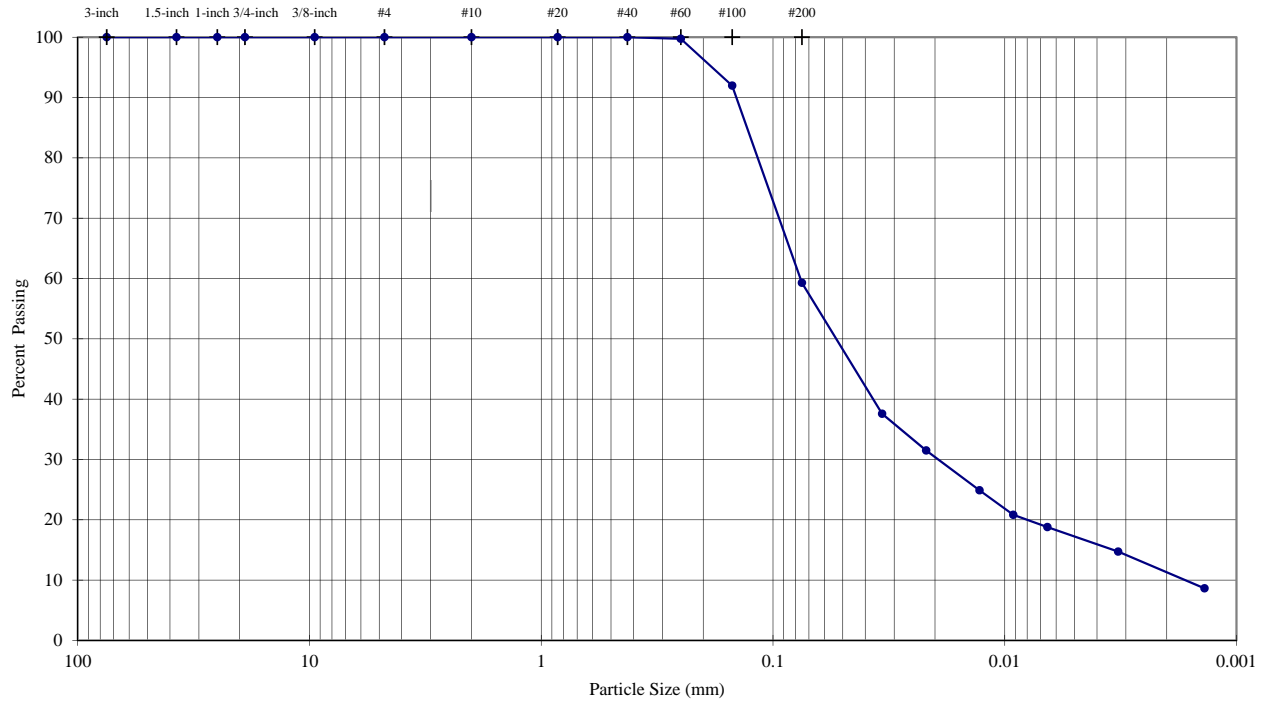
Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	24.0	0.00	0.00	0.0	3.94	3.94	24.0	100.0
100	149.0	18.50	16.70	222.6	0.21	0.21	1.5	40.28	36.34	221.1	99.3
150	104.0	30.60	12.10	161.3	2.52	2.31	16.7	64.04	23.76	144.6	89.6
200	74.0	44.50	13.90	185.3	13.31	10.79	78.2	81.64	17.60	107.1	57.8
270	53.0	56.40	11.90	158.6	28.73	15.42	111.7	89.35	7.71	46.9	29.6
325	45.0	61.10	4.70	62.7	35.47	6.74	48.8	91.62	2.27	13.8	22.1
400	37.0	63.70	2.60	34.7	39.45	3.98	28.9	92.58	0.95	5.8	16.7
-400	-37.0	100.00	36.30	483.9	100.00	60.55	438.7	100.00	7.42	45.2	9.3
<b>TOTAL</b>				<b>1333.00</b>			<b>724.53</b>			<b>608.47</b>	<b>45.6</b>

**APPENDIX B.2  
TAILINGS GRADATIONS**

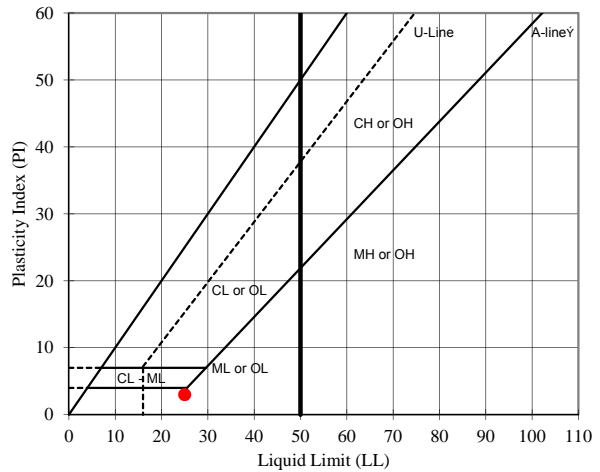


### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Drum** DEPTH (ft): **--**  
 TYPE: **Drum**



		Particle Size				
		Sieve	(mm)	% Passing		
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel	
		1.5-inch	37.5	100.0		Percentage
		1-inch	25.0	100.0		
		3/4-inch	19.0	100.0		
		3/8-inch	9.5	100.0	Fine Gravel	
		#4	4.75	100.0		
		#10	2.0	100.0	Coarse Sand	
		#20	0.85	100.0		
		#40	0.425	100.0	Medium Sand	
		#60	0.25	99.7		
Hydrometer Analysis		#100	0.15	92.0	Fine Sand	
		#200	0.075	59.3		
			0.034	37.6	Silt or Clay Fines	
			0.022	31.5		
			0.013	24.9		
			0.009	20.8		
			0.007	18.8		
		0.003	14.7			
		0.001	8.6			



USCS Description (ASTM D 2487):

Dry, yellow sandy silt

LL	PL	PI	SpG
25	22	3	2.64

As-Received Moisture Content (%)

--

USCS Group Symbol

ML

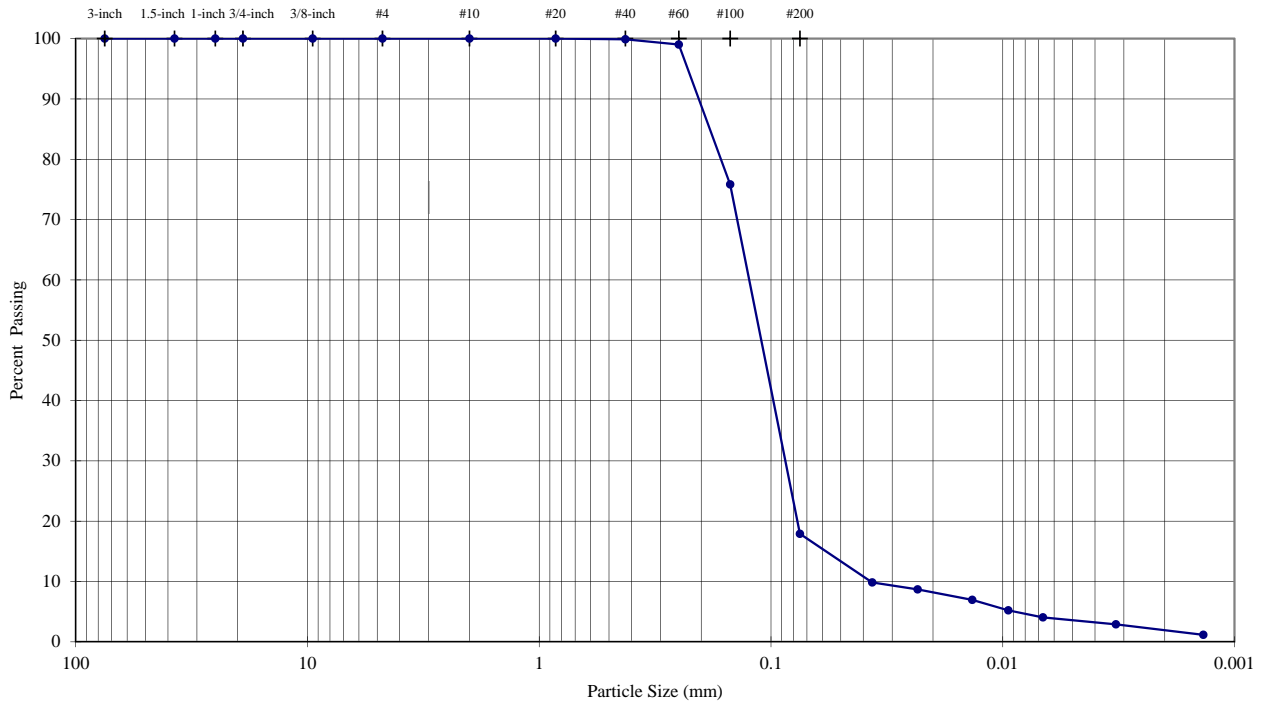
Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	11/13/2012
REVIEW	MB

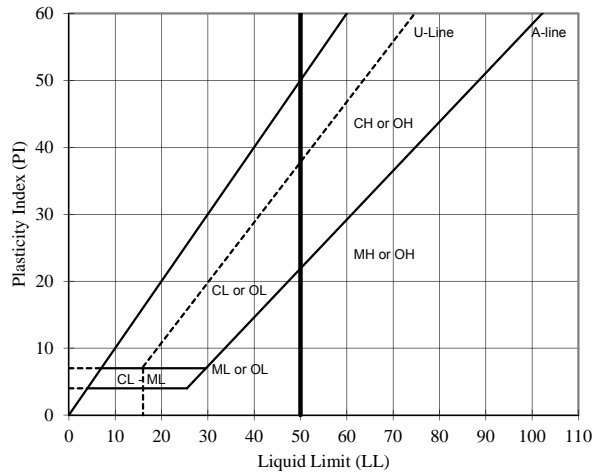
### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Underflow**  
 TYPE: **Pail**

DEPTH (ft): --



		Particle Size				
		Sieve	(mm)	% Passing		
Sieve Analysis (Initial Separation on No. 4 Sieve)	3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>0.00</b>	
	1.5-inch	37.5	<b>100.0</b>			
	1-inch	25.0	<b>100.0</b>			
	3/4-inch	19.0	<b>100.0</b>	Fine Gravel	<b>0.00</b>	
	3/8-inch	9.5	<b>100.0</b>			
	#4	4.75	<b>100.0</b>	Coarse Sand	<b>0.00</b>	
	#10	2.0	<b>100.0</b>			
	#20	0.85	<b>100.0</b>			
	Hydrometer Analysis	#40	0.425	<b>99.9</b>	Medium Sand	<b>0.12</b>
		#60	0.25	<b>99.0</b>		
#100		0.15	<b>75.8</b>	Fine Sand	<b>81.98</b>	
#200		0.075	<b>17.9</b>			
		0.037	<b>9.8</b>			
		0.023	<b>8.7</b>			
		0.014	<b>6.9</b>			
		0.009	<b>5.2</b>			
	0.007	<b>4.1</b>				
	0.003	<b>2.9</b>				
	0.001	<b>1.2</b>				
				Silt or Clay Fines	<b>17.91</b>	



USCS Description (ASTM D 2487):

Wet, light, yellowish brown silty sand

LL	PL	PI	Spg (assumed)
<b>NP</b>	<b>NP</b>	<b>NP</b>	<b>2.7</b>

As-Received Moisture Content (%)

--

USCS Group Symbol

SM

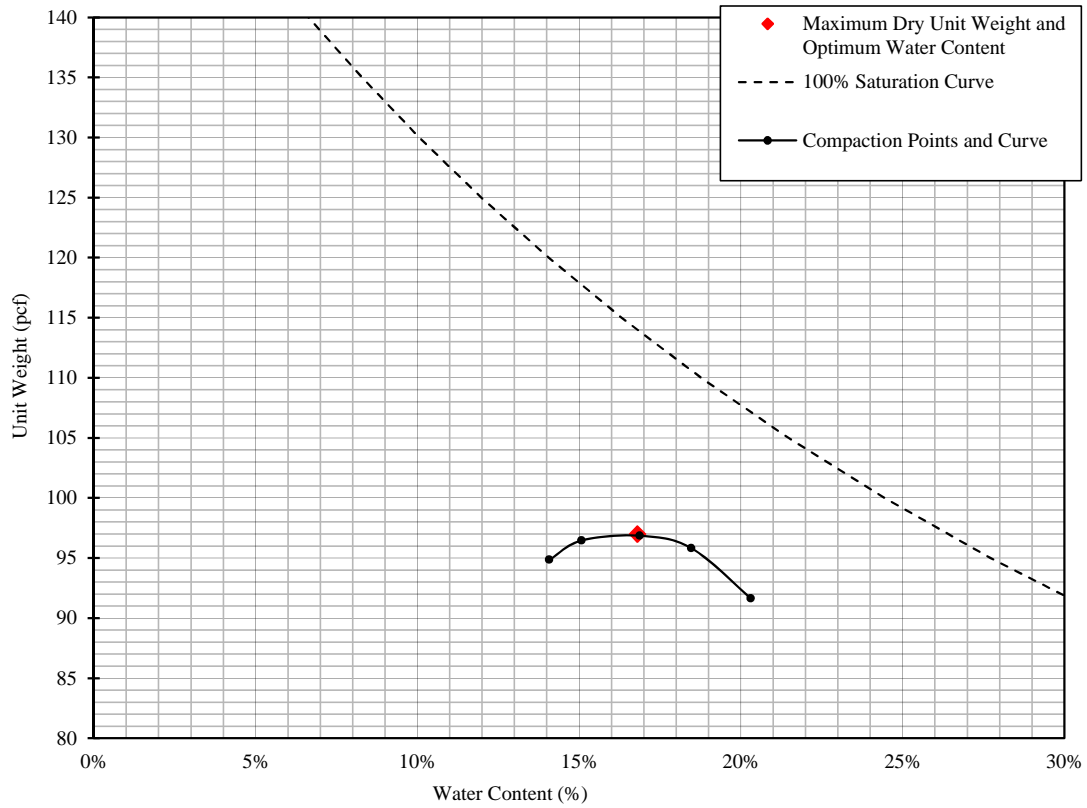
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	MC/SRS
DATE	10/24/2012
REVIEW	MB

# LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer    Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
SAMPLE ID: **Tailings Underflow**      DEPTH (ft): --  
TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>100%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM D854)	<b>2.64</b>

Maximum Dry Unit Weight (pcf)	<b>97.0</b>
Optimum Water Content (%)	<b>16.8</b>

USCS Description (ASTM D 2487): Wet, light, yellowish brown silty sand

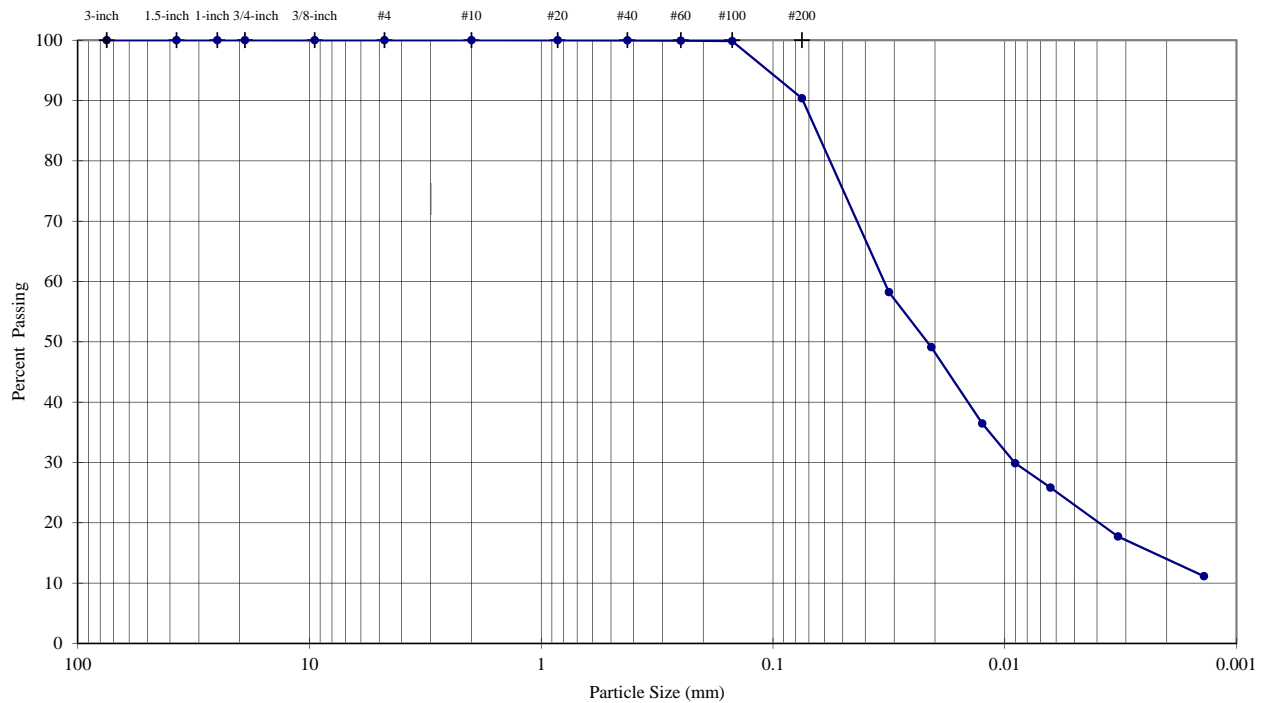
USCS SM

TECH	ACE/MC
DATE	10-25-12
REVIEW	MB

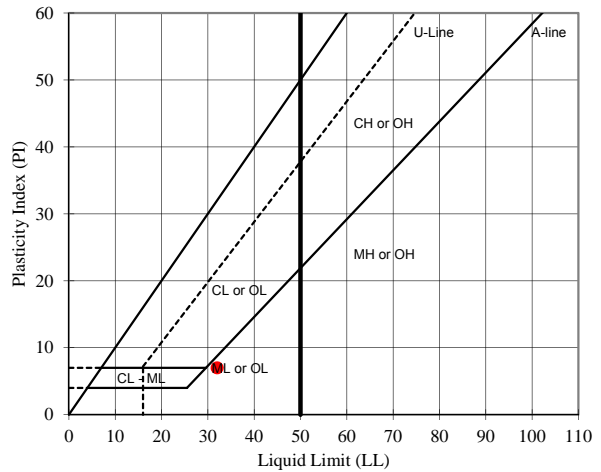
### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow**  
 TYPE: **Drum**

DEPTH (ft): --



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel <b>0.00</b>
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Fine Gravel <b>0.00</b>
		#10	2.0	100.0	
		#20	0.85	100.0	
		#40	0.425	100.0	
		#60	0.25	99.9	
Hydrometer Analysis		#100	0.15	99.9	Fine Sand <b>9.61</b>
		#200	0.075	90.4	
			0.032	58.2	
			0.021	49.1	
			0.013	36.5	
			0.009	29.9	Silt or Clay Fines <b>90.37</b>
			0.006	25.8	
			0.003	17.7	
			0.001	11.1	



USCS Description (ASTM D 2487):  
 Dry, olive yellow SILT

LL	PL	PI	SpG (assumed)
32	25	7	2.64

As-Received Moisture Content (%)  
--

USCS Group Symbol  
ML

Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	1/2/2013
REVIEW	MB

**APPENDIX B.3  
CYCLONE UNDERFLOW TEST RESULTS**

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.526 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.311 lb  
 Moisture Content = 16.4%  
 Wet Unit Weight = 107.3 pcf  
 Dry Unit Weight = 92.2 pcf  
 Void Ratio = 0.79  
 Percent Saturation = 55%

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.523 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.312 lb  
 Moisture Content = 16.1%  
 Wet Unit Weight = 107.1 pcf  
 Dry Unit Weight = 92.3 pcf  
 Void Ratio = 0.78  
 Percent Saturation = 54%

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.517 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.306 lb  
 Moisture Content = 16.1%  
 Wet Unit Weight = 106.7 pcf  
 Dry Unit Weight = 91.9 pcf  
 Void Ratio = 0.79  
 Percent Saturation = 54%

**After Consolidation**  
 Length = 4.989 in  
 Diameter = 2.470 in  
 Area = 4.792 in<sup>2</sup> (Method B)  
 Volume = 23.906 in<sup>3</sup>  
 Moisture Content = 27.9%  
 Wet Unit Weight = 121.2 pcf  
 Dry Unit Weight = 94.7 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.977 in  
 Diameter = 2.469 in  
 Area = 4.787 in<sup>2</sup> (Method B)  
 Volume = 23.824 in<sup>3</sup>  
 Moisture Content = 27.6%  
 Wet Unit Weight = 121.4 pcf  
 Dry Unit Weight = 95.2 pcf  
 Void Ratio = 0.73  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.971 in  
 Diameter = 2.429 in  
 Area = 4.633 in<sup>2</sup> (Method B)  
 Volume = 23.033 in<sup>3</sup>  
 Moisture Content = 25.7%  
 Wet Unit Weight = 123.2 pcf  
 Dry Unit Weight = 98.0 pcf  
 Void Ratio = 0.68  
 Percent Saturation = 100%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.97  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.082% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

Cell Pressure = 120 psi  
 Back Pressure = 100 psi  
 Confining Pressure = 20 psi

Cell Pressure = 150 psi  
 Back Pressure = 100 psi  
 Confining Pressure = 50 psi

Cell Pressure = 170 psi  
 Back Pressure = 80 psi  
 Confining Pressure = 90 psi

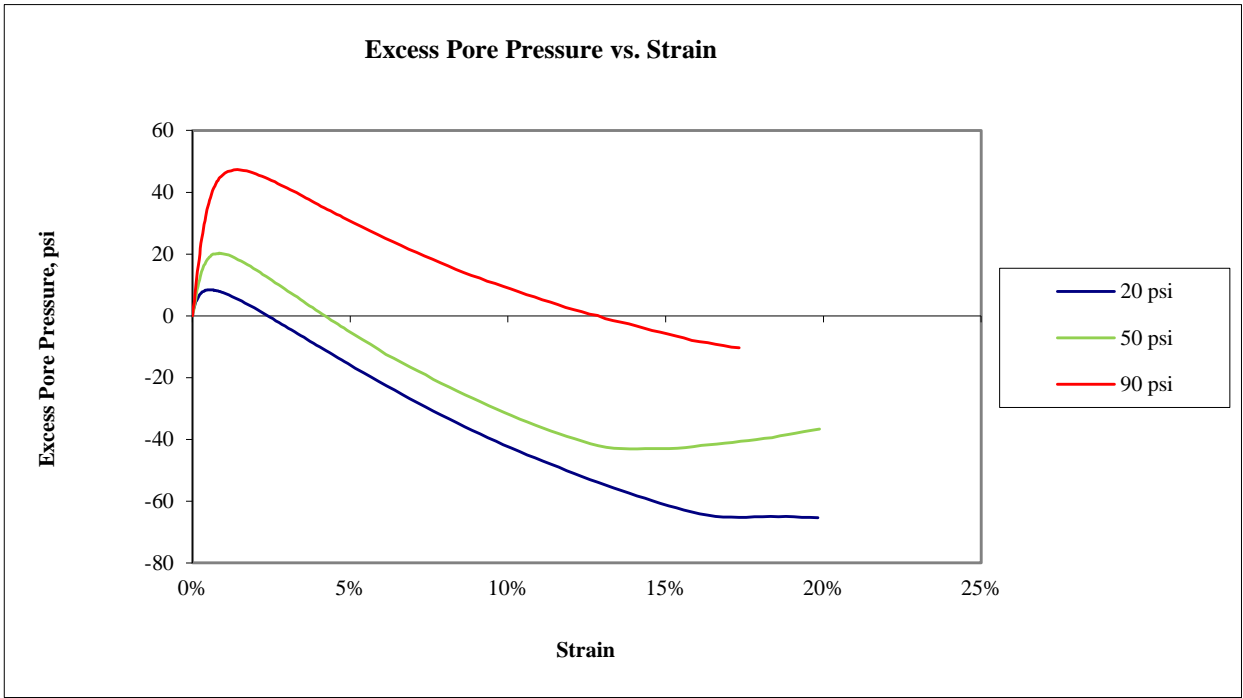
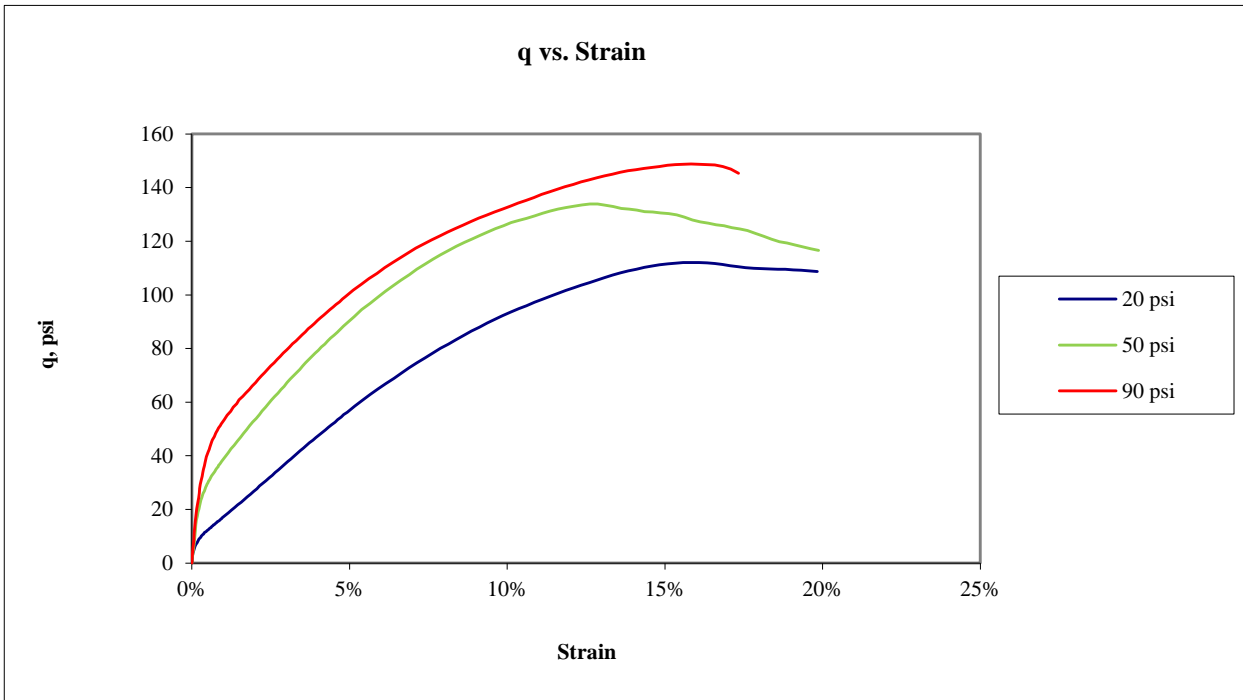
Notes: Sample description: Light yellowish brown silty sand

Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 18% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

Intact	X
Cuttings	X
Wet	
(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	
Corrected	

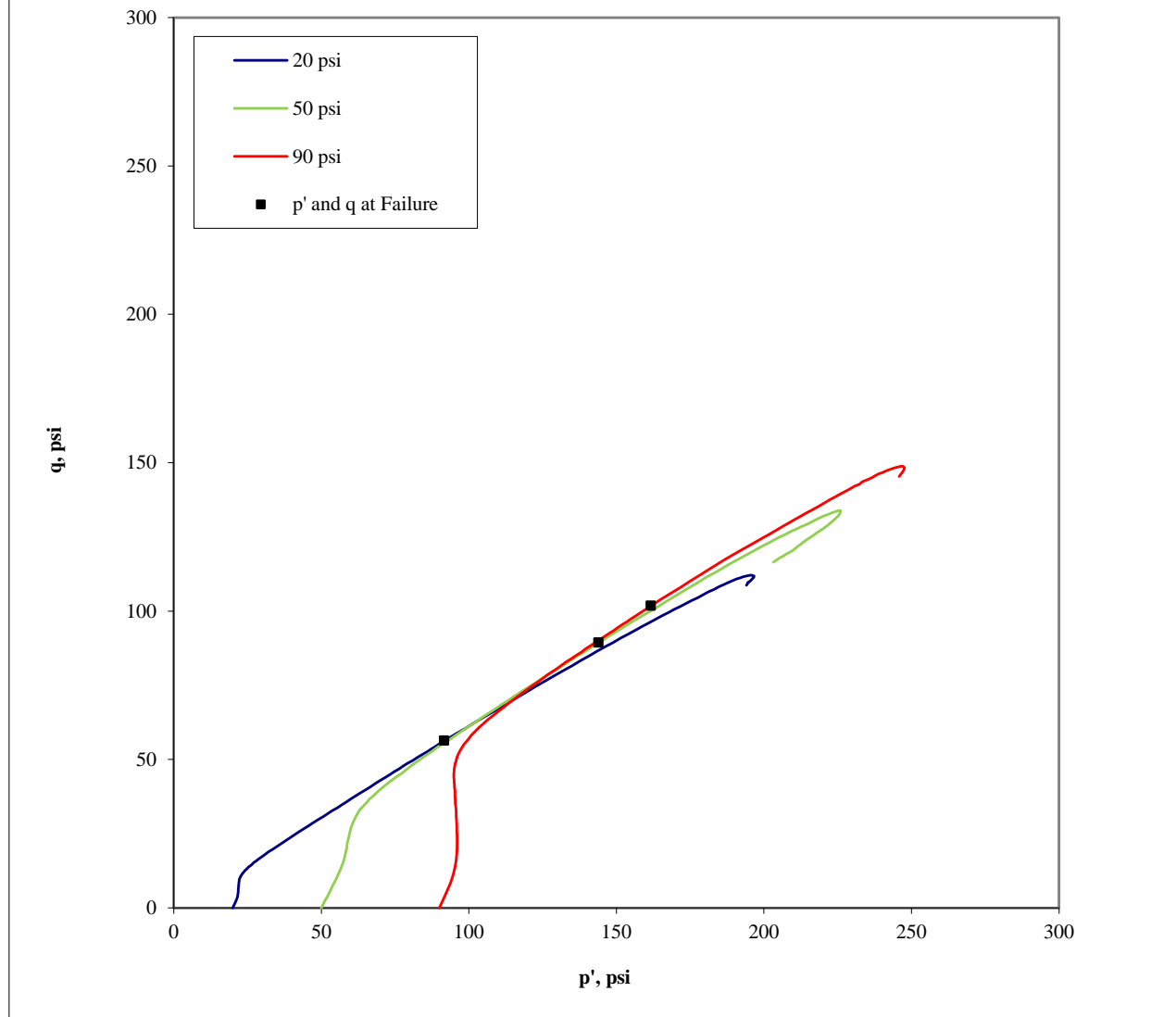
 Reconstituted Remold targets: 92.2 pcf (dry) at 16.8% moisture (+/- 2%)  
 Moisture from: Entire specimen  
 Saturation method: X Wet Dry  
 Failure criterion: (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub> 5 % strain  
 Membrane effect: X Corrected Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 2

Stress Path (p'-q) Plot

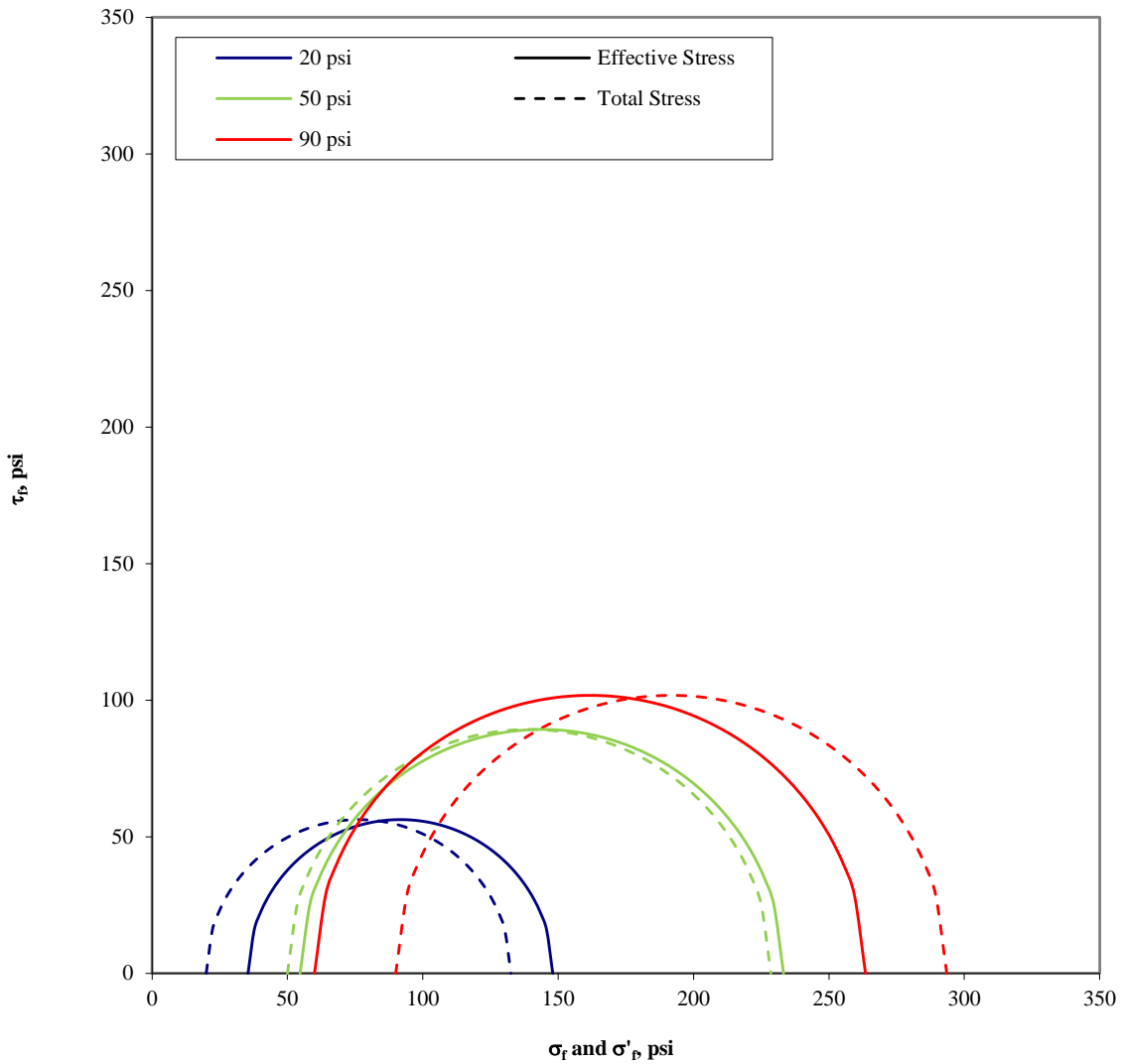


Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	76.3	91.7	56.3
50	139.3	143.9	89.3
90	191.8	161.7	101.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>Tailing Underflow</b></p>	<p>Technician:  <b>RJM/PRH</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>1/8/2013</b></p>	<p>Job Number:  <b>103-92557</b></p>	<p>Figure:  <b>3</b></p>



### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
20	148.0	35.4	132.6	20.0
50	233.2	54.7	228.6	50.0
90	263.5	60.0	293.5	90.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM</b>			
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>Tailing Underflow</b>	Technician: <b>RJM/PRH</b>	Reviewed: <b>CCS</b>	Date: <b>1/8/2013</b>	Job Number: <b>103-92557</b>	Figure: <b>4</b>



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
<b>Sample:</b>	Tailing Underflow	<b>Technician:</b>	RJM/PRH	<b>Reviewed:</b>	CCS
		<b>Date:</b>	1/8/2013	<b>Job Number:</b>	103-92557
				<b>Figure:</b>	5



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
		<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013
					<b>Figure:</b> 6

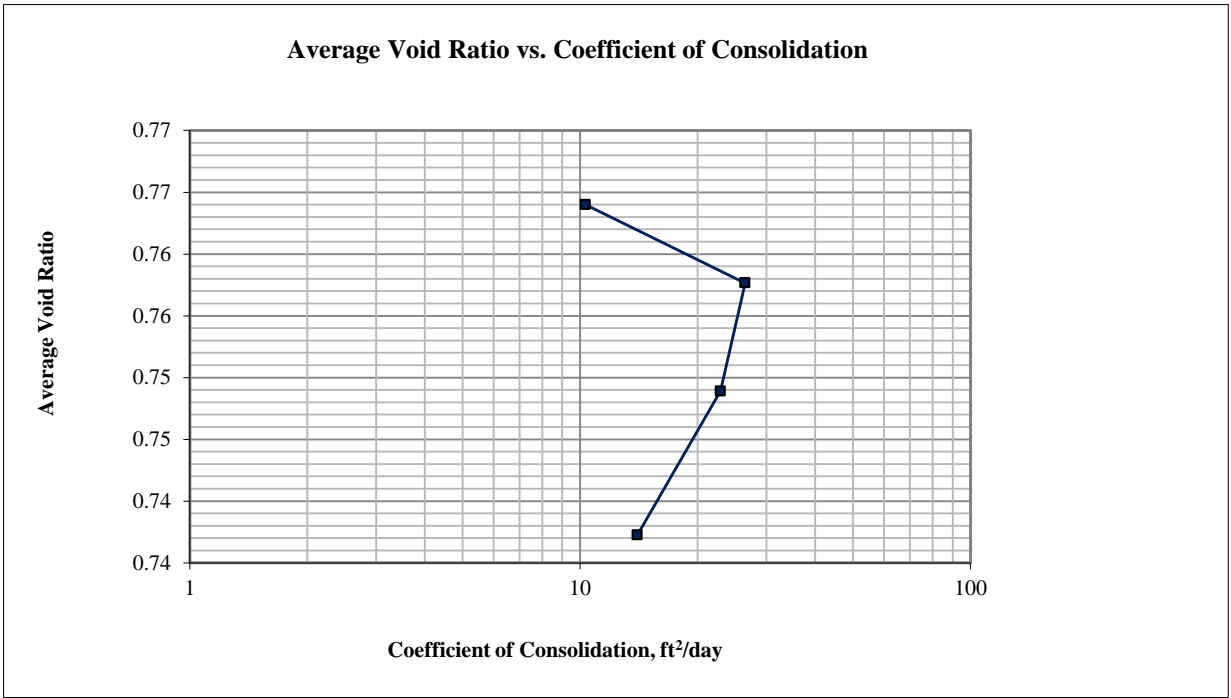
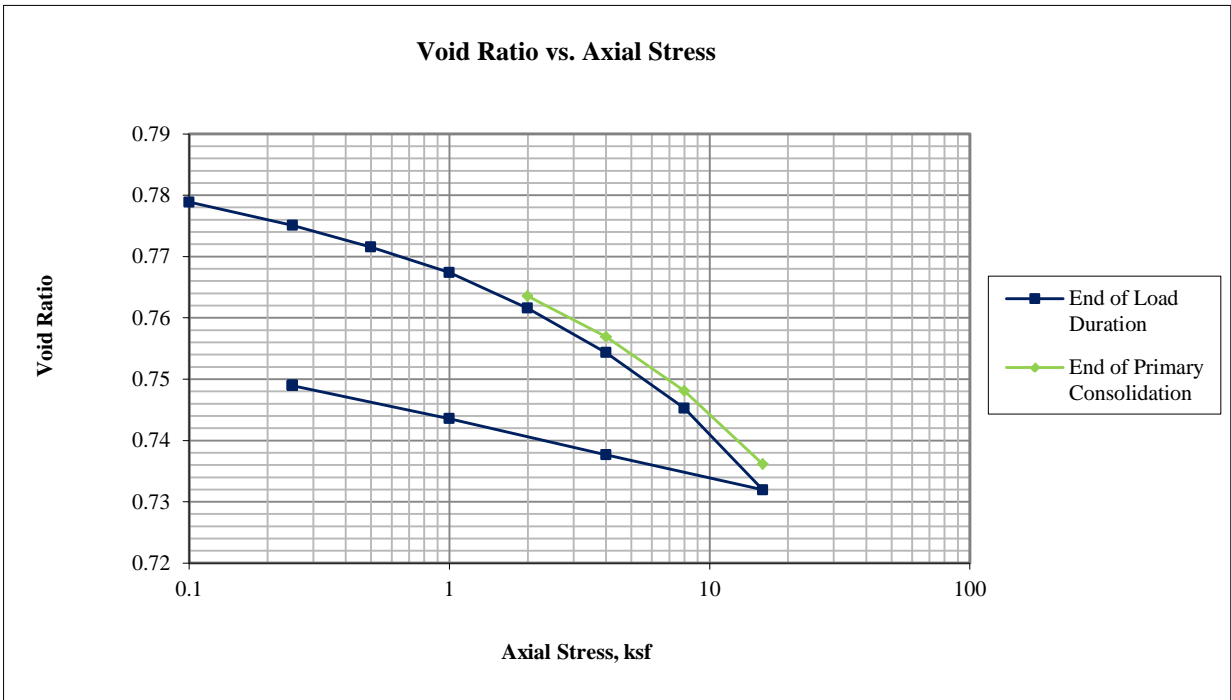


<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
		<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013
				<b>Figure:</b> 7	

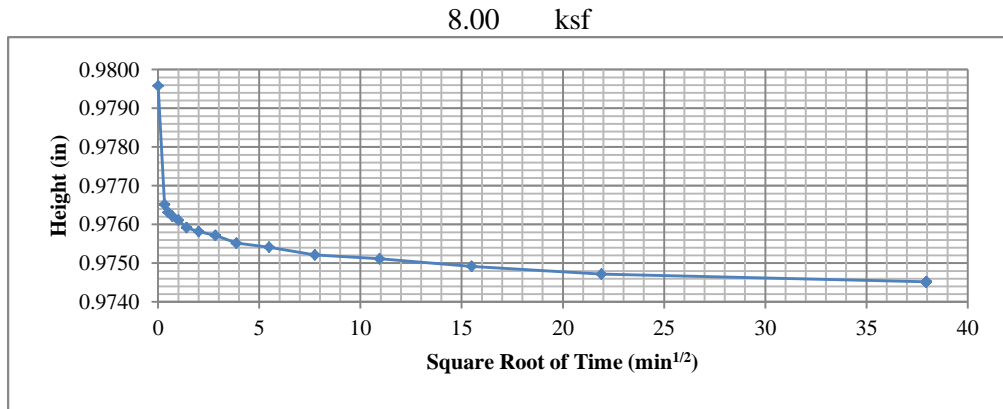
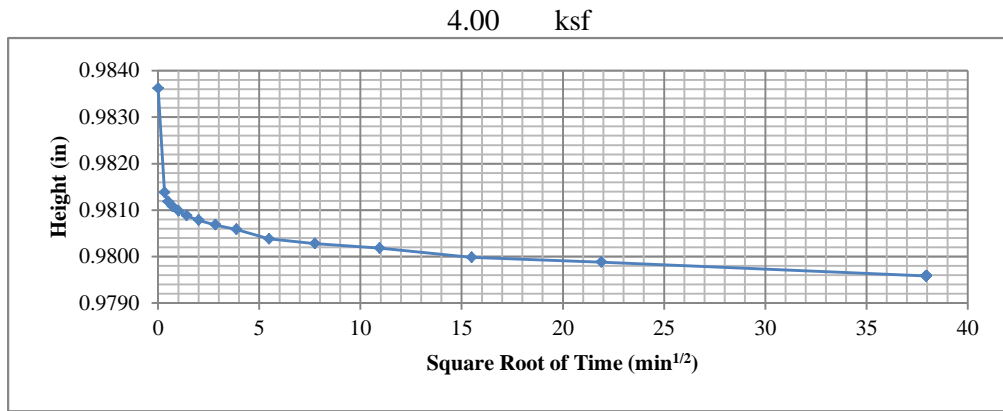
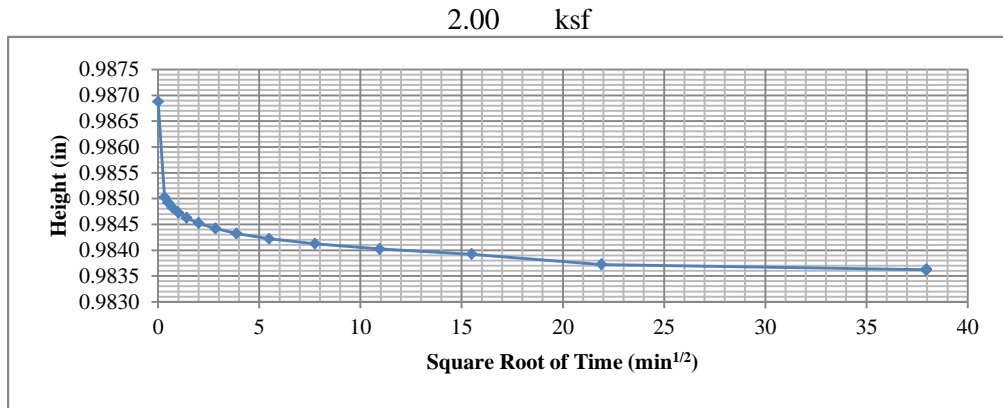
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.994 in		0.973 in	Visual description (Golder procedure):	Damp, yellowish brown SILTY SAND
Diameter =	2.501 in		2.501 in	Atterberg Limits (ASTM D4318):	LL = NP PL = NP PI = NP
Area =	4.913 in <sup>2</sup>		4.913 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 100% No. 200 = 18%
Volume =	4.883 in <sup>3</sup>		4.780 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	16.5%		25.0%	Remold Targets:	92.2 pcf (dry) at 16.8% moisture (+/- 2.0%)
Specific Gravity =	2.64 (Assumed)		2.64 (Assumed)	Water Content of Trimmings (ASTM D2216):	16.6%
Height of Solids =	0.5584 in		0.5584 in	Trimming Procedure:	Specimen trimmed in ring
Void Ratio =	0.780		0.743	Inundation:	<input type="checkbox"/> Not inundated <input checked="" type="checkbox"/> Inundated at 0.1 ksf
Degree of Saturation =	55.8%		88.9%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.304 lb		0.326 lb	Apparatus:	Frame No. 2 (Wykeham Farrance 24251)
Dry Mass =	0.261 lb		0.261 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	107.6 pcf		118.0 pcf	Final Differential Height:	0.0036 in
Dry Unit Weight =	92.4 pcf		94.4 pcf	Estimated Preconsolidation Stress:	3.7 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	160					0.0000	0.9937	0.00	0.780				
1	0.10	1000					0.0004	0.9933	0.04	0.779				
2	0.25	1500					0.0025	0.9912	0.25	0.775				
3	0.50	1440					0.0045	0.9892	0.45	0.772				
4	1.00	1440					0.0068	0.9869	0.68	0.767				
5	2.00	1440	0.0090	0.9847	0.90	0.764	0.0101	0.9836	1.01	0.762	2 (Root time)	0.764	10.328	0.3
6	4.00	1440	0.0127	0.9810	1.27	0.757	0.0141	0.9796	1.42	0.754	2 (Root time)	0.758	26.471	0.3
7	8.00	1440	0.0176	0.9761	1.77	0.748	0.0192	0.9745	1.93	0.745	2 (Root time)	0.749	22.898	0.3
8	16.00	1440	0.0243	0.9694	2.44	0.736	0.0266	0.9671	2.68	0.732	2 (Root time)	0.737	14.013	0.4
9	4.00	150					0.0234	0.9703	2.35	0.738				
10	1.00	870					0.0201	0.9736	2.02	0.744				
11	0.25	1145					0.0171	0.9766	1.72	0.749				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  Tailings Underflow	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  11/12/2012	<b>Job Number:</b>  103-92557	<b>Figure:</b>  1

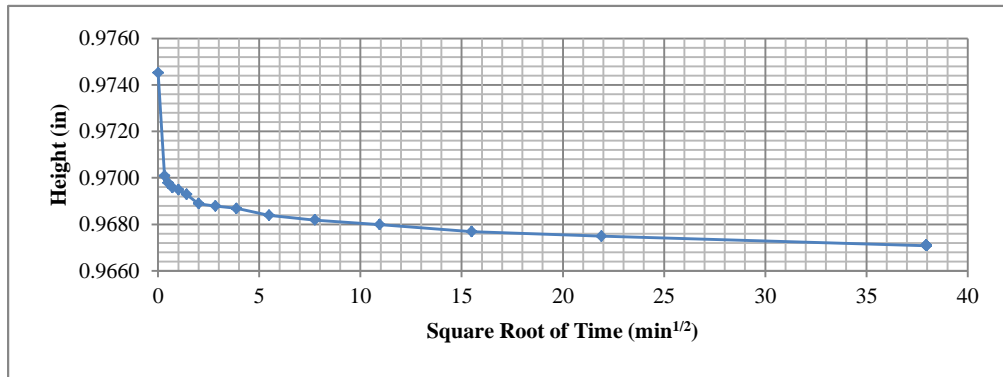


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailings Underflow	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 2



<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS (1)				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailings Underflow	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3

16.00 ksf



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS (2)			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailings Underflow	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 4



**APPENDIX B.4  
CYCLONE OVERFLOW AND WHOLE TAILINGS  
TEST RESULTS**

**APPENDIX B.4.1  
CYCLONE OVERFLOW TEST REPORTS**

Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 4.183 in  
 Diameter = 1.924 in  
 Wet Mass = 0.851 lb  
 Area = 2.907 in<sup>2</sup>  
 Volume = 12.162 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.666 lb  
 Moisture Content = 27.7%  
 Wet Unit Weight = 120.9 pcf  
 Dry Unit Weight = 94.6 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 99%

**Initial**  
 Length = 4.127 in  
 Diameter = 1.930 in  
 Wet Mass = 0.832 lb  
 Area = 2.926 in<sup>2</sup>  
 Volume = 12.074 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.653 lb  
 Moisture Content = 27.4%  
 Wet Unit Weight = 119.1 pcf  
 Dry Unit Weight = 93.5 pcf  
 Void Ratio = 0.76  
 Percent Saturation = 95%

**Initial**  
 Length = 4.257 in  
 Diameter = 1.929 in  
 Wet Mass = 0.868 lb  
 Area = 2.922 in<sup>2</sup>  
 Volume = 12.441 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.682 lb  
 Moisture Content = 27.3%  
 Wet Unit Weight = 120.5 pcf  
 Dry Unit Weight = 94.7 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 98%

**After Consolidation**  
 Length = 4.091 in  
 Diameter = 1.894 in  
 Area = 2.816 in<sup>2</sup> (Method B)  
 Volume = 11.522 in<sup>3</sup>  
 Moisture Content = 24.5%  
 Wet Unit Weight = 124.4 pcf  
 Dry Unit Weight = 99.9 pcf  
 Void Ratio = 0.65  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.080 in  
 Diameter = 1.866 in  
 Area = 2.734 in<sup>2</sup> (Method B)  
 Volume = 11.156 in<sup>3</sup>  
 Moisture Content = 23.7%  
 Wet Unit Weight = 125.2 pcf  
 Dry Unit Weight = 101.2 pcf  
 Void Ratio = 0.63  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.224 in  
 Diameter = 1.849 in  
 Area = 2.685 in<sup>2</sup> (Method B)  
 Volume = 11.339 in<sup>3</sup>  
 Moisture Content = 22.1%  
 Wet Unit Weight = 126.9 pcf  
 Dry Unit Weight = 103.9 pcf  
 Void Ratio = 0.58  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.051% /min.  
 t<sub>50</sub> = 7.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.99  
 Shear Rate = 0.084% /min.  
 t<sub>50</sub> = 3.7 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 4.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 60 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 20 psi

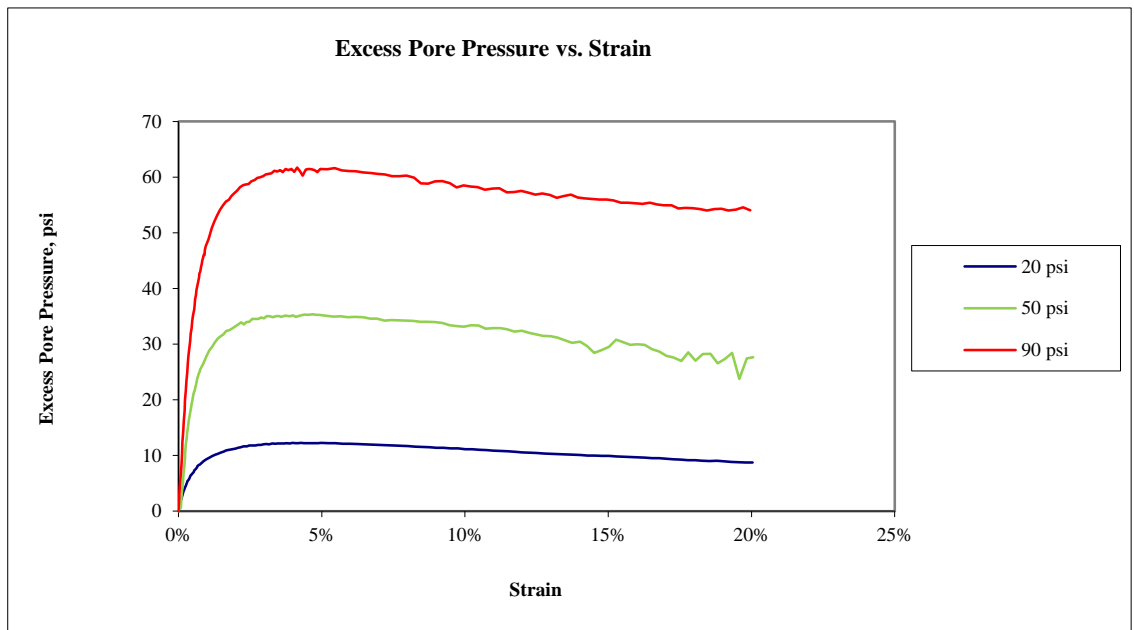
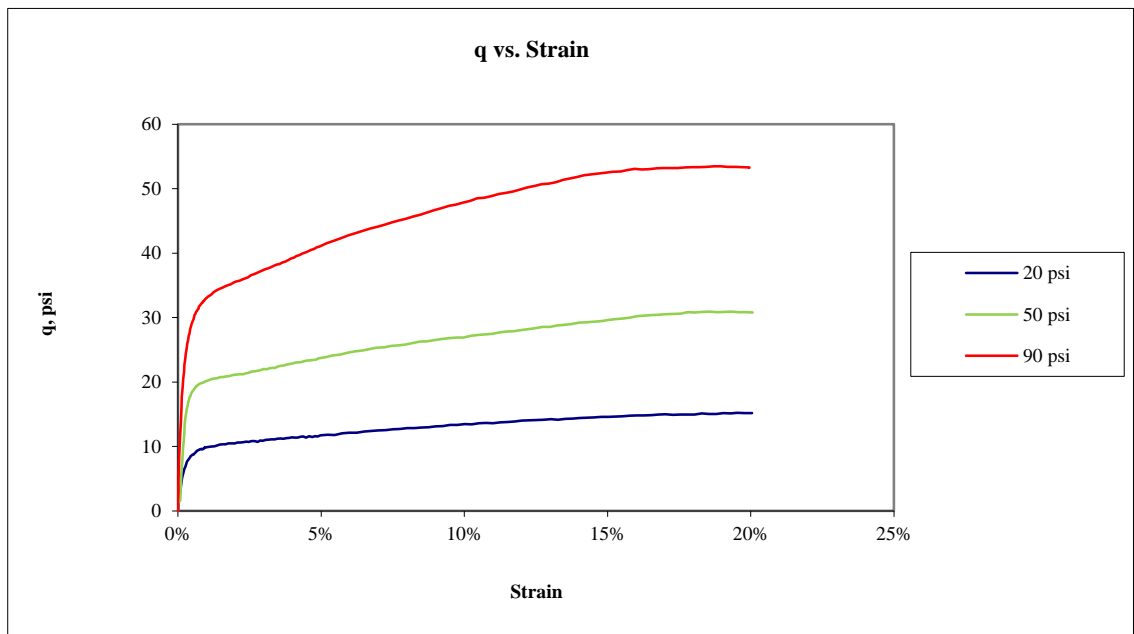
Cell Pressure = 90 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 50 psi

Cell Pressure = 120 psi  
 Back Pressure = 30 psi  
 Confining Pressure = 90 psi

Notes: Sample description: Silt, pale yellow, moist

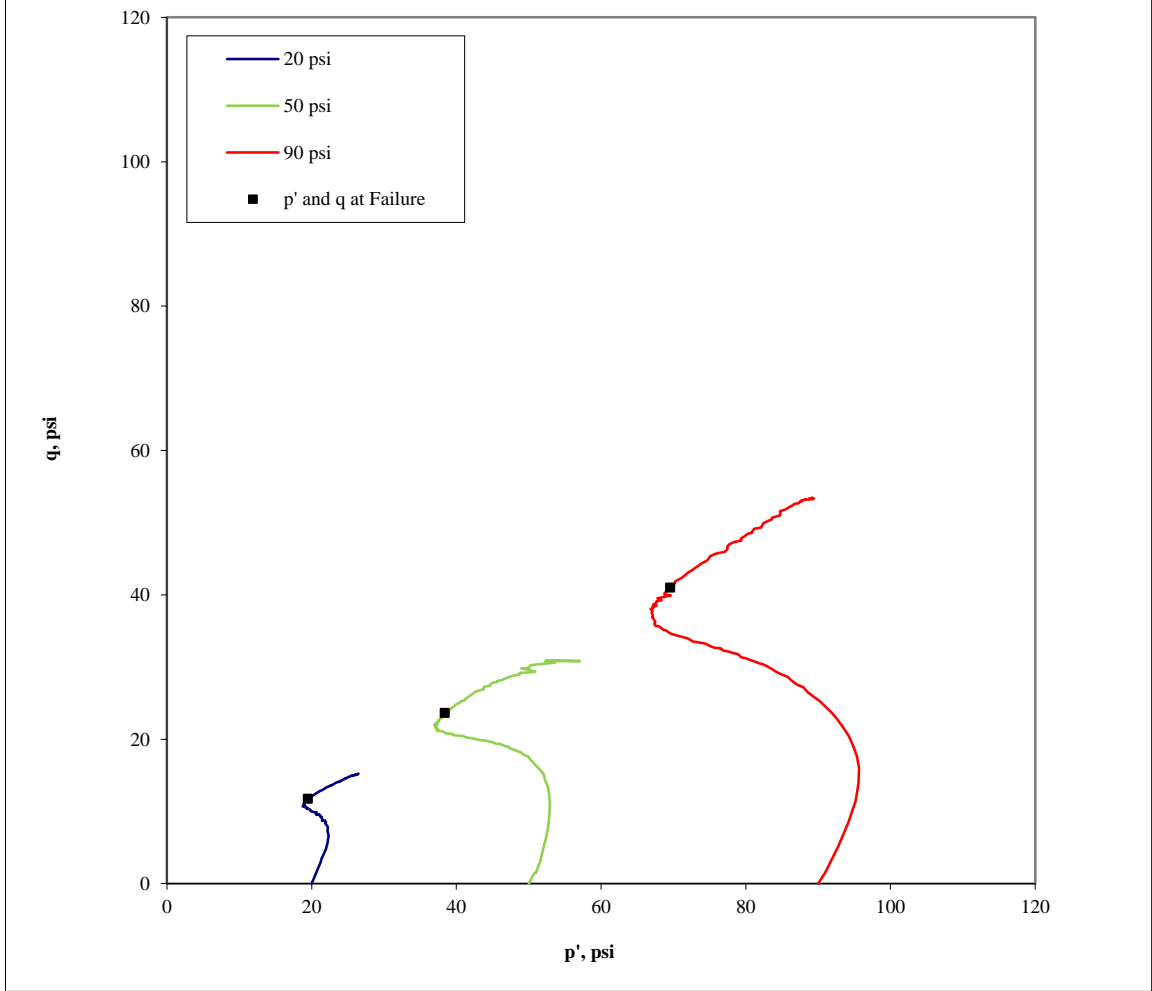
Atterberg limits: LL = 28 PL = 25 PI = 3 (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 84% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type:  Intact  Reconstituted  Slurry consolidated in tube and extruded  
 Moisture from:  Cuttings  Entire specimen  
 Saturation method:  Wet  Dry  
 Failure criterion:  (σ<sub>1</sub>/σ<sub>3</sub>)<sub>max</sub>  (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub>  5 % strain  
 Membrane effect:  Corrected  Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>ASTM D4767</b> <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>			
<b>Job Short Title:</b> <b>Copper Flat Tailngs Design Study</b>					
<b>Sample:</b> <b>Overflow Flume Test Beach</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>DAR</b>	<b>Date:</b> <b>3/12/2013</b>	<b>Job Number:</b> <b>103-92557.010</b>	<b>Figure:</b> <b>1</b>



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 2

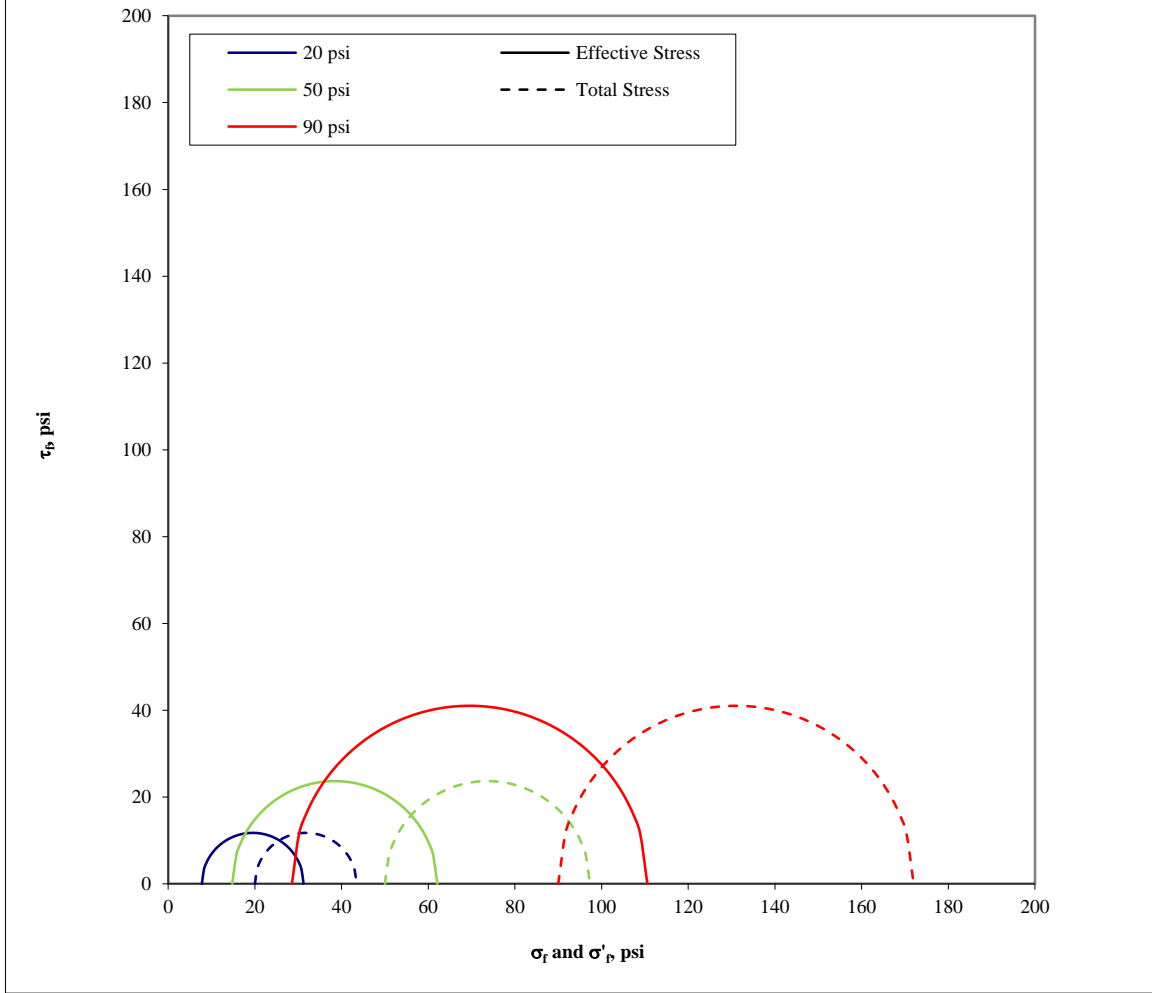
**Stress Path (p'-q) Plot**



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	31.7	19.5	11.7
50	73.7	38.4	23.7
90	131.0	69.6	41.0

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT STRESS PATH PLOT			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 3

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
20	31.2	7.8	43.5	20.0
50	62.1	14.8	97.3	50.0
90	110.6	28.5	172.0	90.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM				
Job Short Title: Copper Flat Tailings Design Study						
Sample: Overflow Flume Test Beach	Technician: RJM	Reviewed: DAR	Date: 3/12/2013	Job Number: 103-92557.010	Figure: 4	



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
<b>Job Short Title:</b> Copper Flat Tailngs Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 5

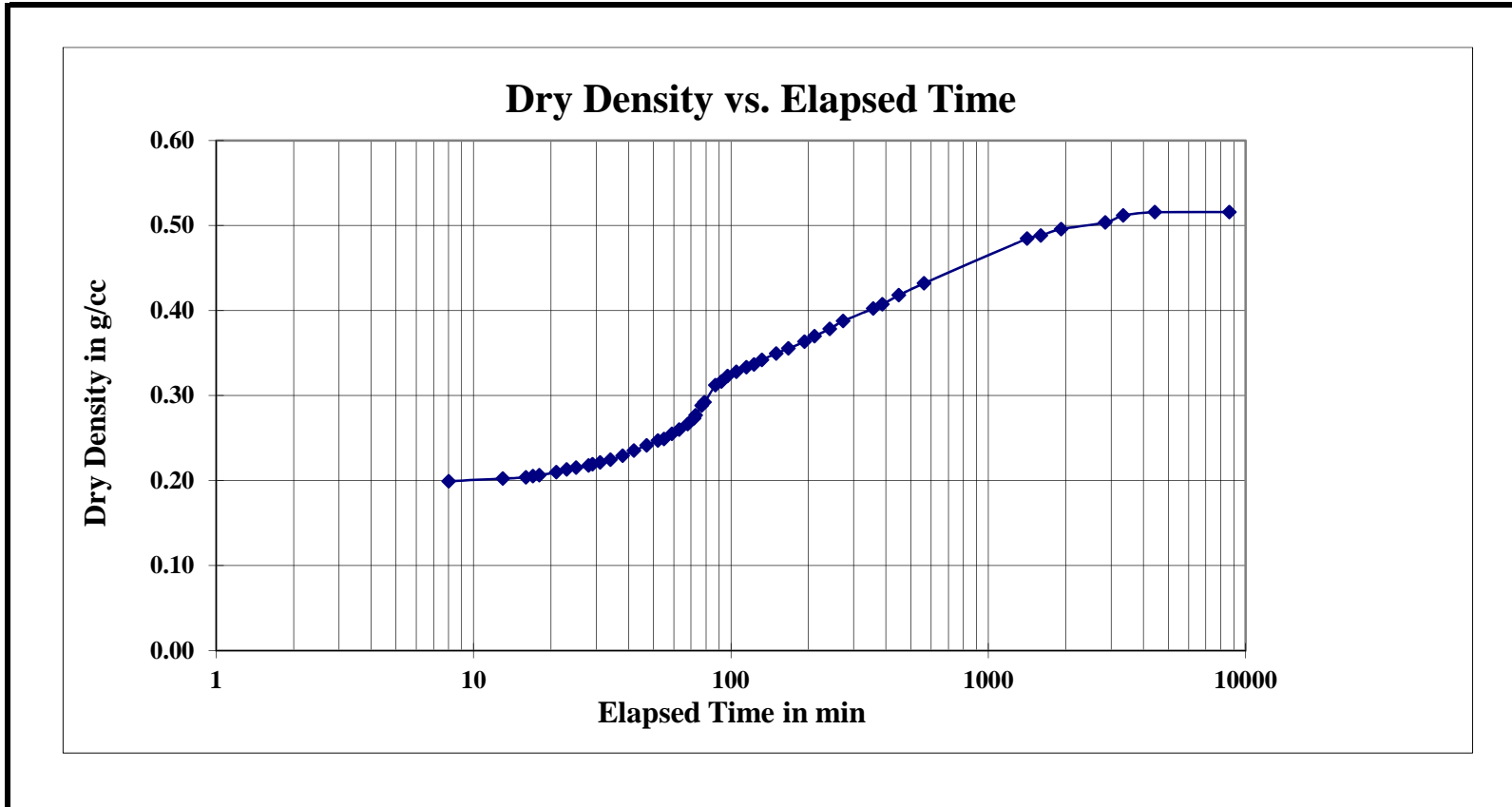


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 6





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 7

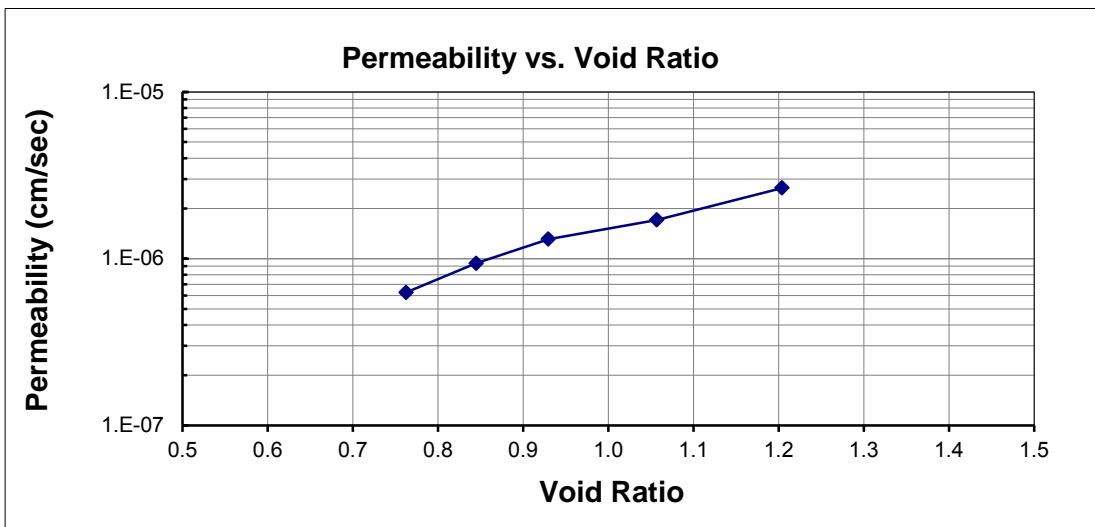
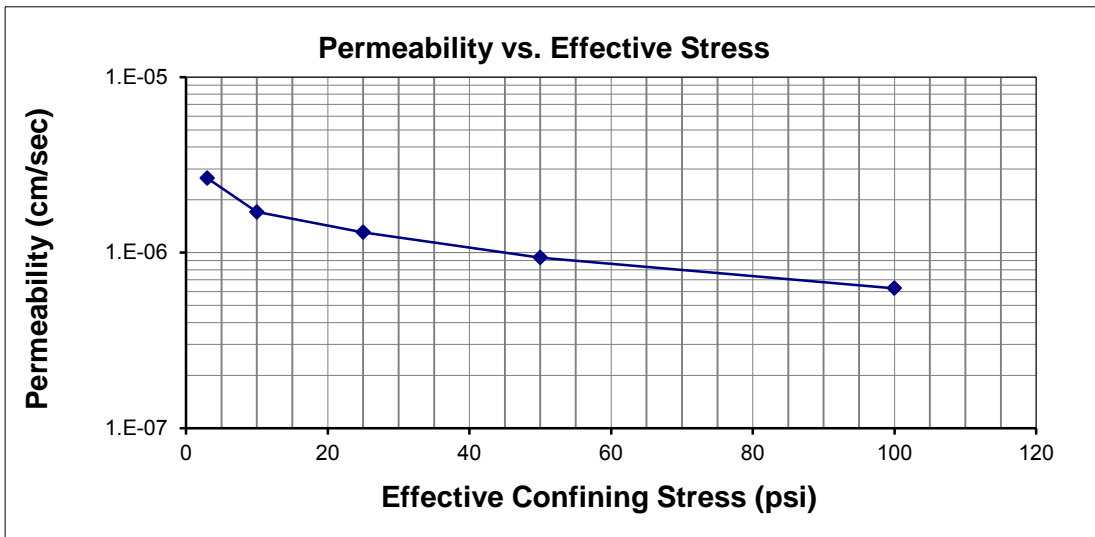


<b>Golder Associates, Inc. Denver, Colorado</b>			<b>Title:</b> <b>SEDIMENTATION TESTING GRAPHICAL DATA</b>			
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>						
<b>Sample No.</b> <b>Tailings Overflow Beach</b>	<b>System</b> <b>Single Drain</b>	<b>Reviewed:</b> MB	<b>Date:</b> 08-Nov-12	<b>Job Number:</b> 103-92557	<b>Figure:</b> 2	

	Initial	Final	
Length =	7.826	1.25	cm
Diameter =	7.11	7.11	cm
Wet Mass =	358.37	98.17	g
Area =	39.70	39.70	cm <sup>2</sup>
Volume =	310.7	49.5	cm <sup>3</sup>
Moisture Content =	378.6%	32.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	74.88	74.09	g
Unit Weight =	1.15	1.98	g/cm <sup>3</sup>
Dry Unit Weight =	0.24	1.50	g/cm <sup>3</sup>
Unit Weight =	72.00	123.62	lb/ft <sup>3</sup>
Dry Unit Weight =	15.04	93.30	lb/ft <sup>3</sup>
Percent Solids =	20.9%	75.5%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
6.1	4.70E-06	1.24E-03	9.4	2.82E-06	2.1E-02	13.4	1.86E-06	3.2E-02	18.5	1.29E-06	5.1E-02	25.8	8.81E-07	7.0E-02
13.3	3.51E-06	9.28E-04	19.1	2.28E-06	1.7E-02	26.1	1.56E-06	2.7E-02	35.8	1.09E-06	4.4E-02	49.9	7.48E-07	5.9E-02
17.5	3.27E-06	8.65E-04	24.8	2.16E-06	1.6E-02	33.7	1.49E-06	2.5E-02	46.0	1.04E-06	4.2E-02	64.4	7.11E-07	5.6E-02
22.3	3.10E-06	8.19E-04	31.41	2.06E-06	1.5E-02	42.27	1.44E-06	2.4E-02	57.41	1.01E-06	4.0E-02	80.74	6.86E-07	5.4E-02
27.8	2.98E-06	7.88E-04	38.77	2.00E-06	1.5E-02	51.99	1.40E-06	2.4E-02	70.07	9.92E-07	4.0E-02	99.34	6.69E-07	5.3E-02
34.3	2.89E-06	7.63E-04	47.58	1.94E-06	1.4E-02	63.55	1.37E-06	2.3E-02	85.49	9.71E-07	3.9E-02	121.11	6.55E-07	5.2E-02
42.2	2.81E-06	7.43E-04	58.69	1.89E-06	1.4E-02	77.12	1.35E-06	2.3E-02	103.67	9.57E-07	3.8E-02	147.26	6.44E-07	5.1E-02
52.5	2.72E-06	7.19E-04	72.92	1.83E-06	1.4E-02	94.4	1.33E-06	2.3E-02	126.62	9.46E-07	3.8E-02	179.88	6.36E-07	5.0E-02
66.3	2.66E-06	7.03E-04	93.21	1.76E-06	1.3E-02	117.21	1.32E-06	2.2E-02	157	9.40E-07	3.8E-02	205.76	6.28E-07	5.0E-02
88.2	2.58E-06	6.82E-04	126.08	1.69E-06	1.2E-02	152.46	1.31E-06	2.2E-02	183.27	9.31E-07	3.7E-02	224.62	6.28E-07	5.0E-02
			145.49	1.67E-06	1.2E-02	175.79	1.30E-06	2.2E-02	203.01	9.40E-07	3.8E-02	248.31	6.24E-07	4.9E-02
Average (of final 3 values)	2.66E-06	7.01E-04	Average (of final 3 values)	1.71E-06	1.26E-02	Average (of final 3 values)	1.31E-06	2.23E-02	Average (of final 3 values)	9.37E-07	3.74E-02	Average (of final 3 values)	6.26E-07	4.95E-02

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Tailings Overflow Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 12/3/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**

Job Short Title:  
 Copper Flat Tailings Design Study

**PERMEABILITY DATA**

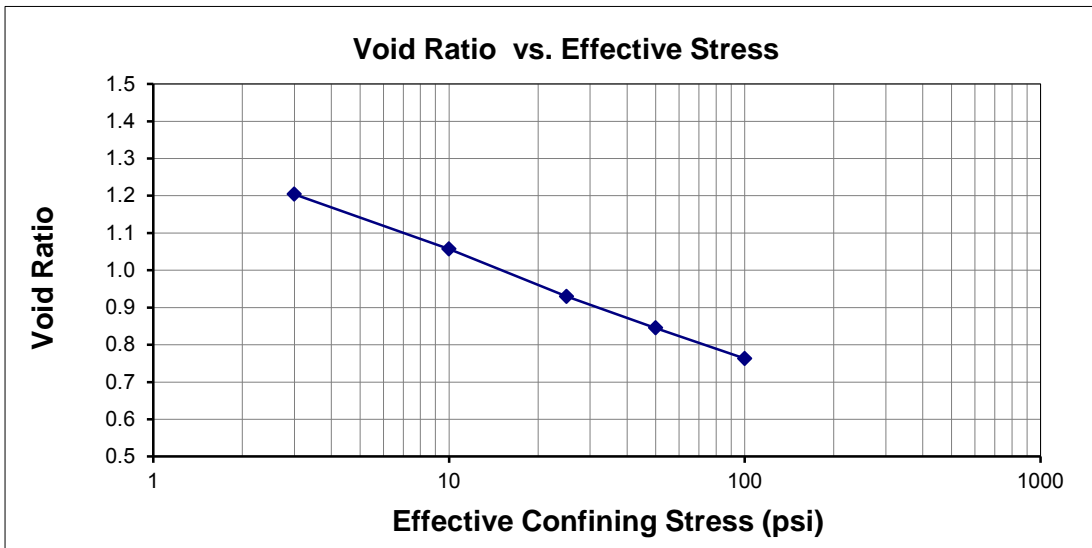
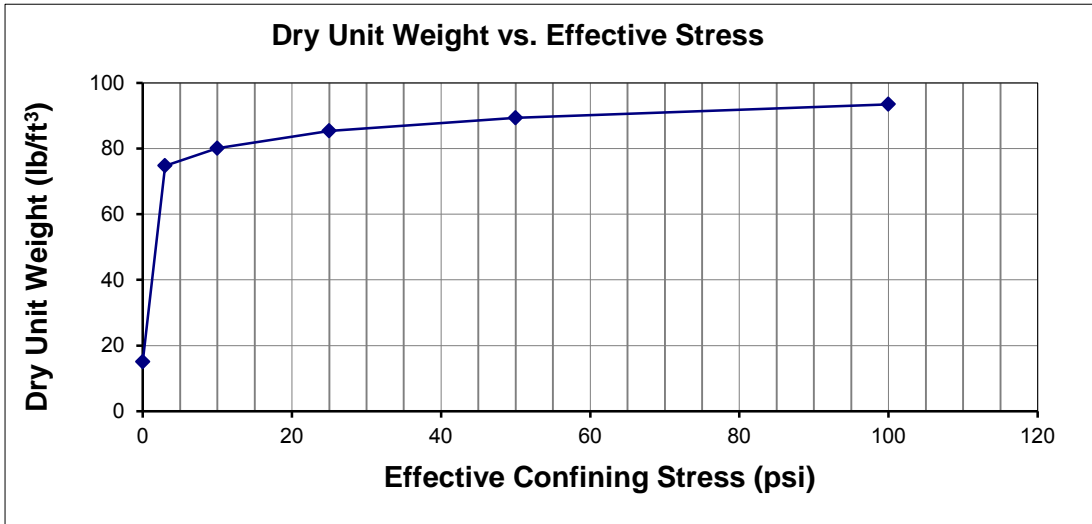
Sample No.  
 Tailings Overflow Flume Test Beach

Reviewed:  
 CCS

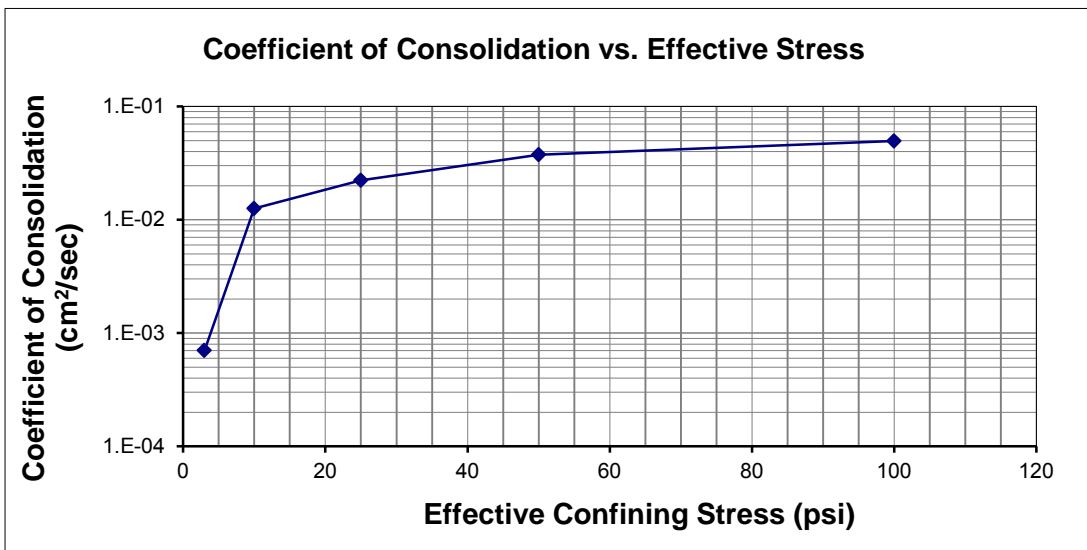
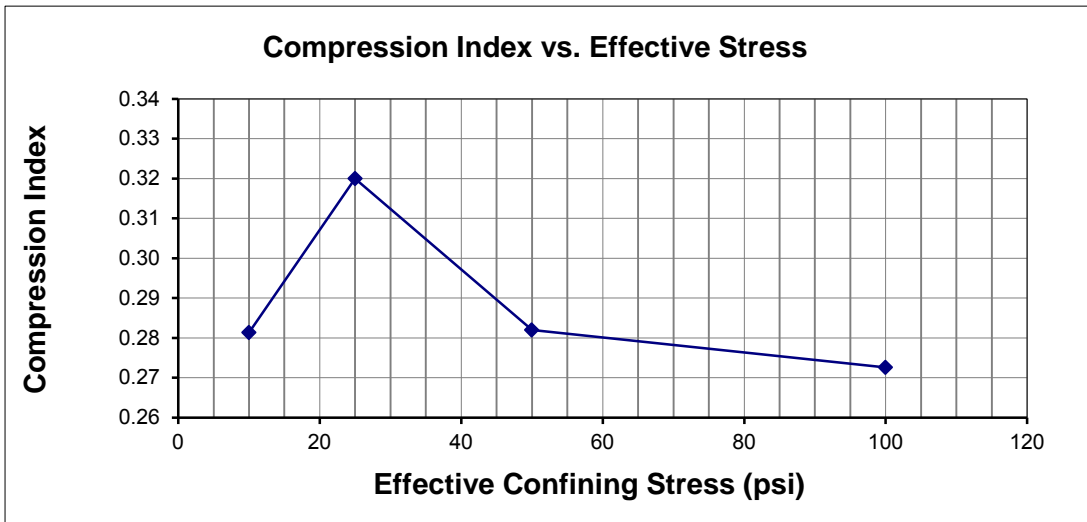
Date:  
 12/3/2012

Job Number:  
 103-92557

Figure:  
 2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>DENSITY DATA</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Tailings Overflow Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 12/3/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample No.  
 Tailings Overflow Flume Test Beach

Reviewed:  
 CCS

Date:  
 12/3/2012

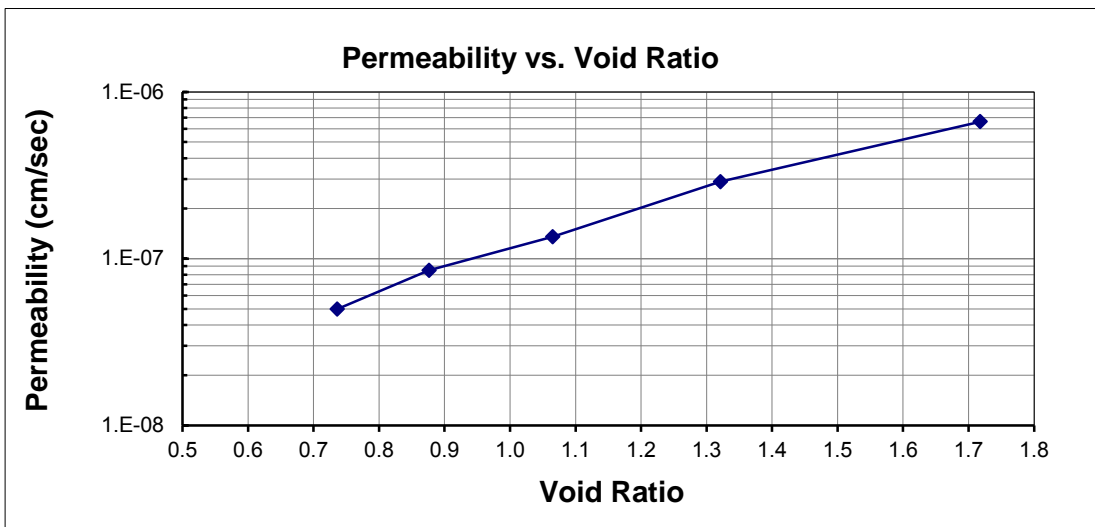
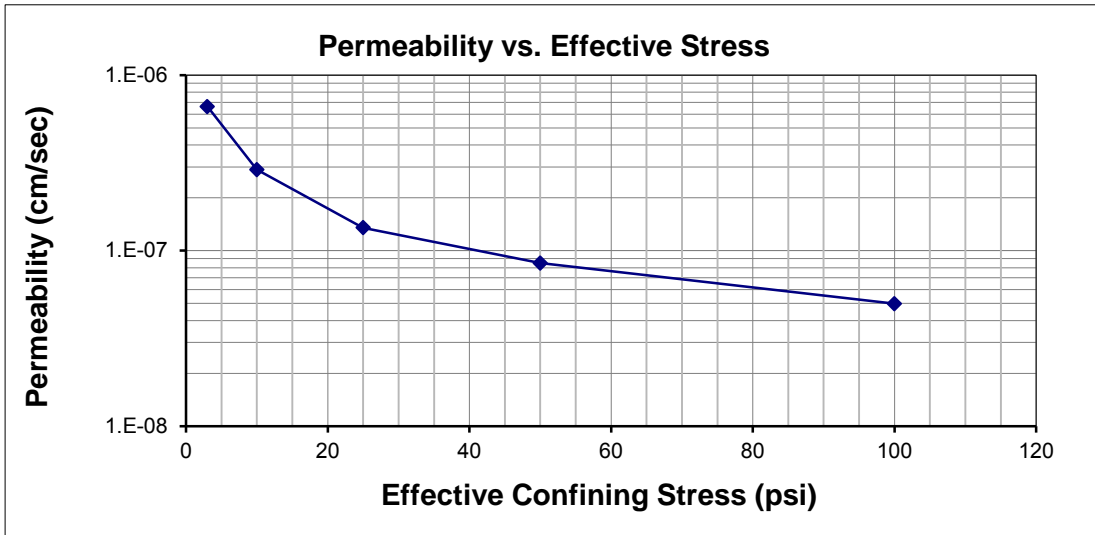
Job Number:  
 103-92557

Figure:  
 4

	Initial	Final	
Length =	8.172	0.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	351.12	53.21	g
Area =	39.64	39.64	cm <sup>2</sup>
Volume =	323.9	25.5	cm <sup>3</sup>
Moisture Content =	786.2%	37.1%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	39.62	38.81	g
Unit Weight =	1.08	2.08	g/cm <sup>3</sup>
Dry Unit Weight =	0.12	1.52	g/cm <sup>3</sup>
Unit Weight =	67.67	130.13	lb/ft <sup>3</sup>
Dry Unit Weight =	7.64	94.92	lb/ft <sup>3</sup>
Percent Solids =	11.3%	72.9%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
22.46	8.20E-07	1.98E-04	50.77	3.10E-07	1.0E-03	98.24	1.42E-07	1.4E-03	144.92	8.78E-08	1.7E-03	228.9	5.14E-08	2.4E-03
41.52	7.28E-07	1.76E-04	86.68	2.98E-07	1.0E-03	166.02	1.38E-07	1.3E-03	243.05	8.59E-08	1.7E-03	384.0	5.03E-08	2.4E-03
52.62	7.06E-07	1.70E-04	107.84	2.94E-07	9.9E-04	205.40	1.37E-07	1.3E-03	301.11	8.52E-08	1.6E-03	474.8	5.00E-08	2.4E-03
65.16	6.89E-07	1.66E-04	131.71	2.91E-07	9.8E-04	250.52	1.36E-07	1.3E-03	364.17	8.52E-08	1.6E-03	576.64	4.98E-08	2.3E-03
79.43	6.78E-07	1.64E-04	158.8	2.90E-07	9.8E-04	299.74	1.36E-07	1.3E-03	436.95	8.51E-08	1.6E-03	690.24	4.98E-08	2.3E-03
96.23	6.68E-07	1.61E-04	190.02	2.89E-07	9.7E-04	359.30	1.36E-07	1.3E-03	524.36	8.46E-08	1.6E-03	824.08	4.98E-08	2.3E-03
115.84	6.63E-07	1.60E-04	226.55	2.90E-07	9.8E-04	430.18	1.36E-07	1.3E-03	625.14	8.49E-08	1.6E-03			
140.26	6.61E-07	1.60E-04				521.68	1.35E-07	1.3E-03	751.92	8.51E-08	1.6E-03			
172.37	6.63E-07	1.60E-04				590.52	1.35E-07	1.3E-03	849.86	8.51E-08	1.6E-03			
						638.40	1.36E-07	1.3E-03						
						707.55	1.35E-07	1.3E-03						
Average (of final 3 values)	6.62E-07	1.60E-04	Average (of final 3 values)	2.89E-07	9.77E-04	Average (of final 3 values)	1.35E-07	1.29E-03	Average (of final 3 values)	8.50E-08	1.64E-03	Average (of final 3 values)	4.98E-08	2.34E-03

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.</b> Tailings Overflow Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 12/17/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**PERMEABILITY DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample No.  
 Tailings Overflow Flume Test Slime

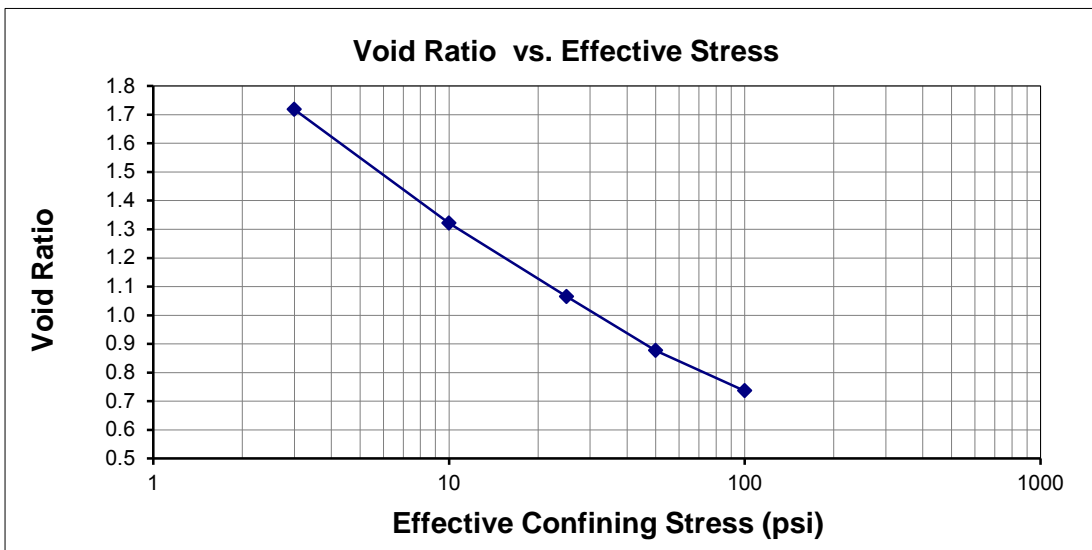
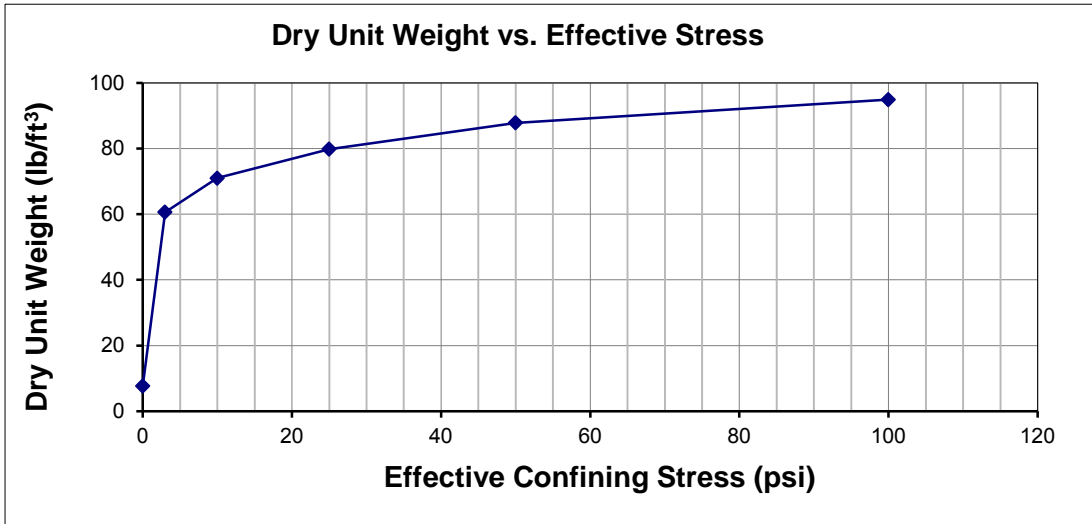
Reviewed:  
 CCS

Date:  
 12/17/2012

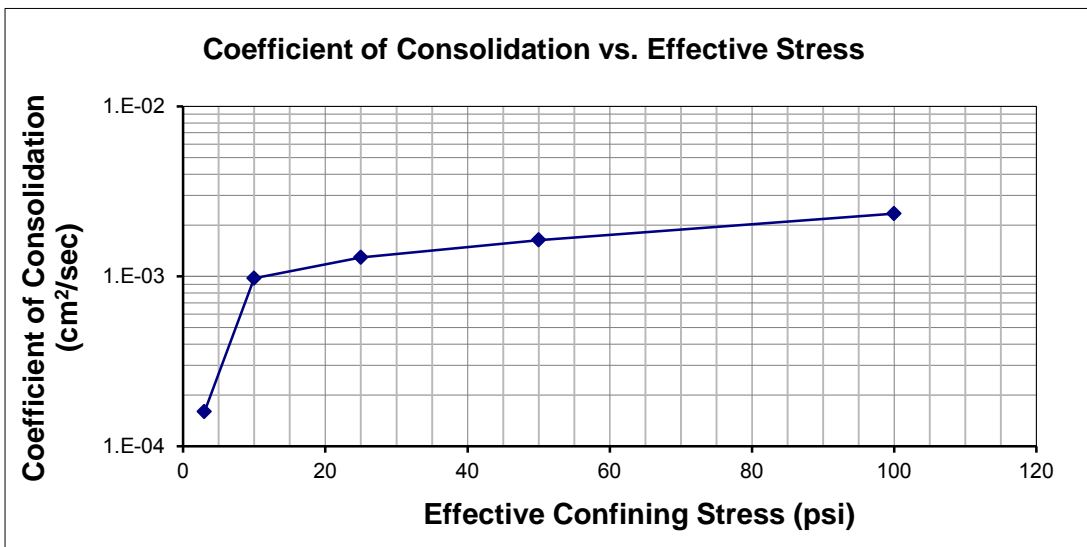
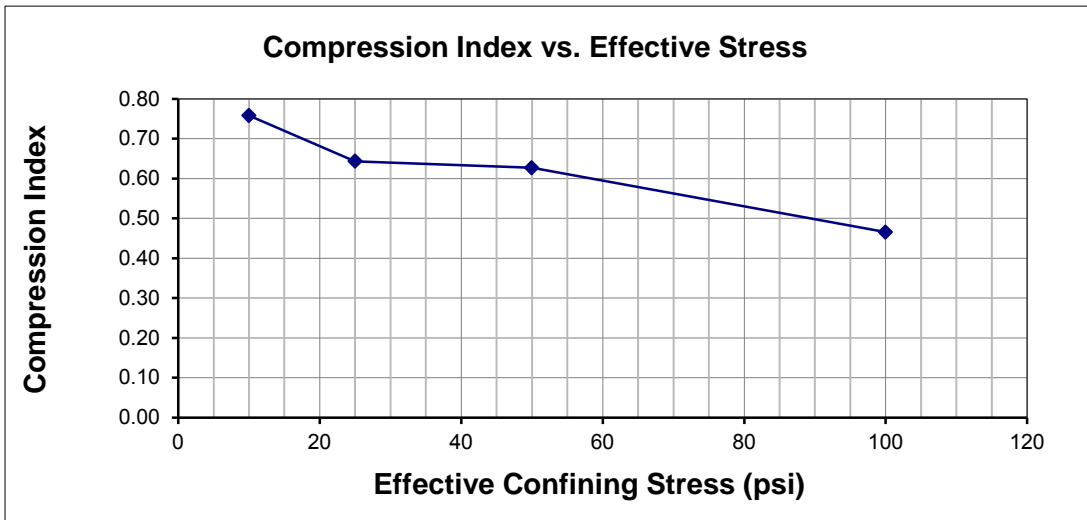
Job Number:  
 103-92557

Figure:  
 2





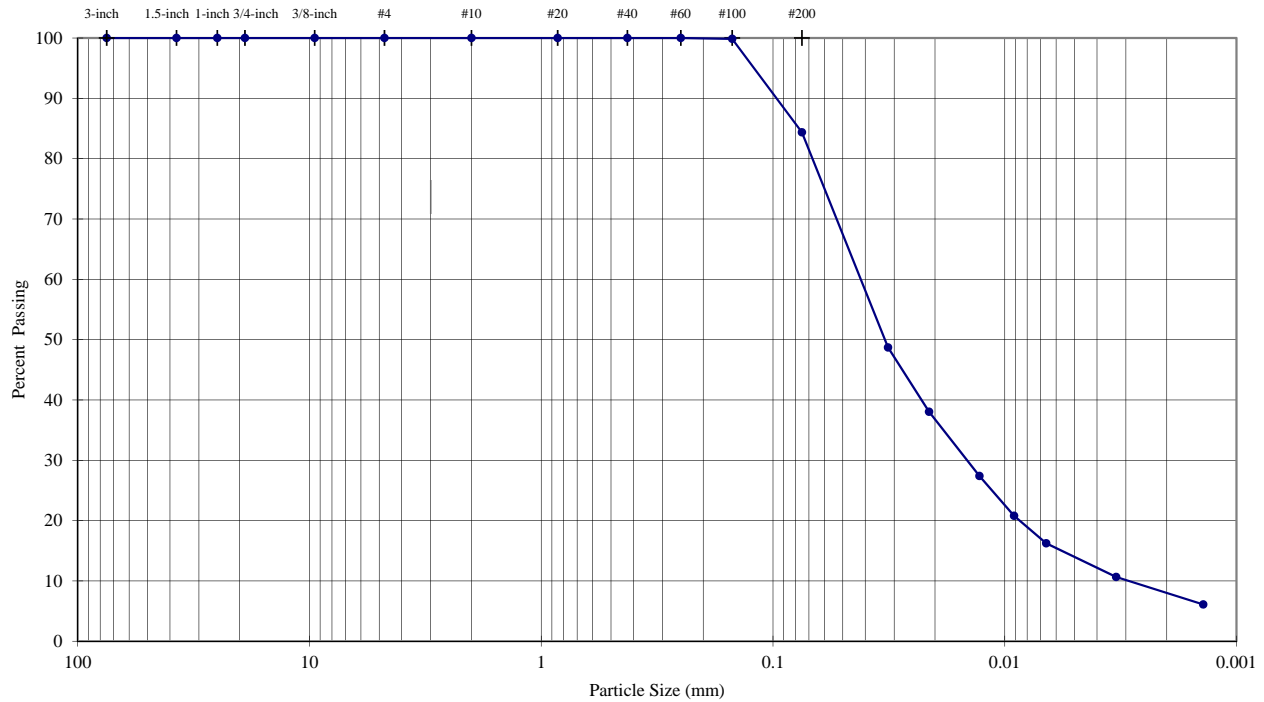
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>DENSITY DATA</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.</b> Tailings Overflow Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 12/17/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3



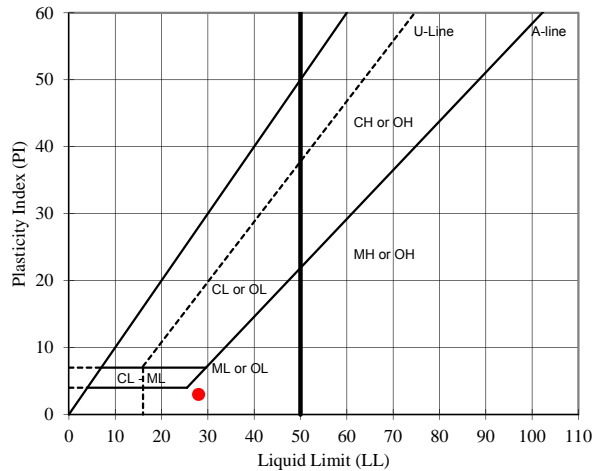
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: <b>SLURRY CONSOLIDATION TEST RESULTS COMPRESSION DATA</b>			
Job Short Title: Copper Flat Tailings Design Study					
Sample No. Tailings Overflow Flume Test Slime	Reviewed: CCS	Date: 12/17/2012	Job Number: 103-92557	Figure: 4	

## PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Beach**  
 TYPE: **Flume Test**



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	Fine Gravel
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Coarse Sand
		#10	2.0	100.0	
		#20	0.85	100.0	Medium Sand
		#40	0.425	100.0	
		#60	0.25	100.0	Fine Sand
	#100	0.15	99.9		
	#200	0.075	84.4		
Hydrometer Analysis			0.032	48.7	Silt or Clay Fines
			0.021	38.0	
			0.013	27.4	
			0.009	20.8	
			0.007	16.2	
			0.003	10.7	
		0.001	6.1		
					<b>84.36</b>



USCS Description (ASTM D 2487):

Dry, pale yellow silt

LL	PL	PI	SpG (assumed)
28	25	3	2.64

As-Received Moisture Content (%)

#DIV/0!

USCS Group Symbol

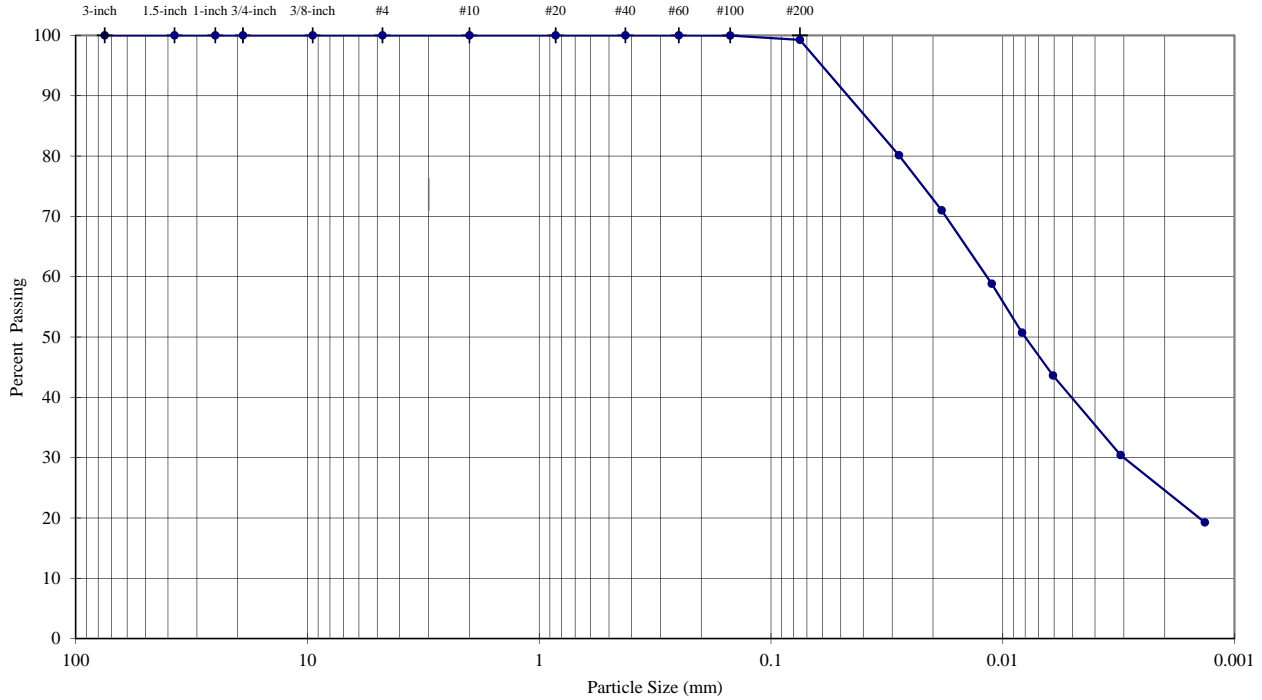
ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

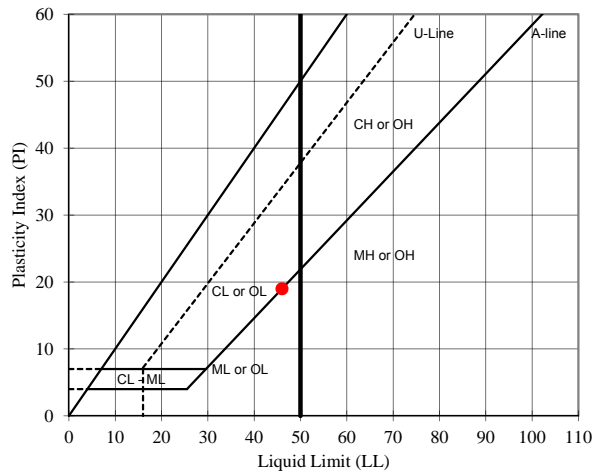
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

## PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Slime**  
 TYPE: **Flume Test**



	Particle Size		Description	Percentage		
	Sieve	(mm)			% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)	3-inch	75.0	100.0	Coarse Gravel	0.00	
	1.5-inch	37.5	100.0			
	1-inch	25.0	100.0			
		3/4-inch	19.0	100.0	Fine Gravel	0.00
		3/8-inch	9.5	100.0		
		#4	4.75	100.0	Coarse Sand	0.00
		#10	2.0	100.0		
		#20	0.85	100.0		
		#40	0.425	100.0	Medium Sand	0.00
		#60	0.25	100.0		
	#100	0.15	100.0	Fine Sand	0.74	
	#200	0.075	99.3			
Hydrometer Analysis		0.028	80.1	Silt or Clay Fines	99.26	
		0.018	71.0			
		0.011	58.8			
		0.008	50.7			
		0.006	43.6			
		0.003	30.4			
	0.001	19.3				



Visual Description (Golder Procedure):  
 Dry, pale yellow silty clay

LL	PL	PI	SpG (assumed)
46	27	19	2.64

As-Received Moisture Content (%) USCS Group Symbol  
-- CL-ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

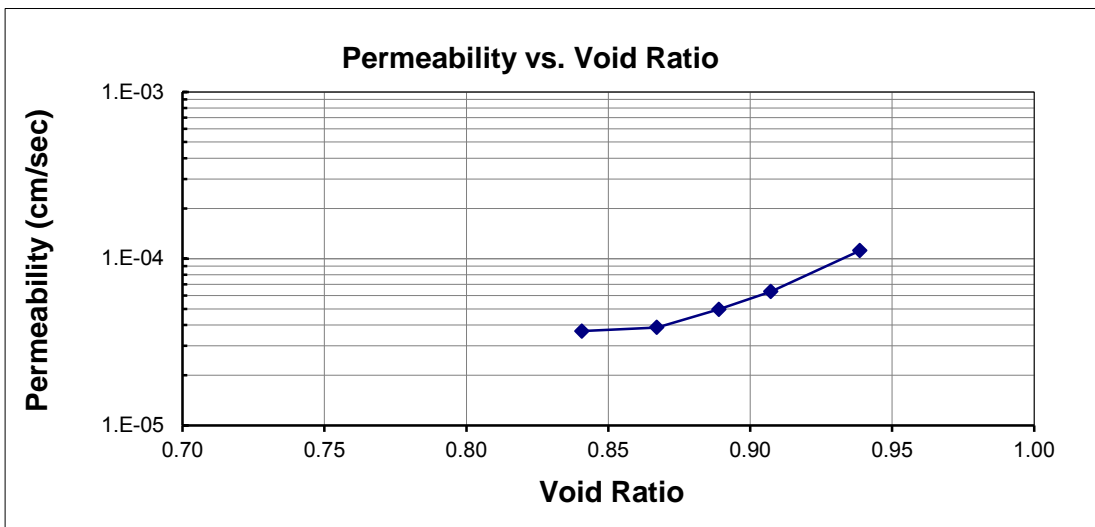
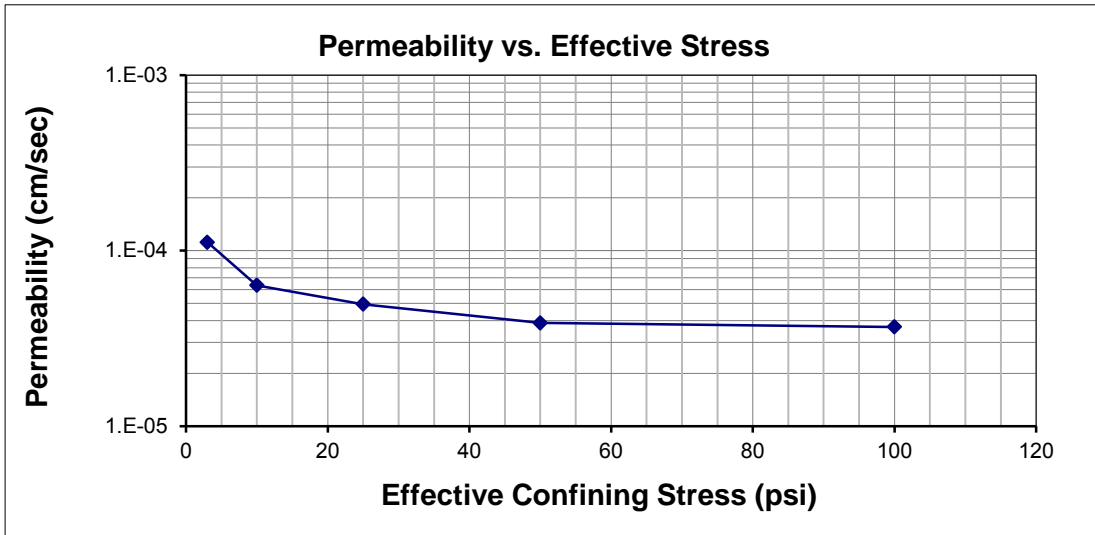
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

**APPENDIX B.4.2  
WHOLE TAILINGS TEST REPORTS**

	Initial	Final	
Length =	7.852	5.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	515.99	424.51	g
Area =	39.64	39.64	cm <sup>2</sup>
Volume =	311.2	223.6	cm <sup>3</sup>
Moisture Content =	60.1%	32.4%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	322.29	320.63	g
Unit Weight =	1.66	1.90	g/cm <sup>3</sup>
Dry Unit Weight =	1.04	1.43	g/cm <sup>3</sup>
Unit Weight =	103.50	118.30	lb/ft <sup>3</sup>
Dry Unit Weight =	64.65	89.35	lb/ft <sup>3</sup>
Percent Solids =	62.5%	75.5%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
3.85	1.12E-04	9.86E-02	6.70	6.33E-05	1.93	10.15	4.96E-05	5.46	10.77	3.86E-05	5.86	8.18	3.68E-05	9.14
4.60	1.12E-04	9.88E-02	8.03	6.33E-05	1.93	12.44	4.92E-05	5.41	12.80	3.89E-05	5.90	9.75	3.69E-05	9.16
5.68	1.11E-04	9.73E-02	9.69	6.38E-05	1.94	15.42	4.99E-05	5.50	15.65	3.86E-05	5.87	11.77	3.65E-05	9.07
Average (of final 3 values)	1.12E-04	9.83E-02	Average (of final 3 values)	6.35E-05	1.93E+00	Average (of final 3 values)	4.96E-05	5.46E+00	Average (of final 3 values)	3.87E-05	5.88E+00	Average (of final 3 values)	3.67E-05	9.12E+00

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Whole Tailings Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**

Job Short Title:  
 Copper Flat Tailings Design Study

**PERMEABILITY DATA**

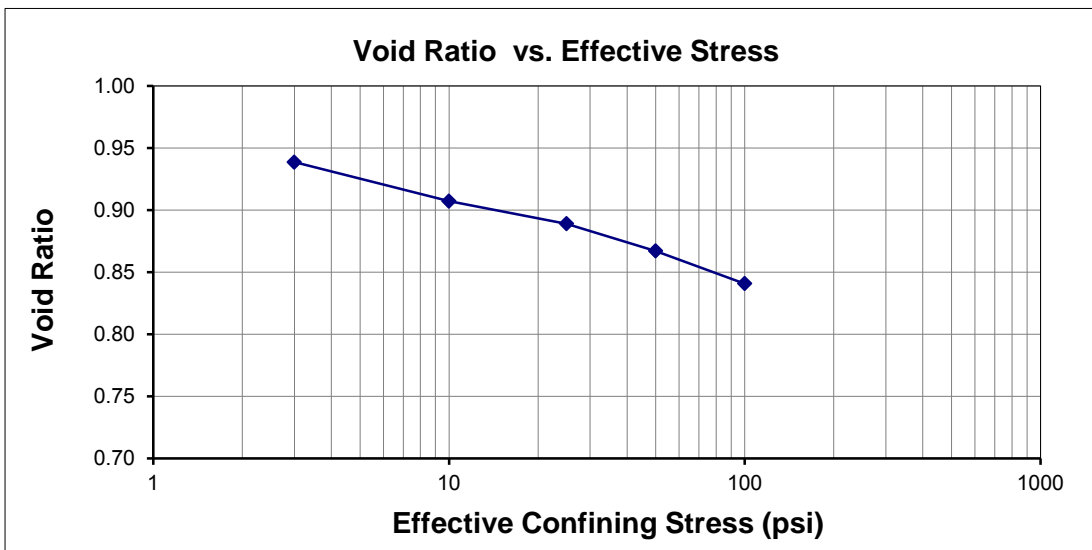
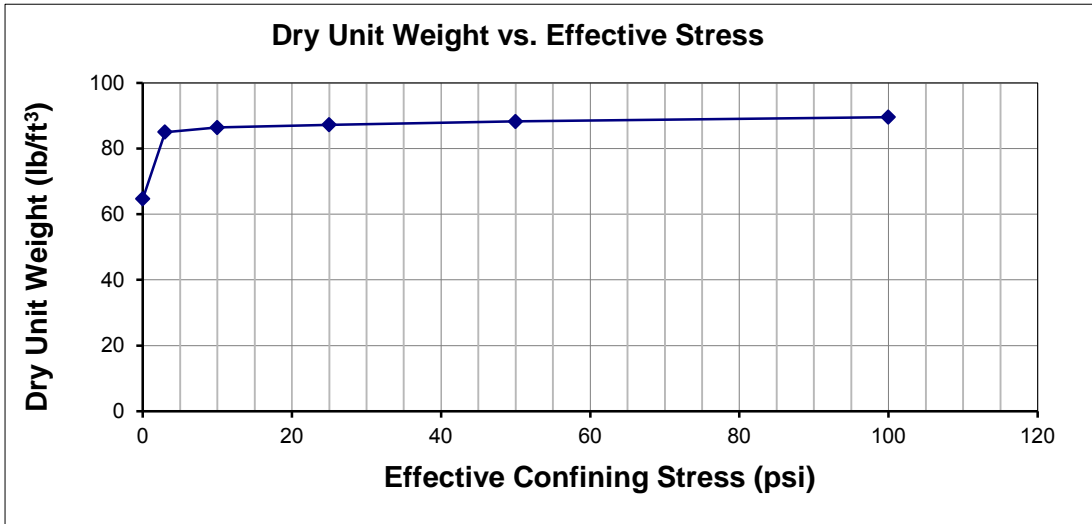
Sample No.  
 Whole Tailings Flume Test Beach

Reviewed:  
 CCS

Date:  
 11/12/2012

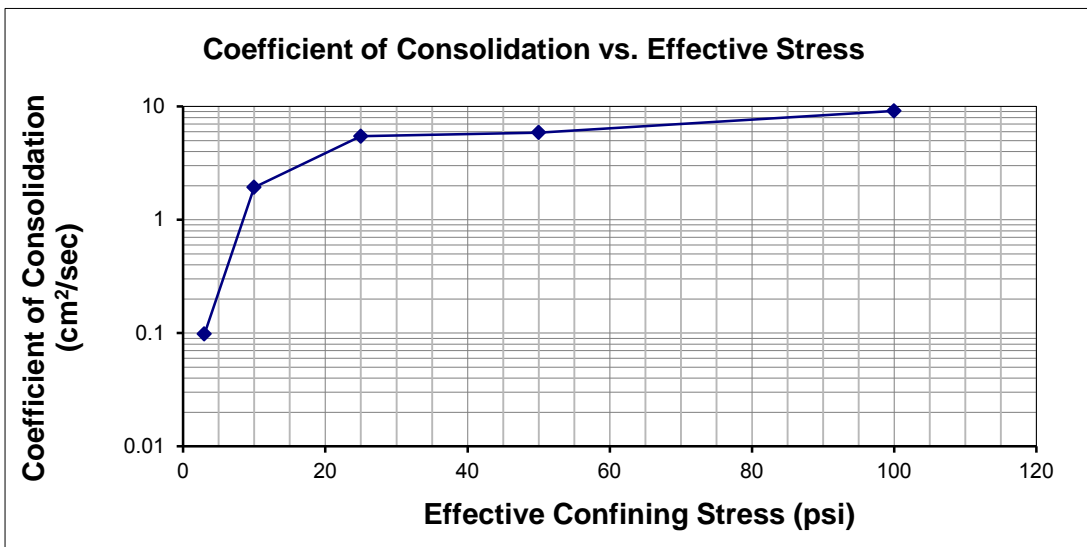
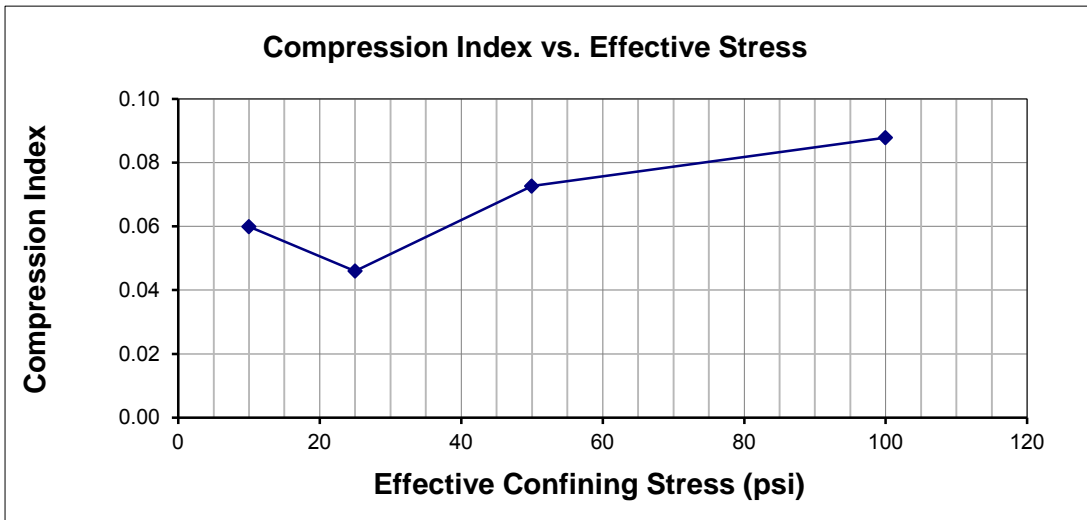
Job Number:  
 103-92557

Figure:  
 2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>DENSITY DATA</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.:</b> Whole Tailings Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3





**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

**Job Short Title:**  
 Copper Flat Tailings Design Study

**Sample No.**  
 Whole Tailings Flume Test Beach

**Reviewed:**  
 CCS

**Date:**  
 11/12/2012

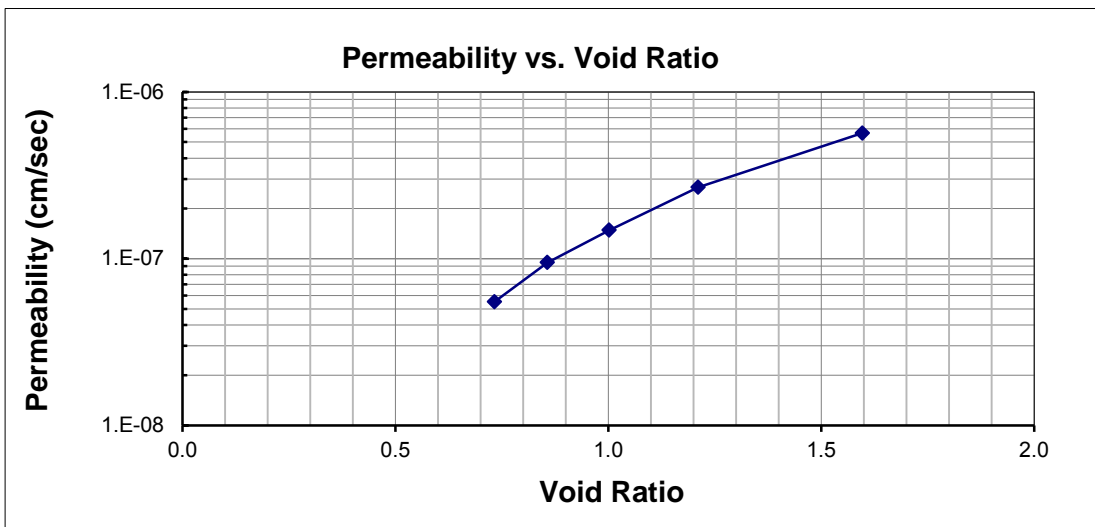
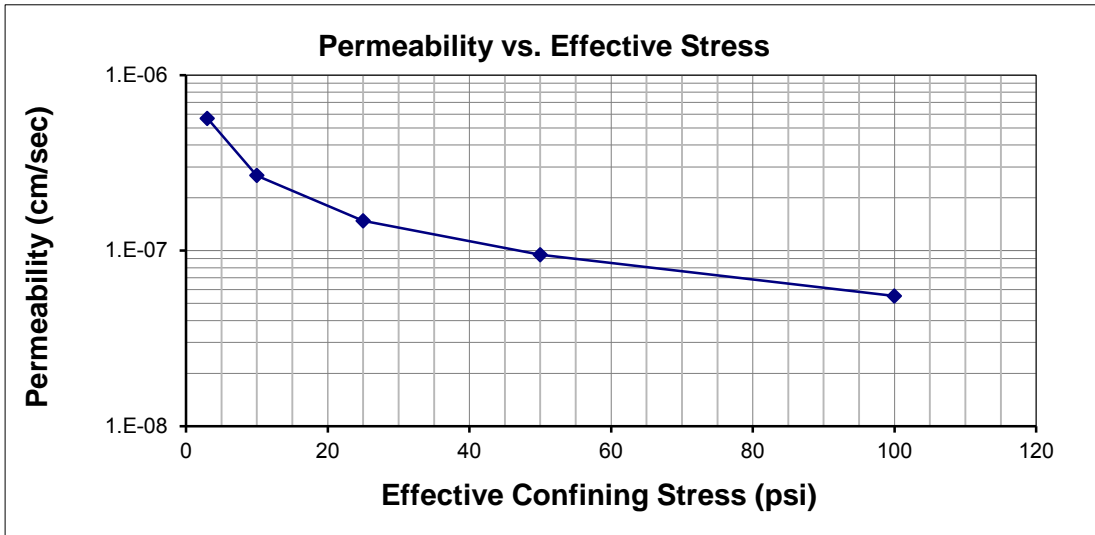
**Job Number:**  
 103-92557

**Figure:**  
 4

	Initial	Final	
Length =	7.285	0.740	cm
Diameter =	7.099	7.099	cm
Wet Mass =	338.65	60.46	g
Area =	39.58	39.58	cm <sup>2</sup>
Volume =	288.3	29.29	cm <sup>3</sup>
Moisture Content =	628.5%	35.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	46.49	44.62	g
Unit Weight =	1.174	2.064	g/cm <sup>3</sup>
Dry Unit Weight =	0.1612	1.523	g/cm <sup>3</sup>
Unit Weight =	73.32	128.6	lb/ft <sup>3</sup>
Dry Unit Weight =	10.06	94.90	lb/ft <sup>3</sup>
Percent Solids =	13.7%	73.8%	

Piston Pressure:	8 psi	562.5 g/cm <sup>2</sup>	Piston Pressure:	15 psi	1,054.6 g/cm <sup>2</sup>	Piston Pressure:	30 psi	2,109.2 g/cm <sup>2</sup>	Piston Pressure:	55 psi	3,866.9 g/cm <sup>2</sup>	Piston Pressure:	105 psi	7,382.3 g/cm <sup>2</sup>
Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>
Consolidation pressure:	3 psi	210.9 g/cm <sup>2</sup>	Consolidation Pressure:	10 psi	703.1 g/cm <sup>2</sup>	Consolidation Pressure:	25 psi	1,757.7 g/cm <sup>2</sup>	Consolidation Pressure:	50 psi	3,515.4 g/cm <sup>2</sup>	Consolidation Pressure:	100 psi	7,030.8 g/cm <sup>2</sup>
<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>		
Initial Sample Height:	7.29	cm	Initial Sample Height:	1.11	cm	Initial Sample Height:	0.94	cm	Initial Sample Height:	0.86	cm	Initial Sample Height:	0.79	cm
Initial Dry Unit Weight:	0.16	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.02	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.19	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.32	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.42	g/cm <sup>3</sup>
Initial Void Ratio:	15.38		Initial Void Ratio:	1.60		Initial Void Ratio:	1.21		Initial Void Ratio:	1.00		Initial Void Ratio:	0.86	
<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>		
Final Sample Height:	1.11	cm	Final Sample Height:	0.94	cm	Final Sample Height:	0.86	cm	Final Sample Height:	0.79	cm	Final Sample Height:	0.74	cm
Final Dry Unit Weight:	1.02	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.19	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.32	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.42	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.52	g/cm <sup>3</sup>
Final Void Ratio:	1.60		Final Void Ratio:	1.21		Final Void Ratio:	1.00		Final Void Ratio:	0.86		Final Void Ratio:	0.73	
<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>		
Coefficient of Compressibility, a <sub>v</sub>	6.53E-02	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	7.85E-04	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	1.98E-04	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	8.26E-05	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	3.53E-05	cm <sup>2</sup> /g
Coefficient of Volume Compressibility, m <sub>v</sub>	3.99E-03	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	3.02E-04	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	8.94E-05	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	4.13E-05	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	1.90E-05	cm <sup>2</sup> /g
Compression Index, C <sub>c</sub>	-		Compression Index, C <sub>c</sub>	0.74		Compression Index, C <sub>c</sub>	0.52		Compression Index, C <sub>c</sub>	0.48		Compression Index, C <sub>c</sub>	0.41	
<b>Δ Time</b>	<b>Permeability k</b>	<b>Coefficient of Consolidation, c<sub>v</sub></b>	<b>Δ Time</b>	<b>Permeability k</b>	<b>Coefficient of Consolidation, c<sub>v</sub></b>	<b>Δ Time</b>	<b>Permeability k</b>	<b>Coefficient of Consolidation, c<sub>v</sub></b>	<b>Δ Time</b>	<b>Permeability k</b>	<b>Coefficient of Consolidation, c<sub>v</sub></b>	<b>Δ Time</b>	<b>Permeability k</b>	<b>Coefficient of Consolidation, c<sub>v</sub></b>
(sec)	(cm/sec)	(cm <sup>2</sup> /sec)	(sec)	(cm/sec)	(cm <sup>2</sup> /sec)	(sec)	(cm/sec)	(cm <sup>2</sup> /sec)	(sec)	(cm/sec)	(cm <sup>2</sup> /sec)	(sec)	(cm/sec)	(cm <sup>2</sup> /sec)
204	5.66E-07	1.42E-04	367	2.67E-07	8.8E-04	604	1.47E-07	1.6E-03	533	9.48E-08	2.3E-03	1024	5.51E-08	2.9E-03
221	5.69E-07	1.43E-04	397	2.69E-07	8.9E-04	652	1.49E-07	1.7E-03	639	9.46E-08	2.3E-03	1233	5.52E-08	2.9E-03
245	5.65E-07	1.42E-04	440	2.67E-07	8.8E-04	718	1.48E-07	1.7E-03	770	9.47E-08	2.3E-03	1396	5.51E-08	2.9E-03
Average (of final 3 values)	5.66E-07	1.42E-04	Average (of final 3 values)	2.68E-07	8.86E-04	Average (of final 3 values)	1.48E-07	1.66E-03	Average (of final 3 values)	9.47E-08	2.30E-03	Average (of final 3 values)	5.51E-08	2.90E-03

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>	
<b>Job Short Title:</b> Copper Flat Tailings Design Study			
<b>Sample No.</b> Whole Tailings Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 10/30/2012	<b>Job Number:</b> 103-92557
			<b>Figure:</b> 1



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**

Job Short Title:  
 Copper Flat Tailings Design Study

**PERMEABILITY DATA**

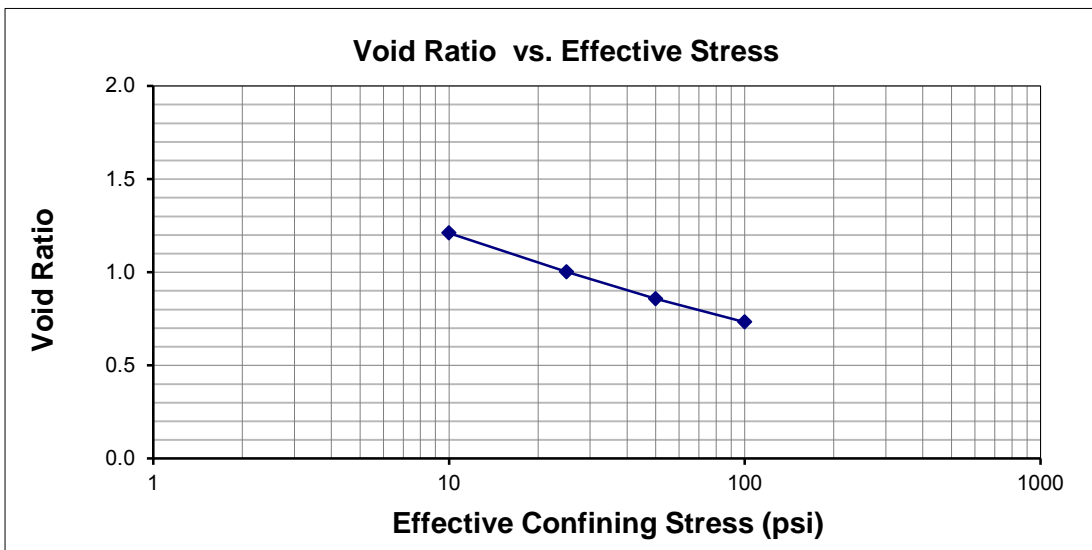
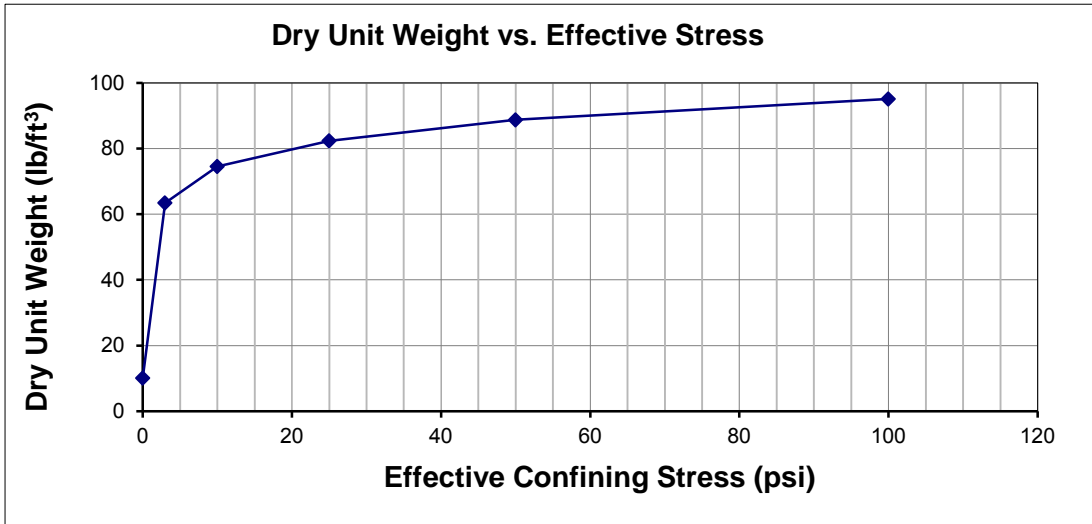
Sample No.  
 Whole Tailings Flume Test Slime

Reviewed:  
 CCS

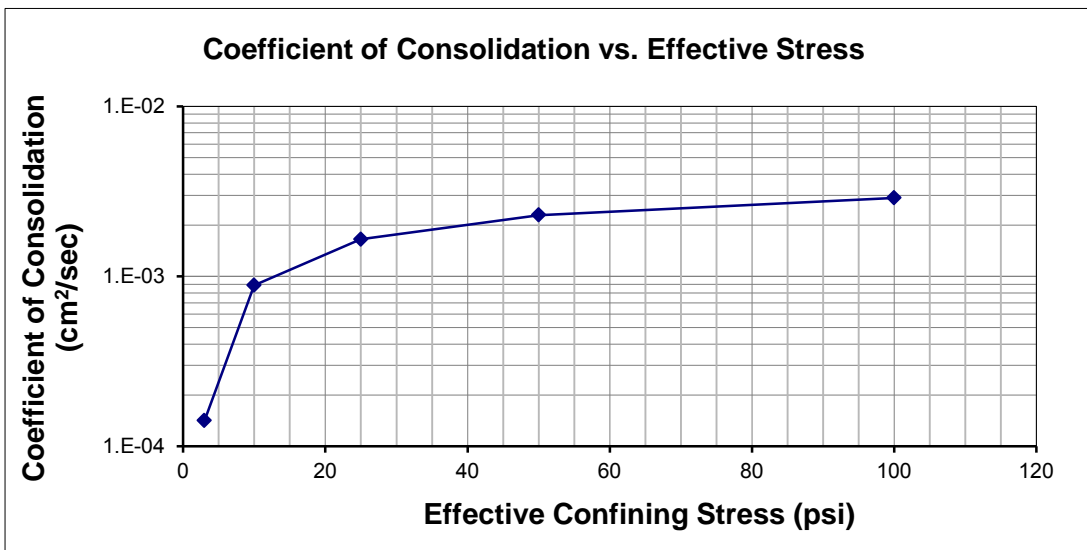
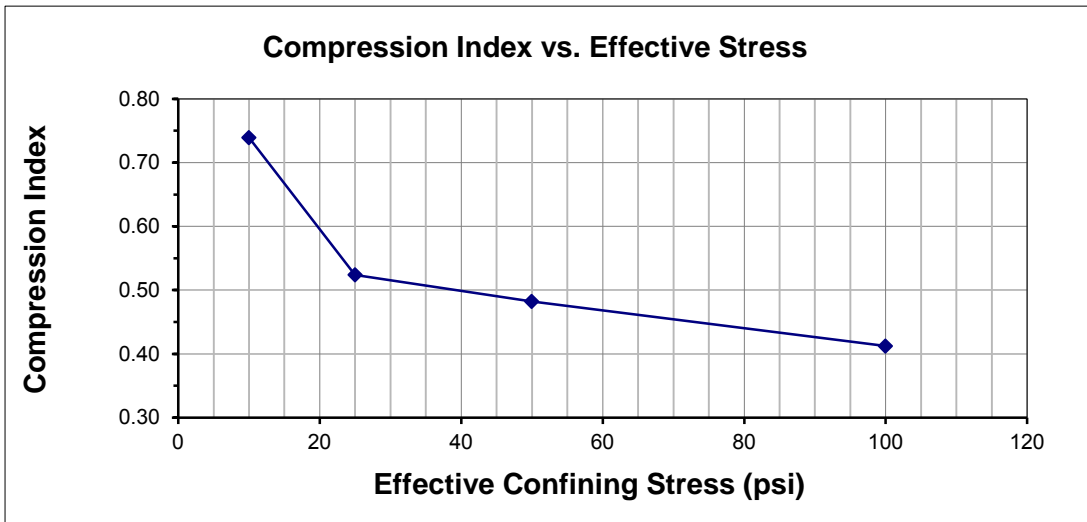
Date:  
 10/30/2012

Job Number:  
 103-92557

Figure:  
 2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>DENSITY DATA</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.</b> Whole Tailings Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 10/30/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample No.  
 Whole Tailings Flume Test Slime

Reviewed:  
 CCS

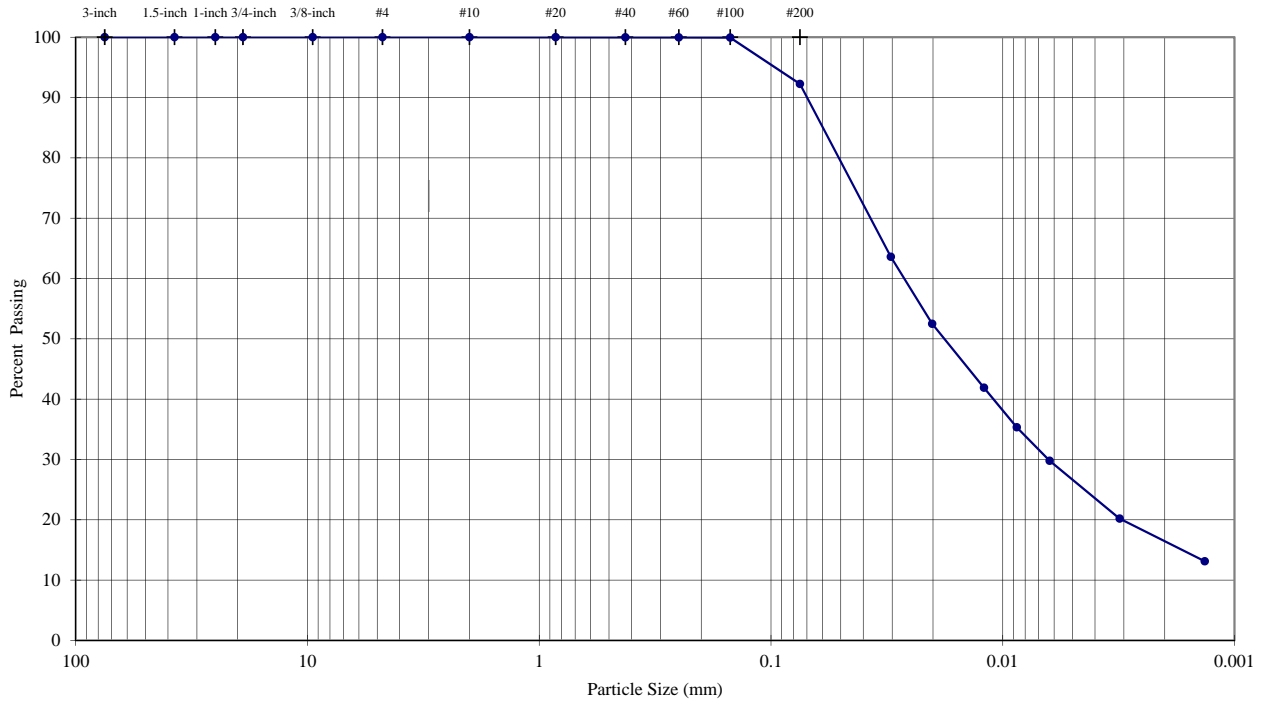
Date:  
 10/30/2012

Job Number:  
 103-92557

Figure:  
 4

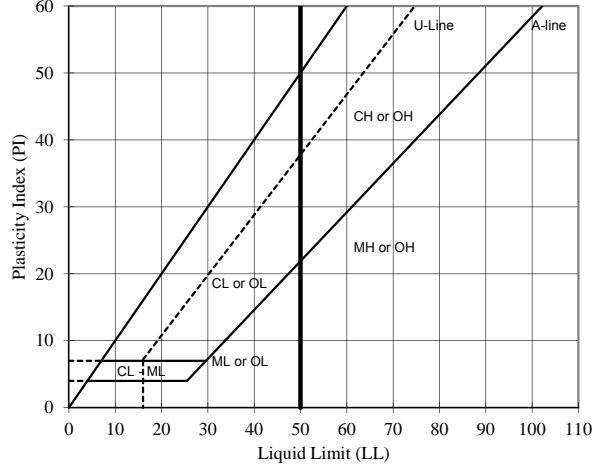
### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Flume** DEPTH (ft): **Beach**  
 TYPE: **Single Drain**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.02
#40	0.425		
#60	0.25	Fine Sand	7.72
#100	0.15		
#200	0.075		
	0.030	Silt or Clay Fines	92.26
	0.020		
	0.012		
	0.009		
	0.006		
	0.003		
	0.001		



Visual Description (Golder Procedure):  
**Wet, light yellowish brown SITLY SAND**

LL	PL	PI	Spg (assumed)
--	--	--	2.64

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 --

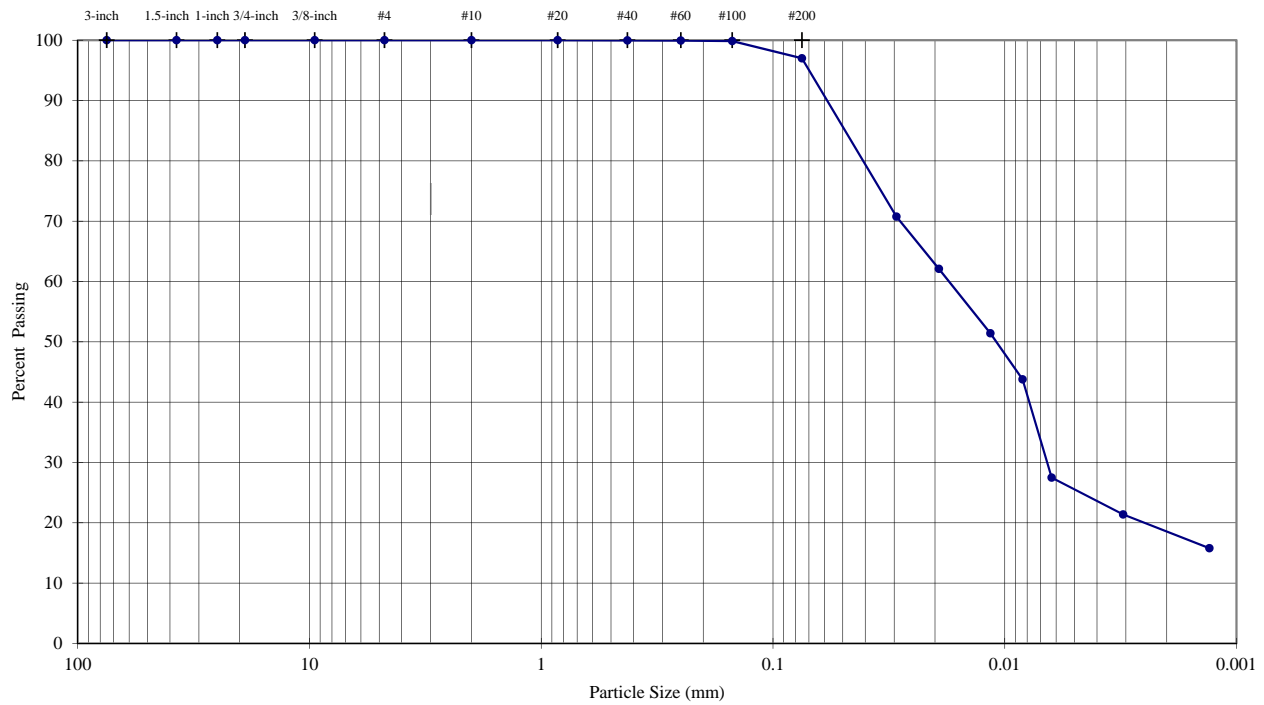
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH: **RJM/SRS**  
 DATE: **11/8/2012**  
 REVIEW: **MB**

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

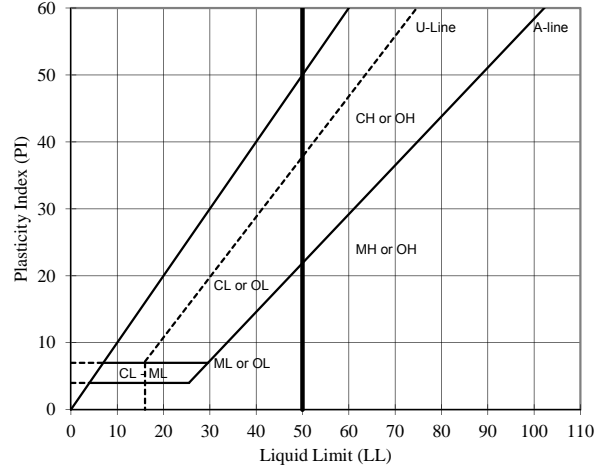
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings**  
 TYPE: **Flume**

DEPTH (ft): **Slime**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.04
#40	0.425		
#60	0.25	Fine Sand	2.96
#100	0.15		
#200	0.075		
	0.029		
	0.019	Silt or Clay Fines	97.00
	0.012		
	0.008		
	0.006		
	0.003		
	0.001		



Visual Description (Golder Procedure):  
**Wet, light yellowish brown silty sand**

LL	PL	PI	SpG (assumed)
--	--	--	2.64

As-Received Moisture Content (%)  
 --

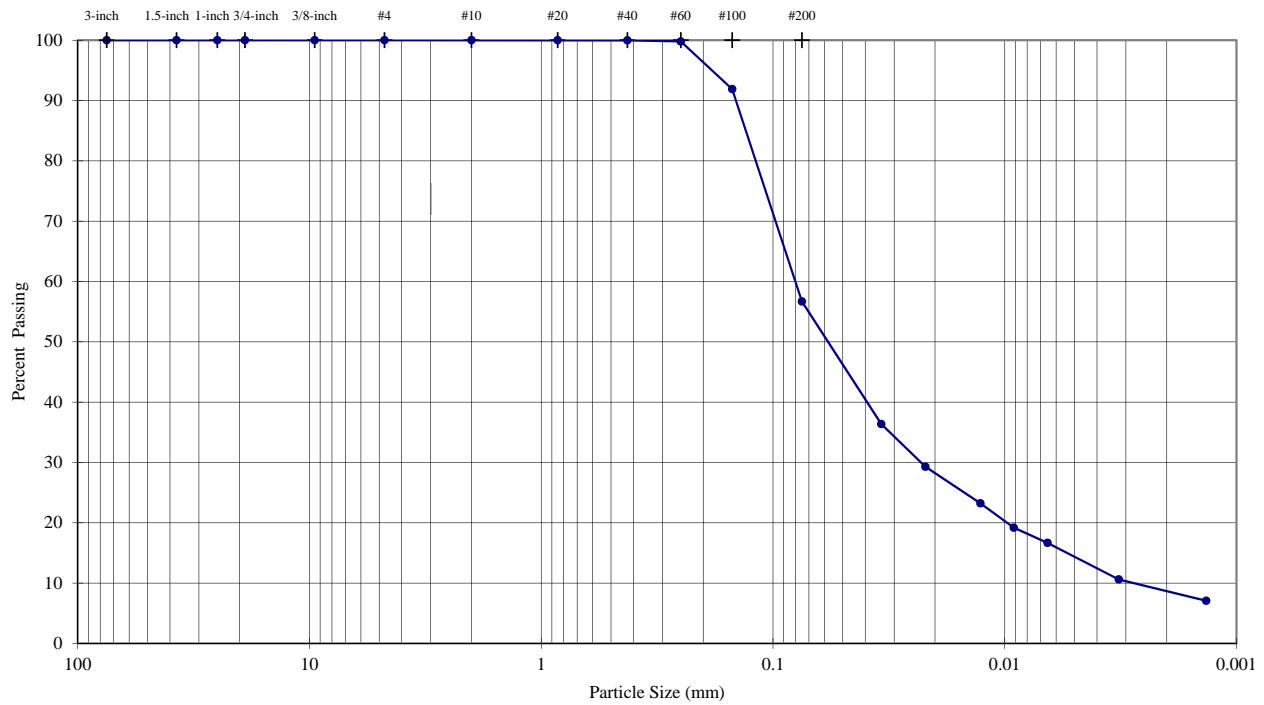
USCS Group Symbol  
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Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH AM/SRS  
 DATE 11/8/2012  
 REVIEW MB

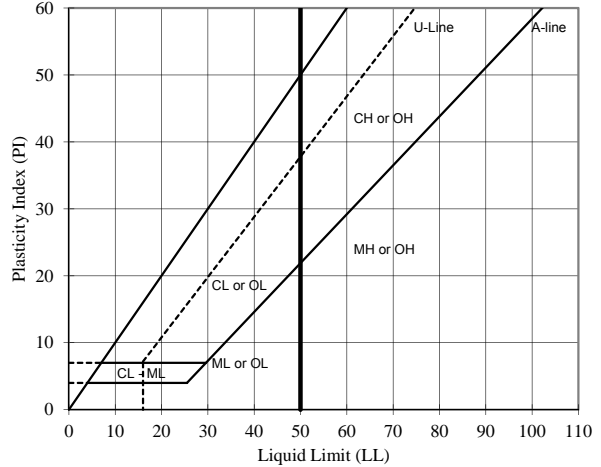
### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Flume** DEPTH (ft): **Slime**  
 TYPE: **Single Drain**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		100.0
1.5-inch	37.5		100.0
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.02
#40	0.425		
#60	0.25	Fine Sand	43.29
#100	0.15		
#200	0.075		
0.034	36.4		
0.022	29.3	Silt or Clay Fines	56.69
0.013	23.2		
0.009	19.2		
0.007	16.7		
0.003	10.6		
0.001	7.1		



Visual Description (Golder Procedure):  
**Wet, light yellowish brown SITLY SAND**

LL	PL	PI	SpG (assumed)
--	--	--	2.64

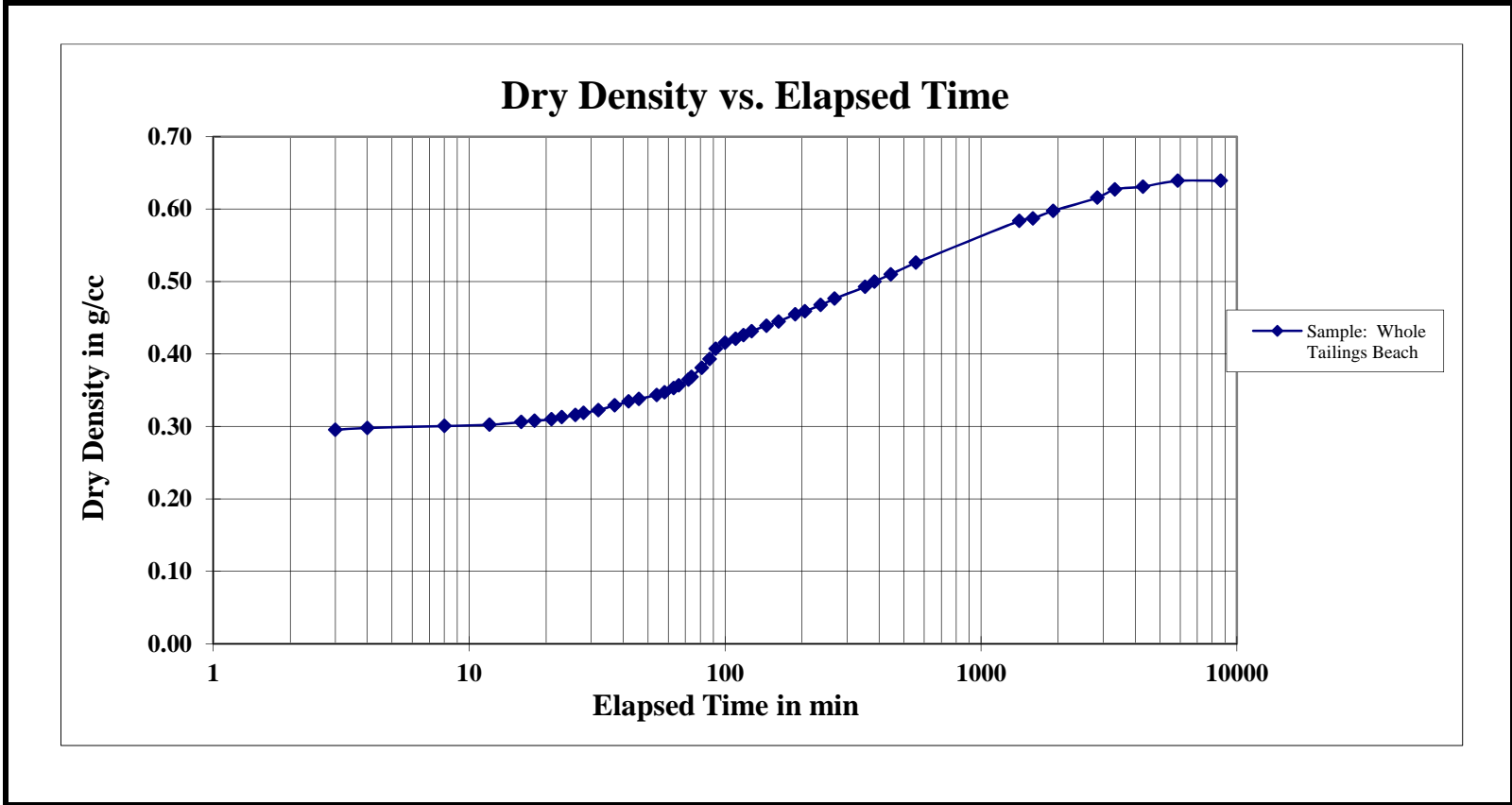
As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 --

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH: RJM/SRS  
 DATE: 11/8/2012  
 REVIEW: MB





<b>Golder Associates, Inc. Denver, Colorado</b>			Title: <b>SEDIMENTATION TESTING GRAPHICAL DATA</b>		
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample No. <b>Whole Tailings Beach</b>	System <b>Single Drain</b>	Reviewed: MB	Date: 23-Oct-12	Job Number: 103-92557	Figure: 2



**APPENDIX C**  
**FSCONSOL MODEL OUTPUT**

<b>Date:</b>	November 11, 2013	<b>Made by:</b>	CDJ and DMW
<b>Project No.:</b>	133-92505	<b>Checked by:</b>	GM
<b>Subject:</b>	Cyclone Overflow Beach and Slimes Tailings Consolidation Analyses	<b>Reviewed by:</b>	GG
<b>Project Short Title:</b>	<b>COPPER FLAT FEASIBILITY DESIGN</b>		

## 1.0 OBJECTIVE

Estimate the Tailing Storage Facility (TSF) impoundment capacity by computing the average dry density of tailings at the end-of-filling using a one-dimensional (1D) large strain consolidation model to obtain the void ratio profile with depth. Use calculated 1D percent errors to compute an adjusted average dry density, which is then weighted based on the tailings cyclone overflow split between tailings “beach” versus tailings “slimes” within the TSF impoundment.

## 2.0 ASSUMPTIONS

- Total tailings production rate (overflow and underflow): 27,618 tons per day
- Total tailings capacity (overflow and underflow): 112 million tons
- Cyclone Availability: 100 percent (i.e. no whole tailings deposited in the TSF impoundment based on the recommendation to produce as much cyclone underflow tailings sand as possible)
- Percentage split of cyclone overflow tailings: 54.75 percent of total
- Tailings production rate (overflow): 15,121 tons per day (calculated based on total tailings production rate and percentage of cyclone overflow)
- Percentage split of slimes: 40 percent by mass of storage in TSF impoundment
- Initial Tailings Overflow Solids Content: 25.9 percent
- Tailings Overflow filling rate
  - Years 0-1:13.3 million kilograms per day
  - Years 1-5:15.4 million kilograms per day
  - Years 5-11:14.2 million kilograms per day
  - Years 11-11.12:13.2 million kilograms per day
- Impermeable Bottom Boundary condition

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Tucson, AZ 85705 USA

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### 3.0 INPUTS

#### 3.1 Geometry

Model geometry was based on the existing ground topography (THEMAC Resources, 2011) and Golder's optimized TSF design. Stage storage curve data used for TSF modeling is displayed in Table 1.

- Bottom Elevation: 5,196 feet above mean sea level (ft-msl)
- Top Dam Crest Elevation: 5,450 ft-msl
- Maximum depth of Impoundment: 254 feet
- Maximum Overflow Impoundment Area: 13.66 million square feet
- Impoundment volume used in the FSConsol analyses: 96.88 million cubic yards (MCY)

**Table 1: Elevation–Area-Volume Relationship**

Elevation (ft-msl)	Height (ft)	Area (acre)	Cumulative Volume (yd <sup>3</sup> )
5,196	0	0.0	0
5,210	14	16.3	367,859
5,225	29	58.6	1,786,931
5,235	39	101.7	3,427,369
5,250	54	141.8	6,859,887
5,265	69	180.3	11,223,775
5,280	84	203.4	16,145,709
5,295	99	240.6	21,968,132
5,310	114	255.1	28,141,570
5,325	129	280.1	34,919,345
5,340	144	288.9	41,911,903
5,355	159	300.0	49,171,933
5,370	174	305.0	56,552,360
5,385	189	308.6	64,019,422
5,400	204	312.6	71,583,431
5,425	229	313.6	84,231,795
5,435	239	313.6	89,291,466
5,450	254	313.6	96,880,975

#### 3.2 Material Properties

Material properties and numerical model inputs are based on laboratory tests performed by Golder Associates Inc. in Denver, CO. Slurry consolidation tests were performed to determine void ratio and permeability versus applied effective stresses. The specific gravity of the tailings material is 2.65 based on laboratory tests.

## 4.0 METHOD

Calculations were performed using the computer program FSConsol (GWP Software, 1999), which performs a one-dimensional, large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967).

### 4.1 Numerical Model

#### 4.1.1 Equations

For modeling purposes, it is convenient to express constitutive relationships (permeability and compressibility, respectively) in a closed form. Non-linear relationships are proposed by Abu-Hejleh and Znidarcic (1994 and 1996), defined by Equations 4.1 and 4.2, which are used in consolidation and desiccation numerical models.

$$k = C e^D \quad \text{Equation 4.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 4.2}$$

When using FSConsol model, Equation 4.1 remains the same; however, Equation 4.2 is rewritten by the modified power law form to represent compressibility, shown in Equation 4.3.

$$e = A(\sigma')^B + M \quad \text{Equation 4.3}$$

In the above relationships,  $e$  is the void ratio,  $\sigma'$  is the effective stress, and  $k$  is hydraulic conductivity functionally dependent on void ratio.

#### 4.1.2 Parameters

The five material parameters,  $A$ ,  $B$ ,  $C$ ,  $D$ , and  $M$  (or  $Z$ ), were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 through 4, which are on the following next two pages (pages 4 and 5). The fitted parameters calculated for those constitutive relationships are shown in Tables 2 and 3 for different systems of units.

**Table 2: Permeability Input Parameters for Cyclone Overflow and Whole Tailings**

Sample	C (centimeters per second)	D (dimensionless) <sup>1</sup>
Slimes	1.380x10 <sup>-7</sup>	3.353
Beach	1.523x10 <sup>-6</sup>	3.035

**Table 3: Compressibility Input Parameters for Cyclone Overflow and Whole Tailings**

Sample	A (1/kilopascals) <sup>B</sup>	B (dimensionless) <sup>1</sup>	M (dimensionless) <sup>1</sup>
Slimes	3.144	-0.1952	-0.1424
Beach	1.787	-0.2983	0.5224

Note:

<sup>1</sup> B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second (cgs) units

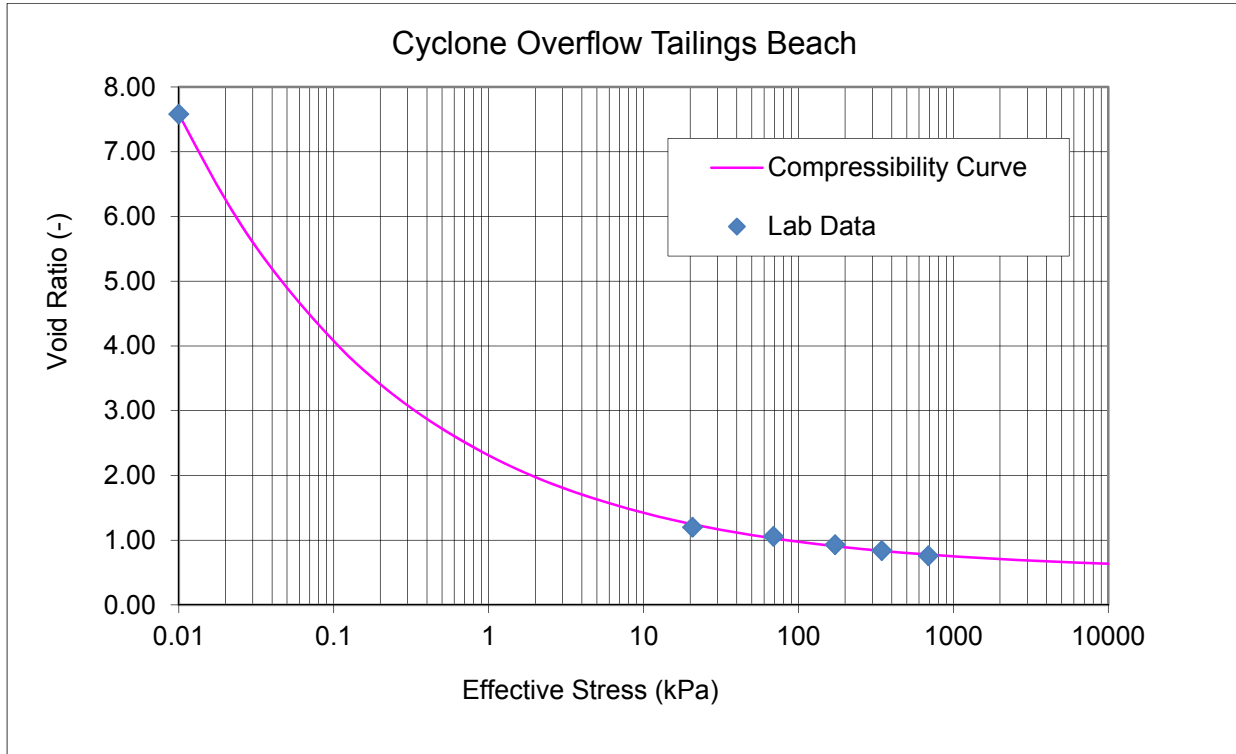


Figure 1: Cyclone Overflow Tailings Beach Compressibility Constitutive Relationship versus Lab Data

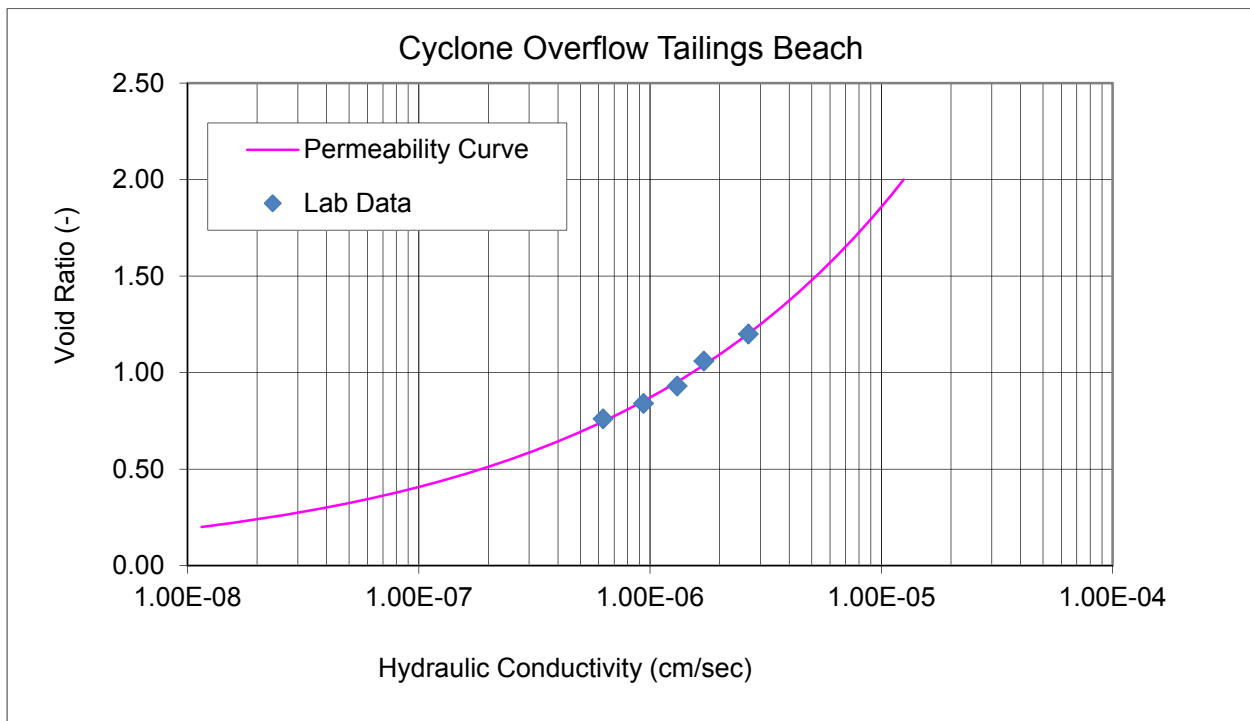


Figure 2: Cyclone Overflow Tailings Beach Permeability Constitutive Relationship versus Lab Data

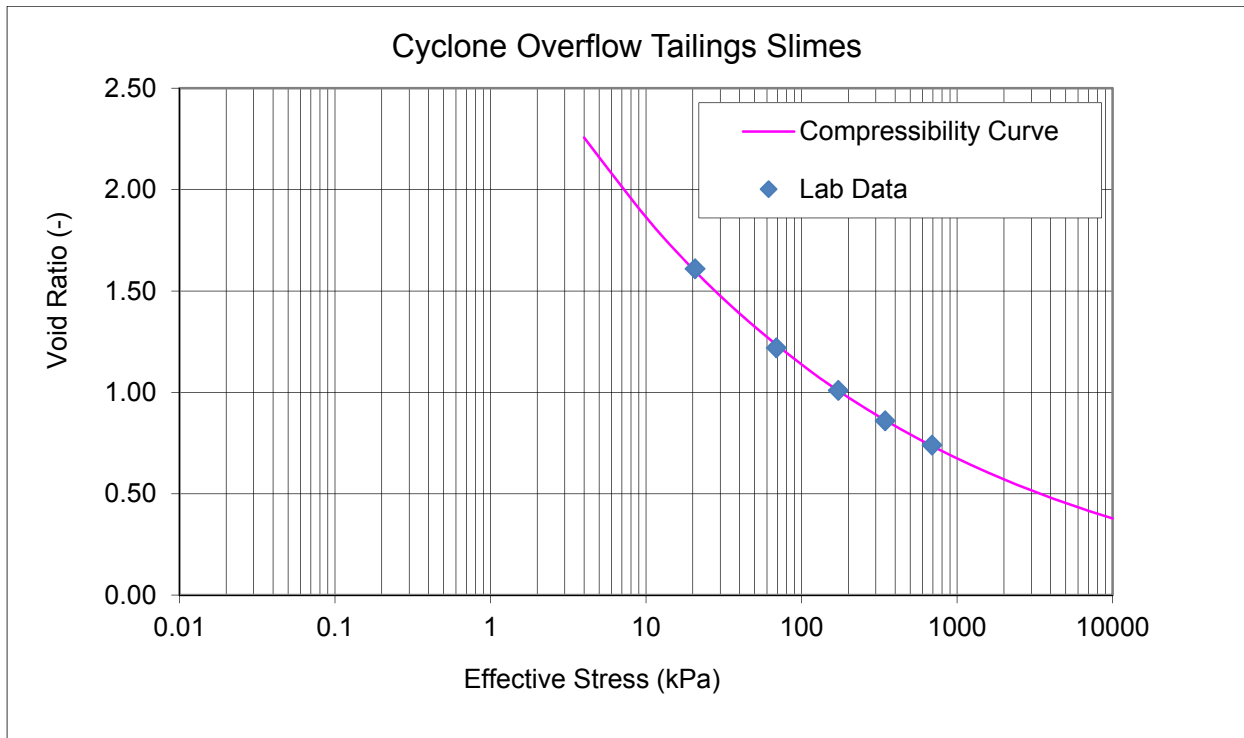


Figure 3: Cyclone Overflow Tailings Slimes Compressibility Constitutive Relationship versus Lab Data

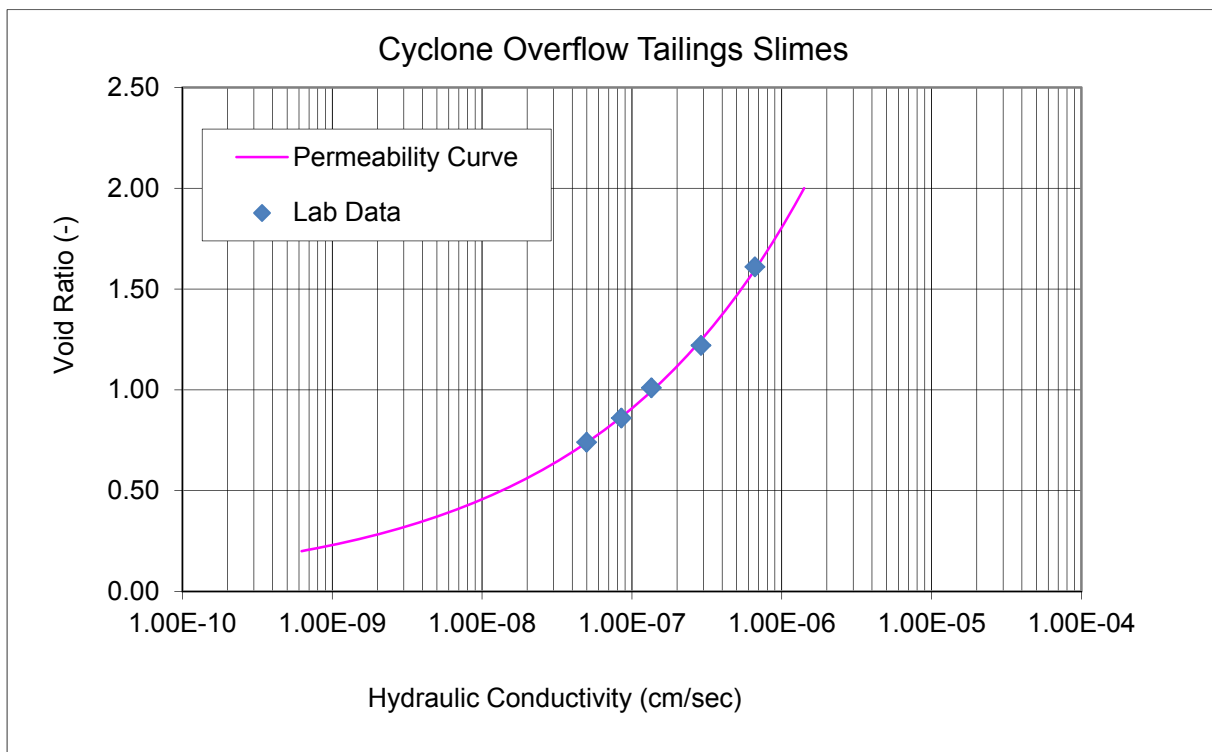


Figure 4: Cyclone Overflow Tailings Slimes Permeability Constitutive Relationship versus Lab Data

## 4.2 Procedure

The tailings cyclone overflow portion of the TSF was modeled by running a consolidation analysis on a single tailings column located in the deepest part of the impoundment. Using the parameters described above in Section 4.1.2 to define the compressibility and permeability relationships, separate analyses were performed for each material representing either the tailings beach or tailings slimes components of the cyclone overflow.

In each respective model run, the model analyzed each material component assuming it to represent 100 percent of the inflow into the model storage volume (i.e. the impoundment). In the model, the incremental area was calculated based on elevation and volume differences with Golder's TSF design volumes determined using AutoCAD Civil 3D (2013). The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities from the final void ratio profile of each tailings component to obtain an overall density that is weighted based on laboratory grain-size distribution results, which is detailed further in Section 4.2.1. Furthermore, the calculated densities based on the FSConsol analyses are adjusted for 1D percent errors as discussed in Section 4.2.2.

### 4.2.1 Overflow Beach/Slimes Split

Grain-size distribution tests were performed by Golder in the Denver, CO laboratory on tailings beach and slime samples from laboratory flume tests and on total overflow tailings before the flume tests. From the grain-size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the tailings beach and tailings slime that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the tailings beach and tailings slimes samples should equate to the percentage for the original cyclone overflow head sample. Table 4 depicts the results of the laboratory test and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes).

**Table 4: Laboratory and Split Results**

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids by Weight	% Sand of Total Overflow	% Fines of Total Overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head	9.63	90.37	100	9.63	90.37

### 4.2.2 1D Percent Error

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall geometry of the TSF (i.e. "bowl" shaped), and that mass and volume quantities are larger towards the top than at the bottom of the TSF due to an increase in surface area. The approximated average dry density of each tailings component is corrected based on the calculated average percent error between 1D versus three-dimensional (3D) by checking: height of solids, volume of solids, and mass of solids.



Based on Golder's experience, 1D analysis resulting in less than an average 15 percent error reasonably depicts expected consolidation in the field, and 3D impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error.

## 5.0 RESULTS

The average dry density generally increases during filling until the ultimate height of tailings is reached. At closure, the tailings density continues to increase as the consolidation continues. If 100 percent of the inflow into the TSF impoundment is represented by the cyclone overflow tailings slimes samples from the tail section of the flume (i.e., the finest fraction of the tailings), FSConsol predicts that the final average density of the slimes fraction over the modeled profile will be approximately 33.55 pcf. With 100 percent of the inflow representative of cyclone overflow tailings beach materials, FSConsol predicts a final average tailings density of approximately 76.65 pcf. Attachment 1 contains FSConsol outputs for each respective material properties used in the consolidation analysis.

Because a 1D model was used, percent error calculations were performed to account for 3D effects. The error is expected to be greater for cone-shaped impoundments than for flat-based (i.e. bowl shaped) impoundments, and greater with higher rates of rise than lower rates of rise. Based on calculated material properties, the TSF geometry, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1.1 to 13.7 percent with an average error ranging between 2.7 to 5.4 percent when considering calculated impoundment capacities. Attachment 2 contains the error calculations for each respective cyclone overflow tailings components.

After accounting for 3D characteristics of the impoundment geometry, the recommended dry density of each tailings component is 31.7 and 74.6 pcf, respectively. In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components.

## 6.0 CONCLUSIONS

Based on model results and error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow tailings slimes and cyclone overflow tailings beach materials at the end-of-filling. Capacity has been estimated with 40 percent of the total represented by tailings slime materials, which is based on the calculated beach to slime ratio split discussed previously in Section 4.2.1. Therefore without consideration of 3D and managed deposition effects, the calculated weighted density of tailings slimes and tailings beach based on each component's volume within the TSF impoundment is 48.4 pcf. At this density, the TSF will require approximately 93.8 MCY of storage, which can be achieved by building to an ultimate tailings surface elevation of 5450 ft-msl and an ultimate cyclone underflow tailings sand dam crest elevation of 5460 ft-msl.

A summary for the cyclone overflow tailings beach and slimes components is located in Attachment 3, which contains the percent errors, adjusted densities, corresponding volumes, and the weighted density based on the percentage of the overall volume occupied by that tailings materials.

3D and managed deposition effects could potentially increase the post deposition density to reduce storage volume requirements. On average, a density increase in 5 pcf results in approximately 10 MCY of additional TSF impoundment storage capacity and a decrease in ultimate crest elevation by approximately 20 feet.

Actual flows into the impoundment will be mixed, with cyclone overflow tailings beach inter-fingered with slimes layers as the decant pond migrates upstream away from the embankment and points of cyclone overflow discharge are relocated. The denser and more permeable beach layers will increase the rate of consolidation in the underlying slimes material, resulting in a higher rate of consolidation than that calculated in the model. In addition, the consolidation modeling does not account for managed deposition of tailings. Thin-lift, sub-aerial deposition techniques will result in increases of dry density and a decrease in required storage volume by enhancing evaporation of exposed tailings. Managed deposition will increase consolidation of both the cyclone overflow tailings beach and slimes. However due to the height of the proposed tailings embankment and the design out-slopes, an embankment raise of several feet will also be feasible near the end of the mining operation if additional capacity is needed.

## 7.0 REFERENCES

- Abu-Hejleh, A.N., and D. Znidarcic, 1994. *Estimation of the Consolidation Constitutive Relations*, Computer Methods and Advances in Geomechanics, Siriwardane and Zaman (eds), Balkema, Rotterdam, pp. 499-504.
- Abu-Hejleh, A.N., and D. Znidarcic, 1996. *Consolidation Characteristics of Phosphatic Clays*, Journal of Geotechnical Engineering", ASCE, New-York, Vol. 122, No. 4, pp. 295-301.
- AutoCAD Civil 3D Package, Version 2013.
- Gibson, R.E., England, G. L., and Hussey, M. J. L. (1967). The Theory of One-Dimensional Consolidation of Saturated Clays. *Geotechnique*, 17: 261-273.
- GWP Software, 1999. *FSConsol User's Manual*, GWP Software Inc.
- THEMAC Resources, 2011. *2-Foot Topography*. Developed. and provided by Cooper Aerial Survey Co., June 18, 2011.

Attachments: Attachment 1- FSConsol Model Output  
Attachment 2 - FSConsol Error Checking  
Attachment 3 - FSConsol Model Summary

**ATTACHMENT 1  
FSCONSOL MODEL OUTPUT**

FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.  
Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 12:58:34  
input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-slimes.fs1

Overflow - slimes (Non-thickened)  
Revised flow rates and pond areas 11/11/2013  
CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond  
Analysis Time = 16.115 years  
Maximum Pond Height = 77.42 meters  
Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 3.144; B = -0.1952 ; Mconst = -0.1424

Permeability Parameters (cm/second)

C = 1.38e-07; D = 3.353

#### BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

#### NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

#### OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	8.952323	27.665	99.338	0.662	4.84E+08	32.2474
73	11.974489	28.534	99.338	0.662	9.69E+08	23.1181
109.5	13.962894	29.275	99.29	0.71	1.45E+09	23.1181
146	15.951077	29.826	99.268	0.732	1.94E+09	23.1181
182.5	17.505217	30.356	99.233	0.767	2.42E+09	18.1828
219	18.909844	30.834	99.199	0.801	2.91E+09	18.1828
255.5	20.314418	31.243	99.176	0.824	3.39E+09	18.1828
292	21.599814	31.627	99.153	0.847	3.87E+09	16.1217
328.5	22.7607	31.998	99.128	0.872	4.36E+09	16.1217
365	23.92199	32.331	99.109	0.891	4.84E+09	18.7619
401.5	25.394898	32.553	99.113	0.887	5.41E+09	18.7619
438	26.573655	32.822	99.103	0.897	5.97E+09	15.86
474.5	27.703532	33.079	99.093	0.907	6.53E+09	15.86
511	28.833398	33.316	99.085	0.915	7.10E+09	15.86
547.5	29.963253	33.533	99.08	0.92	7.66E+09	15.86
584	31.006515	33.755	99.073	0.927	8.23E+09	14.9583
620.5	32.029764	33.966	99.066	0.934	8.79E+09	14.9583
657	33.053007	34.164	99.06	0.94	9.35E+09	14.9583
693.5	34.076244	34.349	99.056	0.944	9.92E+09	14.9583
730	35.045301	34.535	99.051	0.949	1.05E+10	13.6253
766.5	35.910971	34.735	99.042	0.958	1.10E+10	13.6253
803	36.776635	34.924	99.035	0.965	1.16E+10	13.6253
839.5	37.642296	35.105	99.028	0.972	1.22E+10	13.6253
876	38.507953	35.276	99.023	0.977	1.27E+10	13.6253
912.5	39.37049	35.441	99.018	0.982	1.33E+10	13.207
949	40.186694	35.608	99.012	0.988	1.39E+10	13.207
985.5	41.002895	35.768	99.007	0.993	1.44E+10	13.207
1022	41.819093	35.922	99.002	0.998	1.50E+10	13.207
1058.5	42.635291	36.069	98.998	1.002	1.56E+10	13.207
1095	43.4515	36.211	98.995	1.005	1.61E+10	13.207
1131.5	44.241255	36.353	98.991	1.009	1.67E+10	12.7196
1168	45.000136	36.496	98.987	1.013	1.72E+10	12.7196
1204.5	45.759528	36.633	98.982	1.018	1.78E+10	12.7196
1241	46.519832	36.766	98.979	1.021	1.84E+10	12.7196
1277.5	47.281436	36.893	98.975	1.025	1.89E+10	12.7196
1314	48.04463	37.015	98.972	1.028	1.95E+10	12.7196
1350.5	48.798638	37.134	98.969	1.031	2.01E+10	12.5125
1387	49.541361	37.25	98.966	1.034	2.06E+10	12.5125
1423.5	50.286082	37.362	98.963	1.037	2.12E+10	12.5125
1460	51.032752	37.47	98.961	1.039	2.18E+10	12.5125
1496.5	51.7813	37.573	98.959	1.041	2.23E+10	12.5125

1533	52.53164	37.672	98.957	1.043	2.29E+10	12.5125
1569.5	53.278167	37.768	98.956	1.044	2.34E+10	12.3672
1606	54.015293	37.863	98.954	1.046	2.40E+10	12.3672
1642.5	54.754054	37.954	98.953	1.047	2.46E+10	12.3672
1679	55.494353	38.041	98.952	1.048	2.51E+10	12.3672
1715.5	56.236103	38.126	98.951	1.049	2.57E+10	12.3672
1752	56.979224	38.207	98.951	1.049	2.63E+10	12.3672
1788.5	57.720815	38.286	98.951	1.049	2.68E+10	12.2088
1825	58.448575	38.365	98.95	1.05	2.74E+10	11.2472
1861.5	59.070262	38.461	98.946	1.054	2.79E+10	11.2472
1898	59.694243	38.553	98.943	1.057	2.84E+10	11.2472
1934.5	60.319981	38.642	98.94	1.06	2.89E+10	11.2472
1971	60.947381	38.728	98.937	1.063	2.95E+10	11.2472
2007.5	61.576356	38.812	98.934	1.066	3.00E+10	11.2472
2044	62.206642	38.893	98.932	1.068	3.05E+10	11.21
2080.5	62.834453	38.972	98.929	1.071	3.10E+10	11.21
2117	63.463654	39.049	98.927	1.073	3.15E+10	11.21
2153.5	64.094161	39.124	98.926	1.074	3.21E+10	11.21
2190	64.725911	39.197	98.924	1.076	3.26E+10	11.21
2226.5	65.358846	39.268	98.923	1.077	3.31E+10	11.21
2263	65.992908	39.337	98.921	1.079	3.36E+10	11.21
2299.5	66.628047	39.404	98.92	1.08	3.41E+10	11.21
2336	67.264213	39.469	98.919	1.081	3.47E+10	11.21
2372.5	67.90136	39.532	98.919	1.081	3.52E+10	11.21
2409	68.539445	39.594	98.918	1.082	3.57E+10	11.21
2445.5	69.178426	39.654	98.918	1.082	3.62E+10	11.21
2482	69.818263	39.713	98.917	1.083	3.67E+10	11.2094
2518.5	70.458862	39.77	98.917	1.083	3.73E+10	11.2094
2555	71.100248	39.826	98.917	1.083	3.78E+10	11.2094
2591.5	71.742389	39.88	98.917	1.083	3.83E+10	11.2094
2628	72.385254	39.933	98.917	1.083	3.88E+10	11.2094
2664.5	73.028814	39.985	98.917	1.083	3.93E+10	11.2094
2701	73.67304	40.036	98.918	1.082	3.99E+10	11.2094
2737.5	74.317906	40.085	98.918	1.082	4.04E+10	11.2094
2774	74.963389	40.134	98.919	1.081	4.09E+10	11.2094
2810.5	75.609464	40.181	98.919	1.081	4.14E+10	11.2094
2847	76.256109	40.227	98.92	1.08	4.19E+10	11.2094
2883.5	76.903302	40.272	98.921	1.079	4.24E+10	11.2094













Table with 4 columns: Row ID, Column 1, Column 2, Column 3. The table contains numerical data for 2883 rows and 3 columns.

FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.  
Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 14:40:56  
input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-beach.fs1

Overflow - beach (Non-thickened)  
Revised flow rates and pond areas 11/11/2013  
CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond  
Analysis Time = 16.115 years  
Maximum Pond Height = 77.42 meters  
Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 1.787; B = -0.2983 ; Mconst = 0.5224

Permeability Parameters (cm/second)

C = 1.523e-06; D = 3.035

#### BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

#### NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

#### OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	6.098115	39.528	96.536	3.464	4.84E+08	55.9199
73	8.737703	43.656	95.973	4.027	9.69E+08	55.9199
109.5	9.507187	47.682	94.382	5.618	1.45E+09	32.2474
146	10.476951	50.081	93.328	6.672	1.94E+09	32.2474
182.5	11.558634	51.655	92.671	7.329	2.42E+09	32.2474
219	12.236617	53.232	91.655	8.345	2.91E+09	23.1181
255.5	12.810342	54.565	90.672	9.328	3.39E+09	23.1181
292	13.436283	55.614	89.868	10.132	3.87E+09	23.1181
328.5	14.096505	56.462	89.212	10.788	4.36E+09	23.1181
365	14.780841	57.164	88.681	11.319	4.84E+09	26.904
401.5	15.714982	57.566	88.64	11.36	5.41E+09	26.904
438	16.566093	58.003	88.43	11.57	5.97E+09	21.1605
474.5	17.159336	58.556	87.926	12.074	6.53E+09	21.1605
511	17.773119	59.023	87.525	12.475	7.10E+09	21.1605
547.5	18.397756	59.433	87.186	12.814	7.66E+09	21.1605
584	19.030285	59.798	86.897	13.103	8.23E+09	21.1605
620.5	19.668975	60.125	86.649	13.351	8.79E+09	21.1605
657	20.312613	60.421	86.438	13.562	9.35E+09	21.1605
693.5	20.960293	60.689	86.258	13.742	9.92E+09	21.1605
730	21.488683	61.011	85.902	14.098	1.05E+10	18.7619
766.5	22.017166	61.298	85.604	14.396	1.10E+10	18.7619
803	22.551957	61.559	85.34	14.66	1.16E+10	18.7619
839.5	23.091308	61.8	85.105	14.895	1.22E+10	18.7619
876	23.634322	62.023	84.893	15.107	1.27E+10	18.7619
912.5	24.180393	62.229	84.703	15.297	1.33E+10	18.7619
949	24.729068	62.422	84.532	15.468	1.39E+10	18.7619
985.5	25.279985	62.603	84.378	15.622	1.44E+10	18.7619
1022	25.762592	62.813	84.108	15.892	1.50E+10	15.86
1058.5	26.168928	63.039	83.774	16.226	1.56E+10	15.86
1095	26.583726	63.244	83.478	16.522	1.61E+10	15.86
1131.5	27.003419	63.436	83.204	16.796	1.67E+10	15.86
1168	27.426795	63.616	82.95	17.05	1.72E+10	15.86
1204.5	27.853193	63.785	82.711	17.289	1.78E+10	15.86
1241	28.282173	63.946	82.487	17.513	1.84E+10	15.86
1277.5	28.713408	64.098	82.277	17.723	1.89E+10	15.86
1314	29.146638	64.242	82.08	17.92	1.95E+10	15.86
1350.5	29.581652	64.38	81.894	18.106	2.01E+10	15.86
1387	30.018268	64.511	81.72	18.28	2.06E+10	15.86
1423.5	30.42497	64.651	81.498	18.502	2.12E+10	14.9583
1460	30.82099	64.787	81.28	18.72	2.18E+10	14.9583
1496.5	31.21996	64.915	81.079	18.921	2.23E+10	14.9583



1533	31.620997	65.037	80.889	19.111	2.29E+10	14.9583
1569.5	32.023724	65.154	80.709	19.291	2.34E+10	14.9583
1606	32.42791	65.266	80.539	19.461	2.40E+10	14.9583
1642.5	32.833389	65.373	80.378	19.622	2.46E+10	14.9583
1679	33.24003	65.477	80.225	19.775	2.51E+10	14.9583
1715.5	33.647726	65.576	80.079	19.921	2.57E+10	14.9583
1752	34.056384	65.671	79.941	20.059	2.63E+10	14.9583
1788.5	34.465926	65.764	79.81	20.19	2.68E+10	14.9583
1825	34.852455	65.864	79.636	20.364	2.74E+10	12.5521
1861.5	35.144936	65.994	79.325	20.675	2.79E+10	12.5521
1898	35.447468	66.11	79.059	20.941	2.84E+10	12.5521
1934.5	35.754018	66.22	78.81	21.19	2.89E+10	12.5521
1971	36.063325	66.325	78.572	21.428	2.95E+10	12.5521
2007.5	36.374798	66.426	78.345	21.655	3.00E+10	12.5521
2044	36.688086	66.523	78.126	21.874	3.05E+10	12.5521
2080.5	37.002952	66.616	77.914	22.086	3.10E+10	12.5521
2117	37.319222	66.706	77.71	22.29	3.15E+10	12.5521
2153.5	37.636758	66.793	77.513	22.487	3.21E+10	12.5521
2190	37.955451	66.877	77.322	22.678	3.26E+10	12.5521
2226.5	38.275209	66.958	77.137	22.863	3.31E+10	12.5521
2263	38.59595	67.038	76.958	23.042	3.36E+10	12.5521
2299.5	38.917606	67.114	76.785	23.215	3.41E+10	12.5521
2336	39.240116	67.189	76.617	23.383	3.47E+10	12.5521
2372.5	39.548675	67.267	76.425	23.575	3.52E+10	12.1668
2409	39.855497	67.342	76.241	23.759	3.57E+10	12.1668
2445.5	40.163729	67.415	76.064	23.936	3.62E+10	12.1668
2482	40.473023	67.485	75.893	24.107	3.67E+10	12.1668
2518.5	40.783223	67.553	75.728	24.272	3.73E+10	12.1668
2555	41.094232	67.62	75.567	24.433	3.78E+10	12.1668
2591.5	41.405978	67.685	75.411	24.589	3.83E+10	12.1668
2628	41.718406	67.748	75.26	24.74	3.88E+10	12.1668
2664.5	42.031468	67.809	75.113	24.887	3.93E+10	12.1668
2701	42.345125	67.869	74.971	25.029	3.99E+10	12.1668
2737.5	42.65934	67.927	74.832	25.168	4.04E+10	12.1668
2774	42.974082	67.984	74.698	25.302	4.09E+10	12.1668
2810.5	43.28932	68.04	74.567	25.433	4.14E+10	12.1668
2847	43.605029	68.094	74.44	25.56	4.19E+10	12.1668
2883.5	43.918571	68.149	74.309	25.691	4.24E+10	11.7178
2920	44.213906	68.207	74.151	25.849	4.30E+10	11.7178
2956.5	44.511467	68.263	74.005	25.995	4.35E+10	11.7178
2993	44.810075	68.317	73.864	26.136	4.40E+10	11.7178
3029.5	45.109458	68.369	73.728	26.272	4.45E+10	11.7178
3066	45.409486	68.42	73.596	26.404	4.50E+10	11.7178
3102.5	45.710079	68.47	73.468	26.532	4.56E+10	11.7178
3139	46.01118	68.519	73.344	26.656	4.61E+10	11.7178
3175.5	46.312747	68.567	73.222	26.778	4.66E+10	11.7178
3212	46.614745	68.614	73.105	26.895	4.71E+10	11.7178

3248.5	46.917146	68.659	72.99	27.01	4.76E+10	11.7178
3285	47.219924	68.704	72.878	27.122	4.82E+10	11.7178
3321.5	47.523058	68.748	72.769	27.231	4.87E+10	11.7178
3358	47.826528	68.791	72.663	27.337	4.92E+10	11.7178
3394.5	48.130317	68.833	72.559	27.441	4.97E+10	11.7178
3431	48.434408	68.875	72.458	27.542	5.02E+10	11.7178
3467.5	48.730334	68.918	72.344	27.656	5.08E+10	11.527
3504	49.026746	68.96	72.236	27.764	5.13E+10	11.527
3540.5	49.323742	69.001	72.131	27.869	5.18E+10	11.527
3577	49.621177	69.04	72.029	27.971	5.23E+10	11.527
3613.5	49.918983	69.08	71.93	28.07	5.28E+10	11.527
3650	50.21712	69.118	71.834	28.166	5.34E+10	11.527
3686.5	50.515559	69.156	71.74	28.26	5.39E+10	11.527
3723	50.814276	69.193	71.649	28.351	5.44E+10	11.527
3759.5	51.113254	69.229	71.56	28.44	5.49E+10	11.527
3796	51.412476	69.264	71.473	28.527	5.54E+10	11.527
3832.5	51.71193	69.3	71.388	28.612	5.60E+10	11.527
3869	52.011603	69.334	71.305	28.695	5.65E+10	11.527
3905.5	52.311483	69.368	71.224	28.776	5.70E+10	11.527
3942	52.611561	69.401	71.146	28.854	5.75E+10	11.527
3978.5	52.911828	69.434	71.069	28.931	5.80E+10	11.527
4015	53.20818	69.467	70.986	29.014	5.85E+10	10.5821
4051.5	53.463995	69.511	70.833	29.167	5.90E+10	10.5821
4088	53.301506	69.666	69.884	30.116	5.91E+10	0
4124.5	53.129642	69.794	69.049	30.951	5.91E+10	0
4161	52.978348	69.906	68.274	31.726	5.91E+10	0
4197.5	52.839417	70.01	67.527	32.473	5.91E+10	0
4234	52.709426	70.107	66.798	33.202	5.91E+10	0
4270.5	52.586503	70.199	66.079	33.921	5.91E+10	0
4307	52.469474	70.287	65.368	34.632	5.91E+10	0
4343.5	52.357536	70.372	64.661	35.339	5.91E+10	0
4380	52.250103	70.453	63.958	36.042	5.91E+10	0
4416.5	52.14673	70.531	63.257	36.743	5.91E+10	0
4453	52.047066	70.607	62.558	37.442	5.91E+10	0
4489.5	51.950825	70.68	61.86	38.14	5.91E+10	0
4526	51.857772	70.751	61.164	38.836	5.91E+10	0
4562.5	51.767705	70.82	60.468	39.532	5.91E+10	0
4599	51.680452	70.887	59.773	40.227	5.91E+10	0
4635.5	51.59586	70.952	59.079	40.921	5.91E+10	0
4672	51.513796	71.015	58.385	41.615	5.91E+10	0
4708.5	51.43414	71.076	57.693	42.307	5.91E+10	0
4745	51.356784	71.136	57.001	42.999	5.91E+10	0
4781.5	51.281628	71.194	56.31	43.69	5.91E+10	0
4818	51.208581	71.25	55.62	44.38	5.91E+10	0
4854.5	51.137561	71.305	54.932	45.068	5.91E+10	0
4891	51.068488	71.359	54.244	45.756	5.91E+10	0
4927.5	51.001291	71.411	53.559	46.441	5.91E+10	0

4964	50.9359	71.462	52.875	47.125	5.91E+10	0
5000.5	50.872254	71.512	52.193	47.807	5.91E+10	0
5037	50.810289	71.56	51.513	48.487	5.91E+10	0
5073.5	50.749951	71.607	50.836	49.164	5.91E+10	0
5110	50.691184	71.653	50.16	49.84	5.91E+10	0
5146.5	50.633936	71.698	49.488	50.512	5.91E+10	0
5183	50.578159	71.742	48.818	51.182	5.91E+10	0
5219.5	50.523806	71.784	48.151	51.849	5.91E+10	0
5256	50.470832	71.826	47.488	52.512	5.91E+10	0
5292.5	50.419194	71.867	46.827	53.173	5.91E+10	0
5329	50.368851	71.906	46.17	53.83	5.91E+10	0
5365.5	50.319763	71.945	45.517	54.483	5.91E+10	0
5402	50.271894	71.983	44.867	55.133	5.91E+10	0
5438.5	50.225206	72.02	44.221	55.779	5.91E+10	0
5475	50.179665	72.056	43.58	56.42	5.91E+10	0
5511.5	50.135237	72.091	42.942	57.058	5.91E+10	0
5548	50.09189	72.125	42.309	57.691	5.91E+10	0
5584.5	50.049592	72.159	41.68	58.32	5.91E+10	0
5621	50.008314	72.192	41.056	58.944	5.91E+10	0
5657.5	49.968027	72.224	40.437	59.563	5.91E+10	0
5694	49.928703	72.255	39.822	60.178	5.91E+10	0
5730.5	49.890315	72.286	39.213	60.787	5.91E+10	0
5767	49.852837	72.315	38.608	61.392	5.91E+10	0
5803.5	49.816243	72.345	38.008	61.992	5.91E+10	0
5840	49.780511	72.373	37.414	62.586	5.91E+10	0
5876.5	49.745615	72.401	36.825	63.175	5.91E+10	0

Observation Point Data

Number of Observation Points: 50
Number of Observation Point Readings: 162
Observation Point Time Step: 36.50 days

EXCESS PORE PRESSURE (kPa)

Time Height from Bottom (metres)

Table with columns for days (0 to 366.5) and observation points (0 to 49). Each cell contains a numerical value representing excess pore pressure in kPa at a specific depth and time.

























**ATTACHMENT 2  
FSCONSOL ERROR CHECKING**



**1-DIMENSIONAL VERSUS 3-DIMENSIONAL  
PERCENT ERROR CALCULATION SUMMARY**

Parameters used for modeling						
H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

Check 1D Calc - Volume	
Compare volume based on filling time, production rate, & average density w/ stage-curve volume	
Time	2,883.50 days
Production	15,121.0 ton/day
Total mass	43,601,404 ton
Void Ratio	3.9303
Dry Density	33.540 pcf
Volume	2,599,975,881 ft^3
Cum Vol	2,615,786,325 ft^3
Error	-0.60%

Note: from FSConsol Time Step Data

Note: volume=Qm\*/rho\_dry

EL 5,448.31 Note: from Rate of Rise Ht vs. Cap Table

Material Properties	
Gs	2.65
A	5.6895 (1/psf)^B
B	-0.1952
M	-0.1424
e0	7.581
w	286.1%
s	25.9%
C	3.91E-04 ft/day
D	3.35

Production	
	15,121 t/day
	182,886 ft^3/day of dry solids
	1,569,366 ft^3/day of tailing
	2,883.50 days
	7.90 years

Check 1D Calc - height of solids				
Compare height of solids based on model inputs and based on the calculated void ratio profile				
Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	17.40	2.2121	4.49
2	29	36.04	0.6144	1.33
3	39	71.95	0.3543	1.48
4	54	157.93	0.2540	2.55
5	69	275.86	0.1998	2.75
6	84	407.95	0.1771	2.73
7	99	554.92	0.1497	2.56
8	114	718.78	0.1412	2.70
9	129	910.33	0.1286	2.87
10	144	1,115.33	0.1247	2.98
11	159	1,334.27	0.1201	3.06
12	174	1,557.63	0.1181	3.07
13	189	1,782.92	0.1168	3.07
14	204	2,042.42	0.1153	3.49
15	229	2,480.92	0.1149	5.87
16	239	2,654.21	0.1149	2.32
17	254	2,883.50	0.1149	3.07
Total				50.38
Hs_FSConsol				51.35
Error				1.93%

Note: based on e profile

**1-DIMENSIONAL VERSUS 3-DIMENSIONAL  
PERCENT ERROR CALCULATION SUMMARY**

**Check 1D calc - Mass**

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = 2,883.50

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	1.434	
2	5.080	1.533	2.044
3	10.160	1.645	1.962
4	14.000	1.740	1.426
5	15.240	1.771	0.450
6	20.320	1.911	1.788
7	25.400	2.064	1.700
8	29.000	2.180	1.153
9	30.480	2.227	0.462
10	35.560	2.397	1.534
11	39.000	2.513	0.996
12	40.639	2.569	0.463
13	45.719	2.740	1.390
14	50.799	2.908	1.328
15	54.000	3.010	0.809
16	55.879	3.069	0.465
17	60.959	3.225	1.225
18	66.039	3.373	1.182
19	69.000	3.455	0.671
20	71.119	3.514	0.472
21	76.199	3.649	1.109
22	81.279	3.778	1.078
23	84.000	3.844	0.566
24	86.359	3.901	0.484
25	91.439	4.020	1.024
26	96.519	4.134	1.001
27	99.000	4.187	0.481
28	101.599	4.243	0.498
29	106.678	4.349	0.959
30	111.758	4.452	0.941
31	114.000	4.496	0.409
32	116.838	4.552	0.514
33	121.918	4.648	0.907
34	126.998	4.742	0.892
35	129.000	4.778	0.348
36	132.078	4.834	0.530
37	137.158	4.923	0.864
38	142.238	5.010	0.851
39	144.000	5.039	0.292
40	147.318	5.095	0.547
41	152.398	5.178	0.828
42	157.478	5.260	0.817
43	159.000	5.284	0.243
44	162.558	5.340	0.564
45	167.637	5.418	0.796
46	172.717	5.495	0.787
47	174.000	5.514	0.197
48	177.797	5.570	0.580
49	182.877	5.644	0.769
50	187.957	5.717	0.760
51	189.000	5.732	0.155
52	193.037	5.789	0.597
53	198.117	5.859	0.744
54	203.197	5.929	0.737
55	204.000	5.939	0.116
56	208.277	5.997	0.614
57	213.357	6.064	0.723
58	218.437	6.131	0.716
59	223.517	6.196	0.709
60	228.597	6.261	0.703
61	229.000	6.266	0.056
62	233.677	6.324	0.641
63	238.757	6.387	0.691
64	239.000	6.390	0.033
65	243.836	6.449	0.652
66	248.916	6.510	0.679
67	254.000	7.581	0.632

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
Δ Height (ft)	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
Height (ft)	14	29	39	54	69	84	99	114	129
Hs (ft)	5.43	5.09	2.99	3.99	3.54	3.22	2.99	2.81	2.66
e_avg	1.58	1.95	2.34	2.76	3.23	3.65	4.02	4.34	4.64
rho_dry (pcf)	64.17	56.12	49.47	43.99	39.06	35.55	32.96	30.95	29.33
Vol (ft^3)	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
Mass (ton)	318,662	1,075,213	1,095,477	2,038,244	2,300,832	2,362,077	2,590,583	2,579,371	2,683,614

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
Δ Height (ft)	15.0	15.0	15.0	15.0	15.0	25.0	10.0	15.0
Height (ft)	144.0	159.0	174.0	189.0	204.0	229.0	239.0	254.0
Hs (ft)	2.54	2.43	2.34	2.26	2.19	3.52	1.36	1.96
e_avg	4.91	5.16	5.40	5.62	5.84	6.10	6.33	6.64
rho_dry (pcf)	27.98	26.83	25.84	24.97	24.19	23.28	22.57	21.64
Vol (ft^3)	188,799,066	196,020,810	199,271,529	201,610,674	204,228,243	341,505,828	136,611,117	204,916,743
Mass (ton)	2,641,449	2,630,075	2,574,663	2,516,815	2,470,154	3,975,111	1,541,374	2,217,043

Time	2,883.5	days
Production	15,121	ton/day
Total mass	43,601,404	ton
TDF - total mass	37,610,756	ton
Error (%)	13.74%	

Note: based on the time of filling and production rate  
Note: based on the integration of the void ratio profile

### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

#### Parameters used for modeling

H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

#### Check 1D calc - Volume

Compare volume based on filling time, production rate, & average density w/ stage-curve volume

Time	4,057.00	days
Production	15,121.0	ton/day
Total mass	61,345,897	ton
Void Ratio	1.1581	
Dry Density	76.623	pcf
Volume	1,601,244,549	ft^3
Cum Vol	1,547,514,922	ft^3
Error	3.47%	

Note: from FSConsol Time Step Data

Note: volume=Qm\*t/rho\_dry

Note: from Rate of Rise Ht vs. Cap Table

EL 5,371.53

#### Material Properties

Gs	2.65
A	4.4241 (1/psf)^B
B	-0.2983
M	0.5224
e0	7.581
w	286.1%
s	25.9%
C	4.32E-03 ft/day
D	3.04

#### Production

15,121	t/day
182,886	ft^3/day of dry solids
1,569,327	ft^3/day of tailing
4,057.00	days
11.12	years

#### Check 1D Calc - height of solids

Compare height of solids based on model inputs and based on the calculated void ratio profile

Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	25.54	2.2121	6.58
2	29	77.82	0.6144	3.74
3	39	200.19	0.3543	5.05
4	54	433.42	0.2540	6.90
5	69	698.40	0.1998	6.17
6	84	1,009.95	0.1771	6.43
7	99	1,401.09	0.1497	6.83
8	114	1,815.07	0.1412	6.81
9	129	2,345.37	0.1286	7.95
10	144	2,880.33	0.1247	7.77
11	159	3,434.57	0.1201	7.76
12	174	3,993.71	0.1181	7.70
13	189	4,057.00	0.1168	0.86
14	204	4,057.00	0.1153	0.00
15	229	4,057.00	0.1149	0.00
16	239	4,057.00	0.1149	0.00
17	254	4,057.00	0.1149	0.00
Total				80.56
Hs_FSConsol				81.47
Error				1.13%

Note: based on e profile

### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

**Check 1D calc - Mass**

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = **4,057.00**

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	0.885	
2	5.080	0.892	2.688
3	10.160	0.899	2.680
4	14.000	0.905	2.019
5	15.240	0.907	0.650
6	20.320	0.916	2.658
7	25.400	0.924	2.646
8	29.000	0.931	1.868
9	30.480	0.934	0.766
10	35.560	0.944	2.620
11	39.000	0.951	1.767
12	40.639	0.955	0.839
13	45.719	0.966	2.591
14	50.799	0.978	2.576
15	54.000	0.986	1.615
16	55.879	0.991	0.945
17	60.959	1.005	2.542
18	66.039	1.020	2.524
19	69.000	1.030	1.462
20	71.119	1.036	1.042
21	76.199	1.054	2.484
22	81.279	1.072	2.463
23	84.000	1.083	1.310
24	86.359	1.092	1.130
25	91.439	1.114	2.416
26	96.519	1.136	2.391
27	99.000	1.149	1.158
28	101.599	1.161	1.206
29	106.678	1.188	2.336
30	111.758	1.216	2.307
31	114.000	1.229	1.009
32	116.838	1.246	1.268
33	121.918	1.278	2.246
34	126.998	1.313	2.213
35	129.000	1.328	0.863
36	132.078	1.350	1.316
37	137.158	1.390	2.144
38	142.238	1.433	2.106
39	144.000	1.450	0.722
40	147.318	1.481	1.346
41	152.398	1.537	2.025
42	157.478	1.603	1.977
43	159.000	1.630	0.582
44	162.558	1.691	1.337
45	167.637	1.830	1.840
46	172.717	2.190	1.688
47	174.000	2.340	0.393
48	177.797	2.787	1.066
49	175.533	7.581	-0.366

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
<b>Δ Height (ft)</b>	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
<b>Height (ft)</b>	14	29	39	54	69	84	99	114	129
<b>Hs (ft)</b>	7.39	7.82	5.15	7.62	7.47	7.30	7.09	6.86	6.59
<b>e_avg</b>	0.90	0.92	0.94	0.97	1.01	1.06	1.11	1.19	1.28
<b>rho_dry (pcf)</b>	87.25	86.22	85.20	84.02	82.38	80.46	78.21	75.60	72.64
<b>Vol (ft^3)</b>	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
<b>Mass (ton)</b>	433,286	1,651,849	1,886,806	3,893,369	4,853,484	5,346,370	6,147,490	6,300,939	6,646,956

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
<b>Δ Height (ft)</b>	15.0	15.0	15.0	1.5				
<b>Height (ft)</b>	144.0	159.0	174.0	175.5				
<b>Hs (ft)</b>	6.29	5.93	5.26	0.70				
<b>e_avg</b>	1.39	1.53	1.85	1.19				
<b>rho_dry (pcf)</b>	69.32	65.36	57.96	75.44				
<b>Vol (ft^3)</b>	188,799,066	196,020,810	199,271,529	201,610,674				
<b>Mass (ton)</b>	6,543,555	6,406,050	5,775,192	7,605,173				

Time	4,057	days
Production	15,121	ton/day
Total mass	61,345,897	ton
TDF - total mass	63,490,519	ton
<b>Error (%)</b>	<b>-3.50%</b>	

Note: based on the time of filling and production rate  
 Note: based on the integration of the void ratio profile

**ATTACHMENT 3  
FSCONSOL MODEL SUMMARY**

## Input Parameters

Total Mine Tailings	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Cyclone Availability	100.00%		Assumed Plant Operations
Sand Recovery	45.25%		Average Recovery from Cyclone Plant
Slimes Percentage	40.00%		Slimes vs. Beach (Overflow + Whole)
Overflow Percentage	54.75%		Tailings (Overflow + Whole)
Whole Percentage	0.00%		Tailings (Whole versus Total)
Production	27,618	t/day	Total Tailings (Underflow + Overflow)
	15,121	t/day	Tailings Overflow + Whole
	0	t/day	Tailings Whole (Beach + Slimes)
	15,121	t/day	Tailings Overflow (Beach + Slimes)

## Overflow

Production	6,048	t/day	Tailings Overflow Slimes
Production	9,073	t/day	Tailings Overflow Beach
Production	0	t/day	Tailings Whole Slimes
Production	0	t/day	Tailings Whole Beach
Capacity	6.13E+07	tons	Tailings Overflow (Beach + Slimes)
Time	4,057	days	Tailings Overflow (Beach + Slimes)
	11.115	years	

## FSConsol 1D Percent Errors

	Height	Volume	Mass	Average
OF-slimes	1.93%	0.60%	13.74%	5.42%
OF-beach	1.13%	3.47%	3.50%	2.70%
Whole-slimes	0.00%	0.00%	0.00%	0.00%
Whole-beach	0.00%	0.00%	0.00%	0.00%

## FSConsol average densities (for deepest column in TSF)

	Unadjusted	Adjusted
OF-slimes	33.55 pcf	31.73 pcf
OF-beach	76.65 pcf	74.58 pcf
Whole-slimes	0.00 pcf	0.00 pcf
Whole-beach	0.00 pcf	0.00 pcf

## Required Capacity

	5.07E+07	tons	Tailings Underflow Cyclone Sand
	2.45E+07	tons	Tailings Overflow Slimes
	3.68E+07	tons	Tailings Overflow Beach
	0.00E+00	tons	Tailings Whole Slimes
	0.00E+00	tons	Tailings Whole Beach
	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Check	0.0000	tons	OK

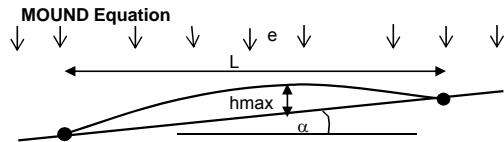
## Required Volume and Weighted Density

	Unadjusted	Adjusted	Volume	Density
OF-slimes	1.46E+09 ft <sup>3</sup>	1.55E+09 ft <sup>3</sup>	61.04%	19.37 pcf
OF-beach	9.60E+08 ft <sup>3</sup>	9.87E+08 ft <sup>3</sup>	38.96%	29.05 pcf
Whole-slimes	0.00E+00 ft <sup>3</sup>	0.00E+00 ft <sup>3</sup>	0.00%	0.00 pcf
Whole-beach	0.00E+00 ft <sup>3</sup>	0.00E+00 ft <sup>3</sup>	0.00%	0.00 pcf
<b>Total</b>	<b>8.97E+07 yd<sup>3</sup></b>	<b>9.38E+07 yd<sup>3</sup></b>	<b>100.00%</b>	<b>48.42 pcf</b>

**APPENDIX D  
UNDERDRAIN DESIGN CALCULATIONS**

**APPENDIX D.1  
DRAIN PIPE SPACING CALCULATIONS**





$$h_{max} = \frac{L\sqrt{C}}{2} \left[ \frac{\tan^2 \alpha}{C} + \left( 1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing  
C=Inflow Ratio (e/K)  
e=Constant Recharge Rate  
k=Drain Media Hydraulic Conductivity  
tan α=Slope Between Pipes

$h_{max}$ (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.41	45	5.07E-04	1.44E+00	1.00E-01	2.83E+02	5.07E-03	0.01

Assume sand applied over a 600 foot by 100 foot area.  
Estimate an equivalent constant recharge rate (e) for the drainage through the sand in the active sand application area  
Sand K will be higher but flow will be limited by water in underflow

**Estimate Equivalent Constant Recharge Rate (e)**

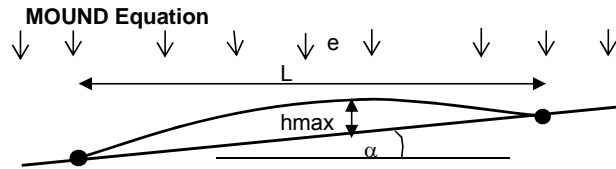
Tons milled	30,000 tpd
Cyclone recovery	46 percent
Q sand	13,800 tpd
Solids content	70 wt percent
Total underflow	19,714 tpd
Water in UF	5,914 tpd
Water in UF	1,042 gpm
Water retained	42.00%
Water lost to evap	15.00%
Seepage	448 gpm

Application area	600 L (feet)	Estimate
Application area	100 w (feet)	Estimate
Application area	60,000 ft <sup>2</sup>	

Application rate 1.66392E-05 feet/sec  
**Application rate (e) 5.07E-04 cm/sec**  
Equivalent flow rate for active sand application area

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,<sup>1</sup> Yunmin Chen,<sup>2</sup> and Chuangbing Huang<sup>3</sup>  
Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers  
J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)



$$h_{max} = \frac{L\sqrt{C}}{2} \left[ \frac{\tan^2 \alpha}{C} + \left( 1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing  
 C=Inflow Ratio (e/K)  
 e=Constant Recharge Rate  
 k=Drain Media Hydraulic Conductivity  
 tan α=Slope Between Pipes

$h_{max}$ (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.33	35	2.75E-07	7.80E-04	3.80E-05	1.08E-01	7.24E-03	0.01

**K drain test result 3.8e-05 cm/sec, select (native)drain fill**  
**Average of beach and slimes hydraulic conductivity applied (2.75x10<sup>7</sup>cm/sec).**

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,<sup>1</sup> Yunmin Chen,<sup>2</sup> and Chuangbing Huang<sup>3</sup>  
 Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers  
 J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)

**APPENDIX D.2  
DRAIN PIPE DEFLECTION CALCULATIONS**

<b>Date:</b>	April 23, 2013	<b>Made by:</b>	CDJ
<b>Project No.:</b>	103-92557	<b>Checked by:</b>	GM
<b>Subject:</b>	Underdrain Pipe Deflection Calculations	<b>Reviewed by:</b>	MJG
<b>Project Short Title: COPPER FLAT FEASIBILITY DESIGN</b>			

## 1.0 OBJECTIVE

Determine the amount of deflection that will be experienced by the piping system within the tailings storage facility (TSF) underdrains caused by the surcharge load of the ultimate tailings height.

## 2.0 ASSUMPTIONS

- Underdrain Fill has a cohesion of 0 pounds per square inch (psi) and a friction angle of 39 degrees based on Golder laboratory tests for tailings underflow cyclone sand material.
- Underdrain Fill has a stress versus strain relationship similar to Silty Sand at a relative density of 80 percent per Standard AASHTO compaction specifications.
- Linear relationship between an applied vertical load of 52.1 to 69.4 psi to extrapolate the appropriate strain for an applied vertical load of 200 psi, which is the total vertical stress component imposed by the ultimate tailings height.
- The maximum burial depth is 240 feet.
- All pipes are perforated corrugated polyethylene (PCPE) N-12 pipes with a flexural modulus of 22,000 psi for a 50-year life span.
- The optimum dry density of the tailings underflow cyclone sand in the TSF is 97 pounds per cubic foot (pcf) per laboratory soil density/moisture content compaction proctor tests. As-delivered moisture content is 30 percent for a wet density of 126 pcf. After initial draindown and evaporation, moisture content of approximately 17 percent (optimum water content) for a wet density of 113.5 pcf. Therefore, use the worse-case scenario of 126 pcf for the loading conditions applied to the underdrain.
- Underdrain pipes will be subject to surface loads associated with saturated tailings overflow. The average dry density of the tailings overflow thickened slimes is estimated to be 73 pcf with a wet density of 108 pcf. The loading condition imposed by the cyclone sand dam fill represents the worst-case scenario for pipe deflection analyses.
- Maximum deflection allowed is 15 percent.

## 3.0 CALCULATION OF PIPE DEFLECTIONS

Golder analyzed the pipe stresses and deformations based on the work of Burns and Richards (Burns and Richards, 1964) and Hoeg (1968) with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression. Calculations of deflections were completed using an Excel™ spreadsheet. Calculations are included in Attachment 1.

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix D.2 Drain Pipe Deflection Calculations

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**Golder Associates: Operations in Africa, Asia, Australasia, Europe, North America and South America**

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## 4.0 RESULTS

A summary of the maximum deflections of the underdrain collection system pipes within the Underdrain Fill for the TSF is shown below in Table 1.

**Table 1: Summary of Maximum Pipe Deflections**

Pipe Information	No Interface Slippage Vertical Deflection (%)	Full Slippage Vertical Deflection (%)
4-inch PCPE N-12	12.6	13.5
10-inch PCPE N-12	12.9	13.8
12-inch PCPE N-12	10.9	11.5

## 5.0 CONCLUSIONS

The calculated deflections of the 4-inch, 10-inch and 12-inch diameter PCPE N-12 pipes proposed for use in the impoundment and embankment underdrains at the Copper Flat TSF is estimated to be within the allowable limits under the anticipated worst-case loading and placement conditions. The maximum deflection is estimated to be 11 to 11.5 percent for 12-inch diameter pipe and 12.5 to 14 percent for 4-inch to 10-inch diameter pipes. The maximum allowable deflection of these pipes is 15 percent.

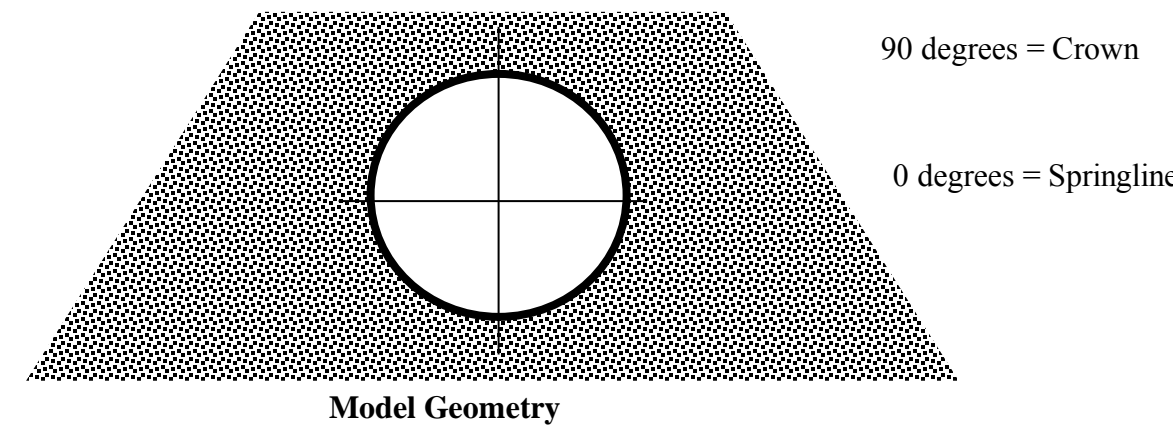
## 6.0 REFERENCES

- Burns, J.Q. and R.M. Richard, 1964. Attenuation of Stresses for Buried Conduits. Proceedings of the Symposium Soil-Structure Interaction, University of Arizona.
- Hoeg, K., 1968. Stresses Against Underground Structural Cylinders. J. Soil Mech. and Foundation Div. ASCE, Vol. 94, No. SM4.
- Lupo, J.F., 2001. Stability of HDPE Pipes Under High Heap Loads. SME Annual Meeting, Denver, Colorado. February 26-28, 2001.

**ATTACHMENT 1  
PIPE DEFLECTION CALCULATION**

**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
4 IN N-12 PIPE

By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

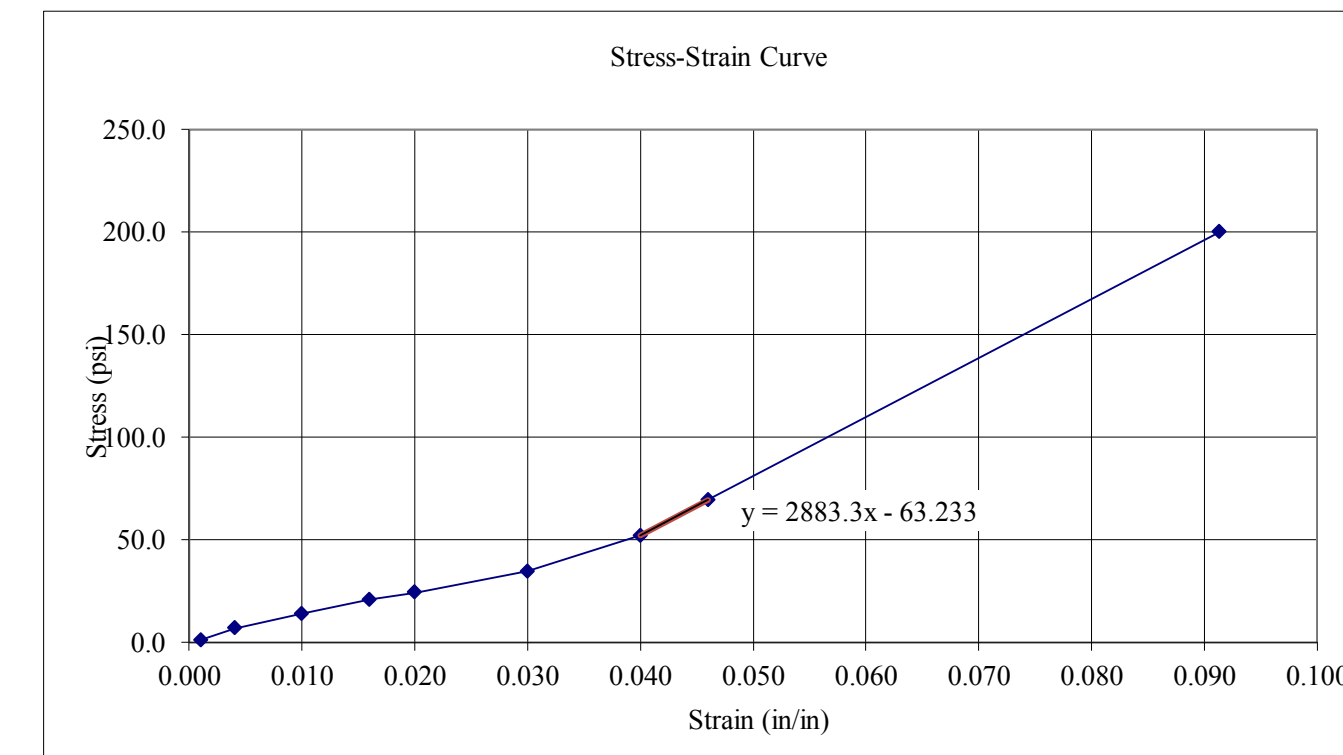
**SOIL and PIPE Input Data**

Lateral Pressure Parameters						
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30
Pipe Diameter OD (in):	4.78					
Pipe ID (in):	4.10					
Weight of Pipe (lb/ft):	0.41					
Pipe Corrugated (y/n):	y		64.50			
Prescribed Constrained Modulus (y/n):	n		0.23			
Prescribed Constrained Modulus (psi):	na					
Pipe Wall Thickness (in):	0.02					
Pipe Area (in <sup>2</sup> /in):	0.07					
Flexural Modulus (psi), E <sub>f</sub> =	22,000					
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000					
C value (in)	0.29					
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	11.8					
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0014	selected I:	0.0014			
Stiffness Coefficients						
Flexural Stiffness	13.7					
Ring Compression Stiffness	647.1					
Ring Stiffness Factor:		2.3	Pipe Stiffness Less Than Soil			

Shell-Medium Parameters		
UF	4.72	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	97.0	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	2.38	
Depth of Burial (ft):	240.0	
Applied Surface Stress (psf):	0.0	
Soil Density (pcf):	120.0	
Total Vertical Stress Component (psf):	28800.0	Free Field Stress Values
Total Vertical Stress Component (psi):	200.0	

Critical Stress For Buckling Failure (psf): 194.7  
Ovality: 0.3  
Corrected Stress (psf): 58.4



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	119.6	308.1	0.0	-0.074	0.00E+00	280.7	7.7	4009.8	0.1823	0.0757	1593	1483	5602	5493
10	117.2	301.5	29.6	-0.054	5.92E-02	274.6	7.3	3923.3	0.1783	0.0741	1516	1412	5440	5335
20	110.3	282.5	55.6	0.005	1.11E-01	257.2	6.3	3674.5	0.1670	0.0694	1297	1208	4972	4882
30	99.7	253.4	75.0	0.095	1.50E-01	230.5	4.6	3293.2	0.1497	0.0622	961	895	4254	4188
40	86.8	217.7	85.2	0.205	1.70E-01	197.8	2.7	2825.5	0.1284	0.0533	549	511	3375	3337
50	73.0	179.7	85.2	0.322	1.70E-01	162.9	0.5	2327.8	0.1058	0.0440	110	103	2438	2431
60	60.0	144.0	75.0	0.433	1.50E-01	130.2	-1.5	1860.1	0.0846	0.0351	-302	-281	1558	1579
70	49.5	114.9	55.6	0.522	1.11E-01	103.5	-3.1	1478.9	0.0672	0.0279	-638	-594	841	885
80	42.6	95.9	29.6	0.581	5.92E-02	86.1	-4.1	1230.0	0.0559	0.0232	-857	-798	373	432
90	40.2	89.3	0.0	0.602	2.12E-17	80.1	-4.5	1143.6	0.0520	0.0216	-933	-869	210	275

Vertical Deflection (%):	12.58		149.93	
Horizontal Deflection (%):	-3.11		258.17	
Radial Soil Pressure at Crown (psi):	40.2	5788		Max. Compressive Stress (psi): 5602
Circumferential Shortening (in):	1.95			Max. Tensile Stress (psi): No Tensile Stress
Arc length of each sector (in) =	0.42			

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.9	81.3	0.0	-0.116	0.00E+00	183.4	8.4	2620.0	0.1191	0.0495	1748	1628	4368	4247
10	76.1	88.4	121.8	-0.093	1.31E-01	183.2	8.0	2617.4	0.1190	0.0494	1663	1548	4280	4165
20	76.8	108.8	229.0	-0.027	2.46E-01	182.7	6.8	2609.8	0.1186	0.0493	1416	1319	4026	3928
30	77.9	140.0	308.5	0.074	3.31E-01	181.9	5.0	2598.3	0.1181	0.0491	1039	967	3637	3566
40	79.2	178.3	350.8	0.198	3.77E-01	180.9	2.8	2584.2	0.1175	0.0488	576	536	3160	3121
50	80.6	219.1	350.8	0.330	3.77E-01	179.8	0.4	2569.2	0.1168	0.0485	83	78	2653	2647
60	81.9	257.4	308.5	0.453	3.31E-01	178.9	-1.8	2555.0	0.1161	0.0482	-379	-353	2176	2202
70	83.0	288.6	229.0	0.554	2.46E-01	178.0	-3.7	2543.5	0.1156	0.0480	-757	-705	1787	1839
80	83.7	309.0	121.8	0.620	1.31E-01	177.5	-4.8	2536.0	0.1153	0.0479	-1003	-934	1533	1602
90	83.9	316.1	0.0	0.643	4.69E-17	177.3	-5.3	2533.4	0.1152	0.0478	-1089	-1013	1445	1520

Vertical Deflection (%):	13.46
Horizontal Deflection (%):	-4.87
Radial Soil Pressure at Crown (psi):	83.9
Circumferential Shortening (in):	1.95
Arc length of each sector (in) =	0.42

12084

Max. Compressive Stress (psi): 4368  
Max. Tensile Stress (psi): No Tensile Stress

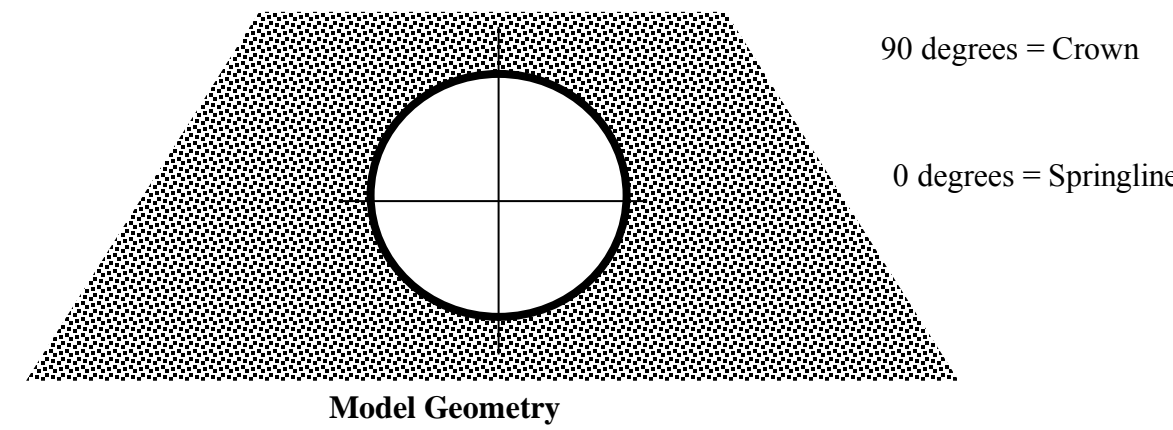
Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 19404 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
2.38	177.3	80.1	316.1	89.3	183.4	280.7	81.3	308.1
2.88	216.6	97.8	198.1	96.5	224.0	342.9	159.9	261.5
3.38	255.9	115.5	147.9	95.9	264.6	405.1	187.5	239.6
3.88	295.2	133.2	123.0	93.6	305.2	467.2	198.2	227.6
4.38	334.4	150.9	109.2	91.4	345.9	529.4	202.6	220.4
4.88	373.7	168.6	100.9	89.4	386.5	591.6	204.3	215.7
5.38	413.0	186.3	95.5	87.8	427.1	653.8	204.9	212.5
5.88	452.3	204.0	91.8	86.5	467.7	716.0	204.9	210.2
6.38	491.5	221.7	89.3	85.4	508.3	778.2	204.7	208.5
6.88	530.8	239.4	87.4	84.6	548.9	840.3	204.4	207.2
7.38	570.1	257.1	85.9	83.8	589.6	902.5	204.0	206.1
7.88	609.4	274.8	84.8	83.2	630.2	964.7	203.7	205.3
8.38	648.6	292.5	84.0	82.7	670.8	1026.9	203.4	204.7
8.88	687.9	310.2	83.3	82.3	711.4	1089.1	203.1	204.1
9.38	727.2	328.0	82.7	81.9	752.0	1151.3	202.9	203.7
9.88	766.5	345.7	82.2	81.6	792.6	1213.4	202.6	203.3
10.38	805.7	363.4	81.9	81.3	833.3	1275.6	202.4	203.0
10.88	845.0	381.1	81.5	81.1	873.9	1337.8	202.2	202.7
11.38	884.3	398.8	81.2	80.9	914.5	1400.0	202.1	202.4
11.88	923.6	416.5	81.0	80.7	955.1	1462.2	201.9	202.2
12.38	962.8	434.2	80.8	80.5	995.7	1524.4	201.8	202.1
12.88	1002.1	451.9	80.6	80.4	1036.3	1586.5	201.7	201.9
13.38	1041.4	469.6	80.4	80.3	1077.0	1648.7	201.6	201.7
13.88	1080.7	487.3	80.3	80.1	1117.6	1710.9	201.5	201.6
14.38	1119.9	505.0	80.2	80.0	1158.2	1773.1	201.4	201.5
Check Values:	648.6	292.5	155.8	92.0	670.8	1026.9	176.9	240.8
Soil Arching:	Positive Arch	Positive Arch	Negative Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch



**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
10 IN N-12 PIPE

By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

**SOIL and PIPE Input Data**

Lateral Pressure Parameters						
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30

Pipe Diameter OD (in):	11.36
Pipe ID (in):	9.90
Weight of Pipe (lb/ft):	2.26
Pipe Corrugated (y/n):	y
Prescribed Constrained Modulus (y/n):	n
Prescribed Constrained Modulus (psi):	na
Pipe Wall Thickness (in):	0.03
Pipe Area (in <sup>2</sup> /in):	0.15
Flexural Modulus (psi), E <sub>f</sub> =	22,000
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000
C value (in)	0.34
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	346.0
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0110
<b>Stiffness Coefficients</b>	
Flexural Stiffness	8.0
Ring Compression Stiffness	562.9

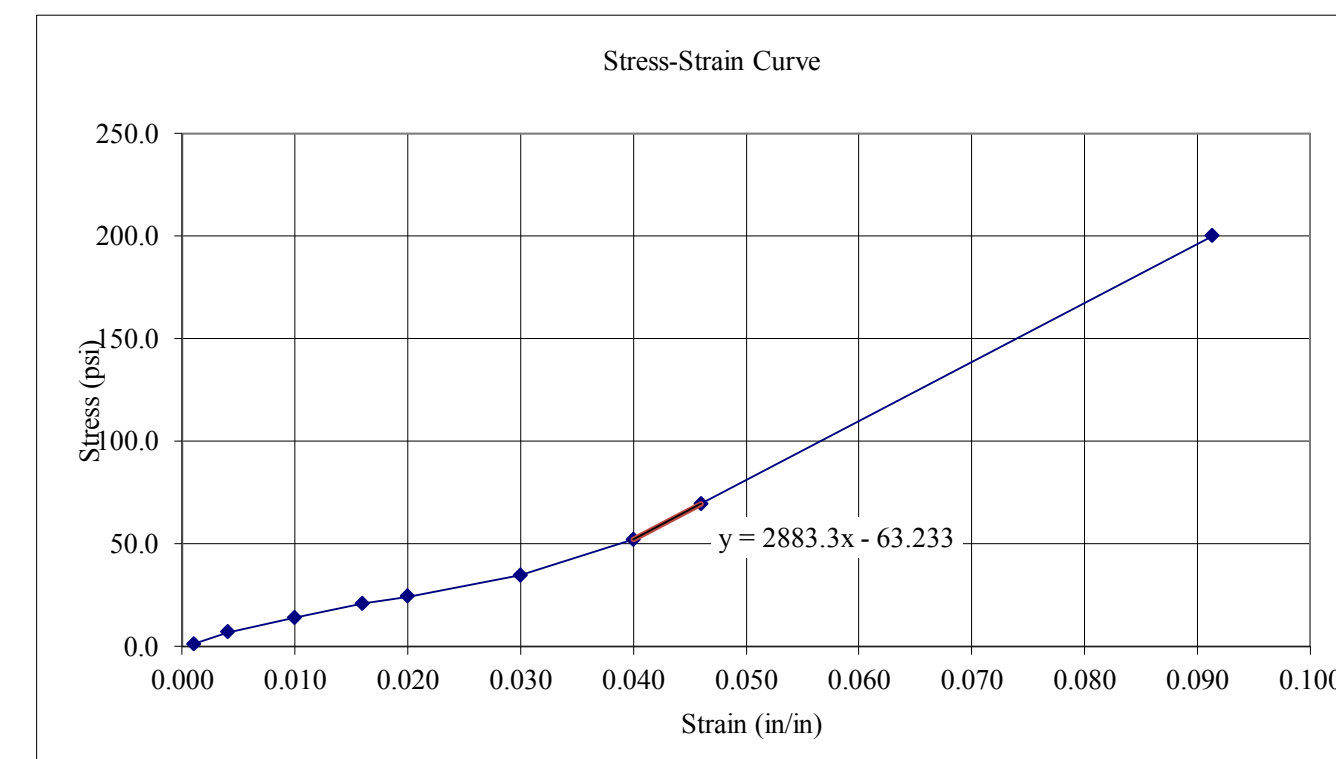
64.50  
0.23  
selected I: 0.0110  
Ring Stiffness Factor: 1.3 Pipe Stiffness Less Than Soil

Shell-Medium Parameters		
UF	5.42	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	166.7	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	5.67
Depth of Burial (ft):	240.0
Applied Surface Stress (psf):	0.0
Soil Density (pcf):	120.0
Total Vertical Stress Component (psf):	28800.0
Total Vertical Stress Component (psi):	200.0

Free Field Stress Values

Critical Stress For Buckling Failure (psf): 642.5  
Ovality: 0.3  
Corrected Stress (psf): 192.8



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	118.2	313.4	0.0	-0.172	0.00E+00	652.7	25.6	4501.3	0.2046	0.2024	793	734	5294	5236
10	115.8	306.6	29.3	-0.123	1.41E-01	638.5	24.4	4403.7	0.2002	0.1980	755	699	5159	5103
20	108.7	287.1	55.1	0.019	2.65E-01	597.8	20.9	4122.8	0.1874	0.1854	646	598	4769	4721
30	97.9	257.2	74.2	0.237	3.57E-01	535.4	15.5	3692.3	0.1678	0.1660	479	444	4171	4136
40	84.6	220.6	84.3	0.504	4.06E-01	458.8	8.9	3164.3	0.1438	0.1423	274	254	3439	3418
50	70.4	181.6	84.3	0.788	4.06E-01	377.3	1.8	2602.3	0.1183	0.1170	57	52	2659	2655
60	57.1	144.9	74.2	1.055	3.57E-01	300.8	-4.8	2074.3	0.0943	0.0933	-148	-137	1926	1937
70	46.3	115.1	55.1	1.273	2.65E-01	238.4	-10.2	1643.8	0.0747	0.0739	-315	-292	1329	1352
80	39.2	95.6	29.3	1.415	1.41E-01	197.6	-13.7	1362.9	0.0619	0.0613	-424	-393	939	970
90	36.8	88.8	0.0	1.464	5.05E-17	183.5	-14.9	1265.3	0.0575	0.0569	-462	-428	804	837

Vertical Deflection (%):	12.89
Horizontal Deflection (%):	-3.03
Radial Soil Pressure at Crown (psi):	36.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

5295 149.93 258.17  
Max. Compressive Stress (psi): 5294  
Max. Tensile Stress (psi): No Tensile Stress

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.2	82.0	0.0	-0.271	0.00E+00	422.3	28.1	2912.3	0.1324	0.1309	869	805	3781	3717
10	75.3	89.2	123.0	-0.216	3.15E-01	422.0	26.7	2910.6	0.1323	0.1309	826	766	3737	3676
20	75.7	109.9	231.1	-0.057	5.93E-01	421.3	22.8	2905.5	0.1321	0.1306	704	652	3610	3558
30	76.3	141.5	311.4	0.187	7.98E-01	420.2	16.7	2897.8	0.1317	0.1303	517	479	3415	3377
40	77.1	180.4	354.1	0.487	9.08E-01	418.8	9.3	2888.3	0.1313	0.1299	288	266	3176	3155
50	77.9	221.8	354.1	0.805	9.08E-01	417.3	1.4	2878.3	0.1308	0.1294	43	40	2922	2918
60	78.7	260.6	311.4	1.105	7.98E-01	416.0	-6.0	2868.8	0.1304	0.1290	-186	-173	2683	2696
70	79.3	292.3	231.1	1.349	5.93E-01	414.9	-12.1	2861.0	0.1300	0.1286	-373	-346	2488	2515
80	79.7	313.0	123.0	1.508	3.15E-01	414.1	-16.0	2856.0	0.1298	0.1284	-495	-459	2361	2397
90	79.8	320.2	0.0	1.564	1.13E-16	413.9	-17.4	2854.3	0.1297	0.1283	-538	-498	2316	2356

Vertical Deflection (%):	13.76
Horizontal Deflection (%):	-4.79
Radial Soil Pressure at Crown (psi):	79.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

11497

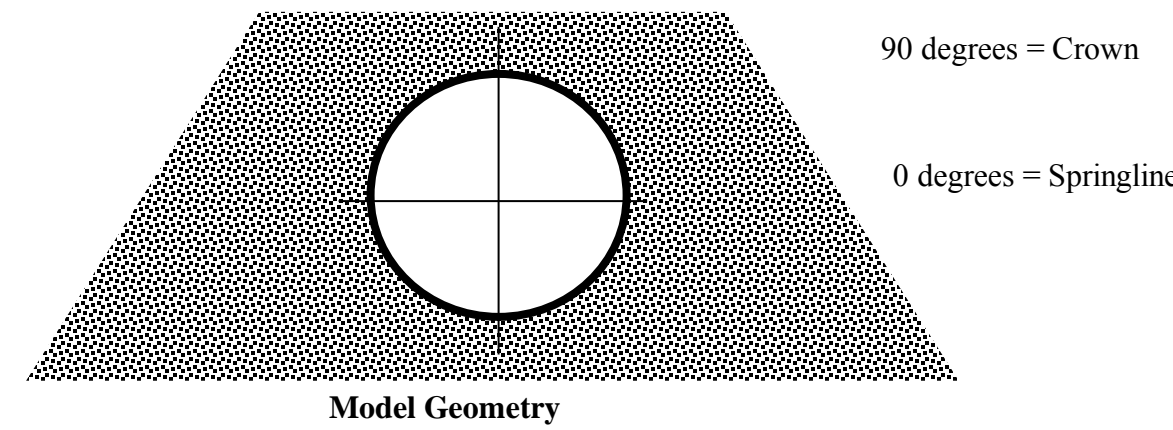
Max. Compressive Stress (psi): 3781  
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 33347 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
5.67	413.9	183.5	320.2	88.8	422.3	652.7	82.0	313.4
6.17	452.2	200.5	256.3	94.2	461.4	713.2	125.7	287.8
6.67	490.6	217.5	213.4	96.5	500.6	773.7	153.0	270.0
7.17	529.0	234.4	183.6	97.1	539.7	834.2	170.5	257.0
7.67	567.3	251.4	162.2	96.9	578.9	894.7	182.0	247.3
8.17	605.7	268.4	146.5	96.2	618.0	955.2	189.7	240.0
8.67	644.0	285.4	134.6	95.3	657.1	1015.8	194.9	234.2
9.17	682.4	302.4	125.5	94.3	696.3	1076.3	198.4	229.6
9.67	720.8	319.4	118.4	93.3	735.4	1136.8	200.8	225.9
10.17	759.1	336.4	112.7	92.4	774.6	1197.3	202.5	222.8
10.67	797.5	353.4	108.2	91.5	813.7	1257.8	203.6	220.3
11.17	835.9	370.4	104.4	90.6	852.9	1318.3	204.3	218.2
11.67	874.2	387.4	101.4	89.8	892.0	1378.8	204.8	216.4
12.17	912.6	404.4	98.8	89.1	931.1	1439.3	205.1	214.8
12.67	950.9	421.4	96.7	88.4	970.3	1499.8	205.2	213.5
13.17	989.3	438.4	94.8	87.8	1009.4	1560.4	205.3	212.3
13.67	1027.7	455.4	93.3	87.2	1048.6	1620.9	205.3	211.3
14.17	1066.0	472.4	91.9	86.7	1087.7	1681.4	205.2	210.4
14.67	1104.4	489.3	90.8	86.2	1126.8	1741.9	205.1	209.6
15.17	1142.7	506.3	89.7	85.8	1166.0	1802.4	205.0	208.9
15.67	1181.1	523.3	88.8	85.4	1205.1	1862.9	204.8	208.3
16.17	1219.5	540.3	88.0	85.0	1244.3	1923.4	204.7	207.8
16.67	1257.8	557.3	87.3	84.6	1283.4	1983.9	204.6	207.3
17.17	1296.2	574.3	86.7	84.3	1322.6	2044.4	204.4	206.8
17.67	1334.6	591.3	86.2	84.0	1361.7	2105.0	204.3	206.4
Check Values:	874.2	387.4	202.4	95.0	892.0	1378.8	156.8	264.2
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
12 IN N-12 PIPE

By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

**SOIL and PIPE Input Data**

Lateral Pressure Parameters						
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30

Pipe Diameter OD (in):	14.45
Pipe ID (in):	12.15
Weight of Pipe (lb/ft):	3.19
Pipe Corrugated (y/n):	y
Prescribed Constrained Modulus (y/n):	n
Prescribed Constrained Modulus (psi):	na
Pipe Wall Thickness (in):	0.04
Pipe Area (in <sup>2</sup> /in):	0.19
Flexural Modulus (psi), E <sub>f</sub> =	22,000
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000
C value (in)	0.53
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	1070.4
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0410
<b>Stiffness Coefficients</b>	
Flexural Stiffness	14.5
Ring Compression Stiffness	573.8

64.50  
0.23

selected I: 0.0410

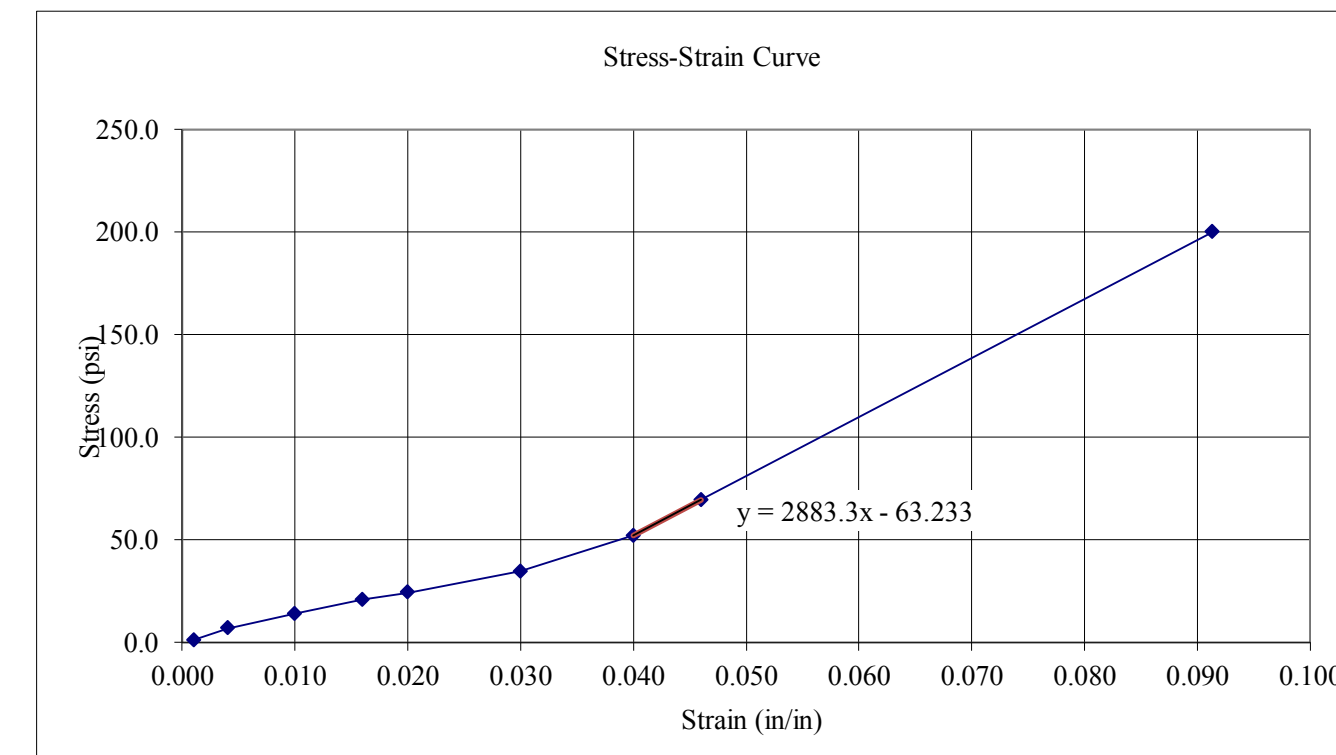
Ring Stiffness Factor: 2.4 Pipe Stiffness Less Than Soil

Shell-Medium Parameters		
UF	5.32	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	92.0	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	7.21
Depth of Burial (ft):	240.0
Applied Surface Stress (psf):	0.0
Soil Density (pcf):	120.0
Total Vertical Stress Component (psf):	28800.0
Total Vertical Stress Component (psi):	200.0

Free Field Stress Values

Critical Stress For Buckling Failure (psf): 1883.1  
Ovality: 0.3  
Corrected Stress (psf): 564.9



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	97.9	343.0	0.0	-0.122	0.00E+00	693.6	63.5	3689.5	0.1677	0.2110	820	766	4510	4456
10	95.8	335.2	25.4	-0.071	1.40E-01	678.0	60.5	3606.5	0.1639	0.2062	782	730	4389	4337
20	90.0	312.9	47.7	0.075	2.64E-01	633.1	51.9	3367.6	0.1531	0.1926	671	627	4039	3994
30	80.9	278.8	64.2	0.301	3.56E-01	564.3	38.8	3001.4	0.1364	0.1716	502	468	3503	3470
40	69.9	236.9	73.0	0.577	4.04E-01	479.8	22.7	2552.3	0.1160	0.1459	293	274	2846	2826
50	58.1	192.3	73.0	0.871	4.04E-01	390.0	5.6	2074.3	0.0943	0.1186	72	67	2146	2141
60	47.1	150.4	64.2	1.147	3.56E-01	305.5	-10.5	1625.2	0.0739	0.0929	-136	-127	1489	1498
70	38.1	116.2	47.7	1.372	2.64E-01	236.7	-23.7	1259.0	0.0572	0.0720	-306	-286	953	973
80	32.2	93.9	25.4	1.519	1.40E-01	191.8	-32.2	1020.1	0.0464	0.0583	-417	-389	603	631
90	30.1	86.2	0.0	1.570	5.03E-17	176.2	-35.2	937.1	0.0426	0.0536	-455	-425	482	512

Vertical Deflection (%):	10.86
Horizontal Deflection (%):	-1.70
Radial Soil Pressure at Crown (psi):	30.1
Circumferential Shortening (in):	5.29
Arc length of each sector (in) =	1.26

4340

149.93  
258.17

Max. Compressive Stress (psi): 4510  
Max. Tensile Stress (psi): No Tensile Stress

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	60.5	96.7	0.0	-0.214	0.00E+00	442.9	68.7	2356.0	0.1071	0.1347	889	830	3245	3186
10	60.7	103.8	122.2	-0.157	3.23E-01	442.4	65.4	2353.4	0.1070	0.1346	846	790	3199	3144
20	61.3	124.3	229.6	0.006	6.07E-01	441.1	56.0	2346.0	0.1066	0.1341	723	676	3069	3022
30	62.2	155.6	309.3	0.255	8.18E-01	438.9	41.4	2334.7	0.1061	0.1335	536	500	2870	2835
40	63.4	194.1	351.7	0.561	9.30E-01	436.3	23.6	2320.7	0.1055	0.1327	305	285	2626	2606
50	64.6	235.0	351.7	0.887	9.30E-01	433.5	4.6	2305.9	0.1048	0.1318	60	56	2366	2362
60	65.8	273.5	309.3	1.192	8.18E-01	430.9	-13.2	2291.9	0.1042	0.1311	-170	-159	2122	2133
70	66.7	304.9	229.6	1.442	6.07E-01	428.8	-27.7	2280.6	0.1037	0.1304	-358	-335	1922	1946
80	67.3	325.3	122.2	1.605	3.23E-01	427.4	-37.2	2273.2	0.1033	0.1300	-481	-449	1792	1824
90	67.6	332.4	0.0	1.661	1.16E-16	426.9	-40.5	2270.6	0.1032	0.1298	-523	-489	1747	1782

Vertical Deflection (%):	11.50
Horizontal Deflection (%):	-2.96
Radial Soil Pressure at Crown (psi):	67.6
Circumferential Shortening (in):	5.29
Arc length of each sector (in) =	1.26


9728

Max. Compressive Stress (psi): 3245  
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 18401 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
7.21	426.9	176.2	332.4	86.2	442.9	693.6	96.7	343.0
7.71	458.3	189.1	278.0	93.0	475.5	744.7	131.0	316.0
8.21	489.7	202.0	238.5	96.8	508.1	795.8	154.3	295.9
8.71	521.1	215.0	209.0	98.7	540.7	846.9	170.3	280.6
9.21	552.5	227.9	186.6	99.4	573.3	897.9	181.4	268.6
9.71	583.9	240.8	169.3	99.5	605.9	949.0	189.3	259.1
10.21	615.3	253.8	155.7	99.1	638.5	1000.1	194.9	251.5
10.71	646.8	266.7	144.8	98.5	671.1	1051.2	198.9	245.2
11.21	678.2	279.6	136.0	97.7	703.7	1102.2	201.8	240.1
11.71	709.6	292.6	128.8	96.9	736.3	1153.3	203.8	235.8
12.21	741.0	305.5	122.9	96.0	768.9	1204.4	205.3	232.1
12.71	772.4	318.4	117.9	95.2	801.5	1255.5	206.3	229.0
13.21	803.8	331.4	113.7	94.3	834.1	1306.5	207.0	226.3
13.71	835.2	344.3	110.1	93.5	866.7	1357.6	207.4	224.0
14.21	866.6	357.2	107.1	92.8	899.3	1408.7	207.7	222.0
14.71	898.1	370.2	104.5	92.0	931.9	1459.7	207.8	220.2
15.21	929.5	383.1	102.2	91.4	964.4	1510.8	207.9	218.7
15.71	960.9	396.0	100.2	90.7	997.0	1561.9	207.8	217.3
16.21	992.3	409.0	98.4	90.1	1029.6	1613.0	207.7	216.1
16.71	1023.7	421.9	96.9	89.5	1062.2	1664.0	207.6	215.0
17.21	1055.1	434.8	95.5	89.0	1094.8	1715.1	207.5	214.0
17.71	1086.5	447.8	94.3	88.5	1127.4	1766.2	207.3	213.1
18.21	1118.0	460.7	93.2	88.0	1160.0	1817.3	207.1	212.3
18.71	1149.4	473.6	92.2	87.6	1192.6	1868.3	206.9	211.5
19.21	1180.8	486.6	91.3	87.2	1225.2	1919.4	206.7	210.9
Check Values:	803.8	331.4	224.2	96.1	834.1	1306.5	159.7	287.8
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

**APPENDIX E  
TSF UNDERDRAIN COLLECTION POND  
INFLOW ESTIMATION**

		<b>Tailings Storage Facility - Underdrain Collection Pond Inflow Estimate</b>			
<b>Client</b>	<b>New Mexico Copper Corp</b>	<b>By</b>	<b>GM/DW</b>	<b>Date</b>	<b>19-Apr-13</b>
		<b>Checked</b>	<b>GM</b>	<b>Date</b>	<b>15-Jul-13</b>
<b>Project No.</b>	<b>103-92557 Phase 011</b>	<b>Reviewed</b>	<b>MJG</b>	<b>Date</b>	

Requirements	Criteria	
1 Collect downstream embankment face and toe area runoff	100-yr 24 hour storm	3.73 inches
2 Collect Underflow drainage	Upset period	24 hours
3 Collect impoundment seepage	Upset period	24 hours
4 Provide for contact of free water pond with exposed underdrain	Upset period	24 hours

Product	k (cm/sec)	K (ft/sec)
Cyclone overflow beach (cob)	5.00E-07	1.64E-08
Cyclone overflow slimes (cos)	5.00E-08	1.64E-09

The above assume fully consolidated material against drain

**1) Downstream Dam Face Runoff**

Worst case prior to Final Build-out	Runoff Volume	12.1 acre-feet
	See HEC model Output	3,942,528 Gallons

**2) Underflow Drainage**

Mining Rate		30000 tpd
Underflow percentage	(Cyclone Simulation)	45.5 percent
Underflow solids content	(Cyclone Simulation)	70 percent by weight
Slurry Volume	(Cyclone Simulation)	568 tons per hour max
Water volume	(Cyclone Simulation)	260 tons per hour max

Delivered water (wt water/wt solids)	1042 gpm
Available for Drainage and Evaporation	58.00% estimate
Entrainment	42.00% estimate
Evaporative loss	15.00% estimate
Draindown	43.00%
Draindown (gpm)	448 gpm

**3) Impoundment Interior Seepage**

Assume Areal Split	Beach	60%	Slimes	40%
--------------------	-------	-----	--------	-----

	Total Area (ft <sup>3</sup> )	Beach Area	Slimes Area
Final Build-out	14,012,401	8,407,441	5,604,960

Assume unit hydraulic gradient	Hydraulic conductivity based on results of slurry consolidation testing					
Beach Seepage	K floor (COB)			Slimes Seepage		
	1.64E-08 ft/sec			1.64E-09 ft/sec		
	ft <sup>3</sup> /sec	gpm	Upset (gal)	ft <sup>3</sup> /sec	gpm	upset (gal)
	0.1379	62	89,132	0.0092	4.13	5,942

**4) Seepage Through Exposed Drain**

Area exposed	20 Acre	871,200 ft <sup>3</sup>	Assumption/estimate
K drain surface layer	3.8000E-05 cm/sec	1.2467E-06 ft/sec	K drain based on permeability test result
Assume max depth 5 feet, average depth 2.5 feet	i=	2.5	
	cfs	gpm	Upset (gal)
Q	2.7154	1,219	1,754,857

Summary	Pond Capacity Requirements	Duration	Storage (gallons)
Dam exterior Runoff		Event	3,942,528 (HEC runoff estimate)
Beach Seepage		Upset	89,132
Slimes Seepage		Upset	5,942
Exposed impoundment Underdrain Seepage		Upset	1,754,857
Dam underdrainage		Upset	645,206
Total			6,437,666
			gpm
Normal flows		Dam drain	448 gpm
		Beach/slimes	66 gpm
		Free Water Pond seepage	1,219 gpm
		<b>Total</b>	<b>1,733 gpm</b>

**APPENDIX F  
HYDROLOGIC CALCULATIONS**

**APPENDIX F.1  
IMPOUNDMENT DIVERSION DITCH CALCULATIONS**



**Copper Flat Diversions, Copper Flat New Mexico**  
**Hydrologic and Hydraulic Calculations / Calculation of Time Concentration (t<sub>c</sub>)**  
**AMC II Moisture Conditions**

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile			S	L	H1	H2	Y	Lag	t <sub>c</sub>	t <sub>c</sub>	Lag	Area
	Area		Area		Area	Wt.	CN										
	(ac)	CN	(ac)	CN	(ac)	CN	CN		(ft)	(ft)	(ft)	(%)	(hr)	(hr)	(min)	(min)	(mi <sup>2</sup> )
Northeast- PH1	66.41	85	0.00	92	0.00	50	85.0	1.76	5,318	5,537	5,317	4.13	0.50	0.84	50.5	30	0.10376
Southwest- PH1	47.92	85	0.00	92	0.00	50	85.0	1.76	5,835	5,452	5,307	2.48	0.70	1.17	70.2	42	0.07488
SW-Periphery- PH3	26.09	85	0.00	92	0.00	50	85.0	1.76	4,052	5,452	5,346	2.60	0.51	0.85	51.1	31	0.04076

**Curve Number Estimation:**

*Undisturbed Native Ground*  
 Arid and semiarid rangelands  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*Compacted Cover*  
 Arid and semiarid rangelands  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*Fill*  
 OutSlope  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*TSF*  
 OutSlope  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

**Notes:**

ac = acres  
 CN = curve number  
 Wt. CN = weighted curve number  
 S = soil and cover parameter  
 L = length of longest flow path (feet)  
 H1 = elevation at top of longest flow path (feet)  
 H2 = elevation at bottom of longest flow path (feet)  
 Y = H1-H2/L x 100 = slope (%)  
 Lag = Lag time (hours)  
 t<sub>c</sub> = time of concentraton (hr or min)

Hydrologic Condition:  
 Poor: <30% ground cover  
 Fair: 30 to 70% ground cover  
 Good: >70% ground cover

Copper Flat Diversions - Hydrologic and Hydraulic Results - PMP Storm Evaluation

**Global Summary Results for Run "PMP Evaluation"**

Project: 103-92557 Phase 1 Diversions      Simulation Run: PMP Evaluation  
 Start of Run: 23Apr2013, 00:00      Basin Model: Copper Flat  
 End of Run: 28Apr2013, 00:01      Meteorologic Model: PMP  
 Compute Time: 26Apr2013, 13:07:57      Control Specifications: 5-Day

Show Elements: All Elements      Volume Units:  IN  AC-FT      Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Northeast-1	0.10376	524.666	25Apr2013, 00:35	143.715
D-1	0.10376	524.666	25Apr2013, 00:35	143.715
Southwest-1	0.07488	339.047	25Apr2013, 00:47	103.714
D-2	0.07488	339.047	25Apr2013, 00:47	103.714
Southwest Periphery	0.04076	204.294	25Apr2013, 00:36	56.456
D-3	0.04076	204.294	25Apr2013, 00:36	56.456

**Subbasin Area [Copper Flat]**

Subbasin	Area (MI2)
Northeast-1	0.10376
Southwest-1	0.07488
Southwest Periphery	0.04076

**Curve Number Loss [Copper Flat]**

Subbasin	Initial Abstraction (IN)	Curve Number
Northeast-1		85
Southwest-1		85
Southwest Periphery		85

**SCS Transform [Copper Flat]**

Subbasin	Lag Time (MIN)
Northeast-1	30
Southwest-1	42
Southwest Periphery	31

**Precipitation Parameters:**  
 Met Name: PMP  
 Probability: 0.2 Percent  
 Input Type: Partial Duration  
 Output Type: Annual Duration  
 Intensity Duration: 15 Minutes  
 Storm Duration: 4 Days  
 Intensity Position: 50 Percent  
 Storm Area (MI2): 0.7903  
 5 Minutes (IN):  
 \*15 Minutes (IN): 1.9500  
 \*1 Hour (IN): 8.0000  
 \*2 Hours (IN): 10.750  
 \*3 Hours (IN): 12.500  
 \*6 Hours (IN): 15.000  
 \*12 Hours (IN): 18.500  
 \*1 day (IN): 22.000  
 \*2 Days (IN): 24.500  
 \*4 Days (IN): 28.000  
 7 Days (IN):  
 10 Days (IN):

**Status Bar:**  
 WARNING 20045: Control specifications time intervals less than duration or maximum intensity. Precipitation data will be interpolated.  
 NOTE 40040: The basin model contains 3 outlets: D-1, D-2, D-3  
 NOTE 40049: Found no parameter problems in basin model "Copper Flat".  
 NOTE 10185: Finished computing simulation run "PMP Evaluation" at time 26Apr2013, 13:07:57.

# 103-92557 Copper Flat Stormwater Diversions (PMP) Report

Label	Solve For	Friction Method	Roughness Coefficient	Channel Slope (ft/ft)
D-1 North PMP	Normal Depth	Manning Formula	0.040	0.00500
D-2 West PMP	Normal Depth	Manning Formula	0.045	0.00500
D-3 SW Periphery PMP	Normal Depth	Manning Formula	0.045	0.00500

Normal Depth (ft)	Left Side Slope (ft/ft (H:V))	Right Side Slope (ft/ft (H:V))	Bottom Width (ft)	Discharge (ft <sup>3</sup> /s)
4.52	3.50	2.00	10.00	525.00
4.15	2.00	2.00	10.00	340.00
3.20	2.00	2.00	10.00	205.00

Flow Area (ft <sup>2</sup> )	Wetted Perimeter (ft)	Hydraulic Radius (ft)	Top Width (ft)	Critical Depth (ft)
101.29	36.54	2.77	34.85	3.27
75.89	28.55	2.66	26.59	2.73
52.54	24.32	2.16	22.81	2.04

Critical Slope (ft/ft)	Velocity (ft/s)	Velocity Head (ft)	Specific Energy (ft)	Froude Number
0.01893	5.18	0.42	4.93	0.54
0.02529	4.48	0.31	4.46	0.47
0.02713	3.90	0.24	3.44	0.45

Flow Type	Notes	Messages
-----------	-------	----------

Subcritical

Subcritical

Subcritical

**APPENDIX F.2  
DAM AND TSF UNDERDRAIN COLLECTION POND  
AREA HYDROLOGIC CALCULATIONS**

**Copper Flat Dam, Copper Flat, New Mexico  
Seepage and Runoff / Calculation of Time Runoff (t<sub>c</sub>)**

**Hydrologic and Hydraulic Calculations  
AMC II Moisture Conditions**

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile												Rational method Inputs				
	Area		Area		Area	Wt.		L	H1	H2	Y	Lag	t <sub>c</sub>	t <sub>c</sub>	Lag	Area	Area	C	Intensity	Q	
	(ac)	CN	(ac)	CN	(ac)	CN	CN	S	(ft)	(ft)	(ft)	(%)	(hr)	(hr)	(min)	(min)	(mi <sup>2</sup> )	(acre)		(in/hr)	(cfs)
<b>Phase 4</b>	71.09	85	0.00	92	91.82	50	65.3	5.32	8,640	5,385	5,170	2.49	1.71	2.85	171.0	103	0.25454	162.91	0.38	0.907	71.1

**Curve Number Estimation:**

*Undisturbed Native Ground*

Arid and semiarid rangelands

Cover type = Desert Shrub / Pinyon-Juniper

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = C

Curve number = **85**

C = **0.43**

*Compacted Cover*

Arid and semiarid rangelands

Cover type = Fill

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = D

Curve number = **92**

C = **0.46**

*TSF*

OutSlope

Cover type = Sand

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = A

Curve number = **50**

C = **0.35**

**Notes:**

ac = acres

CN = curve number

Wt. CN = weighted curve number

S = soil and cover parameter

L = length of longest flow path (feet)

H1 = elevation at top of longest flow path (feet)

H2 = elevation at bottom of longest flow path (feet)

Y = H1-H2/L x 100 = slope (%)

Lag = Lag time (hours)

t<sub>c</sub> = time of concentration (hr or min)

C = Rational Method Coefficient

C<sub>f</sub> = Correction factor for 100-year storm event (1.25)

I = rainfall intensity (inches/hour)

A = Area (acres)

Hydrologic Condition:

Poor: <30% ground cover

Fair: 30 to 70% ground cover

Good: >70% ground cover

Rational Method

Q=CiA

Q=C<sub>f</sub>CiA

i= 0.907

Includes correction factor for 100-yr storm duration.

100-year, 3 hour storm intensity

**Copper Flat Dam Toe / Seepage and Runoff  
Hydrologic and Hydraulic Results / 100-yr, 24-h Storm Volume**

**Global Summary Results for Run "100-yr, 24-hr"**

Project: 103-92557 Copper Flat      Simulation Run: 100-yr, 24-hr  
 Start of Run: 15Mar2013, 00:00      Basin Model: Basin 1  
 End of Run: 20Mar2013, 12:01      Meteorologic Model: 100 yr-24hr  
 Compute Time: 26Apr2013, 16:33:13      Control Specifications: 5 day

Show Elements: All Elements      Volume Units:  IN  AC-FT      Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Copper Flat Outslope...	0.25454	31.790	15Mar2013, 13:53	12.101

**Subbasin Area [Basin 1]**

Show Elements: All Elements      Sorting: Hydrologic

Subbasin	Area (MI2)
Copper Flat Outslo...	0.25454

**Curve Number Loss [Basin 1]**

Show Elements: All Elements      Sorting: Hydrologic

Subbasin	Initial Abstraction (IN)	Curve Number	Impervious (%)
Copper Flat Outslo...		65.3	0.0

**SCS Transform[Basin 1]**

Show Elements: All Elements      Sorting: Hydrologic

Subbasin	Lag Time (MIN)
Copper Flat Outslo...	103

**Precipitation**

Met Name: 100 yr-24hr  
 Method: Type 2  
 \*Depth (IN) 3.73

NOTE 10180: Opened meteorologic model "100 yr-24hr" at time 26Apr2013, 16:33:13.  
 NOTE 10184: Began computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.  
 NOTE 20364: Found no parameter problems in meteorologic model "100 yr-24hr".  
 NOTE 40049: Found no parameter problems in basin model "Basin 1".  
 NOTE 10185: Finished computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.

## Worksheet for Dam Toe Seepage & Runoff Collection Trench

### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.012	
Channel Slope	0.02500	ft/ft
Left Side Slope	2.00	ft/ft (H:V)
Right Side Slope	2.00	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	71.10	ft <sup>3</sup> /s

### Results

Normal Depth	0.54	ft
Flow Area	5.94	ft <sup>2</sup>
Wetted Perimeter	12.40	ft
Hydraulic Radius	0.48	ft
Top Width	12.14	ft
Critical Depth	1.08	ft
Critical Slope	0.00226	ft/ft
Velocity	11.98	ft/s
Velocity Head	2.23	ft
Specific Energy	2.77	ft
Froude Number	3.02	
Flow Type	Supercritical	

### GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

### GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.54	ft
Critical Depth	1.08	ft
Channel Slope	0.02500	ft/ft

---

## Worksheet for Dam Toe Seepage & Runoff Collection Trench

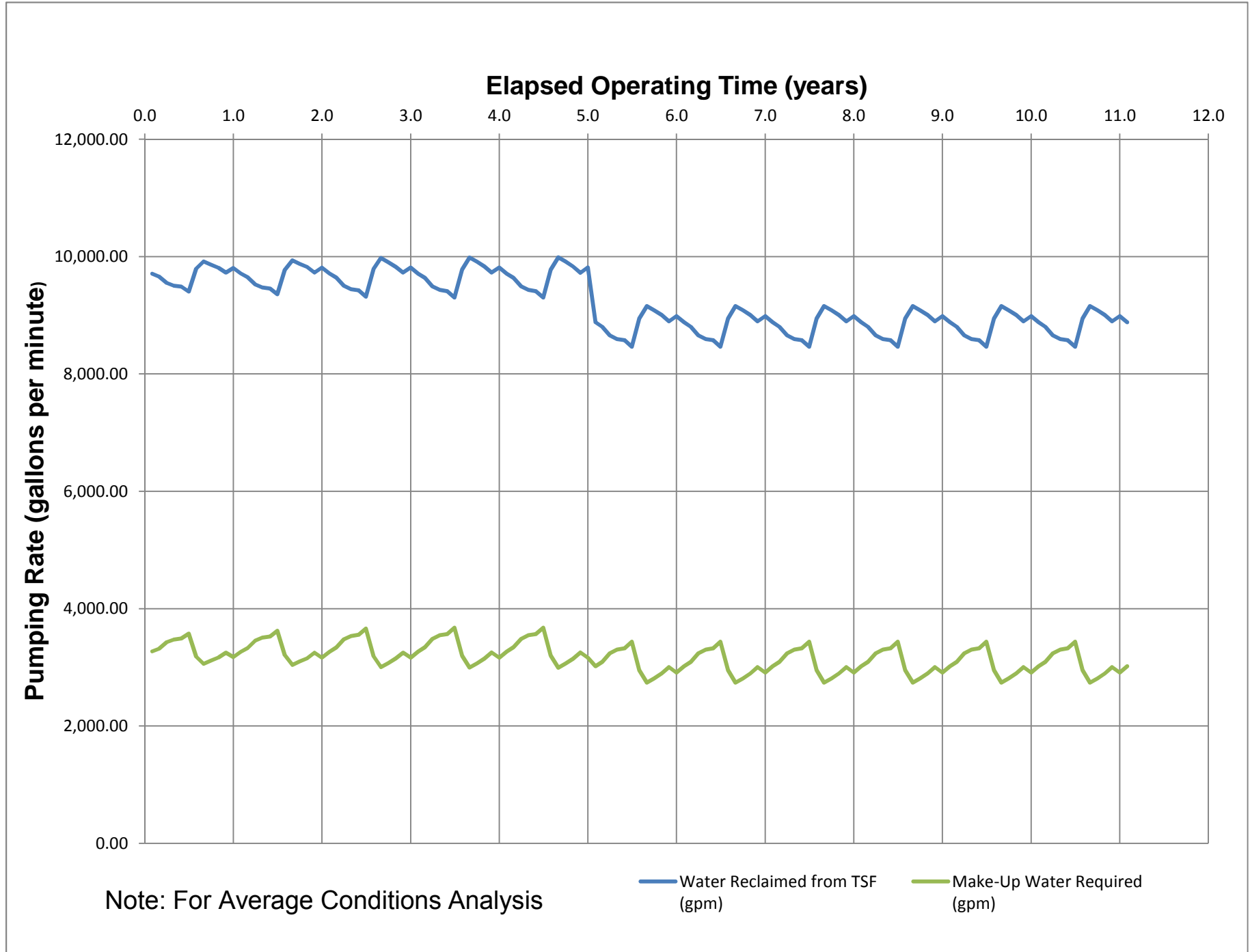
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### GVF Output Data

Critical Slope 0.00226 ft/ft



**APPENDIX G  
WATER BALANCE CALCULATIONS**



**Table 1: Water Balance Calculations**

<b>Date:</b>	12/16/2013	<b>Made by:</b>	4/29/2013 RL/GM
<b>Project No.:</b>	103-92557	<b>Checked by:</b>	7/15/2013 GM
<b>Subject:</b>	Copper Flat Climate Summary	<b>Reviewed</b>	7/15/2013 MG
<b>Project Short Title:</b>	<b>Copper Flat Feasibility Study, New Mexico</b>	<b>Page:</b>	1 of 3

Elevation at the site is approximately 5180-5270 feet.

**Average Precipitation (mm)**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,270.00	4.00
Caballo Dam (1936-2010)	0.42	0.38	0.27	0.24	0.37	0.70	1.80	2.03	1.55	0.89	0.44	0.56	9.65	4,190.00	12.30
Copper Flat Site Estimate	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,200.00	

**Average Days with Rain**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	3	3	2	2	3	3	9	9	6	4	2	3	49	5,270.00	4.00
Caballo Dam (1936-2010)	3	2	2	1	2	3	8	9	6	4	2	3	45	4,190.00	12.30
Copper Flat Site Estimate	3	3	2	2	3	3	9	9	6	4	2	3	49	5,200.00	

**Local Normal Evaporation (mm)**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
														4,190.00	
Copper Flat Site Estimate	4.22	5.57	8.55	9.52	11.15	14.25	10.34	5.94	6.18	3.96	3.68	2.74	86.10	5,385.00	

Table 2: Water Balance Calculations Year 1-5 30,000 tpd mine plan

<b>Date:</b> 12/16/2013		<b>Made by:</b> 4/29/2013 RL	
<b>Project No.:</b> 103-92557		<b>Checked by:</b> 11/15/2013 GM	
<b>Subject:</b> Tailings Properties and Flow rates		<b>Reviewed:</b> 11/15/2013 BNS	
<b>Project Short Title:</b> Copper Flat Feasibility Study, New Mexico		<b>Page:</b> 2 of 3	
<b>Year 1-5</b>			
<b>Tailings Discharge</b>		Design Discharge Rate	1,333 tph
		Cyclone Recovery	608.47 tph
		Overflow	725 tph
		Total	1333 tph
		Net (post concentrate recovery)	1316.0 tph
		Factor for average TSF inputs	99%
Cyclone feed solids	29.10%		
<b>Cyclone Discharge</b>		Result (entrainment only)	
		UF Fraction (wt%)	45.60%
		UF Solids content (wt%)	70.00%
		COF Solids content (wt%)	19.52%
		Beach to Slimes Split	60%
Year 1-5 delivery	31,992 tpd	1,333.0 tph	=input values
SG	2.64		
<b>Cyclone Feed</b>		<b>Underflow Tailings</b>	
Total (wt)	109,938 tpd	14,996 gpm	20,862 tpd
Water (wt)	77,946 tpd	12,978 gpm	6,258.55 tpd
Solids (wt)	31,992 tpd	2,018 gpm	14,603 tpd
		<b>Cyclone Overflow (Slimes)</b>	
		Total (wt)	35,633 tpd
		Water (wt)	28,677 tpd
		Solids (wt)	6,955 tpd
		<b>Cyclone Overflow (Beach)</b>	
		Total (wt)	53,449 tpd
		Water (wt)	43,016 tpd
		Solids (wt)	10,433 tpd
		Average Reclaim	9,749 gpm
		Average Make-up	3,229 gpm
<b>Post Deposition</b>		<b>Post Deposition</b>	
		Dry Unit Weight	95.00 pcf
		Vsolids	0.58 ft^3
		Vvoids	0.42 ft^3
		Void ratio	0.73
		Saturated Moisture (wt%)	28%
		Assumed Moisture (wt%)	18%
		Vwater	0.27 ft^3
		Total weight	112.10 lbs
		Entrained Water	84,250 ft^3/day
		Drainage and Evaporation	604.42 gpm
		Evaporation	156.31 gpm
		Drainage	448.11 gpm
		Dry Unit Weight	31.70 pcf
		Vsolids	0.19 ft^3
		Vvoids	0.81 ft^3
		Void ratio	4.20
		Saturated Moisture (wt%)	159%
		Assumed Moisture (wt%)	159%
		Vwater	0.81 ft^3
		Total weight	82.09 lbs
		Entrained Water	354,388 ft^3/day
		Free Water	2,934 gpm
		Dry Unit Weight	74.60 pcf
		Vsolids	0.45 ft^3
		Vvoids	0.55 ft^3
		Void ratio	1.21
		Saturated Moisture (wt%)	46%
		Assumed Moisture (wt%)	46%
		Vwater	0.55 ft^3
		Total weight	108.74 lbs
		Entrained Water	153,045 ft^3/day
		Free Water	6,367 gpm
<b>Year 6-11</b>			
<b>Tailings Discharge</b>		Design Discharge Rate	1,222 tph
		Cyclone Recovery	552.15 tph
		Overflow	670 tph
		Total	1222 tph
		Net (post concentrate recovery)	1206.0 tph
		Factor for average TSF inputs	99%
Cyclone feed solids	29.10%		
<b>Cyclone Discharge</b>		Result (entrainment only)	
		UF Fraction (wt%)	45.20%
		UF Solids content (wt%)	70.00%
		COF Solids content (wt%)	19.64%
		Beach to Slimes Split	60%
Year 6-11 delivery	29,328 tpd	1,222.0 tph	=input values
SG	2.64		
<b>Cyclone Feed</b>		<b>Underflow Tailings</b>	
Total (wt)	100,784 tpd	13,747 gpm	18,931 tpd
Water (wt)	71,456 tpd	11,897 gpm	5,679.26 tpd
Solids (wt)	29,328 tpd	1,850 gpm	13,252 tpd
		<b>Cyclone Overflow (Slimes)</b>	
		Total (wt)	32,742 tpd
		Water (wt)	26,312 tpd
		Solids (wt)	6,431 tpd
		<b>Cyclone Overflow (Beach)</b>	
		Total (wt)	49,113 tpd
		Water (wt)	39,467 tpd
		Solids (wt)	9,646 tpd
		Average Reclaim	8,922 gpm
		Average Make-up	2,975 gpm
		Dry Unit Weight	95.00 pcf
		Vsolids	0.58 ft^3
		Vvoids	0.42 ft^3
		Void ratio	0.73
		Saturated Moisture (wt%)	28%
		Assumed Moisture (wt%)	18%
		Vwater	0.27 ft^3
		Total weight	112.10 lbs
		Entrained Water	76,452 ft^3/day
		Drainage and Evaporation	548 gpm
		Evaporation	142 gpm
		Drainage	406.63 gpm
		Dry Unit Weight	31.70 pcf
		Vsolids	0.19 ft^3
		Vvoids	0.81 ft^3
		Void ratio	4.20
		Saturated Moisture (wt%)	159%
		Assumed Moisture (wt%)	159%
		Vwater	0.81 ft^3
		Total weight	82.09 lbs
		Entrained Water	327,642 ft^3/day
		Free Water	2,679 gpm
		Dry Unit Weight	74.60 pcf
		Vsolids	0.45 ft^3
		Vvoids	0.55 ft^3
		Void ratio	1.21
		Saturated Moisture (wt%)	46%
		Assumed Moisture (wt%)	46%
		Vwater	0.55 ft^3
		Total weight	108.74 lbs
		Entrained Water	141,495 ft^3/day
		Free Water	5,836 gpm

**Table 3: Water Balance Calculations Rev 1.**

<b>Date:</b>	12/16/2013	<b>Made by:</b>	4/29/2013 RL
<b>Project No.:</b>	103-92557	<b>Checked:</b>	11/15/2013 GM
<b>Subject:</b>	Average Year Rainfall Conditions	<b>Reviewed:</b>	11/15/2013 BNS
<b>Project Short Title:</b>	Copper Flat Feasibility Study, New Mexico	<b>Page:</b>	3 of 3

Month	Days	Precip. (in)	Pan evap. (in)	Storms/Month	Precip/Storm	S	Runoff/storm (in)	Monthly runoff (in)
January	31	0.58	4.22	1	0.58	0.87	0.13	0.13
February	28	0.56	5.57	1	0.56	0.87	0.12	0.12
March	31	0.38	8.55	1	0.38	0.87	0.04	0.04
April	30	0.33	9.52	1	0.33	0.87	0.02	0.02
May	31	0.53	11.15	1	0.53	0.87	0.10	0.10
June	30	0.72	14.25	1	0.72	0.87	0.21	0.21
July	31	2.33	10.34	1	2.33	0.87	1.54	1.54
August	31	2.46	5.94	3	0.82	0.87	0.28	0.83
September	30	2.11	6.18	3	0.70	0.87	0.20	0.60
October	31	1.18	3.96	3	0.39	0.87	0.04	0.13
November	30	0.55	3.68	1	0.55	0.87	0.11	0.11
December	31	0.81	2.74	1	0.81	0.87	0.27	0.27

**Variables used in Water Balance**

Water in Delivered Tailings Year 1-5	12,978	gpm
Water in Delivered Tailings Year 6-11	11,897	gpm
Beach Loss evap factor	50%	of Pan Evap
Surface area subject to wetting	40%	
Net discharge rate factor (post concentrate recovery)	99.0%	
Entrained water (Sand) Year 1-5	433	gpm
Entrained water (slimes) Year 1-5	1,822	gpm
Entrained water (beach) Year 1-5	787	gpm
Entrained water (Sand) Year 6-11	393	gpm
Entrained water (slimes) Year 6-11	1,685	gpm
Entrained water (beach) Year 6-11	728	gpm
TSF Free Water area	40%	of Tailings Area
Maximum Pond Area (Acre)	40	1,742,400
TSF Free Water Pan Evap. coefficient	75%	
Reclaim Pump capacity	12,978	gpm
Minimum Water Storage	500,000	gal
CN-Native ground areas	92	
Dam Evap (underflow water) Year 1-5	155	gpm
Dam Evap (underflow water) Year 6-11	140	gpm

Average Make-up (gpm)	
Result	3,169
Entrainment	2924
Precip/runon	2098
Evap	4931
Water in	12438

**Notes:**  
 Tailings direct precip. reports 100% to water balance  
 Impoundment refers to the interior, cyclone overflow and whole tailings storage area  
 This model does not consider whole tailings discharge which accounts for approximately 7.5% of inflow, except as noted below.  
 Average post deposition densities reflect periodic whole tailings discharge

elev	Phase	Time	Total Imp. Area	Lined Area (ft <sup>2</sup> )	Undert Area (ft <sup>2</sup> )
5250	1	0.8		8,482,454	3,369,410
5280	2	1.8		10,415,749	1,317,126
5310	3	3.0	13,385,899	12,231,496	1,154,403
5340	4	4.5	13,385,899	12,964,378	421,521
5380	5	6.7	13,385,899	13,385,899	0
5435	6	9.74	13,385,899	13,385,899	0

Year	Month	Elapsed Years	Total Impoundment Area (ft <sup>2</sup> )	Pond Area (ft <sup>2</sup> )	Beach Area (ft <sup>2</sup> )	Water Inflows			Water Losses				Monthly Balance at TSF (gal)	Water Reclaimed from TSF (gal)	End of Month Water Storage (gal)	Water Reclaimed from TSF (gpm)	Make-Up Water Required (gpm)
						Tailings Water (gal)	Direct Precip. (gal)	Runon from undervented Area (gal)	Total Entrained Water (UF) (gal)	Free Water Pond Evap (gal)	Impoundment Beach Evap. (gal)	Embankment Evap. (gal)					
1	Jan-15	0.08	8,482,454	1,742,400	6,740,054	579,339,937	3,066,903	271,525	135,827,440	3,437,732	2,659,608	6,907,787	433,845,798	433,345,798	500,000	9,708	3,270
1	Feb-15	0.16	8,482,454	1,742,400	6,740,054	523,274,781	2,961,147	249,348	122,682,849	4,537,481	3,510,430	6,239,291	390,015,225	389,515,225	500,000	9,661	3,317
1	Mar-15	0.25	8,482,454	1,742,400	6,740,054	579,339,937	2,009,350	82,934	135,827,440	6,965,074	5,388,542	6,907,787	426,843,379	426,343,379	500,000	9,551	3,427
1	Apr-15	0.33	8,482,454	1,742,400	6,740,054	560,651,552	1,744,962	49,893	131,445,910	7,755,264	5,999,873	6,684,955	411,060,405	410,560,405	500,000	9,504	3,474
1	May-15	0.41	8,482,454	1,742,400	6,740,054	579,339,937	2,802,514	217,295	135,827,440	8,083,108	7,027,162	6,907,787	424,014,249	423,514,249	500,000	9,487	3,491
1	Jun-15	0.50	8,482,454	1,742,400	6,740,054	560,651,552	3,807,189	442,457	131,445,910	11,808,456	8,980,903	6,684,955	406,880,975	406,180,975	500,000	9,402	3,576
1	Jul-15	0.58	8,482,454	1,742,400	6,740,054	579,339,937	12,320,488	3,227,141	135,827,440	8,423,259	6,516,669	6,907,787	437,712,411	437,212,411	500,000	9,794	3,184
1	Aug-15	0.67	8,482,454	1,742,400	6,740,054	579,339,937	13,007,897	1,735,434	135,827,440	4,838,893	3,743,818	6,907,787	443,265,529	442,765,529	500,000	9,919	3,059
1	Sep-15	0.75	8,482,454	1,742,400	6,740,054	560,651,552	11,157,180	1,282,444	131,445,910	5,034,404	3,894,876	6,684,955	426,511,035	426,011,035	500,000	9,861	3,117
1	Oct-15	0.83	10,415,749	1,742,400	8,673,349	579,339,937	7,661,662	108,901	135,827,440	3,225,929	3,211,617	6,907,787	438,437,727	437,937,727	500,000	9,810	3,168
1	Nov-15	0.92	10,415,749	1,742,400	8,673,349	560,651,552	3,571,114	93,230	131,445,910	2,997,833	2,984,533	6,684,955	420,702,665	420,202,665	500,000	9,727	3,251
1	Dec-15	1.00	10,415,749	1,742,400	8,673,349	579,339,937	5,259,277	220,641	135,827,440	2,232,082	2,222,179	6,907,787	438,130,366	437,630,366	500,000	9,804	3,174
2	Jan-16	1.08	10,415,749	1,742,400	8,673,349	579,339,937	3,765,902	106,141	135,827,440	3,437,732	3,422,481	6,907,787	434,116,539	433,616,539	500,000	9,714	3,284
2	Feb-16	1.16	10,415,749	1,742,400	8,673,349	523,274,781	3,636,043	97,472	122,682,849	4,537,481	4,517,350	6,239,291	389,531,325	389,031,325	500,000	9,649	3,329
2	Mar-16	1.25	10,415,749	1,742,400	8,673,349	579,339,937	2,467,315	32,420	135,827,440	6,965,074	6,934,173	6,907,787	425,705,197	425,205,197	500,000	9,525	3,453
2	Apr-16	1.33	10,415,749	1,742,400	8,673,349	560,651,552	2,142,668	19,503	131,445,910	7,755,264	7,720,857	6,684,955	409,706,738	409,206,738	500,000	9,472	3,506
2	May-16	1.41	10,415,749	1,742,400	8,673,349	579,339,937	3,441,255	84,942	135,827,440	9,083,108	9,042,811	6,907,787	422,504,988	422,004,988	500,000	9,454	3,525
2	Jun-16	1.50	10,415,749	1,742,400	8,673,349	560,651,552	4,674,912	172,960	131,445,910	11,808,456	11,556,955	6,684,955	404,703,147	404,203,147	500,000	9,357	3,621
2	Jul-16	1.58	10,415,749	1,742,400	8,673,349	579,339,937	15,128,536	1,261,512	135,827,440	8,423,259	8,385,889	6,907,787	436,685,610	436,185,610	500,000	9,771	3,207
2	Aug-16	1.67	10,415,749	1,742,400	8,673,349	579,339,937	15,972,618	678,393	135,827,440	4,838,893	4,817,426	6,907,787	444,099,402	443,599,402	500,000	9,937	3,047
2	Sep-16	1.75	10,415,749	1,742,400	8,673,349	560,651,552	13,700,091	493,500	131,445,910	5,034,404	5,012,069	6,684,955	427,167,804	426,667,804	500,000	9,877	3,101
2	Oct-16	1.83	12,231,496	1,742,400	10,489,096	579,339,937	8,997,297	95,447	135,827,440	3,225,929	3,883,962	6,907,787	439,087,563	438,587,563	500,000	9,825	3,153
2	Nov-16	1.92	12,231,496	1,742,400	10,489,096	560,651,552	4,193,656	81,712	131,445,910	2,997,833	3,609,339	6,684,955	420,688,883	420,188,883	500,000	9,727	3,251
2	Dec-16	2.00	12,231,496	1,742,400	10,489,096	579,339,937	6,176,111	193,382	135,827,440	2,232,082	2,687,388	6,907,787	438,554,733	438,054,733	500,000	9,813	3,165
3	Jan-17	2.08	12,231,496	1,742,400	10,489,096	579,339,937	4,422,400	93,028	135,827,440	3,437,732	4,138,970	6,907,787	434,043,436	433,543,436	500,000	9,712	3,266
3	Feb-17	2.16	12,231,496	1,742,400	10,489,096	523,274,781	4,269,904	85,430	122,682,849	4,537,481	5,463,048	6,239,291	389,207,446	388,707,446	500,000	9,641	3,337
3	Mar-17	2.25	12,231,496	1,742,400	10,489,096	579,339,937	2,897,435	28,414	135,827,440	6,965,074	6,885,827	6,907,787	424,679,658	424,179,658	500,000	9,502	3,476
3	Apr-17	2.33	12,231,496	1,742,400	10,489,096	560,651,552	2,516,193	17,094	131,445,910	7,755,264	7,720,857	6,684,955	408,461,509	407,961,509	500,000	9,444	3,534
3	May-17	2.41	12,231,496	1,742,400	10,489,096	579,339,937	4,041,159	74,448	135,827,440	9,083,108	10,935,904	6,907,787	421,201,305	420,701,305	500,000	9,424	3,554
3	Jun-17	2.50	12,231,496	1,742,400	10,489,096	560,651,552	5,489,876	151,592	131,445,910	11,808,456	13,976,379	6,684,955	403,077,319	402,577,319	500,000	9,319	3,659
3	Jul-17	2.58	12,231,496	1,742,400	10,489,096	579,339,937	17,765,850	1,105,660	135,827,440	8,423,259	10,141,457	6,907,787	437,411,503	436,911,503	500,000	9,787	3,191
3	Aug-17	2.67	12,231,496	1,742,400	10,489,096	579,339,937	18,757,077	594,582	135,827,440	4,838,893	5,825,943	6,907,787	445,791,532	445,291,532	500,000	9,975	3,003
3	Sep-17	2.75	12,231,496	1,742,400	10,489,096	560,651,552	16,088,387	432,531	131,445,910	5,034,404	6,061,335	6,684,955	428,445,866	427,945,866	500,000	9,906	3,072
3	Oct-17	2.83	12,231,496	1,742,400	10,489,096	579,339,937	8,997,297	95,447	135,827,440	3,225,929	3,883,962	6,907,787	439,087,563	438,587,563	500,000	9,825	3,153
3	Nov-17	2.92	12,231,496	1,742,400	10,489,096	560,651,552	4,193,656	81,712	131,445,910	2,997,833	3,609,339	6,684,955	420,688,883	420,188,883	500,000	9,727	3,251
3	Dec-17	3.00	12,231,496	1,742,400	10,489,096	579,339,937	6,176,111	193,382	135,827,440	2,232,082	2,687,388	6,907,787	438,554,733	438,054,733	500,000	9,813	3,165
4	Jan-18	3.08	12,964,378	1,742,400	11,221,978	579,339,937	4,687,380	33,968	135,827,440	3,437,732	4,428,163	6,907,787	433,960,163	433,460,163	500,000	9,710	3,268
4	Feb-18	3.16	12,964,378	1,742,400	11,221,978	523,274,781	4,525,746	31,194	122,682,849	4,537,481							

Table 3: Water Balance Calculations Rev 1.

Date:	12/16/2013	Made by:	4/29/2013 RL
Project No.:	103-92557	Checked:	11/15/2013 GM
Subject:	Average Year Rainfall Conditions	Reviewed:	11/15/2013 BNS
Project Short Title:	Copper Flat Feasibility Study, New Mexico	Page:	3 of 3

Precipitation and Evaporation Data

Month	Days	Precip. (in)	Pan evap. (in)	Storms/Month	Precip/Storm	S	Runoff/ storm (in)	Monthly runoff (in)
January	31	0.58	4.22	1	0.58	0.87	0.13	0.13
February	28	0.56	5.57	1	0.56	0.87	0.12	0.12
March	31	0.38	8.55	1	0.38	0.87	0.04	0.04
April	30	0.33	9.52	1	0.33	0.87	0.02	0.02
May	31	0.53	11.15	1	0.53	0.87	0.10	0.10
June	30	0.72	14.25	1	0.72	0.87	0.21	0.21
July	31	2.33	10.34	1	2.33	0.87	1.54	1.54
August	31	2.46	5.94	3	0.82	0.87	0.28	0.83
September	30	2.11	6.18	3	0.70	0.87	0.20	0.60
October	31	1.18	3.96	3	0.39	0.87	0.04	0.13
November	30	0.55	3.68	1	0.55	0.87	0.11	0.11
December	31	0.81	2.74	1	0.81	0.87	0.27	0.27

Average Make-up (gpm)  
 Result 3,169  
 Entrainment 2924  
 Precip/runon 2098  
 Evap 4931  
 Water in 12438

Notes:  
 Tailings direct precip. reports 100% to water balance  
 Impoundment refers to the interior, cyclone overflow and whole tailings storage area  
 This model does not consider whole tailings discharge which accounts for approximately 7.5% of inflow, except as noted below.  
 Average post deposition densities reflect periodic whole tailings discharge

Variables used in Water Balance

Water in Delivered Tailings Year 1-5	12,978	gpm
Water in Delivered Tailings Year 6-11	11,897	gpm
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Surface area subject to wetting	40%	
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Entrained water (slimes) Year 1-5	1,822	gpm
Entrained water (beach) Year 1-5	787	gpm
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Entrained water (slimes) Year 6-11	1,685	gpm
Entrained water (beach) Year 6-11	728	gpm
TSF Free Water area	40%	of Tailings Area
Maximum Pond Area (Acre)	40	1,742,400 ft <sup>2</sup>
TSF Free Water Pan Evap. coefficient	75%	
Reclaim Pump capacity	12,978	gpm
Minimum Water Storage	500,000	gal
CN-Native ground areas	92	
Dam Evap (underflow water) Year 1-5	155	gpm
Dam Evap (underflow water) Year 6-11	140	gpm

elev	Phase	Time	Total Imp. Area	Lined Area (ft <sup>2</sup> )	Undiverted Area (ft <sup>2</sup> )
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5280	2	1.8		10,415,749	1,317,126
5310	3	3.0	13,385,899	12,231,496	1,154,403
5340	4	4.5	13,385,899	12,964,378	421,521
5380	5	6.7	13,385,899	13,385,899	0
5435	6	9.74	13,385,899	13,385,899	

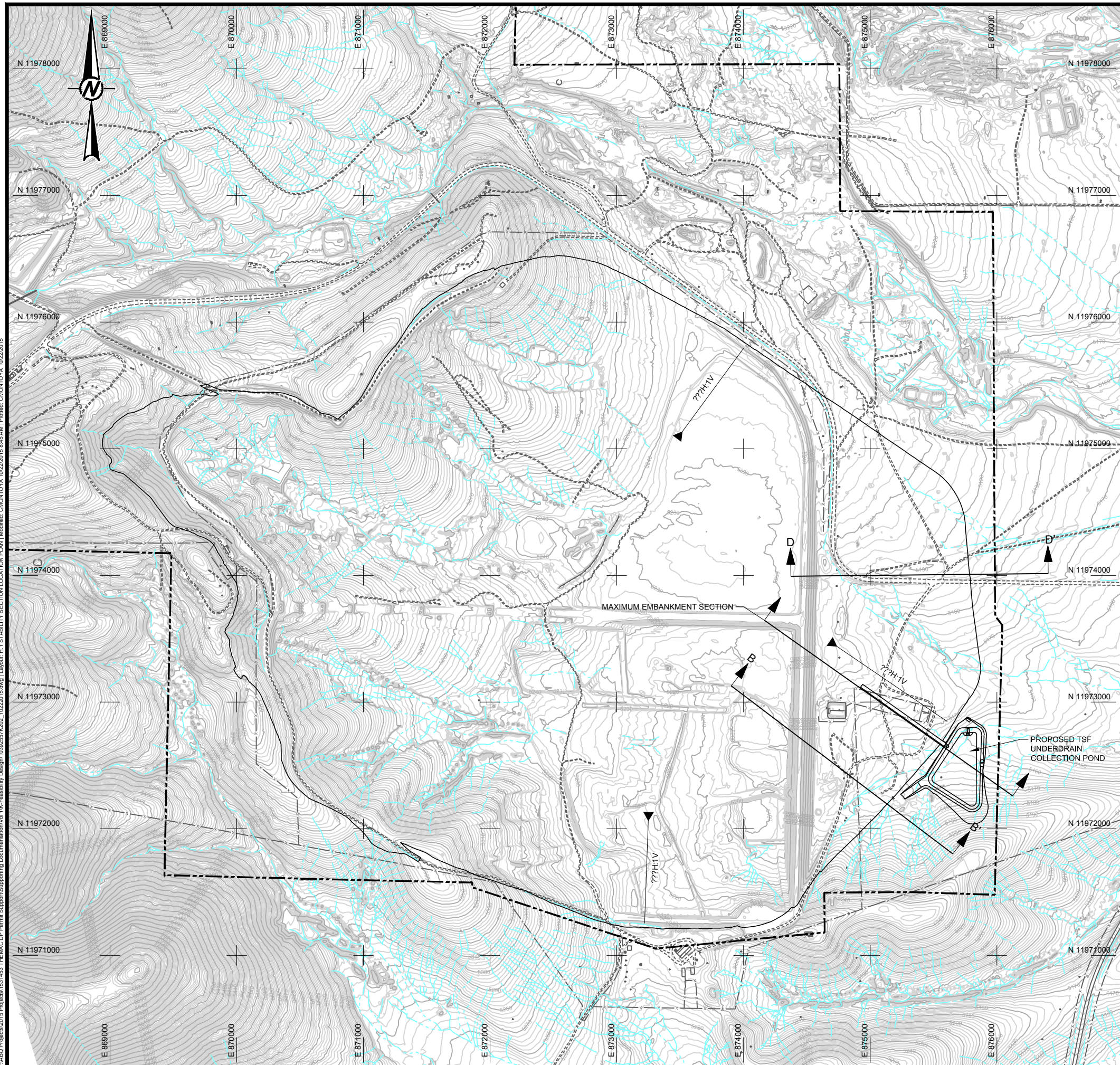
Results Average Reclaim 9,215 gpm  
 Average Make-up 3,169 gpm

Year	Month	Elapsed Years	Total Impoundment Area (ft <sup>2</sup> )	Pond Area (ft <sup>2</sup> )	Beach Area (ft <sup>2</sup> )	Water Inflows			Water Losses				Monthly Balance at TSF (gal)	Water Reclaimed from TSF (gal)	End of Month Water Storage (gal)	Water Reclaimed from TSF (gpm)	Make-Up Water Required (gpm)	
						Tailings Water (gal)	Direct Precip. (gal)	Runon from undiverted Area (gal)	Total Entrained Water (UF) (gal)	Free Water Pond Evap (gal)	Impoundment Beach Evap. (gal)	Embankment Evap. (gal)						
10	Jan-24	9.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
	Feb-24	9.16	13,385,899	1,742,400	11,643,499	479,701,263	4,672,895	-	113,125,473	4,537,481	6,064,297	5,661,782	355,485,125	354,985,125	500,000	8,804	3,093	
	Mar-24	9.25	13,385,899	1,742,400	11,643,499	531,097,826	3,170,893	-	125,246,059	6,965,074	9,308,750	6,268,402	386,980,435	386,480,435	500,000	8,658	3,240	
	Apr-24	9.33	13,385,899	1,742,400	11,643,499	513,965,638	2,753,670	-	121,205,864	7,755,264	10,364,830	6,066,195	371,827,156	371,327,156	500,000	8,596	3,302	
	May-24	9.41	13,385,899	1,742,400	11,643,499	531,097,826	4,422,562	-	125,246,059	9,083,108	12,139,481	6,268,402	383,283,338	382,783,338	500,000	8,575	3,322	
	Jun-24	9.50	13,385,899	1,742,400	11,643,499	513,965,638	6,008,008	-	121,205,864	11,608,456	15,514,583	6,066,195	366,078,548	365,578,548	500,000	8,462	3,435	
	Jul-24	9.58	13,385,899	1,742,400	11,643,499	531,097,826	19,442,582	-	125,246,059	8,423,259	11,257,599	6,268,402	399,845,090	399,345,090	500,000	8,946	2,951	
	Aug-24	9.67	13,385,899	1,742,400	11,643,499	531,097,826	20,527,362	-	125,246,059	4,838,893	6,467,132	6,268,402	409,304,702	408,804,702	500,000	9,158	2,740	
	Sep-24	9.75	13,385,899	1,742,400	11,643,499	513,965,638	17,606,802	-	121,205,864	5,034,404	6,728,430	6,066,195	393,037,547	392,537,547	500,000	9,087	2,811	
	Oct-24	9.83	13,385,899	1,742,400	11,643,499	531,097,826	9,846,458	-	125,246,059	3,225,929	4,311,421	6,268,402	402,392,473	401,892,473	500,000	9,003	2,894	
	Nov-24	9.92	13,385,899	1,742,400	11,643,499	513,965,638	4,589,451	-	121,205,864	2,997,833	4,006,573	6,066,195	384,778,624	384,278,624	500,000	8,895	3,002	
	Dec-24	10.00	13,385,899	1,742,400	11,643,499	531,097,826	6,759,009	-	125,246,059	2,232,082	2,983,155	6,268,402	401,627,138	401,127,138	500,000	8,986	2,912	
11	Jan-25	10.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
	Feb-25	10.16	13,385,899	1,742,400	11,643,499	479,701,263	4,672,895	-	113,125,473	4,537,481	6,064,297	5,661,782	355,485,125	354,985,125	500,000	8,804	3,093	
	Mar-25	10.25	13,385,899	1,742,400	11,643,499	531,097,826	3,170,893	-	125,246,059	6,965,074	9,308,750	6,268,402	386,980,435	386,480,435	500,000	8,658	3,240	
	Apr-25	10.33	13,385,899	1,742,400	11,643,499	513,965,638	2,753,670	-	121,205,864	7,755,264	10,364,830	6,066,195	371,827,156	371,327,156	500,000	8,596	3,302	
	May-25	10.41	13,385,899	1,742,400	11,643,499	531,097,826	4,422,562	-	125,246,059	9,083,108	12,139,481	6,268,402	383,283,338	382,783,338	500,000	8,575	3,322	
	Jun-25	10.50	13,385,899	1,742,400	11,643,499	513,965,638	6,008,008	-	121,205,864	11,608,456	15,514,583	6,066,195	366,078,548	365,578,548	500,000	8,462	3,435	
	Jul-25	10.58	13,385,899	1,742,400	11,643,499	531,097,826	19,442,582	-	125,246,059	8,423,259	11,257,599	6,268,402	399,845,090	399,345,090	500,000	8,946	2,951	
	Aug-25	10.67	13,385,899	1,742,400	11,643,499	531,097,826	20,527,362	-	125,246,059	4,838,893	6,467,132	6,268,402	409,304,702	408,804,702	500,000	9,158	2,740	
	Sep-25	10.75	13,385,899	1,742,400	11,643,499	513,965,638	17,606,802	-	121,205,864	5,034,404	6,728,430	6,066,195	393,037,547	392,537,547	500,000	9,087	2,811	
	Oct-25	10.83	13,385,899	1,742,400	11,643,499	531,097,826	9,846,458	-	125,246,059	3,225,929	4,311,421	6,268,402	402,392,473	401,892,473	500,000	9,003	2,894	
	Nov-25	10.92	13,385,899	1,742,400	11,643,499	513,965,638	4,589,451	-	121,205,864	2,997,833	4,006,573	6,066,195	384,778,624	384,278,624	500,000	8,895	3,002	
	Dec-25	11.00	13,385,899	1,742,400	11,643,499	531,097,826	6,759,009	-	125,246,059	2,232,082	2,983,155	6,268,402	401,627,138	401,127,138	500,000	8,986	2,912	
12	Jan-26	11.08	13,385,899	1,742,400	11,643,499	531,097,826	4,839,784	-	125,246,059	3,437,732	4,594,494	6,268,402	396,890,923	396,390,923	500,000	8,880	3,018	
						<b>542,619,654</b>	<b>8,173,890</b>		127,890	127,590,729	5,826,860	7,227,106	6,434,351	404,240,459	403,740,459	500,000	<b>9,215</b>	<b>3,169</b>

Average (gpm) 12,368      186      3      2,909      133      165      147      9,215  
 Average Make-up water 3,169 gpm  
 Maximum Make-up water 3,676 gpm

**APPENDIX H  
STABILITY ANALYSIS SUPPORTING DATA  
AND COMPUTER OUTPUT**

P:\ABO Projects\2015 Projects\1531453 THE MAC DP Permit Support\Supporting Documentation\Vol 1\Facility Design\10392557\202\_10222015.dwg | Layout: H.1 STABILITY SECTION LOCATION PLAN | Modified: CMONTOYA\_10222015 8:45 AM | Plotted: CMONTOYA\_10222015

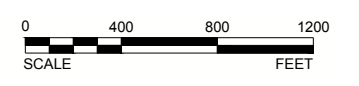


**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE STABILITY SECTION LOCATION

**NOTES**

- TAILINGS STORAGE FACILITY SHOWN AT FINAL BUILDOUT.



PROJECT  
**THE MAC** RESOURCES  
Environmentally Responsible. Community-Minded. Local Opportunities.

NEW MEXICO COPPER CORPORATION

DRAFT COPPER FLAT PROJECT  
 30 TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

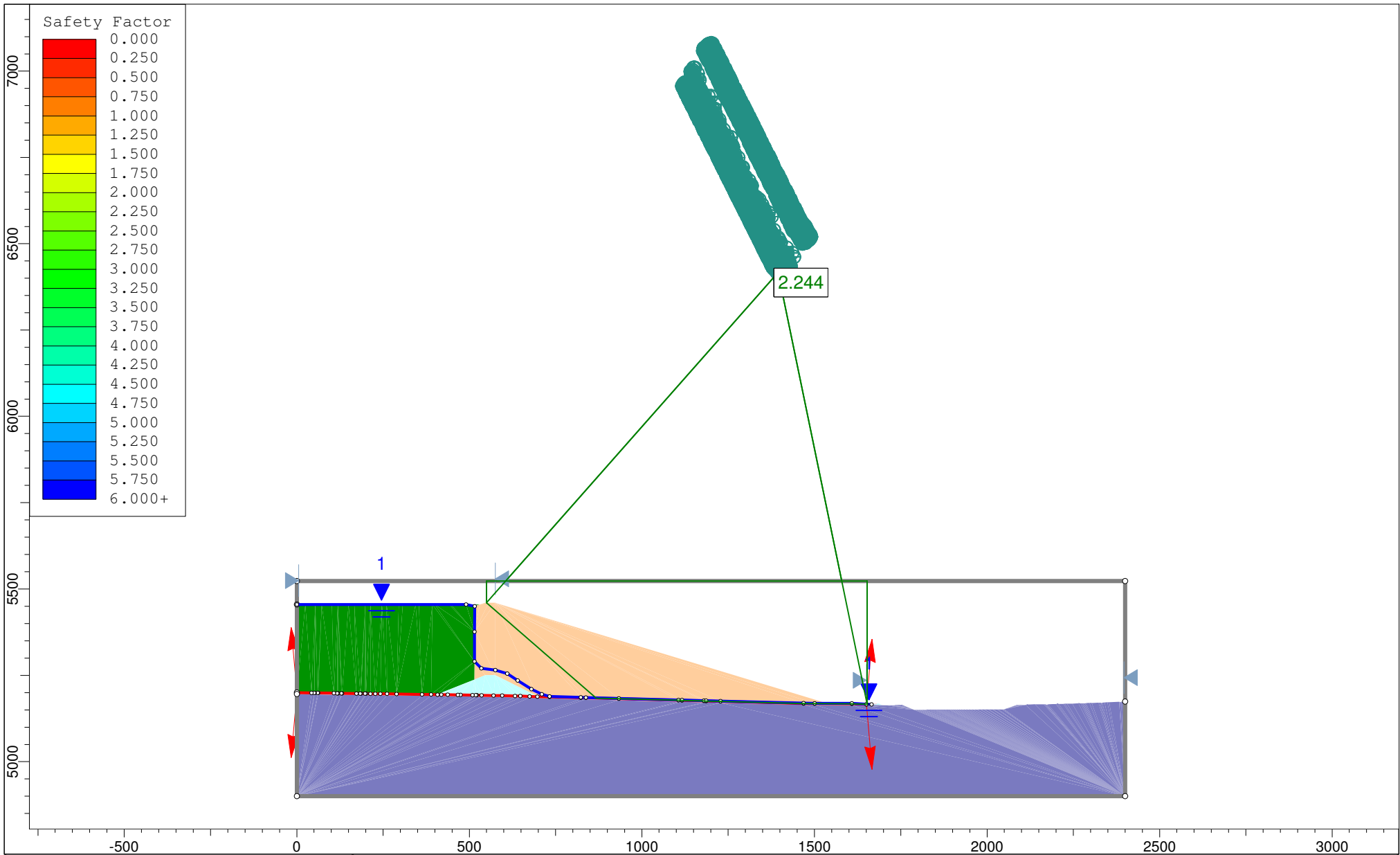
TITLE

**STABILITY SECTION LOCATION PLAN**

	PROJECT No.	103-92557	FILE No.	10392557K202_10222015	
	DESIGN	DMW	2013-04-30	SCALE	AS SHOWN
	CADD	JHR	2013-04-30	DRAWING	
	CHECK	GM	2013-05-07		
	REVIEW	DAK	2013-05-07		



**APPENDIX H.1  
MAXIMUM EMBANKMENT SECTION**



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Block Failure, Global	
Drawn By	GS	Scale	1:4620
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	1a - SectionA 5460R DS_S_B_G.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 1a - SectionA 5460R DS\_S\_B\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.244240  
 Axis Location: 1394.865, 6416.541  
 Left Slip Surface Endpoint: 549.279, 5460.000  
 Right Slip Surface Endpoint: 1652.746, 5166.147  
 Left Slope Intercept: 549.279 5522.990  
 Right Slope Intercept: 1652.746 5522.990  
 Resisting Moment=8.08765e+009 lb-ft

Driving Moment=3.60374e+009 lb-ft  
 Resisting Horizontal Force=5.71009e+006 lb  
 Driving Horizontal Force=2.54433e+006 lb

## Global Minimum Coordinates

### Method: spencer

X	Y
549.279	5460
868.036	5183.72
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1608.27	5167.61
1623.49	5166.77
1652.75	5166.15
1652.75	5522.99

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 757  
 Number of Invalid Surfaces: 4064

#### Error Codes:

- Error Code -108 reported for 7 surfaces
- Error Code -111 reported for 2282 surfaces
- Error Code -112 reported for 19 surfaces
- Error Code -116 reported for 1756 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1+\tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

**Slice Data**

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	52.9277	124924	Cyclone Underflow	0	39	591.131	1326.64	1638.28	0	1638.28
2	52.9277	315093	Cyclone Underflow	0	39	1658.67	3722.45	4596.85	0	4596.85
3	52.9277	493540	Cyclone Underflow	0	39	2613.17	5864.58	7242.16	0	7242.16
4	52.9277	671986	Cyclone Underflow	0	39	3567.68	8006.72	9887.48	0	9887.48
5	52.9277	850433	Cyclone Underflow	0	39	4522.2	10148.9	12532.8	0	12532.8
6	52.9277	1.02888e+006	Cyclone Underflow	0	39	5476.69	12291	15178.1	0	15178.1
7	1.19075	25204	Liner Interface Zone	0	26.5	4101.89	9205.63	18494.8	31.1774	18463.6
8	65.4393	1.32103e+006	Liner Interface Zone	0	26.5	4296.98	9643.45	19404.1	62.3547	19341.7
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	3873.77	8693.66	17499.2	62.3583	17436.9
10	57.4662	946268	Liner Interface Zone	0	26.5	3476.66	7802.45	15711.7	62.3583	15649.3
11	57.4662	842833	Liner Interface Zone	0	26.5	3079.55	6911.24	13924.2	62.3583	13861.8
12	10.1889	138642	Liner Interface Zone	0	26.5	2845.74	6386.52	12871.8	62.3576	12809.4
13	63.358	789325	Liner Interface Zone	0	26.5	2592.07	5817.23	11729.9	62.3559	11667.6
14	6.66227	75711.8	Liner Interface Zone	0	26.5	2350.93	5276.05	10644.5	62.3581	10582.1
15	41.4898	440289	Liner Interface Zone	0	26.5	2184.77	4903.15	9896.55	62.3559	9834.2
16	48.1756	443630	Liner Interface Zone	0	26.5	1875.42	4208.9	8504.11	62.3594	8441.75
17	48.1756	370848	Liner Interface Zone	0	26.5	1542.11	3460.86	7003.76	62.3594	6941.4
18	48.1756	298067	Liner Interface Zone	0	26.5	1208.79	2712.81	5503.4	62.3594	5441.04
19	48.1756	225285	Liner Interface Zone	0	26.5	875.468	1964.76	4003.07	62.3594	3940.71
20	48.1756	152503	Liner Interface Zone	0	26.5	542.148	1216.71	2502.71	62.3594	2440.36
21	31.9454	59717.2	Liner Interface Zone	0	26.5	256.986	576.739	1219.16	62.3993	1156.76

22	53.9361	28069.4	Liner Interface Zone	0	26.5	6.26742	14.0656	90.6102	62.3991	28.2111
23	53.9361	6472.33	Liner Interface Zone	0	26.5	10.742	24.1077	110.752	62.3991	48.3526
24	15.217	1826.04	Liner Interface Zone	0	26.5	0	0	0.432806	62.2097	-61.7769
25	29.2604	1755.63	Liner Interface Zone	0	26.5	2857.95	6413.92	12895.4	31.1049	12864.3

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	549.279	5460	1.98387e-022	0	0
2	602.206	5414.12	-113840	11116	-5.57701
3	655.134	5368.25	-69899.2	6825.37	-5.57702
4	708.062	5322.37	28843.8	-2816.47	-5.577
5	760.99	5276.5	182389	-17809.5	-5.57701
6	813.917	5230.62	390735	-38153.7	-5.57702
7	866.845	5184.75	653884	-63849.1	-5.57702
8	868.036	5183.72	664679	-64903.2	-5.57702
9	933.475	5181.95	217883	-21275.3	-5.577
10	990.941	5180.47	-174451	17034.4	-5.57701
11	1048.41	5178.98	-565551	55223.8	-5.57702
12	1105.87	5177.5	-955418	93292.6	-5.57701
13	1116.06	5177.23	-1.02438e+006	100027	-5.57705
14	1179.42	5175.55	-1.45202e+006	141784	-5.57703
15	1186.08	5175.37	-1.49696e+006	146172	-5.57702
16	1227.57	5174.27	-1.77617e+006	173435	-5.57699
17	1275.75	5173.04	-2.10008e+006	205064	-5.57701
18	1323.92	5171.81	-2.42308e+006	236604	-5.57702
19	1372.1	5170.58	-2.74518e+006	268055	-5.57701
20	1420.28	5169.35	-3.06636e+006	299418	-5.57703
21	1468.45	5168.13	-3.38664e+006	330691	-5.57701
22	1500.4	5168.02	-3.59915e+006	351442	-5.57701
23	1554.33	5167.81	-3.83666e+006	374634	-5.57702
24	1608.27	5167.61	-3.84174e+006	375130	-5.57702
25	1623.49	5166.77	-3.86037e+006	376950	-5.57703
26	1652.75	5166.15	6.36684e-021	0	0

### List Of Coordinates

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63



632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5

0	5195.5
---	--------

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23

1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54

1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6

2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**



X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

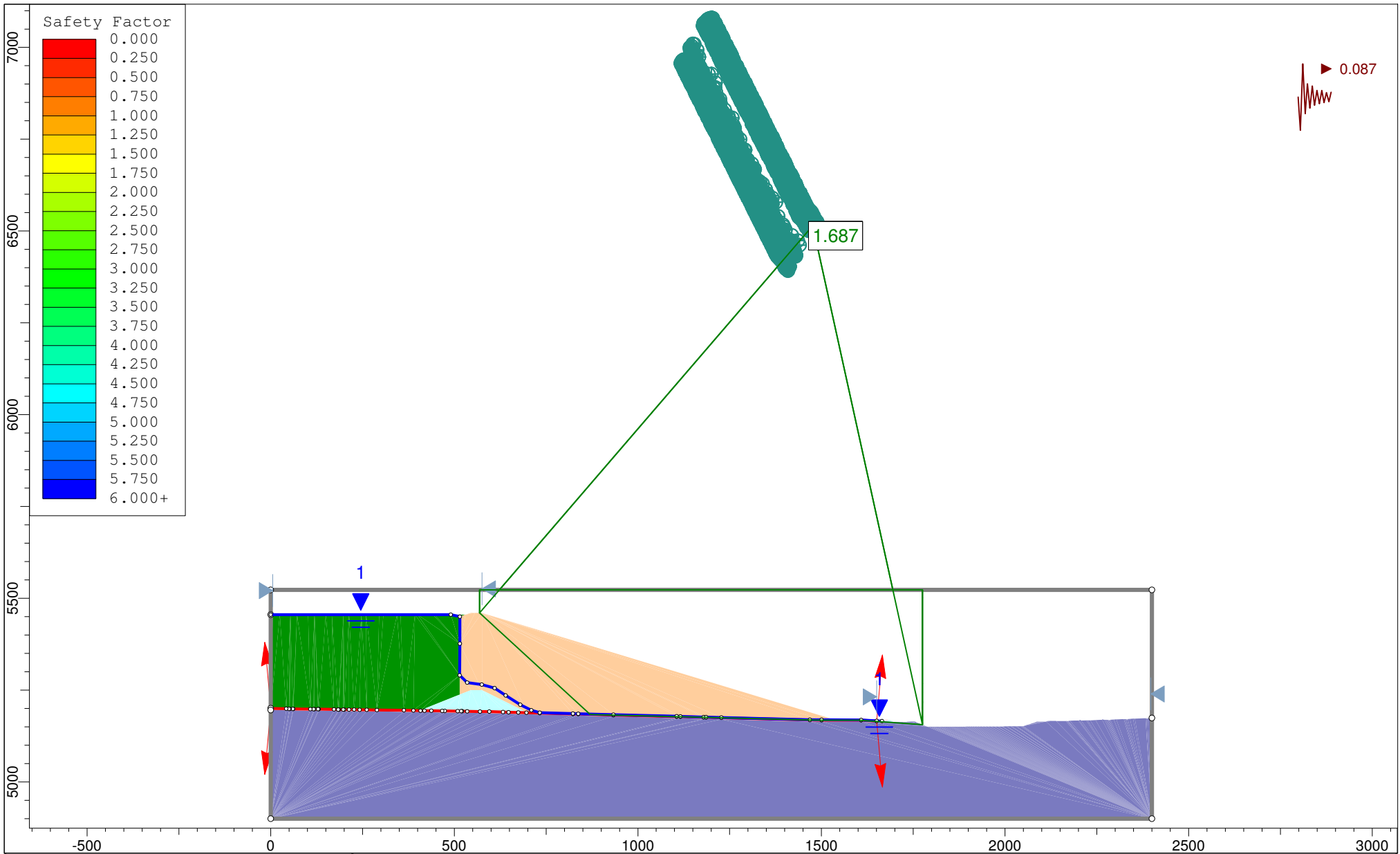
**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32





SLIDEINTERPRET 6.008

<i>Project</i>			
Copper Flat			
<i>Analysis Description</i>			
Section A Stability: Downstream, Pseudo Static, Block Failure, Global			
<i>Drawn By</i>	GS	<i>Scale</i>	1:4338
<i>Company</i>	Golder Associates Inc.		
<i>Date</i>	11/4/2013, 3:02:21 PM	<i>File Name</i>	2a - SectionA 5460R DS_PS_B_G.slim

# **Slide Analysis Information**

## **Copper Flat**

### **Project Summary**

---

File Name: 2a - SectionA 5460R\_DS\_PS\_B\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

### **Analysis Options**

---

#### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

## Random Numbers

---

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

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Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined







## Loading

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Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

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Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

---

Method: spencer

FS: 1.686520

Axis Location: 1476.350, 6514.282  
 Left Slip Surface Endpoint: 568.826, 5460.000  
 Right Slip Surface Endpoint: 1775.261, 5155.693  
 Left Slope Intercept: 568.826 5522.990  
 Right Slope Intercept: 1775.261 5522.990  
 Resisting Moment=8.24296e+009 lb-ft  
 Driving Moment=4.88754e+009 lb-ft  
 Resisting Horizontal Force=5.38918e+006 lb  
 Driving Horizontal Force=3.19543e+006 lb

**Global Minimum Coordinates**

---

**Method: spencer**

X	Y
568.826	5460
870.106	5183.66
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1601.92	5167.63
1775.26	5155.69
1775.26	5522.99

**Valid / Invalid Surfaces**

---

**Method: spencer**

Number of Valid Surfaces: 518  
 Number of Invalid Surfaces: 4303

**Error Codes:**

- Error Code -108 reported for 22 surfaces
- Error Code -111 reported for 2509 surfaces
- Error Code -112 reported for 16 surfaces
- Error Code -116 reported for 1756 surfaces

**Error Codes**

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

- 111 = safety factor equation did not converge
- 112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

## Slice Data

### Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	50.0261	97432.4	Cyclone Underflow	0	39	554.036	934.392	1153.88	0	1153.88
2	50.0261	271871	Cyclone Underflow	0	39	1829.51	3085.51	3810.28	0	3810.28
3	50.0261	445560	Cyclone Underflow	0	39	3022.48	5097.47	6294.86	0	6294.86
4	50.0261	619248	Cyclone Underflow	0	39	4215.44	7109.43	8779.42	0	8779.42
5	50.0261	792937	Cyclone Underflow	0	39	5408.41	9121.4	11264	0	11264
6	50.0261	966626	Cyclone Underflow	0	39	6601.4	11133.4	13748.6	0	13748.6
7	1.12325	23701.6	Liner Interface Zone	0	26.5	5119.91	8634.83	17350	31.1774	17318.8
8	63.369	1.27719e+006	Liner Interface Zone	0	26.5	5707.87	9626.44	19369.9	62.3547	19307.6
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	5154.91	8693.86	17499.6	62.3583	17437.2
10	57.4662	946268	Liner Interface Zone	0	26.5	4626.75	7803.1	15713	62.3583	15650.6
11	57.4662	842833	Liner Interface Zone	0	26.5	4098.59	6912.35	13926.4	62.3583	13864
12	10.1889	138642	Liner Interface Zone	0	26.5	3787.56	6387.79	12874.2	62.3576	12811.9
13	63.358	789325	Liner Interface Zone	0	26.5	3450.05	5818.58	11732.6	62.3559	11670.3
14	6.66227	75711.8	Liner Interface Zone	0	26.5	3129.49	5277.95	10648.3	62.3581	10585.9
15	41.4898	440289	Liner Interface Zone	0	26.5	2908.36	4905.01	9900.29	62.3559	9837.93
16	60.2195	543166	Liner Interface Zone	0	26.5	2441.73	4118.02	8321.82	62.3594	8259.46
17	60.2195	429444	Liner Interface Zone	0	26.5	1887.56	3183.41	6447.29	62.3594	6384.93
18	60.2195	315723	Liner Interface Zone	0	26.5	1333.4	2248.8	4572.76	62.3594	4510.4
			Liner Interface							

		Zone								
20	31.9454	59717.2	Liner Interface Zone	0	26.5	345.09	582.001	1229.71	62.3993	1167.31
21	50.7606	27688.4	Liner Interface Zone	0	26.5	9.96282	16.8025	96.0997	62.3991	33.7006
22	50.7606	6091.28	Liner Interface Zone	0	26.5	14.4972	24.4499	111.438	62.3991	49.0388
23	62.898	13110.9	Liner Interface Zone	0	26.5	0	0	102.782	109.089	-6.30725
24	55.2228	27974.9	Foundation Materials	150	29	213.592	360.228	379.262	0	379.262
25	55.2228	30469.2	Foundation Materials	150	29	2490.45	4200.2	7306.76	0	7306.76

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	568.826	5460	1.98387e-022	0	0
2	618.852	5414.12	-147187	14297.2	-5.54809
3	668.878	5368.23	-119420	11600	-5.54809
4	718.904	5322.35	-36557.4	3551.05	-5.54809
5	768.93	5276.46	101402	-9849.77	-5.54807
6	818.957	5230.58	294457	-28602.4	-5.54808
7	868.983	5184.69	542608	-52706.9	-5.54809
8	870.106	5183.66	553563	-53771	-5.54809
9	933.475	5181.95	142080	-13801.1	-5.54808
10	990.941	5180.47	-232631	22596.9	-5.54809
11	1048.41	5178.98	-607567	59016.7	-5.54808
12	1105.87	5177.5	-982726	95458.3	-5.54809
13	1116.06	5177.23	-1.04924e+006	101919	-5.54808
14	1179.42	5175.55	-1.46262e+006	142073	-5.54807
15	1186.08	5175.37	-1.50616e+006	146303	-5.5481
16	1227.57	5174.27	-1.77711e+006	172622	-5.5481
17	1287.79	5172.73	-2.17125e+006	210907	-5.54809
18	1348.01	5171.2	-2.56556e+006	249209	-5.54809
19	1408.23	5169.66	-2.96006e+006	287529	-5.54809
20	1468.45	5168.13	-3.35475e+006	325867	-5.54808
21	1500.4	5168.02	-3.56488e+006	346279	-5.54809
22	1551.16	5167.82	-3.79988e+006	369105	-5.54807
23	1601.92	5167.63	-3.80432e+006	369537	-5.54809
24	1664.82	5163.3	-3.87274e+006	376183	-5.54808
25	1720.04	5159.5	-3.93678e+006	382404	-5.54809
26	1775.26	5155.69	6.74534e-021	0	0

## List Of Coordinates

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

### Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92

732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98



2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92

837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5

1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61

2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83

2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199

117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8

1675.27	5168.86
1680.63	5168.86

**Material Boundary**

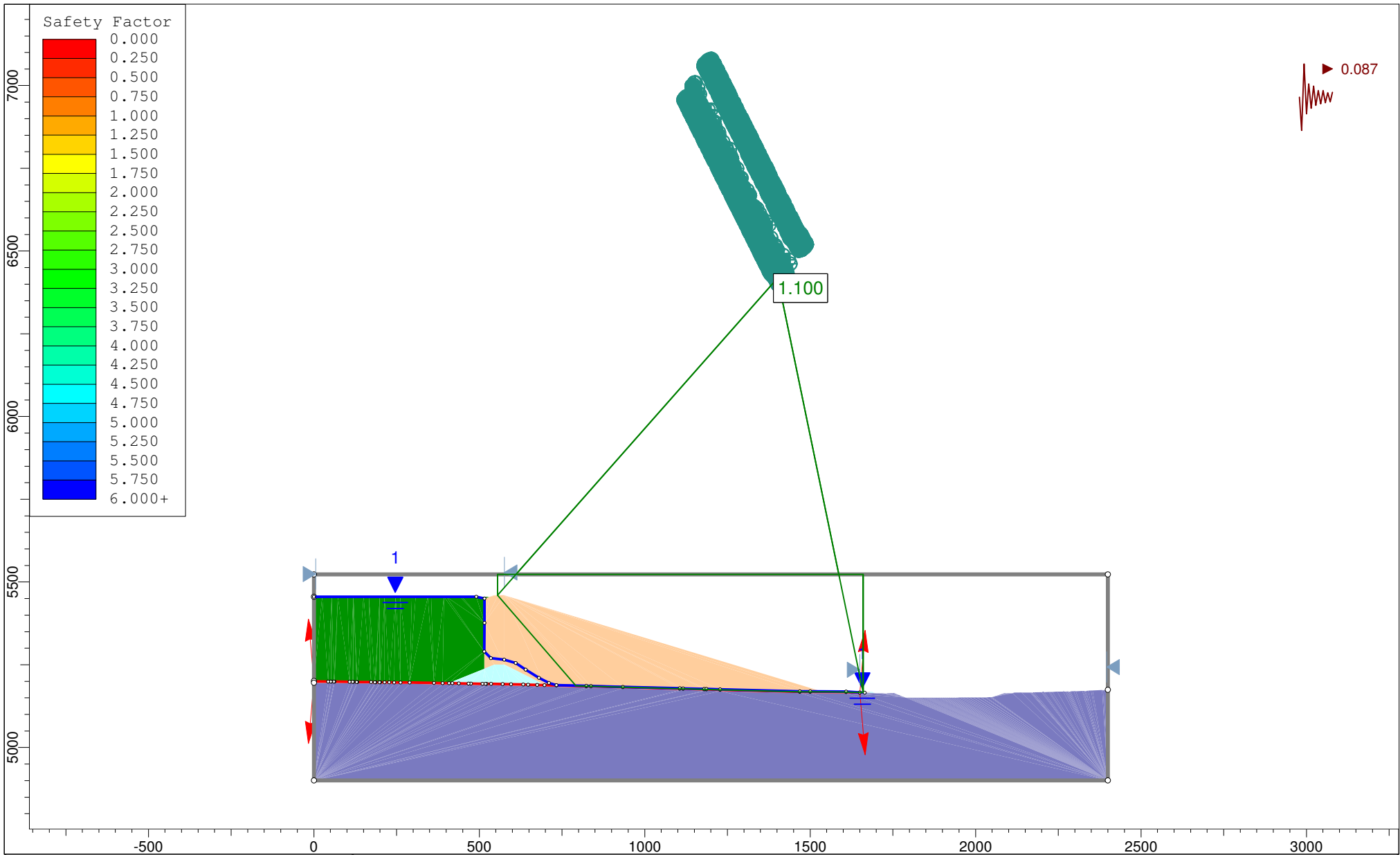
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

<i>Project</i>				Copper Flat			
<i>Analysis Description</i>				Section A Stability: Downstream, Pseudo Static, Block Failure, Global			
<i>Drawn By</i>		GS		<i>Scale</i>		1:4820	
<i>Date</i>				11/4/2013, 3:02:21 PM		<i>Company</i>	
						Golder Associates Inc.	
						<i>File Name</i>	
						3a - SectionA 5460R DS_PS_B_G_min_phi.slim	



# Slide Analysis Information

## Copper Flat

### Project Summary

---

File Name: 3a - SectionA 5460R\_DS\_PS\_B\_G\_min\_phi.slim  
Last saved with Slide version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:

103-92557  
Material Property Edits 12/2013  
Liner interface min phi=13.6deg

### General Settings

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

### Analysis Options

---

#### Analysis Methods Used

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

### Groundwater Analysis

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3







## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## List Of Coordinates

### Piezoline

X	Y

0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03

595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

### Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**





X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**



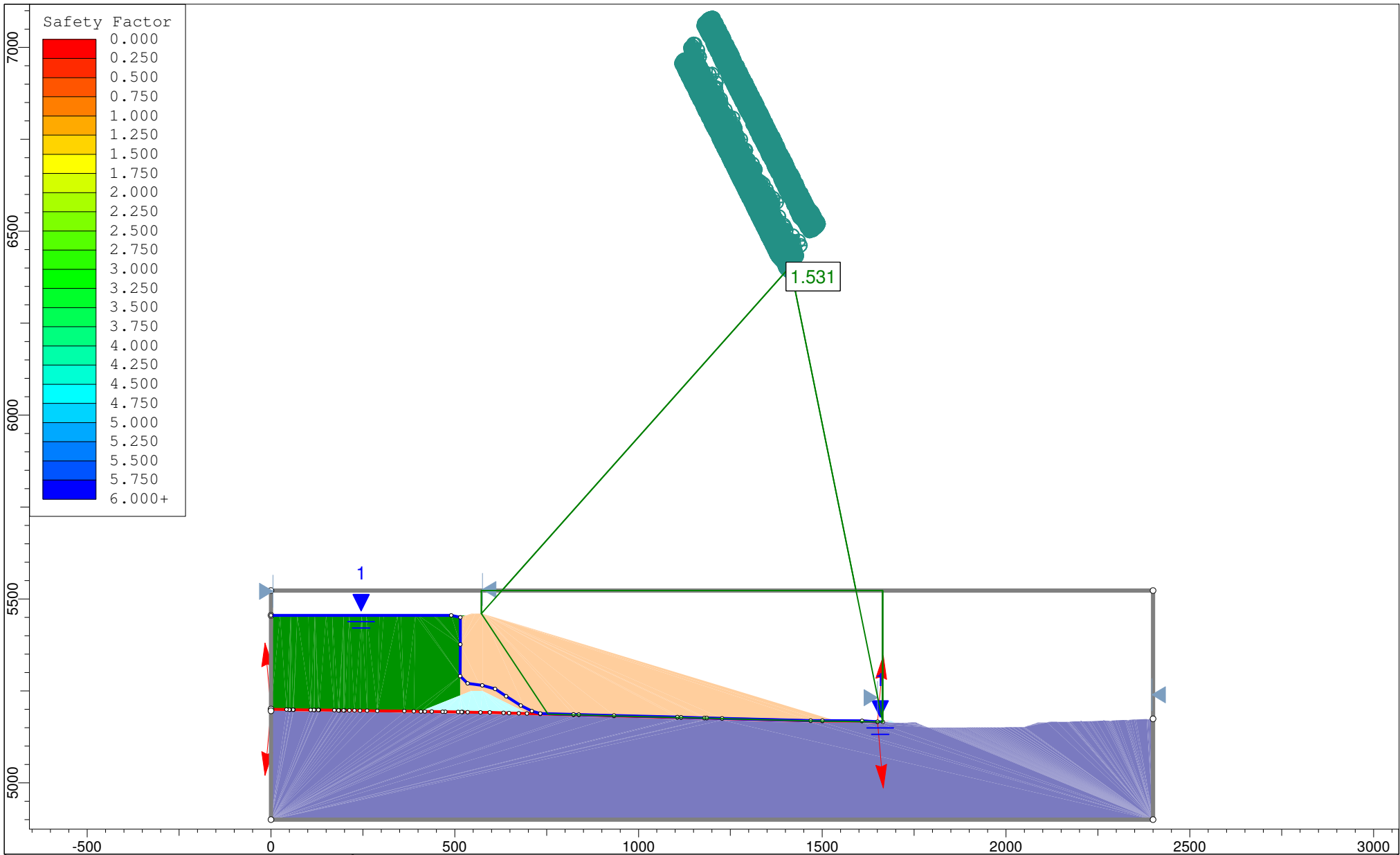
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Block Failure, Global	
Drawn By	GS	Scale	1:4338
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	3b - SectionA 5460R DS_S_B_G_min_phi.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 3b - SectionA 5460R DS\_S\_B\_G\_min\_phi.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:

103-92557  
Material Property Edits 12/2013  
Liner Interface phi =13.6deg

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 1.530610  
 Axis Location: 1412.961, 6404.397  
 Left Slip Surface Endpoint: 572.648, 5460.000  
 Right Slip Surface Endpoint: 1664.291, 5165.509  
 Left Slope Intercept: 572.648 5522.990  
 Right Slope Intercept: 1664.291 5522.990  
 Resisting Moment=5.18841e+009 lb-ft

Driving Moment=3.38976e+009 lb-ft  
 Resisting Horizontal Force=3.37882e+006 lb  
 Driving Horizontal Force=2.20749e+006 lb

## Global Minimum Coordinates

### Method: spencer

X	Y
572.648	5460
753.658	5186.79
823.107	5184.92
837.37	5184.54
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1456.16	5168.44
1664.29	5165.51
1664.29	5522.99

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 1279  
 Number of Invalid Surfaces: 3542

#### Error Codes:

Error Code -108 reported for 11 surfaces  
 Error Code -111 reported for 1763 surfaces  
 Error Code -112 reported for 12 surfaces  
 Error Code -116 reported for 1756 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha =  $\cos(\alpha)(1+\tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

**Slice Data**

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	45.0838	142718	Cyclone Underflow	0	39	872.365	1335.25	1648.9	0	1648.9
2	45.0838	419922	Cyclone Underflow	0	39	2833.01	4336.24	5354.81	0	5354.81
3	45.0838	696992	Cyclone Underflow	0	39	4722.56	7228.4	8926.34	0	8926.34
4	45.0838	974063	Cyclone Underflow	0	39	6612.14	10120.6	12497.8	0	12497.8
5	0.674563	16680.5	Liner Interface Zone	0	13.6	3447.02	5276.04	21839.8	31.1774	21808.6
6	69.4483	1.64551e+006	Liner Interface Zone	0	13.6	3646.43	5581.26	23132.6	62.3548	23070.2
7	14.2633	319322	Liner Interface Zone	0	13.6	3440.05	5265.38	21826.8	62.3568	21764.5
8	48.0526	1.02905e+006	Liner Interface Zone	0	13.6	3286.15	5029.81	20853.1	62.3547	20790.8
9	48.0526	957013	Liner Interface Zone	0	13.6	3049.14	4667.05	19353.6	62.3547	19291.3
10	57.4662	1.0497e+006	Liner Interface Zone	0	13.6	2788.59	4268.24	17705.2	62.3583	17642.8
11	57.4662	946268	Liner Interface Zone	0	13.6	2504.03	3832.69	15904.8	62.3583	15842.4
12	57.4662	842833	Liner Interface Zone	0	13.6	2219.46	3397.13	14104.4	62.3583	14042.1
13	10.1889	138642	Liner Interface Zone	0	13.6	2051.94	3140.72	13044.6	62.3576	12982.2
14	63.358	789325	Liner Interface Zone	0	13.6	1870.22	2862.57	11894.8	62.3559	11832.4
15	6.66227	75711.8	Liner Interface Zone	0	13.6	1697.36	2597.99	10801.2	62.3581	10738.8
16	41.4898	440289	Liner Interface Zone	0	13.6	1578.34	2415.83	10048.2	62.3559	9985.82
17	45.7181	422762	Liner Interface Zone	0	13.6	1362.68	2085.73	8683.74	62.3594	8621.38
18	45.7181	357216	Liner Interface Zone	0	13.6	1136.02	1738.8	7249.7	62.3594	7187.34
19	45.7181	291670	Liner Interface Zone	0	13.6	909.35	1391.86	5815.63	62.3594	5753.27
20	45.7181	226124	Liner Interface Zone	0	13.6	682.689	1044.93	4381.59	62.3594	4319.23
21	45.7181	160579	Liner Interface Zone	0	13.6	456.025	697.997	2947.52	62.3594	2885.17
			Liner Interface							



Zone										
23	52.032	22869.3	Liner Interface Zone	0	13.6	5.72276	8.75932	132.838	96.6312	36.2067
24	52.032	12959.2	Liner Interface Zone	0	13.6	16.2967	24.9439	233.108	130.003	103.105
25	52.032	6697.29	Liner Interface Zone	0	13.6	1011.39	1548.04	6465.57	66.7275	6398.84

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	572.648	5460	1.98387e-022	0	0
2	617.732	5391.95	-106388	9073.38	-4.87472
3	662.816	5323.9	59899.5	-5108.59	-4.87473
4	707.9	5255.86	372435	-31763.5	-4.87473
5	752.984	5187.81	831219	-70891.4	-4.87474
6	753.658	5186.79	849640	-72462.5	-4.87474
7	823.107	5184.92	472351	-40285	-4.87474
8	837.37	5184.54	393696	-33576.8	-4.87474
9	885.422	5183.25	126906	-10823.3	-4.87473
10	933.475	5181.95	-143674	12253.4	-4.87474
11	990.941	5180.47	-473336	40368.9	-4.87473
12	1048.41	5178.98	-808251	68932.5	-4.87473
13	1105.87	5177.5	-1.14842e+006	97944.2	-4.87473
14	1116.06	5177.23	-1.20925e+006	103132	-4.87473
15	1179.42	5175.55	-1.59087e+006	135679	-4.87473
16	1186.08	5175.37	-1.63143e+006	139138	-4.87473
17	1227.57	5174.27	-1.88531e+006	160790	-4.87472
18	1273.29	5173.1	-2.16872e+006	184961	-4.87472
19	1319.01	5171.94	-2.45542e+006	209413	-4.87473
20	1364.73	5170.77	-2.74542e+006	234146	-4.87473
21	1410.44	5169.6	-3.03871e+006	259160	-4.87474
22	1456.16	5168.44	-3.3353e+006	284454	-4.87472
23	1508.2	5167.71	-3.67744e+006	313635	-4.87474
24	1560.23	5166.97	-3.86453e+006	329590	-4.87473
25	1612.26	5166.24	-3.87412e+006	330409	-4.87474
26	1664.29	5165.51	6.38965e-021	0	0

### List Of Coordinates

#### Piezoline

X	Y
---	---

0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03

595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

### Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**



X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13



183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**



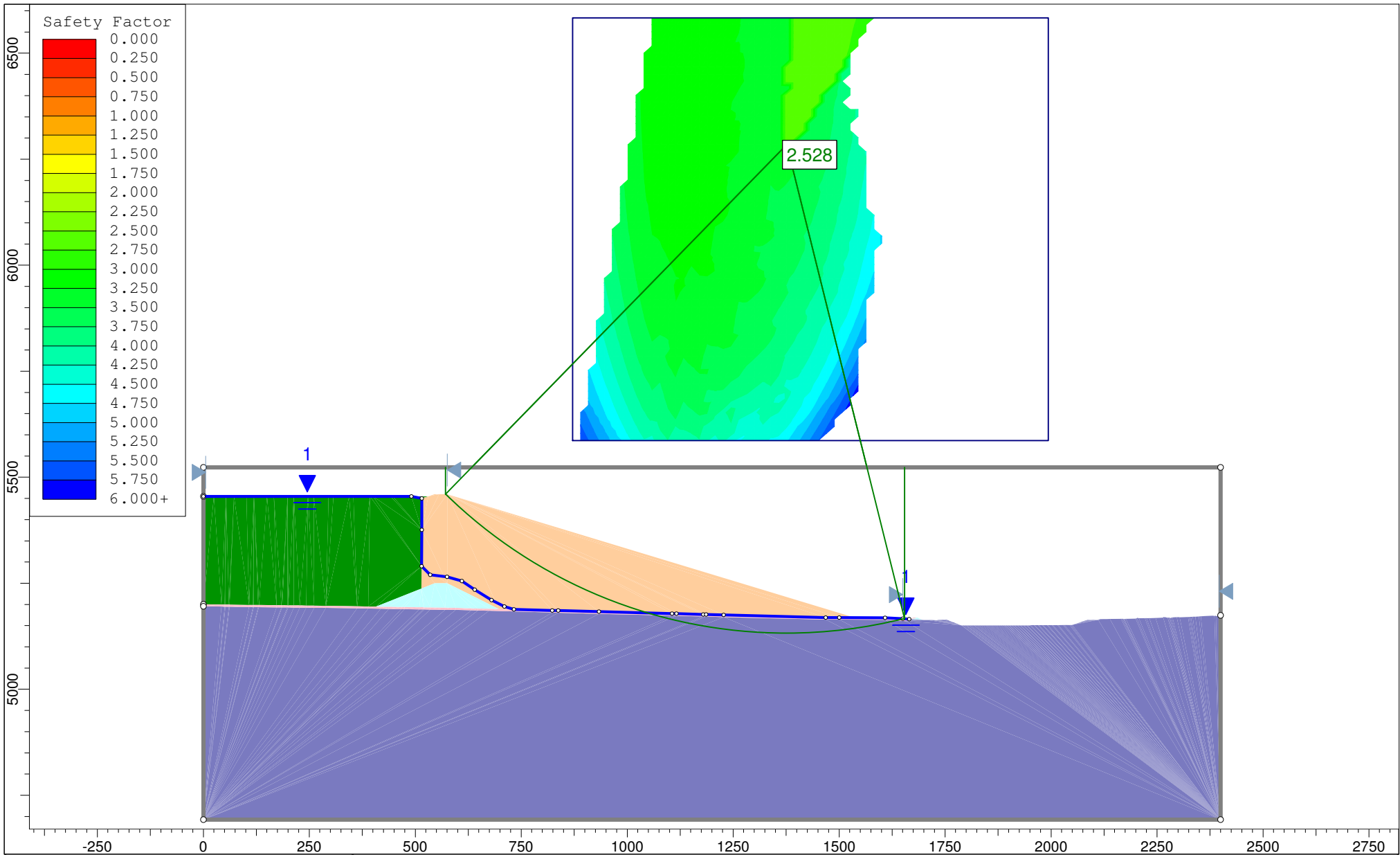
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Circular Failure, Global	
Drawn By	GS	Scale	1:3762
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	4a - SectionA 5460R DS_S_C_G.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 4a - SectionA 5460R DS\_S\_C\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Circular Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.527760  
 Center: 1376.188, 6284.432  
 Radius: 1152.416  
 Left Slip Surface Endpoint: 570.971, 5460.000  
 Right Slip Surface Endpoint: 1654.244, 5166.064  
 Left Slope Intercept: 570.971 5522.990  
 Right Slope Intercept: 1654.244 5522.990  
 Resisting Moment=8.33108e+009 lb-ft  
 Driving Moment=3.29583e+009 lb-ft  
 Resisting Horizontal Force=6.7452e+006 lb

Driving Horizontal Force=2.66845e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 11730

Number of Invalid Surfaces: 29201

#### Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4317 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3813 surfaces
- Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.216	953.512	1177.49	0	1177.49
2	47.6079	233141	Cyclone Underflow	0	39	1249.89	3159.43	3901.57	0	3901.57
3	47.6079	356305	Cyclone Underflow	0	39	1954.83	4941.33	6102.03	0	6102.03
4	47.6079	458937	Cyclone Underflow	0	39	2561.02	6473.65	7994.29	0	7994.29
5	47.6079	543134	Cyclone Underflow	0	39	3074.2	7770.84	9596.18	0	9596.18

6	47.6079	610541	Cyclone Underflow	0	39	3499.06	8844.78	10922.4	0	10922.4
7	47.6079	662469	Cyclone Underflow	0	39	3839.45	9705.2	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.49	10360	12793.5	0	12793.5
9	47.6079	723916	Cyclone Underflow	0	39	4278.69	10815.5	13356	0	13356
10	47.6079	734997	Cyclone Underflow	0	39	4381.94	11076.5	13678.4	0	13678.4
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.41	7119.2	14434.9	155.948	14278.9
12	43.9418	677489	Foundation Materials	150	29	3185.32	8051.73	14255.1	0	14255.1
13	43.9418	666065	Foundation Materials	150	29	3142.02	7942.28	14057.7	0	14057.7
14	43.9418	645159	Foundation Materials	150	29	3052.52	7716.04	13649.5	0	13649.5
15	43.9418	614993	Foundation Materials	150	29	2917.32	7374.28	13032.9	0	13032.9
16	43.9418	575744	Foundation Materials	150	29	2736.7	6917.73	12209.3	0	12209.3
17	43.9418	527526	Foundation Materials	150	29	2510.7	6346.44	11178.7	0	11178.7
18	43.9418	470419	Foundation Materials	150	29	2239.12	5659.95	9940.21	0	9940.21
19	43.9418	404462	Foundation Materials	150	29	1921.59	4857.32	8492.24	0	8492.24
20	43.9418	329659	Foundation Materials	150	29	1557.52	3937.03	6831.97	0	6831.97
21	43.9418	246078	Foundation Materials	150	29	1146.59	2898.3	4958.07	0	4958.07
22	43.9418	156715	Foundation Materials	150	29	744.157	1881.05	3122.9	0	3122.9
23	43.9418	104701	Foundation Materials	150	29	581.701	1470.4	2382.07	0	2382.07
24	43.9418	56711	Foundation Materials	150	29	328.326	829.93	1226.63	0	1226.63
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4370.71	11048.1	22314.9	155.762	22159.1

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146707	13731.9	-5.34735

3	666.187	5376.71	-127700	11952.8	-5.34734
4	713.795	5341.41	-93307	8733.58	-5.34733
5	761.402	5309.7	-62718.8	5870.51	-5.34733
6	809.01	5281.25	-50008.5	4680.82	-5.34733
7	856.618	5255.79	-65397.2	6121.21	-5.34733
8	904.226	5233.09	-116109	10867.9	-5.34735
9	951.834	5212.99	-206964	19371.9	-5.34732
10	999.442	5195.34	-340792	31898.3	-5.34733
11	1047.05	5180.02	-518731	48553.5	-5.34734
12	1066.08	5174.52	-568159	53179.9	-5.34733
13	1110.02	5163.18	-727759	68118.6	-5.34733
14	1153.96	5153.65	-924346	86519.3	-5.34734
15	1197.9	5145.89	-1.15617e+006	108218	-5.34733
16	1241.85	5139.87	-1.42056e+006	132965	-5.34733
17	1285.79	5135.57	-1.71391e+006	160423	-5.34734
18	1329.73	5132.95	-2.03175e+006	190173	-5.34734
19	1373.67	5132.02	-2.36865e+006	221707	-5.34734
20	1417.61	5132.76	-2.71825e+006	254430	-5.34734
21	1461.56	5135.18	-3.07316e+006	287649	-5.34733
22	1505.5	5139.29	-3.42492e+006	320574	-5.34733
23	1549.44	5145.11	-3.67939e+006	344392	-5.34732
24	1593.38	5152.67	-3.72663e+006	348815	-5.34734
25	1637.32	5161.99	-3.78937e+006	354686	-5.34732
26	1654.24	5166.06	6.3698e-021	0	0

## List Of Coordinates

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5



1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27

1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455

530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

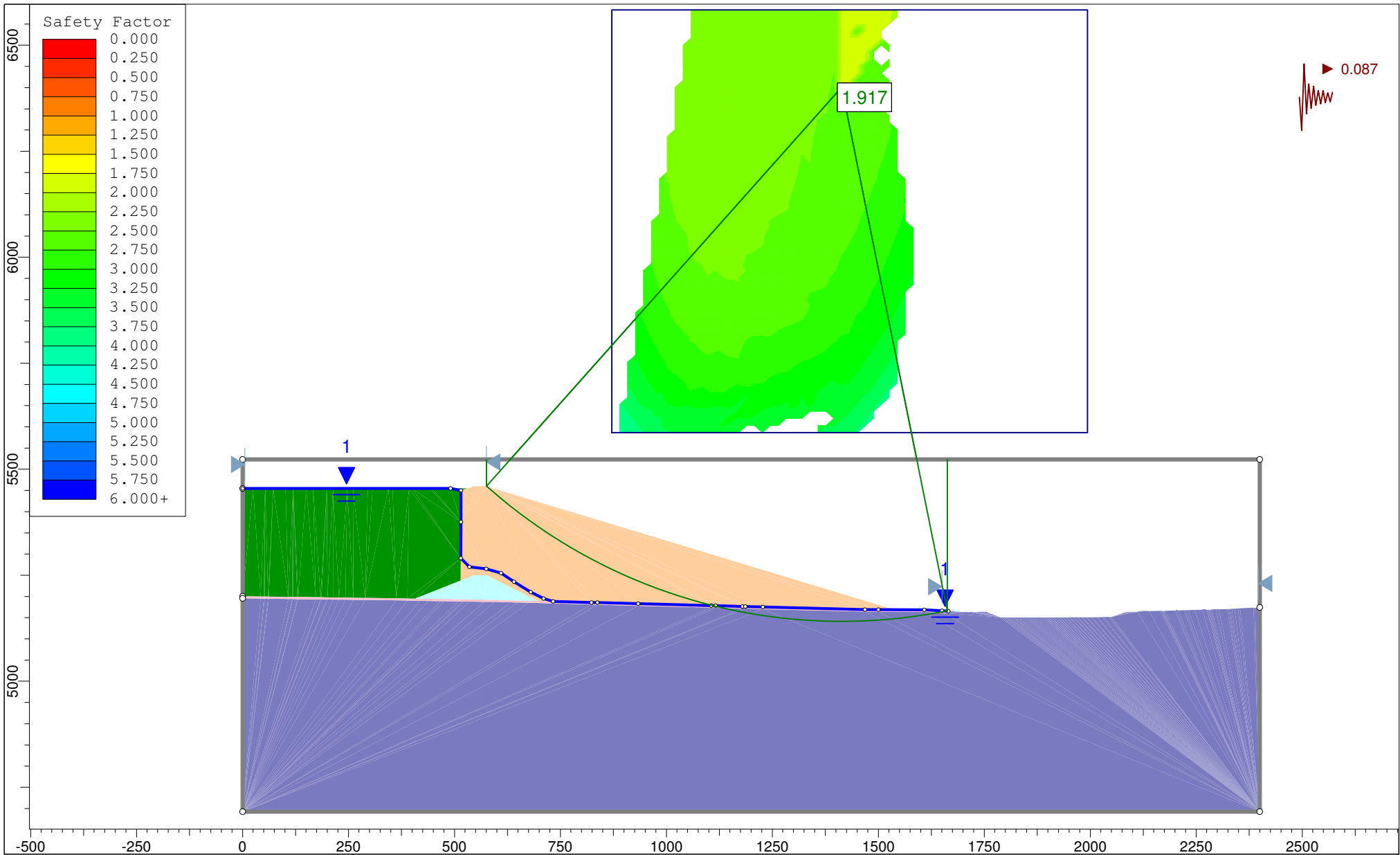
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

Project				Copper Flat			
Analysis Description				Section A Stability: Downstream, Pseudo Static, Circular Failure, Global			
Drawn By		GS		Scale		1:3762	
Company				Golder Associates Inc.			
Date		11/4/2013, 3:02:21 PM		File Name		5a - SectionA 5460R DS_PS_C_G.slim	



## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 5a - SectionA 5460R\_DS\_PS\_C\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Circular Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3







## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 1.916600  
 Center: 1413.608, 6400.750  
 Radius: 1259.972  
 Left Slip Surface Endpoint: 575.445, 5460.000

Right Slip Surface Endpoint: 1662.506, 5165.607  
 Left Slope Intercept: 575.445 5522.990  
 Right Slope Intercept: 1662.506 5522.990  
 Resisting Moment=7.97722e+009 lb-ft  
 Driving Moment=4.16218e+009 lb-ft  
 Resisting Horizontal Force=5.93993e+006 lb  
 Driving Horizontal Force=3.0992e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 10557  
 Number of Invalid Surfaces: 30374

#### Error Codes:

Error Code -101 reported for 408 surfaces  
 Error Code -103 reported for 2326 surfaces  
 Error Code -108 reported for 4489 surfaces  
 Error Code -110 reported for 440 surfaces  
 Error Code -111 reported for 4814 surfaces  
 Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.9166

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	44.3652	60862.7	Cyclone Underflow	0	39	321.259	615.725	760.358	0	760.358
2	44.3652	174209	Cyclone Underflow	0	39	1244.73	2385.64	2946.02	0	2946.02
			Cyclone							

			Underflow								
4	44.3652	354600	Cyclone Underflow	0	39	2655.19	5088.94	6284.31	0	6284.31	
5	44.3652	424335	Cyclone Underflow	0	39	3230.76	6192.08	7646.58	0	7646.58	
6	44.3652	481812	Cyclone Underflow	0	39	3723.19	7135.87	8812.07	0	8812.07	
7	44.3652	527840	Cyclone Underflow	0	39	4134.48	7924.15	9785.52	0	9785.52	
8	44.3652	563095	Cyclone Underflow	0	39	4466.28	8560.07	10570.8	0	10570.8	
9	44.3652	588143	Cyclone Underflow	0	39	4719.87	9046.11	11171	0	11171	
10	44.3652	603459	Cyclone Underflow	0	39	4896.24	9384.13	11588.4	0	11588.4	
11	44.3652	609444	Cyclone Underflow	0	39	4996.03	9575.4	11824.6	0	11824.6	
12	44.3652	606436	Cyclone Underflow	0	39	5019.62	9620.6	11880.4	0	11880.4	
13	23.4132	316539	Liner Interface Zone	0	26.5	3196.81	6127.01	12445	156.077	12288.9	
14	46.4639	615182	Foundation Materials	150	29	3573.33	6848.64	12084.7	0	12084.7	
15	46.4639	590270	Foundation Materials	150	29	3445.2	6603.07	11641.7	0	11641.7	
16	46.4639	555377	Foundation Materials	150	29	3255.27	6239.06	10985	0	10985	
17	46.4639	510675	Foundation Materials	150	29	3003.43	5756.37	10114.2	0	10114.2	
18	46.4639	456280	Foundation Materials	150	29	2689.22	5154.15	9027.73	0	9027.73	
19	46.4639	392275	Foundation Materials	150	29	2311.95	4431.08	7723.26	0	7723.26	
20	46.4639	318701	Foundation Materials	150	29	1870.65	3585.29	6197.43	0	6197.43	
21	46.4639	235654	Foundation Materials	150	29	1364.61	2615.41	4447.71	0	4447.71	
22	46.4639	146214	Foundation Materials	150	29	861.912	1651.94	2709.57	0	2709.57	
23	46.4639	98503.8	Foundation Materials	150	29	693.875	1329.88	2128.55	0	2128.55	
24	46.4639	56028.4	Foundation Materials	150	29	408.79	783.486	1142.84	0	1142.84	
25	20.1633	6058.23	Liner Interface Zone	0	26.5	4794.2	9188.57	18585.2	155.762	18429.4	

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 1.9166



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	575.445	5460	1.98387e-022	0	0
2	619.81	5422.28	-162525	15041.2	-5.28749
3	664.175	5387.89	-170970	15822.8	-5.2875
4	708.54	5356.52	-168647	15607.8	-5.2875
5	752.906	5327.9	-167853	15534.3	-5.28749
6	797.271	5301.82	-178146	16486.9	-5.2875
7	841.636	5278.09	-206894	19147.4	-5.28749
8	886.001	5256.57	-259659	24030.7	-5.2875
9	930.366	5237.13	-340500	31512.3	-5.2875
10	974.731	5219.68	-452179	41847.8	-5.28749
11	1019.1	5204.13	-596323	55187.9	-5.28749
12	1063.46	5190.41	-773548	71589.6	-5.2875
13	1107.83	5178.45	-983538	91023.5	-5.28749
14	1131.24	5172.83	-1.06166e+006	98253.7	-5.2875
15	1177.7	5163.06	-1.26537e+006	117106	-5.28748
16	1224.17	5155.1	-1.50305e+006	139103	-5.2875
17	1270.63	5148.92	-1.77205e+006	163998	-5.28749
18	1317.1	5144.48	-2.06868e+006	191450	-5.28749
19	1363.56	5141.77	-2.38827e+006	221027	-5.28749
20	1410.02	5140.78	-2.7251e+006	252200	-5.28749
21	1456.49	5141.51	-3.07236e+006	284338	-5.2875
22	1502.95	5143.95	-3.42212e+006	316707	-5.28749
23	1549.42	5148.12	-3.68084e+006	340651	-5.2875
24	1595.88	5154.03	-3.72095e+006	344363	-5.28749
25	1642.34	5161.71	-3.78664e+006	350442	-5.28749
26	1662.51	5165.61	6.38612e-021	0	0

### List Of Coordinates

#### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195

732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81

213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164

1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85



1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42

2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455

441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01

570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

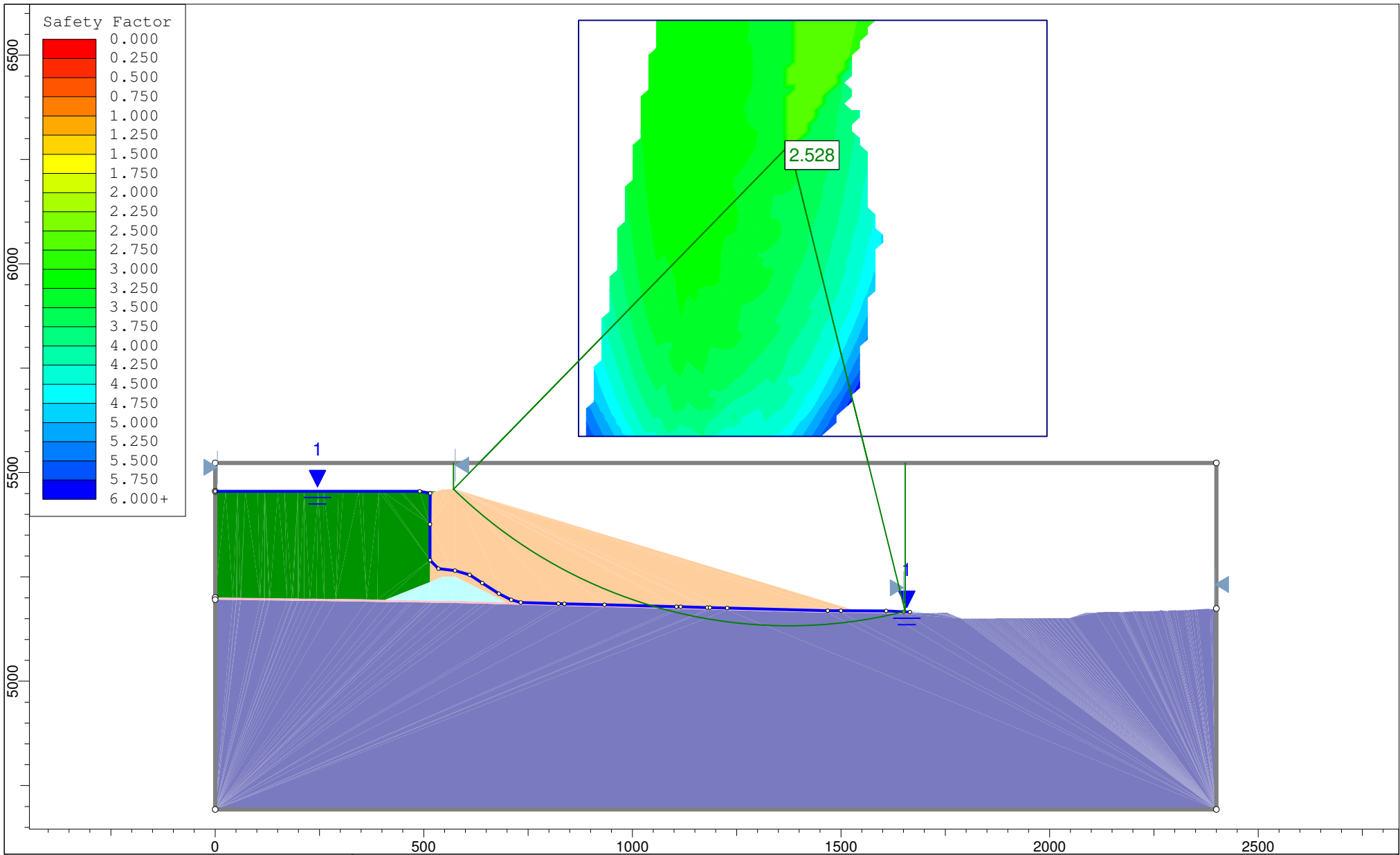
**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63

545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

<i>Project</i>		Copper Flat	
<i>Analysis Description</i>		Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength	
<i>Drawn By</i>	GS	<i>Scale</i>	1:3822
<i>Date</i>	11/4/2013, 3:02:21 PM	<i>Company</i>	Golder Associates Inc.
		<i>File Name</i>	6a - SectionA 5460R DS_S_C_G_postliq.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 6a - SectionA 5460R DS\_S\_C\_G\_postliq.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.05
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.527620  
 Center: 1376.188, 6284.432  
 Radius: 1152.416  
 Left Slip Surface Endpoint: 570.971, 5460.000  
 Right Slip Surface Endpoint: 1654.244, 5166.064  
 Left Slope Intercept: 570.971 5522.990  
 Right Slope Intercept: 1654.244 5522.990  
 Resisting Moment=8.3306e+009 lb-ft  
 Driving Moment=3.29583e+009 lb-ft  
 Resisting Horizontal Force=6.74519e+006 lb



Driving Horizontal Force=2.6686e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 11802

Number of Invalid Surfaces: 29129

#### Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4262 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3796 surfaces
- Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.344	953.782	1177.82	0	1177.82
2	47.6079	233141	Cyclone Underflow	0	39	1249.93	3159.35	3901.47	0	3901.47
3	47.6079	356305	Cyclone Underflow	0	39	1954.88	4941.2	6101.88	0	6101.88
4	47.6079	458937	Cyclone Underflow	0	39	2561.11	6473.51	7994.12	0	7994.12
5	47.6079	543134	Cyclone Underflow	0	39	3074.33	7770.73	9596.06	0	9596.06

6	47.6079	610541	Cyclone Underflow	0	39	3499.23	8844.72	10922.3	0	10922.3
7	47.6079	662469	Cyclone Underflow	0	39	3839.67	9705.22	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.76	10360.1	12793.7	0	12793.7
9	47.6079	723916	Cyclone Underflow	0	39	4279.01	10815.7	13356.3	0	13356.3
10	47.6079	734997	Cyclone Underflow	0	39	4382.34	11076.9	13678.8	0	13678.8
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.62	7119.35	14435.1	155.948	14279.2
12	43.9418	677489	Foundation Materials	150	29	3185.59	8051.96	14255.5	0	14255.5
13	43.9418	666065	Foundation Materials	150	29	3142.32	7942.59	14058.2	0	14058.2
14	43.9418	645159	Foundation Materials	150	29	3052.84	7716.42	13650.2	0	13650.2
15	43.9418	614993	Foundation Materials	150	29	2917.65	7374.71	13033.7	0	13033.7
16	43.9418	575744	Foundation Materials	150	29	2737.05	6918.22	12210.2	0	12210.2
17	43.9418	527526	Foundation Materials	150	29	2511.05	6346.98	11179.6	0	11179.6
18	43.9418	470419	Foundation Materials	150	29	2239.47	5660.53	9941.26	0	9941.26
19	43.9418	404462	Foundation Materials	150	29	1921.94	4857.93	8493.33	0	8493.33
20	43.9418	329659	Foundation Materials	150	29	1557.85	3937.65	6833.1	0	6833.1
21	43.9418	246078	Foundation Materials	150	29	1146.9	2898.93	4959.21	0	4959.21
22	43.9418	156715	Foundation Materials	150	29	744.38	1881.51	3123.73	0	3123.73
23	43.9418	104701	Foundation Materials	150	29	581.769	1470.49	2382.23	0	2382.23
24	43.9418	56711	Foundation Materials	150	29	328.39	830.044	1226.83	0	1226.83
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4364.66	11032.2	22282.9	155.762	22127.2

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146699	13710.3	-5.33928

3	666.187	5376.71	-127697	11934.4	-5.33928
4	713.795	5341.41	-93312.8	8720.9	-5.33928
5	761.402	5309.7	-62734.3	5863.08	-5.33929
6	809.01	5281.25	-50034.2	4676.14	-5.33929
7	856.618	5255.79	-65433.1	6115.3	-5.33929
8	904.226	5233.09	-116155	10855.7	-5.33928
9	951.834	5212.99	-207020	19347.9	-5.3393
10	999.442	5195.34	-340860	31856.3	-5.33927
11	1047.05	5180.02	-518810	48487.4	-5.33929
12	1066.08	5174.52	-568241	53107.1	-5.33928
13	1110.02	5163.18	-727848	68023.8	-5.33929
14	1153.96	5153.65	-924444	86397.4	-5.33928
15	1197.9	5145.89	-1.15628e+006	108064	-5.33926
16	1241.85	5139.87	-1.42067e+006	132774	-5.33928
17	1285.79	5135.57	-1.71404e+006	160192	-5.33928
18	1329.73	5132.95	-2.03189e+006	189898	-5.33929
19	1373.67	5132.02	-2.36881e+006	221386	-5.33928
20	1417.61	5132.76	-2.71843e+006	254061	-5.33928
21	1461.56	5135.18	-3.07335e+006	287232	-5.33929
22	1505.5	5139.29	-3.42512e+006	320108	-5.33929
23	1549.44	5145.11	-3.67961e+006	343892	-5.33929
24	1593.38	5152.67	-3.72686e+006	348308	-5.33929
25	1637.32	5161.99	-3.7896e+006	354171	-5.33928
26	1654.24	5166.06	6.3698e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5

1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27

1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455



530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

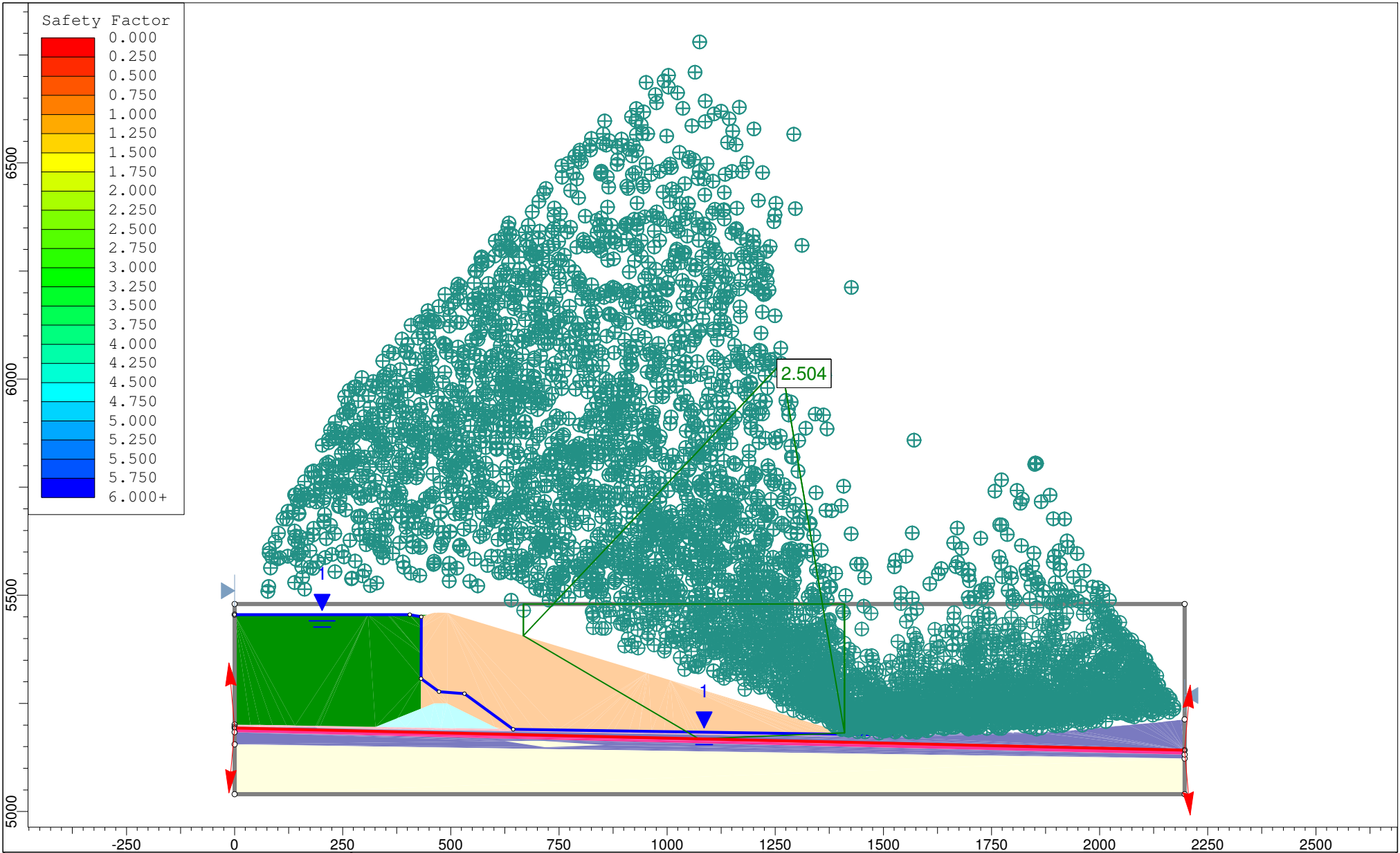
**Material Boundary**


X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32

**APPENDIX H.2  
STABILITY SECTION B-B'**



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Static, Block Failure		
	Drawn By	GS	Scale	1:3686	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	1 - Section B 5460R - DS_S_B.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 1 - Section B 5460R - DS\_S\_B.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Static, Block Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/13

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft3  
Advanced Groundwater Method: None

#### **Random Numbers**









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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft <sup>3</sup> ]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765

2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5

22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7



42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6

62656.7	15211.8
---------	---------

## Global Minimums

---

### Method: spencer

FS: 2.504180  
 Axis Location: 1263.734, 6036.472  
 Left Slip Surface Endpoint: 667.648, 5406.642  
 Right Slip Surface Endpoint: 1409.947, 5181.705  
 Left Slope Intercept: 667.648 5480.000  
 Right Slope Intercept: 1409.947 5480.000  
 Resisting Moment=2.81387e+009 lb-ft  
 Driving Moment=1.12367e+009 lb-ft  
 Resisting Horizontal Force=3.03106e+006 lb  
 Driving Horizontal Force=1.2104e+006 lb

## Global Minimum Coordinates

---

### Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

## Valid / Invalid Surfaces

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### Method: spencer

Number of Valid Surfaces: 584  
 Number of Invalid Surfaces: 4416

#### Error Codes:

Error Code -106 reported for 2 surfaces  
 Error Code -107 reported for 1331 surfaces  
 Error Code -108 reported for 2123 surfaces  
 Error Code -110 reported for 70 surfaces  
 Error Code -111 reported for 595 surfaces  
 Error Code -112 reported for 295 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 106 = Average slice width is less than 0.0001 \* (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the

driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation.

This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

### Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.50418

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	25.4457	63.7207	78.6884	0	78.6884
2	31.651	48443.9	Cyclone Underflow	0	39	393.5	985.395	1216.86	0	1216.86
3	31.651	80739.8	Cyclone Underflow	0	39	671.529	1681.63	2076.63	0	2076.63
4	31.651	113036	Cyclone Underflow	0	39	949.556	2377.86	2936.4	0	2936.4
5	31.651	145332	Cyclone Underflow	0	39	1227.58	3074.09	3796.18	0	3796.18
6	31.651	177628	Cyclone Underflow	0	39	1505.61	3770.32	4655.95	0	4655.95
7	31.651	209923	Cyclone Underflow	0	39	1783.64	4466.55	5515.73	0	5515.73
8	31.651	242219	Cyclone Underflow	0	39	2061.66	5162.78	6375.5	0	6375.5
9	31.651	274515	Cyclone Underflow	0	39	2339.69	5859.01	7235.27	0	7235.27
10	31.651	306847	Cyclone Underflow	0	39	2618.13	6556.28	8096.33	0	8096.33
11	31.651	339198	Cyclone Underflow	0	39	2896.61	7253.64	8957.49	0	8957.49
12	31.651	371504	Cyclone Underflow	0	39	3174.66	7949.91	9817.32	0	9817.32
13	8.75148	108572	Liner Interface Zone	0	26.5	2185.06	5471.78	11170.6	195.919	10974.6
14	16.9638	218358	Foundation Materials	150	29	2579.64	6459.89	11383.3	0	11383.3
15	30.2226	380408	Foundation Materials	150	29	2774.58	6948.05	12264	0	12264
16	30.2226	344600	Foundation Materials	150	29	2511.83	6290.08	11077	0	11077
17	30.2226	308792	Foundation Materials	150	29	2249.09	5632.12	9890.02	0	9890.02
18	30.2226	272984	Foundation Materials	150	29	1986.34	4974.16	8703	0	8703
19	30.2226	237176	Foundation Materials	150	29	1723.6	4316.2	7516.03	0	7516.03
20	30.2226	201368	Foundation Materials	150	29	1460.85	3658.23	6329.01	0	6329.01
21	31.0757	169713	Liner Interface Zone	0	26.5	967.095	2421.78	5130.02	272.678	4857.34
22	31.0757	131843	Liner Interface Zone	0	26.5	745.37	1866.54	3908.11	164.386	3743.72
23	31.0757	93991.2	Liner Interface Zone	0	26.5	523.772	1311.62	2686.8	56.0937	2630.7
24	31.1043	56349.3	Cyclone Underflow	0	39	474.407	1188	1467.06	0	1467.06
25	31.1043	18783.1	Cyclone Underflow	0	39	1844.88	4619.92	5705.13	0	5705.13

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.50418

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214017	13186.2	-3.5257
3	730.95	5369.4	-256320	15792.6	-3.5257
4	762.601	5350.78	-297148	18308.2	-3.52571
5	794.252	5332.16	-336500	20732.8	-3.52571
6	825.903	5313.53	-374377	23066.5	-3.52571
7	857.554	5294.91	-410779	25309.3	-3.5257
8	889.205	5276.29	-445705	27461.2	-3.52571
9	920.856	5257.67	-479156	29522.2	-3.5257
10	952.507	5239.05	-511131	31492.3	-3.52571
11	984.158	5220.43	-541429	33359	-3.5257
12	1015.81	5201.81	-570296	35137.6	-3.5257
13	1047.46	5183.19	-597825	36833.8	-3.52571
14	1056.21	5178.04	-590828	36402.6	-3.5257
15	1073.18	5168.06	-583080	35925.3	-3.52571
16	1103.4	5169.28	-796681	49085.8	-3.5257
17	1133.62	5170.51	-1.00613e+006	61990.4	-3.52569
18	1163.84	5171.73	-1.21142e+006	74639.1	-3.5257
19	1194.07	5172.96	-1.41256e+006	87031.9	-3.5257
20	1224.29	5174.18	-1.60955e+006	99168.8	-3.52569
21	1254.51	5175.41	-1.80238e+006	111050	-3.52571
22	1285.59	5176.66	-1.98927e+006	122565	-3.52571
23	1316.66	5177.92	-2.17327e+006	133902	-3.52572
24	1347.74	5179.18	-2.35439e+006	145061	-3.52571
25	1378.84	5180.44	-2.53815e+006	156383	-3.52571
26	1409.95	5181.7	4.449e-021	0	0

List Of Coordinates

Piezoline

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

Block Search Lines

X	Y

0	5193.11
2196.33	5141.83

**External Boundary**

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28

814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18

745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77



1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13

1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250

491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

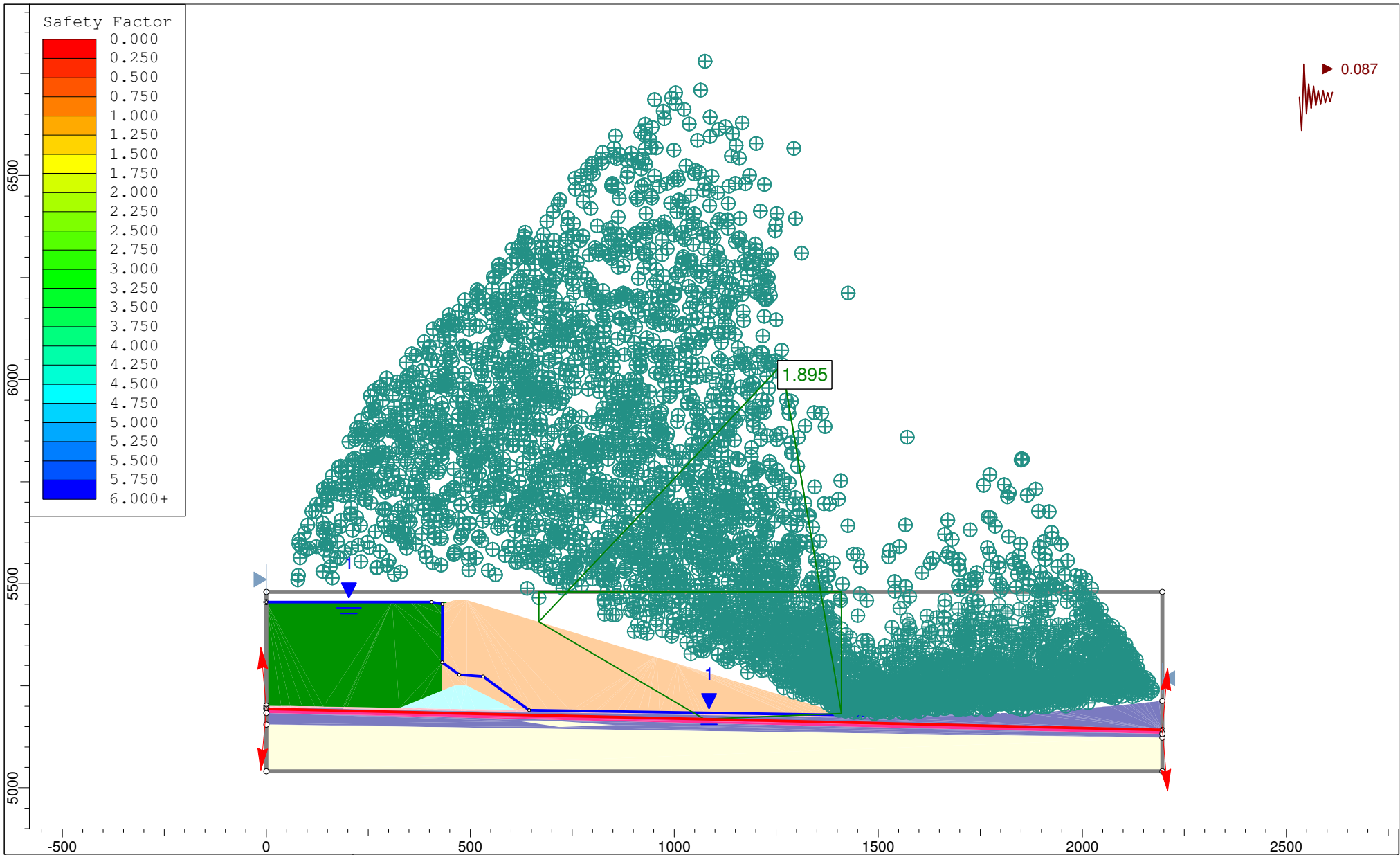
X	Y
0	5154.97
2196.33	5122.6

**Material Boundary**

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section B-B' Stability: Downstream, Pseudo Static, Block Failure	
Drawn By	GS	Scale	1:3908
		Company	Golder Associates Inc.
Date	11/4/2013, 11:49:04 AM	File Name	2 - Section B 5460R - DS_PS_B.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 2 - Section B 5460R - DS\_PS\_B.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Block Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/2013

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

#### **Random Numbers**

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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3









## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft <sup>3</sup> ]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186

835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2

20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7



40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

## Global Minimums

---

### Method: spencer

FS: 1.895340  
 Axis Location: 1263.734, 6036.472  
 Left Slip Surface Endpoint: 667.648, 5406.642  
 Right Slip Surface Endpoint: 1409.947, 5181.705  
 Left Slope Intercept: 667.648 5480.000  
 Right Slope Intercept: 1409.947 5480.000  
 Resisting Moment=2.74873e+009 lb-ft  
 Driving Moment=1.45026e+009 lb-ft  
 Resisting Horizontal Force=2.96703e+006 lb  
 Driving Horizontal Force=1.56543e+006 lb

## Global Minimum Coordinates

---

### Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

## Valid / Invalid Surfaces

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### Method: spencer

Number of Valid Surfaces: 562  
 Number of Invalid Surfaces: 4438

#### Error Codes:

Error Code -106 reported for 2 surfaces  
 Error Code -107 reported for 578 surfaces  
 Error Code -108 reported for 2726 surfaces  
 Error Code -110 reported for 101 surfaces  
 Error Code -111 reported for 671 surfaces  
 Error Code -112 reported for 360 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 106 = Average slice width is less than 0.0001 \* (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha =  $\cos(\alpha)(1+\tan(\alpha)\tan(\phi)/F) < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	0	0	-57.4289	0	-57.4289
2	31.651	48443.9	Cyclone Underflow	0	39	484.264	917.844	1133.44	0	1133.44
3	31.651	80739.8	Cyclone Underflow	0	39	833.882	1580.49	1951.75	0	1951.75
4	31.651	113036	Cyclone Underflow	0	39	1183.5	2243.14	2770.05	0	2770.05
5	31.651	145332	Cyclone Underflow	0	39	1533.12	2905.79	3588.35	0	3588.35
6	31.651	177628	Cyclone Underflow	0	39	1882.74	3568.44	4406.65	0	4406.65
7	31.651	209923	Cyclone Underflow	0	39	2232.36	4231.08	5224.95	0	5224.95
8	31.651	242219	Cyclone Underflow	0	39	2581.98	4893.73	6043.25	0	6043.25
9	31.651	274515	Cyclone Underflow	0	39	2931.6	5556.38	6861.55	0	6861.55
10	31.651	306847	Cyclone Underflow	0	39	3281.79	6220.1	7681.19	0	7681.19
11	31.651	339198	Cyclone Underflow	0	39	3631.96	6883.8	8500.79	0	8500.79
12	31.651	371504	Cyclone Underflow	0	39	3981.57	7546.43	9319.07	0	9319.07
13	8.75148	108572	Liner Interface Zone	0	26.5	2802.54	5311.77	10849.7	195.919	10653.8
14	16.9638	218358	Foundation Materials	150	29	3293.35	6242.01	10990.3	0	10990.3
15	30.2226	380408	Foundation Materials	150	29	3636.26	6891.94	12162.8	0	12162.8
16	30.2226	344600	Foundation Materials	150	29	3288.53	6232.88	10973.8	0	10973.8
17	30.2226	308792	Foundation Materials	150	29	2940.8	5573.81	9784.82	0	9784.82
18	30.2226	272984	Foundation Materials	150	29	2593.07	4914.75	8595.82	0	8595.82
19	30.2226	237176	Foundation Materials	150	29	2245.34	4255.68	7406.86	0	7406.86
20	30.2226	201368	Foundation Materials	150	29	1897.61	3596.62	6217.86	0	6217.86
21	31.0757	169713	Liner Interface Zone	0	26.5	1250.03	2369.24	5024.65	272.678	4751.98
22	31.0757	131843	Liner Interface Zone	0	26.5	956.103	1812.14	3798.98	164.386	3634.59
23	31.0757	93991.2	Liner Interface Zone	0	26.5	662.33	1255.34	2573.92	56.0937	2517.83
24	31.1043	56349.3	Cyclone Underflow	0	39	574.915	1089.66	1345.62	0	1345.62
25	31.1043	18783.1	Cyclone Underflow	0	39	3181.77	6030.54	7447.09	0	7447.09

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214342	17977.3	-4.79429
3	730.95	5369.4	-256846	21542.3	-4.79432
4	762.601	5350.78	-298096	25002	-4.79431
5	794.252	5332.16	-338091	28356.5	-4.79431
6	825.903	5313.53	-376832	31605.8	-4.79431
7	857.554	5294.91	-414318	34749.8	-4.79431
8	889.205	5276.29	-450549	37788.6	-4.79431
9	920.856	5257.67	-485526	40722.2	-4.79431
10	952.507	5239.05	-519248	43550.5	-4.79431
11	984.158	5220.43	-551514	46256.8	-4.79431
12	1015.81	5201.81	-582571	48861.6	-4.79431
13	1047.46	5183.19	-612510	51372.6	-4.79431
14	1056.21	5178.04	-603106	50583.9	-4.79431
15	1073.18	5168.06	-592352	49682	-4.79432
16	1103.4	5169.28	-798697	66988.6	-4.79431
17	1133.62	5170.51	-1.00144e+006	83993.1	-4.79431
18	1163.84	5171.73	-1.20058e+006	100696	-4.79434
19	1194.07	5172.96	-1.39612e+006	117096	-4.79432
20	1224.29	5174.18	-1.58806e+006	133194	-4.7943
21	1254.51	5175.41	-1.7764e+006	148990	-4.79428
22	1285.59	5176.66	-1.95716e+006	164151	-4.79429
23	1316.66	5177.92	-2.13608e+006	179158	-4.79431
24	1347.74	5179.18	-2.31317e+006	194011	-4.79431
25	1378.84	5180.44	-2.49498e+006	209260	-4.79431
26	1409.95	5181.7	4.449e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

**Block Search Lines**

X	Y
0	5193.11
2196.33	5141.83

**External Boundary**

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8

692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69

617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17

1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73



1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89

1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y

325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

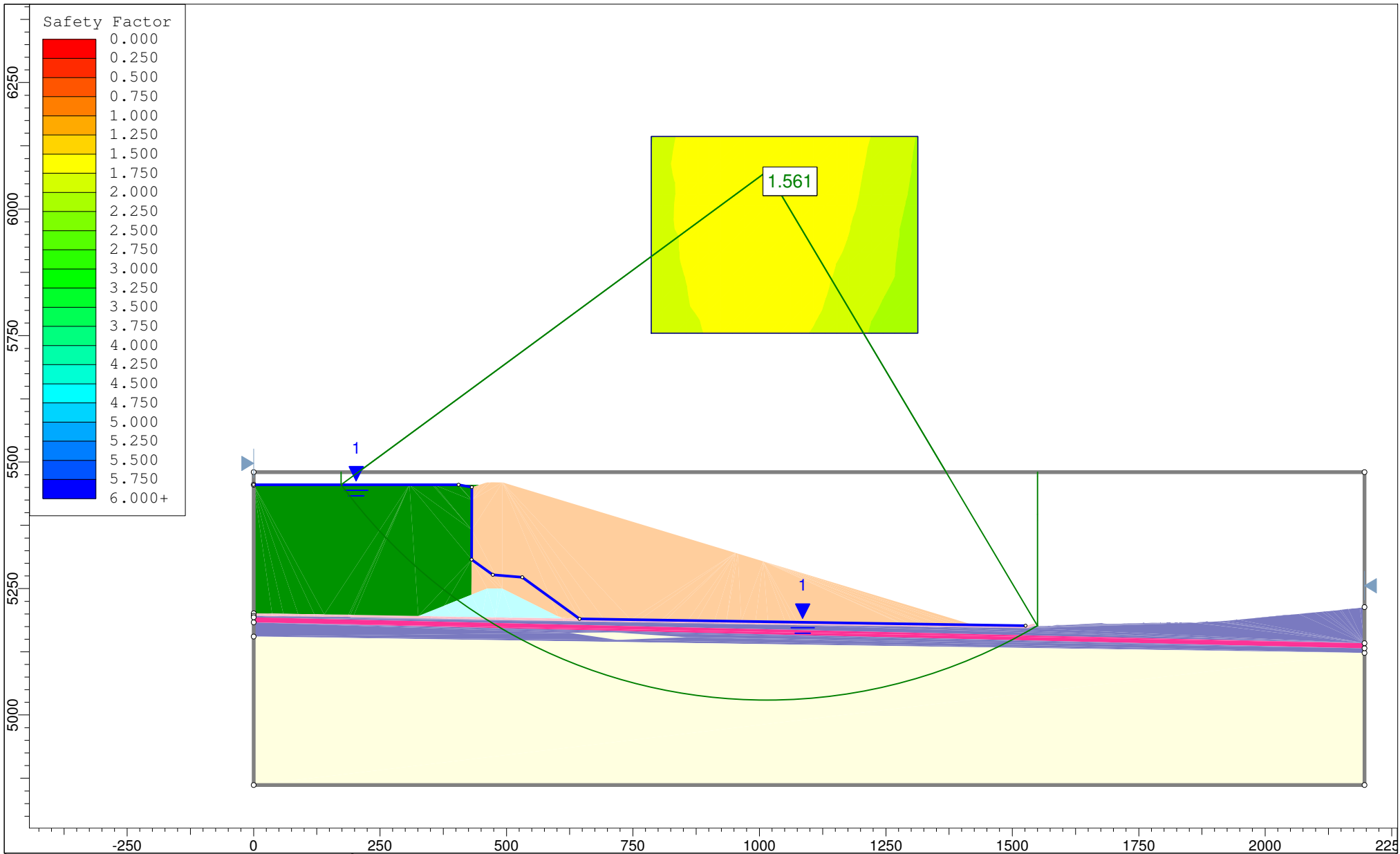
X	Y
0	5154.97
2196.33	5122.6


**Material Boundary**

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			
	Copper Flat			
	Analysis Description			
	Section B-B' Stability: Downstream, Static, Circular Failure			
	Drawn By		Scale	Company
GS		1:3149	Golder Associates Inc.	
Date		File Name		
11/4/2013, 11:49:04 AM		3 - Section B 5460R - DS_S_C.slim		

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 3 - Section B 5460R - DS\_S\_C.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Static, Circular Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft3  
Advanced Groundwater Method: None

#### **Random Numbers**









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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit								
Weight [lbs/ft3]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3

3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8



43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

**Global Minimums**

**Method: spencer**

FS: 1.560970  
 Center: 1015.357, 6074.806  
 Radius: 1045.828  
 Left Slip Surface Endpoint: 173.020, 5454.947  
 Right Slip Surface Endpoint: 1550.009, 5175.971  
 Left Slope Intercept: 173.020 5480.000  
 Right Slope Intercept: 1550.009 5480.000  
 Resisting Moment=1.06294e+010 lb-ft  
 Driving Moment=6.80947e+009 lb-ft  
 Resisting Horizontal Force=9.56258e+006 lb  
 Driving Horizontal Force=6.12606e+006 lb

**Valid / Invalid Surfaces**

**Method: spencer**

Number of Valid Surfaces: 2506  
 Number of Invalid Surfaces: 2246

**Error Codes:**

Error Code -103 reported for 588 surfaces  
 Error Code -108 reported for 568 surfaces  
 Error Code -110 reported for 58 surfaces  
 Error Code -111 reported for 1032 surfaces

**Error Codes**

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

**Slice Data**

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	76.7812	384179	Cyclone Overflow	421.855	0	270.252	421.855	3388.77	2894.28	494.494
2	76.7812	1.07712e+006	Cyclone Overflow	1183.95	0	758.471	1183.95	10074.5	8108.62	1965.9
3	76.7812	1.63914e+006	Cyclone Overflow	1802.07	0	1154.46	1802.07	16242.9	12337.8	3905.11
4	49.0448	1.33572e+006	Structural Fill Liner Interface	0	29	7124.22	11120.7	20062.4	0	20062.4

		Zone								
6	9.92233	312599	Foundation Materials	150	29	8606.51	13434.5	23965.9	0	23965.9
7	17.4329	567165	Soft Clay	851.163	13.3468	4488.85	7006.95	25946.3	0	25946.3
8	44.5694	1.5136e+006	Foundation Materials	150	29	9663.09	15083.8	26941.3	0	26941.3
9	67.0136	2.36955e+006	Clay	958.267	13.1388	5094.31	7952.07	29962.1	0	29962.1
10	67.0136	2.45379e+006	Clay	1025.15	13.0217	5445.36	8500.05	32321.5	0	32321.5
11	67.0136	2.50548e+006	Clay	1089.15	12.9174	5739.42	8959.07	34313.8	0	34313.8
12	67.0136	2.50972e+006	Clay	1124.79	12.8623	5943.34	9277.38	35704	0	35704
13	67.0136	2.47166e+006	Clay	1148.83	12.8262	6062.3	9463.07	36518	0	36518
14	67.0136	2.39734e+006	Clay	1148.83	12.8262	6103.92	9528.04	36803.3	0	36803.3
15	67.0136	2.28693e+006	Clay	1148.83	12.8262	6063.17	9464.42	36524	0	36524
16	67.0136	2.13981e+006	Clay	1124.79	12.8623	5933.03	9261.28	35633.4	0	35633.4
17	67.0136	1.95593e+006	Clay	1072.75	12.9435	5704.6	8904.71	34077.2	0	34077.2
18	67.0136	1.7351e+006	Clay	1017.45	13.0347	5367	8377.72	31793.1	0	31793.1
19	67.0136	1.47639e+006	Clay	923.265	13.2038	4903.36	7653.99	28688	0	28688
20	67.0136	1.1784e+006	Clay	802.785	13.4506	4288.2	6693.74	24631.1	0	24631.1
21	67.0136	839198	Clay	656.503	13.8133	3481.23	5434.09	19431.3	0	19431.3
22	67.0136	496697	Clay	415.25	14.6733	2204.49	3441.14	11555.9	0	11555.9
23	29.3922	131593	Foundation Materials	150	29	3334.74	5205.43	9120.25	0	9120.25
24	17.8998	52163.6	Soft Clay	192.084	16.2309	1043.45	1628.79	4935.28	0	4935.28
25	32.0382	41314	Foundation Materials	150	29	0	0	-38306.9	0	-38306.9

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	173.02	5454.95	3.13838e-023	0	0
2	249.802	5362.29	273636	97200.6	19.556
3	326.583	5287.82	965550	342981	19.556
4	403.364	5226.74	1.86896e+006	663889	19.556
5	452.409	5193.42	2.19051e+006	778111	19.556
6	460.564	5188.26	2.26451e+006	804395	19.556
7	470.486	5182.13	2.32639e+006	826376	19.556
8	487.919	5171.72	2.51812e+006	894484	19.556
9	532.488	5147.12	2.72923e+006	969472	19.556
10	599.502	5115.21	3.28961e+006	1.16853e+006	19.556
11	666.515	5088.87	3.69589e+006	1.31285e+006	19.556
12	733.529	5067.67	3.93299e+006	1.39707e+006	19.556
13	800.543	5051.28	3.98821e+006	1.41669e+006	19.556
14	867.556	5039.47	3.85556e+006	1.36957e+006	19.556
15	934.57	5032.1	3.53467e+006	1.25558e+006	19.556
16	1001.58	5029.07	3.03059e+006	1.07652e+006	19.556
17	1068.6	5030.33	2.35334e+006	835950	19.556

18	1135.61	5035.91	1.52056e+006	540130	19.5559
19	1202.62	5045.88	558004	198213	19.5559
20	1269.64	5060.36	-497755	-176812	19.556
21	1336.65	5079.55	-1.59529e+006	-566675	19.5559
22	1403.67	5103.74	-2.6616e+006	-945449	19.556
23	1470.68	5133.3	-3.26006e+006	-1.15803e+006	19.5559
24	1500.07	5148.09	-3.50629e+006	-1.2455e+006	19.556
25	1517.97	5157.67	-3.58004e+006	-1.2717e+006	19.556
26	1550.01	5175.97	4.62169e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

**External Boundary**

X	Y
2196.33	4861.27
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	4861.27

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04

87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178

1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32



1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07

431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

X	Y
0	5154.97
2196.33	5122.6

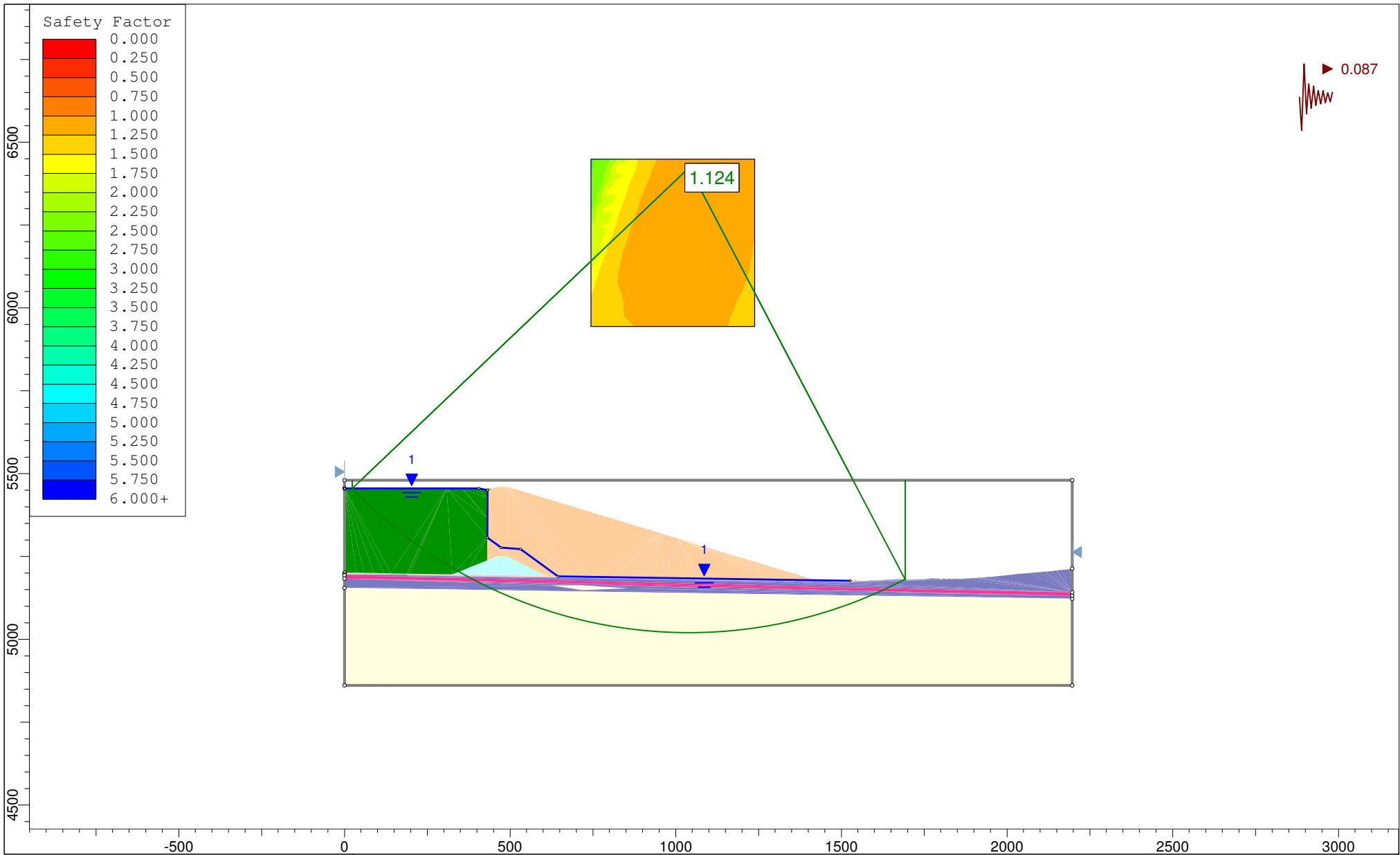
**Material Boundary**


X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54

712.872 5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Pseudo Static, Circular Failure		
	Drawn By	GS	Scale	1:4812	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	4 - Section B 5460R - DS_PS_C.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 4 - Section B 5460R - DS\_PS\_C.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Circular Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

#### **Random Numbers**

---

Slide Interpretation

Slide: 1

Microscopic Description:

Microscopic Description:

Component	Color	Shape	Size	Location
Cellular debris	Amorphous	Irregular	Small	Background
Microorganisms	Clusters	Spherical	Variable	Background
Spores	Clusters	Spherical	Variable	Background
Hyphae	Linear	Branching	Variable	Background

Microscopic Description:

Microscopic Description:









**Slide**  
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Sample	Element	Concentration	Unit
1	Al	0.0000	g/g
1	As	0.0000	g/g
1	B	0.0000	g/g
1	Br	0.0000	g/g
1	Ca	0.0000	g/g
1	Cd	0.0000	g/g
1	Co	0.0000	g/g
1	Cu	0.0000	g/g
1	Fe	0.0000	g/g
1	Hg	0.0000	g/g
1	Mn	0.0000	g/g
1	Ni	0.0000	g/g
1	Pb	0.0000	g/g
1	P	0.0000	g/g
1	S	0.0000	g/g
1	Se	0.0000	g/g
1	Si	0.0000	g/g
1	Sr	0.0000	g/g
1	Ti	0.0000	g/g
1	V	0.0000	g/g
1	Zn	0.0000	g/g

Sample	Element	Concentration	Unit
1	Al	0.0000	g/g
1	As	0.0000	g/g
1	B	0.0000	g/g
1	Br	0.0000	g/g
1	Ca	0.0000	g/g
1	Cd	0.0000	g/g
1	Co	0.0000	g/g
1	Cu	0.0000	g/g
1	Fe	0.0000	g/g
1	Hg	0.0000	g/g
1	Mn	0.0000	g/g
1	Ni	0.0000	g/g
1	Pb	0.0000	g/g
1	P	0.0000	g/g
1	S	0.0000	g/g
1	Se	0.0000	g/g
1	Si	0.0000	g/g
1	Sr	0.0000	g/g
1	Ti	0.0000	g/g
1	V	0.0000	g/g
1	Zn	0.0000	g/g

Table with 2 columns: **Sample** and **Concentration**. The table contains several rows of data, but the text is too small to read accurately.



TOC science  
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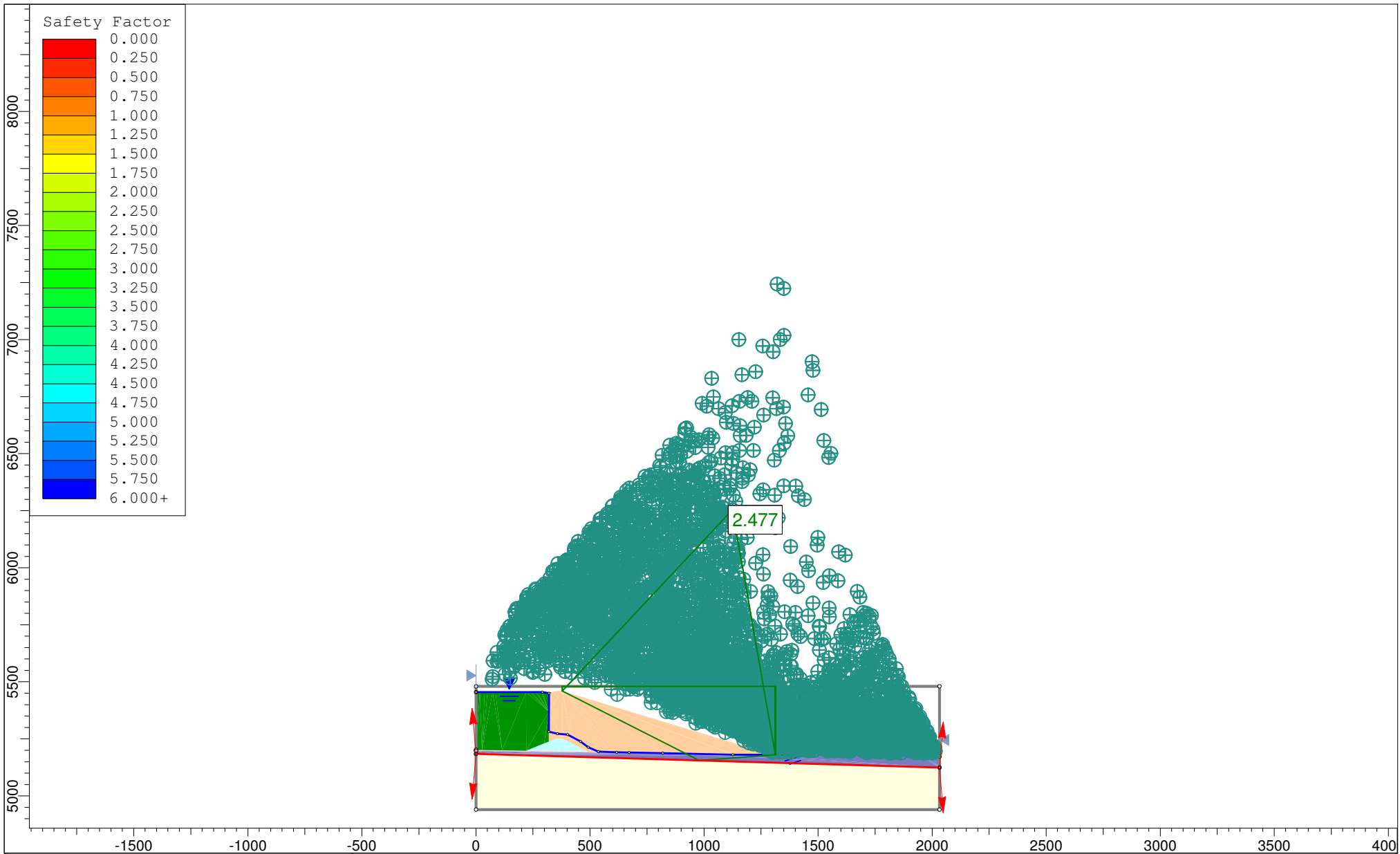




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**APPENDIX H.3  
STABILITY SECTION D-D'**



SLIDEINTERPRET 6.008

<i>Project</i>			
Copper Flat			
<i>Analysis Description</i>			
Section D-D' Stability: Downstream, Static, Block Failure			
<i>Drawn By</i>	GS	<i>Scale</i>	1:6984
<i>Company</i>	Golder Associates		
<i>Date</i>	11/4/2013, 1:11:49 PM		<i>File Name</i>
	1 - Section D 5460R - DS_S_B.slim		

## Slide Analysis Information

### Copper Flat

#### Project Summary

---

- File Name: 1 - Section D 5460R - DS\_S\_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

---

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None








## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

## Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma							0.2

Ratio							
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9

12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6

26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7



40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4

58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1388.41	5182.87
---------	---------

**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04

1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01

1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45

1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

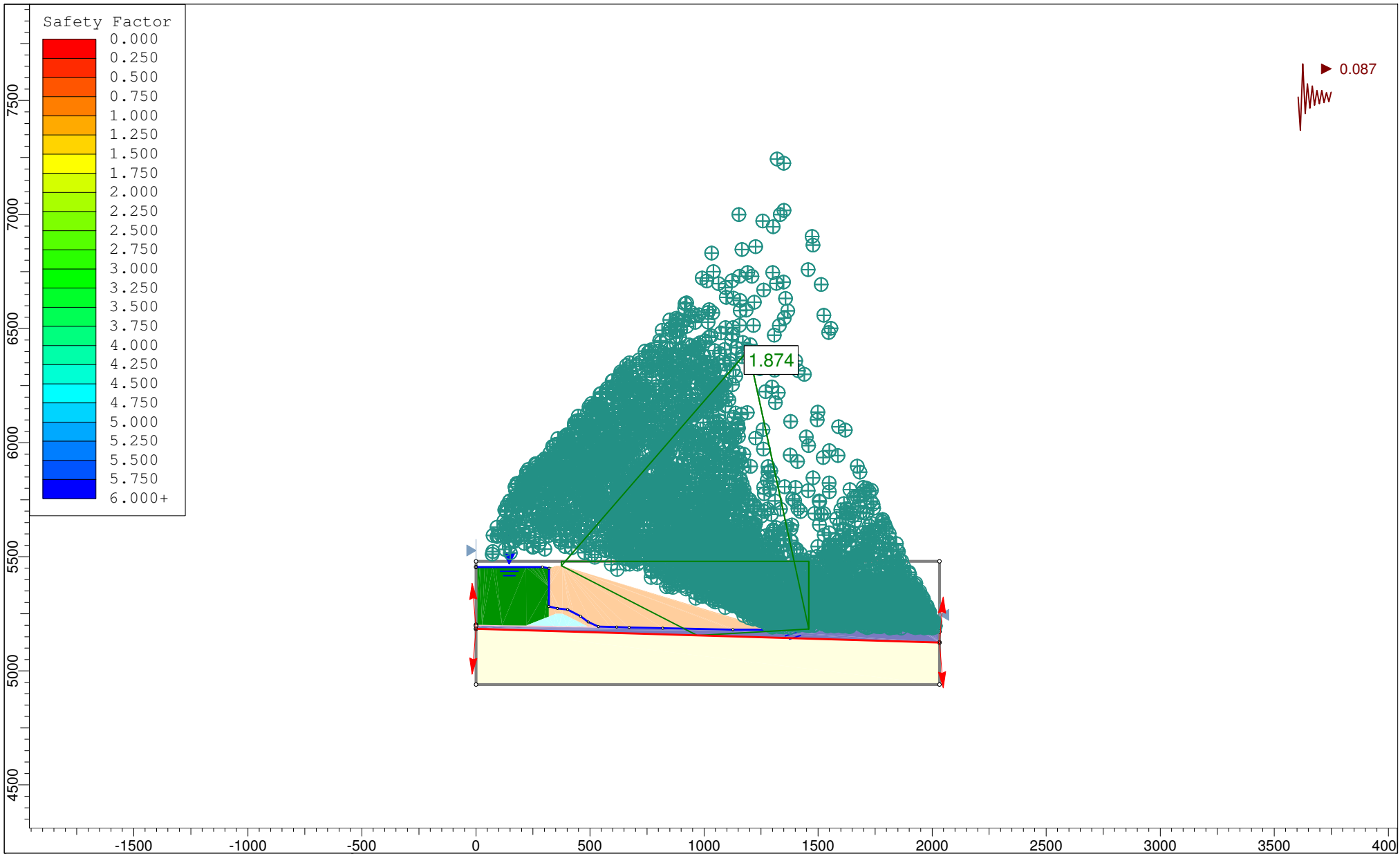
X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455



	Project			Copper Flat			
	Analysis Description						Section D-D' Stability: Downstream, Pseudo Static, Block Failure
	Drawn By	GS	Scale	1:6984	Company	Golder Associates	
	Date	11/4/2013, 1:11:49 PM			File Name	2 - Section D 5460R - DS_PS_B.slim	

SLIDEINTERPRET 6.008

## Slide Analysis Information

### Copper Flat

#### Project Summary

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- File Name: 2 - Section D 5460R - DS\_PS\_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material properties edits 12/2013

#### General Settings

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- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>
- Advanced Groundwater Method: None

## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








## Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Loading

- Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight	1e-025	113	120	120	120	127	108

[lbs/ft <sup>3</sup> ]							
Cohesion [psf]	0	0	150	0			
Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5

9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2

21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2

32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8

50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450



-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04

1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

491.186	5194.33
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**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

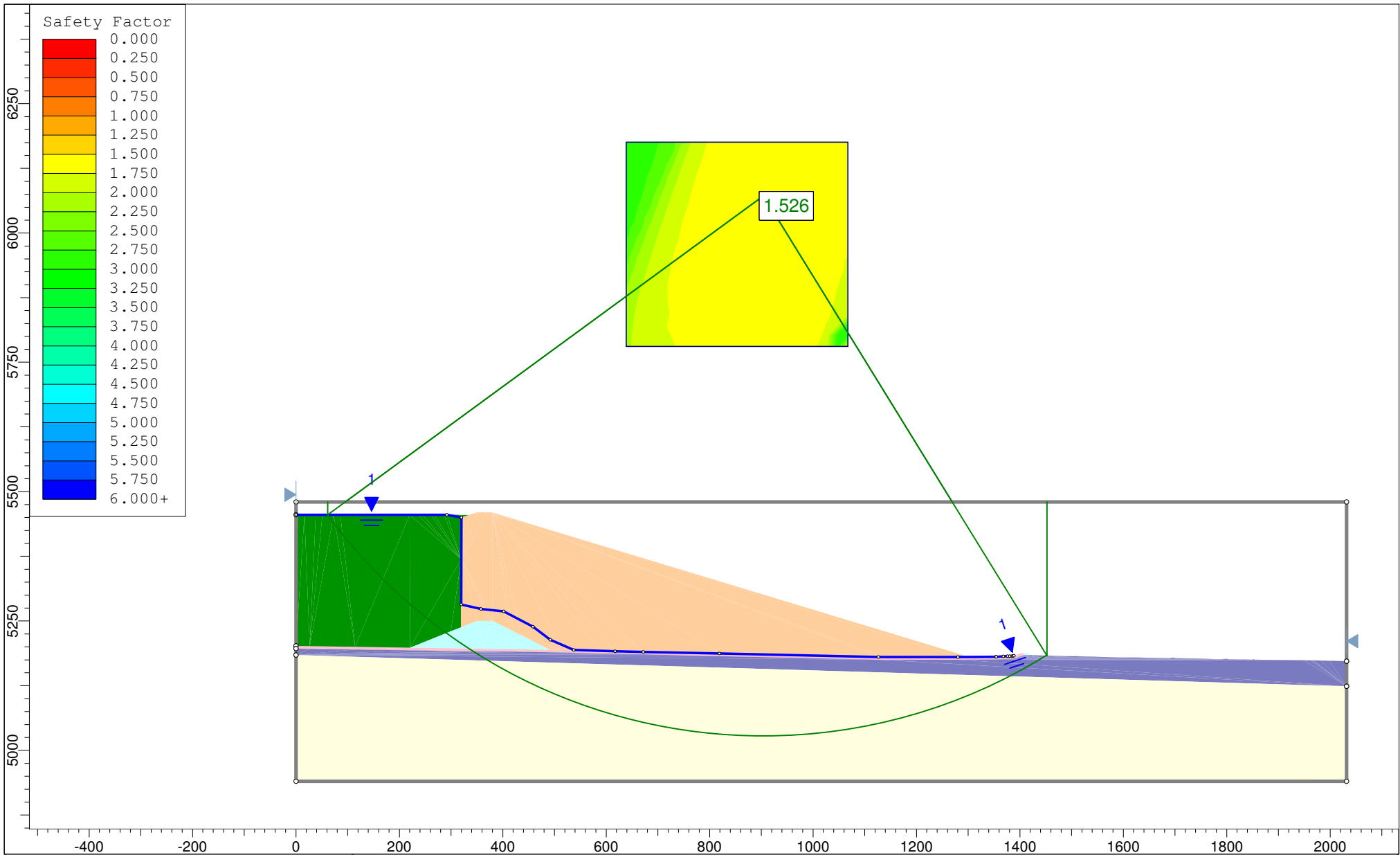
X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77


**Material Boundary**

X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52

**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76



	Project			Copper Flat								
	Analysis Description						Section D-D' Stability: Downstream, Static, Circular Failure					
	Drawn By			Scale			Company					
	GS			1:3083			Golder Associates					
Date			11/4/2013, 1:11:49 PM			File Name			3 - Section D 5460R - DS_S_C.slim			

SLIDEINTERPRET 6.008

## Slide Analysis Information

### Copper Flat

#### Project Summary

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- File Name: 3 - Section D 5460R - DS\_S\_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

---

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>
- Advanced Groundwater Method: None








## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

## Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft <sup>3</sup> ]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1

Hu Value	Automatically Calculated	Automatically Calculated	Automatically Calculated
Ru Value	0	0	0

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4

13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3

28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5

43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

2031.79	5123.76
---------	---------

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01

**Block Search Lines**

X	Y
0	5184.09

1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92

1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14



1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

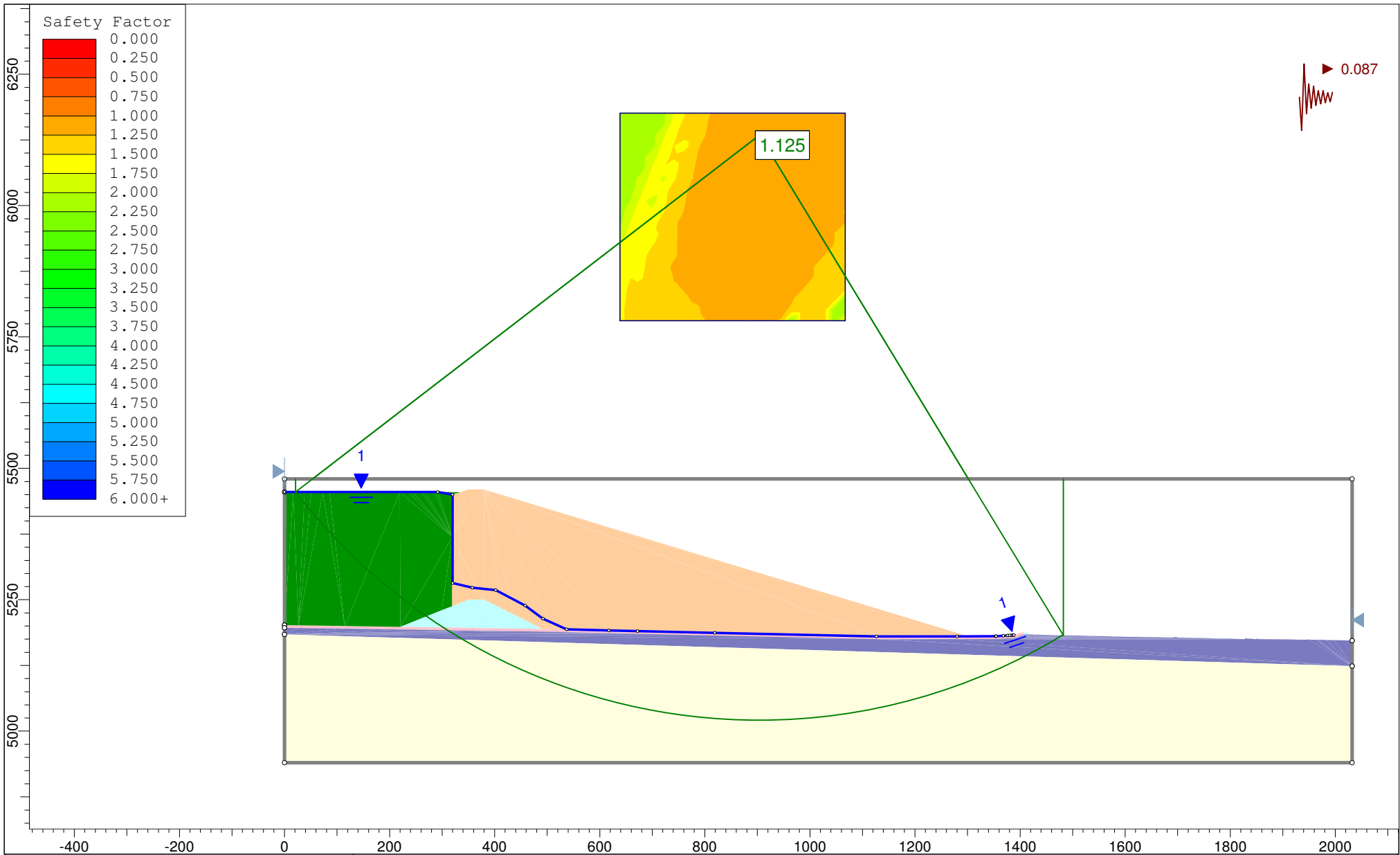
X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455



	Project			Copper Flat								
	Analysis Description						Section D-D' Stability: Downstream, Pseudo Static, Circular Failure					
	Drawn By			Scale			Company					
	Date			File Name								
GS			1:3035			Golder Associates						
11/4/2013, 1:11:49 PM			4 - Section D 5460R - DS_PS_C.slim									

SLIDEINTERPRET 6.008

## Slide Analysis Information

### Copper Flat

#### Project Summary

---

- File Name: 4 - Section D 5460R - DS\_PS\_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

---

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None

## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








## Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Loading

- Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		

Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio				0.2			
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1

10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8

36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6

54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92

**Material Boundary**

X	Y
---	---

1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26

1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23

458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33

**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455



**APPENDIX I**  
**FOUNDATION SETTLEMENT POTENTIAL EVALUATION**

**APPENDIX I.1**  
**SETTLEMENT POTENTIAL, COPPER FLAT TAILINGS EMBANKMENT FOUNDATION**  
**REVISED JUNE 2016**

**Date:** April 23, 2013  
**Project No.:** 103-92557  
**Subject:** Settlement Potential, Copper Flat  
Tailings Embankment Foundation  
**Made by:** DP  
**Checked by:** GM  
**Reviewed by:** MJG  
**Project Short Title:** COPPER FLAT FEASIBILITY STUDY TAILINGS DISPOSAL FACILITY DESIGN

### 1.0 OBJECTIVE

Evaluate the consolidation characteristics of the soil strata, and estimate the post-construction subgrade settlement under the containment embankments for the Copper Flat Tailings Impoundment.

### 2.0 BACKGROUND

Golder is completing the feasibility level designing of the Copper Flat tailings impoundment embankment which will impound slurried tailings to an estimated maximum depth of approximately 230 feet at the impoundment embankment. An earthen starter dam will be constructed to a height of approximately 50 feet and the remainder of the dam will be constructed with sand recovered from the tailings in a cyclone plant. A geotechnical investigation was performed in the vicinity of the proposed embankment, which included standard penetration testing and sample collection from the surface to a depth of 53 feet. Drilling indicated that the tailings embankment area consists primarily of alluvial deposits that include silt, sand and gravel, underlain by clay.

For the settlement analysis, representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Additionally, select samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing. The results are summarized in the following sections.

Drill hole logs were input onto a site plan and cross-sections through the embankment were developed. Two cross-sections (B-B' and D-D') were further developed for settlement potential analyses with the addition of information from the site investigation conducted in 1980 for the former Quintana mining operation (*Tailings Dam and Disposal Area, Quintana Minerals Corporation, Copper Flats Project, Golddust, New Mexico, Sergeant Hauskins and Beckwith, 1980*). Generalized soil and rock designations were assigned to the strata.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because the results for the post-construction condition demonstrate that the amount of predicted settlement (approximately 2.1 feet under the heaviest loaded portion of embankment) is tolerable, and, will be adequately mitigated by fill placement during ongoing embankment construction.

Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc. for the analysis of settlement and consolidation under foundations and embankments.

The results of the consolidation tests performed on the samples obtained during geotechnical investigations are included in Attachment 1. Tables summarizing the laboratory consolidation test results and the computer modeling input parameters are included in Attachment 2. The SETTLE3D computer model output files are included in Attachment 3. Figures depicting the two embankment sections

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix I Foundation Settlement Potential Evaluation

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analyzed for foundation settlement, as well as a graphical representation of the analysis results, are included in Attachment 4.

### 3.0 LABORATORY ANALYSIS

Laboratory one-dimensional consolidation analyses were performed on representative samples obtained during the geotechnical investigation. The results of the geotechnical laboratory analyses are summarized in the Tables included in Attachment 1. Consolidation tests were generally conducted on the fine grain fraction of the soil- strata, on remolded drill hole samples.

As shown, the soil strata generally have compression index values ( $C_c$ ) ranging from 0.035 to 0.09, and estimated initial void ratio values ranging from 0.71 to 0.75. The compression index results were obtained from review of the laboratory graphs of void ratio versus stress, and were obtained for loads ranging from 16 to 32 kilopounds per square foot (ksf).

### 4.0 COMPUTER MODELING

Computer modeling was performed using SETTLE3D v. 2.0. The results of the modeling are included in Attachment 3.

The following parameters were incorporated into the analyses:

- Settlement Index ( $C_c$ ) values obtained from laboratory testing of soil fraction of field samples.
- Initial void ratio ( $e_0$ ) values estimated from review of the consolidation test graphs.
- Unit weights estimated from the consolidation test results, and based on previous experience with similar soils.
- An overconsolidation ratio (OCR) of 1.0 assumed.
- A Poisson's Ratio of 0.3 was assumed, based on a published range of values from 0.2 to 0.4 for sandy clays.
- The model was initialized for primary consolidation, with non-linear settlement characteristics.

A conservative value of 4,000 ksf was assigned for the constrained modulus of the coarse sand and gravel, caliche and weathered basalt layers. The constrained modulus was assigned to represent inter-particle consolidation in the gravels and sands. Settlement in these layers was demonstrated to be minimal, even with the conservative modulus value used in the model. (Reference: US Army Corps of Engineers, EM 1110-1-1904, 1990).

Modeling was performed on an individual point basis, at a minimum of 5 analysis points on each embankment cross-section. The model was initialized for 1-dimensional Boussinesq Methodology.

### 5.0 RESULTS

#### 5.1 Settlement Beneath Embankment

The results of the consolidation analyses are summarized on Figures 5 and 7 in Attachment 4. As shown on the figures, calculated settlement beneath the impoundment embankment ranges from approximately 2.1 to 0.2 feet, with settlement decreasing as the weight of post-construction loading decreases towards the outer toe of the embankment. The results of the consolidation testing indicate the various soils at the site exhibit generally similar consolidation characteristics. Based on the consolidation testing and computer modeling, settlement beneath the embankment is projected to be generally uniform.

The consolidation analyses results are considered conservative, as the laboratory consolidation testing was performed on the fine-grained fraction of soil samples. The effects of inter-particle bearing contact within the gravel-rich strata, which will reduce the estimated settlement, have not been considered in the analyses. Based on the field Standard Penetration Testing (SPT) results, which showed the strata to

generally be very dense to hard, actual post-construction consolidation settlement of less than 1 foot is anticipated.

As the embankment will be constructed continuously, it is anticipated that observed settlement will be accommodated by managing dam fill placement during embankment construction.

## 5.2 Settlement Beneath Outlet Piping

Section F-F' was developed along the proposed outlet pipe alignment, as shown on Figure 9. The pipe will be comprised of 14-inch diameter Schedule 80 carbon steel, encased in a minimum 42-inch by 42-inch concrete-filled trench.

From review of Section F-F', it appears the inlet of the outlet pipe will be founded within clay overlaying a basalt rock layer. Settlement of the outlet pipe foundation may be reduced at the inlet side by the basalt, which appears to transition to soil-gravel strata at some undefined location along the pipe downstream of the inlet. Stresses induced by the limited differential settlement are expected to be negligible, and limited by the concrete backfill and steel pipe strength. The pipe currently is proposed with a vertical drop of approximately 31 feet along the 1290-foot length of pipe. Post consolidation pipe slope (assuming a worst-case settlement of 2.1 feet at the pipe inlet) would average 2.3 percent along the length of the pipe. As described above, field standard penetration tests suggest actual post-construction settlement will be less than that calculated on the basis of consolidation test results.

### **Attachments**

- Attachment 1 – Laboratory Test Summary Tables and Consolidation Test Results
- Attachment 2 – Consolidation Testing Summary and SETTLE 3D Model Input Tables
- Attachment 3 – SETTLE 3D Output Files
- Attachment 4 – Embankment Analysis Cross-sections with Graphical Output Results  
(Sections B-B' and D-D' only)

**ATTACHMENT 1**  
**LABORATORY TEST SUMMARY TABLES AND CONSOLIDATION TEST RESULTS**

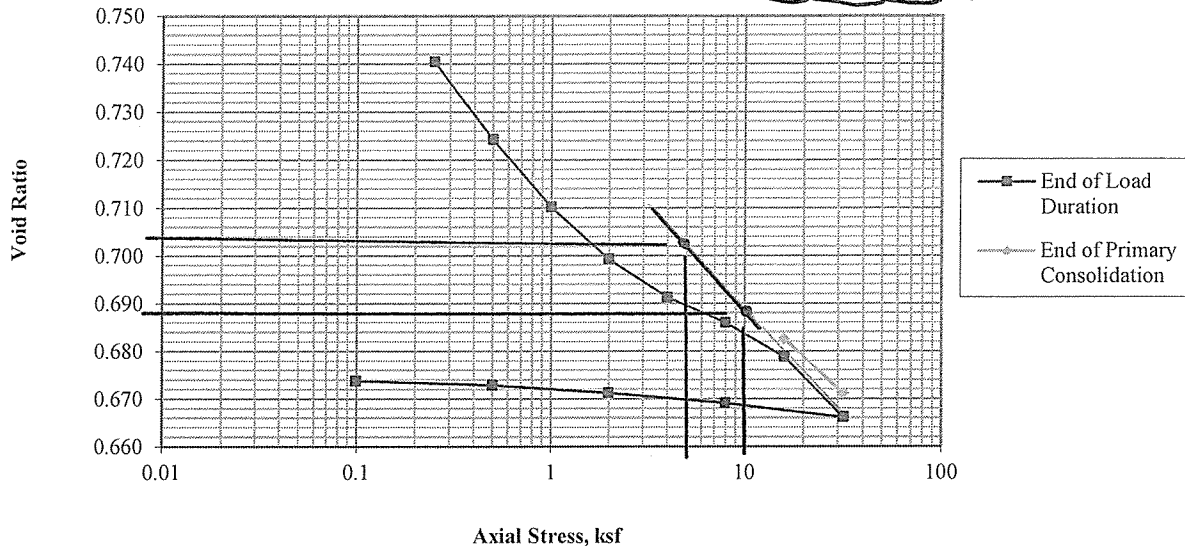
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>				
Height =	0.994 in		0.960 in	USCS description (ASTM D2487):	Lean clay with sand, yellowish red, moist			
Diameter =	2.499 in		2.499 in	Atterberg Limits (ASTM D4318):	LL = 47	PL = 20	PI = 27	
Area =	4.905 in <sup>2</sup>		4.905 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 99%	No. 200 = 81%	
Volume =	4.875 in <sup>3</sup>		4.709 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted		
Water Content =	14.4%		4.8%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture		
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.6%			
Height of Solids =	0.5641 in		0.5641 in	Trimming Procedure:	Specimen remolded in ring			
Void Ratio =	0.762		0.702	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated		
Degree of Saturation =	51.1%		18.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B		
Wet Mass =	0.308 lb		0.282 lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)		
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial		
Wet Unit Weight =	109.3 pcf		103.6 pcf	Final Differential Height:	-0.0158 in			
Dry Unit Weight =	95.5 pcf		98.9 pcf	Estimated Preconsolidation Stress:	Not Computed			

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

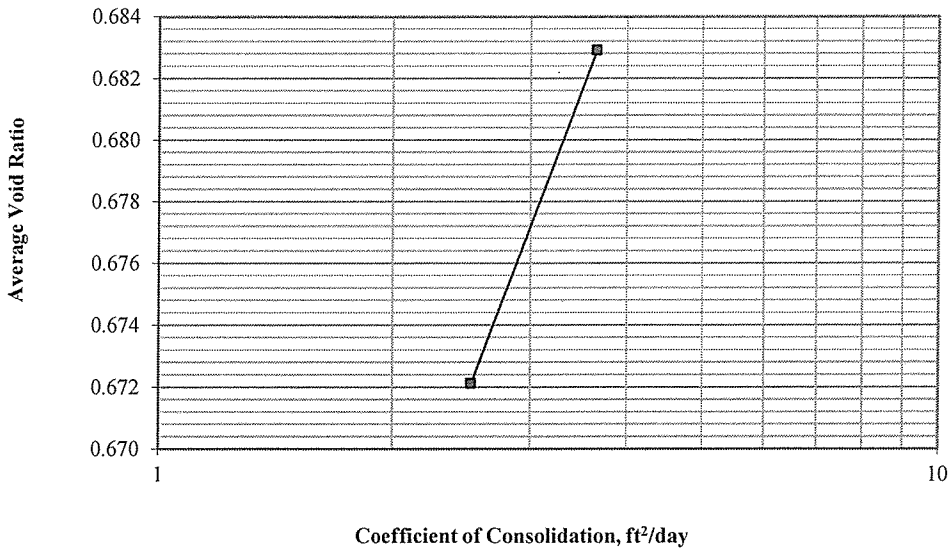
<b>Golder Associates Inc.</b> Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-10 @ 19-33 ft		Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 1

Void Ratio vs. Axial Stress

$$C_c = \frac{.702 - .688}{\log\left(\frac{10}{5}\right)} = 0.05$$



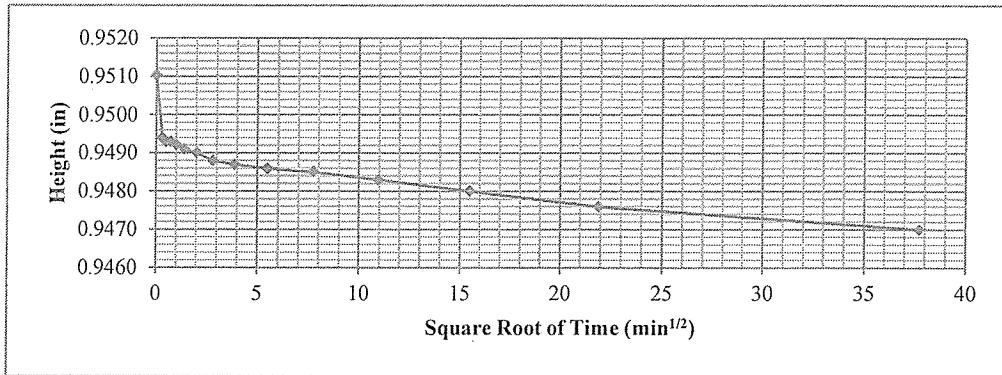
Average Void Ratio vs. Coefficient of Consolidation



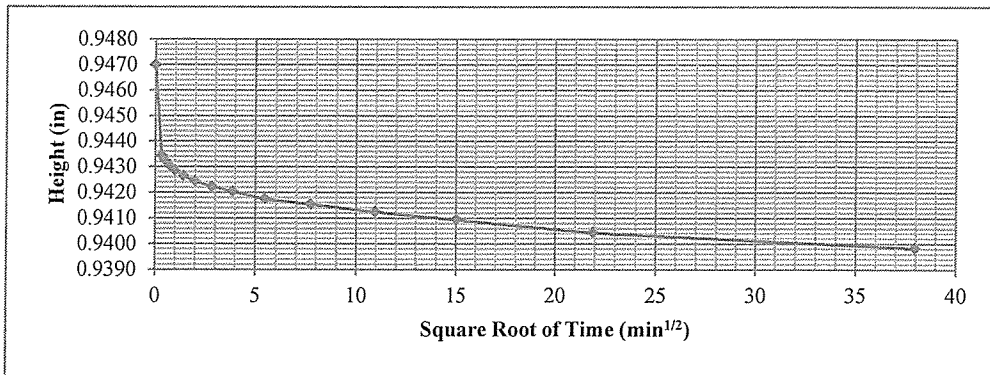
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-10 @ 19-33 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2	



16.00 ksf



32.00 ksf

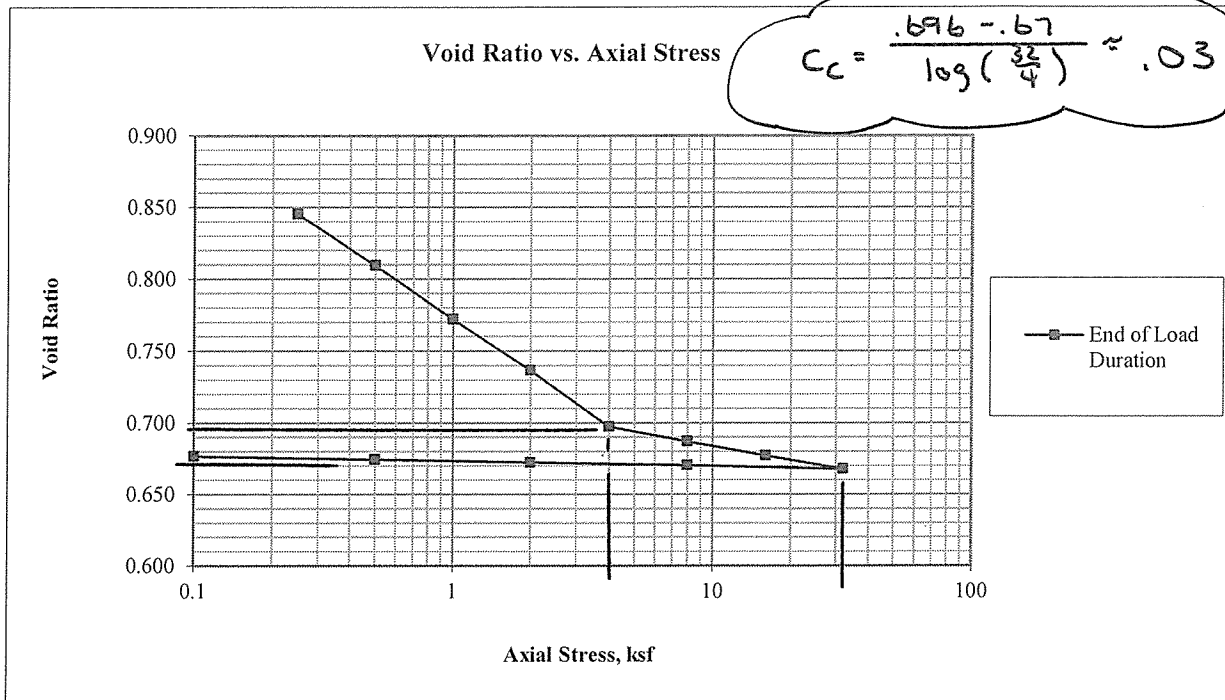


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-10 @ 19-33 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in <sup>2</sup>		4.893 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in <sup>3</sup>		4.428 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
<b>Job Short Title:</b> Copper Flat Tailings Design Study		<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1
<b>Sample:</b> BH-12 @ 33.5-48.5 ft						



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<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-12 @ 33.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

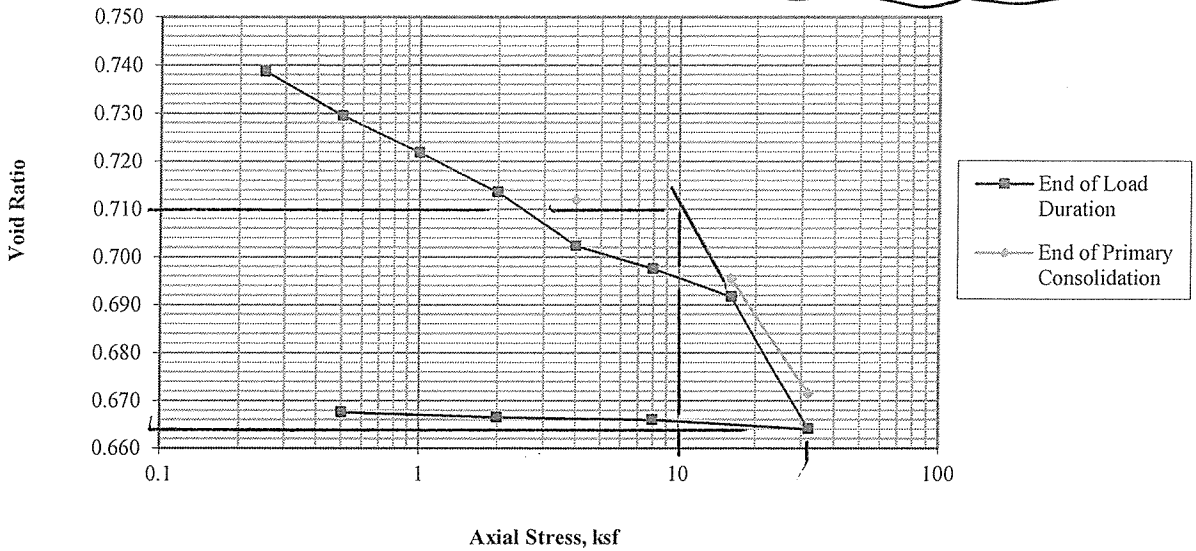
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.993	in	0.887	in	USCS description (ASTM D2487):	Clayey sand with gravel, yellowish brown, dry	
Diameter =	2.500	in	2.500	in	Atterberg Limits (ASTM D4318):	LL = 23	PL = 14
Area =	4.909	in <sup>2</sup>	4.909	in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 83%
Volume =	4.874	in <sup>3</sup>	4.354	in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted
Water Content =	14.1%		1.7%		Remold Targets:	95.0	pcf (dry) at 15.0% moisture
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%	
Height of Solids =	0.5636	in	0.5636	in	Trimming Procedure:	Specimen remolded in ring	
Void Ratio =	0.762		0.574		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated
Degree of Saturation =	49.8%		8.0%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B
Wet Mass =	0.307	lb	0.274	lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial
Wet Unit Weight =	108.9	pcf	108.7	pcf	Final Differential Height:	0.0528 in	
Dry Unit Weight =	95.5	pcf	106.9	pcf	Estimated Preconsolidation Stress:	16.4 ksf	
							PI = 9
							No. 200 = 36%

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

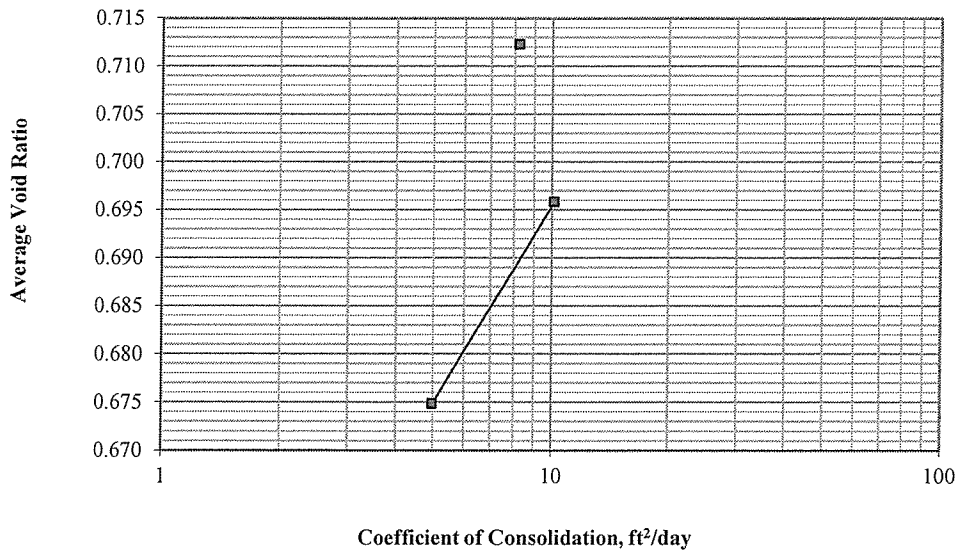
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-16 @ 29-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> I	

Void Ratio vs. Axial Stress

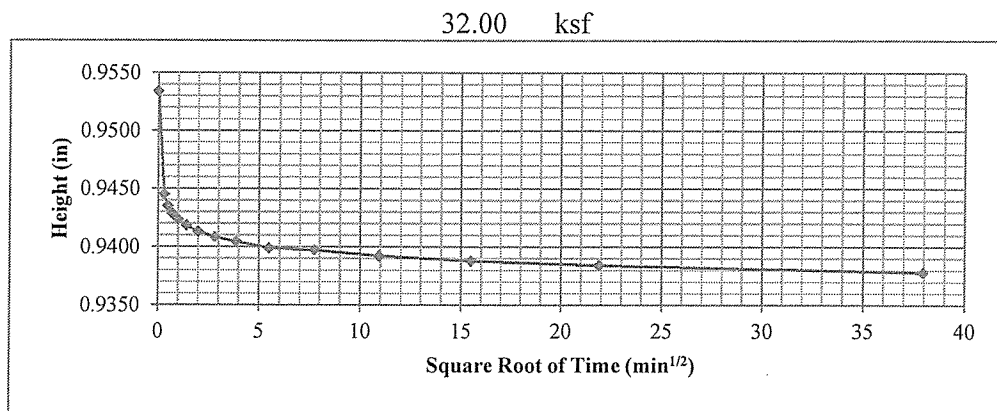
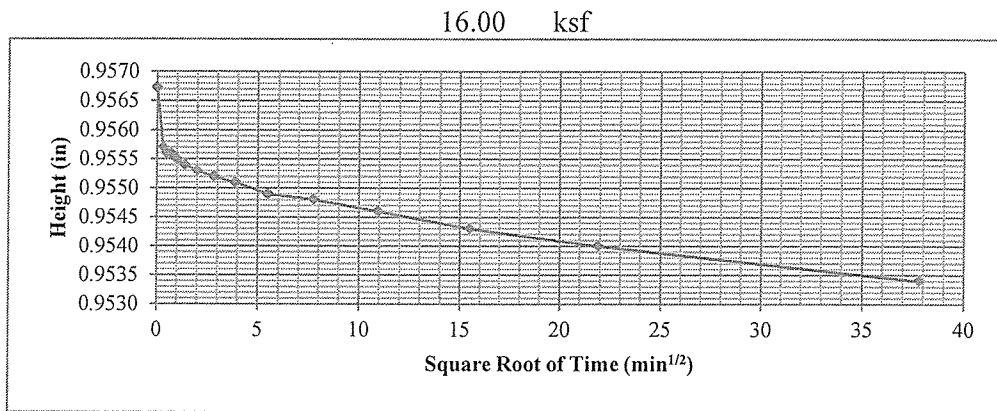
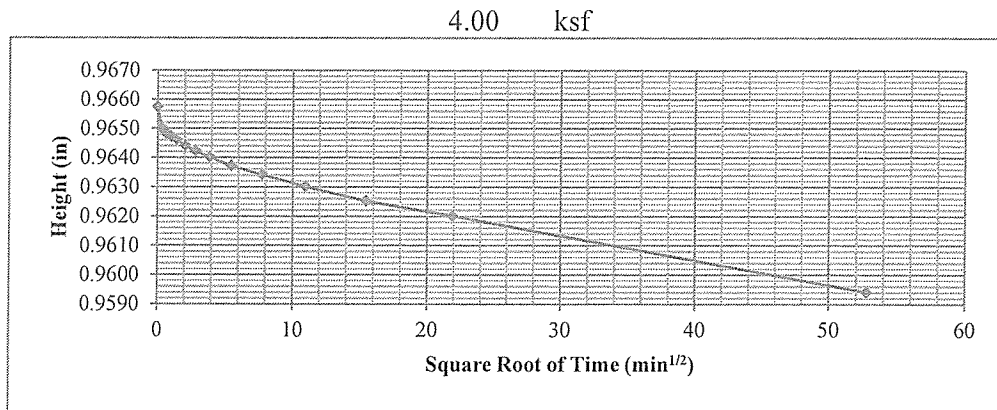
$$C_c = \frac{.71 - .664}{\log\left(\frac{32}{10}\right)} \approx .09$$



Average Void Ratio vs. Coefficient of Consolidation



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>BH-16 @ 29-34 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.960 in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 14	PI = 11
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 52%
Volume =	4.886 in <sup>3</sup>		4.705 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.5%		2.7%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%		
Height of Solids =	0.5638 in		0.5638 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.768		0.703	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		10.4%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308 lb		0.276 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.0 pcf		101.5 pcf	Final Differential Height:	0.0045 in		
Dry Unit Weight =	95.1 pcf		98.8 pcf	Estimated Preconsolidation Stress:	14.7 ksf		

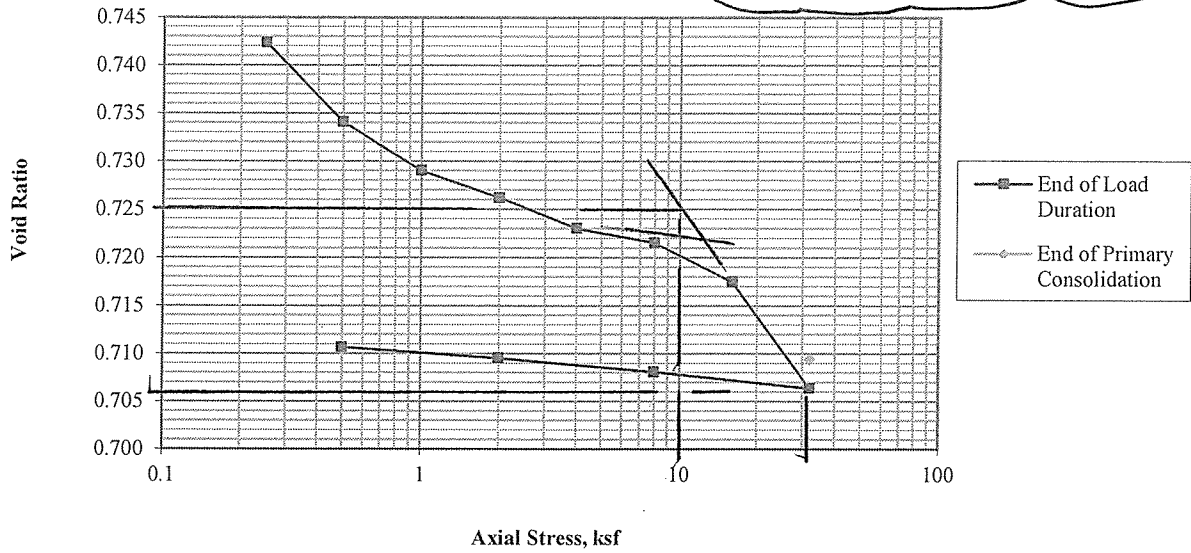
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	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

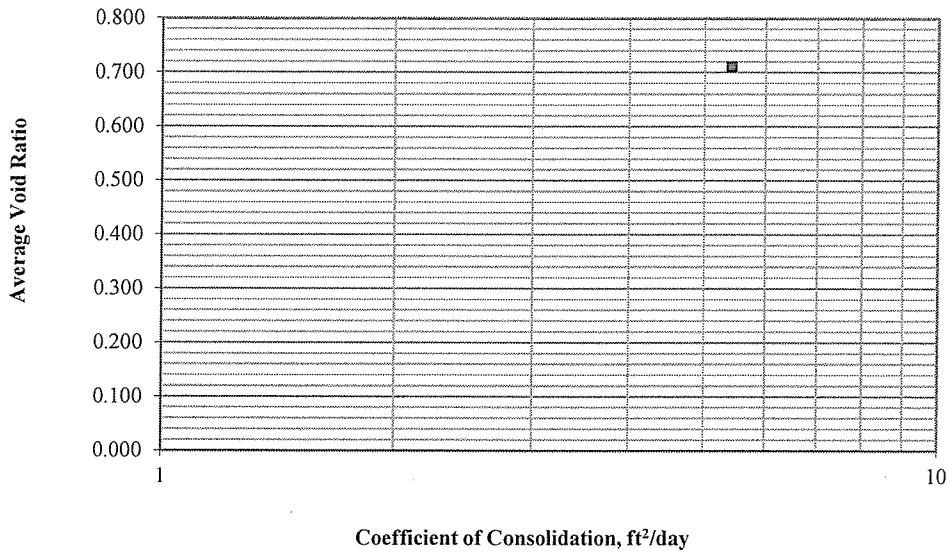
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

Void Ratio vs. Axial Stress

$$C_c = \frac{.725 - .706}{\log\left(\frac{32}{10}\right)} \approx .04$$



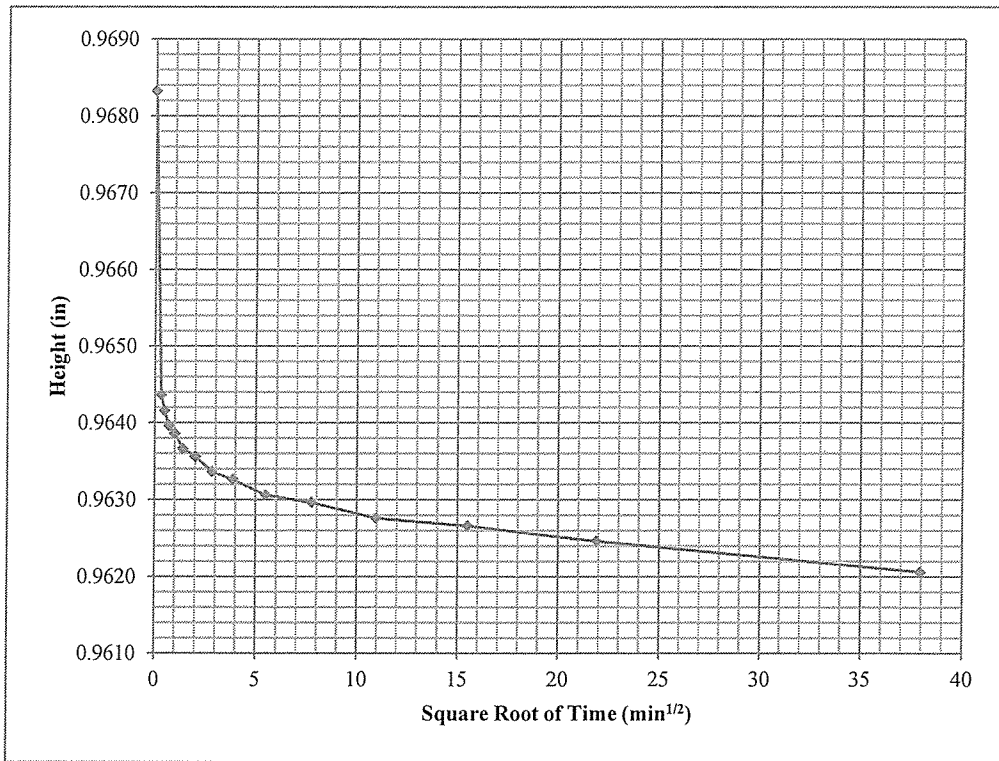
Average Void Ratio vs. Coefficient of Consolidation



<b>Golder Associates Inc.</b> Denver, Colorado		Title: <b>ASTM D2435                  ONE-DIMENSIONAL CONSOLIDATION TEST REPORT                  CONSOLIDATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	



32.00 ksf



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3	

	Initial		Final	
Height =	0.994	in	0.924	in
Diameter =	2.498	in	2.498	in
Area =	4.901	in <sup>2</sup>	4.901	in <sup>2</sup>
Volume =	4.871	in <sup>3</sup>	4.528	in <sup>3</sup>
Water Content =	28.9%		9.0%	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)
Height of Solids =	0.5085	in	0.5085	in
Void Ratio =	0.955		0.817	
Degree of Saturation =	81.7%		29.7%	
Wet Mass =	0.313	lb	0.264	lb
Dry Mass =	0.243	lb	0.243	lb
Wet Unit Weight =	110.9	pcf	100.9	pcf
Dry Unit Weight =	86.1	pcf	92.6	pcf

Notes

USCS description (ASTM D2487):

Atterberg Limits (ASTM D4318):

Percent Finer (ASTM D422):

Specimen Type:

Remold Targets:

Water Content of Trimings (ASTM D2216):

Trimming Procedure:

Inundation:

Test Method:

Apparatus:

Final Water Content Specimen:

Final Differential Height:

Estimated Preconsolidation Stress:

Fat clay with sand, dark red, wet

LL = 62                      PL = 20                      PI = 42

3/4 in. = 100%                      No. 4 = 100%                      No. 200 = 82%

Intact                       Reconstituted

86.0 pcf (dry) at                      29.0% moisture

29.1%

Specimen remolded in ring

Not inundated                       Inundated

A                       B

Frame No. 5                      (ELE C-320A)

Entire                       Partial

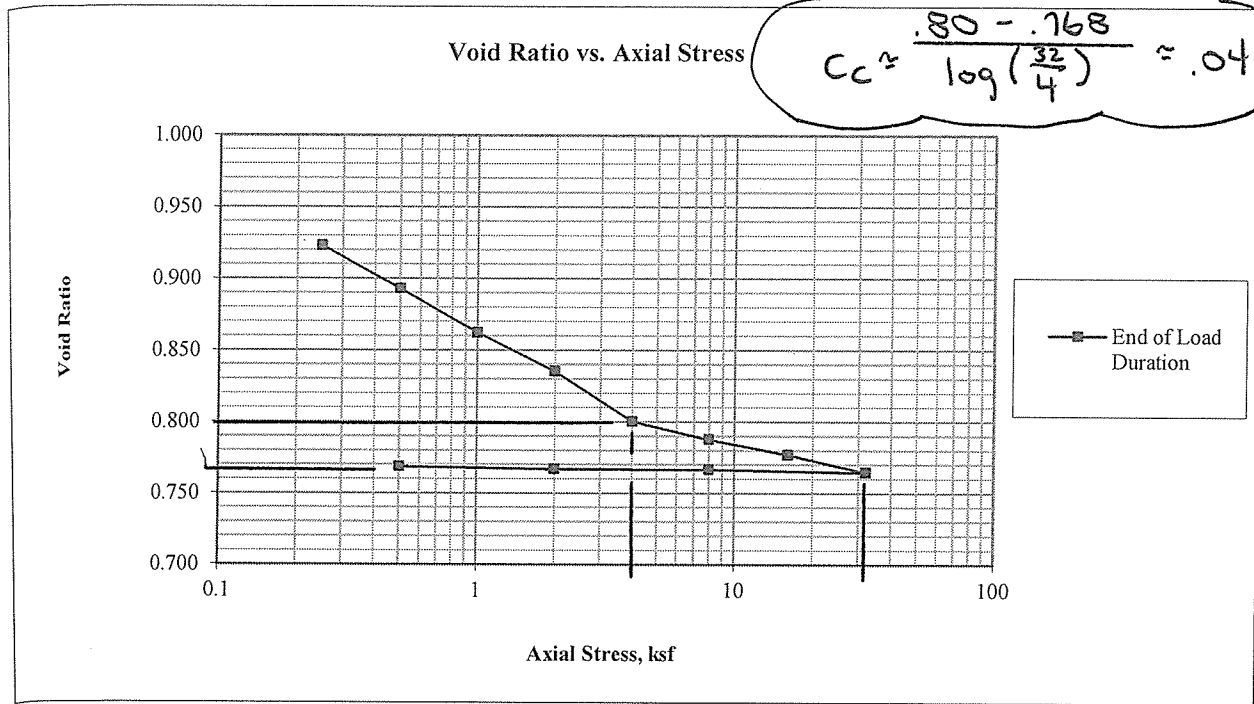
-0.0246 in

-- ksf

-- indicates test was not performed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Sample: BH-18 @ 43.5-48.5 ft		Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006
					Figure: 1



<b>Golder Associates Inc.</b> Denver, Colorado	Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-18 @ 43.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2

	Initial		Final	
Height =	1.000	in	0.981	in
Diameter =	2.497	in	2.497	in
Area =	4.897	in <sup>2</sup>	4.897	in <sup>2</sup>
Volume =	4.897	in <sup>3</sup>	4.804	in <sup>3</sup>
Water Content =	9.7%		1.1%	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)
Height of Solids =	0.5363	in	0.5363	in
Void Ratio =	0.865		0.829	
Degree of Saturation =	30.3%		3.6%	
Wet Mass =	0.281	lb	0.259	lb
Dry Mass =	0.256	lb	0.256	lb
Wet Unit Weight =	99.0	pcf	93.0	pcf
Dry Unit Weight =	90.2	pcf	92.0	pcf

Notes

USCS description (ASTM D2487): Sandy silty clay, brownish yellow, moist

Atterberg Limits (ASTM D4318): LL = 25 PL = 21 PI = 4

Percent Finer (ASTM D422): 3/4 in. = 99% No. 4 = 99% No. 200 = 52%

Specimen Type:  Intact  Reconstituted

Remold Targets: 90.0 pcf (dry) at 10.0% moisture

Water Content of Trimmings (ASTM D2216): 9.4%

Trimming Procedure: Specimen remolded in ring

Inundation:  Not inundated  Inundated

Test Method:  A  B

Apparatus: Frame No. 6 (ELE C-320A)

Final Water Content Specimen:  Entire  Partial

Final Differential Height: -0.0094 in

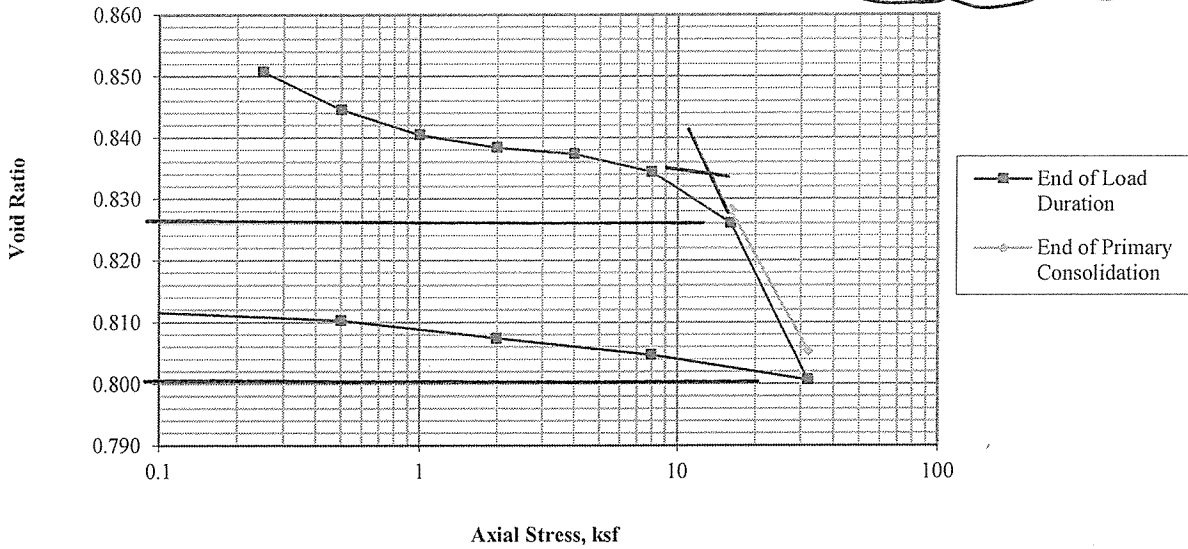
Estimated Preconsolidation Stress: 13.0 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

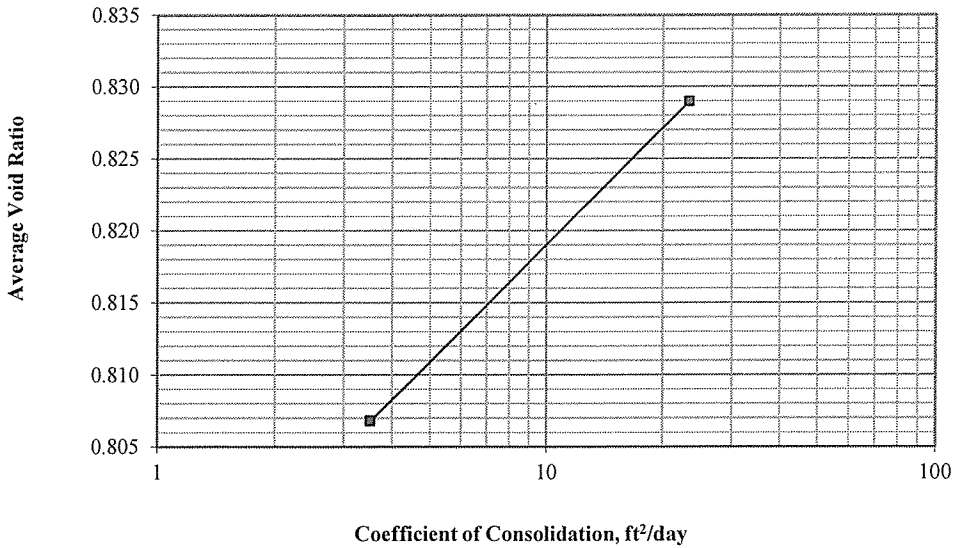
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-22 @ 0-8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

Void Ratio vs. Axial Stress

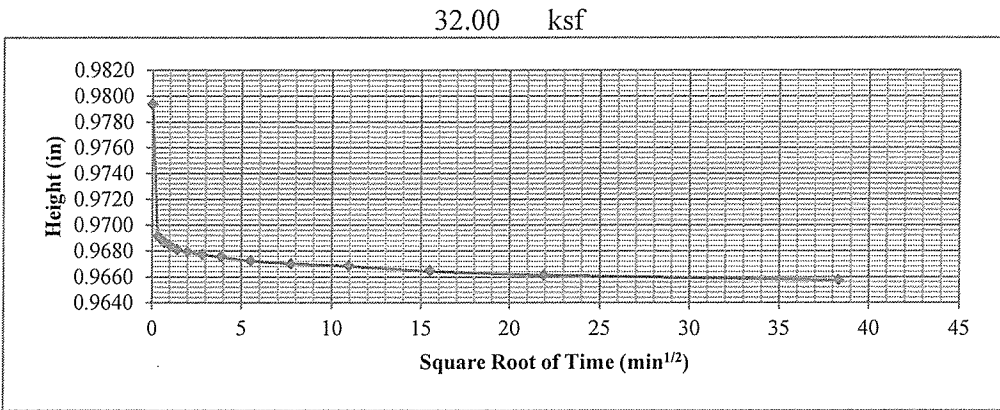
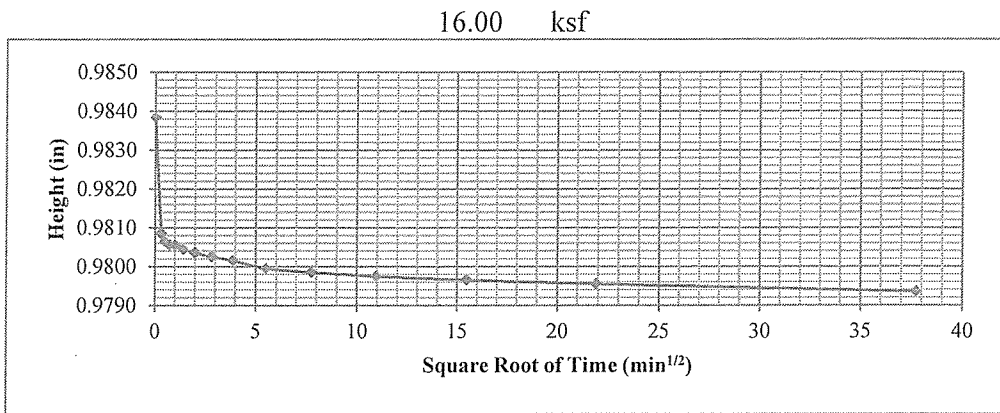
$$C_c = \frac{.826 - .80}{\log\left(\frac{32}{16}\right)} = 0.09$$



Average Void Ratio vs. Coefficient of Consolidation



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
	Job Short Title: Copper Flat Tailings Design Study				
Sample: <b>BH-22 @ 0-8.5 ft</b>	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-22 @ 0-8.5 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/11/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>3</b>

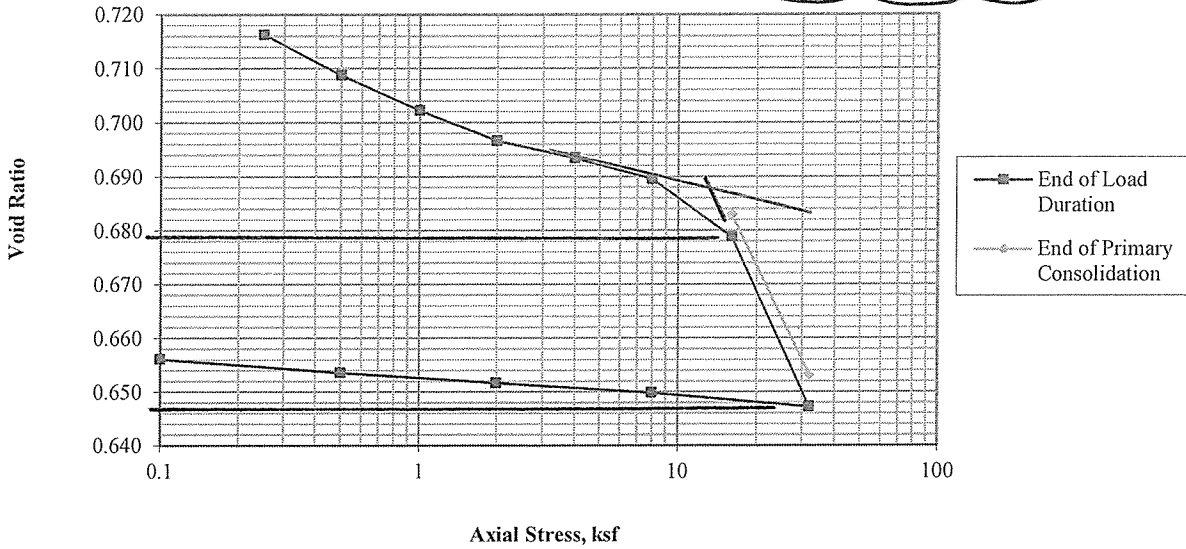
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36	PL = 18	PI = 18
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 82%	No. 200 = 37%
Volume =	4.886 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%		
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in		
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

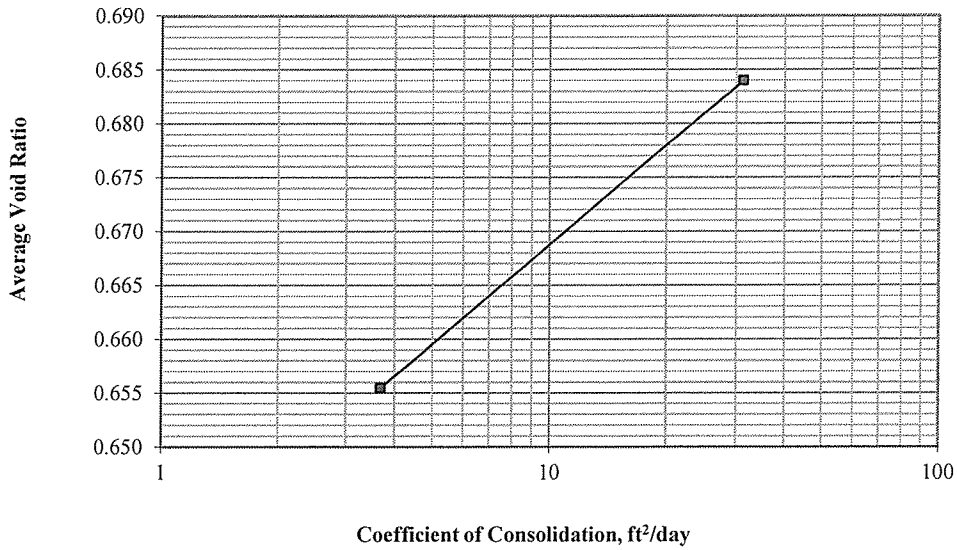
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA					
<b>Job Short Title:</b> Copper Flat Tailings Design Study							
<b>Sample:</b> BH-22 @ 28-30 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1		

Void Ratio vs. Axial Stress

$$C_c = \frac{.679 - .647}{\log\left(\frac{33}{16}\right)} = 0.11$$



Average Void Ratio vs. Coefficient of Consolidation



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 CONSOLIDATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-22 @ 28-30 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/11/2013

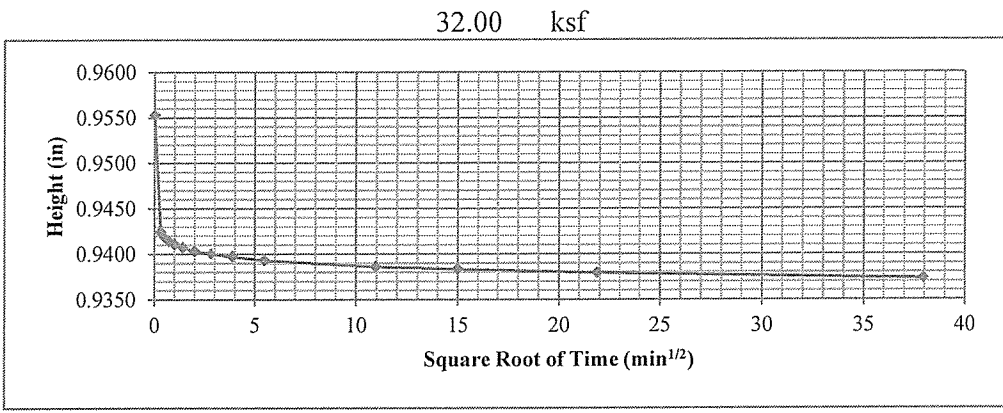
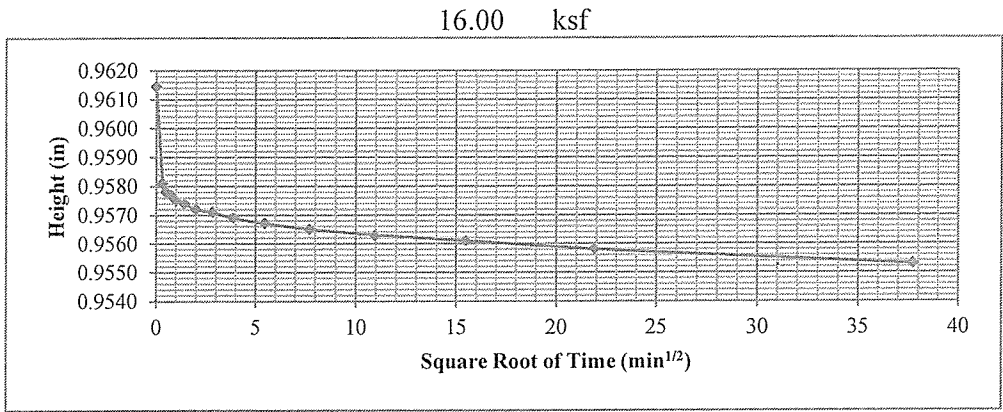
Job Number:

103-92557.006

Figure:

2





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 28-30 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3

	Initial	Final
Height =	1.000 in	0.973 in
Diameter =	2.498 in	2.498 in
Area =	4.901 in <sup>2</sup>	4.901 in <sup>2</sup>
Volume =	4.901 in <sup>3</sup>	4.769 in <sup>3</sup>
Water Content =	9.8%	4.2%
Specific Gravity =	2.70 (Assumed)	2.70 (Assumed)
Height of Solids =	0.5558 in	0.5558 in
Void Ratio =	0.799	0.751
Degree of Saturation =	33.0%	15.1%
Wet Mass =	0.291 lb	0.276 lb
Dry Mass =	0.265 lb	0.265 lb
Wet Unit Weight =	102.7 pcf	100.1 pcf
Dry Unit Weight =	93.5 pcf	96.1 pcf

Notes

USCS description (ASTM D2487): Clayey sand, reddish brown, moist

Atterberg Limits (ASTM D4318): LL = 37 PL = 17 PI = 20

Percent Finer (ASTM D422): 3/4 in. = 100% No. 4 = 95% No. 200 = 39%

Specimen Type:  Intact  Reconstituted

Remold Targets: 93.0 pcf (dry) at 10.0% moisture

Water Content of Trimmings (ASTM D2216): 9.9%

Trimming Procedure: Specimen remolded in ring

Inundation:  Not inundated  Inundated

Test Method:  A  B

Apparatus: Frame No. 6 (ELE C-320A)

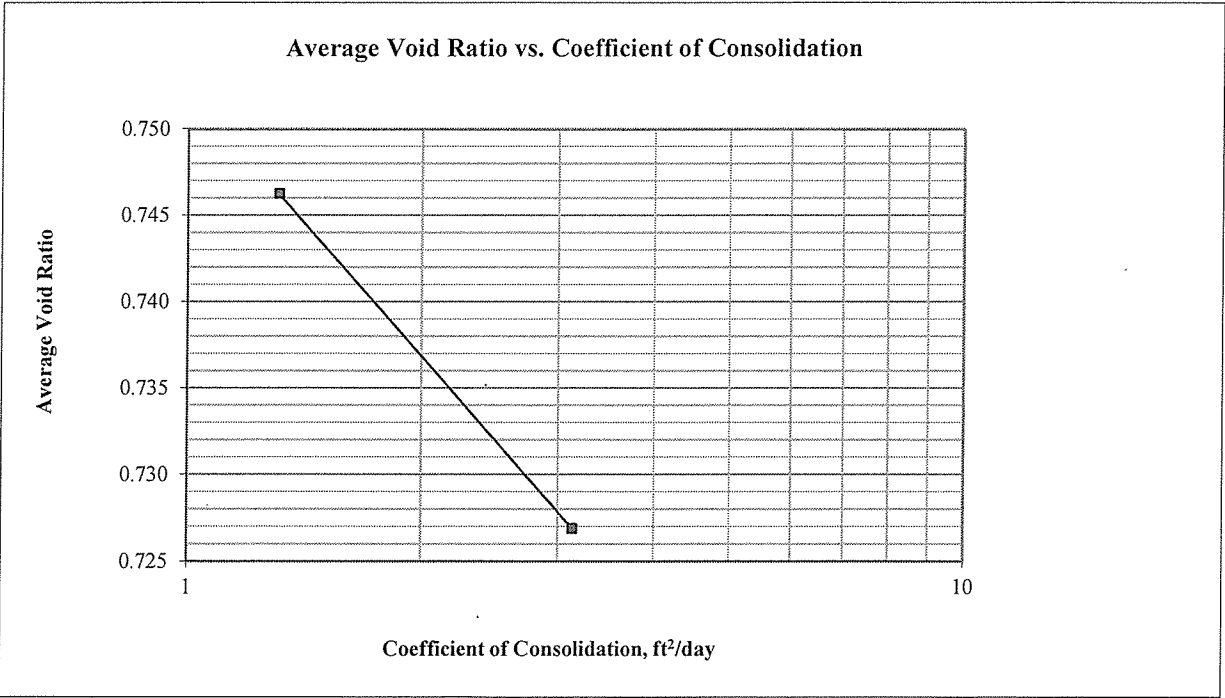
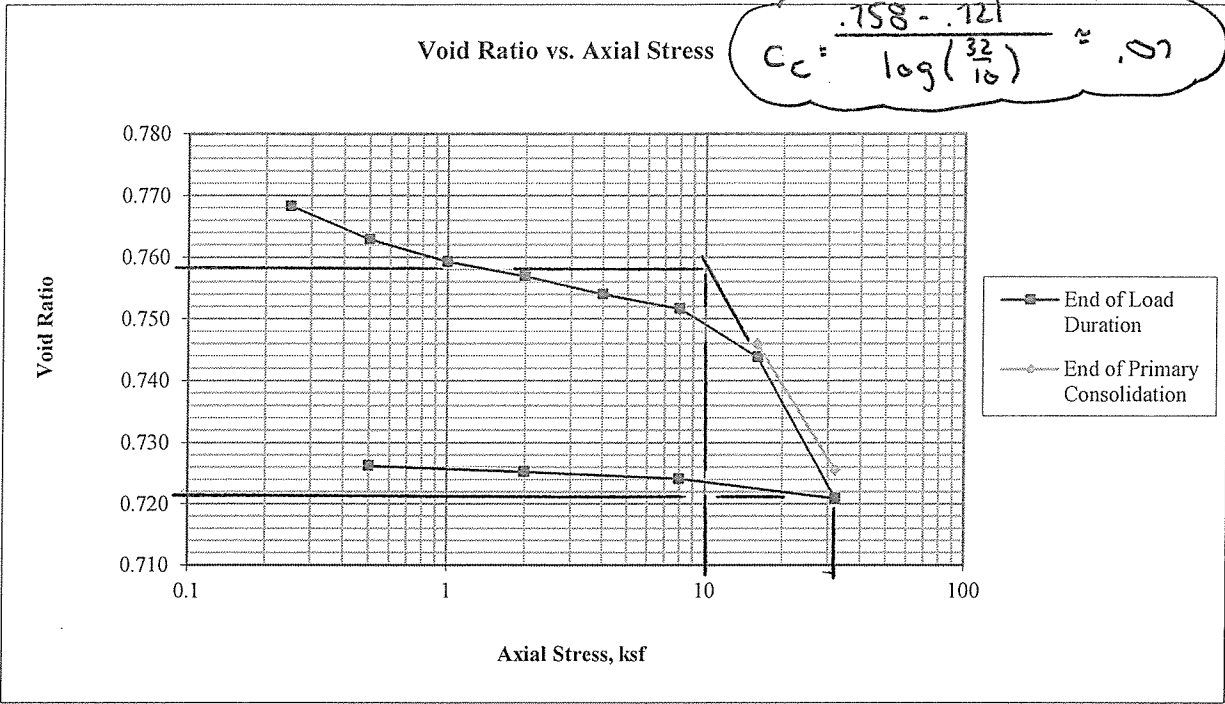
Final Water Content Specimen:  Entire  Partial

Final Differential Height: -0.0136 in

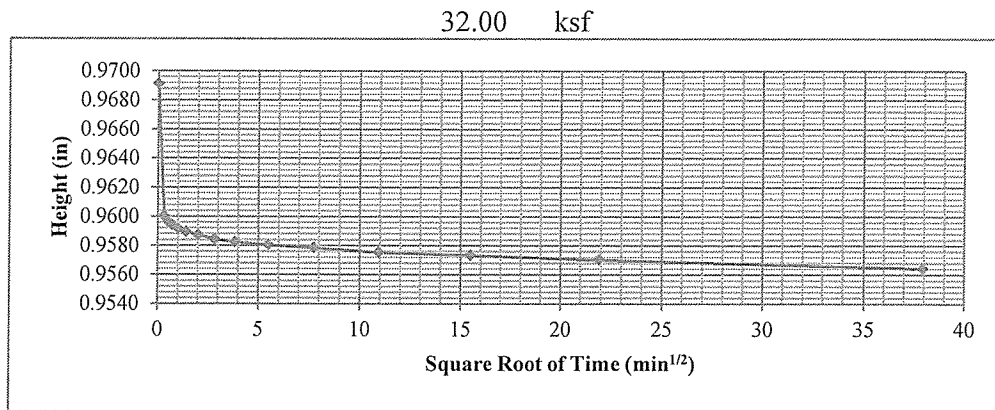
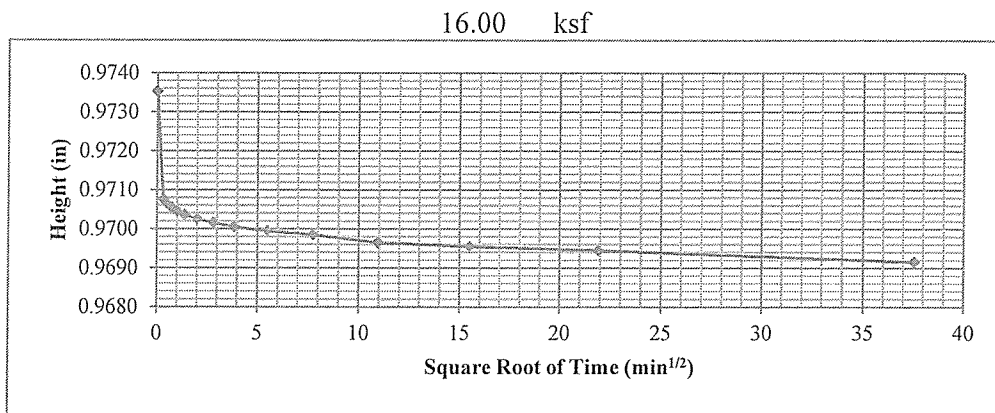
Estimated Preconsolidation Stress: 13.8 ksf

Seating	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title:  ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>					
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-25 @ 22-34 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>2</b>	



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 22-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 3

**ATTACHMENT 2**  
**CONSOLIDATION TESTING SUMMARY AND SETTLE 3D MODEL INPUT TABLES**

**Table 1: Summary Results Of Consolidation Tests -- Copper Flat, Sierra County, New Mexico**

Boring No.	Sample Depth	In-situ Overburden Pressure at Midpoint of Tested Stratum (kips, Note 1)	Material Classification	Void Ratio, min	Void Ratio, max	Void Ratio, mean	Estimated Initial Void Ratio (from consolidation curve)	Cc for Analysis (Note 2)	Estimated Moist Density (final) (pcf) (Note 3)
BH-10	19-33	2.86	CL	0.666	0.74	0.703	0.69	0.05	104
BH-12	33.5-48.5	4.51	CH	0.67	0.845	0.7575	0.69	0.3	104
BH-16	29-34	3.465	SC	0.664	0.738	0.701	0.704	0.09	110
BH-18	23-33.5	3.1075	CL	0.706	0.742	0.724	0.724	0.04	103
BH-18	43.5-48.5	5.06	CH	0.767	0.925	0.846	0.79	0.04	102
BH-22	0-8.5	0.4675	ML	0.8	0.85	0.825	0.844	0.09	98
BH-22	28-30	3.19	SC	0.646	0.716	0.681	0.695	0.11	108
BH-25	22-34	3.08	SC	0.727	0.746	0.7365	0.756	0.07	101

**Notes**

1. Based on an assumed unit weight of 110 pcf.
2. Generally based on portion of curve developed for loading between 16 and 32 kips.
3. Moist Density estimated for water content ranging from 3 to 6 percent.

**Table 2: Stratum Parameters For Settlement Analysis -- Copper Flat, Sierra County, New Mexico**

Stratum Designation	Soil Types	Assumed Initial Void Ratio	Assumed Cc Value	Referenced Consolidation Tests	Assumed Unit Weight (pcf)	Young's Modulus (ksf)
1	ML, CL-ML, SM	0.74	0.09	BH-16, BH-22 (2), BH-25	106	NA
2	CL	0.71	0.045	BH-10, BH-18	104	NA
3	CH, MH	0.75	0.035	BH-12, BH-18	103	NA
4 (Note 1)	Gravel, Sand	NA	NA	Ref. 1	110	4000
5 (Note 2)	Basalt, Caliche	NA	NA	Ref. 1	125	4000
6 (Note 3)	Embankment Fill	NA	NA	NA	97	NA
7 (Note 4)	Tailings	NA	NA	NA	97	NA

**Notes**

1. Settlement within embankment fill not calculated for foundation settlement evaluation.
2. Settlement within tailings not analyzed for foundation settlement evaluation.

**References**

1. US Army Corps of Engineer, EM-1110-1904, 1990.

**Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico**

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

**Notes**

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.



**Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico**

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

**Notes**

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.

**ATTACHMENT 3  
SETTLE 3D OUTPUT FILES**

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

Document Name: Section B-B', Line 1.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

Stage #	Name
1	Stage 1

### Results

Time taken to compute: 0.49066 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.97303
Consolidation Settlement [ft]	0	1.97303
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	22.7
Total Stress [ksf]	0	31.6782
Total Strain	-0	0.10667
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.020072	31.6754
Over-consolidation Ratio	1	1
Void Ratio	0.527594	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 22.7 ksf  
 Depth: 0 ft

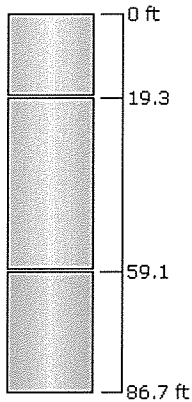
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	19.3	0
2	Soil Stratum 2	39.8	19.3
3	Soil Stratum 3	27.6	59.1



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

Document Name: Section B-B', Line 2.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

Stage #	Name
1	Stage 1

### Results

Time taken to compute: 0.646645 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98619
Consolidation Settlement [ft]	0	1.98619
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.9
Total Stress [ksf]	0	27.3058
Total Strain	-0	0.0869309
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.006916	27.303
Over-consolidation Ratio	1	1
Void Ratio	0.561348	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 13.9 ksf  
 Depth: 0 ft

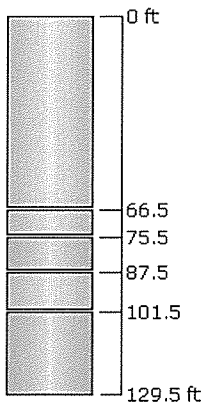
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	66.5	0
2	Soil Stratum 2	9	66.5
3	Soil Stratum 3	12	75.5
4	Soil Stratum 2	14	87.5
5	Soil Stratum 3	28	101.5



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 59

## Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01



# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 3.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.561914 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.4179
Consolidation Settlement [ft]	0	1.4179
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.3
Total Stress [ksf]	0	21.0132
Total Strain	-0	0.103067
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.01612	21.0103
Over-consolidation Ratio	1	1
Void Ratio	0.533756	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 13.3 ksf  
 Depth: 0 ft

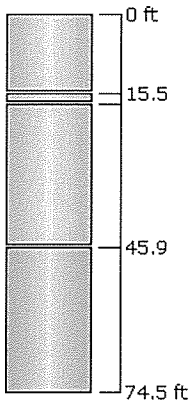
Installation Stage: Stage 1

**Coordinates**



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	15.5	0
2	Soil Stratum 3	2.1	15.5
3	Soil Stratum 2	28.3	17.6
4	Soil Stratum 3	28.6	45.9



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

**Field Point Grid**

Number of points: 289  
Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 4.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.491452 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.00705
Consolidation Settlement [ft]	0	1.00705
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	7.3
Total Stress [ksf]	0	14.379
Total Strain	-0	0.0723724
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.065	14.3761
Over-consolidation Ratio	1	1
Void Ratio	0.586243	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 7.3 ksf  
 Depth: 0 ft

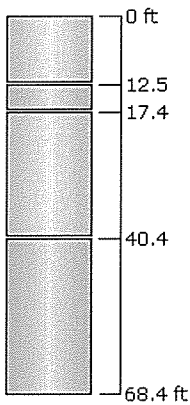
Installation Stage: Stage 1

**Coordinates**



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	12.5	0
2	Soil Stratum 3	4.9	12.5
3	Soil Stratum 2	23	17.4
4	Soil Stratum 3	28	40.4



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 53

**Field Point Grid**

Number of points: 289  
Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 5.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.562388 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.508677
Consolidation Settlement [ft]	0	0.508677
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	1
Total Stress [ksf]	0	8.18236
Total Strain	-0	0.139728
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019928	8.17977
Over-consolidation Ratio	1	1
Void Ratio	0.496874	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 1 ksf  
 Depth: 0 ft

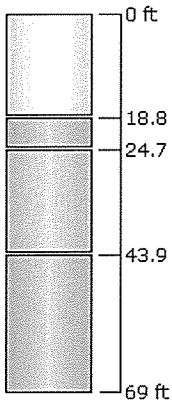
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.8	0
2	Soil Stratum 3	5.9	18.8
3	Soil Stratum 2	19.2	24.7
4	Soil Stratum 3	25.1	43.9



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

**Field Point Grid**



Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 1.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.499416 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	2.11072
Consolidation Settlement [ft]	0	2.11072
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	23.4
Total Stress [ksf]	0	28.7145
Total Strain	-0	0.211235
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019292	28.7127
Over-consolidation Ratio	1	1
Void Ratio	0.372451	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 23.4 ksf  
 Depth: 0 ft

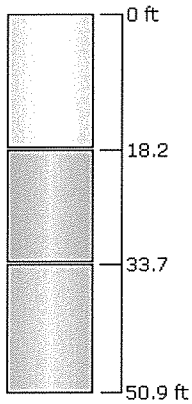
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.2	0
2	Soil Stratum 3	15.5	18.2
3	Soil Stratum 2	17.2	33.7



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 2.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.498987 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98358
Consolidation Settlement [ft]	0	1.98358
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	18.5
Total Stress [ksf]	0	24.1716
Total Strain	-0	0.206331
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.018974	24.1698
Over-consolidation Ratio	1	1
Void Ratio	0.380985	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 18.5 ksf  
 Depth: 0 ft

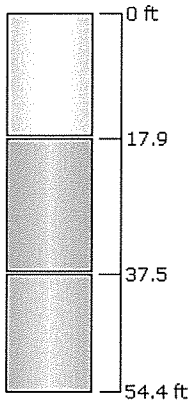
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	17.9	0
2	Soil Stratum 3	19.6	17.9
3	Soil Stratum 2	16.9	37.5



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 3.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.576466 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.40443
Consolidation Settlement [ft]	0	1.35522
Immediate Settlement [ft]	0	0.0492125
Loading Stress [ksf]	0	12.7
Total Stress [ksf]	0	18.5709
Total Strain	-0	0.203888
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.014522	18.5692
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 12.7 ksf  
 Depth: 0 ft



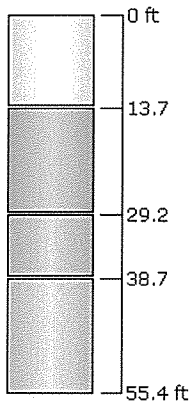
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	13.7	0
2	Soil Stratum 4	15.5	13.7
3	Soil Stratum 3	9.5	29.2
4	Soil Stratum 2	16.7	38.7



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4
Color				
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11
Immediate Settlement	Disabled	Disabled	Disabled	Enabled
Es [ksf]				4000
Esur [ksf]				4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear	
Cc	0.09	0.035	0.045	
Cr	0.1	0.1	0.1	
e0	0.74	0.75	0.71	
OCR	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 65

## Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 4.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.636294 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.948079
Consolidation Settlement [ft]	0	0.914599
Immediate Settlement [ft]	0	0.03348
Loading Stress [ksf]	0	6.2
Total Stress [ksf]	0	12.7328
Total Strain	-0	0.141231
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.05777	12.7313
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 6.2 ksf  
 Depth: 0 ft

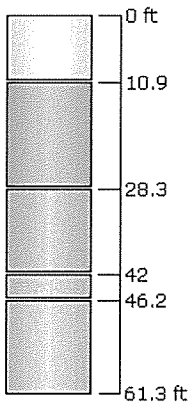
Installation Stage: Stage 1

**Coordinates**






X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	10.9	0
2	Soil Stratum 4	17.4	10.9
3	Soil Stratum 3	13.7	28.3
4	Soil Stratum 5	4.2	42
5	Soil Stratum 2	15.1	46.2



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	4000
Esur [ksf]				4000	4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 71

### Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

#### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

Document Name: Section D-D', Line 5.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

Stage #	Name
1	Stage 1

### Results

Time taken to compute: 0.583683 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.175852
Consolidation Settlement [ft]	0	0.173272
Immediate Settlement [ft]	0	0.00257999
Loading Stress [ksf]	0	0.4
Total Stress [ksf]	0	7.63261
Total Strain	-0	0.0875971
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.04134	7.61899
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 0.4 ksf  
 Depth: 0 ft

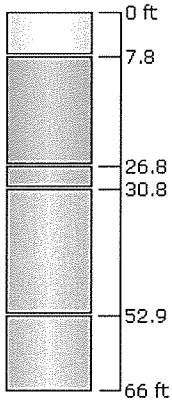
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	7.8	0
2	Soil Stratum 4	19	7.8
3	Soil Stratum 3	4	26.8
4	Soil Stratum 5	22.1	30.8
5	Soil Stratum 2	13.1	52.9



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	13000
Esur [ksf]				4000	13000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 63

### Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

#### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01



**ATTACHMENT 4**  
**EMBANKMENT ANALYSIS CROSS-SECTIONS WITH GRAPHICAL OUTPUT RESULTS**  
**(SECTIONS B-B' AND D-D' ONLY)**

# SECTION B-B' PROFILE

B' NORTHWEST  
B' SOUTHEAST

## LEGEND

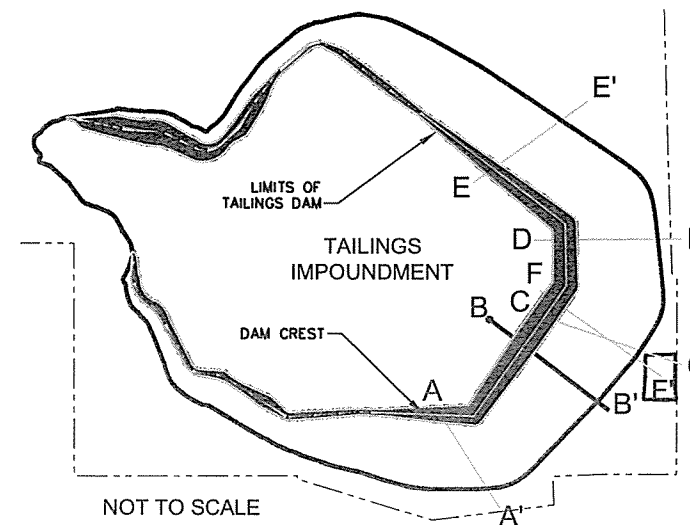
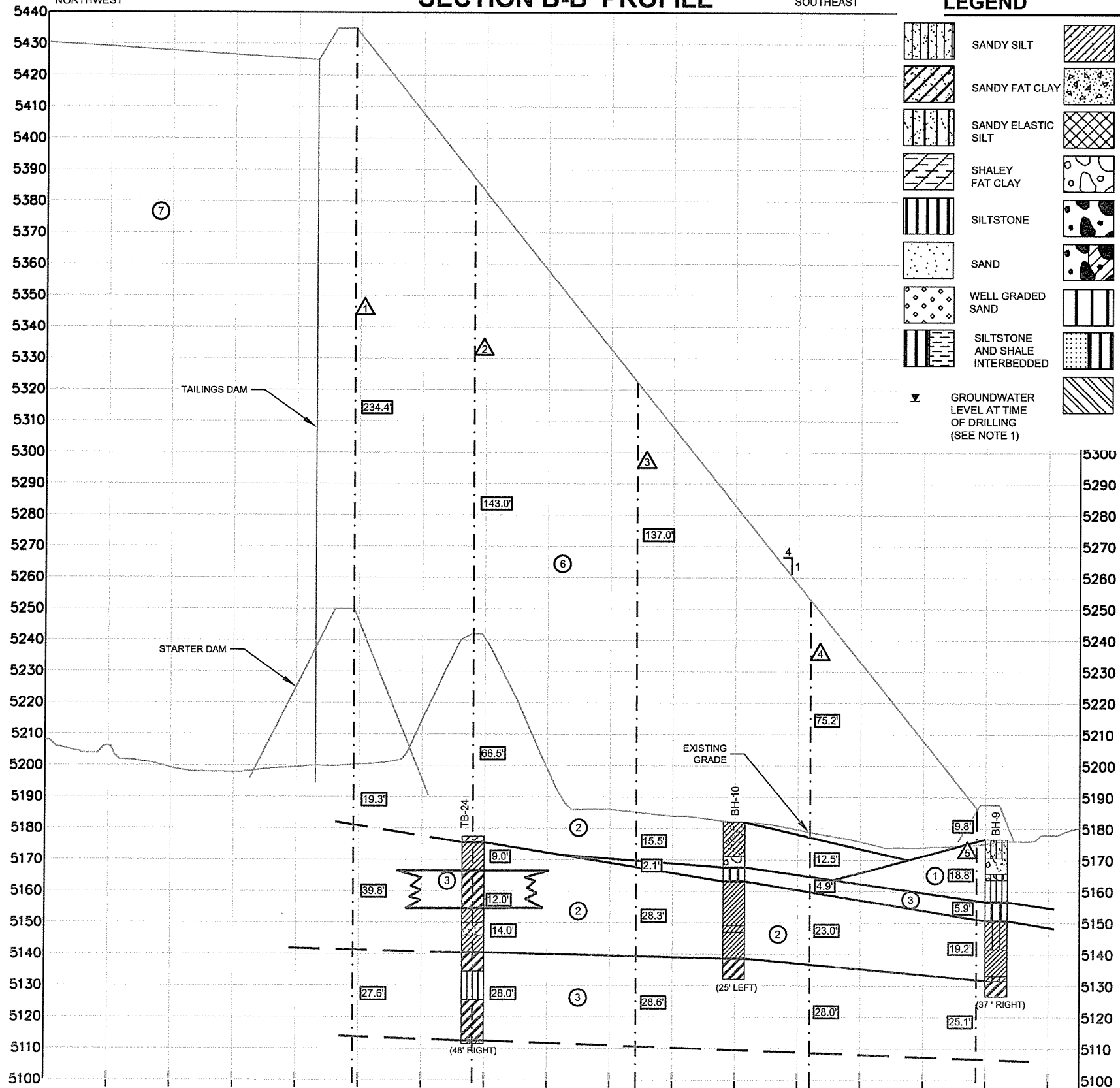
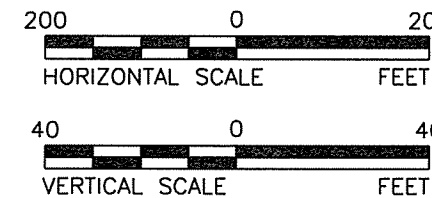
	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		USCS CL-CH		POORLY GRADED SAND WITH SILT

## STRATUM DESCRIPTORS

①	ML, CL-ML, SM
②	CL
③	CH, MH
④	GRAVEL, SAND
⑤	BASALT, CALICHE
⑥	EMBANKMENT FILL
⑦	TAILINGS (CONSOLIDATED)
△	ANALYSIS SECTION
12.0'	STRATUM/FILL THICKNESS

## NOTES

- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.

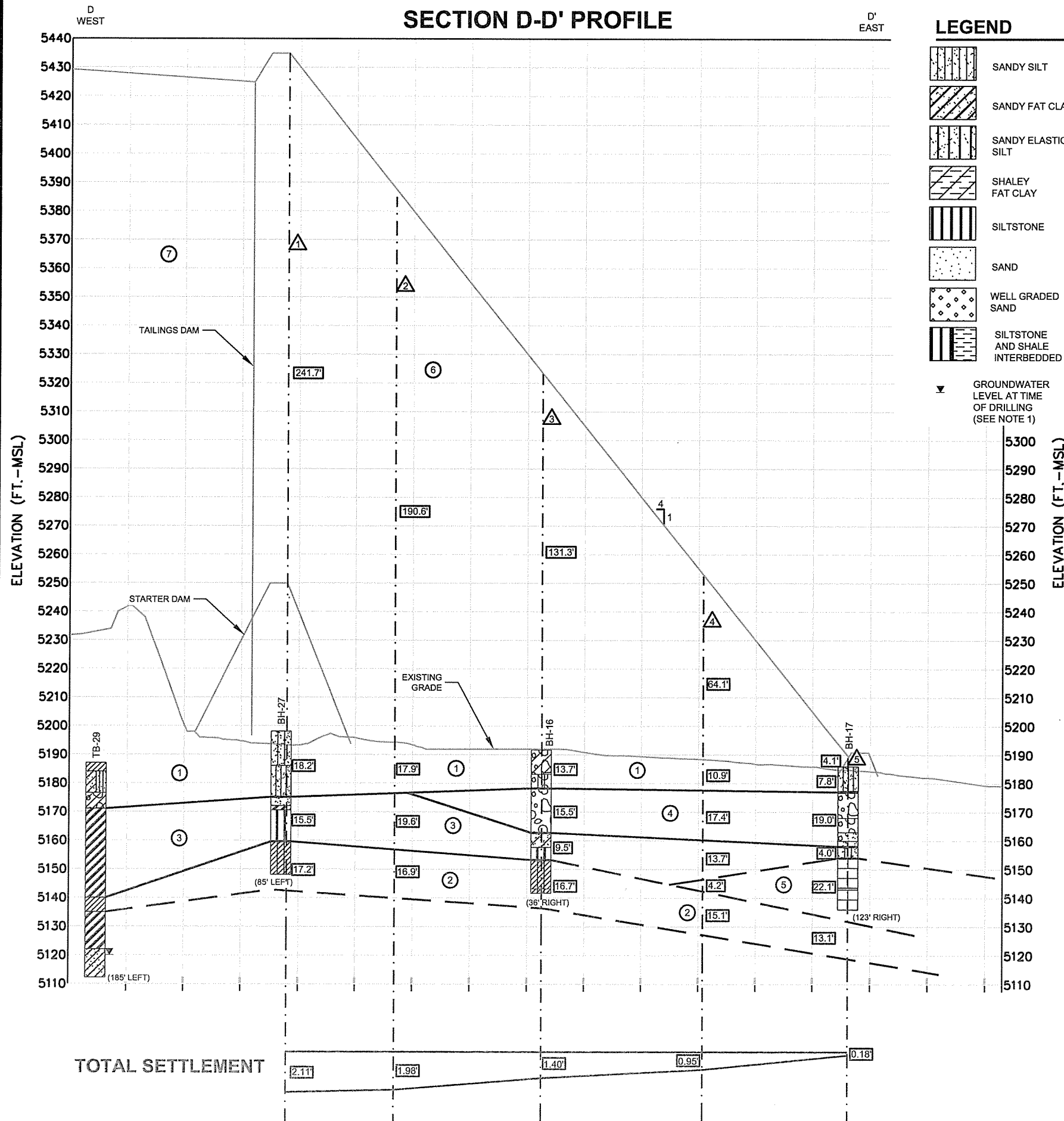


DRAFT

PROJECT	COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO			
TITLE	GEOLOGIC CROSS SECTION B-B'			
PROJECT No.	10392557	FILE No.	10392557A003	
DESIGN	CMT	2/21/13	SCALE	AS SHOWN
CADD	CMT	2/21/13	FIGURE	
CHECK	DEP	2/21/13		
REVIEW	GM	2/21/13		



# SECTION D-D' PROFILE



## LEGEND

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	CALICHE		POORLY GRADED SAND WITH SILT		

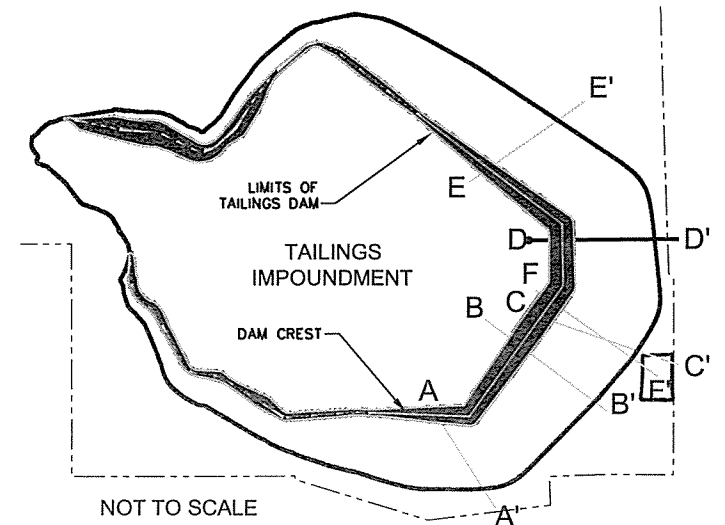
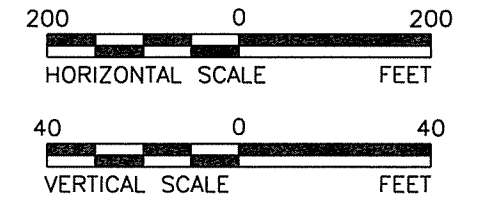
▽ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

## STRATUM DESCRIPTORS

①	ML, CL-ML, SM
②	CL
③	CH, MH
④	GRAVEL, SAND
⑤	BASALT, CALICHE
⑥	EMBANKMENT FILL
⑦	TAILINGS (CONSOLIDATED)
△	ANALYSIS SECTION
12.0'	STRATUM/FILL THICKNESS

## NOTES

- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.



**DRAFT**

PROJECT				COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO			
TITLE				GEOLOGIC CROSS SECTION D-D'			
PROJECT No.		10392557		FILE No.		10392557A005	
DESIGN	CMT	2/21/13	SCALE	AS SHOWN			
CADD	CMT	2/21/13	FIGURE	7			
CHECK	DEP	2/21/13					
REVIEW	GM	2/21/13					



**APPENDIX I.2  
SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS**

**REVISED JUNE 2016  
REVISED NOVEMBER 2016**

Made By: JL  
 Checked by: GM  
 Reviewed by: MP  
 Revised by: TS

## SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS

### 1.0 OBJECTIVE

Estimate the tensile strain caused by differential settlement of the in-situ subsurface materials inferred below the proposed Copper Flat tailing facility.

### 2.0 METHODOLOGY

The proposed geomembrane liner system may experience tensile strain because of differential settlement caused from the loading (tailings and embankment) of the subsurface soils.

#### 2.1 Settlement Analysis

Settlement was calculated using the finite element software SigmaW from the 2012 GeoStudio package. Cross sections A and B (both shown in plan view on Figure 1 and in profile view in Figure 2) showing the proposed tailing facility and tailings embankment layout/dimensions, inferred subsurface soils and boundaries were imported into the software for analyses. Geotechnical properties for each subsurface material layer were selected from previous reports (Refs. 1 and 2) and from experience with similar soils. The geotechnical properties were incorporated into the software and used for the settlement analyses. Figure 3 provides information on the geologic units and geologic structures (Refs. 4, 5 and 6) associated with cross sections A and B along with the the proposed tailing facility and tailings embankment layout/dimensions. Table 1 below provides a list of the geotechnical subsurface material layers and properties.

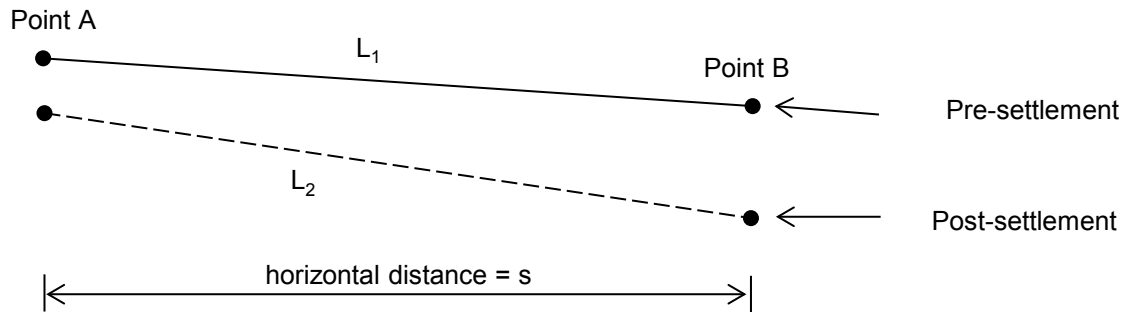
**Table 1: Geotechnical Subsurface Material Layers and Properties**

Material No.	Material Name	Geotechnical Properties		
		Unit Weight (lb/ft <sup>3</sup> )	Poisson's Ratio (-)	Effective Modulus (lb/ft <sup>2</sup> )
0	Tailings/Embankment	97	0.49	10,000,000
1	Well-Graded Gravel	110	0.30	4,000,000
2	Well-Graded Sand with Silt and Gravel	110	0.30	4,000,000
3	Conglomerate	130	0.30	5,000,000
4	Basalt	160	0.30	100,000,000
5	Lean Clay, Fat Clay, Silty Clay	104	0.30	790,600
6	Silt	106	0.30	671,400
7	Caliche	125	0.30	100,000,000
8	Bedrock	175	0.30	100,000,000

## 2.1 Tensile Strain from Differential Settlement

Settlement results from the SigmaW runs were used to calculate the induced strain in the geomembrane liner system along Cross Section A and B shown in Figure 1 and Figure 2.

The tensile strain of a base liner system caused by differential settlement can be estimated by the following equation:



**Illustration: Liner Differential Settlement**

$$\varepsilon = \frac{L_2 - L_1}{L_1}$$

$$L = \sqrt{(Elev.A - Elev.B)^2 + s^2}$$

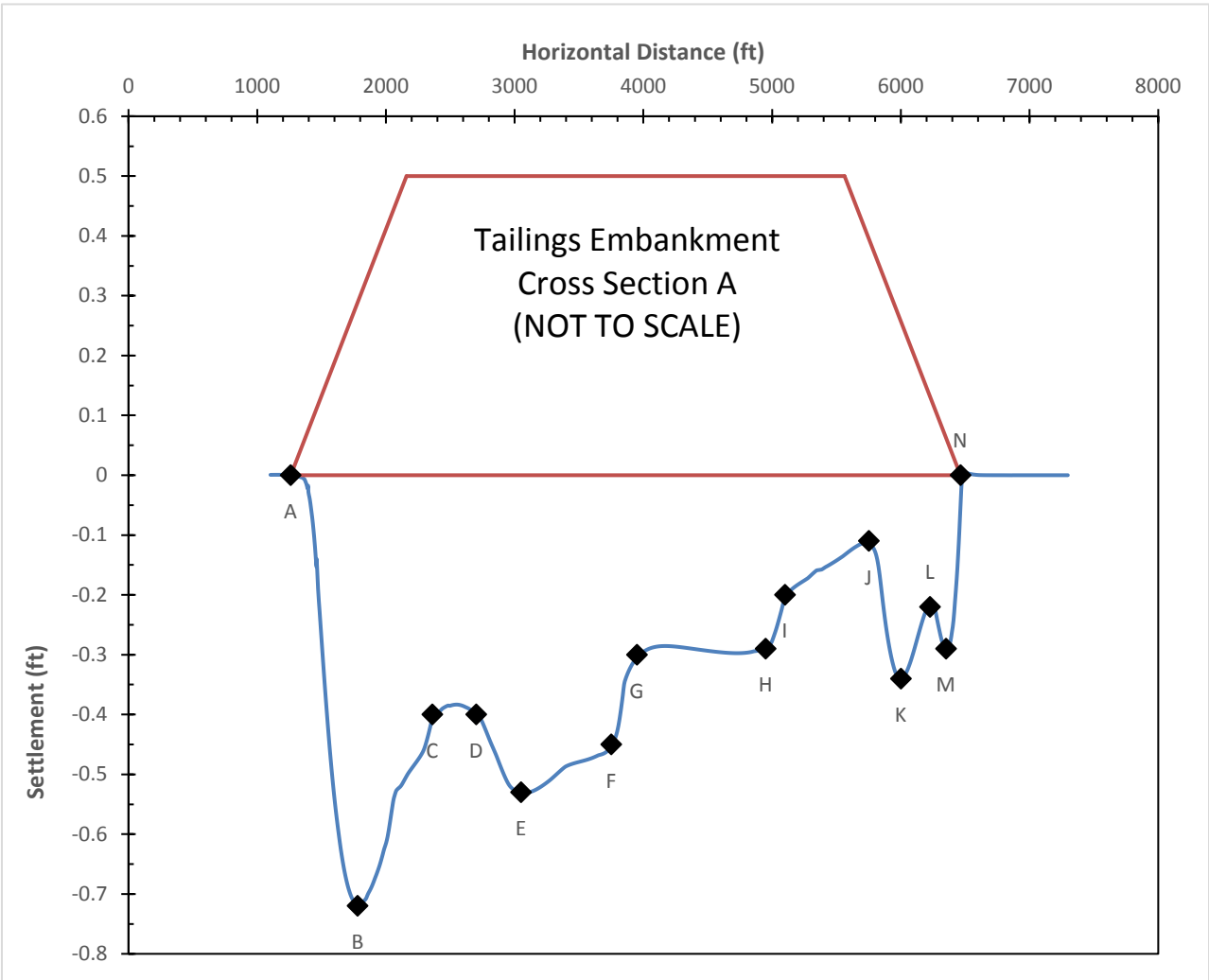
Where:

- $\varepsilon$  = Tensile strain in liner system between Points A and B
- $L_1$  = Distance between Points A and B, pre-settlement
- $L_2$  = Distance between Points A and B, post-settlement
- $s$  = Horizontal distance between Points A and B

## 3.0 CALCULATIONS AND RESULTS

### 3.1 Tensile Strain from Differential Settlement

The settlement results for Cross Section A and Cross Section B are illustrated below. Points for liner strain evaluation were selected at locations where peaks or valleys were observed in the results. The liner strain evaluations due to differential settlement of the subsurface materials are summarized in Table 2 and Table 3.



**Illustration: Settlement Profile - Cross Section A**  
(refer to Figure 2 for location along horizontal distance)

**Table 2: Liner Integrity Analysis Results - Cross Section A**

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L <sub>1</sub> ), feet	Post-settlement Dist. (L <sub>2</sub> ), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5383.4	5383.4	0.00	520.00	524.920	525.019	0.0188%
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
C	5275.5	5275.1	0.40	350.00	350.352	350.358	0.0017%
D	5267.5	5267.1	0.40	350.00	350.352	350.358	0.0017%
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
G	5230.2	5229.9	0.30	150.00	155.850	155.826	Under Compression
H	5239.6	5239.3	0.29	150.00	155.850	155.826	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
N	5168.5	5168.5	0.00				
Maximum Tensile Strain due to Differential Settlement =							0.0188%



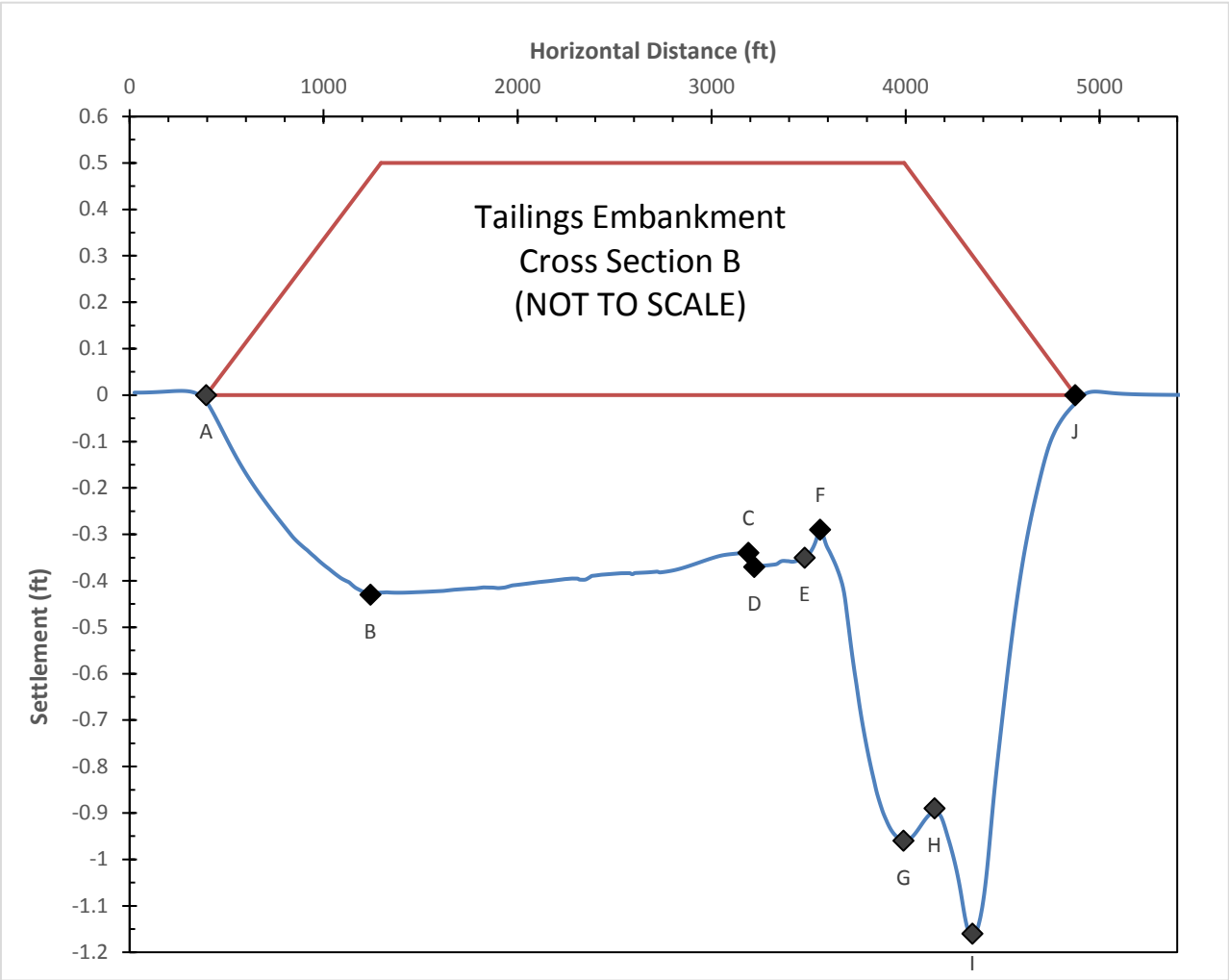


Illustration: Settlement Profile - Cross Section B  
(refer to Figure 2 for location along horizontal distance)

**Table 3: Liner Integrity Analysis Results - Cross Section B**

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L <sub>1</sub> ), feet	Post-settlement Dist. (L <sub>2</sub> ), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5280.2	5280.2	0.00	847.00	848.707	848.734	0.0032%
B	5226.4	5226.0	0.43				
C	5213.7	5213.4	0.34	30.00	34.000	33.986	Under Compression
D	5229.7	5229.3	0.37				
E	5239.3	5239.0	0.35	79.00	83.716	83.696	Under Compression
F	5211.6	5211.3	0.29				
F	52116.0	52115.7	0.29	430.00	46921.770	46922.440	0.0014%
G	5196.2	5195.2	0.96				
G	5196.2	5195.2	0.96	160.00	160.004	160.004	0.0003%
H	5197.3	5196.4	0.89				
H	5197.3	5196.4	0.89	195.00	195.004	195.003	Under Compression
I	5198.6	5197.4	1.16				
I	5198.6	5197.4	1.16	530.00	530.000	530.003	0.0005%
J	5199.3	5199.3	0.00				
Maximum Tensile Strain due to Differential Settlement =							0.0032%

#### 4.0 DISCUSSION AND CONCLUSIONS

It is understood that the liner system will consist of HDPE 80 mil geomembrane liner between a liner bedding fill layer and tailings. The minimum allowable tensile strain for geomembrane is 10% (Refs. 3). Based on the analysis performed herein and available information at the time of this calculation, the estimated tensile strain along Cross Section A and Cross Section B are less than the allowable tensile strain. The allowable strain is presented in Table 4.

**Table 4: Summary of Allowable Liner Strains**

Cross Section	Max. Tensile Strain from Differential Settlement	Liner Component	Allowable Tensile Strain	Tensile Strain less than Allowable?
A	0.0188%	Geomembrane	10%	Yes
B	0.0032%			Yes

The potential strain of the geomembrane liner system was analyzed for overall differential settlement along two cross sections (Cross Section A and B) within the proposed Copper Flat tailing facility. Based on the available information, experience with similar subsurface materials and conservative assumptions, the maximum liner strain is estimated to be 0.02%, from differential settlement which is less than the allowable strain for geomembrane liners.

## 5.0 REFERENCES

1. - Golder 2013, Feasibility Study Copper Flat Project, Sierra County, New Mexico, Volume 1 - Tailings Storage Facility, report dated July 2013, Golder Project No. 103-92557.011
2. - SHB (Sergent, Hauskins and Beckwith), 1980. Tailings Dam and Disposal Area - Quintana Minerals Corporation - Copper Flats Project - Golddust, New Mexico. October 14, 1980
3. - Robert M. Koerner (2005) Designing with Geosynthetics, Fifth Edition, Pearson/Prentice Hall.
4. - A.J Jochems and Others (2014) DRAFT Geologic Map of the Hisslboro 7.5-Minute Quadrangle, Sierra County, New Mexico: New Mexico Bureau of Geology and Mineral Resources pen-File Geologic Map 242
5. - D.J. Koenig and Others (2015) DRAFT Geologic Map of the Skute Stone Arroyo 7.5-Minute Quadrangle, Sierra County, New Mexico: New Mexico Bureau of Geology and Mineral Resources pen-File Geologic Map 252
6. - JSAI (John Shomaker and Associates, Inc.) 2014. Figure 11K-2b of Copper Flat Mine Discharge Permit Application, dated December 2015.

### **GeoStudio File Names**

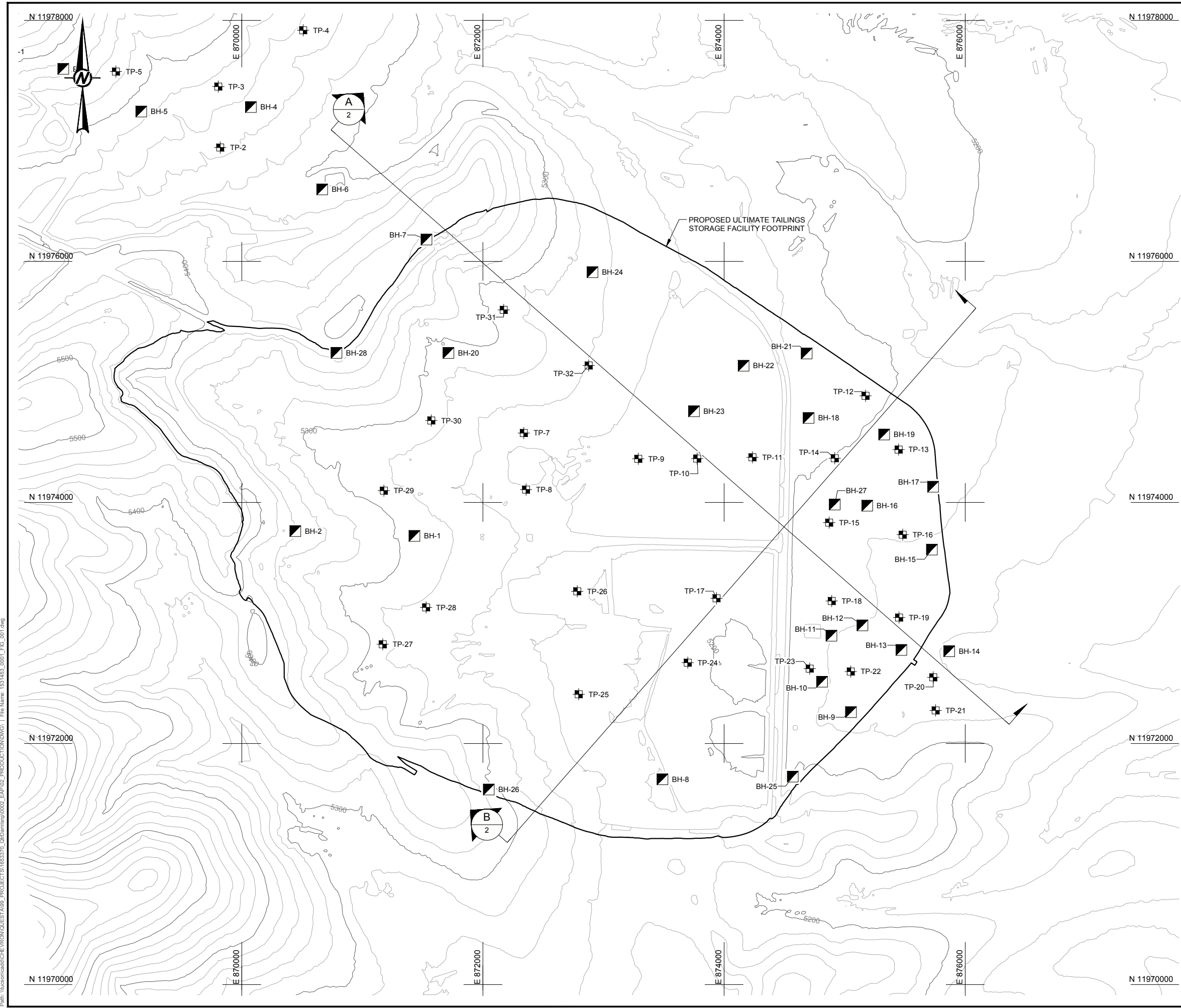
Full Cross Section A.gsz  
Full Cross Section B.gsz

### **Description**

Cross Section A Settlement Analysis  
Cross Section B Settlement Analysis

### **Attachments**

Figure 1: Cross Section Location Plan  
Figure 2: Geologic Cross Sections  
Figure 3: Geologic Cross-Sections Near Tailings Storage Facility



**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- BH-11 HOLLOW STEM AUGER (HSA) BOREHOLE
- TP-8 TEST PIT
- CROSS-SECTION CALLOUT SECTION ID DRAWING SHEET LOCATION



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PROJECT  
**COPPER FLAT**

SIERRA COUNTY, NEW MEXICO

TITLE  
**CROSS-SECTION LOCATION PLAN**

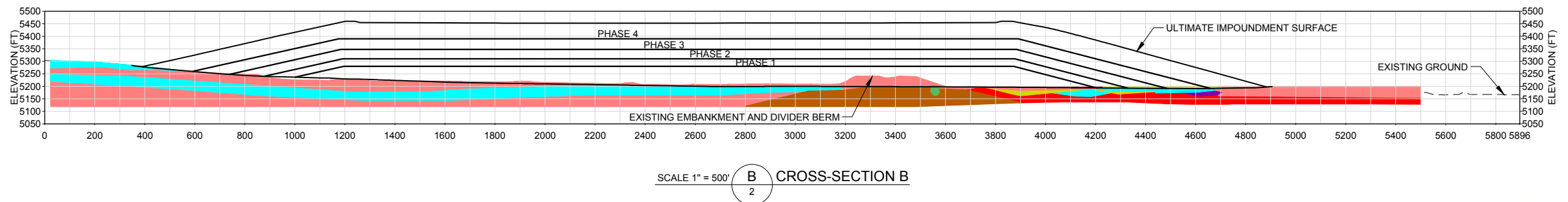
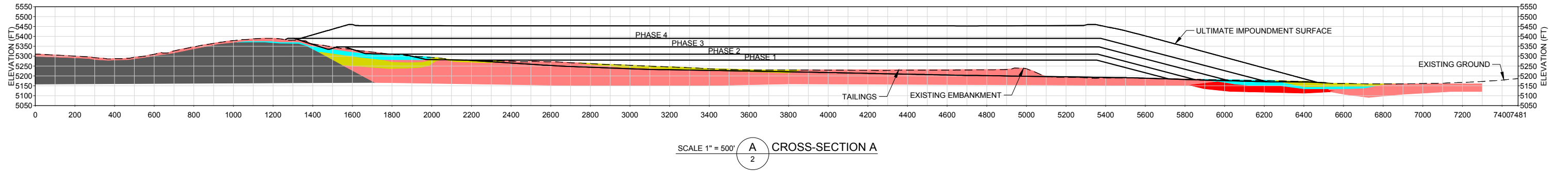
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	DESIGNED	JS
	PREPARED	JHR
	REVIEWED	JL
	APPROVED	GM

PROJECT NO. 1531453 CONTROL 0001 REV. A FIGURE 16716

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1 in. IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B

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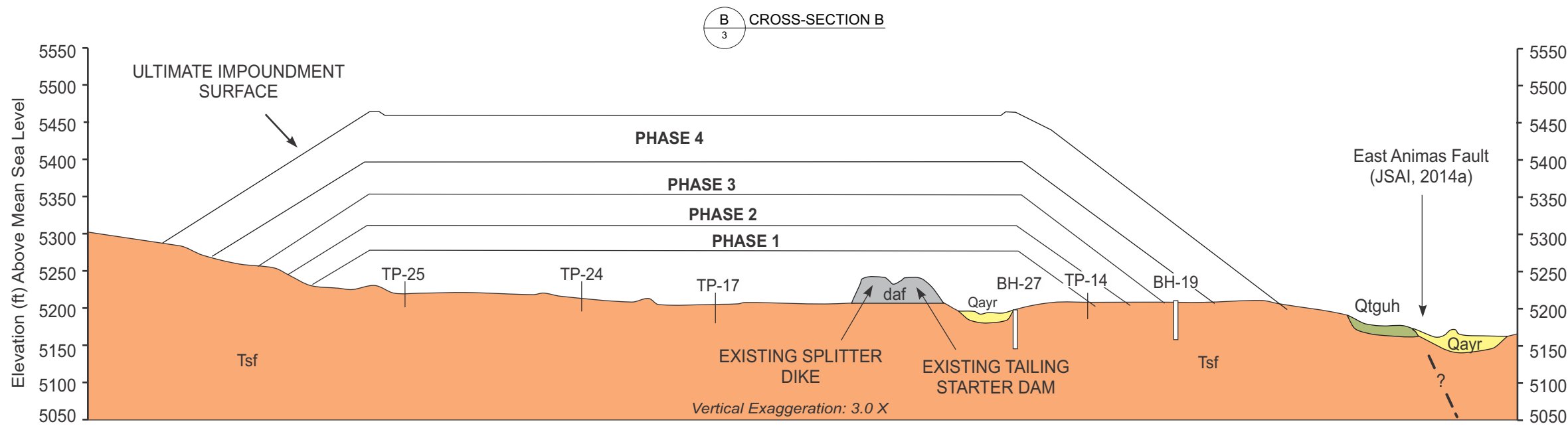
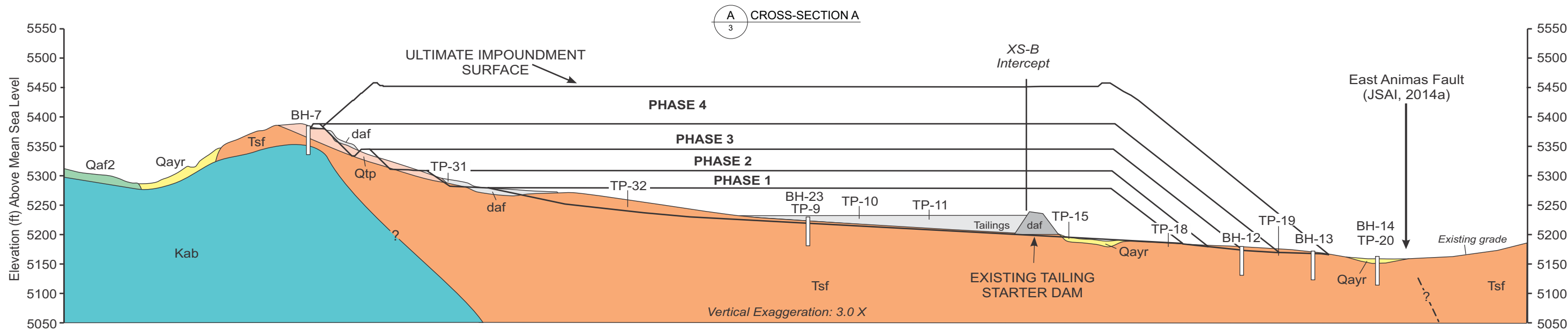
**LEGEND**

	WELL-GRADED GRAVEL
	WELL-GRADED SAND WITH SILT AND GRAVEL
	CONGLOMERATE
	BASALT
	LEAN CLAY, FAT CLAY, SILTY CLAY
	SILT
	CALICHE
	BEDROCK

CLIENT	
<b>THEMAC</b> RESOURCES	NEW MEXICO COPPER CORPORATION
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PROJECT COPPER FLAT	
SIERRA COUNTY, NEW MEXICO	
TITLE <b>GEOLOGIC CROSS-SECTIONS</b>	
CONSULTANT	YYYY-MM-DD 2016-05-11
	DESIGNED JS
	PREPARED JHR
	REVIEWED JL
	APPROVED GM

PROJECT NO. 1531453	CONTROL 0001	REV. A	FIGURE A
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1 in. IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B



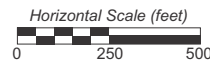
LEGEND

Description of Geologic Units

<p><b>daf</b> Historic mining-related disturbed materials, including borrow materials, dredge materials, excavations and fill</p> <p><b>Qayr</b> Young and recent (modern and historic) alluvium, undivided (present to middle Holocene)</p> <p><b>Qtguh</b> High level terrace deposit associated with smaller canyons, undivided</p> <p><b>Qaf2</b> Alluvial fans graded to the level of level of Qao2 stream terraces (middle Pleistocene?)</p> <p><b>Qtp</b> Palomas Formation of the Santa Fe Group (lower Pliocene to mid Pleistocene), differentiated from underlying Santa Fe Gp. strata by more cobbles, boulders</p>	<p><b>Tsf</b> Santa Fe Group predating the Palomas Fm. (Miocene to Pliocene). Pebbly sandstone and conglomerate</p> <p><b>Kab</b> Laharic flow breccia of Copper Flat</p> <p><b>?</b> Inferred fault location</p>
--	---

Sources of data for surficial geologic units:

- (1) New Mexico Bureau of Geology and Mineral Resources  
\*DRAFT Open-File Geologic Map 242 (A.P. Jochems and Others, 2014)  
\*DRAFT Open-File Geologic Map 252 (D.J. Koning and Others, 2015)  
\*has not been field verified and may not depict site-specific geology with accuracy
- (2) Updated geologic data from existing test pit and borehole logs
- (3) Regional surface geology and location of East Animas Fault presented on Figure 11K-2b of discharge permit application package, taken from JSAI, 2014a



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PROJECT  
**COPPER FLAT**

SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTIONS NEAR TAILINGS STORAGE FACILITY**

CONSULTANT	YYYY-MM-DD	2016-11-2
DESIGNED	LCK	
PREPARED	LCK	
REVIEWED	TS	
APPROVED		

PROJECT NO. 1531453 CONTROL 0001 REV. FIGURE 3

**APPENDIX J  
DRAWINGS**



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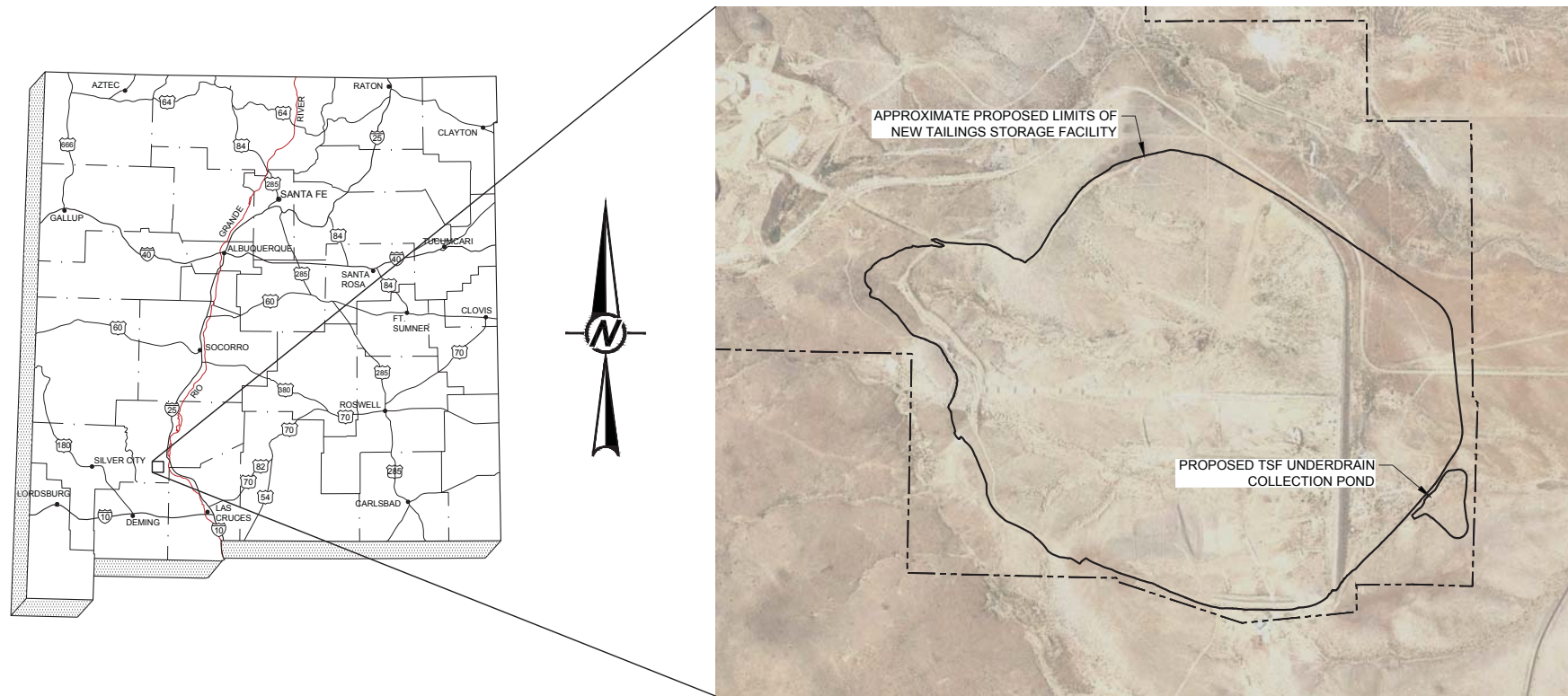
# COPPER FLAT PROJECT

## 30K TPD TAILINGS STORAGE FACILITY

### FEASIBILITY DESIGN

#### SIERRA COUNTY, NEW MEXICO

#### NOVEMBER 2015



STATE OF NEW MEXICO  
NOT TO SCALE

LOCATION MAP  
NOT TO SCALE

#### DRAWING LIST

No.	DRAWING TITLE
1	TITLE SHEET AND LOCATION MAP
2	EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN
3	GEOTECHNICAL TEST AND GEOLOGIC CROSS-SECTION LOCATIONS
4	GEOLOGIC CROSS-SECTION A-A'
5	GEOLOGIC CROSS-SECTION B-B'
6	GEOLOGIC CROSS-SECTION C-C'
7	GEOLOGIC CROSS-SECTION D-D'
8	GEOLOGIC CROSS-SECTION E-E'
9	GEOLOGIC CROSS-SECTION F-F'
10	TAILINGS STORAGE FACILITY GRADING PLAN
11	TAILINGS STORAGE FACILITY PHASE 1 PLAN
12	TAILINGS STORAGE FACILITY AT FINAL BUILDOUT
13	TAILINGS STORAGE FACILITY CROSS-SECTIONS
14	TAILINGS STORAGE FACILITY DETAILS
15	DAM AND IMPOUNDMENT UNDERDRAIN PLAN
16	DAM UNDERDRAIN DETAILS (1 OF 2)
17	DAM UNDERDRAIN DETAILS (2 OF 2)
18	TSF UNDERDRAIN COLLECTION POND PLAN
19	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)
20	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(2 OF 2)
21	CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN
22	SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS
23	GENERAL PROCESS FLOW DIAGRAM
24	TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN
25	TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)
26	TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2)
27	WATER RECLAIM SYSTEM PIPING PLAN
28	WATER RECLAIM SYSTEM DETAILS
29	TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS

#### GENERAL REFERENCES

- 2-FOOT TOPOGRAPHY DEVELOPED BY COOPER AERIAL SURVEY CO. BASED ON JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
- COORDINATE SYSTEM IS IN UTM ZONE 13 NAD83 U.S. FOOT.

#### NOTES

- DRAWINGS TO BE READ IN CONJUNCTION WITH FEASIBILITY LEVEL DESIGN, 30,000 TPD TAILINGS STORAGE FACILITY, SIERRA COUNTY, NEW MEXICO.

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

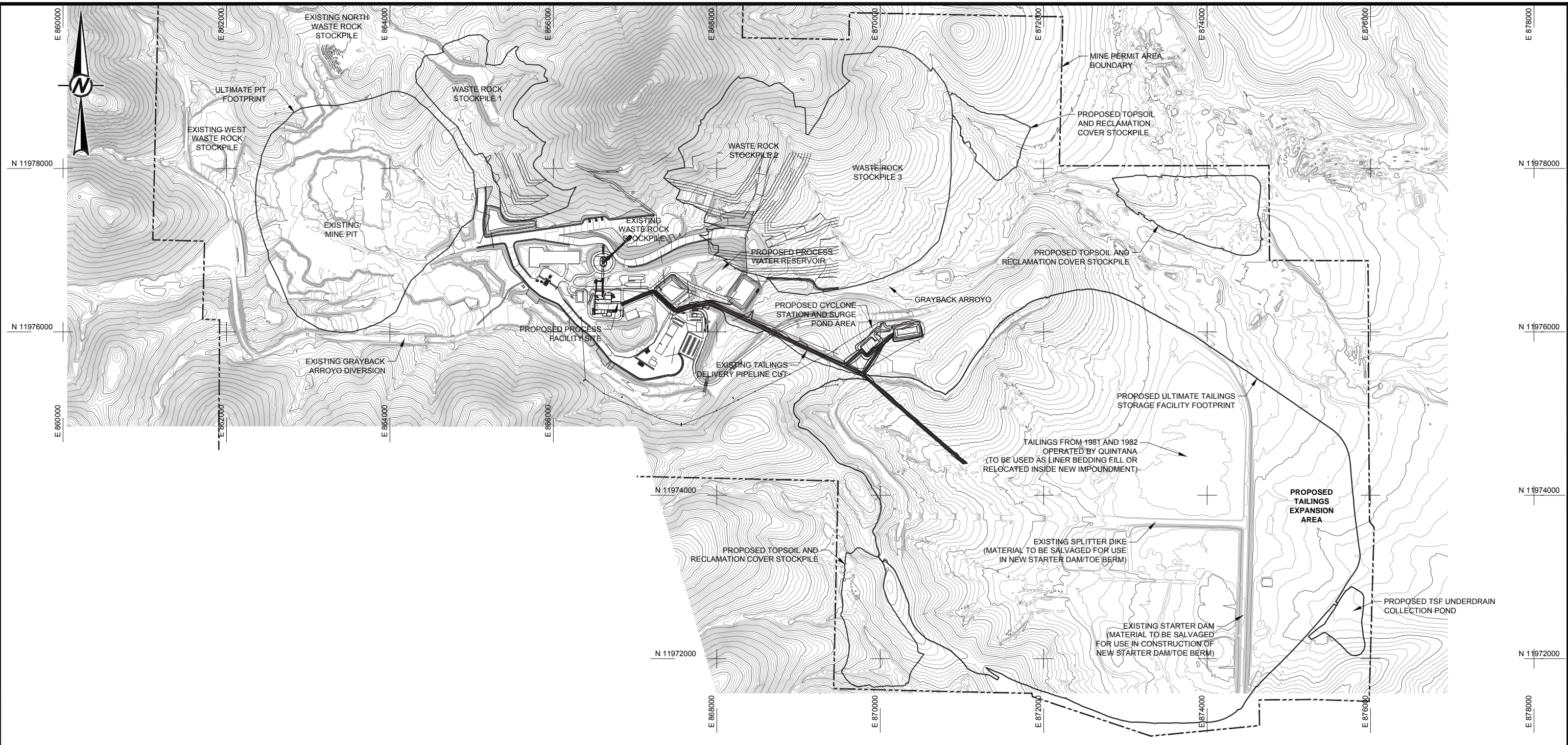
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PROJECT No. 103-92557		FILE No. 10392557K001		
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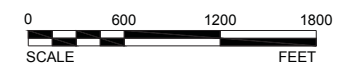


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**NOTES**

1. EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
2. TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.



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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
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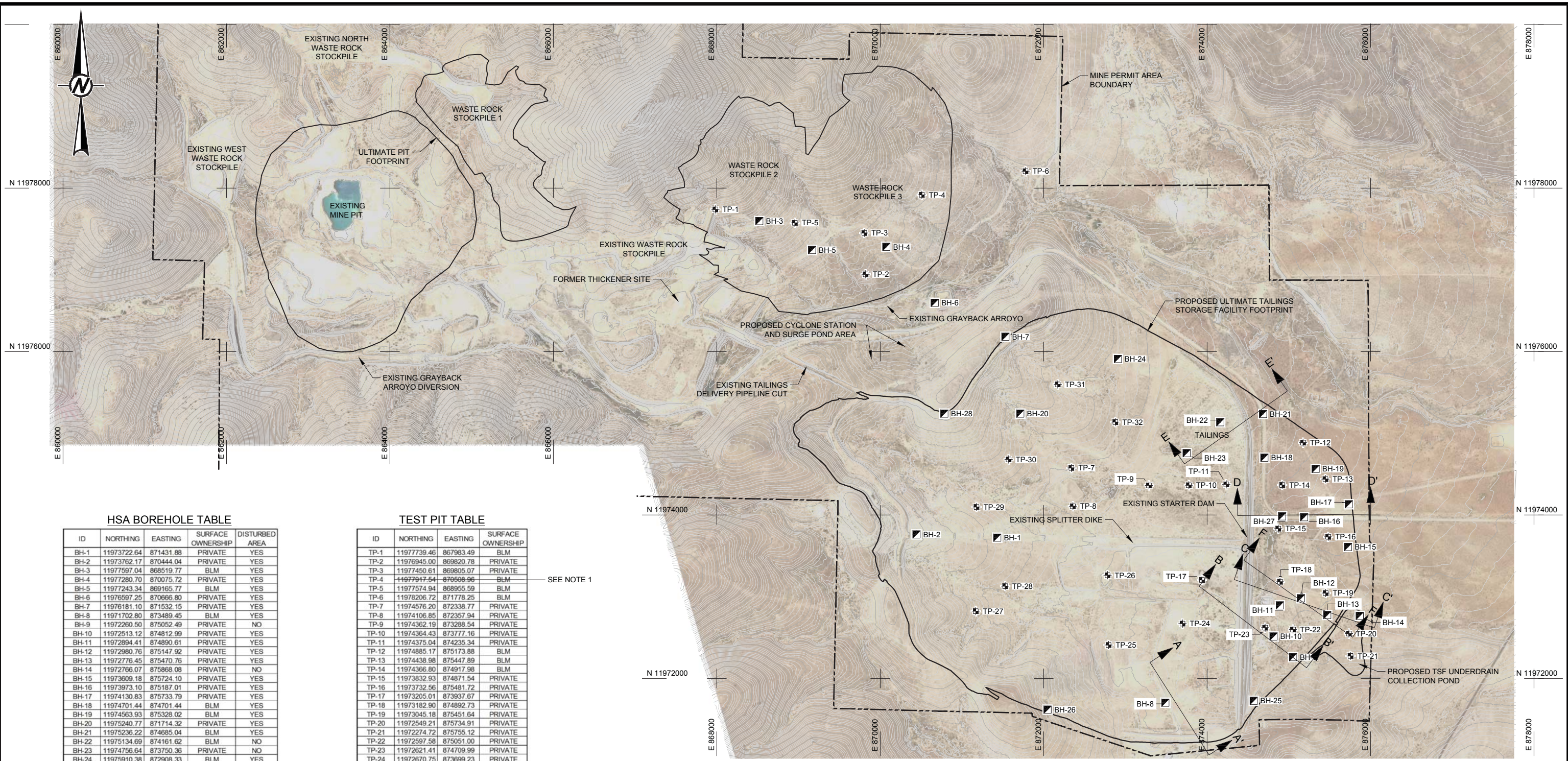
PROJECT: **COPPER FLAT PROJECT**  
**THEMAC** RESOURCES | NEW MEXICO COPPER CORPORATION  
 Environmentally Responsible. Community Minded. Local Opportunities.  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

**EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN**

PROJECT No.	103-92557	FILE No.	10392557K002
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**HSA BOREHOLE TABLE**

ID	NORTHING	EASTING	SURFACE OWNERSHIP	DISTURBED AREA
BH-1	11973722.64	871431.88	PRIVATE	YES
BH-2	11973762.17	870444.04	PRIVATE	YES
BH-3	11977597.04	868519.77	BLM	YES
BH-4	11977280.70	870075.72	PRIVATE	YES
BH-5	11977243.34	869165.77	BLM	YES
BH-6	11976597.25	870666.80	PRIVATE	YES
BH-7	11976181.10	871532.15	PRIVATE	YES
BH-8	11971702.80	873489.45	BLM	YES
BH-9	11972260.50	875052.49	PRIVATE	NO
BH-10	11972513.12	874812.99	PRIVATE	YES
BH-11	11972894.41	874890.61	PRIVATE	YES
BH-12	11972980.76	875147.92	PRIVATE	YES
BH-13	11972776.45	875470.76	PRIVATE	YES
BH-14	11972766.07	875868.08	PRIVATE	YES
BH-15	11973609.18	875724.10	PRIVATE	YES
BH-16	11973973.10	875187.01	PRIVATE	YES
BH-17	11974130.83	875733.79	PRIVATE	YES
BH-18	11974701.44	874701.44	BLM	YES
BH-19	11974563.93	875328.02	BLM	YES
BH-20	11975240.77	871714.32	PRIVATE	YES
BH-21	11975236.22	874685.04	BLM	YES
BH-22	11975134.69	874161.62	BLM	NO
BH-23	11974756.64	873750.36	PRIVATE	NO
BH-24	11975910.38	872908.33	BLM	YES
BH-25	11971725.72	874570.21	BLM	YES
BH-26	11971618.08	872048.45	BLM	YES
BH-27	11973982.94	874917.98	PRIVATE	YES
BH-28	11975241.04	870785.44	PRIVATE	YES

**TEST PIT TABLE**

ID	NORTHING	EASTING	SURFACE OWNERSHIP
TP-1	11977739.46	867983.49	BLM
TP-2	11976945.00	869820.78	PRIVATE
TP-3	11977450.61	869805.07	PRIVATE
TP-4	11977917.64	870569.96	BLM
TP-5	11977574.94	868955.59	BLM
TP-6	11978206.72	871778.25	BLM
TP-7	11974576.20	872338.77	PRIVATE
TP-8	11974106.85	872357.94	PRIVATE
TP-9	11974362.19	873288.54	PRIVATE
TP-10	11974364.43	873777.16	PRIVATE
TP-11	11974375.04	874235.34	PRIVATE
TP-12	11974885.17	875173.88	BLM
TP-13	11974438.98	875447.89	BLM
TP-14	11974366.80	874917.98	BLM
TP-15	11973832.93	874871.54	PRIVATE
TP-16	11973732.56	875481.72	PRIVATE
TP-17	11973205.01	873937.67	PRIVATE
TP-18	11973182.90	874892.73	PRIVATE
TP-19	11973045.18	875451.64	PRIVATE
TP-20	11972549.21	875734.91	PRIVATE
TP-21	11972274.72	875755.12	PRIVATE
TP-22	11972597.58	875051.00	PRIVATE
TP-23	11972621.41	874709.99	PRIVATE
TP-24	11972670.75	873699.23	PRIVATE
TP-25	11972408.78	872794.31	PRIVATE
TP-26	11973262.64	872782.60	PRIVATE
TP-27	11972823.18	871169.18	PRIVATE
TP-28	11973129.48	871528.89	PRIVATE
TP-29	11974098.42	871178.07	PRIVATE
TP-30	11974680.06	871571.56	PRIVATE
TP-31	11975597.72	872172.62	BLM
TP-32	11975136.61	872876.80	BLM

SEE NOTE 1

**LEGEND**

- MINE PERMIT AREA BOUNDARY
- BH-1 HOLLOW STEM AUGER (HSA) BOREHOLE
- ⊕ TP-29 TEST PIT

**NOTES**

- TP-4 TEST PIT NOT EXCAVATED DUE TO LACK OF ACCESS.

**REFERENCES**

- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.

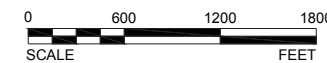
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
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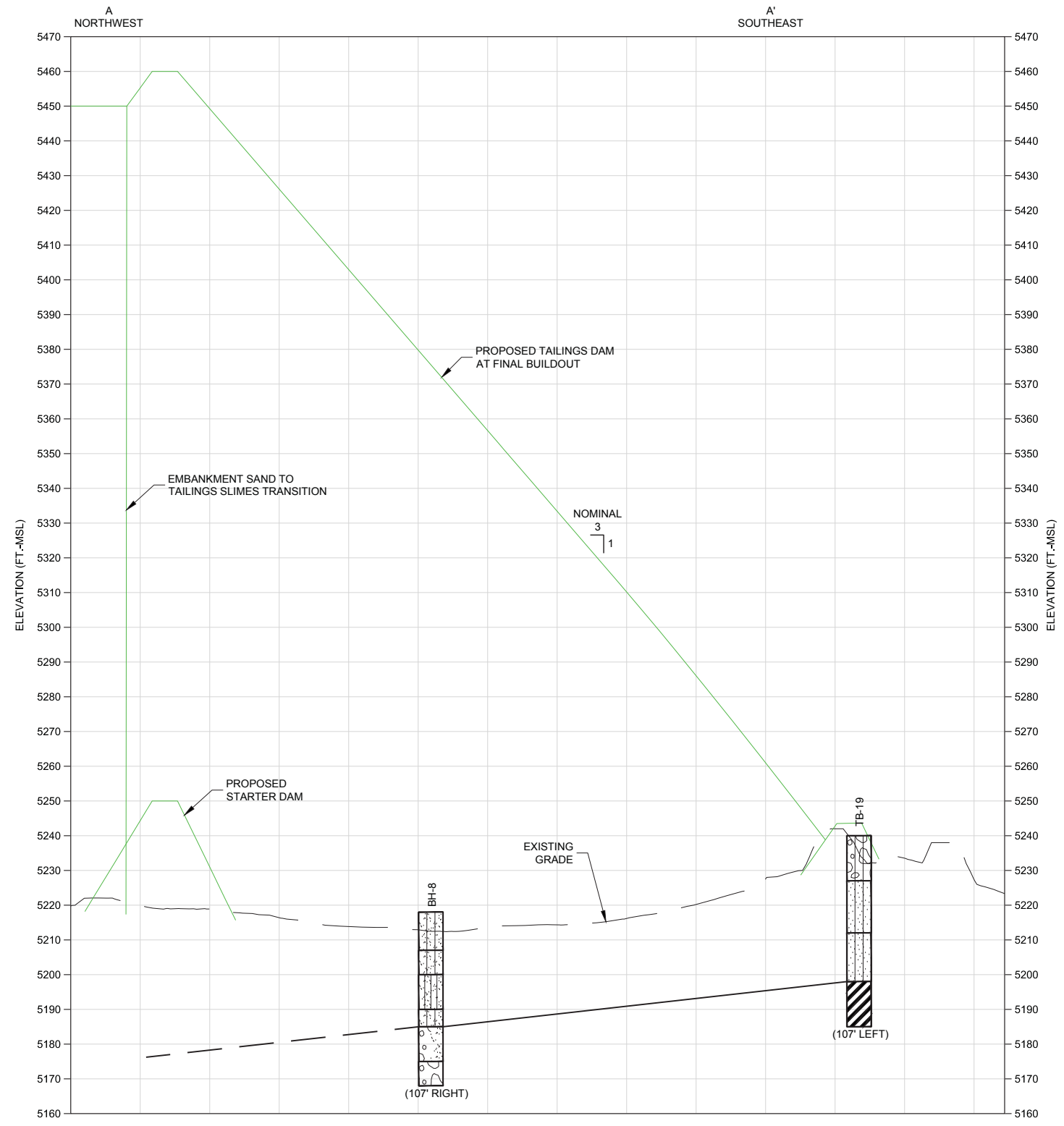
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**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557 FILE No. 10392557K003  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE			
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REVIEW	DAK	2013-07-17	



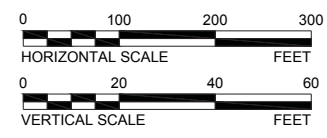
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- NOTES**
- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  - PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  - PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  - SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

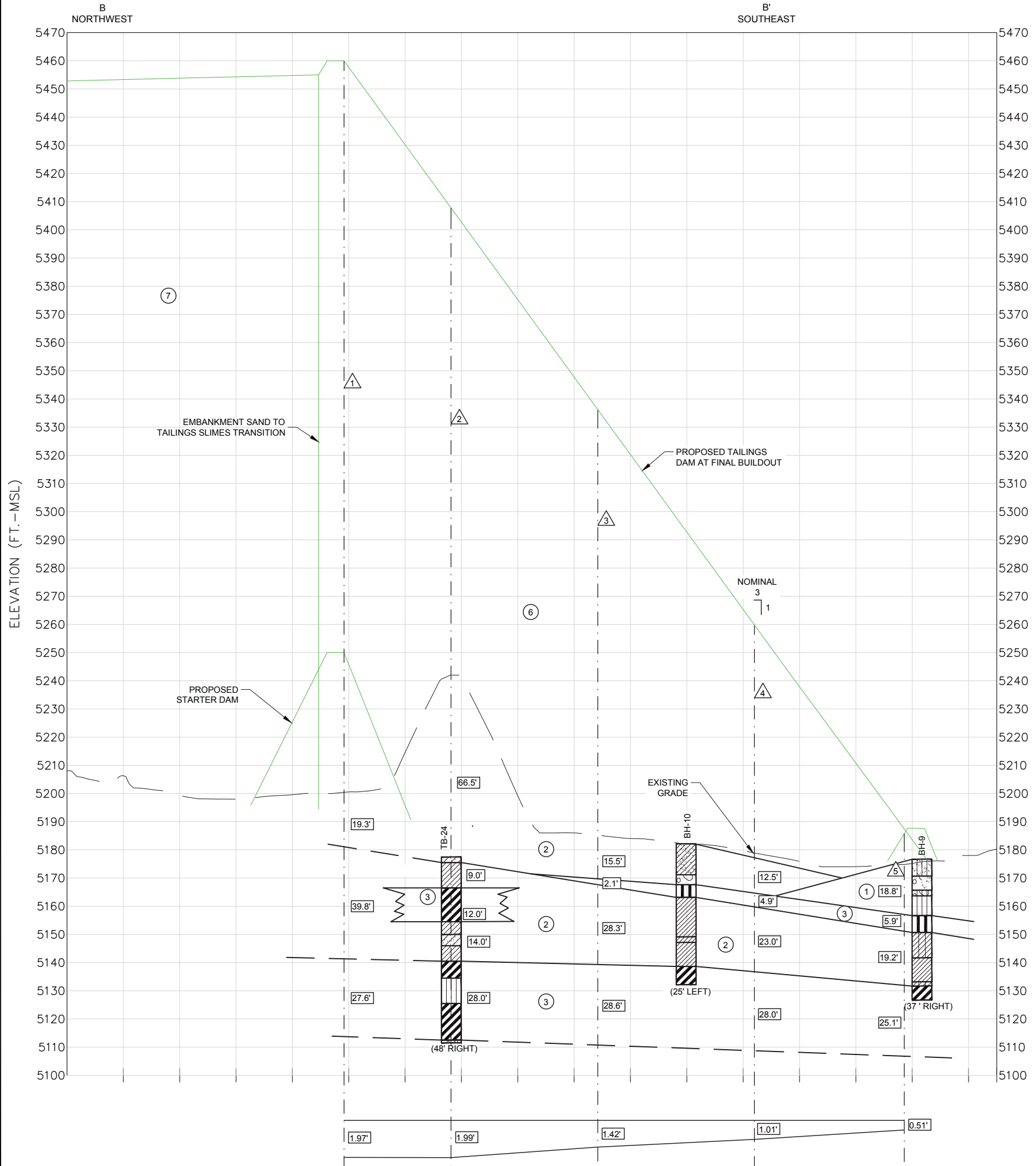
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	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
2013-11-15		ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE <b>PRELIMINARY</b> FOR AGENCY REVIEW	PROJECT <b>THEMAC</b> RESOURCES NEW MEXICO COPPER CORPORATION Environmentally Responsible. Community Minded. Local Opportunities.		COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO		
	TITLE <b>GEOLOGIC CROSS-SECTION A-A'</b>				
	PROJECT No.	103-92557	FILE No.	10392557K004	
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK REVIEW	GM DAK	2013-07-16 2013-07-17		

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**STRATUM DESCRIPTORS**

- ① ML, CL-ML, SM
- ② CL
- ③ CH, MH
- ④ GRAVEL, SAND
- ⑤ BASALT, CALICHE
- ⑥ EMBANKMENT FILL
- ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

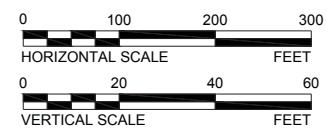
**NOTES**

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)  
 SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
1	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
3	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
4	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
5	2013-05-03	ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

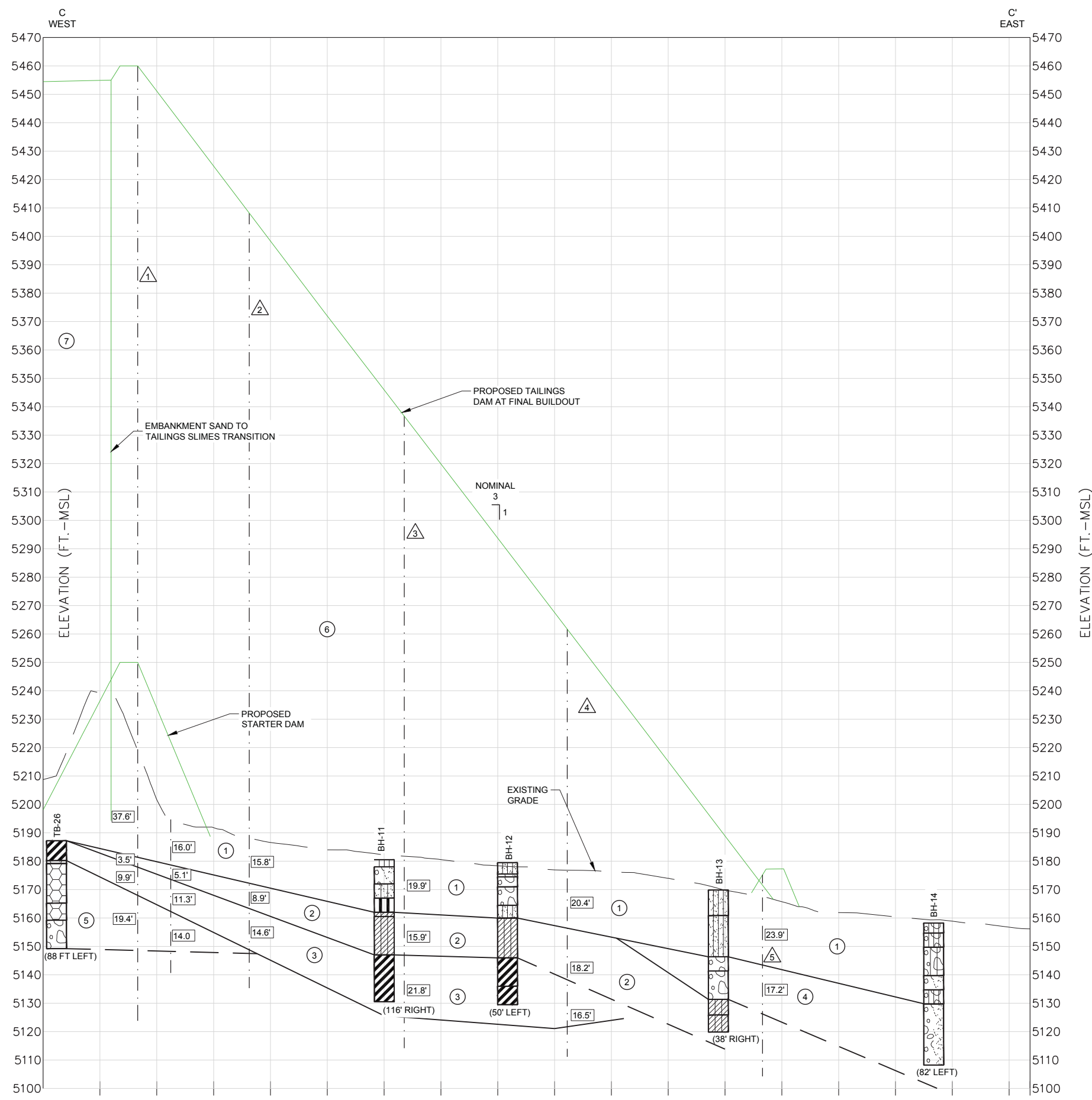
PROJECT: COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557		FILE No. 10392557K004	
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



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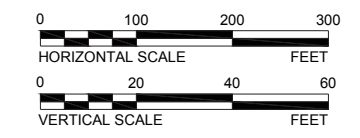
- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
  - 12.0' STRATUM THICKNESS

- NOTES**
1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)  
 SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

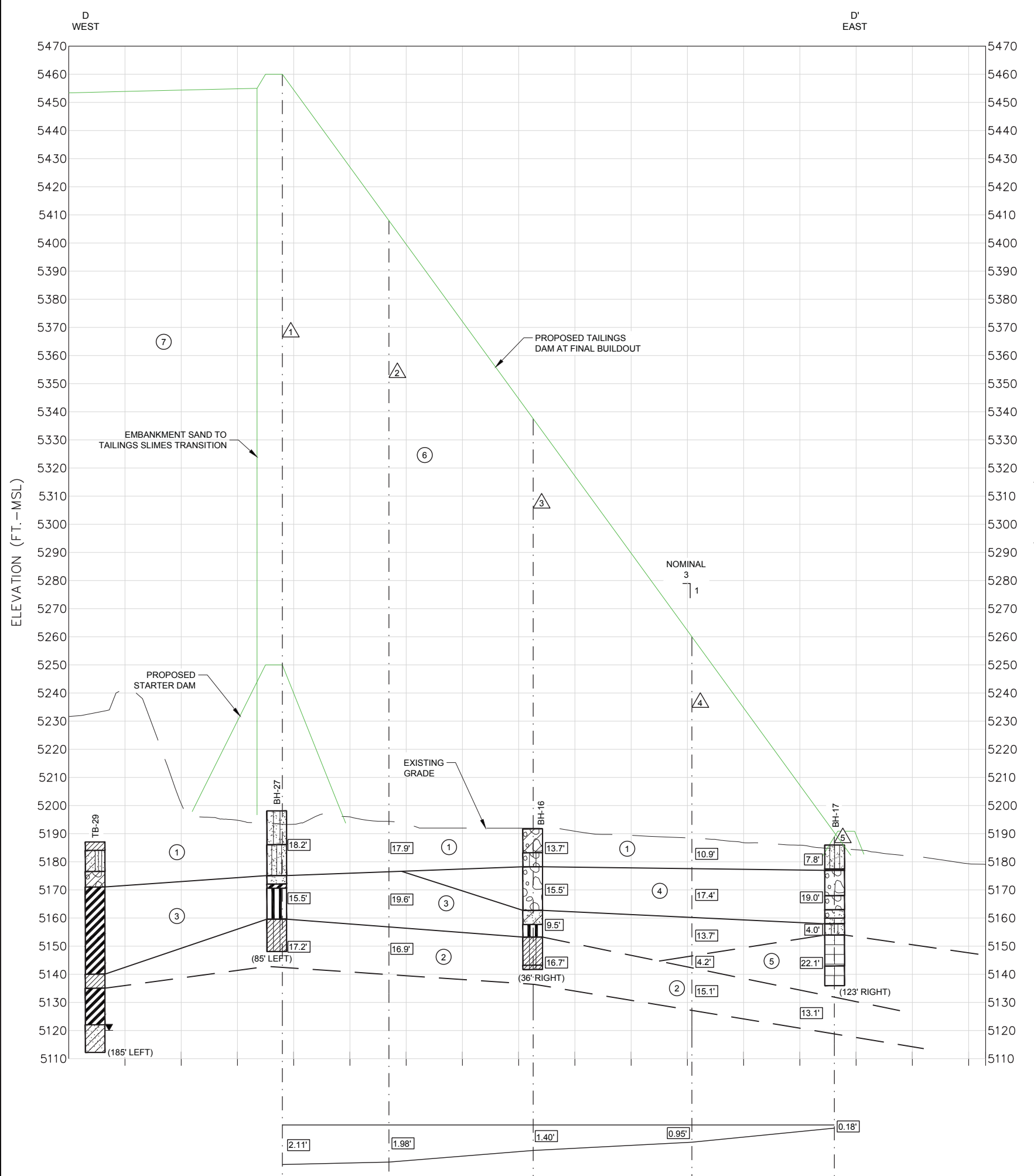
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**GEOLOGIC CROSS-SECTION C-C'**

PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

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- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
  - 12.0' STRATUM THICKNESS

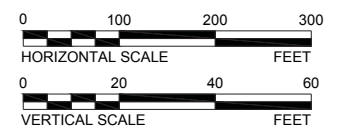
- NOTES**
1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

▼ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

△ SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

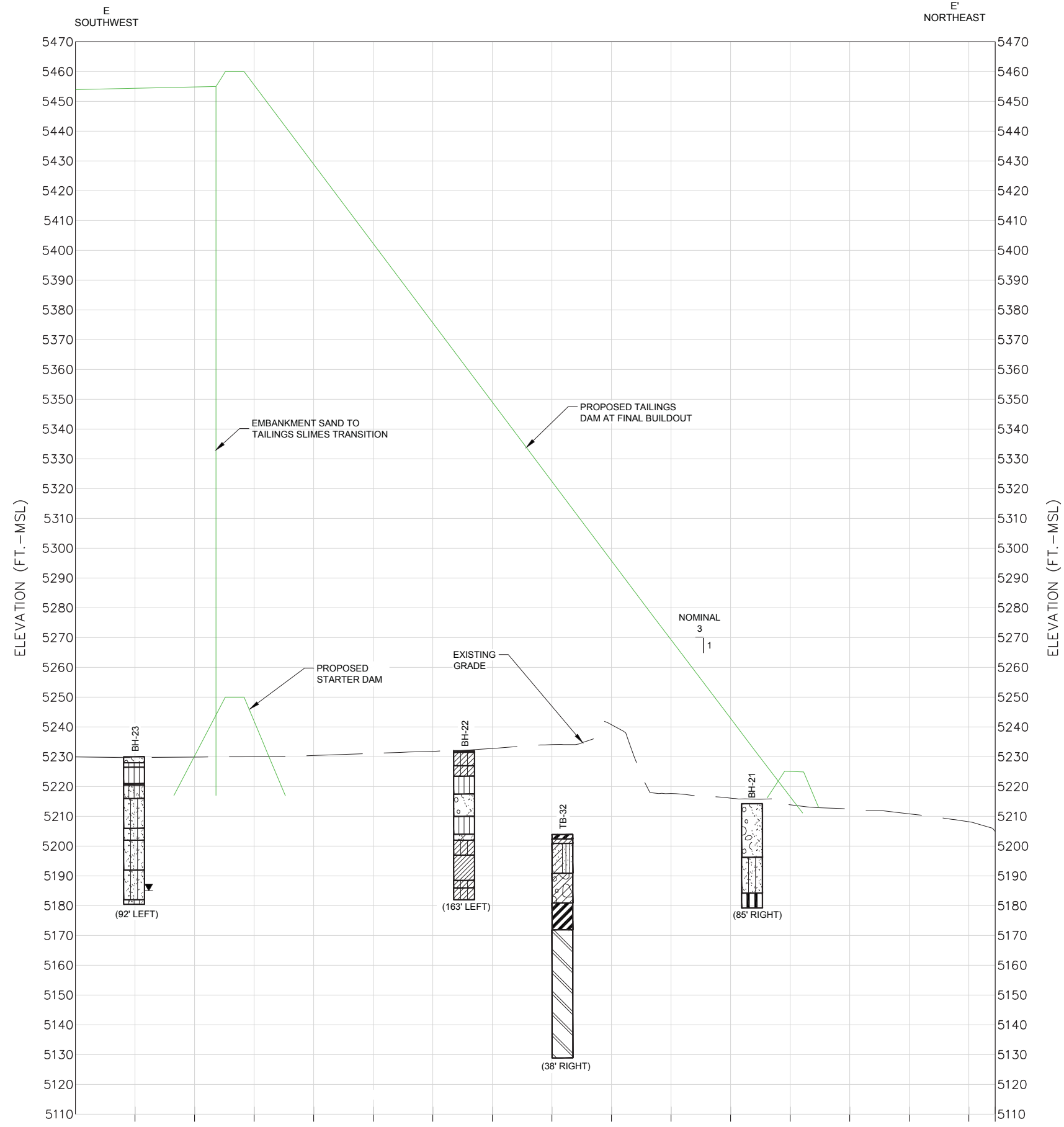
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Resources  
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557 FILE No. 10392557K004  
30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTION D-D'**

DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

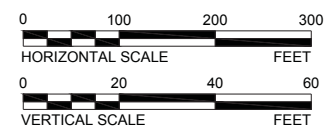
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- NOTES**
- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  - PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  - PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  - SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
1	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
3	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
4	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
5	2013-05-03	ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

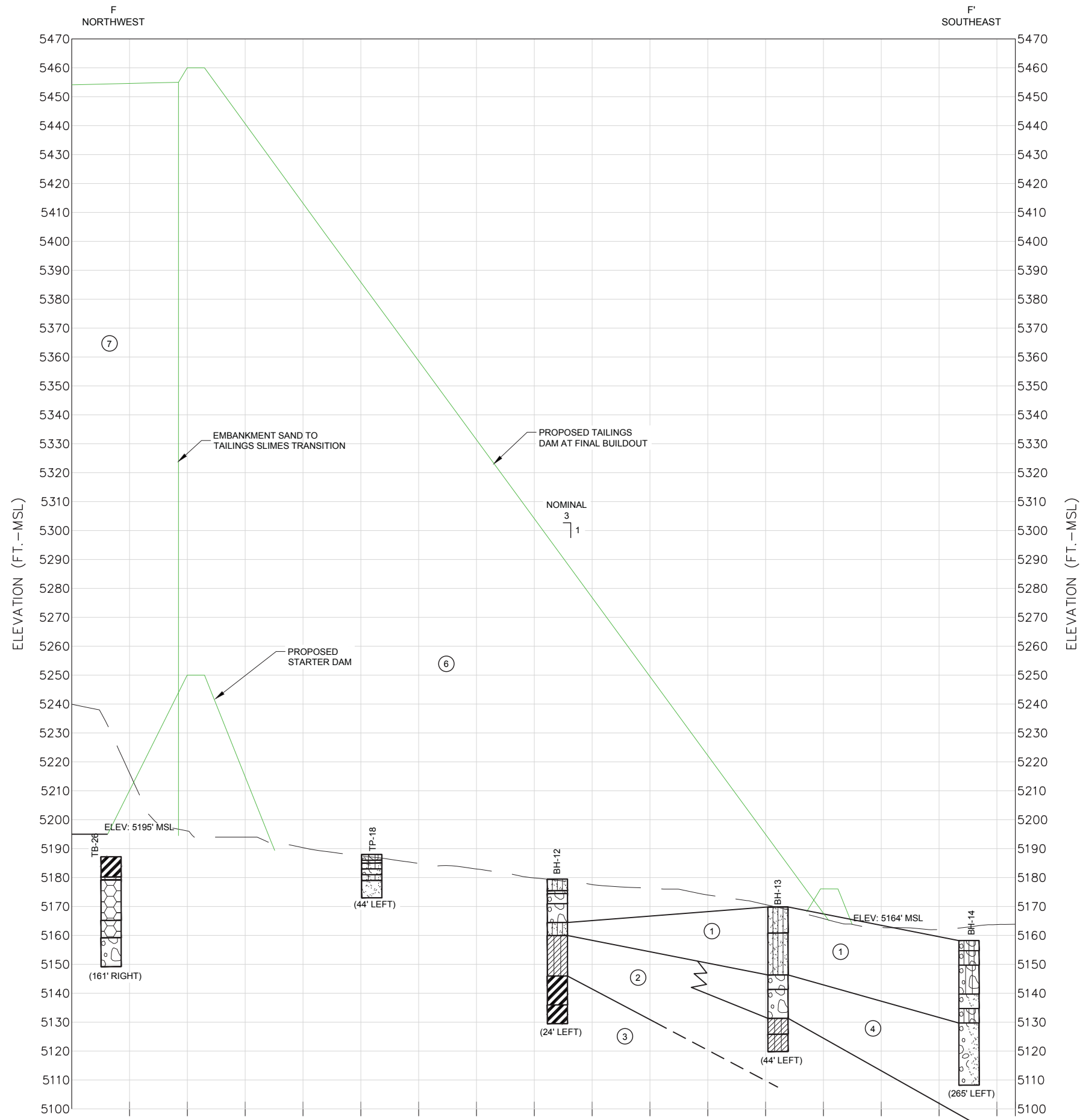
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

**COPPER FLAT PROJECT**  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**GEOLOGIC CROSS-SECTION E-E'**

	PROJECT No.	103-92557	FILE No.	10392557K004	
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
	REVIEW	DAK	2013-07-17		

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**STRATUM DESCRIPTORS**

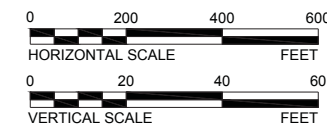
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

**NOTES**

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
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DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

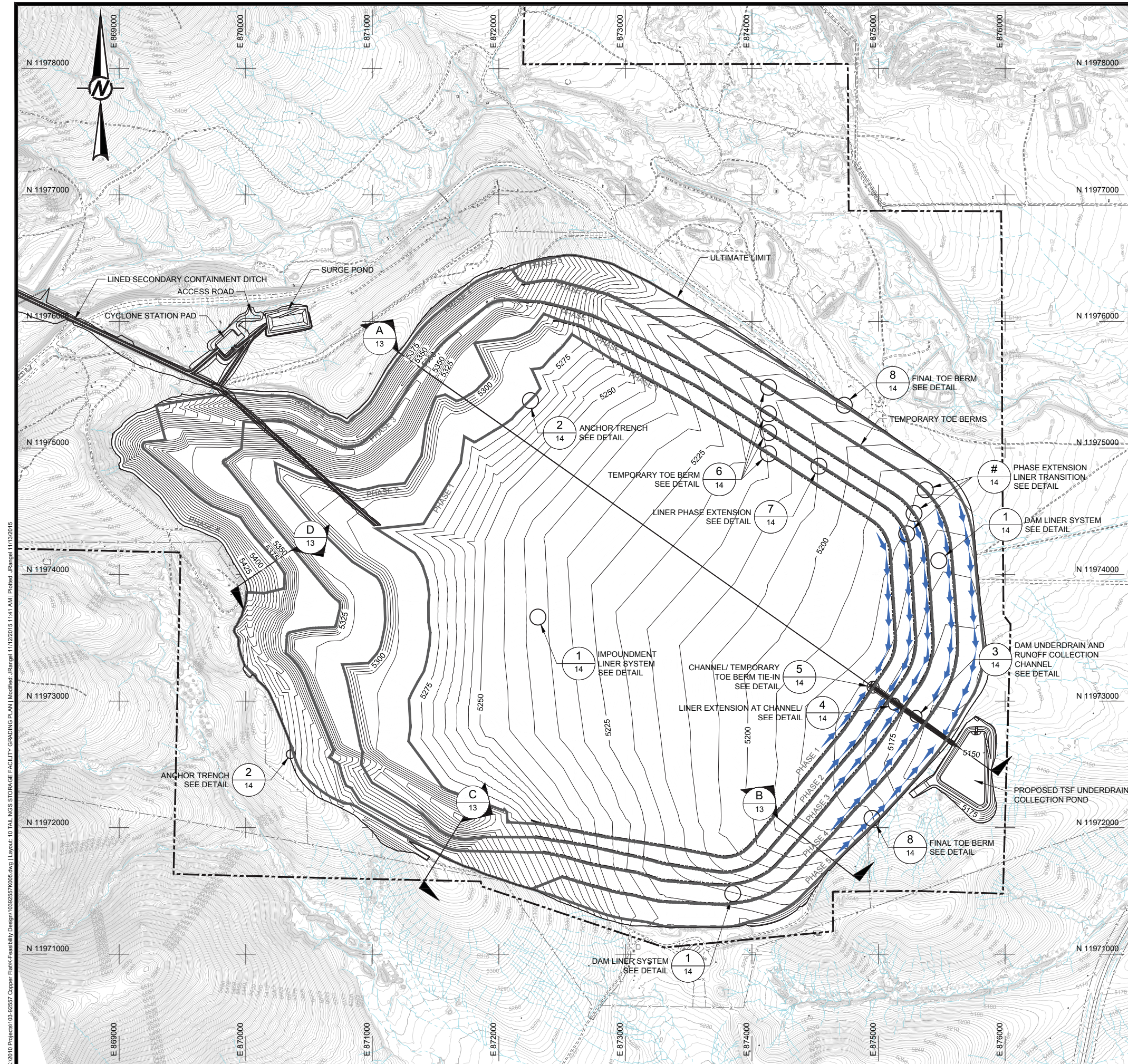
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30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

TITLE <b>GEOLOGIC CROSS-SECTION F-F'</b>					
PROJECT No.	103-92557	FILE No.	10392557K004		
DESIGN	DW	2013-04-08	SCALE	AS SHOWN	
CADD	JHR	2013-07-10	DRAWING		
CHECK	GM	2013-07-16			
REVIEW	DAK	2013-07-17			







**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- DRAINAGE
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

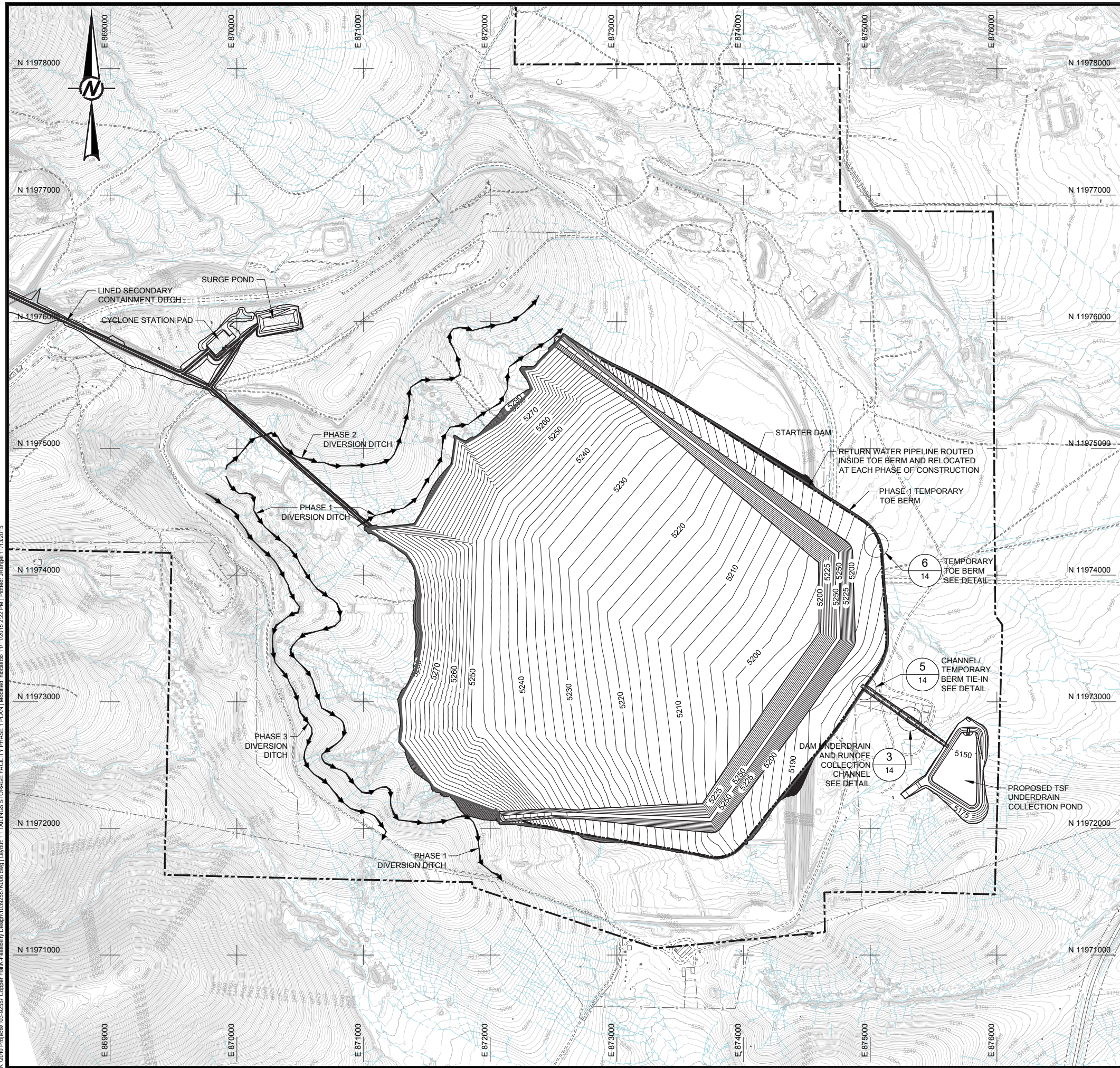
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GRADING PLAN**

PROJECT No.	103-92557	FILE No.	10392557K005
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REVIEW	DAK	2013-07-17	



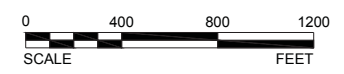
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**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.

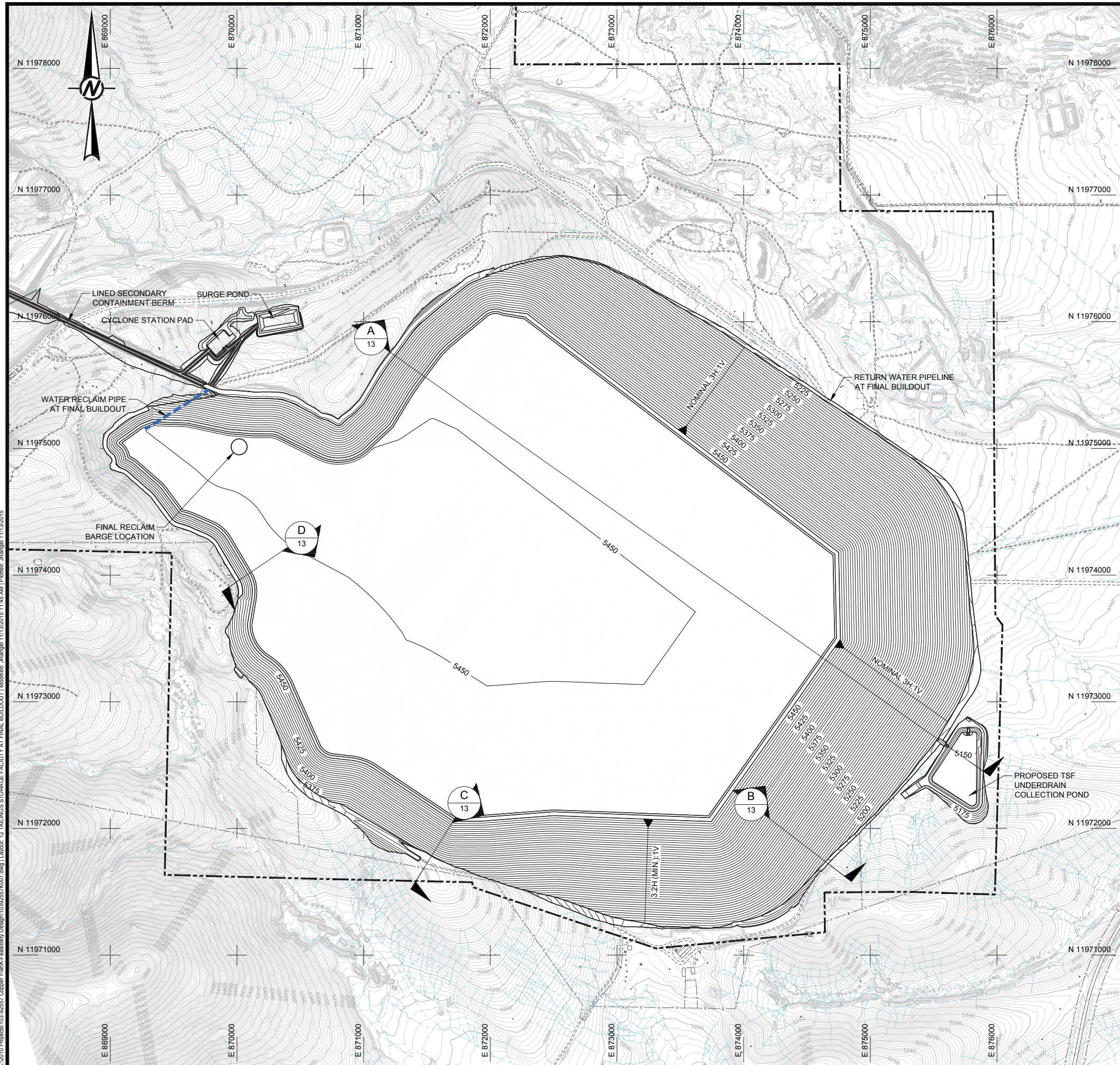
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS STORAGE FACILITY PHASE 1  
PLAN**

PROJECT No.	103-92557	FILE No.	10392557K006
DESIGN	DW 2013-04-08	SCALE	AS SHOWN
CADD	JHR 2013-07-10	DRAWING	
CHECK	GM 2013-07-16		
REVIEW	DAK 2013-07-17		

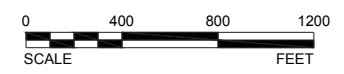


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**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

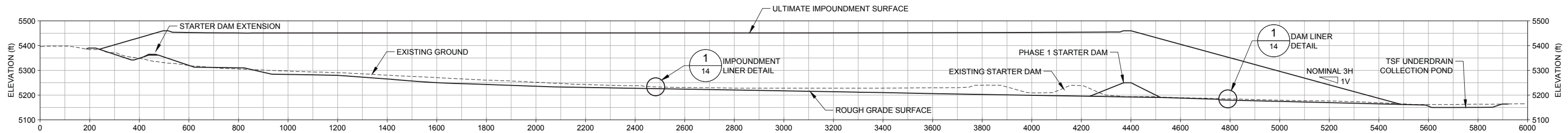
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

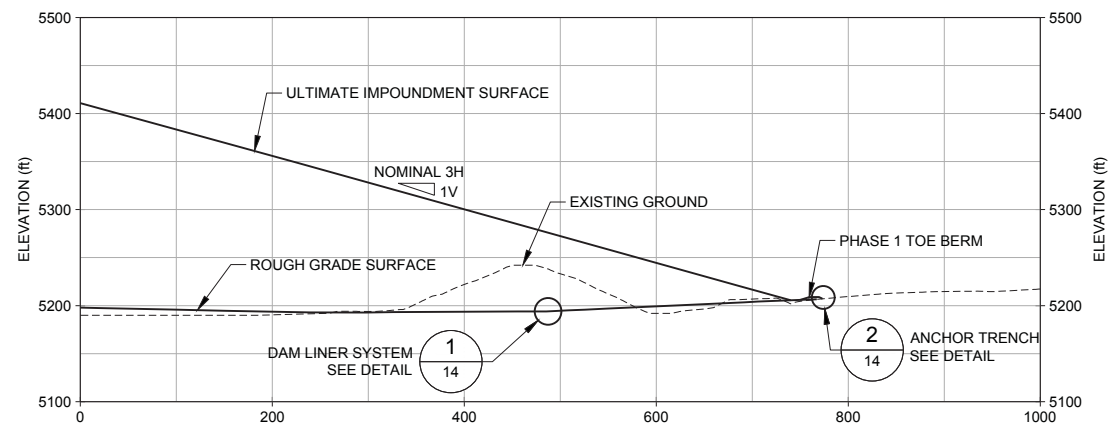
TITLE  
**TAILINGS STORAGE FACILITY AT FINAL BUILDOUT**

PROJECT No.	103-92557	FILE No.	10392557K007
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REVIEW	DAK	2013-07-17	

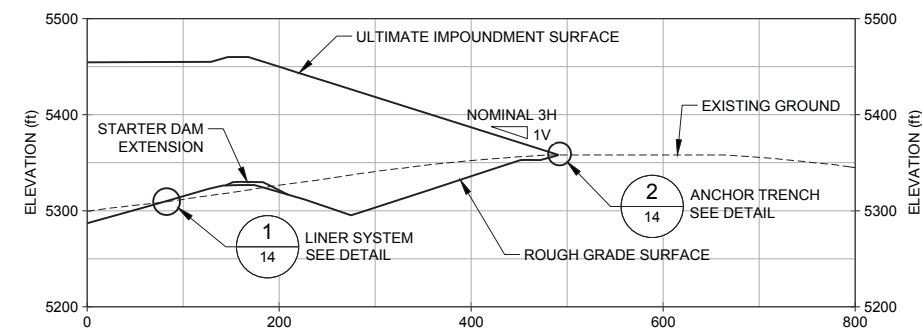




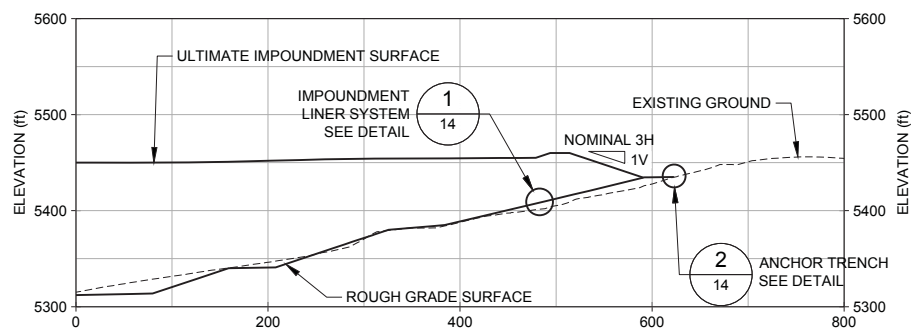
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SCALE B **B** CROSS-SECTION B  
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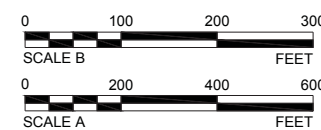


SCALE B **C** CROSS-SECTION C  
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NOTE: STARTER DAM EXTENSION TO BE SHOWN ON DETAILED DESIGN DRAWINGS.

SCALE B **D** CROSS-SECTION D  
13



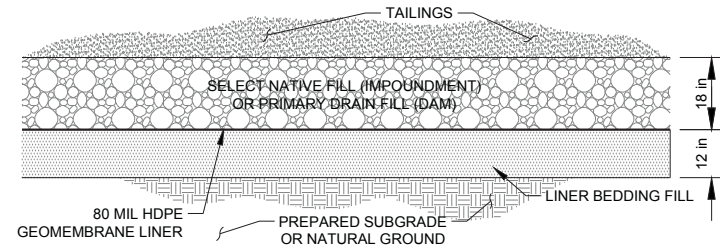
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2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
2013-11-15		ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

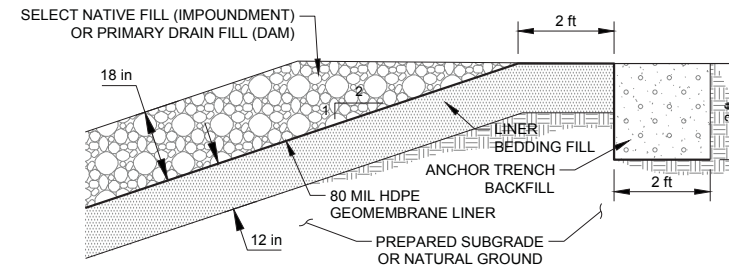
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE					
<b>TAILINGS STORAGE FACILITY CROSS-SECTIONS</b>					
PROJECT No.	103-92557	FILE No.	10392557K008		
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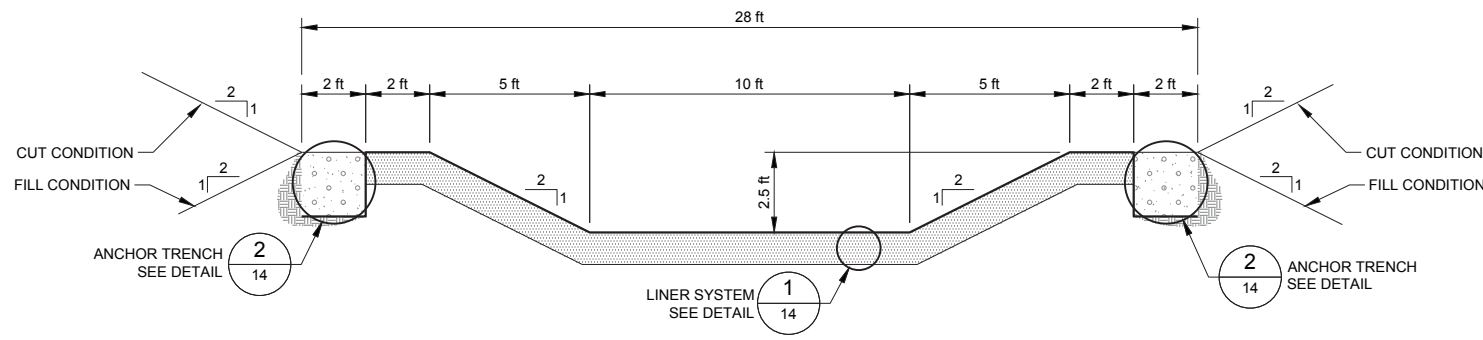




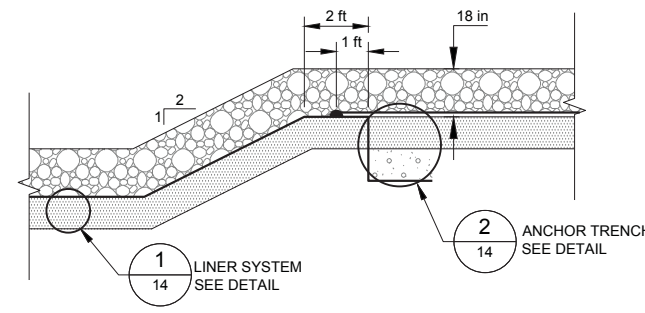
SCALE: N.T.S. **1** IMPOUNDMENT LINER SYSTEM DETAIL  
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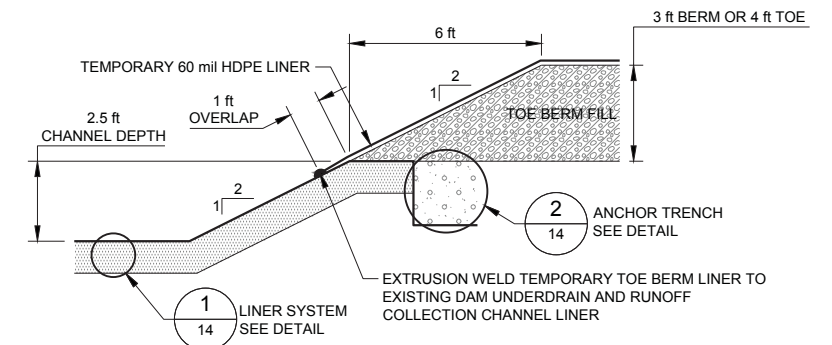
SCALE: N.T.S. **2** ANCHOR TRENCH DETAIL  
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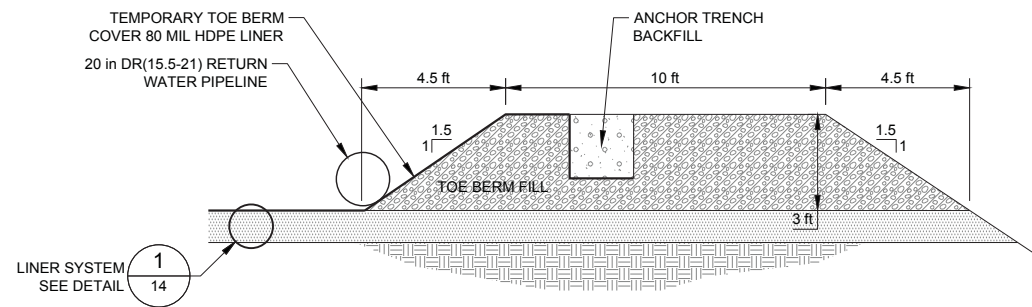
SCALE: N.T.S. **3** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL DETAIL  
14



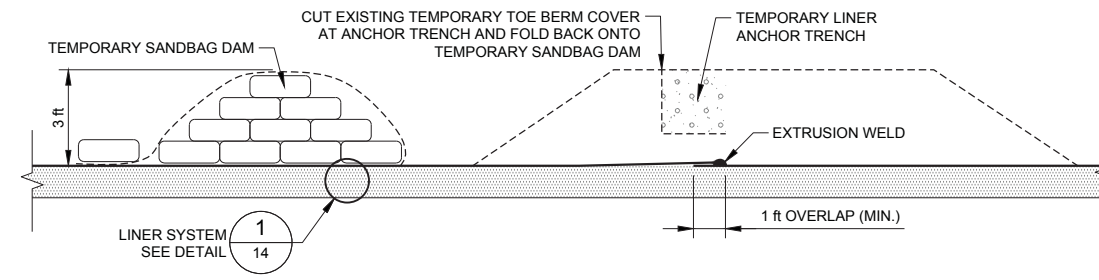
SCALE: N.T.S. **4** LINER EXTENSION WITH CHANNEL ANCHOR DETAIL  
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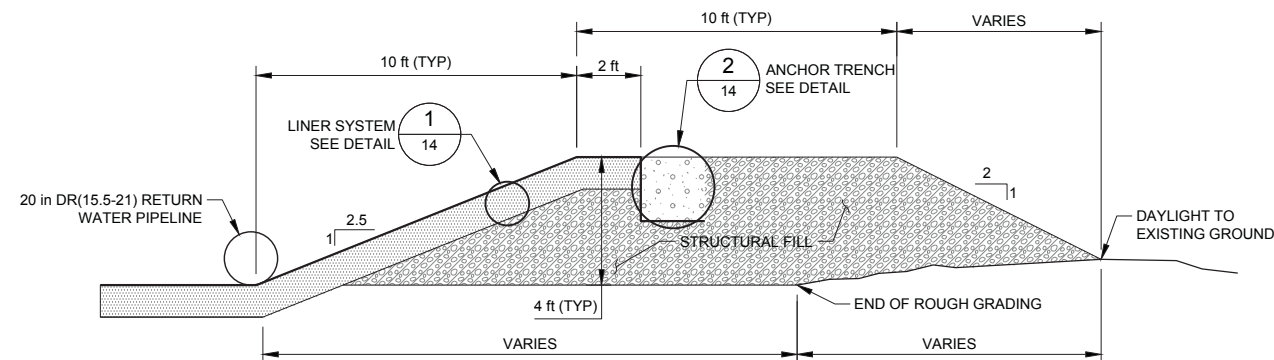
SCALE: N.T.S. **5** CHANNEL/TEMPORARY BERM TIE-IN DETAIL  
14



SCALE N.T.S. **6** TEMPORARY TOE BERM DETAIL  
14



SCALE N.T.S. **7** PHASE EXTENSION LINER TIE-IN AT TOE BERM DETAIL  
14



SCALE: N.T.S. **8** FINAL TOE BERM DETAIL  
14

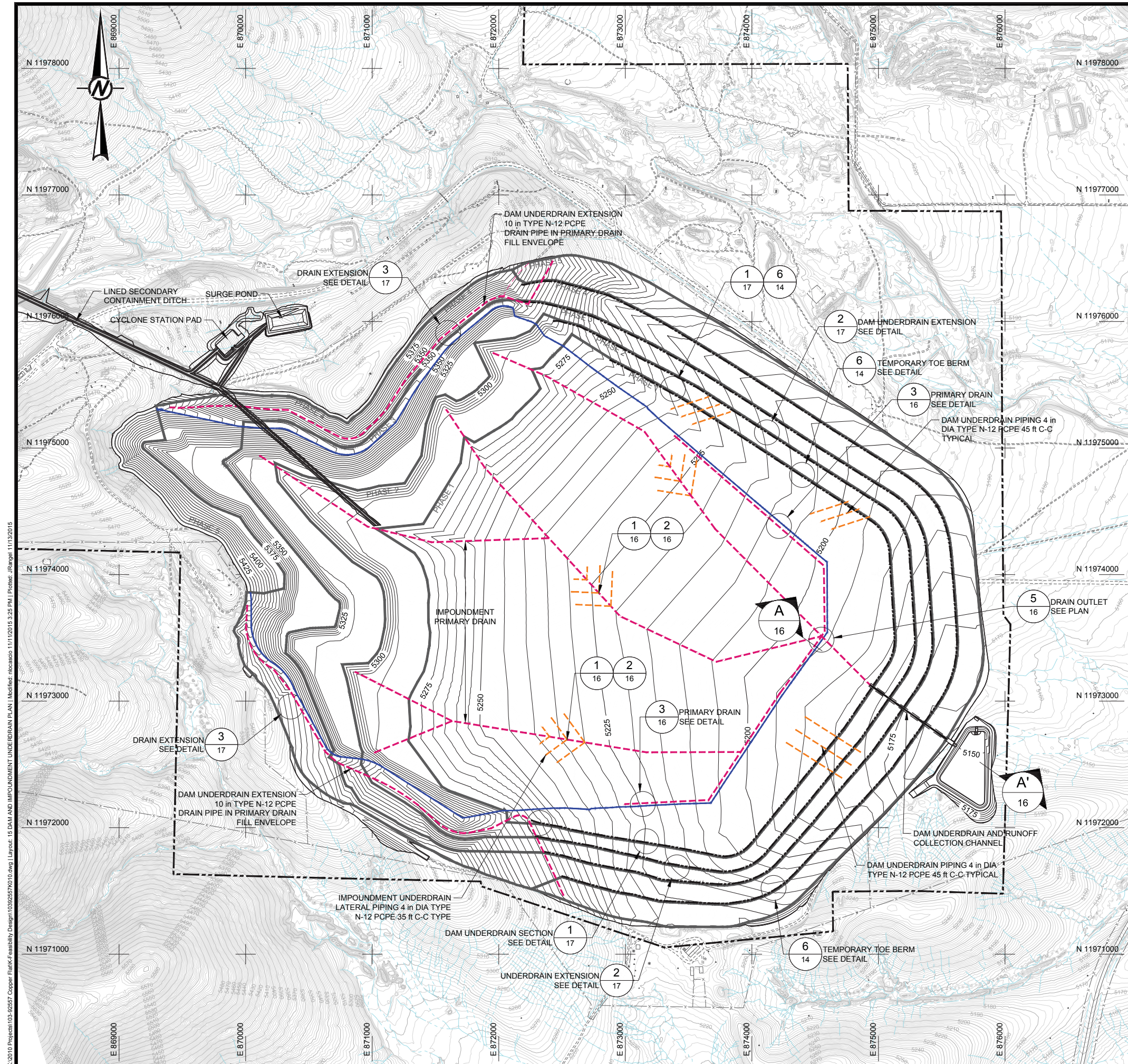
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2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE					
<b>TAILINGS STORAGE FACILITY DETAILS</b>					
PROJECT No.	103-92557	FILE No.	10392557K009		
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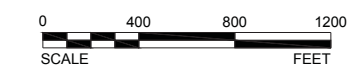




**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 10 IN. DIA. N-12 PCPE PIPE
- 12 IN. DIA. N-12 PCPE PIPE
- 14 IN SCH80 STEEL
- 4 IN. DIA. N-12 PCPE PIPE
- LIMIT IMPOUNDMENT UNDERDRAIN SYSTEMS
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION

- NOTES**
- 1.) TYPICAL ORIENTATION FOR 4 in UNDERDRAIN PIPING SHOWN. COVERAGE WILL BE AT 35 ft C-C SPACING OVER IMPOUNDMENT UNDERDRAIN AREA. COVERAGE WILL BE 45 ft C-C OVER DAM UNDERDRAIN AREA.
  - 2.) THE MAIN BODY OF THE EMBANKMENT WILL BE UNDERLAIN WITH A BLANKET DRAIN WITH INTERNAL 4 in PCPE PIPE NETWORK. DAM UNDERDRAIN EXTENSION IN PHASE 2 THROUGH PHASE 5 WILL INCLUDE A 10 in DIAMETER PCPE DRAIN CONSTRUCTED ON THE OUTER TOE OF THE STARTER DAM



2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK
REV	DATE	DES	CADD	CHK	RVW

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

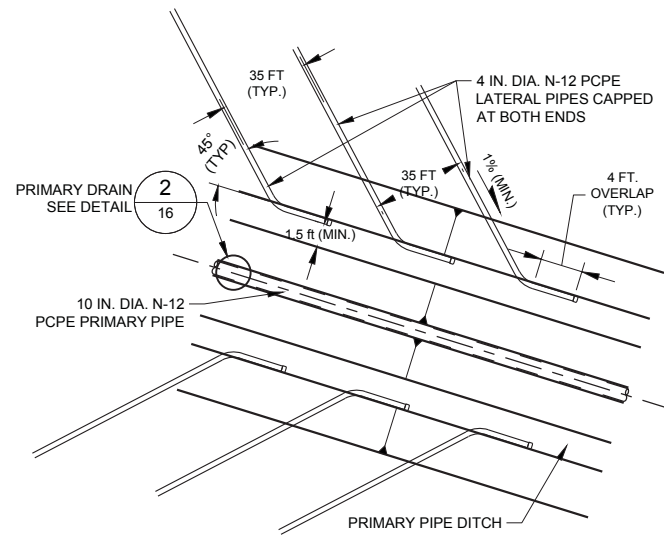
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**DAM AND IMPOUNDMENT UNDERDRAIN PLAN**

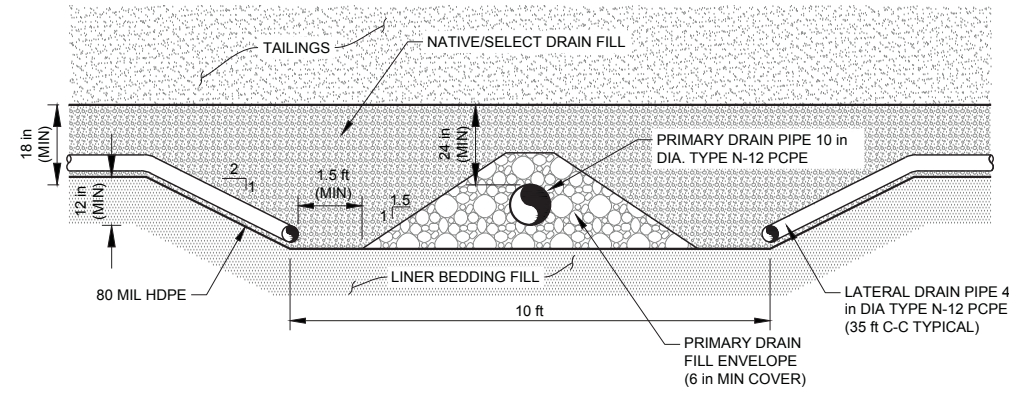
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REVIEW	DAK	2013-07-17	



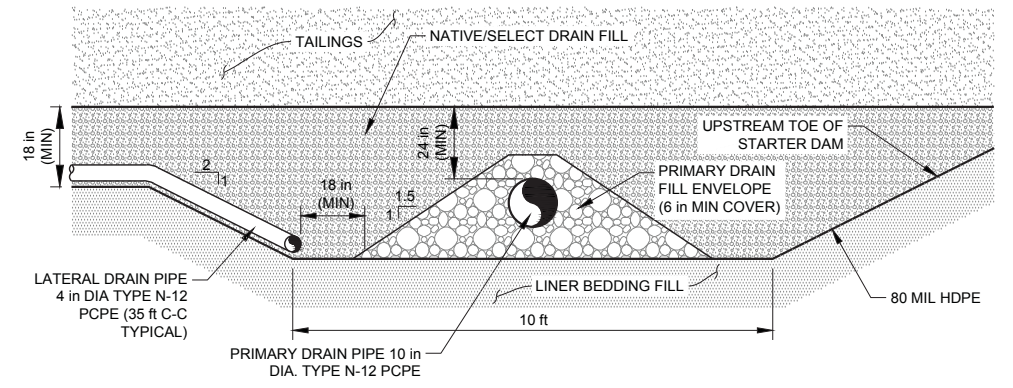
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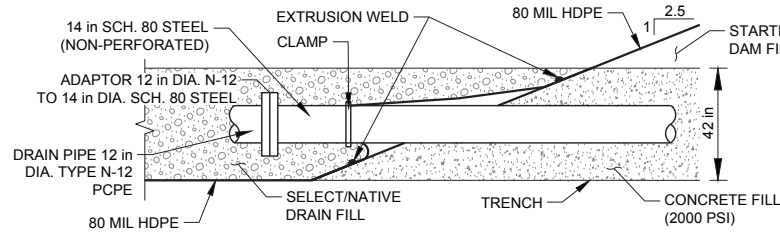
NTS 1 IMPOUNDMENT DRAIN PLACEMENT  
16



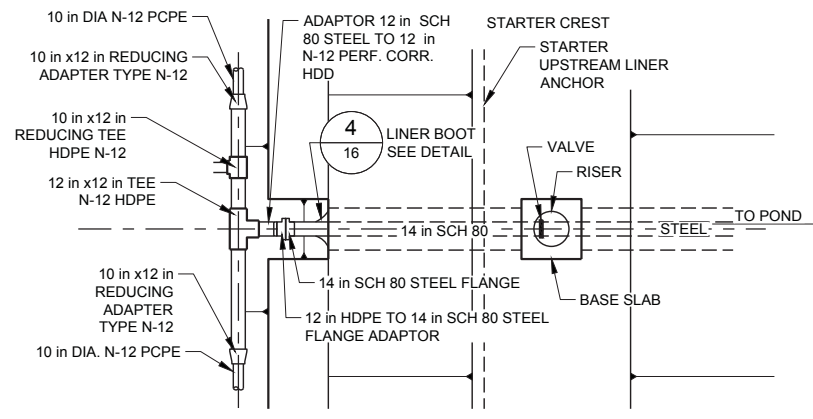
NTS 2 PRIMARY DRAIN PLACEMENT  
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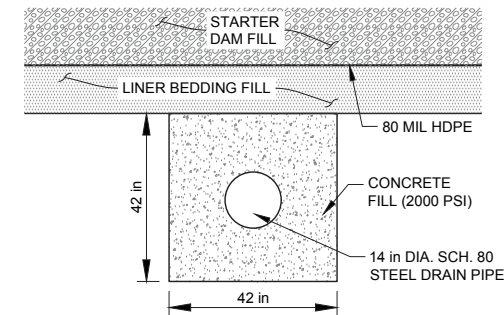
NTS 3 PRIMARY DRAIN PLACEMENT AT STARTER DAM TOE  
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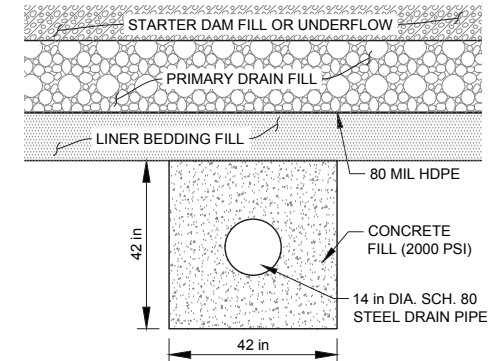
NTS 4 DETAIL DRAIN PIPE TO STARTER DAM ENTRY  
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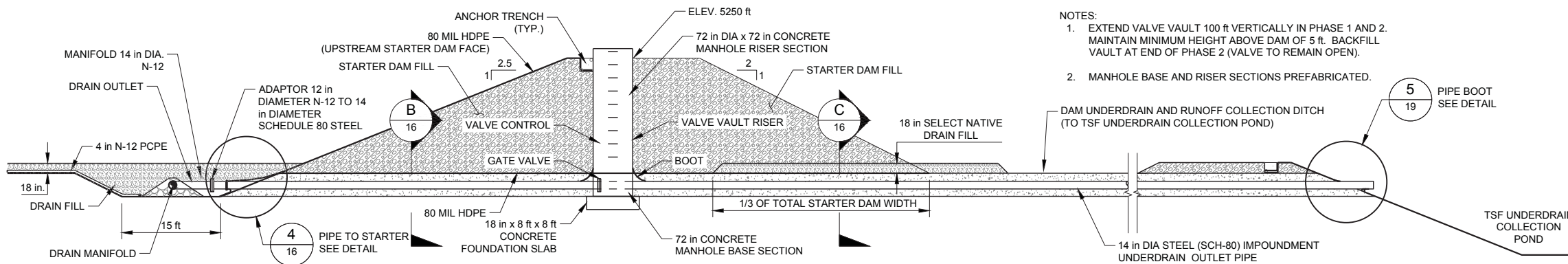
NTS 5 DRAIN OUTLET PLAN  
16



NTS B SECTION B-B  
16



NTS C SECTION C-C  
16



NTS A SECTION A-A DRAIN OUTLET  
16

- NOTES:
1. EXTEND VALVE VAULT 100 ft VERTICALLY IN PHASE 1 AND 2. MAINTAIN MINIMUM HEIGHT ABOVE DAM OF 5 ft. BACKFILL VAULT AT END OF PHASE 2 (VALVE TO REMAIN OPEN).
  2. MANHOLE BASE AND RISER SECTIONS PREFABRICATED.

5 PIPE BOOT SEE DETAIL  
19

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

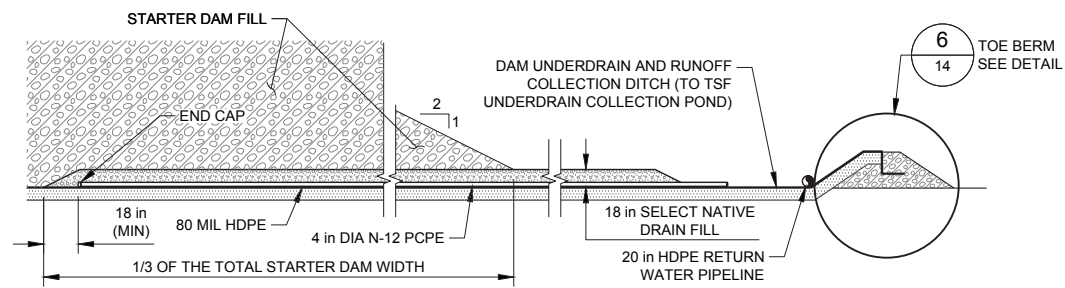
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

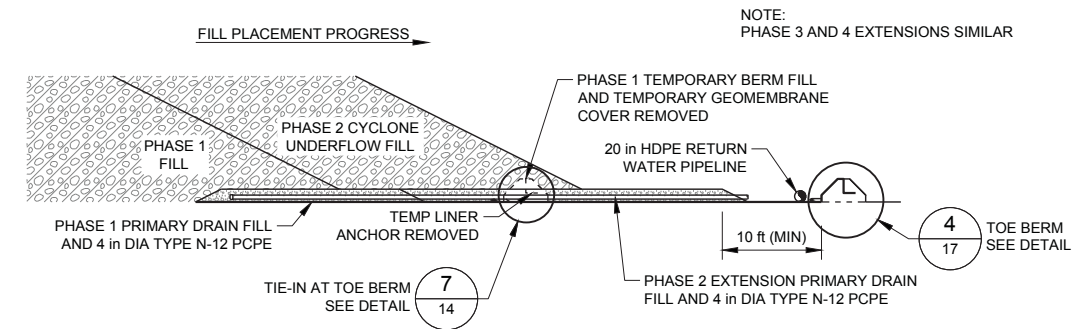
**DAM UNDERDRAIN DETAILS (1 of 2)**

PROJECT No.	103-92557	FILE No.	10392557K011
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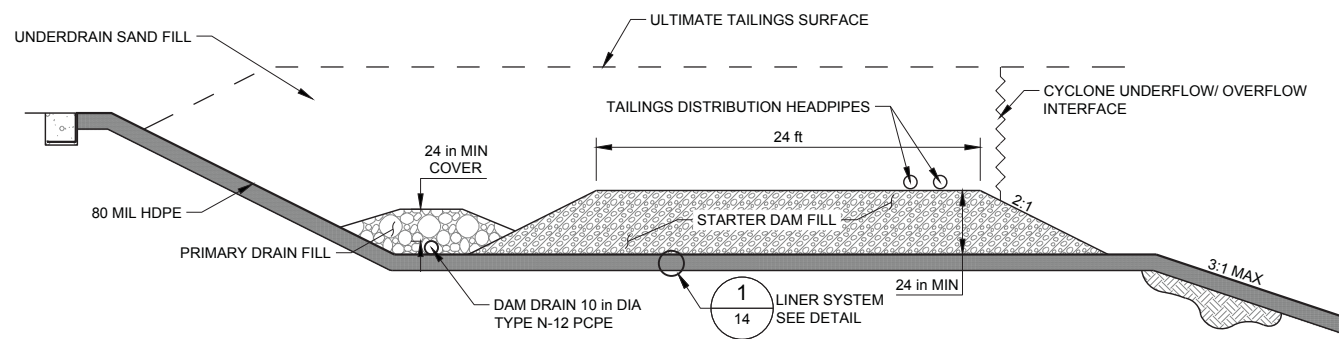




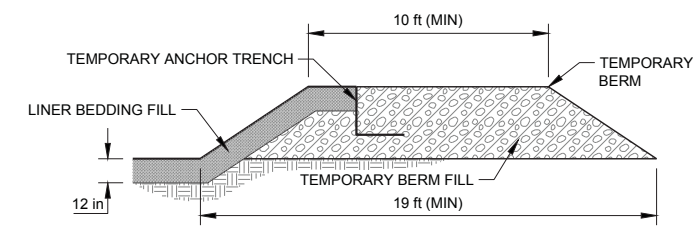
NTS **1** DAM UNDERDRAIN SECTION  
17



NTS **2** PHASE 1 TO PHASE 2 TSF LINER AND DAM UNDERDRAIN EXTENSION  
17



NTS **3** DAM UNDERDRAIN 10 IN PCPE DRAIN EXTENSION  
17



NTS **4** TEMPORARY PERIMETER BERM  
17

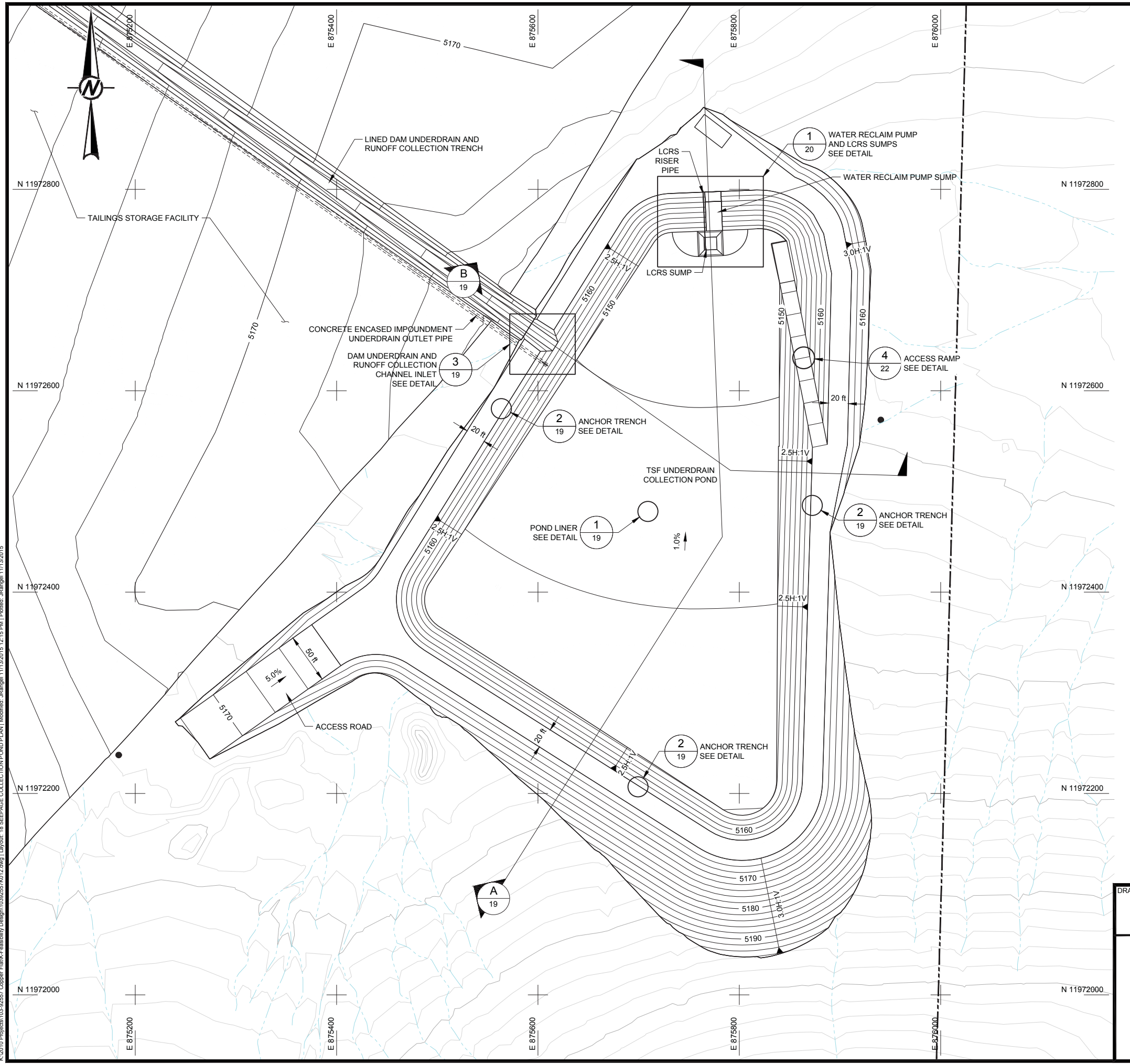
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2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE <b>PRELIMINARY</b> FOR AGENCY REVIEW	PROJECT <b>THEMAC</b> RESOURCES NEW MEXICO COPPER CORPORATION Environmentally Responsible. Community Minded. Local Opportunities.		PROJECT No. 103-92557		FILE No. 10392557K011
	TITLE <b>DAM UNDERDRAIN DETAILS (2 of 2)</b>		30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO		SCALE AS SHOWN
PROJECT No. 103-92557		FILE No. 10392557K011	DESIGN DW 2013-04-08		SCALE AS SHOWN
CADD JHR 2013-07-10		DRAWING		CHECK GM 2013-07-16	
REVIEW DAK 2013-07-17				17 EBIP Exhibit 18	

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K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K012.dwg | Layout: 18 SEEPAGE COLLECTION POND PLAN | Modified: 11/13/2015 12:15 PM | Plotted: 11/13/2015



**LEGEND**

- 3600 --- EXISTING GROUND CONTOUR (ft -MSL)
- - - - - EXISTING ROADS
- - - - - EXISTING DRAINAGE
- x - x - x EXISTING FENCELINE
- - - - - MINE PERMIT AREA BOUNDARY
- == 3600 == REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- ▲ SLOPE INDICATOR
- 3H:1V or 3H 1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1 14 DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- A 13 CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
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DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

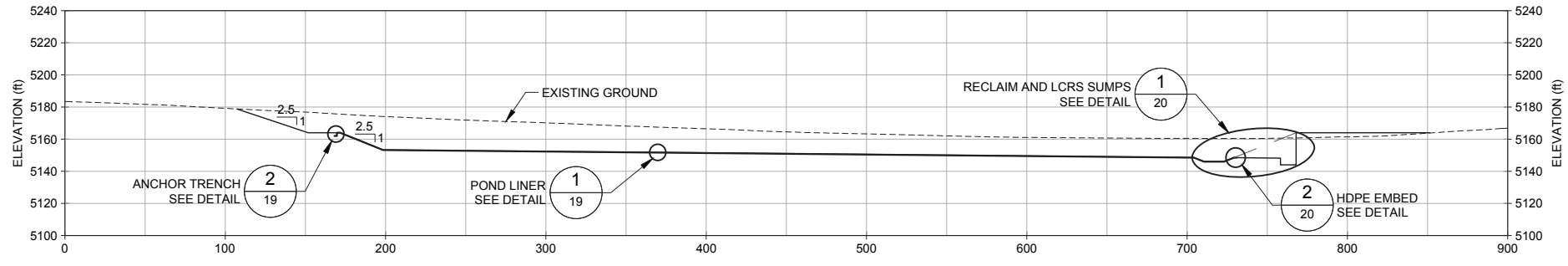
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

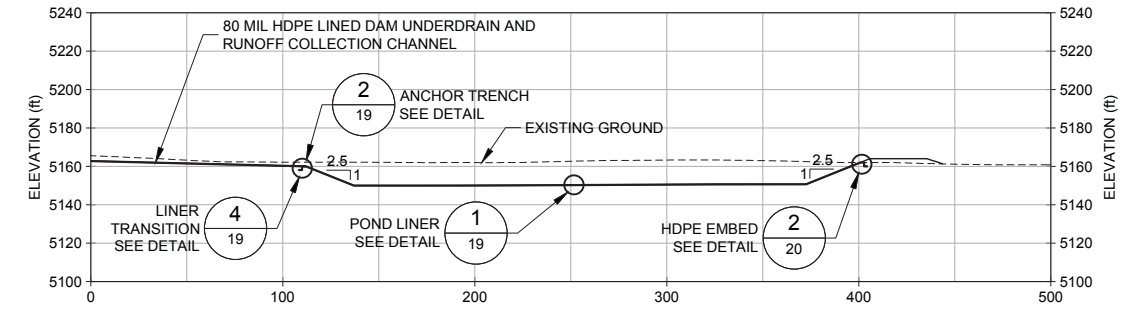
TITLE  
**TSF UNDERDRAIN COLLECTION POND PLAN**

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REVIEW	DAK	2013-07-17	

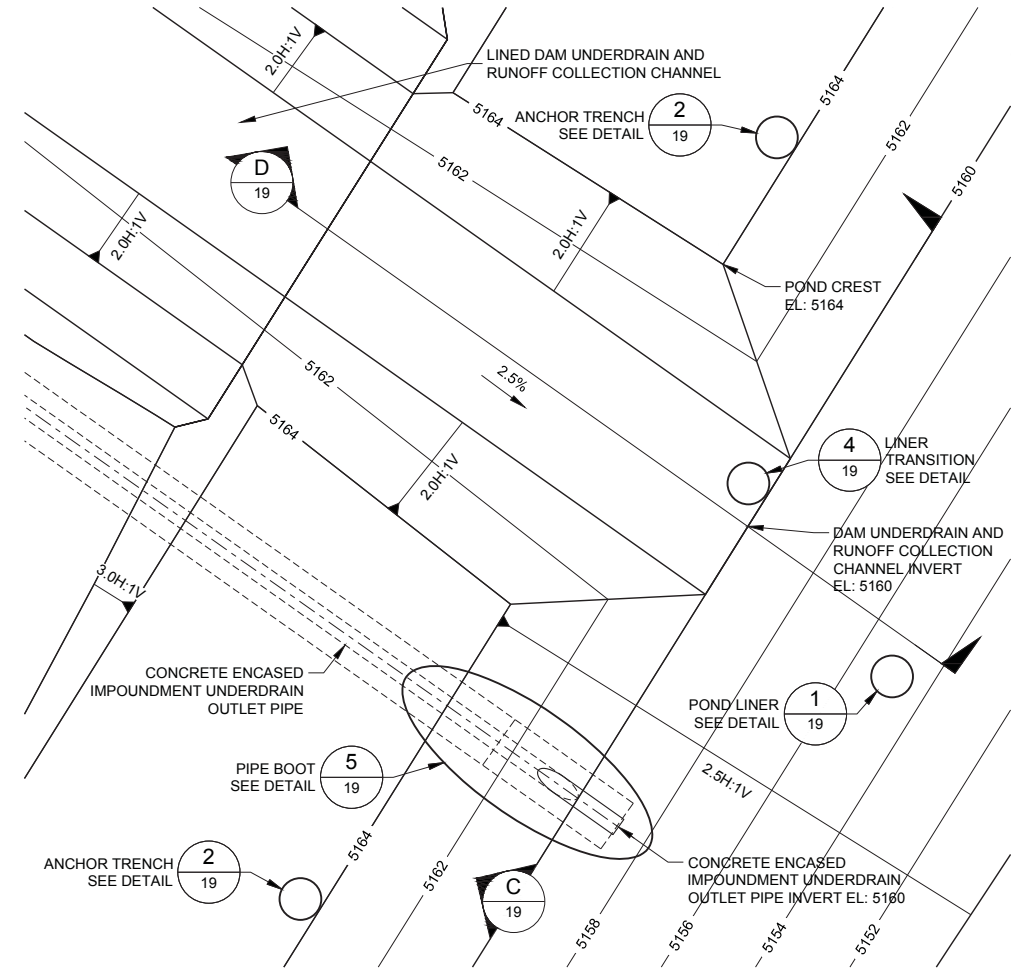




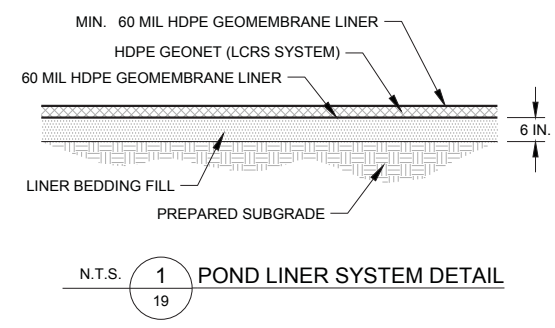
SCALE A **A** CROSS-SECTION A  
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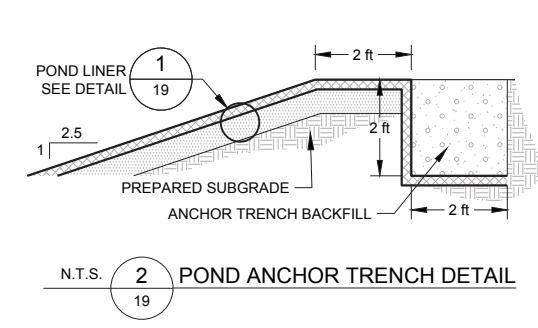
SCALE A **B** CROSS-SECTION B  
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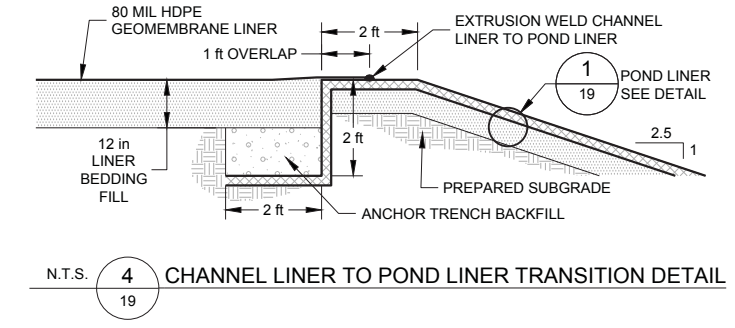
SCALE B **3** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET TO POND  
19



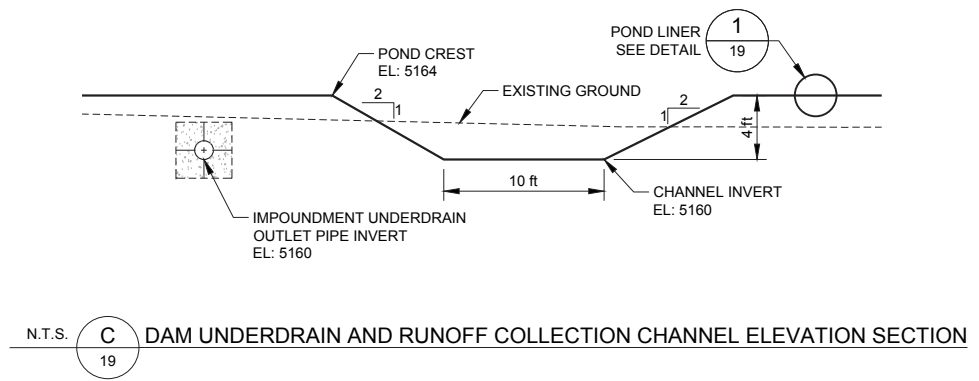
N.T.S. **1** POND LINER SYSTEM DETAIL  
19



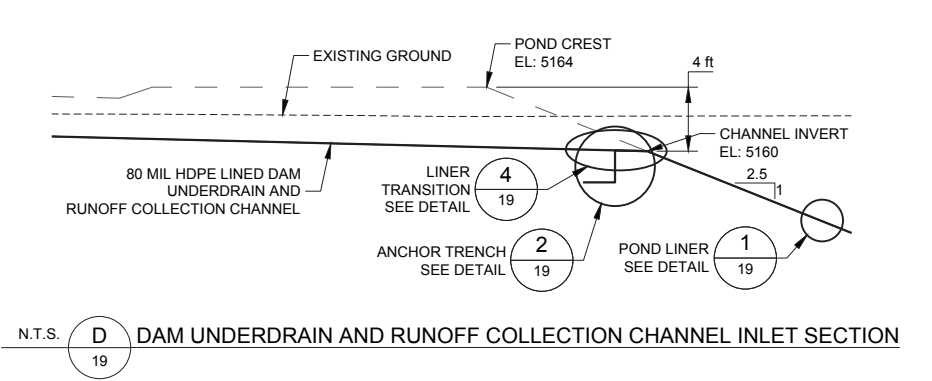
N.T.S. **2** POND ANCHOR TRENCH DETAIL  
19



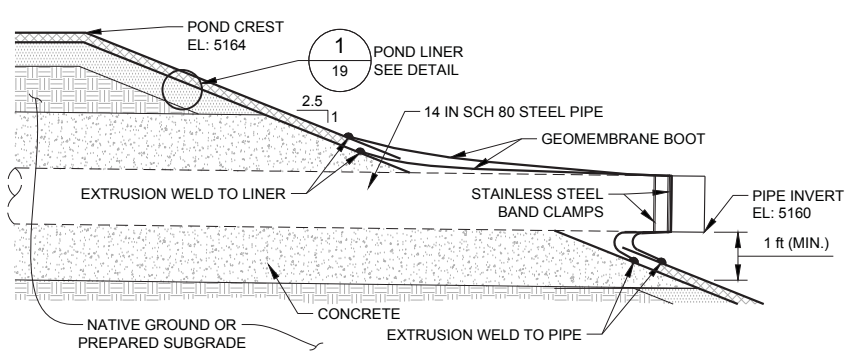
N.T.S. **4** CHANNEL LINER TO POND LINER TRANSITION DETAIL  
19



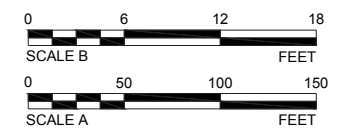
N.T.S. **C** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL ELEVATION SECTION  
19



N.T.S. **D** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET SECTION  
19



N.T.S. **5** IMPOUNDMENT UNDERDRAIN OUTLET PIPE BOOT DETAIL  
19



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

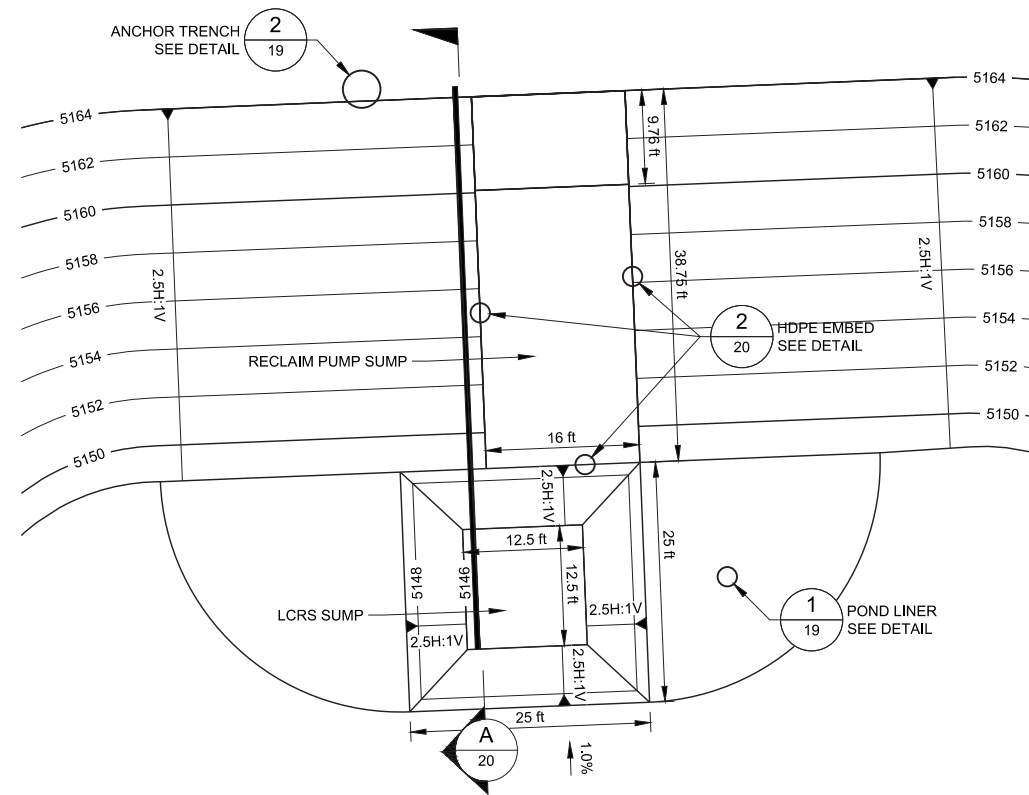
TITLE  
**TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K013
DESIGN	DW 2013-04-08	SCALE	AS SHOWN
CADD	JHR 2013-07-10	DRAWING	
CHECK	GM 2013-07-16		
REVIEW	DAK 2013-07-17		

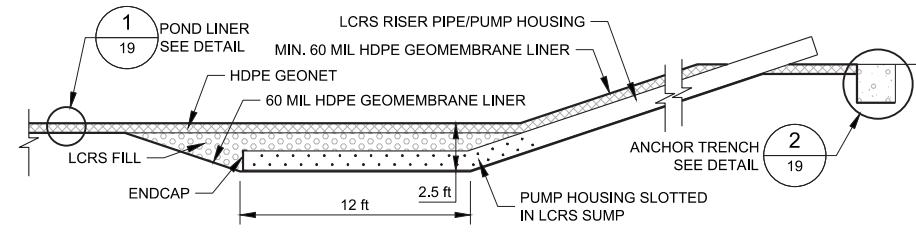


K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K013.dwg | Layout: 19 SEEPAGE COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2) | Modified: mrodriguez 11/11/2015 4:21 PM | Plotter: JRange 11/13/2015

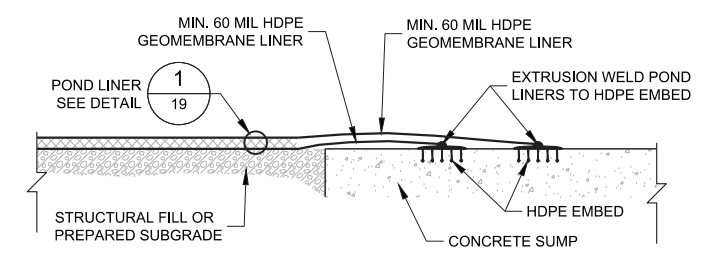
K:\2010 Projects\103-9257 Copper Flat\KF\Feasibility Design\1039257K013.dwg | Layout: 20 SEEPAGE COLLECTION POND CROSS-SECTIONS AND DETAILS (2 OF 2) | Modified: nccasdo 11/11/2015 4:21 PM | Plotted: JRange1 11/13/2015



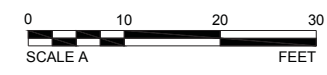
SCALE A **1** RECLAIM PUMP SUMP AND LCRS SUMP DETAIL  
20



N.T.S. **A** LCRS SUMP SECTION  
20



N.T.S. **2** CONCRETE DOUBLE EMBED DETAIL  
20



2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK	
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

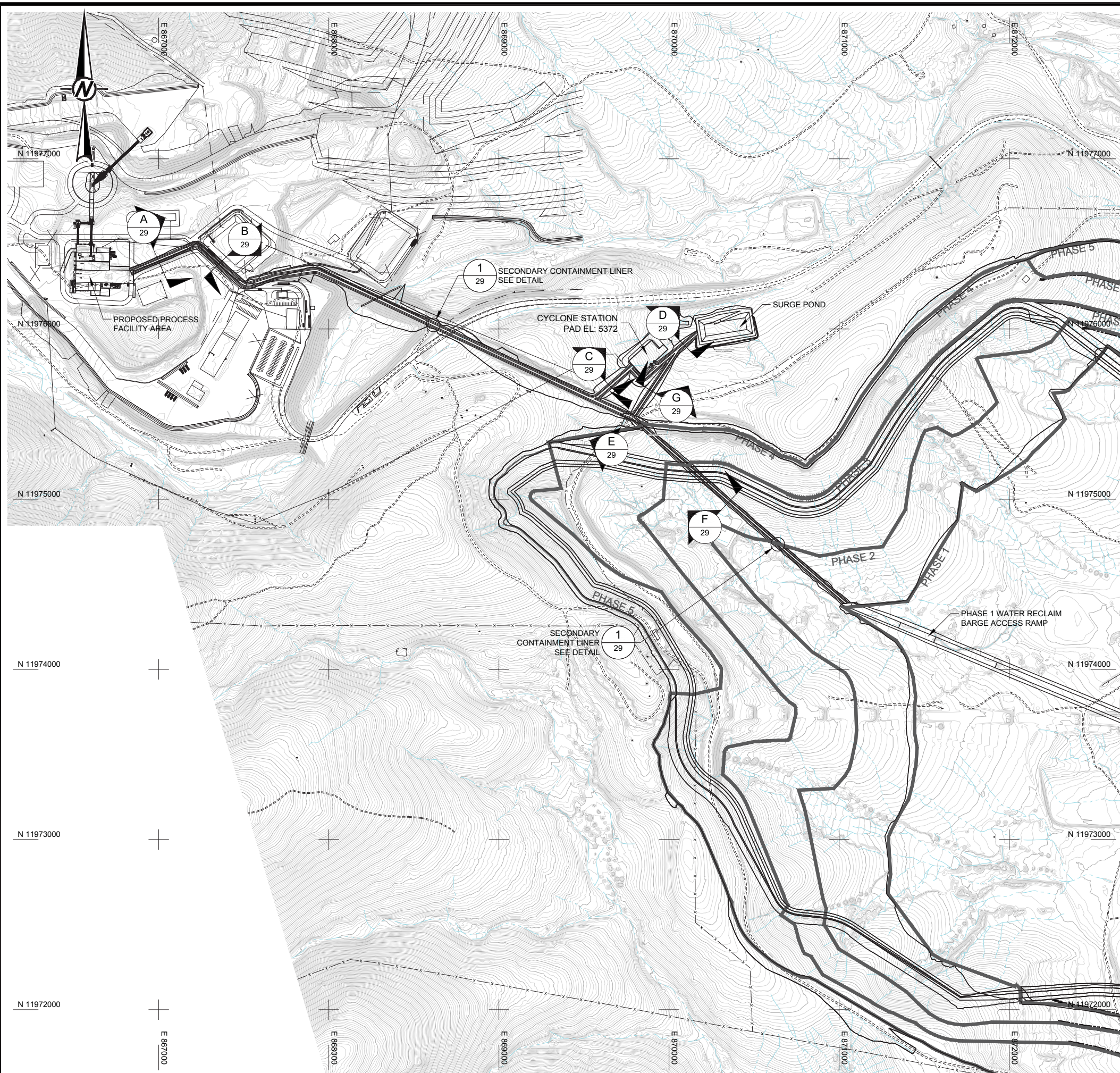
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT: **COPPER FLAT PROJECT**  
**THEMAC** RESOURCES | NEW MEXICO COPPER CORPORATION  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(2 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K013
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

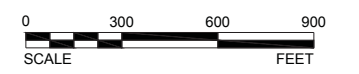




**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION

K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K014.dwg | Layout: 21 CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN | Modified: nbcasico 11/11/2016 4:45 PM | Plotted: JRange 11/13/2015



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

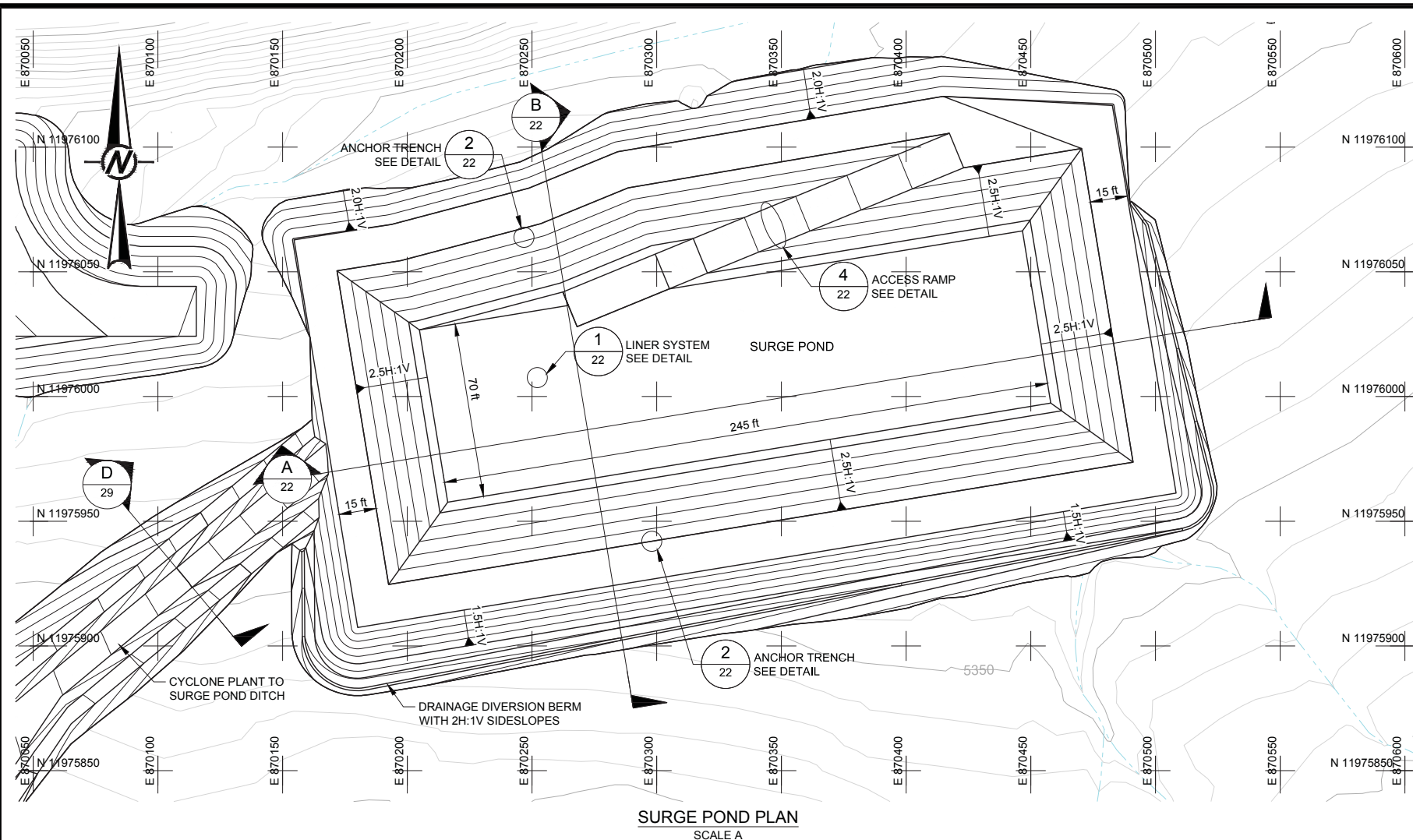
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557 FILE No. 10392557K014  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN**

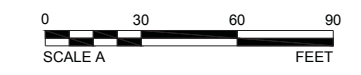
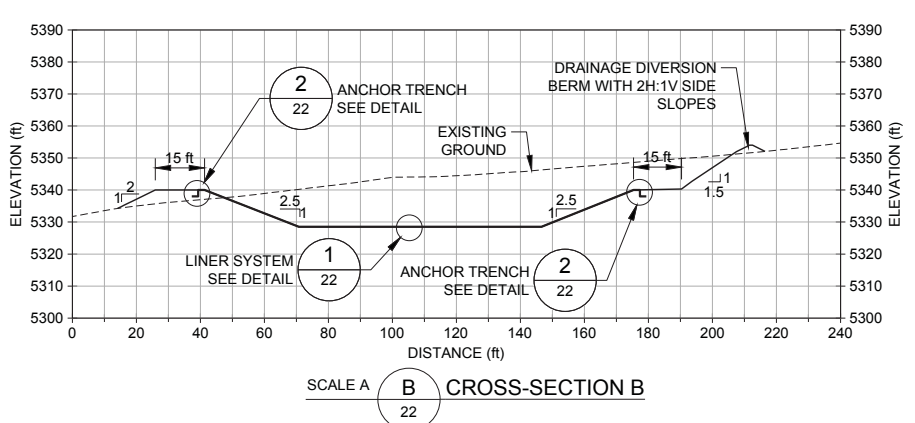
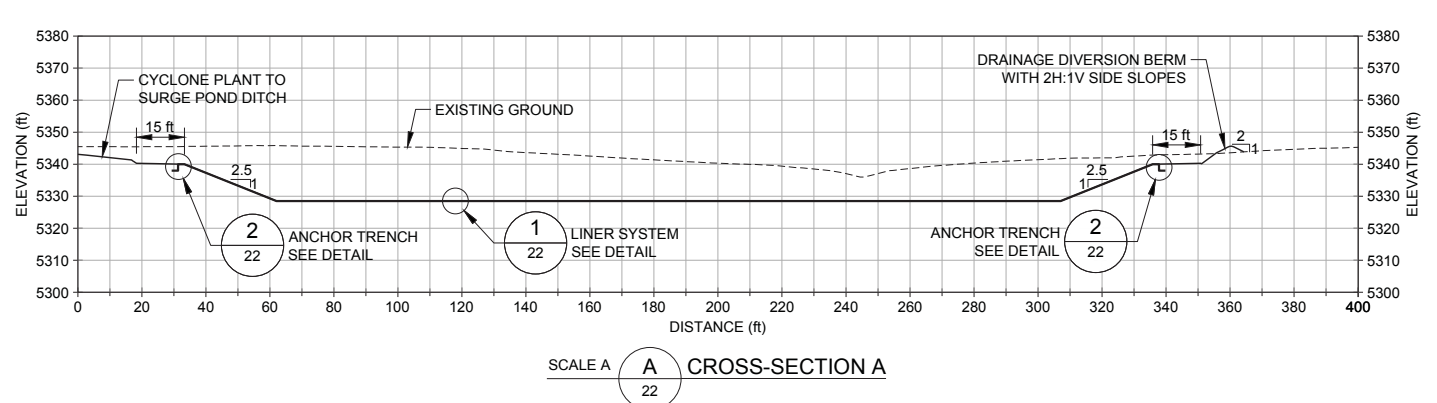
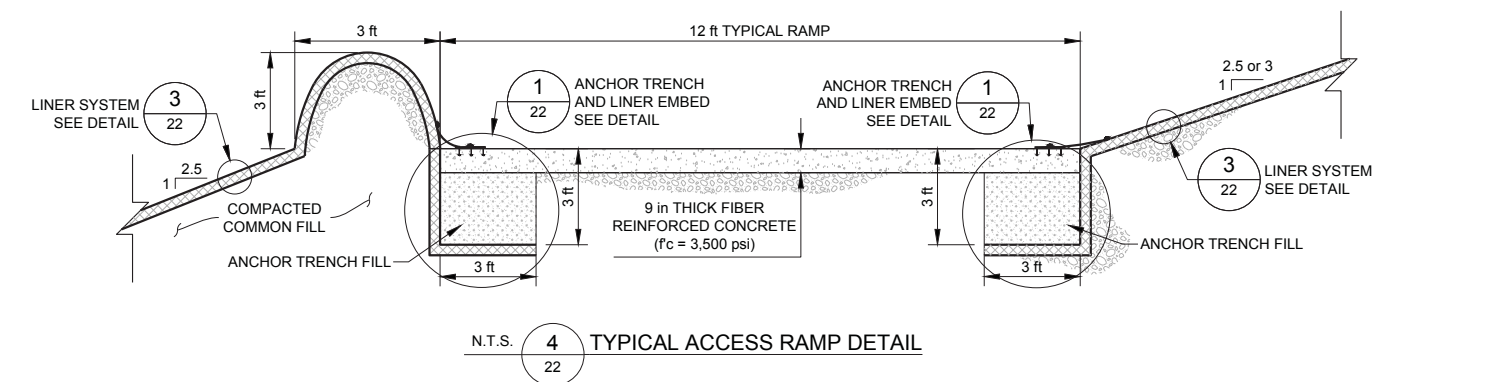
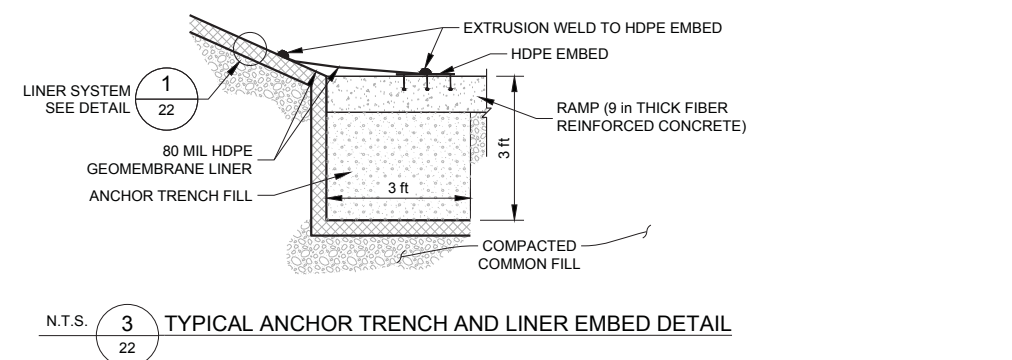
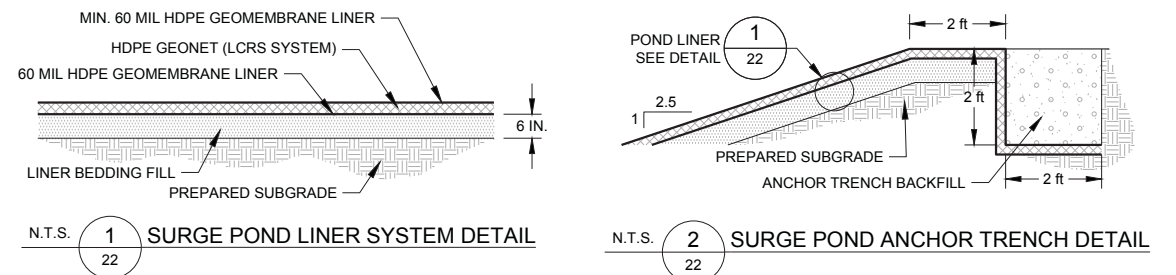
DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

**Golder Associates**



**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- 3600 REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1 2-315 DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- A 2-313 CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK	
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

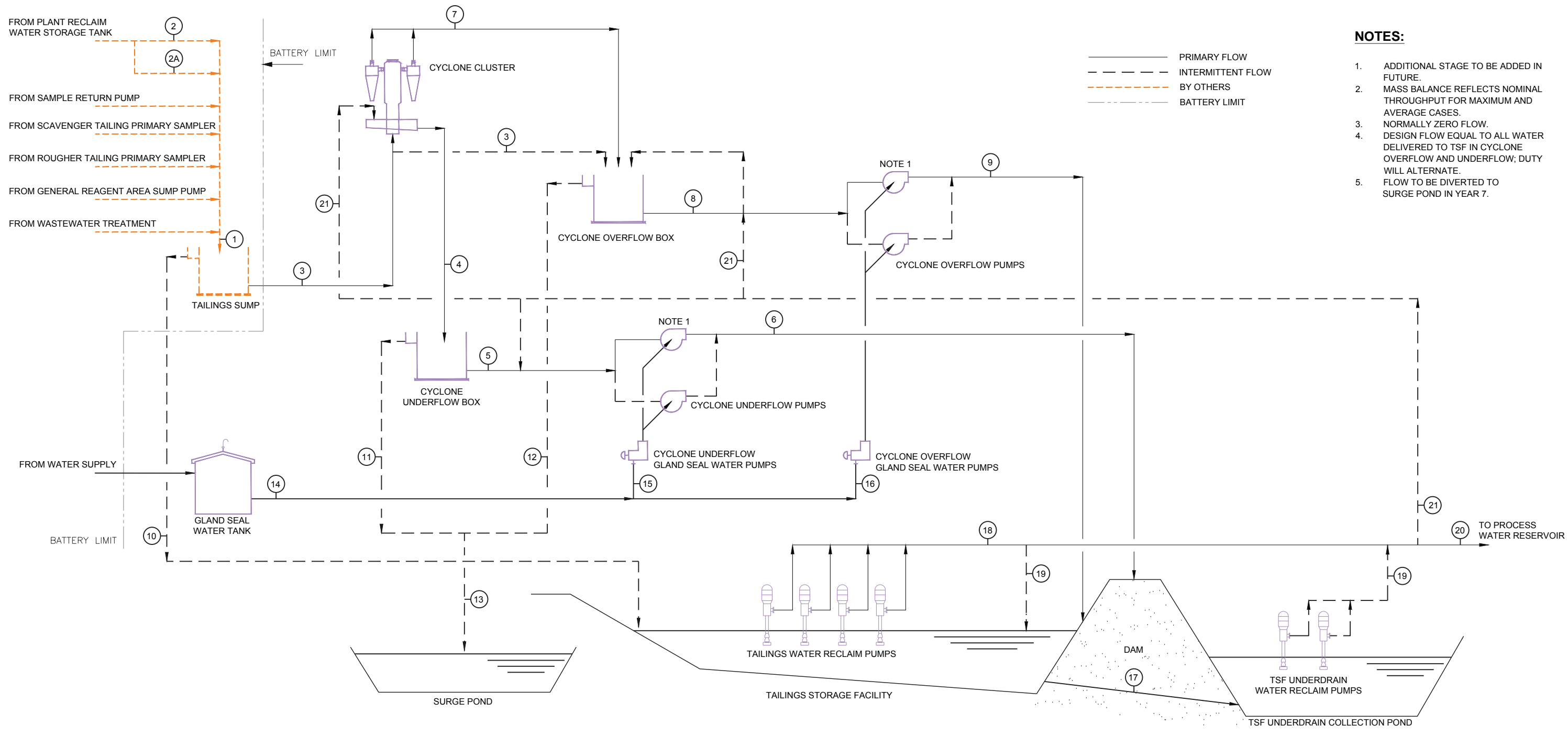
**COPPER FLAT PROJECT**  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS**

PROJECT No.	103-92557	FILE No.	10392557K015
DESIGN	DW	2013-05-03	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K015.dwg | Layout: 22 SURGE POND PLAN CROSS-SECTIONS AND DETAILS | Modified: tccasocio 11/12/2015 7:55 AM | Plotter: JRange | 11/13/2015



- NOTES:**
1. ADDITIONAL STAGE TO BE ADDED IN FUTURE.
  2. MASS BALANCE REFLECTS NOMINAL THROUGHPUT FOR MAXIMUM AND AVERAGE CASES.
  3. NORMALLY ZERO FLOW.
  4. DESIGN FLOW EQUAL TO ALL WATER DELIVERED TO TSF IN CYCLONE OVERFLOW AND UNDERFLOW; DUTY WILL ALTERNATE.
  5. FLOW TO BE DIVERTED TO SURGE POND IN YEAR 7.

— PRIMARY FLOW  
 - - - - - INTERMITTENT FLOW  
 . . . . . BY OTHERS  
 - - - - - BATTERY LIMIT

K:\2010 Projects\103-92557\Copper\Final\Facility Design\10392557K023.dwg | Layout: 23 GENERAL PROCESS FLOW DIAGRAM | Modified: nicascos 11/11/2015 6:09 PM | Plotted: J.Rangel 11/13/2015

Note 2

	Tailings Sump Feed	Tailings Sump Flush Water Note 3	Dilution Water Note 3	Tailings Sump Outlet/ Cyclone Inlet	Cyclone Underflow w	Cyclone Underflow Box Discharge	Cyclone Underflow Pump Discharge	Cyclone Overflow	Cyclone Overflow Box Discharge	Cyclone Overflow Pump Discharge	Tailings Sump Overflow Note 3, 5	Cyclone Underflow Box Overflow Note 3	Cyclone Overflow Box Overflow Note 3	Cyclone Area Surge Discharge Note 3	Gland Seal Water Supply Header	Cyclone Underflow Pump Gland Seal Water	Cyclone Overflow Pump Gland Seal Water	TSF Underflow	TSF Reclaim Water Note 4	TSF Underdrain Collection Pond Reclaim Water Note 4	Reclaim Water to Plant	Cyclone Area Flush Water from TSF Note 3
Stream	1	2	2A	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21
TPD (solids)	32000			32000	14550	14550	14550	17450	17450	17450	32000	14550	32000	32000								
Solids (t/h)	1333			1333	606	606	606	727	727	727	1333	606	1333	1333								
Solution (t/h)	3249	3755	0	3249	260	260	264	2989	2989	3001	3249	260	3249	3249	17	4	13	3267	3267	1001	3267	490
Slurry (t/h)	4582			4582	866	866	870	3716	3716	3728	4582	866	4582	4582								
Solids (%) Cw	29.1%			29.1%	70.0%	70.0%	69.7%	19.6%	19.6%	19.5%	29.1%	70.0%	29.1%	29.1%								
Solids (gpm)	2018			2018	918	918	918	1101	1101	1101	2018	918	2018	2018								
Solution (gpm)	12981	14999	0	12981	1038	1038	1054	11943	11943	11993	12981	1038	12981	12981	66	16	50	13047	13047	4000	13047	1956
Slurry (gpm)	14999			14999	1956	1956	1972	13044	13044	13094	14999	1956	14999	14999								
Solids (%) Cv	13.5%			13.5%	46.9%	46.9%	46.5%	8.4%	8.4%	8.4%	13.5%	46.9%	13.5%	13.5%								
Solids SG	2.64			2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64								
Solution SG	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Slurry SG	1.22			1.22	1.77	1.77	1.76	1.14	1.14	1.14	1.22	1.77	1.77	1.22								

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	MJG	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

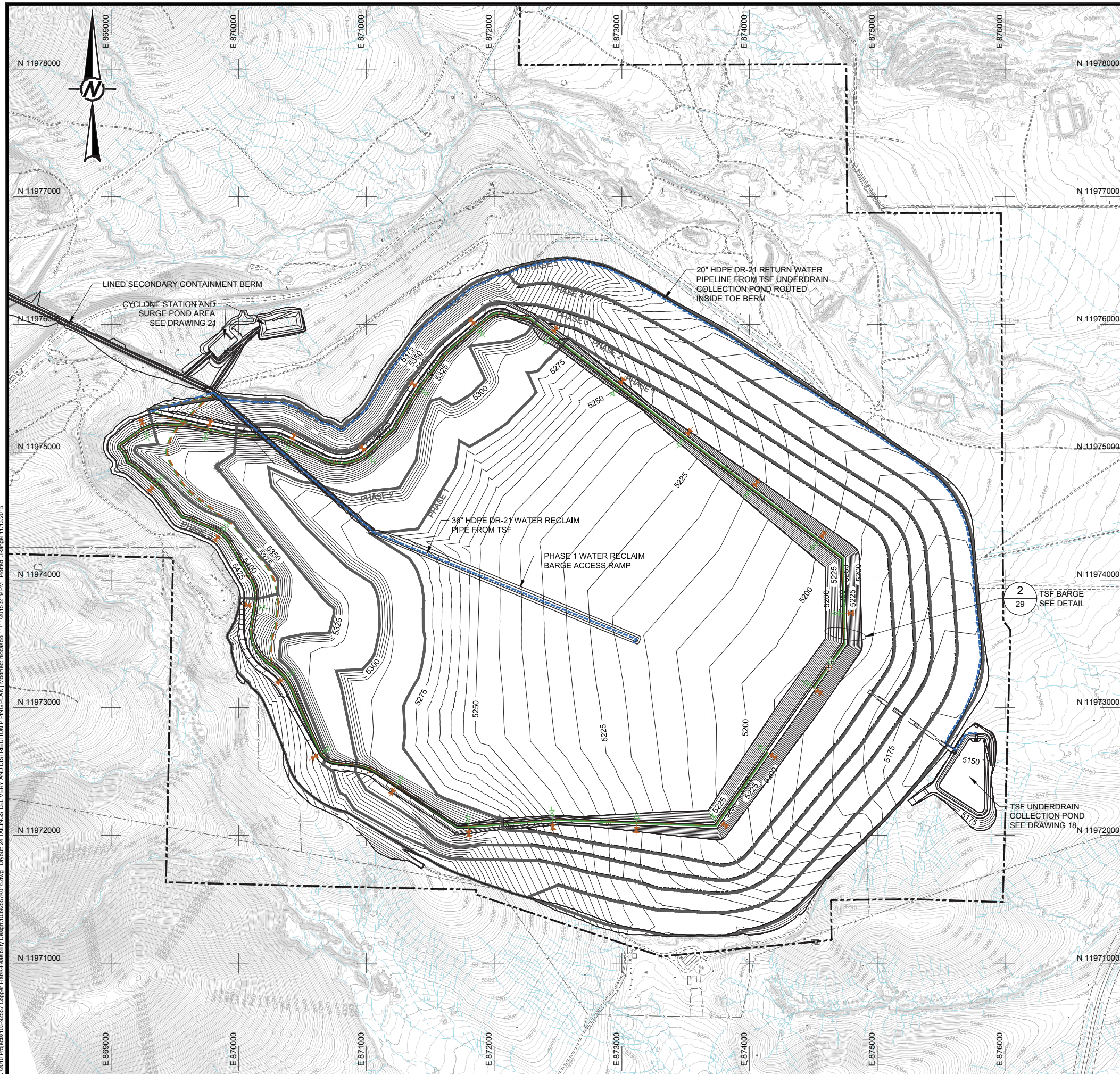
PROJECT  
**THEMAC** RESOURCES  
 NEW MEXICO COPPER CORPORATION  
 Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

PROJECT No.	103-92557	FILE No.	10392557K023	
DESIGN	DMW	2013-04-08	SCALE	NTS
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		



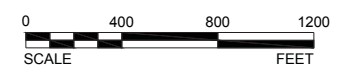
K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K016.dwg | Layout: 24 TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN | Modified: 11/11/2015 5:19 PM | Plotter: JRange1 11/13/2015



**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- PHASE 1 PHASE BOUNDARY
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - PERMANENT ALIGNMENT
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - TEMPORARY ALIGNMENT
- 12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - FINAL ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - PERMANENT ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - TEMPORARY ALIGNMENT
- 30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - FINAL ALIGNMENT
- PINCH VALVES
- KNIFE GATES
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION

- NOTES**
- PHASE 1 TAILINGS DISTRIBUTION BEGINS AT EL: 5250 AND ENDS AT EL: 5280.
  - KNIFE GATE VALVES ON CYCLONE UNDERFLOW AND CYCLONE OVERFLOW DISTRIBUTION PIPES AT 2000 ft SPACING. (SHOWN SPACED FOR CLARITY)
  - SPIGOTS ON CYCLONE OVERFLOW PIPE AT 660 ft SPACING. (SHOWN SPACED FOR CLARITY)
  - SPIGOTS ON CYCLONE UNDERFLOW PIPE AT 330 ft SPACING. (SHOWN SPACED FOR CLARITY)



2  
29  
TSE BARGE  
SEE DETAIL

TSE UNDERDRAIN  
COLLECTION POND  
SEE DRAWING 18

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

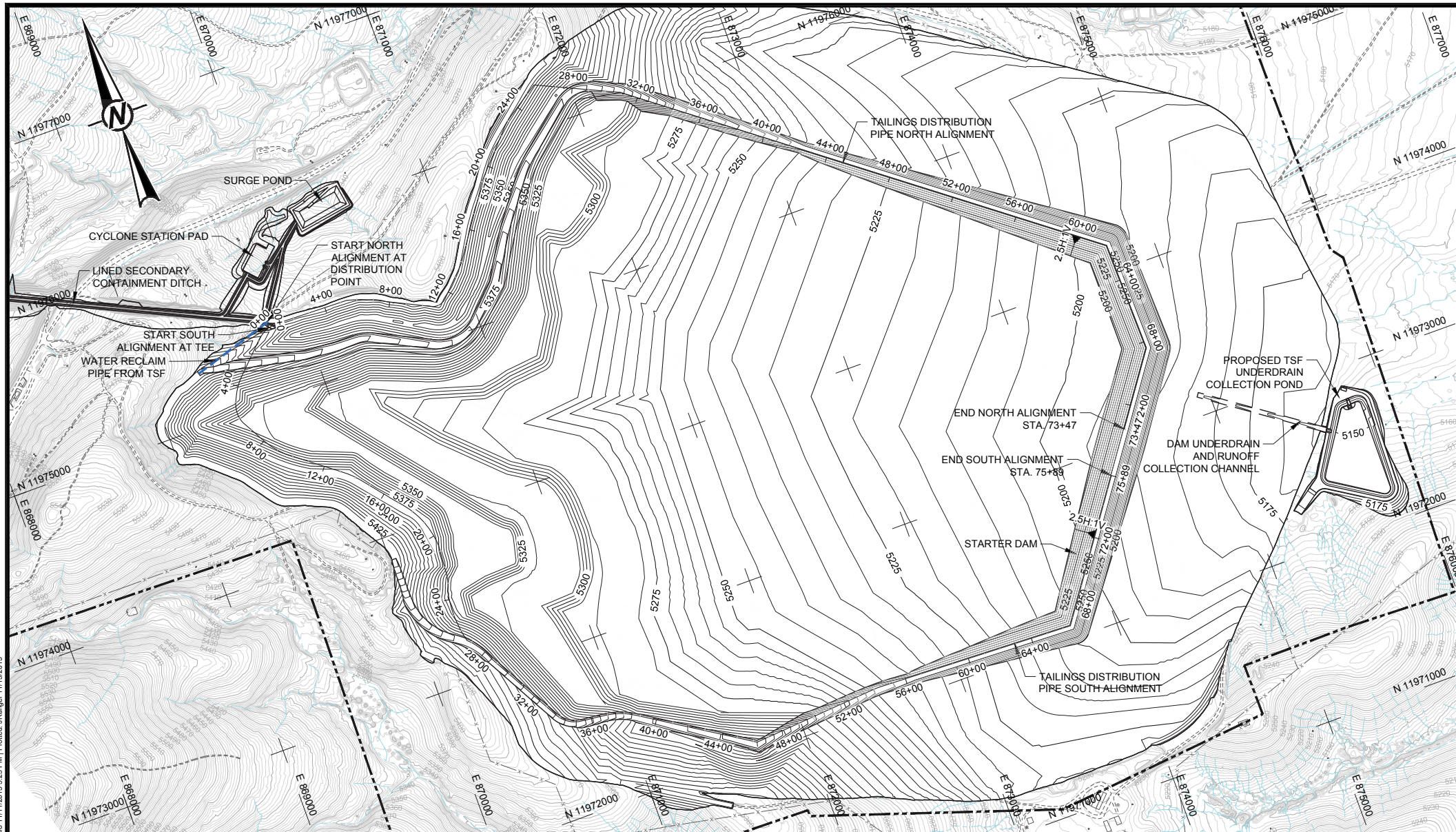
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN**

PROJECT No.	103-92557	FILE No.	10392557K016
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

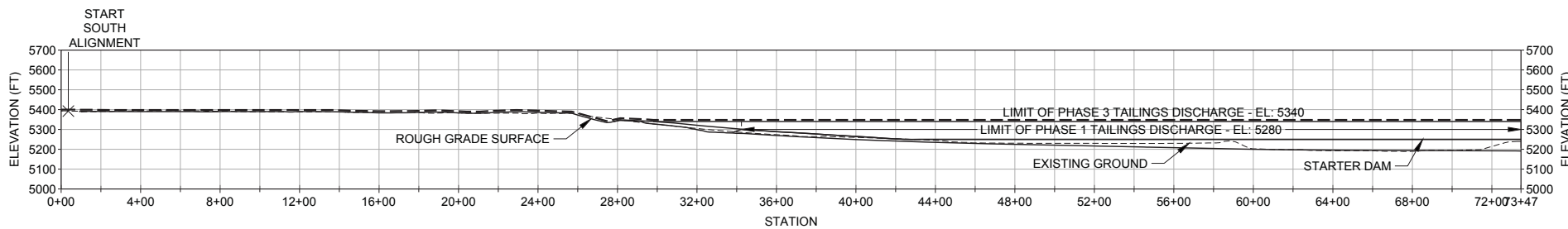




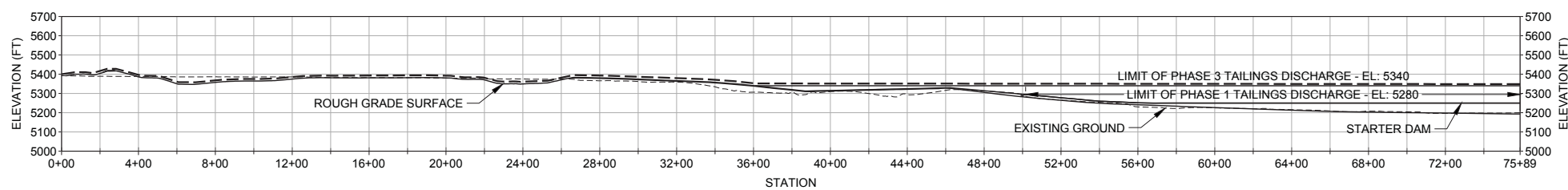
PHASES 1-3 TAILINGS DISTRIBUTION PLAN  
SCALE A

**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- $3H:1V$  or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- TAILINGS DELIVERY AND DISTRIBUTION PIPE AT PHASE 3 LIMITS
- WATER RECLAIM PIPE



PHASES 1-3 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE  
SCALE A



PHASES 1-3 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE  
SCALE A

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT: **COPPER FLAT PROJECT**  
**THEMAC** RESOURCES INC. NEW MEXICO COPPER CORPORATION  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

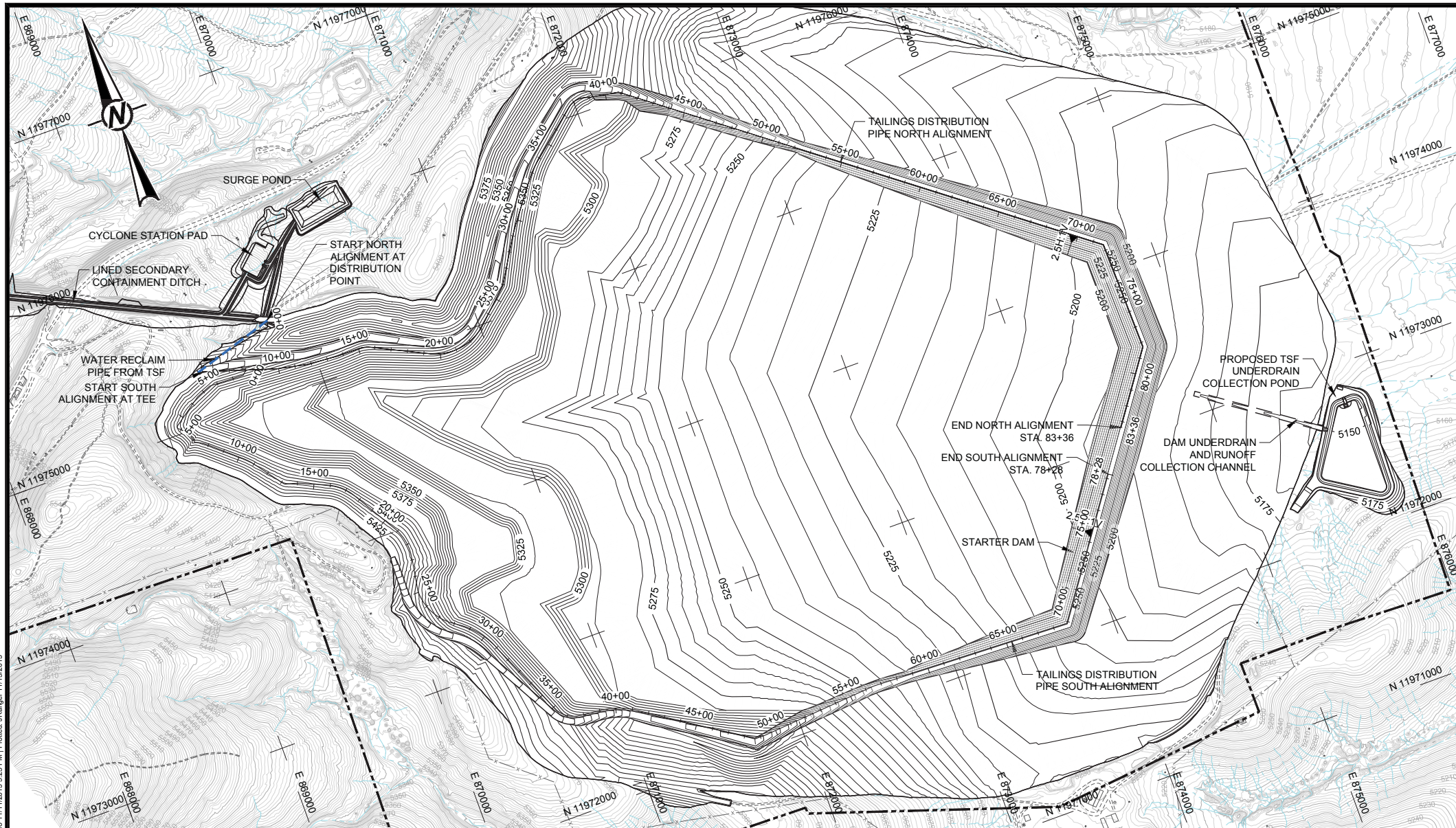
TITLE: **TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K017
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K017.dwg | Layout: 25 TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2) | Modified: 11/11/2015 5:25 PM | Plotted: J.Rangel 11/13/2015

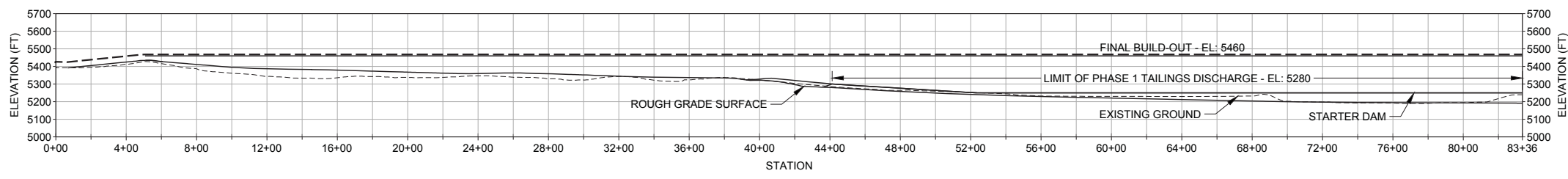




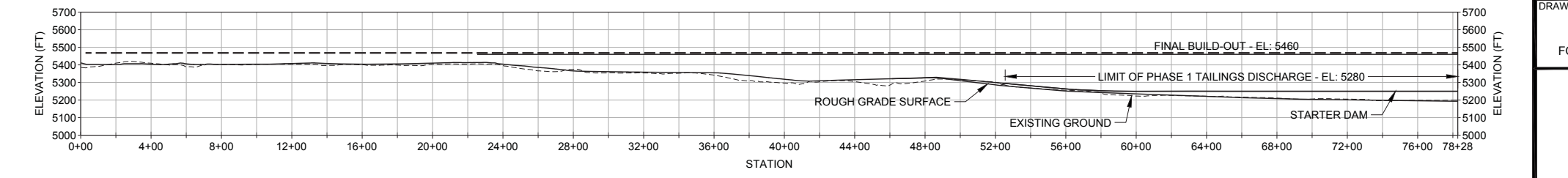
**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- TAILINGS DELIVERY AND DISTRIBUTION PIPE AT FINAL BUILD-OUT
- WATER RECLAIM PIPE

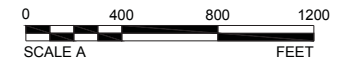
PHASES 4-5 TAILINGS DISTRIBUTION PLAN  
SCALE A



PHASES 4-5 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE  
SCALE A



PHASES 4-5 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE  
SCALE A



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT: COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

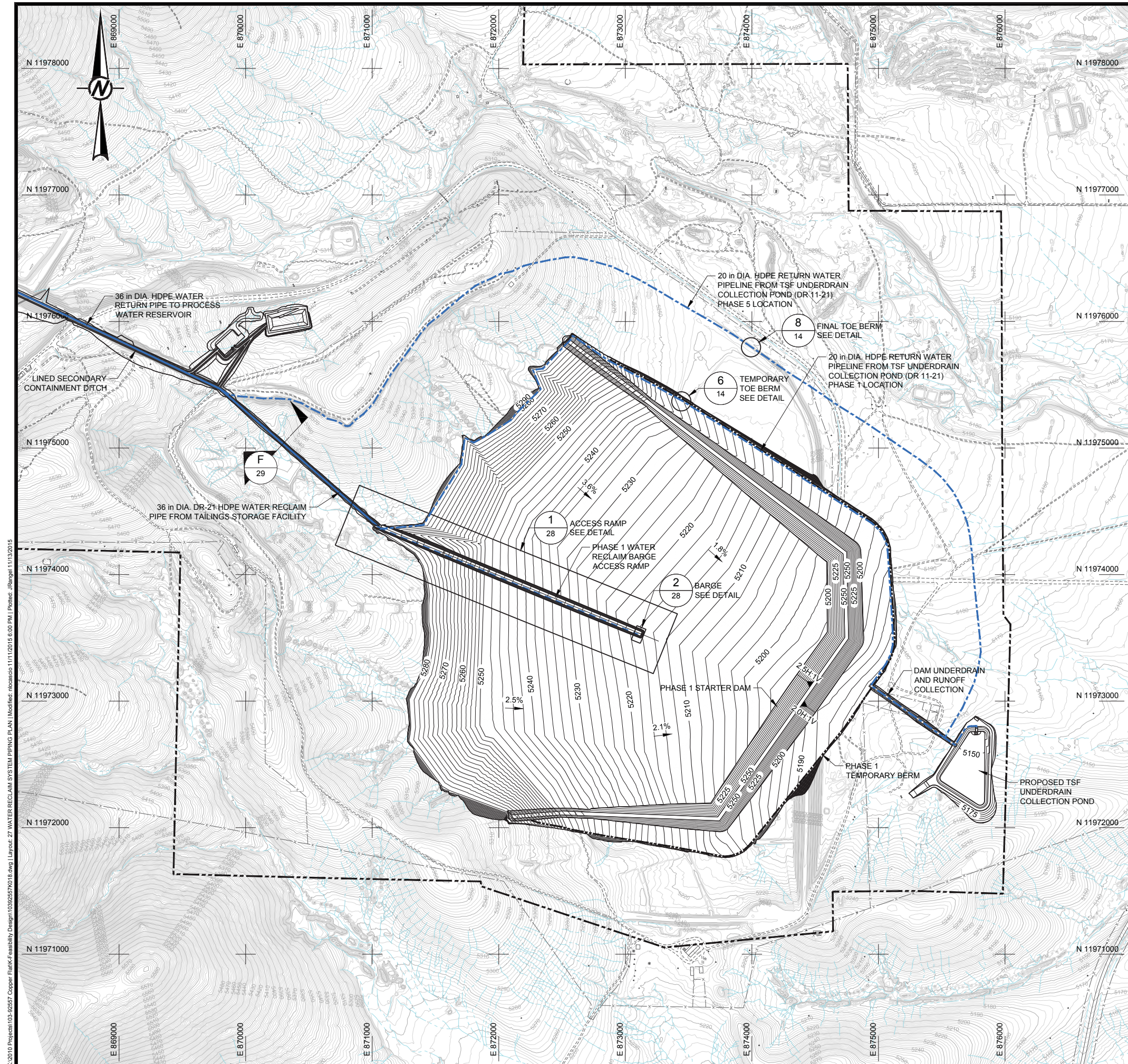
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

TITLE  
**TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K017
DESIGN	DMW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



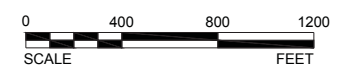
K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K017.dwg | Layout: 26 TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2) | Modified: 11/11/2015 5:25 PM | Plotted: J:\Range | 11/13/2015



**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- INTERMEDIATE TOE BERM CENTERLINE
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1 14 DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION

- NOTES**
1. SEE DRAWING 29 FOR RETURN WATER PIPE DITCH SECTIONS AND DETAILS.
  2. RETURN WATER PIPELINE FROM TSF UNDERDRAIN AND RUNOFF COLLECTION POND WILL BE ROUTED INSIDE THE EMBANKMENT TOE BERM AND WILL REQUIRE RELOCATION AT EACH CONSTRUCTION PHASE.
  3. RETURN WATER PIPELINE WILL BE RELOCATED TO THE TOE BERM AT EACH PHASE OF BUILDOUT. PHASE 1 AND PHASE 5 LOCATIONS ARE SHOWN ON DRAWING.



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

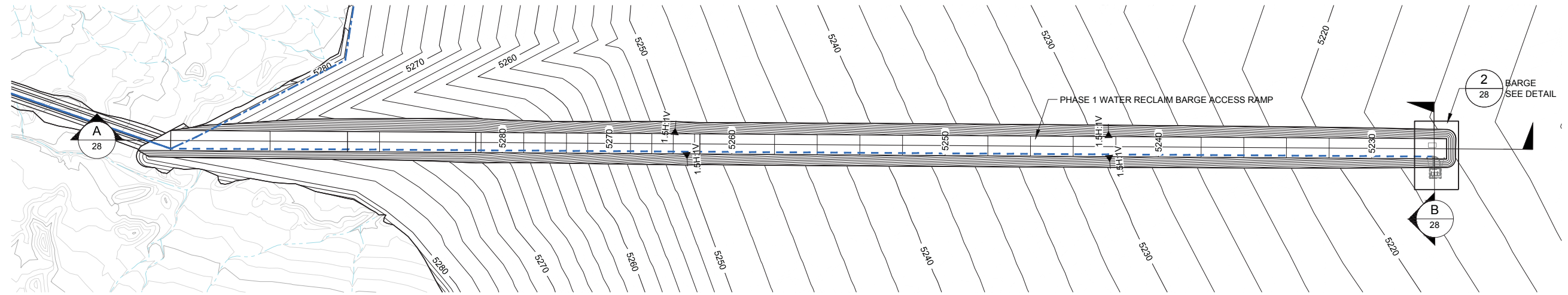
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**WATER RECLAIM SYSTEM PIPING PLAN**

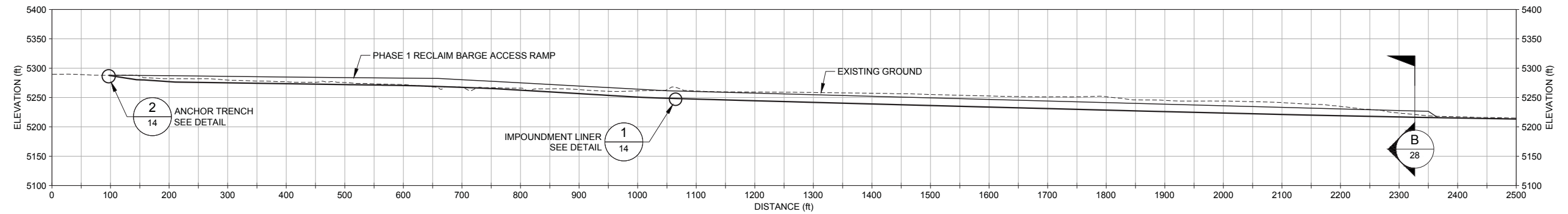
PROJECT No.	103-92557	FILE No.	10392557K018
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



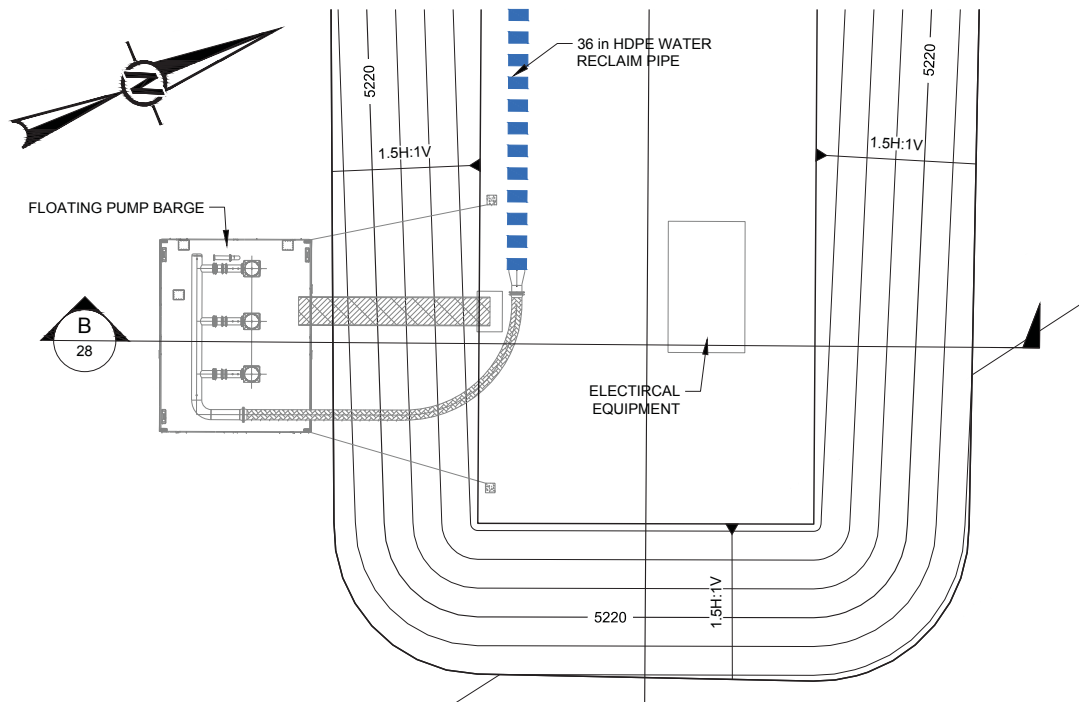
K:\2010 Projects\103-92557 Copper Flat\KFeasibility Design\10392557K018.dwg | Layout: 27 WATER RECLAIM SYSTEM PIPING PLAN | Modified: nbarceno 11/11/2016 6:00 PM | Plotter: JReigel 11/13/2015



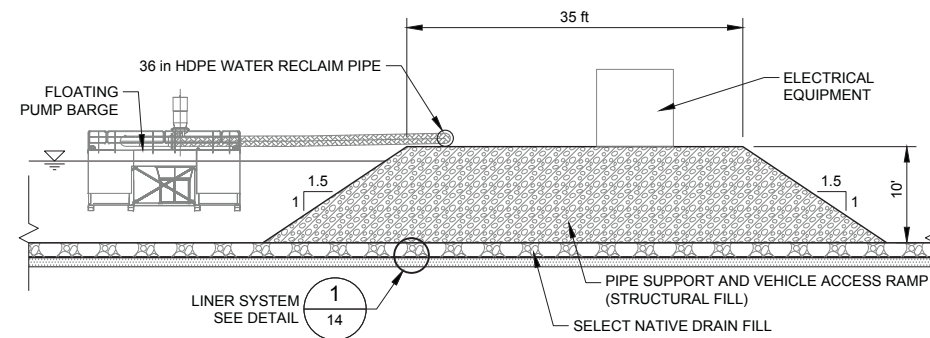
SCALE A **1** PIPE RAMP DETAIL  
28



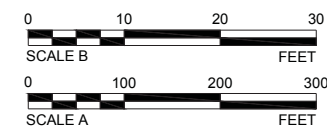
SCALE A **A** PIPE RAMP SECTION  
28



SCALE B **2** RECLAIM BARGE DETAIL  
28



N.T.S. **B** RECLAIM BARGE SECTION  
28



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

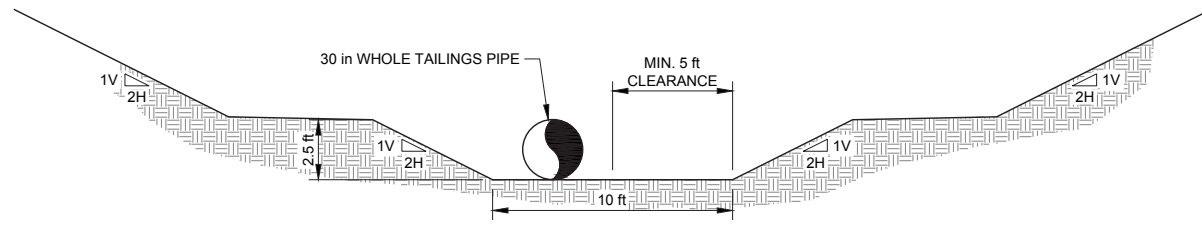
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

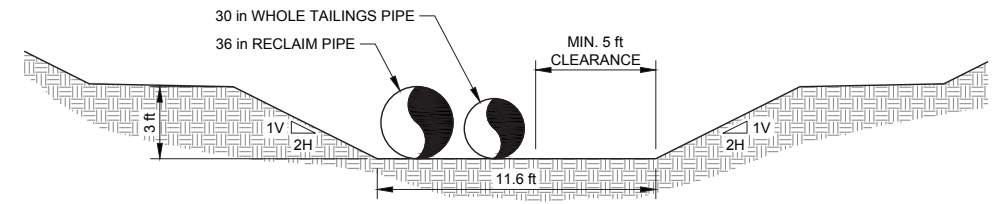
TITLE			
<b>WATER RECLAIM SYSTEM DETAILS</b>			
PROJECT No.	103-92557	FILE No.	10392557K018
DESIGN	DW	2013-04-08	SCALE NOT TO SCALE
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



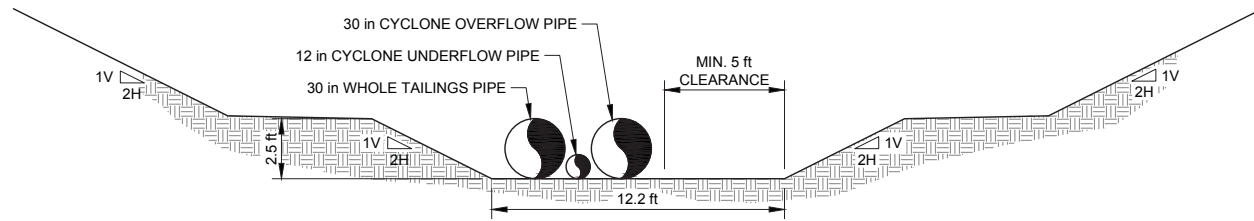
K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K022.dwg | Layout: 20 TAILINGS DISTRIBUTION SECONDARY CONTAINMENT DETAILS AND SECT | Modified: mcaasoo 11/11/2015 6:08 PM | Plotted: Rangal 11/13/2015



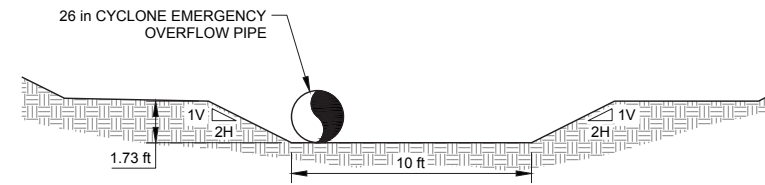
SCALE: N.T.S. **A** 29  
SECONDARY CONTAINMENT MAIN DITCH ABOVE PROCESS WATER RESEVOIR



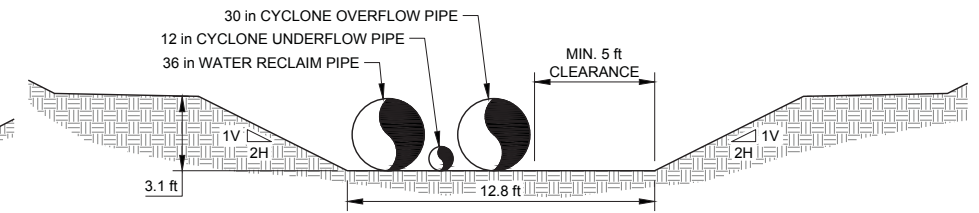
SCALE: N.T.S. **B** 29  
MAIN DITCH BELOW PROCESS WATER RESEVOIR



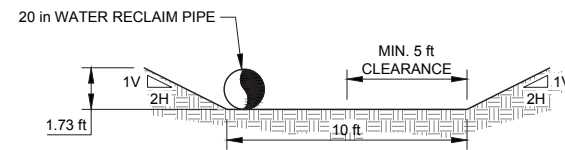
SCALE: N.T.S. **C** 29  
MAIN DITCH TO CYCLONE PLANT



SCALE: N.T.S. **D** 29  
CYCLONE PLANT TO SURGE POND DITCH



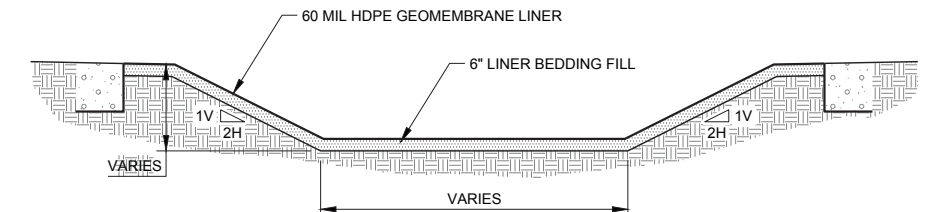
SCALE: N.T.S. **E** 29  
MAIN DITCH BELOW CYCLONE PLANT



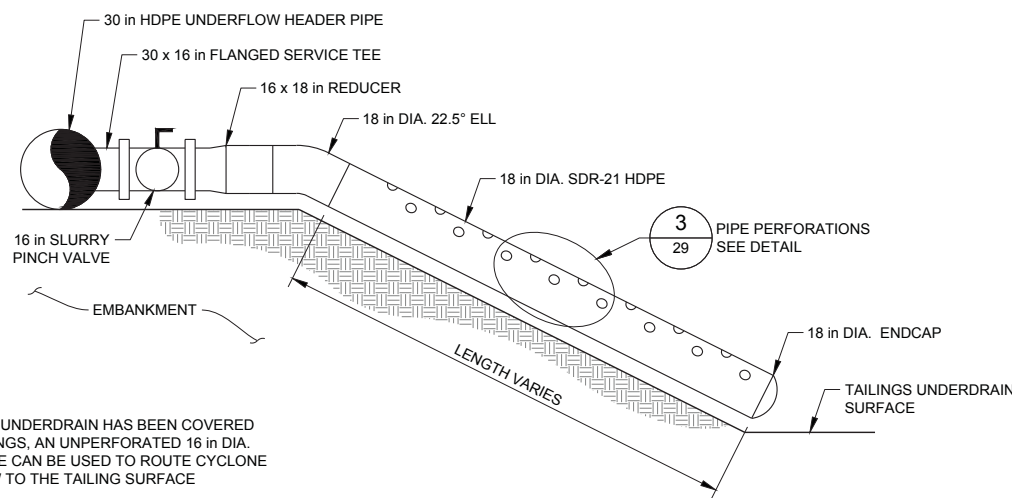
SCALE: N.T.S. **F** 29  
IN TSF RECLAIM PIPE DITCH



SCALE: N.T.S. **G** 29  
POST YEAR 6 SURGE POND DITCH

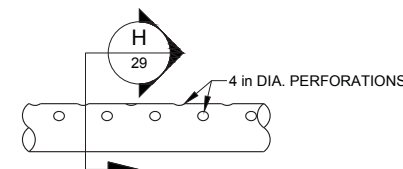


SCALE: N.T.S. **1** 29  
TYPICAL SECONDARY CONTAINMENT LINER

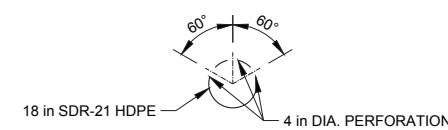


NOTE:  
WHEN THE UNDERDRAIN HAS BEEN COVERED WITH TAILINGS, AN UNPERFORATED 16 in DIA. SDR-32 PIPE CAN BE USED TO ROUTE CYCLONE OVERFLOW TO THE TAILING SURFACE

SCALE: N.T.S. **2** 29  
ENERGY DISSIPATOR/SCOUR PROTECTION CYCLONE OVERFLOW DISTRIBUTION SYSTEM



SCALE: N.T.S. **3** 29  
PIPE PERFORATION DETAIL



SCALE: N.T.S. **H** 29  
PIPE PERFORATION SECTION

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	MJG	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS**

PROJECT No.	103-92557	FILE No.	10392557K022
DESIGN	DMW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



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Europe	+ 356 21 42 30 20
North America	+ 1 800 275 3281
South America	+ 55 21 3095 9500

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Engineering Earth's Development, Preserving Earth's Integrity

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