





**NEW MEXICO ENVIRONMENT DEPARTMENT  
INSPECTION OF PUBLIC RECORD REQUEST FORM**

Please fill out the following information:

1. Date: July 19, 2017
2. Requestor's Name: Jaimie Park, New Mexico Environmental Law Center
3. Requestor's Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505  
\_\_\_\_\_
4. Phone No.: (505) 989-9022
5. Email: jpark@nmelc.org
6. Company Being Represented: New Mexico Environmental Law Center
7. Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505
8. Document or File being requested to be reviewed or copied (please describe the records in sufficient detail to enable Department personnel to reasonably identify & locate the records:

I request that you inform me what documents are available within the scope of this request and when I can inspect those documents. I also request that you not make any copies without first informing me of the number of copies and the cost for the copying that are involved. Finally, I request that if you determine that any documents or portions of documents are exempt from disclosure you inform me of that and provide me with citations to the provisions in the Inspection of Public Records Act that indicate that the documents or portions of documents are exempt from disclosure, describe the type of document being withheld, and identify who sent the document being withheld and who received the document being withheld.

**Records being requested:**

1. NMED's technical completeness determination or additional request for information on New Mexico Copper Corporation's discharge permit application, date range June 28, 2017 through July 19, 2017;
2. NMED determination regarding Copper Flat Copper Mine pit lake being a private water, date range June 28, 2017 through June 19, 2017;
3. Bureau of Land Management request to the Surface Water Quality Bureau to write a letter of concurrence for the Surface Water Quality Bureau's benefit regarding the proposed Copper Flat Copper Mine pit lake and associated patented claims surveys completed and a

finding that the pit lake constitutes a “private water” and is not subject to either surface or groundwater quality standards; and

4. New Mexico Copper Corporation submittals to NMED pertaining to its discharge permit application since June 28, 2017.

9. NMED Bureau where Document/File can be found (if known): Surface Water Quality Bureau, Ground Water Quality Bureau

/s/ Jaimie Park

Signature

The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: melissa.mascareñas@state.nm.us**





SUSANA MARTINEZ  
Governor  
JOHN A. SANCHEZ  
Lt. Governor


NEW MEXICO  
ENVIRONMENT DEPARTMENT

Harold Runnels Building  
1190 Saint Francis Drive (87505)  
PO Box 5469, Santa Fe, NM 87502-5469  
Phone (505) 827-2990 Fax (505) 827-1628  
www.env.nm.gov



BUTCH TONGATE  
Cabinet Secretary  
J.C. BORREGO  
Deputy Secretary

MEMORANDUM

To: Shelly Lemon, Chief, Surface Water Quality Bureau  
Michelle Hunter, Chief, Ground Water Quality Bureau  
From:  Melissa Y. Mascareñas, Department Public Records Custodian  
Date: September 6, 2017  
Subject: Request to Inspect Public Records

We have received a request from Ms. Jaimie Park asking for information regarding:

SEE ATTACHED REQUEST.

The Inspection of Public Records Act requires a response to a requester of public records within fifteen (15) calendar days from receipt of a request. Please respond to the requestor by no later than **September 21, 2017**.

Your response may take several forms:

- a) Provide the requested information; or
- b) Notify the requester of a delay; you must give reasons for the delay and the date when the information will be available; or
- c) Deny the request or part of it; provide the records that can be released and identify the reason(s) for denial of any records; or
- d) Ask for more information or clarification; and
- e) Notify the requester of any mailing or photocopy charges.

A copy of my initial response to this request is attached for your records. **Please provide me with a copy of any responses you make to this request and/or notify me when the records have been made available for inspection.**



**NEW MEXICO ENVIRONMENT DEPARTMENT  
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4. New Mexico Copper Corporation submittals to NMED pertaining to its discharge permit application since June 28, 2017.

9. NMED Bureau where Document/File can be found (if known): Surface Water Quality Bureau, Ground Water Quality Bureau

/s/ Jaimie Park

Signature

The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: melissa.mascareñas@state.nm.us**







September 27, 2017

GROUND WATER

SEP 28 2017

BUREAU

Ms. Shelly Lemon, Chief  
Surface Water Quality Bureau  
New Mexico Environment Department  
1190 St. Francis Dr.  
P.O. Box 5469  
Santa Fe, New Mexico 87502

RE: BLM Review and Concurrence with NMCC-Commissioned Surveys in the Area of the Copper Flat Pit, Sierra County, New Mexico

Dear Ms. Lemon,

I am writing to share with you the results of the Bureau of Land Management (BLM) review of New Mexico Copper Corporation (NMCC)-commissioned land surveys in the area of the current pit at Copper Flat.

Recall that NMCC noted an error in public GIS documents depicting the property boundary between public land and NMCC-owned property at the Copper Pit. This error placed the property boundary over one part of the existing pit lake, while NMCC's surveys placed the boundary approximately 300 feet to the west, which places the existing pit lake entirely on NMCC-owned private property. NMCC met with BLM on this topic and agreed to address this issue by engaging an independent registered land surveyor to prepare a property plat. During discussions, BLM also confirmed NMCC's understanding that the GIS maps available to the public do not necessarily reflect true boundary locations, and BLM thus requires that all boundaries be by survey.

The property plat for the pit area was completed and sealed by a registered land surveyor, recorded with Sierra County, and submitted to BLM for review. A scan copy of the sealed and recorded plat is enclosed for your information. BLM has reviewed the plat and has accepted the survey and platted location of the private property boundaries at the Copper Flat Pit. Please see the enclosed letter from Ida T. Viarreal, Land Law Examiner for the BLM dated September 13, 2017 and the accompanying map. As you will see, BLM has updated their Master Title Plats and have requested updates to the GIS layer for this area to conform to the findings of the NMCC survey.

The survey and plat confirm that the current pit body is entirely on private lands, as shown on the plat submitted to BLM. NMCC has proposed a design that will confine the surface of the water body in the future pit entirely to private lands.

It is our conclusion that the current and future pit water body will meet the exception in 20.6.4.7(S)(5) that excludes “private waters that do not combine with other surface or subsurface water, or any other water under tribal jurisdiction pursuant to Section 518 of the Clean Water Act.” We respectfully request that the Surface Water Quality Bureau review BLM’s conclusion and issue a determination that the current and future pit water will not be a “surface water of the state”. If you have further questions or would like to discuss this matter, please contact me at 520-991-4588 or [jsmith@themacresourcesgroup.com](mailto:jsmith@themacresourcesgroup.com).

Best regards,  
New Mexico Copper Corporation



Jeff Smith  
Chief Operating Officer

Attachments:

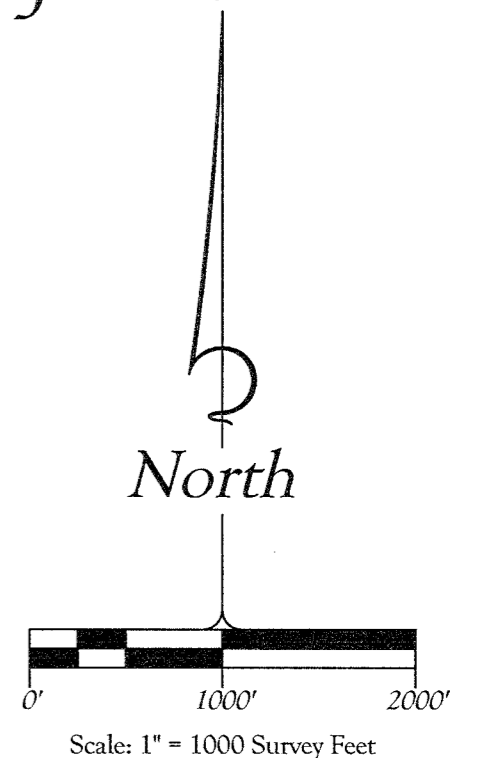
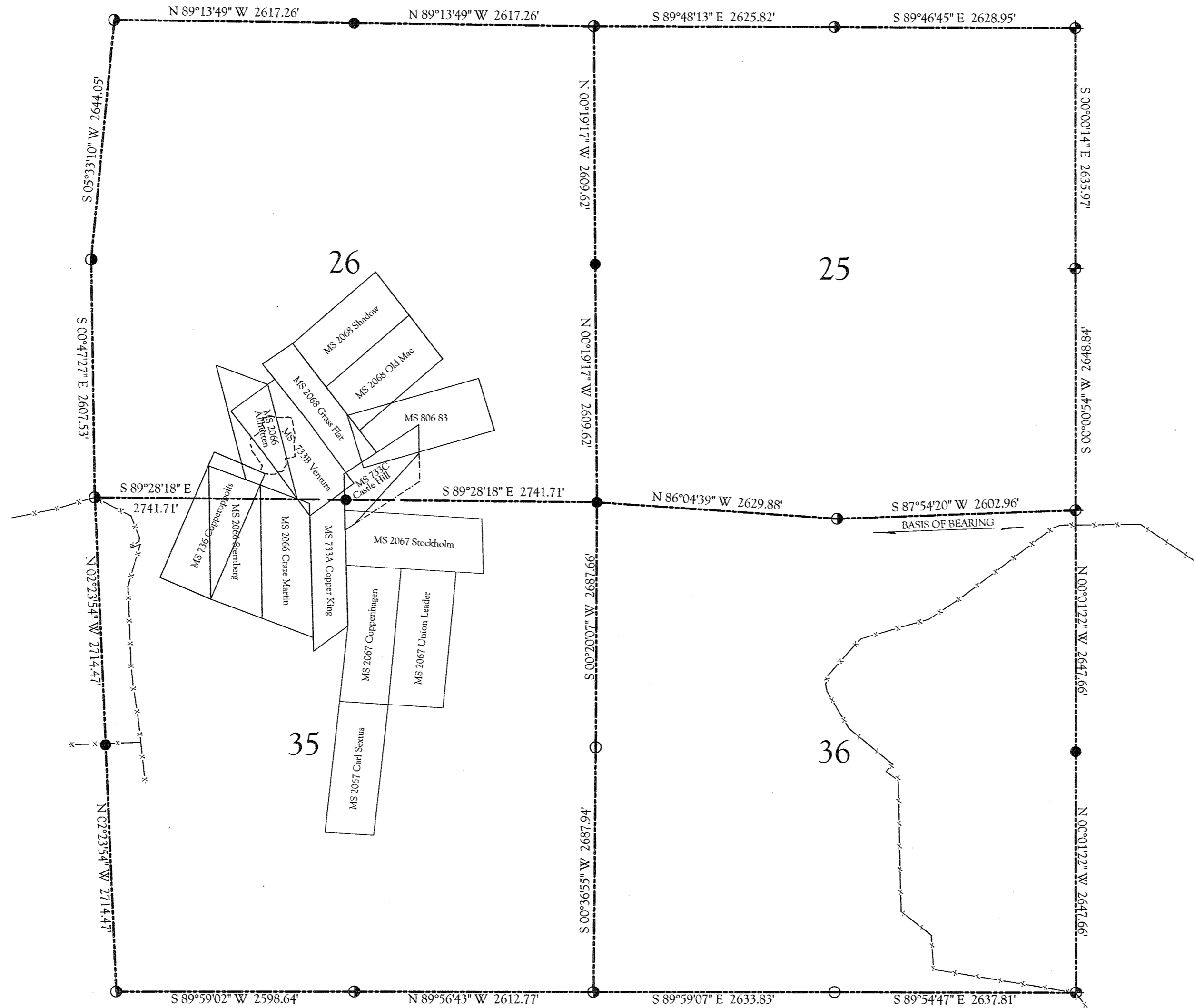
1. Copper Flat Boundary Survey Plat, March 1, 2017
2. BLM letter from Ida Viarreal, September 13, 2017
3. BLM Map Township 15 South, Range 7 West

CC: Douglas Haywood, Bureau of Land Management, Lands/Minerals Supervisor  
Leighandra Keevan, Bureau of Land Management, Geologist  
David Henney, Solv, LLC, Project Manager  
Brad Reid, New Mexico Environment Department, Groundwater Quality Bureau  
David Ennis, Energy, Minerals & Natural Resources Department, Mining & Minerals Division

# BOUNDARY SURVEY PLAT

Book 127  
Page 4193

Sections 25, 26, 35, 36 and Mineral Surveys 733A Copper King, 733B Ventura, 733C Castle Hill, 736 Copperopolis 806 83, 2066 Craze Martin, Sternberg, Allhutzen, 2067 Stockholm, Copenhagen, Union Leader, Carl Sextus, 2068 Grass Flat, Shadow and Old man, Township 15 South, Range 7 West, of the New Mexico Principal Meridian  
Sierra County, New Mexico



**BASIS OF BEARING:**  
The South line of the SE 1/4 of section 25, T. 15 S., R. 7 W., N.M.P.M. bears S. 87°54'20\" W. and is monumented as shown hereon. All other bearings are relative thereto.

- LEGEND**
- Found 3 1/4\" USGLO Brass Cap 1941. Added washer LS 7004.
  - Set 2 1/2\" alum. cap on #6 rebar, 18\" long - LS 7004.
  - USGLO stone, firmly set and properly marked. Added washer LS 7004.
  - Found 1 1/2\" aluminum cap on a #5 rebar - LS 3516. Added washer LS 7004.
  - Found 2\" aluminum cap on a 3/4\" pipe - MS 2317. Added washer LS 7004.
  - X-X- Fence Line
  - - - - Pit lake edge.

- LIST OF REFERENCE PLATS & NOTES:**
- Original Survey T. 15 S., R. 7 W., N.M.P.M. approved February 3, 1882.
  - Dependent Resurvey T. 15 S., R. 6 W., N.M.P.M. approved January 1, 1943.
  - Mineral Survey's 733A, 733B and 733C, approved September 28, 1888.
  - Mineral Survey 736, approved September 8, 1888.
  - Mineral Survey 806, approved September 16, 1889.
  - Mineral Survey 2066, approved September 22, 1937.
  - Mineral Survey 2067, approved October 11, 1937.
  - Mineral Survey 2068, approved October 30, 1937.
  - Mineral Surveys 2312, 2313, 2314, 2315, 2316 and 2317, approved December 28, 1982.

**GENERAL NOTES:**

All fence lines shown hereon are for graphical purposes only. They may not be relied upon to establish property boundaries.

This survey was performed without the benefit of a title policy or commitment and does not constitute a title search by Southwest Land Surveying & Consulting.

**Note:**  
The Mineral Surveys and sectional data as shown on this plat were re-monumented using historic information by surveys performed by Gordon W. McLain, U.S. Mineral Surveyor and LS 4796.

**CERTIFICATE OF SURVEY:**

I, Earnest E. Schaaf, New Mexico Professional Land Surveyor No. 7004, do hereby certify that this BOUNDARY SURVEY PLAT and the actual survey on the ground upon which it is based were performed by me or under my direct supervision; that I am responsible for this survey; that this survey meets the Minimum Standards for Surveying in New Mexico; and that it is true and correct to the best of my knowledge and belief. I further certify that this survey is not a land division or subdivision as defined by the New Mexico Subdivision Act and that this instrument is a BOUNDARY SURVEY PLAT of an existing tract or tracts.

Signature \_\_\_\_\_  
PLS No. 7004  
Date \_\_\_\_\_

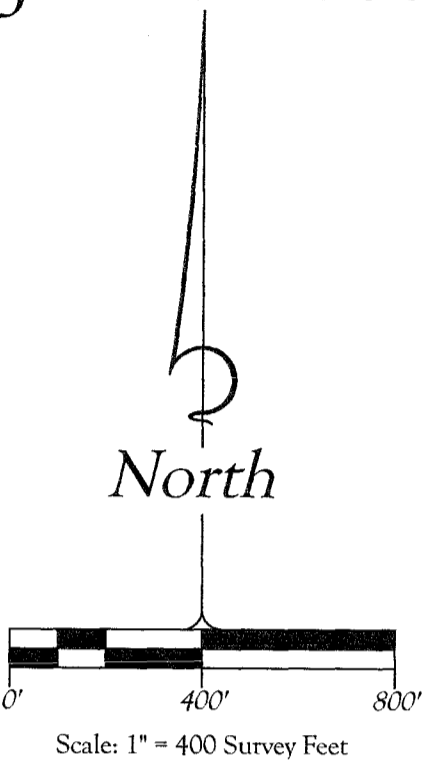
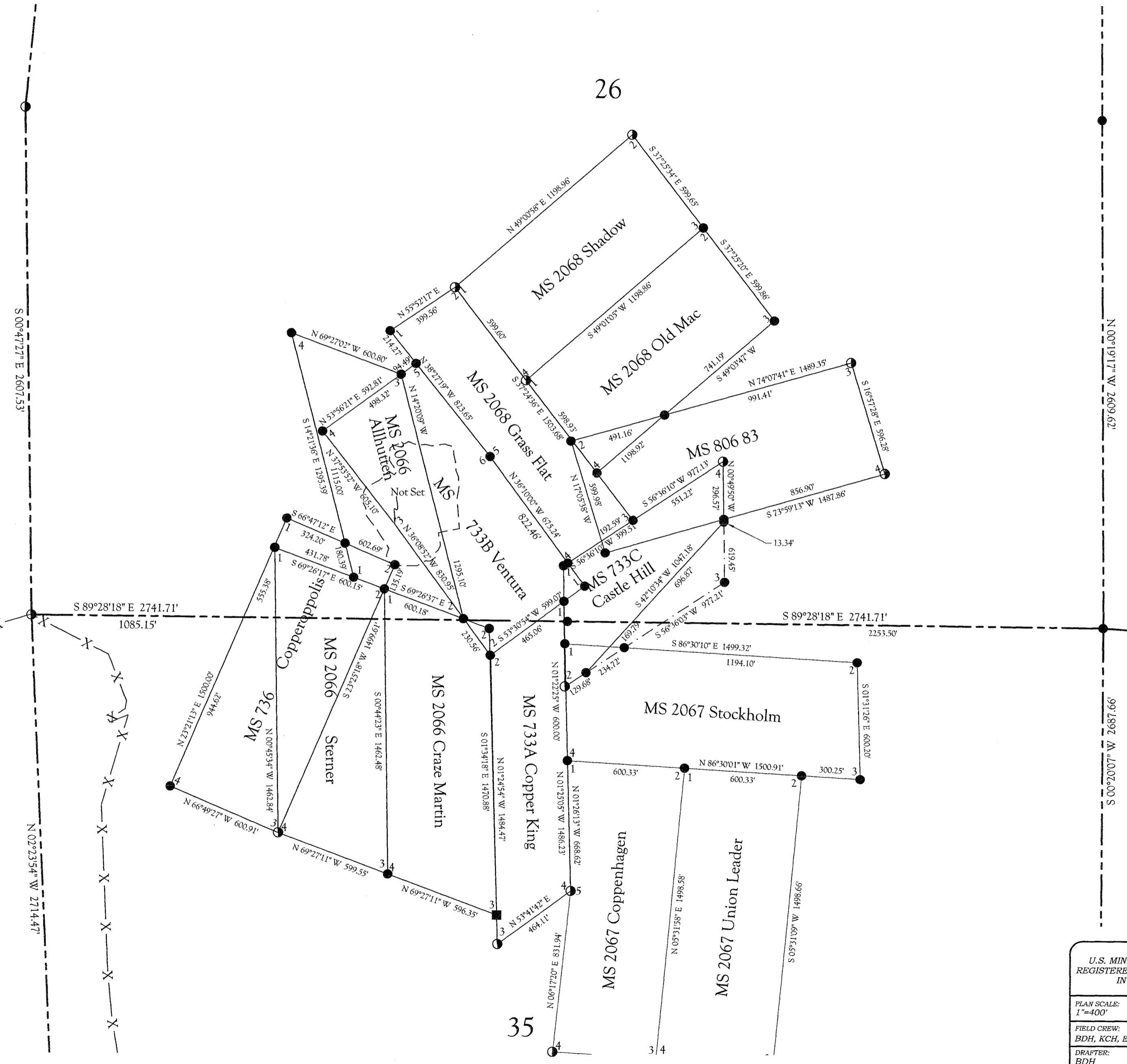
**LAND SURVEY DEPOSIT**  
Date \_\_\_\_\_  
Book \_\_\_\_\_ Page \_\_\_\_\_  
Deposit No. \_\_\_\_\_

U.S. MINERAL SURVEYORS REGISTERED LAND SURVEYORS IN COLORADO		<b>SOUTHWEST LAND SURVEYING &amp; CONSULTING</b> P.O. Box 563 Delta, CO 81416 (970) 387-0600...Silverton (970) 874-2880...Delta EMAIL: dhatter@tresources.us	
PLAN SCALE: 1"=1000' FIELD CREW: BDH, KCH, EES, TTS DRAFTER: BDH	REVISIONS: _____ _____ _____	<b>Boundary Survey Plat</b> Sections 25, 26, 35, 36 and Mineral Surveys 733A, 733B, 733C, 736, 806, 2066, 2067, 2068 T. 15 S., R. 7 W., N.M.P.M., Sierra County, New Mexico	<b>THEMAC RESOURCES</b> 4053 Montgomery Blvd., NE Suite 130 Albuquerque, New Mexico 87109
SHEET 1 of 3		JOB#: 25-16 NMCC	

# BOUNDARY SURVEY PLAT

Book 127  
Page 4194

Sections 25, 26, 35, 36 and Mineral Surveys 733A Copper King, 733B Ventura, 733C Castle Hill, 736 Copperopolis  
806 83, 2066 Craze Martin, Sternberg, Allhutzen, 2067 Stockholm, Coppenhagen, Union Leader, Carl Sextus, 2068  
Grass Flat, Shadow and Old man, Township 15 South, Range 7 West, of the New Mexico Principal Meridian  
Sierra County, New Mexico



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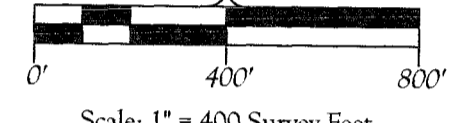
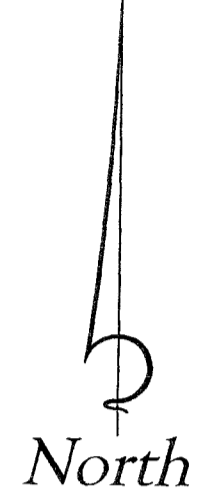
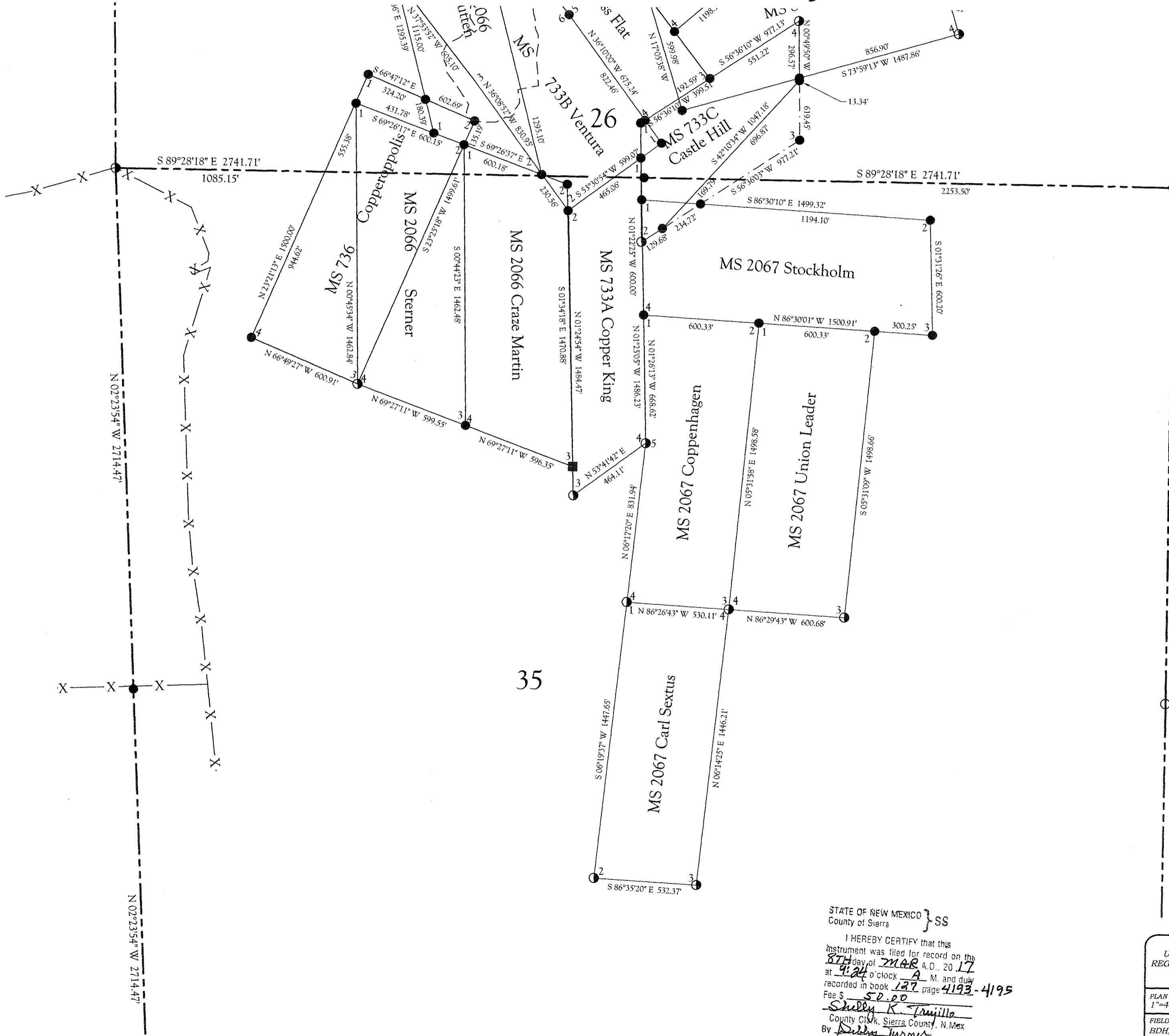
**LAND SURVEY DEPOSIT**  
Date \_\_\_\_\_  
Book \_\_\_\_\_ Page \_\_\_\_\_  
Deposit No. \_\_\_\_\_

U.S. MINERAL SURVEYORS REGISTERED LAND SURVEYORS IN COLORADO		<b>SOUTH WEST LAND SURVEYING &amp; CONSULTING</b> P.O. Box 563 Delta, CO 81416 (970) 387-0600...Siveroni (970) 874-2880...Delta EMAIL: dthatter@itresources.us	
PLAN SCALE: 1"=400'	REVISIONS:	<b>Boundary Survey Plat</b> Sections 25, 26, 35, 36 and Mineral Surveys 733A, 733B, 733C, 736, 806, 2066, 2067, 2068 T. 15 S., R. 7 W., N.M.P.M., Sierra County, New Mexico	<b>THEMAC RESOURCES</b> 4053 Montgomery Blvd., NE Suite 130 Albuquerque, New Mexico 87109
FIELD CREW: BDH, KCH, EES, TTS			
DRAFTER: BDH			
SHEET 2 of 3			
		FW: 1/18/17	JOB #: 25-16 NMCC

# BOUNDARY SURVEY PLAT








Book 127  
Page 4195

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LAND SURVEY DEPOSIT  
Date \_\_\_\_\_  
Book \_\_\_\_\_ Page \_\_\_\_\_  
Deposit No. \_\_\_\_\_

STATE OF NEW MEXICO } SS  
County of Sierra }  
I HEREBY CERTIFY that this instrument was filed for record on the 8th day of MAR A.D. 20 17 at 9:24 o'clock A.M. and duly recorded in book 127 page 4193-4195 Fee \$ 50.00  
*Shelley K. Trujillo*  
County Clerk, Sierra County, N.Mex  
By *Duffy Jansen* Deputy

U.S. MINERAL SURVEYORS REGISTERED LAND SURVEYORS IN COLORADO		SOUTH WEST LAND SURVEYING & CONSULTING P.O. Box 563 Delta, CO 81416 (970) 387-0600...Silverton (970) 874-2880...Delta EMAIL: dthatter@itresources.us	
PLAN SCALE: 1"=400'	REVISIONS:	Boundary Survey Plat Sections 25, 26, 35, 36 and Mineral Surveys 733A, 733B, 733C, 736, 806, 2066, 2067, 2068 T. 15 S., R. 7 W., N.M.P.M., Sierra County, New Mexico	THEMAC RESOURCES 4053 Montgomery Blvd., NE Suite 130 Albuquerque, New Mexico 87109
FIELD CREW: BDH, KCH, EES, TTS			
DRAFTER: BDH			
SHEET 3 of 3		FW: 1/18/17 JOB #: 25-16 NMCC	



## United States Department of the Interior

### BUREAU OF LAND MANAGEMENT

New Mexico State Office  
301 Dinosaur Trail  
P.O. Box 27115  
Santa Fe, New Mexico 87502-0115  
[www.blm.gov/nm](http://www.blm.gov/nm)



In Reply Refer To:  
3833 (NM9212-itv)

September 13, 2017

Jeff Smith  
Thermac Resources  
New Mexico Copper Corp.  
4253 Montgomery Blvd. NE suite 130  
Albuquerque, NM 87109

Dear Mr. Smith:

Based on your request we have review our records for the area of the Copper Flat Mine which is located in Township 15 S., Range 7 W., Sections 25, 26, 35 and 36.

We reviewed all the mineral surveys in the area and updated our Master Title Plats to insure that the status of the lands was noted correctly. We have also requested updates to the GIS layer for this area.

Enclosed is a revised Master Title Plat. I hope this clarifies the status of the lands in your area. We have also placed the survey completed by your surveyor in the mining claim case files and in the 3809 Plan casefiles.

If you have any questions, please call me at (505) 954-2163; facsimile at (505) 954-2079; or write to the attention of NM9212 at the BLM address above or e-mail to [iviarrea@blm.gov](mailto:iviarrea@blm.gov).

Ida T. Viarreal  
Land Law Examiner  
Solid Minerals Adjudication

# TOWNSHIP 15 SOUTH, RANGE 7 WEST, OF THE NEW MEXICO PRIN. MERIDIAN, NEW MEXICO.

## SIERRA COUNTY — 051 NM — 3

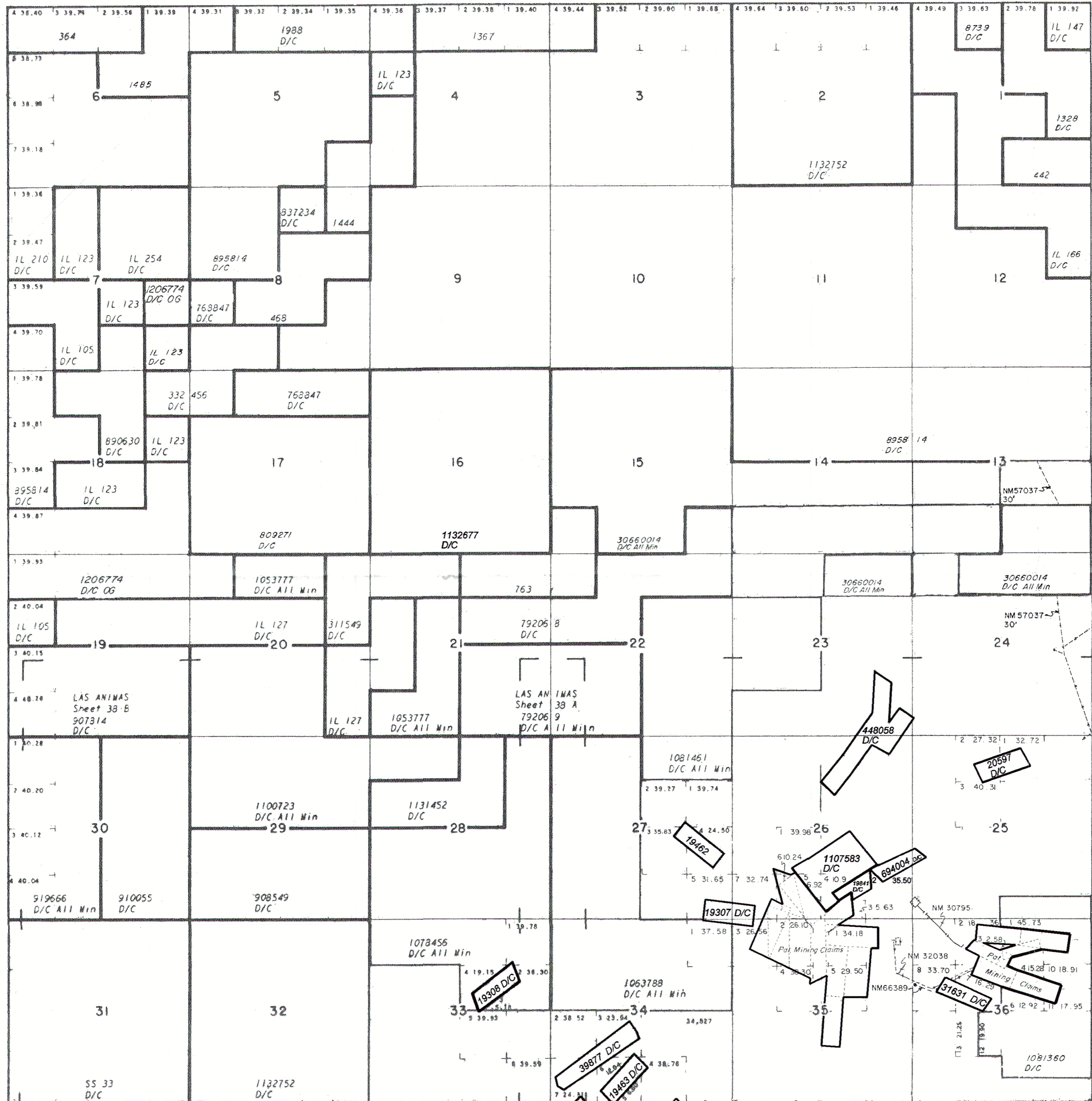
STATUS OF PUBLIC DOMAIN  
LAND AND MINERAL TITLES

### MTP

INDEX TO SEGREGATED TRACTS				
RESURVEY TRACT NO	ORIGINAL SURVEY			
	T	R	SEC	SUBDIVISION

FOR ORDERS EFFECTING DISPOSAL OR USE OF UNIDENTIFIED LANDS WITHIN FOR CLASSIFICATION, MINERALS, WATER AND/OR OTHER PUBLIC PURPOSES, REFER TO INDEX OF MISCELLANEOUS DOCUMENTS.

Sec. 36 IL Base



**WARNING STATEMENT**  
This plat is the Bureau's Record of Title and should be used only as a graphic display of the township survey data. Records hereon do not reflect title changes which may have been effected by lateral movements of rivers or other bodies of water. Refer to the cadastral surveys for official survey information.

Lat.  
Long.



CURRENT TO	BY

T 15 S  
R 7 W  
NMPM







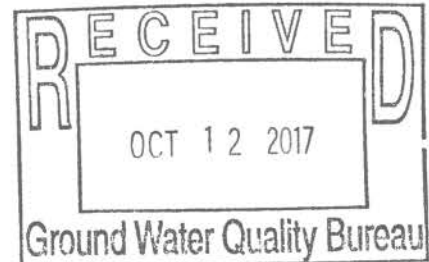
**Memorandum of Meeting or Phone Conversation**

<input type="checkbox"/> Telephone	<input checked="" type="checkbox"/> Meeting	Time: 1:30-2:30 pm	Date: 10-03-17
<b>Individuals Involved</b>			
Brad Reid, Kurt Vollbrecht, (NMED-MECS)		<input type="checkbox"/> called	Jeff Smith, Katie Emmer (New Mexico Copper Corporation "NMCC"); Juan Velasquez (NMCC consultant)
<b>Subject:</b> Discussion regarding DP Application Status			
<b>Discussion:</b> NMED met with representatives from NMCC as well as their consultant that prepared the Discharge Permit Application. NMED requested that NMCC provide additional information regarding the following:			
<ol style="list-style-type: none"> <li>1) Impact of rapid fill on the pit hydrologic evaporative sink and verification that rapid fill will not cause a flow-through pit.</li> <li>2) Water quality proposed to be used for the rapid fill effort (supply wells?).</li> <li>3) Need a figure showing pit ground water capture at closure.</li> </ol>			
<b>Conclusions:</b>			
<b>Distribution:</b> DP-1840 folder			Initialed <span style="border: 1px solid black; padding: 2px;">BR</span>



October 13, 2017

Mr. David Ennis, P.G.  
Reclamation Specialist/Permit Lead  
New Mexico Energy, Minerals and Natural Resources Department  
Mining and Minerals Division  
1220 South St. Francis Drive  
Santa Fe, NM 87505



**Re: Oct. 5, 2017 Request for Additional Information  
Updated MORP Rev.1, 2017  
New Mexico Copper Corporation, Copper Flat Mine  
Permit Tracking No. S1027RN**

Dear Mr. Ennis,

Pursuant to your request of October 5, 2017 New Mexico Copper Corporation (NMCC) hereby submits additional information as follows:

1. Enclosed is a Technical Memorandum from GeoSystems Analysis, Inc. (GSA) to NMCC that provides a literature review of wildlife use of mine pit walls. The enclosed memorandum supports NMCC's statement that the pit walls, post reclamation, will be suitable for achieving a self-sustaining ecosystem appropriate for the life zone of the surrounding area or proposed post-mining land use of steep-canyon wildlife habitat. The GSA memorandum notes that several agencies, including BLM and the US Forest Service, recognize pit walls as nesting and other habitat. Additionally, the GSA memorandum includes as Appendix A, a copy of the New Mexico Department of Game & Fish Habitat Guidance for Mine Operations and Reclamation, which clearly supports the use of pit walls as wildlife habitat. The NMG&F Guidance concludes that vertical habitat diversity is the single-most important factor contributing to avian species diversity and that pit highwalls can provide a vertical habitat feature that mimics natural cliffs or rimrock.
2. Enclosed is NMCC's proposed reclamation/revegetation plan for accessible portions of the dry open pit surface that do not rely on exclusively on self-vegetation. NMCC's plan will reclamation 55 acres of the 129 acre pit shell, and includes 35 acres of revegetated surface area.
3. The information you require to address Sections 19.10.6.602(D)(13)(g)(v)NMAC and 19.10.6.603.C(4) NMAC, i.e., probable hydrologic consequences and hydrologic balance, will be submitted to you under separate cover as soon as it becomes available.

Please contact me at with any questions.

Sincerely,

A handwritten signature in black ink that reads "Jeff Smith".

Jeff Smith  
Chief Operating Officer  
New Mexico Copper Corporation

---

## TECHNICAL MEMORANDUM

September 25, 2017

TO: Katie Emmer, New Mexico Copper Company

FROM: William Widener, GeoSystems Analysis, Inc.

RE: Literature Review of Wildlife Use of Mine Pit Walls

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### Literature Review of Wildlife Use of Mine Pit Walls

Large cliff faces are well documented as providing habitat for wildlife species, specifically birds and raptors. Pit walls of reclaimed mine sites can provide similar substrates, and are also known to be used by some species, most notably Peregrine Falcons. For example, Bell (2001) compiled multiple documentations of Peregrine Falcon and other raptors utilizing high walls in rock quarries in Great Britain, Australia and Germany in looking to expand the practice into the lowlands of Western Washington. Ritchie et al. (1998) documented Peregrine Falcons nesting in quarry sites and road cut banks with a similar structure to hard rock pit walls in Alaska.

Dobra (2002) also documents that raptors have utilized high pit walls of at least one Nevada mine operation. Other raptors, such as red-tailed hawks have also been cited as utilizing open pit mines in New Mexico (Garber et al. 2005). The canyon walls of the Snake River in Idaho provide extensive nesting substrate for the densest concentration of noncolonial nesting raptors, with more than 1,500 raptors, consisting of up to 14 different species, nesting annually (Kochert and Pellant 1986). In New Mexico, the abundance of cliff walls of the Upper Rio Grande Gorge, the Orilla Verde Recreation Area, and the Rio San Antonio Gorge provide ideal nesting substrate for many raptor species, including Golden Eagle, Prairie Falcon, Peregrine Falcon, Red-tailed Hawk and Great Horned Owl (Hawks Aloft, Inc. 2011).

Bat use of abandoned underground mines is well documented. Tuttle & Taylor (1994) found that 30% to 80% of 8,000 mines surveyed across North America showed signs of bat use, though, not much information is available regarding specific bat use of pit walls, which could provide roosting habitat for many species, and more permanent habitat for non-colonial bat species.

Multiple federal agencies, including the United States Forest Service and Bureau of Land Management, recognizes that steep pit walls provide habitat for nesting raptors, other cliff dwellings birds, bats and that the associated talus could also be utilized by other rock dwelling animals, such as reptiles. This is often written into the Environmental Impact Statements for mines (USFS 1997, USFS 1997, BLM 1996). This is consistent with the Draft Environmental Impact Statement for the Copper Flat Copper Mine, which states that following reclamation, "the pit walls and benches would become Chihuahuan Desert wildlife habitat,

providing abundant rock outcroppings, which are regularly utilized by bats for day or night-roosting, or for cliff-dwelling bird species, such as raptors for nesting” (BLM 2015).

In 2004, the New Mexico Department of Game and Fish (NMGF) issued habitat guidelines for mine operations and reclamation. In these guidelines, they recommend that the habitat value of high walls can be “enhanced by design features including an undulating profile, niches or ledges on the face, and placing rubble at the toe of the wall.” (NMGF 2004; Appendix A). Norman (1992) suggest the proper blasting of high walls to leave rough surfaces that provide habitat for birds, such as cliff swallows. The Wyoming Department of Environmental Quality/Land Quality Division’s Guideline No. 5 states that operators should develop high (pit) walls to simulate natural rimrocks, and Benson (2002) goes into detail on how the Buckskin Mine of Wyoming will incorporate microtopography, aspect, ledges and holes to maximize habitat identified for several raptor and other bird species identified as potentially utilizing reclamation areas.

The Federal Geographic Data Committee maintains a list of official datasets, including the U.S Geological Survey (USGS) Gap Analysis Species Distribution Models & USGS Survey Gap Analysis Program Species Ranges. The New Mexico Department of Game and Fish, with help from other supporting agencies, have developed the Biota Information System of New Mexico (BISON-M), which is an on-line database of wildlife species within New Mexico. BISON-M utilizes the USGS’s Gap data, which includes a habitat association category of “Barren: Mines & Quarries”. A BISON-M query, returned a total of 51 species in Sierra County that have either “Casual” or “Important” use of the “Barren: Mines & Quarries” habitat association during any season of the year. This includes eight reptiles, six birds, and 37 mammals, 16 of which are bat species (Table 1).

A baseline data report was conducted during the permitting processes at the Copper Flat mine site. Sixteen species, including four bird and 12 bat species were detected in and around the existing pit lake (Intera, 2012) (Table 2). In 2013, an addendum was produced to respond to various agency comments to the original report. Additional surveys were conducted to address agency comments, specifically, waterfowl and other general use of wildlife at the pit lake. An additional 30 bird species, and five mammal species were observed near the pit lake with these new survey efforts. Undoubtedly, many of these species were attracted by the water in the pit lake, and it cannot be ascertained which of these species were utilizing the pit walls. However, the text of the addendum states multiple observations of wildlife utilizing the rock walls surrounding the pit lake. On one occasion, a Great Horned Owl was heard calling from the hills west of the pit lake. Rock wrens, Northern mockingbirds, Northern flickers, Common ravens, Mourning Doves, White-winged doves and Gambel’s quail are stated to be the most active species during these additional surveys, and are noted as most frequently “heard calling from the hills surrounding the pit lake, typically from the higher tiers to the north of the lake”. Violet-green swallows were observed drinking from the lake before returning to the tiered cliff faces to the northwest of the lake. Chipping sparrows were observed in a bush on the top tier to the south of the pit lake. Additional observations of raptor “white wash” were made on the pit wall west of the lake, indicating raptors perching, though no nesting was confirmed (THEMAC 2013). The closure design will be in similar structure to the baseline conditions, and would expect a similar species composition following reclamation.

Table 1. BISON-M Query Results of Sierra County Wildlife Species Known to Use "Barren: Mines &amp; Quarries" Habitat Association

Life Form	Common Name	Scientific Name
Bird	Bank Swallow	<i>Riparia riparia</i>
Bird	Cliff Swallow	<i>Petrochelidon pyrrhonota</i>
Bird	Great Horned Owl	<i>Bubo virginianus</i>
Bird	N. Rough-winged Swallow	<i>Stelgidopteryx serripennis</i>
Bird	Say's Phoebe	<i>Sayornis saya</i>
Bird	Turkey Vulture	<i>Cathartes aura</i>
Mammal	American Badger	<i>Taxidea taxus</i>
Mammal	Black Bear	<i>Ursus americanus</i>
Mammal	Bobcat	<i>Lynx rufus</i>
Mammal	Brush Mouse	<i>Peromyscus boylii</i>
Mammal	Collared Peccary	<i>Peccari tajacu sonoriensis; angulatus</i>
Mammal	Common Gray Fox	<i>Urocyon cinereoargenteus</i>
Mammal	Common Hog-nosed Skunk	<i>Conepatus leuconotus</i>
Mammal	Common Raccoon	<i>Procyon lotor</i>
Mammal	Coyote	<i>Canis latrans</i>
Mammal	Deer Mouse	<i>Peromyscus maniculatus</i>
Mammal	Desert Bighorn Sheep (delisted pops)	<i>Ovis canadensis mexicana</i>
Mammal	Mexican Wood Rat	<i>Neotoma mexicana mexicana</i>
Mammal	Northern Rock Mouse	<i>Peromyscus nasutus</i>
Mammal	Ringtail	<i>Bassariscus astutus</i>
Mammal	Rock Pocket Mouse	<i>Chaetodipus intermedius</i>
Mammal	Rock Squirrel	<i>Otospermophilus variegatus grammurus</i>
Mammal	Striped Skunk	<i>Mephitis mephitis</i>
Mammal	Texas Antelope Squirrel	<i>Ammospermophilus interpres</i>
Mammal	White-nosed Coati	<i>Nasua narica</i>
Mammal	White-throated Wood Rat	<i>Neotoma albigula</i>
Mammal	White-toothed woodrat	<i>Neotoma leucodon</i>
Mammal (Bat)	Allen's Big-eared Bat	<i>Idionycteris phyllotis</i>
Mammal (Bat)	Big Brown Bat	<i>Eptesicus fuscus</i>
Mammal (Bat)	Brazilian Free-tailed Bat	<i>Tadarida brasiliensis</i>
Mammal (Bat)	California Myotis	<i>Myotis californicus</i>
Mammal (Bat)	Canyon Bat	<i>Parastrellus hesperus</i>
Mammal (Bat)	Fringed Myotis	<i>Myotis thysanodes</i>
Mammal (Bat)	Hoary Bat	<i>Lasiurus cinereus</i>
Mammal (Bat)	Long-eared Myotis	<i>Myotis evotis</i>
Mammal (Bat)	Long-legged Myotis	<i>Myotis volans</i>
Mammal (Bat)	Pale Townsend's Big-eared Bat	<i>Corynorhinus townsendii</i>
Mammal (Bat)	Pallid Bat	<i>Antrozous pallidus</i>
Mammal (Bat)	Silver-haired Bat	<i>Lasionycteris noctivagans</i>
Mammal (Bat)	Southwestern Little Brown Myotis	<i>Myotis occultus</i>
Mammal (Bat)	Southwestern Myotis	<i>Myotis auriculus</i>
Mammal (Bat)	Western Small-footed Myotis	<i>Myotis ciliolabrum</i>
Mammal (Bat)	Yuma Myotis	<i>Myotis yumanensis</i>
Reptile	Crevice Spiny Lizard	<i>Sceloporus poinsettii</i>
Reptile	Eastern Black-tailed Rattlesnake	<i>Crotalus omatus</i>
Reptile	Eastern Collared Lizard	<i>Crotaphytus collaris</i>
Reptile	Great Plains Skink	<i>Plestiodon obsoletus</i>
Reptile	Northern Tree Lizard	<i>Urosaurus omatus</i>
Reptile	Prairie Rattlesnake	<i>Crotalus viridis</i>
Reptile	Twin-spotted Spiny Lizard	<i>Sceloporus bimaculosus</i>
Reptile	Western Diamond-backed Rattlesnake	<i>Crotalus atrox</i>

Table 2. Species around Pit Lake at Copper Flat as documented in BDR &amp; Addendum Reports

<b>Life Form</b>	<b>Common Name</b>	<b>Scientific Name</b>
Bird	American Widgeon	<i>Mareca americana</i>
Bird	Ash-Throated Flycatcher	<i>Myiarchus cinerascens</i>
Bird	Barn Swallow	<i>Hirundo rustica</i>
Bird	Black-Throated sparrow	<i>Amphispiza bilineata</i>
Bird	Blue-Winged Teal	<i>Spatula discors</i>
Bird	Cactus Wren	<i>Campylorhynchus brunneicapillus</i>
Bird	Canvasback	<i>Aythya valisineria</i>
Bird	Canyon Wren	<i>Catherpes mexicanus</i>
Bird	Chipping Sparrow	<i>Spizella passerina</i>
Bird	Cinnamon Teal	<i>Spatula cyanoptera</i>
Bird	Common Raven	<i>Corvus corax</i>
Bird	Crissal Thrasher	<i>Toxostoma crissale</i>
Bird	Eurasian Collared Dove	<i>Streptopelia decaocto</i>
Bird	Gambel's Quail	<i>Callipepla gambelii</i>
Bird	Great Blue Heron	<i>Ardea herodias</i>
Bird	Great Horned Owl	<i>Bubo virginianus</i>
Bird	Greater Roadrunner	<i>Geococcyx californianus</i>
Bird	Homed Lark	<i>Eremophila alpestris</i>
Bird	House Finch	<i>Haemorhous mexicanus</i>
Bird	Killdeer	<i>Charadrius vociferus</i>
Bird	Loggerhead Shrike	<i>Lanius ludovicianus</i>
Bird	Mourning Dove	<i>Zenaida macroura</i>
Bird	Northern Flicker	<i>Colaptes auratus</i>
Bird	Northern Mockingbird	<i>Mimus polyglottos</i>
Bird	Northern Shoveler	<i>Spatula clypeata</i>
Bird	Red-tailed hawk	<i>Buteo jamaicensis</i>
Bird	Rock Wren	<i>Salpinctes obsoletus</i>
Bird	Say's Phoebe	<i>Sayornis saya</i>
Bird	Spotted Sandpiper	<i>Actitis macularius</i>
Bird	Swainson's Hawk	<i>Buteo swainsoni</i>
Bird	Turkey Vulture	<i>Cathartes aura</i>
Bird	Violet-Green Swallow	<i>Tachycineta thalassina</i>
Bird	Western Scrub-Jay	<i>Aphelocoma californica</i>
Bird	White-winged Dove	<i>Zenaida macroura</i>
Mammal	Coyote	<i>Canis latrans</i>
Mammal	Gray Fox	<i>Urocyon cinereoargenteus</i>
Mammal	Mule Deer	<i>Odocoileus hemionus</i>
Mammal	Rock Squirrel	<i>Otospermophilus variegatus</i>
Mammal	Striped Skunk	<i>Mephitis mephitis</i>
Mammal (Bat)	Arizona Myotis	<i>Myotis occultus</i>
Mammal (Bat)	Big Brown Bat	<i>Eptesicus fuscus</i>
Mammal (Bat)	Brazilian Free-tailed Bat	<i>Tadarida brasiliensis</i>
Mammal (Bat)	California Myotis	<i>Myotis californicus</i>
Mammal (Bat)	Canyon Bat	<i>Parastrellus helperus</i>
Mammal (Bat)	Fringed Myotis	<i>Myotis thysanodes</i>
Mammal (Bat)	Pallid Bat	<i>Antrozus pallidus</i>
Mammal (Bat)	Silver-Haired Bat	<i>Lasionycteris noctivagans</i>
Mammal (Bat)	Southern Hoary Bat	<i>Lasiurus cinereus</i>
Mammal (Bat)	Townsend's Big-Eared Bat	<i>Corynorhinus townsendii</i>
Mammal (Bat)	Western Small-Footed Myotis	<i>Myotis ciliolabrum</i>
Mammal (Bat)	Yuma Myotis	<i>Myotis yumanensis</i>

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**APPENDIX A – New Mexico Department of Game and Fish Habitat  
Guidelines for Mine Operations and Reclamation**

**HABITAT GUIDELINES FOR  
MINE OPERATIONS AND RECLAMATION**

**December 2004**

**New Mexico Department of Game and Fish**

## **I. Regulation of Mining Impacts**

The state Energy, Minerals and Natural Resources Department (EMNRD), Mining and Minerals Division, includes three mine regulation programs. The Coal Mine Reclamation Program administers requirements of the federal Surface Mining Control and Reclamation Act of 1977 (SMCRA). The Abandoned Mine Land Program, authorized by SMCRA, reclaims historic abandoned coal and non-coal mines. The Mining Act Reclamation Program administers requirements of the New Mexico Mining Act of 1994 (NMMA), which applies to hard rock mines. Hard rock mining refers to extraction of most solid minerals other than coal. Phosphate mines and sand and gravel pits are not regulated by EMNRD, although local county regulations may apply. The Bureau of Land Management requires a Mine Plan of Operations for mineral extraction activities on their surface, and the Forest Service issues Special Use Permits. New Mexico Environment Department Discharge Permits are required at all mine sites that may affect a groundwater aquifer. The purpose of the Discharge Permits is to ensure compliance with NM Water Quality Control Commission standards. Projects involving more than one acre surface disturbance are required to obtain National Pollutant Discharge Elimination System permits from US Environmental Protection Agency under the Clean Water Act.

The most extensive Department of Game and Fish (Department) involvement takes place on NMMA regulated hard rock operations. The Act requires EMNRD to consult with the Department on all permit approvals and modifications. We also frequently receive requests for consultation from other agencies for other types of mine projects.

### **• Impacts and Mitigation**

**Habitat Loss and Degradation.** Proposed new mines, and modifications to existing mines, should be evaluated for their effect on wildlife habitat. Pre-disturbance surveys are useful to establish baseline data for reclamation. Potential effects on listed species of concern should be identified and mitigated. Special habitat values or features should be identified for replacement during reclamation. Stormwater run-off should be controlled to avoid adding sediment to streams. Excessive sediment load affects aquatic organisms by covering up substrate habitat, carrying toxic elements, alteration of water quality or direct fish kill. Mine-related habitat loss may be temporary where mitigated by adequate reclamation practices, or permanent if reclamation is not required or is not properly implemented. The Department encourages the practice of concurrent reclamation, whereby portions of the affected area no longer in use are reclaimed while active operations continue nearby.

### **Physical and chemical hazards.**

Many mining and milling operations involve the use of chemicals or the contamination of water by acid generating drainage. Open water that may present a hazard to wildlife includes stormwater impoundments, tailings ponds, and pit lakes remaining after the cessation of mining. No pits should be located below the ordinary high water mark of any watercourse, lakebed, sinkhole, or playa lake, or in any wetland. Any open water in an arid environment will attract wildlife of all kinds. Wildlife need to be protected from contacting and ingesting harmful liquids.

Where ponds, pits or open-top tanks contain hazardous liquids, they should be netted, fenced or otherwise protected. The US Fish & Wildlife Service provides technical guidance on protective netting on the internet at <http://www.r6.fws.gov/contaminants/contaminants1c.htm>. Wildlife exclusion fencing may be appropriate for some situations. Exclusion fences must be a minimum eight feet in height, constructed of chain link or woven or welded wire mesh. They should be secured at the ground or preferably buried to prevent animals digging under, and should be wrapped around the base with a durable finer mesh material to deter small mammals and reptiles and amphibians. Fences which are intended to exclude livestock should be designed to minimize potential for causing injury or death to large wildlife attempting to cross over or under. The Department has fence specifications available for a variety of conditions.

Non-toxic ponds, pits and trenches may also present a trapping hazard for wildlife, if they are steep-sided and/or lined with smooth-surfaced material. Textured liner material is available which can be attached to create escape ramps. Depending on the configuration of the trapping hazard, earthen ramps, floating rafts and ladders may also be appropriate solutions. The Department can provide consultation and design specifications on the appropriate technology.

Chemicals stored in containers should be labeled, container integrity maintained in good condition, and secondary containment (berms or sumps) provided around tanks and at points of transfer. Machinery and infrastructure should be maintained in good condition to prevent leaks and spills, and appropriate spill response equipment and procedures should be identified prior to bringing chemicals on site.

Another mine feature which may present a hazard to wildlife is overhead electric supply lines. Please refer to the Department powerline habitat guideline for more information on that subject.

- **Reclamation.**

**Cover and Revegetation.** Traditional mine reclamation has included grading the reclaimed area to a uniform 3:1 slope for the purpose of minimizing erosion. The Department encourages incorporation of topographic variability reflecting the natural site surroundings and fluvial geomorphology where feasible. Vertical habitat diversity is the single most important factor contributing to avian species diversity. Where substrate integrity is sufficient to prevent erosion or slumping, highwalls may provide a vertical habitat feature that mimics natural cliffs or rimrock. The habitat value of highwalls can be enhanced by design features including an undulating profile, niches or ledges on the face, and placing rubble at the toe of the wall. Habitat enhancement features can also be added to a homogeneous slope to provide vertical diversity and opportunities to hide from predators. Features might include clumps or rows of planted shrubs, brush piles, rock piles or constructed perches or nest platforms. The Department is available to help determine optimal configuration of features given the setting and available materials.

The two main purposes of reclaimed mineland vegetation are to prevent surface erosion, and prevent infiltration of rainwater to the depth of buried material which may cause groundwater contamination. Surface preparation should at a minimum include placement of topsoil, either stockpiled from the site or borrowed from elsewhere, furrowing on contour, and mulching after seed is applied. Soil cover should be designed to minimize uptake of toxic materials by plant roots, and from there into the ecological food chain. Seed mixes are typically specified by the regulating agency and/or the surface owner. The Department encourages the use of native species exclusively. Seed lots and mulch should be weed-free and reclaimed areas should be monitored for noxious weed infestation. Plants which are of value to particular wildlife (for example, deer browse) may be recommended where appropriate.

**Water.** Wildlife may need protection from contaminated water sources, as detailed above, during and after reclamation. Conversely, provision of clean drinking water should be considered, to mitigate loss or degradation of natural water sources, or other habitat loss. Earthen tanks may be created where infiltration to contaminated subsurface layers is not a concern. Impermeable rainwater catchment drinkers may be a solution in other situations. The Department is available for consultation and specifications for providing wildlife watering facilities.

**Underground Features.** Many abandoned mine workings, and some active mine sites, have historic underground tunnels, shafts or adits. These features can cause injury or death to people who approach or attempt to enter, so they are often targeted for filling or plugging. However underground features are often used by bats, and some raptors, owls and snakes. Many of the bats are species of concern, because of population declines or simply because there is not enough information to determine their conservation status. Importance of a feature as habitat depends on factors including the particular species present, the type or seasonality of use, and surrounding habitat characteristics. Historic underground mine features should be evaluated by an expert in the field to decide the appropriate method of closure or guarding. Where appropriate, custom bat gates can be installed to protect public safety while maintaining bat access to the interior. In New Mexico, the EMNRD Abandoned Mine Land program has developed a high level of expertise about bat-friendly closures.

#### References

NM Mining Act Rules, NMAC Title 19 Chapter 10  
[http://www.nmcpr.state.nm.us/nmac/\\_title19/T19C010.htm](http://www.nmcpr.state.nm.us/nmac/_title19/T19C010.htm)

Coal Mine Reclamation Program Rules, NMAC Title 19, Chapter 8  
[http://www.nmcpr.state.nm.us/nmac/\\_title19/T19C008.htm](http://www.nmcpr.state.nm.us/nmac/_title19/T19C008.htm)

NM Water Quality Control Commission homepage  
<http://www.nmenv.state.nm.us/Oots/wqcc.htm#Legislation>

**COPPER FLAT  
OPEN PIT RECLAMATION/REVEGETATION PLAN  
OCTOBER 2017**

**INTRODUCTION**

NMCC submitted its Mine Reclamation Plan for the Copper Flat Mine Project to the New Mexico Mining and Minerals Division (MMD) in July, 2017 (See Appendix E of MORP). Upon its review, on October 5, 2017 the MMD asked NMCC to provide a reclamation/revegetation plan for accessible portions of the open pit as a component of reclamation of the pit so that the NMCC reclamation plan does not rely exclusively on self-vegetation inside the open pit. This proposal is limited to proposed enhancement of the reclamation plan for the open pit to complement the MORP and Reclamation Plan submitted to the MMD in July 2107, including rapid-fill of the bottom of the open pit after mining ceases.

NMCC’s proposes to add revegetation of the following three areas within the open pit:

1. The haul road leading from the crest of the pit to the pit water body that will form post-mining as a result of rapid-fill of the pit as described in NMCC’s July 17, 2007 MORP Reclamation Plan (approximately 23 acres);
2. The expanded 4900 Catch Bench (approximately 2 acres); and
3. Several pit benches at the pit crest (approximately 10 acres);

Figure 1 illustrates the revised reclamation plan, including the previously described pit water body that NMCC will develop utilizing fresh water rapid fill, and the three areas listed above that will be covered and revegetated. As indicated in Table 1, with this amendment, a total of 55 acres or 43% of the 129 acre pit shell, will be covered (35 acres cover and revegetate, 20 acres rapid fill as described in NMCC’s MORP and Reclamation Plan). The remaining pit walls represent the steep-walled portion of the post-mining wildlife habitat (PMLU) that will be established as discussed in the MORP and Reclamation Plan.

<b>TABLE 1</b>		
<b>PIT SHELL RECLAMATION/VEGETATION ACREAGE</b>		
	<b>ACRES</b>	<b>% OF FOTAL</b>
<b>PIT SHELL (Total acreage)</b>	<b>129</b>	
<b>PIT HAUL ROAD</b>	<b>23</b>	<b>18%</b>
<b>4900 EXPANDED CATCH BENCH</b>	<b>2</b>	<b>2%</b>
<b>PIT CREST</b>	<b>10</b>	<b>8%</b>
<b>PIT WATER BODY</b>	<b>20</b>	<b>15%</b>
<b>PIT SHELL RECLAMATION</b>	<b>55</b>	<b>43%</b>





Following is a more detailed description of NMCC's proposed revegetation enhancements to reclamation of the pit shell.

### **PIT HAUL ROAD**

The open pit will be mined in benches over a 12 year period, creating a terraced pit wall. As shown in Figure 1 (and Drawing C-014 of Appendix E Reclamation Plan), access into the open pit during mining will be via a 90 foot wide mine haul road left in the pit wall, which will be constructed as mining advances. Also shown in Figure 1 are the locations of several surface water conveyance channels which will be constructed around the northern and eastern pit perimeter, perimeter channel PC-1 which will direct surface runoff from north of the pit shell, haul road channel HC-3 which will direct surface runoff from the reclaimed Waste Rock Stockpile no. 1 (WRSP-1), and Top Surface Channel TSC-2, which will direct surface runoff from reclaimed Existing Waste Rock Stockpile No. 4 (EWRSP-4) into the open pit. The surface water runoff collected by these channels will be routed into the pit through haul road channel HC-5, which will follow the alignment of the open pit haul road as shown in Figure 1.

For reclamation, and where practicable, the haul road will be ripped to a minimum depth of 12 inches and covered with 6 inches of growth media material. In areas where ripping to a 12-inch depth cannot be accomplished, an 18 inch growth media cover will be placed. The reclaimed haul road will include the storm water conveyance ditch and single vehicle access to the bottom of the pit for monitoring.

After grading and contouring to maintain proper drainage and erosion protection, the haul road will be seeded with a seed mix approved by the BLM and MMD. Assistance from the New Mexico Department of Game & Fish (NMDG&F) may also be sought in establishing revegetation in the pit shell in order to take advantage of NMDG&F expertise in establishing appropriate wildlife habitat. The revegetated haul road area represented in Figure 1 is approximately 23 acres in size and includes sections of catch benches on either side of the road that can be safely accessed from the road.

### **EXPANDED 4900 CATCH BENCH**

As shown in Figure 1 (and Drawing C-0013 on Appendix E), the 4900 elevation catch bench will be expanded to approximately 2 acres in size in the northwest corner of the pit. This area will remain above water after rapid-fill is complete.

At closure, and before beginning rapid-fill of the pit, NMCC will construct a ramp from the haul road to access the step-out for revegetation activities. The surface will be ripped to a depth of 12 inches and a 6 inch growth media cover placed or alternatively an 18 inch growth media cover if the bench cannot be ripped. The growth media cover will then be seeded with a seed mix approved by the BLM and MMD. Assistance from the NMDG&F may also be sought in establishing revegetation in the pit shell in order to take advantage of its expertise in establishing appropriate wildlife habitat. The revegetated catch bench area is approximately 2 acres in size.

### **PIT CREST**

Figure 1 identifies areas around the crest of the pit shell that will also be revegetated. These upper benches will be laid back at an approximate 2:1 slope angle at the end of the mine operations in order “soften” the pit edge and to accommodate revegetation. NMCC assumes these benches will require drilling and blasting to accomplish the desired lay back. This area will be blended into the surrounding reclaimed pit perimeter area described in the MORP and Reclamation Plan. Revegetation will be accomplished by ripping the area to a depth of 12 inches and a 6 inch growth media cover placed and re-contoured to blend with reclamation of the pit perimeter area described in the Reclamation Plan. An 18 inch growth media cover will be placed over areas that cannot be ripped. The area will be seeded with a seed mix approved by the BLM and MMD. Assistance from the NMDG&F may also be sought in establishing revegetation in the pit shell in order to take advantage of its expertise in establishing appropriate wildlife habitat. The revegetated area at the pit crest is approximately 10 acres in size. Figure 1 was prepared from Drawing C-014 of the Reclamation Plan. The 10 acres of crest reclamation shown in Figure 1 represents proposed reclamation in addition to the pit perimeter reclamation described in the Reclamation Plan.

### **PIT SHELL REVEGETATION SUCCESS**

This amendment adds a pit revegetation component to its Reclamation Plan in response to MMD’s request for further enhancing the wildlife PMLU of the pit. As noted in Section 5.7 of the Reclamation Plan, NMCC proposes to work with the MMD and the BLM to develop appropriate, reasonable and attainable standards applicable to the pit shell. These standards will be in conformance with 19.10.6.603.G.2(c) NMAC.

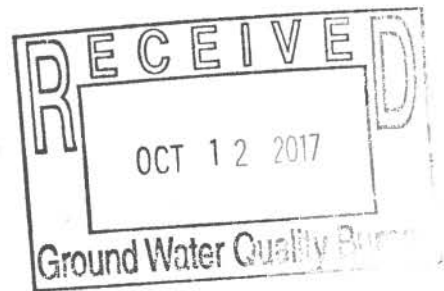
### **PIT SHELL POST-MINING LAND USE**

As stated in Appendix E of NMCC’s Reclamation Plan, the open pit area will be reclaimed to establish a wildlife habitat post-mining land use. NMCC has taken the MMD’s comments regarding reclamation into account in preparing the revisions to its Reclamation Plan in order to further enhance the success of the wildlife PMLU for the pit shell by proposing the revegetation component described herein. NMCC believes that this proposed revision, when considered along with the overall Mine Reclamation Plan in the MORP Appendix E, addresses the MMD’s concerns and completes a technically approvable Reclamation Plan for Copper Flat.



October 13, 2017

Brad Reid  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
Harold L. Runnels Building  
1190 St. Francis Drive  
P.O. Box 5469  
Santa Fe, NM 87502-5496



**Re: Supplemental Information regarding open pit hydrologic sink, DP-1840  
New Mexico Copper Corporation Copper Flat Project**

Dear Mr. Reid,

Enclosed for your review and consideration is information prepared in response to your request for further explanation of NMCC's conclusion that the open pit at the proposed Copper Flat project is currently, and will remain a hydrologic sink upon reclamation and closure of the facility.

NMCC submitted an update to its Mining Operations and Reclamation Plan (MORP) on July 17, 2017, to the Mining and Minerals Division (MMD) and at the same time submitted the Reclamation Plan to your office as the required Closure Plan to complete its Discharge Plan Application. Your review of the July 17 submittal generated a request for additional information relating to NMCC's analysis and determination that rapid-fill of the pit at mine closure will not result in development of a groundwater flow-through system at the pit, either on a temporary or permanent basis. The attached technical memorandum prepared by John Shomaker Associates, Inc. (JSAI) addresses this issue and provides the technical rationale for NMCC's conclusions in this regard.

JSAI's technical memorandum documents the analysis and conclusions reached in the documents already provided to NMED in past submittals rather than to redo the analyses. In this memo JSAI documents the past technical reports, analyses and conclusions regarding the existing and future open pit hydraulic sink and provides additional reinforcement of the technical work and conclusions that have been completed. We believe that the information outlined by JSAI provides ample support for NMED to conclude that the pit water body is currently a hydrologic sink and that it will remain such during operations and after post mining rapid-fill operations are completed.

NMCC, JSAI and NMED have visited this subject on a number of occasions. In its March 21, 2016, request for additional information in Comment No. 14, NMED indicated that it "would likely require additional monitoring wells to verify the area of open pit hydrologic containment initially, and as mining progresses" and that "[A]t a minimum, NMED will require installation of at least one well located at the southeast portion of the open pit in or adjacent to Grayback Arroyo between monitoring wells GWQ11-24 A&B and GWQ96 23 A&B." In our response to this comment, provided to NMED on June 21, 2016, we acknowledged NMED's need to verify the hydrologic sink condition exists and committed to relocate

proposed well PGWQ-1 to the location proposed by NMED (see NMCC's response to Comment No. 14, June 21, 2016). NMCC's proposed monitoring program was revised to incorporate this change and remains committed to demonstrating that the hydrologic sink will remain post-mining per its monitoring program.

Relative to the rapid fill, NMED has asked that NMCC confirm that the water quality of the production wells to be utilized as the source of rapid-fill water will meet the New Mexico WQCC standards. JSAI includes a summary table that provides the results of sampling that was conducted in May 2012 at two of the four production wells from which the rapid-fill water will be sourced. As you can see, the water quality of the production wells meets all of the numerical WQCC standards.

The location of the production well field can be seen in Figure 2-12 of NMCC's Updated MORP, a copy which is attached for your convenience. The well field wells are all completed into the same formation, i.e., the Santa Fe Group, all in the same general location. You may recall that the water quality data was generated as a result of the pump test approved by NMED and conducted by NMCC. In conducting the test, NMCC equipped wells PW-1 and PW-3 for pumping and sampling. The data was provided to NMED in May 2012 and is included in JSAI's memorandum.

Additionally, NMED has requested that NMCC provide an analysis of the temporal hydrologic condition that will occur in the underlying formation as a result of the rapid-fill activity. JSAI's technical memorandum describes pit water "push out" into wall rock that will occur as rapid-fill occurs. Certainly some minor amount of water may move into the formation for a short distance as a result of rapid-fill. However, as discussed in detail by JSAI, there is little or no potential for return/flow in the formation to occur through to ground water. Even if it did, which considered entirely unlikely, as shown by the data presented herewith, the water quality of the rapid fill water meets the WQCC standards and, therefore, would not impact the formation water.

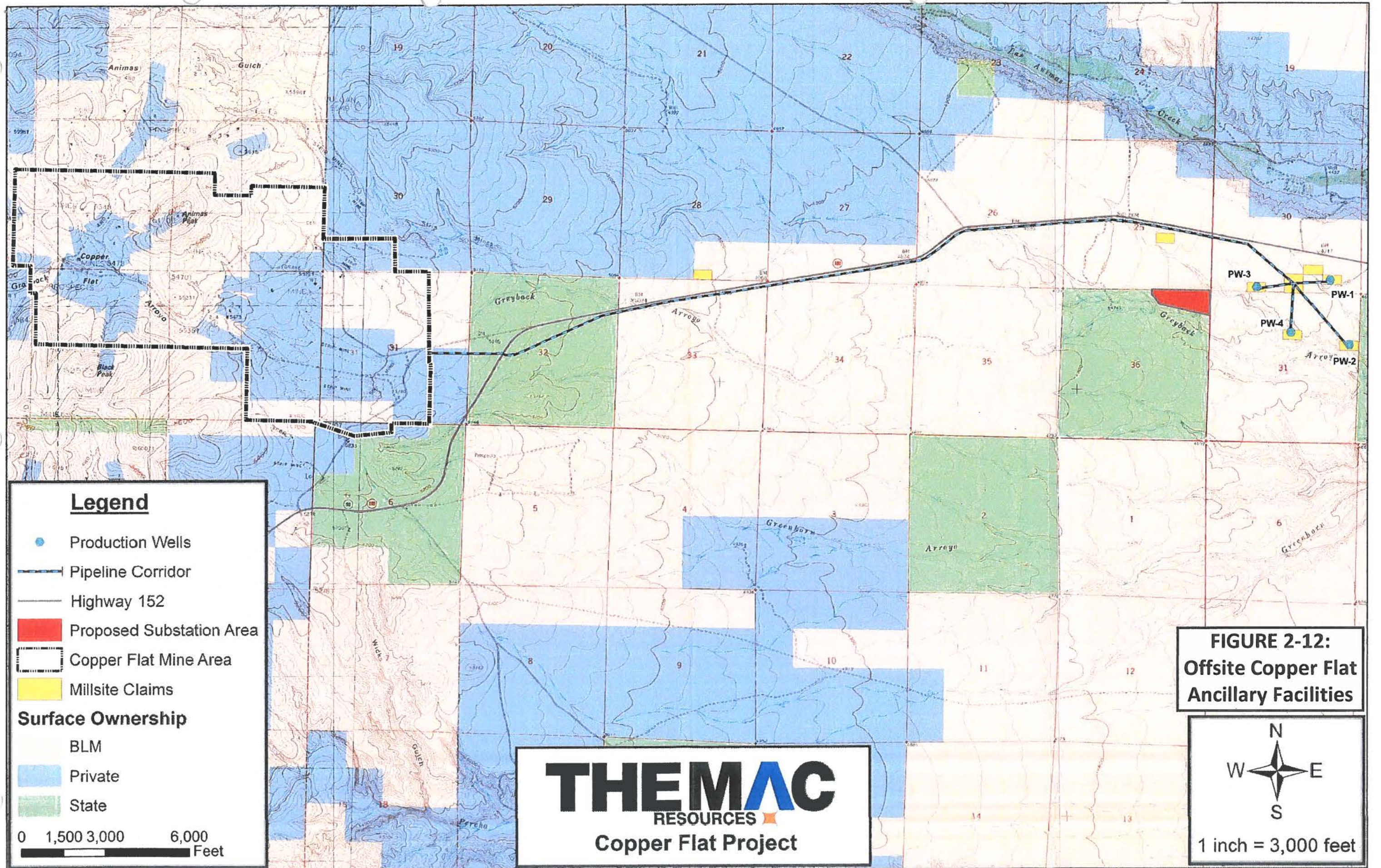
Finally, NMED requested that NMCC describe the ground water capture zone of the hydrologic sink, or the Area of open pit hydraulic containment (AOPHC), that will develop as the pit is dewatered during operations separate from the open pit surface area (OPSDA). JSAI's technical memorandum contains the information requested, including a figure depicting the AOPHC in relation to the OPSDA. The referenced figure, developed from JSAI's groundwater model and analysis, clearly demonstrates the presence of a hydrologic sink around the pit following mining.

NMCC hopes that this information satisfies NMED's request for additional information, allowing it determine that NMCC's Discharge Permit application is technically complete and proceeding to a draft DP.

Sincerely,



Jeff Smith  
Chief Operating Officer  
New Mexico Copper Corporation



**Legend**

- Production Wells
- Pipeline Corridor
- Highway 152
- Proposed Substation Area
- Copper Flat Mine Area
- Millsite Claims

**Surface Ownership**

- BLM
- Private
- State

0 1,500 3,000 6,000 Feet

**THEMAC**  
RESOURCES  
Copper Flat Project

**FIGURE 2-12:**  
Offsite Copper Flat  
Ancillary Facilities

N  
W — E  
S

1 inch = 3,000 feet



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**TECHNICAL MEMORANDUM**

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To: Jeffrey Smith [jsmith@themacresourcesgroup.com](mailto:jsmith@themacresourcesgroup.com)  
New Mexico Copper Corporation

From: Steve Finch, Principal Hydrogeologist-Geochemist  
Michael A. Jones, Principal Hydrologist

Date: October 12, 2017

Subject: Hydrologic Effects of proposed Rapid Fill Reclamation of Copper Flat Open Pit

---

The proposed Copper Flat Project includes a mine pit, supply wells, tailings facility, and waste rock facilities located in the Hillsboro Mining District, Sierra County, New Mexico. The proposed operating scenario reflects a processing rate of 30,000 tons of ore per day for 11.5 years, which is "Alternative 2" in the Copper Flat Draft Environmental Impact Statement (BLM, 2015). Figure 1 shows the configuration of the ultimate open pit and Open Pit Surface Drainage Area (OPSDA).

After mining, rapid-fill of the open-pit reclamation is proposed. Clean water from the supply wells will be used to rapid fill the open pit to the steady state elevation that is projected to occur with natural fill. The primary purpose of rapid fill reclamation is to limit time of exposure of open pit bottom to atmospheric conditions (oxidation of sulfides) and to achieve optimum water quality while maintaining a hydraulic sink. The open pit dewatering and rapid fill effects were evaluated using the numerical model of groundwater flow developed by JSAI (2014) and peer reviewed and adopted by the New Mexico Office of the State Engineer (NMOSE).

At the end of mining, groundwater-level drawdown in the bedrock around the open pit reaches a maximum of about 800 ft at the pit (Fig. 2). A permanent cone of depression will form around the pit, with maximum drawdown of about 600 ft at the edge of the pit. The pit, which currently is an evaporative hydrologic sink, will form an evaporative hydrologic sink again in the future. Current and projected final pit water surface elevations and areas are shown on Figure 2. The existing pit currently has a water surface area of about 5.2 acres. The proposed pit water body will have a steady state surface area of about 22 acres.

### **Existing Open Pit**

The existing Copper Flat open pit water body is a hydraulic sink, as evidenced by more than 20 years of data and analysis (SRK, 1997; INTERA, 2012, SRK, 2013; JSAI, 2014a; BLM, 2015). In addition the U. S. Army Corp of Engineers determined the Copper Flat water body is not a water of the U. S. because it is a hydraulic sink disconnected from natural water courses (Leavitt, 2014).

### **Open-Pit Dewatering**

Projected pit water level, pit-area groundwater level, and dewatering rates are summarized on Figure 3. Long-term total inflow ranges between about 35 and 65 gpm (56 and 105 ac-ft/yr) with an initial minimum of about 20 gpm (32 ac-ft/yr) and a maximum of about 70 gpm (113 ac-ft/yr), as the pit bottom approaches final elevation of 4,650 ft above mean sea level (amsl).

### **Pit Dewatering Effects**

The drawdown effect of model-simulated open pit dewatering is shown on Figures 3 and 4. Open pit dewatering during mining creates a 600 ft cone of depression in the bedrock aquifer. Groundwater drawdown due to pit dewatering remains within the bedrock, and formation of the open pit and associated pit dewatering forms a permanent hydraulic sink (Figs.4 and 5). The area of open pit hydraulic containment (AOPHC) for end of mining is illustrated by the drawdown contours on Figure 4.

### **Post-Mining Open Pit Water Balance**

The post-mining pit water level and water balance were simulated assuming the pit geometry and watershed shown on Figure 1. The area within the pit highwall is about 129 acres, and the total OPSDA watershed area is about 314 acres.

Precipitation on the pit area was estimated for each month based on the record at Hillsboro (JSAI, 2014, Sec. 2.0), with annual average precipitation of 12.5 in. Runoff from the highwall was simulated at 20.7 percent of precipitation, and runoff from the haul road at 30.3 percent. Runoff from the remainder of the watershed was simulated at 7.1 percent of precipitation.

Evaporation from the open pit was assumed at 50 in/yr, less than the 65 in/yr estimated potential evaporation (JSAI, 2014, Sec. 2.4) for the existing open pit. The lower rate reflects the wind and sun sheltering effects of the deeper open pit. Monthly evaporation rates based on the record at Hillsboro were scaled to match the annual rate of 50 in/yr.

### **Open Pit Reclamation**

Post-mining reclamation would include use of the water-supply wells PW-1 through PW-4, and a temporary pipeline to the bottom of the pit, to rapidly fill the pit to the expected long-term post-mining equilibrium water level. The post-mining simulation assumes this "rapid fill" scenario. Rapid filling will result in better water quality in the open pit by filling it with clean water and inhibiting oxidation of sulfide by submerging potential acid-generating sections of the pit wall. The water quality from the PW wells has low total dissolved solids content, buffering capacity from bicarbonate ions, and meets all water quality standards (see attached lab report).

After mining is complete, the pit will be rapid filled to the projected steady-state post-mining water level that maintains the hydraulic sink conditions. The volume placed by rapid fill will still be within the cone of depression cause by open pit dewatering, with hydraulic controls caused by the established AOPHC.



A pumping rate of 2,726 gpm is simulated in the model, sufficient to fill the pit to elevation 4,894 ft amsl in 6 months. Total volume pumped from the supply wells will be 2,200 acre feet. Model simulated open pit water-level elevations due to rapid fill reclamation are presented on Fig. 5). The final open pit water body elevation of about 4,894 ft amsl corresponds to a water-surface area of about 22 acres. Water levels will fluctuate around this mean by a few feet, rising and falling seasonally and with wet and dry climatic conditions, but the hydraulic sink conditions will remain in place.

The simulated (annual) pit water balance is presented on Figure 6, showing a final pit water balance of about 93 ac-ft/yr, with about 57 ac-ft/yr from precipitation and runoff, and 36 ac-ft/yr from groundwater inflow, all discharging as evaporation from the pit water surface to maintain the hydraulic sink.

The rapid filling of the pit will not result in pit water discharging to the groundwater system. In the pit bottom, a dewatered space forms between the groundwater levels at the end of the rapid fill and the pit shell (see Fig. 2). Model simulated flow to this dewatered space during rapid fill amounts to 0.74 acre feet for the six month rapid fill event, and 0.00 acre feet for the following 100 year simulation into the future. Rapid fill water will be confined to the open pit and adjacent dewatered space, and the hydraulic force of groundwater discharge at the pit bottom and sides caused by the 600 ft drawdown cone created during dewatering will prevent discharges to groundwater.

Water levels in the open pit and immediate vicinity of the pit wall will change due to rapid fill and natural fluctuations after rapid fill, but groundwater levels adjacent to the pit will remain as a hydraulic sink, before, during, and after rapid filling. After reclamation, groundwater levels in the bedrock around the open pit will remain below pre-mining levels, due to groundwater flowing to the open pit and discharging as evaporation from the hydrologic sink (Fig. 7). The pit water level will fluctuate naturally (by a few feet) according to climate conditions, tending toward a long-term equilibrium level.

The pit will remain as a hydraulic sink during temporary water level fluctuations, because of the large cone of depression caused by dewatering and maintained by water surface evaporation. In order for it to be possible for water to flow from the pit to groundwater, the hydraulic gradient would have to be higher than surrounding groundwater. No conceivable storm event, wet year or even wet decade could possibly add enough water to the pit to reach the water level (>5,100 ft elevation) required to achieve flow-through. Figure 2 is an east-west cross section through the open pit, showing projected water levels in the pit and downstream (5,100 ft elevation).

### Attachments

Figure 1. Ultimate open pit and watershed area

Figure 2. West-to-east hydrogeologic cross section E-E' showing water-level profile across existing pit and proposed open pit after rapid fill

Figure 3. Projected pit water level, groundwater level, and pit pumping rate

- Figure 4. Projected end-of-mining groundwater drawdown (ft) for Mine area
- Figure 5. Model-simulated water level elevation contours and direction of groundwater flow around open pit at end of mining
- Figure 6. Projected open-pit water level due to rapid fill reclamation
- Figure 7. Projected open-pit water balance during first six months of rapid fill and afterwards
- Figure 8. Model-simulated open pit water level elevation, groundwater level elevation contours, and direction of groundwater flow after rapid fill reclamation
- PW-1 water quality analysis

### References

- [BLM] U. S. Bureau of Land Management, 2015, Copper Flat Copper Mine Draft Environmental Impact Statement: BLM/NM/ES-16-02-1793, November 2015.
- INTERA, 2012, Baseline Data Characterization Report for Copper Flat Mine, Sierra County, New Mexico. Report prepared for New Mexico Copper Corporation, June 2012.
- [JSAI] John Shomaker & Associates, Inc., 2014, Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico: Consultant report prepared for NM Copper Corporation.
- [JSAI] John Shomaker & Associates, Inc., 2014a, Results from first year of Stage 1 Abatement investigation at the Copper Flat Mine Site, Sierra County, New Mexico: Consultant report prepared for NM Copper Corporation.
- [JSAI] John Shomaker & Associates, Inc., 2017, Results from first year of Stage 1 Abatement investigation at the Copper Flat Mine Site, Sierra County, New Mexico: Consultant report prepared for NM Copper Corporation.
- Leavitt, M., 2015, Letter regarding Approved Jurisdictional Determination – Action No. SPA-2014-00364-LCO, Open Pit Water Body Inclusive of the Associated 230 Acre Watershed at Copper Flat Mine in Sierra County, New Mexico to Katie Emmer, New Mexico Copper Corporation, October 6, 2014
- SRK, 1997, Copper Flat Mine Compilation of Pit Lake Studies: Consultant's report prepared by Steffen Robertson and Kirsten, Inc. prepared for Alta Gold Co., December 1997
- SRK, 2013, Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico: Consultant's report prepared by SRK Consulting prepared for TheMAC Resources Group, LTD, September 2013

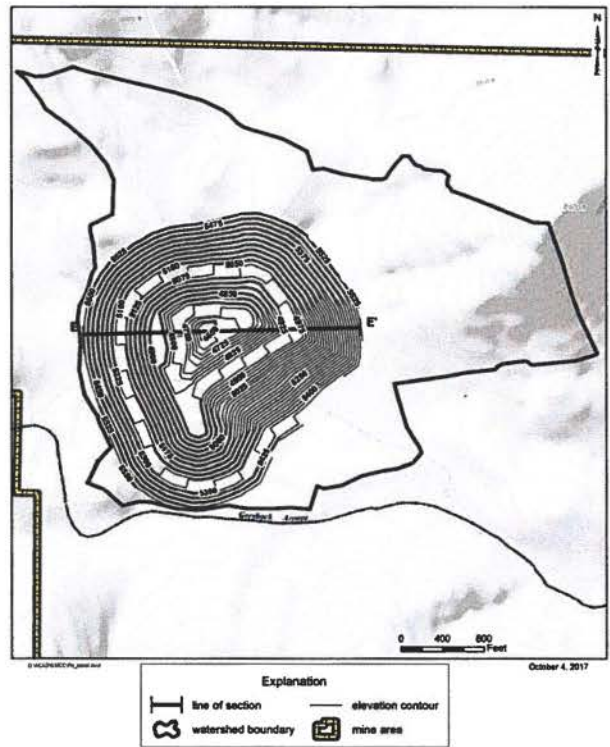


Figure 1. Ultimate open pit and watershed area.

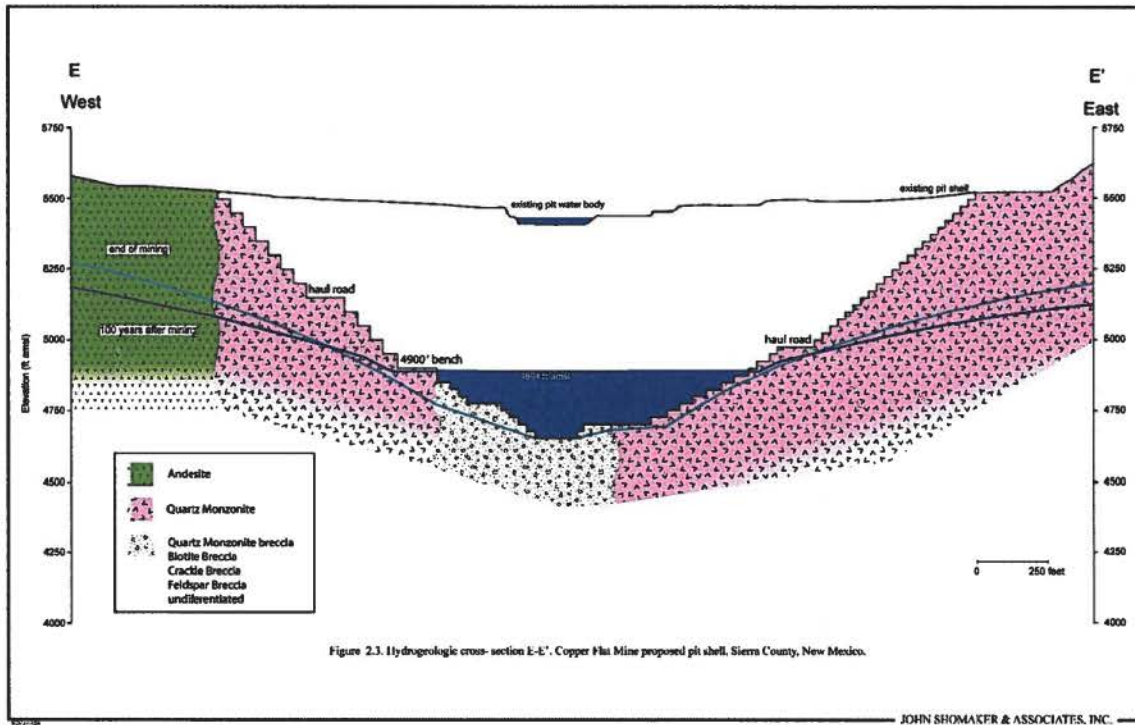


Figure 2.3. Hydrogeologic cross-section E-E'. Copper Flat Mine proposed pit shell, Sierra County, New Mexico.

Figure 2. West-to-east hydrogeologic cross section E-E' showing water-level profile across existing pit and proposed open pit after rapid fill.

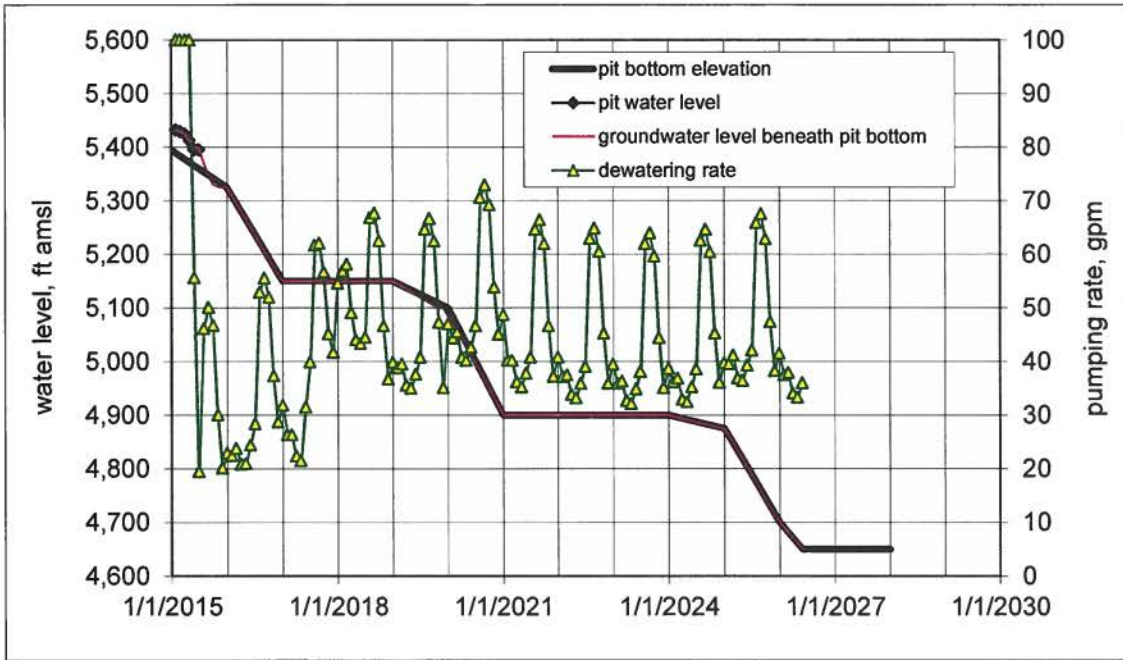


Figure 3. Projected pit water level, groundwater level, and pit pumping rate.

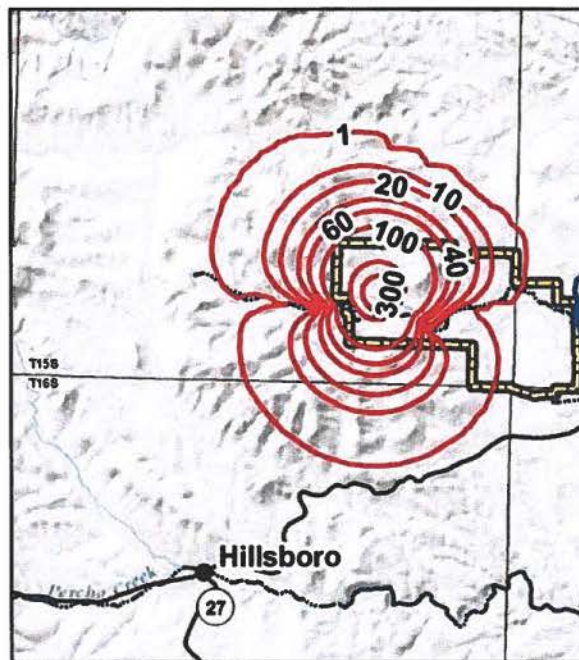
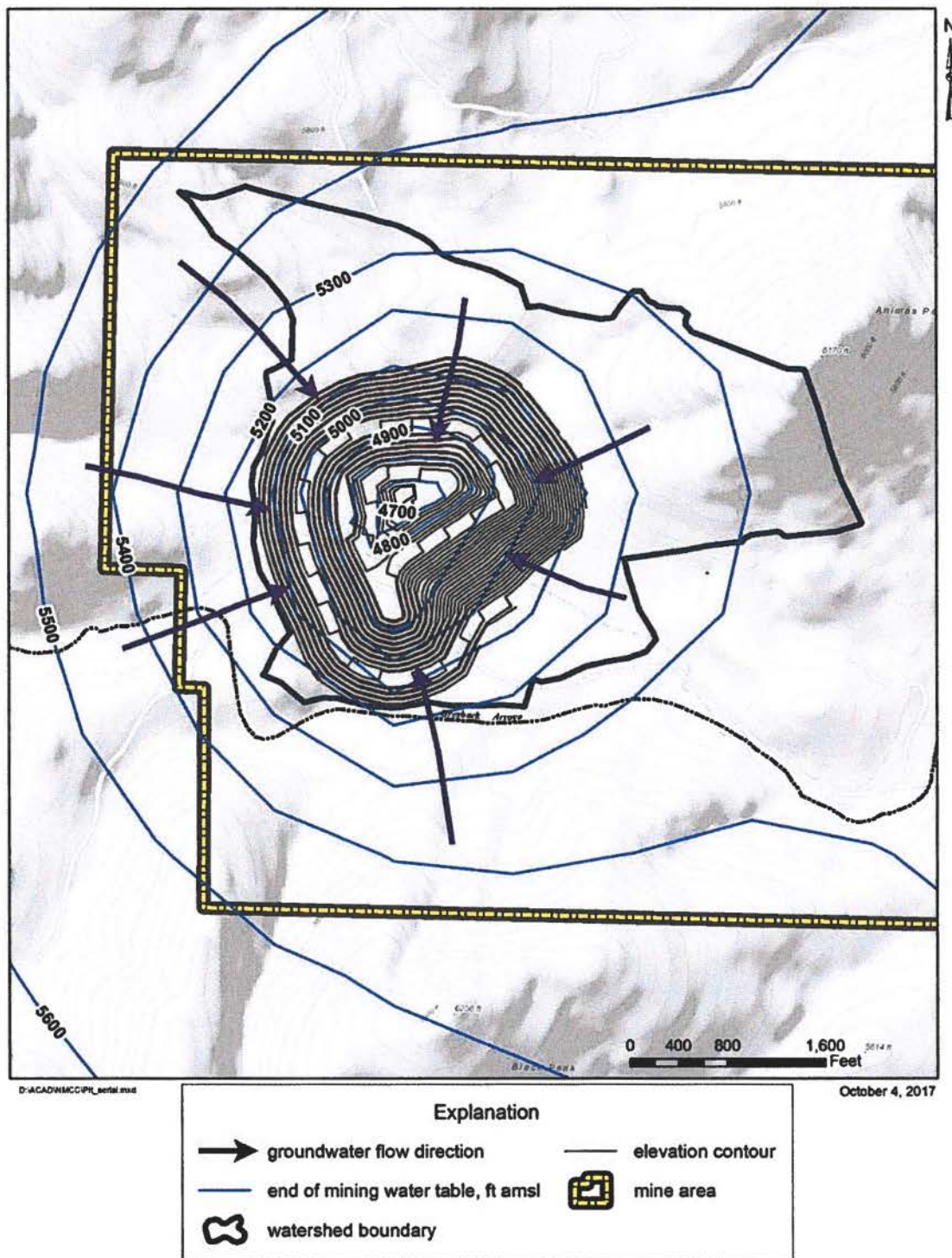


Figure 4. Projected end-of-mining groundwater drawdown (ft) for Mine area.



**Figure 5. Model-simulated water level elevation contours and direction of groundwater flow around open pit at end of mining**

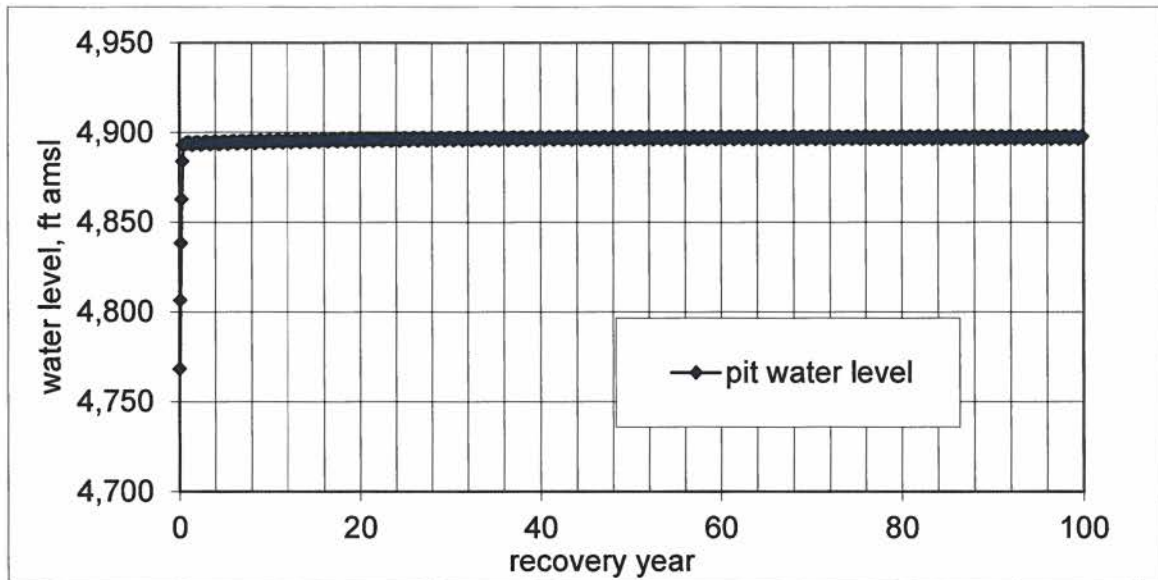


Figure 6. Projected open-pit water level due to rapid fill reclamation

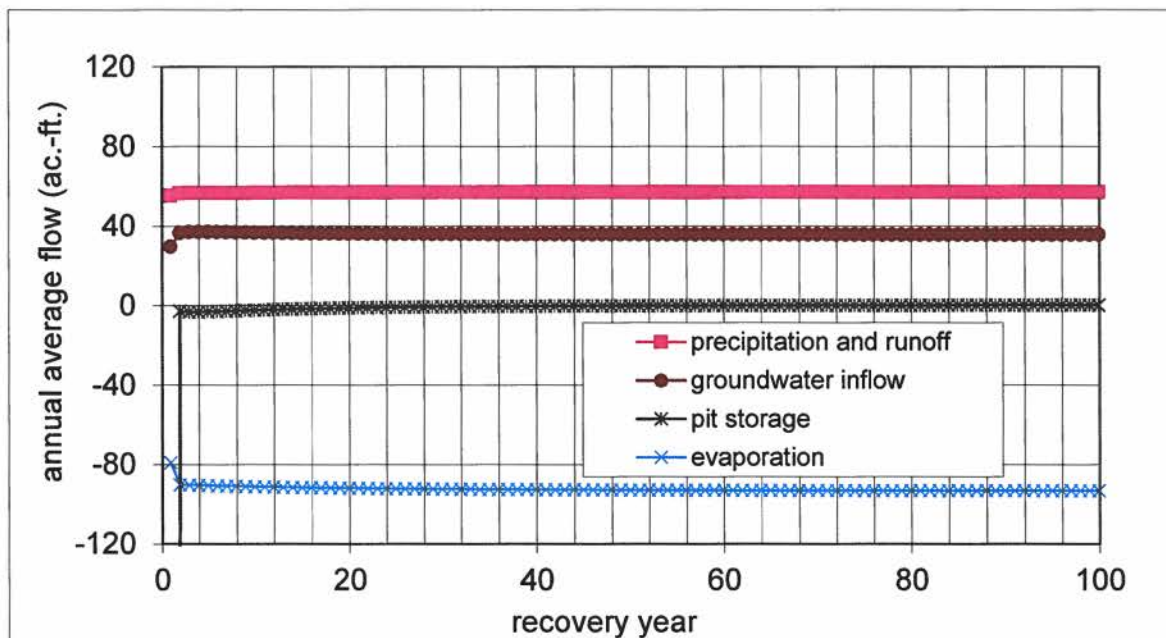
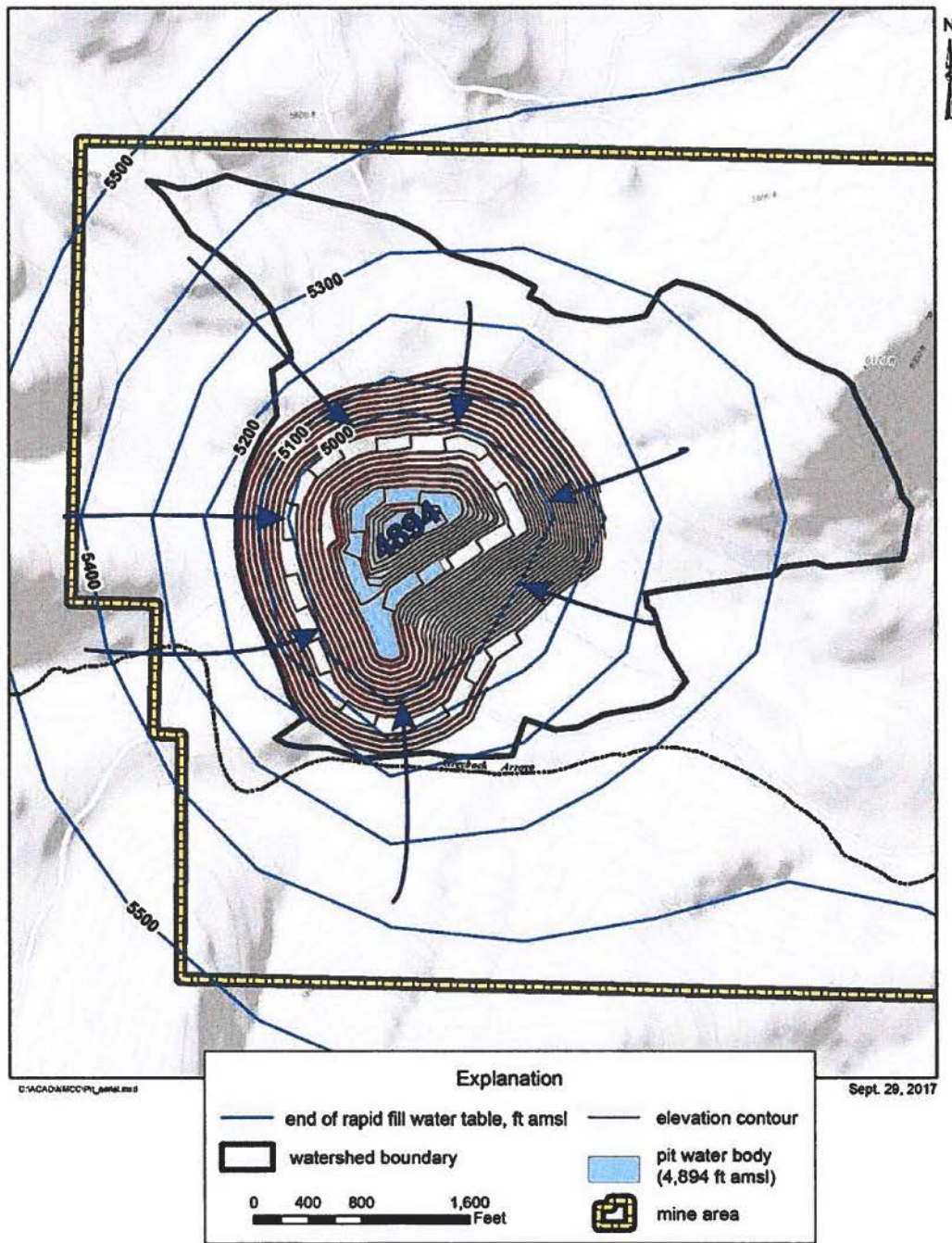


Figure 7. Projected open-pit water balance during first six months of rapid fill and afterwards.



**Figure 8. Model-simulated open pit water level elevation, groundwater level elevation contours, and direction of groundwater flow after rapid fill reclamation**



<b>COPPER FLAT PRODUCTION WELL WATER QUALITY (2012 Data)<sup>1</sup></b>					
	NMGWQ			Detection	
Constituent	Numeric Std mg/l	PW-1	PW-3	Limit	
Arsenic	0.1	0.0033	0.0074		<b>Human Health Standard</b>
Barium	1.0	0.011	0.0078		
Cadmium	0.01	ND <sup>2</sup>	ND	0.002	
Chromium	0.05	ND	0.006		
Cyanide	0.2	ND	ND	0.01	
Fluoride	1.6	1.0	1.9		
Lead	0.05	ND	ND	0.005	
Mercury (total)	0.002	ND	ND	0.0002	
Nitrate	10.0	0.59	0.7		
Selenium	0.05	ND	ND	0.001	
Silver	0.05	ND	ND	0.005	
Uranium	0.03	0.0032	0.0013		
Chloride	250	32	50		
Copper	1.0	ND	ND	0.006	
Iron	1.0	0.04	0.065		
Manganese	0.2	0.0024	0.0026		
Sulfate	600	28	26		
TDS	1000	294	303		
Zinc	10.0	0.024	0.021		
pH	6 to 9	8.02	8.03		<b>Irrigation Use Standard</b>
Aluminum	5.0	ND	ND	0.02	
Boron	0.75	0.065	0.095		
Cobalt	0.05	ND	ND	0.006	
Molybdenum	1.0	ND	ND	0.008	
Nickel	0.2	ND	ND	0.01	

<sup>1</sup>2012 pump test data provided to NMED

<sup>2</sup>ND = none detect, below detection limit



Hall Environmental Analysis Laboratory  
4901 Hawkins NE  
Albuquerque, NM 87109  
TEL: 505-345-3975 FAX: 505-345-4107  
Website: [www.hallenvironmental.com](http://www.hallenvironmental.com)

May 14, 2012

Katie Emmer

New Mexico Copper Corp  
2425 San Pedro Dr NE Ste 100  
Albuquerque, New Mexico 87109  
TEL: (505) 400-7925  
FAX

RE: Cu Flat

OrderNo.: 1205076

Dear Katie Emmer:

Hall Environmental Analysis Laboratory received 1 sample(s) on 5/2/2012 for the analyses presented in the following report.

These were analyzed according to EPA procedures or equivalent. To access our accredited tests please go to [www.hallenvironmental.com](http://www.hallenvironmental.com) or the state specific web sites. See the sample checklist and/or the Chain of Custody for information regarding the sample receipt temperature and preservation. Data qualifiers or a narrative will be provided if the sample analysis or analytical quality control parameters require a flag. All samples are reported as received unless otherwise indicated.

Please don't hesitate to contact HEAL for any additional information or clarifications.

Sincerely,

A handwritten signature in black ink, appearing to read 'Andy Freeman', is written over a horizontal line.

Andy Freeman  
Laboratory Manager  
4901 Hawkins NE  
Albuquerque, NM 87109

**Hall Environmental Analysis Laboratory, Inc.**

**CLIENT:** New Mexico Copper Corp

**Client Sample ID:** PW-1

**Project:** Cu Flat

**Collection Date:** 5/1/2012 2:00:00 PM

**Lab ID:** 1205076-001

**Matrix:** AQUEOUS

**Received Date:** 5/2/2012 7:30:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>EPA METHOD 300.0: ANIONS</b>						Analyst: <b>BRM</b>
Fluoride	1.0	0.10		mg/L	1	5/2/2012 12:52:03 PM
Chloride	32	10		mg/L	20	5/2/2012 1:03:17 PM
Nitrogen, Nitrite (As N)	ND	0.10		mg/L	1	5/2/2012 12:52:03 PM
Nitrogen, Nitrate (As N)	0.59	0.10		mg/L	1	5/2/2012 12:52:03 PM
Sulfate	28	0.50		mg/L	1	5/2/2012 12:52:03 PM
<b>EPA METHOD 200.7: DISSOLVED METALS</b>						Analyst: <b>ELS</b>
Aluminum	ND	0.020		mg/L	1	5/8/2012 8:02:55 AM
Barium	0.011	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Beryllium	ND	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Boron	0.065	0.040		mg/L	1	5/9/2012 8:36:51 AM
Cadmium	ND	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Calcium	36	1.0		mg/L	1	5/9/2012 8:36:51 AM
Chromium	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Cobalt	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Copper	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Iron	0.040	0.020		mg/L	1	5/9/2012 8:36:51 AM
Lead	ND	0.0050		mg/L	1	5/8/2012 8:02:55 AM
Magnesium	3.1	1.0		mg/L	1	5/9/2012 8:36:51 AM
Manganese	0.0024	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Molybdenum	ND	0.0080		mg/L	1	5/8/2012 8:02:55 AM
Nickel	ND	0.010		mg/L	1	5/8/2012 8:02:55 AM
Potassium	3.4	1.0		mg/L	1	5/9/2012 8:36:51 AM
Silicon	17	0.40		mg/L	5	5/8/2012 8:06:09 AM
Silver	ND	0.0050		mg/L	1	5/8/2012 8:02:55 AM
Sodium	58	1.0		mg/L	1	5/9/2012 8:36:51 AM
Vanadium	ND	0.050		mg/L	1	5/8/2012 8:02:55 AM
Zinc	0.024	0.010		mg/L	1	5/8/2012 8:02:55 AM
<b>EPA 200.8: DISSOLVED METALS</b>						Analyst: <b>SNV</b>
Antimony	ND	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Arsenic	0.0033	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Selenium	ND	0.0010		mg/L	1	5/10/2012 2:28:58 PM
Thallium	ND	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Uranium	0.0032	0.0010		mg/L	1	5/10/2012 2:28:58 PM
<b>EPA METHOD 245.1: MERCURY</b>						Analyst: <b>ELS</b>
Mercury	ND	0.00020		mg/L	1	5/9/2012 11:59:45 AM
<b>SM2340B: HARDNESS</b>						Analyst: <b>ELS</b>
Hardness (As CaCO3)	100	6.6		mg/L	1	5/9/2012
<b>EPA 120.1: SPECIFIC CONDUCTANCE</b>						Analyst: <b>DBD</b>
Conductivity	450	0.010		µmhos/cm	1	5/7/2012 12:31:49 PM

**Qualifiers:** \*/X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits  
 S Spike Recovery outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

**Analytical Report**

Lab Order 1205076

Date Reported: 5/14/2012

**Hall Environmental Analysis Laboratory, Inc.**

**CLIENT:** New Mexico Copper Corp

**Client Sample ID:** PW-1

**Project:** Cu Flat

**Collection Date:** 5/1/2012 2:00:00 PM

**Lab ID:** 1205076-001

**Matrix:** AQUEOUS

**Received Date:** 5/2/2012 7:30:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>SM4500-H+B: PH</b>						Analyst: JLF
pH	8.02	1.68	H	pH units	1	5/3/2012 1:22:52 PM
<b>SM2320B: ALKALINITY</b>						Analyst: JLF
Bicarbonate (As CaCO3)	150	20		mg/L CaCO3	1	5/3/2012 1:22:52 PM
Carbonate (As CaCO3)	ND	2.0		mg/L CaCO3	1	5/3/2012 1:22:52 PM
Total Alkalinity (as CaCO3)	150	20		mg/L CaCO3	1	5/3/2012 1:22:52 PM
<b>SM2540C MOD: TOTAL DISSOLVED SOLIDS</b>						Analyst: KS
Total Dissolved Solids	294	20.0		mg/L	1	5/8/2012 3:12:00 PM
<b>SM 2540D: TSS</b>						Analyst: KS
Suspended Solids	ND	4.0		mg/L	1	5/3/2012 5:30:00 PM

**Qualifiers:** \*X Value exceeds Maximum Contaminant Level.  
E Value above quantitation range  
J Analyte detected below quantitation limits  
R RPD outside accepted recovery limits  
S Spike Recovery outside accepted recovery limits

B Analyte detected in the associated Method Blank  
H Holding times for preparation or analysis exceeded  
ND Not Detected at the Reporting Limit  
RL Reporting Detection Limit

# Anatek Labs, Inc.

1282 Alturas Drive • Moscow, ID 83843 • (208) 883-2839 • Fax (208) 882-9246 • email moscow@anateklabs.com  
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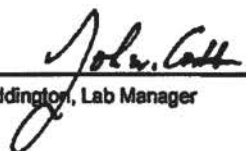
**Client:** HALL ENVIRONMENTAL ANALYSIS LAB      **Batch #:** 120503026  
**Address:** 4901 HAWKINS NE SUITE D      **Project Name:** 1205076  
ALBUQUERQUE, NM 87109  
**Attn:** ANDY FREEMAN

## Analytical Results Report

<b>Sample Number</b>	120503026-001	<b>Sampling Date</b>	5/1/2012	<b>Date/Time Received</b>	5/3/2012 12:24 PM
<b>Client Sample ID</b>	1205076-001D / PW-1	<b>Sampling Time</b>	2:00 PM	<b>Extraction Date</b>	
<b>Matrix</b>	Water	<b>Sample Location</b>			
<b>Comments</b>					

Parameter	Result	Units	PQL	Analysis Date	Analyst	Method	Qualifier
Cyanide	ND	mg/L	0.01	5/11/2012	CRW	EPA 335.4	

Authorized Signature

  
John Coddington, Lab Manager

MCL EPA's Maximum Contaminant Level  
ND Not Detected  
PQL Practical Quantitation Limit

This report shall not be reproduced except in full, without the written approval of the laboratory.  
The results reported relate only to the samples indicated.  
Soil/solid results are reported on a dry-weight basis unless otherwise noted.

Certifications held by Anatek Labs ID: EPA-ID00013; AZ:0701; CO:ID00013; FL(NELAP):E87893; ID:ID00013; IN:C-ID-01; KY:90142; MT:CERT0028; NM: ID00013; OR:ID200001-002; WA:C595  
Certifications held by Anatek Labs WA: EPA:WA00169; ID:WA00169; WA:C595; MT:Cert0095

Friday, May 11, 2012

Page 1 of 1

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# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076  
14-May-12

Client: New Mexico Copper Corp  
Project: Cu Flat

Sample ID	MB	SampType:	MBLK	TestCode:	EPA Method 200.7: Dissolved Metals						
Client ID:	PBW	Batch ID:	R2622	RunNo:	2622						
Prep Date:		Analysis Date:	5/8/2012	SeqNo:	72991		Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	ND	0.020									
Barium	ND	0.0020									
Beryllium	ND	0.0020									
Cadmium	ND	0.0020									
Chromium	ND	0.0060									
Cobalt	ND	0.0060									
Copper	ND	0.0060									
Lead	ND	0.0050									
Manganese	ND	0.0020									
Molybdenum	ND	0.0080									
Nickel	ND	0.010									
Silicon	ND	0.080									
Silver	ND	0.0050									
Vanadium	ND	0.050									
Zinc	ND	0.010									

Sample ID	LCS	SampType:	LCS	TestCode:	EPA Method 200.7: Dissolved Metals						
Client ID:	LCSW	Batch ID:	R2622	RunNo:	2622						
Prep Date:		Analysis Date:	5/8/2012	SeqNo:	72992		Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	0.52	0.020	0.5000	0	105	85	115				
Barium	0.49	0.0020	0.5000	0	98.9	85	115				
Beryllium	0.52	0.0020	0.5000	0	103	85	115				
Cadmium	0.50	0.0020	0.5000	0	99.2	85	115				
Chromium	0.49	0.0060	0.5000	0	98.5	85	115				
Cobalt	0.47	0.0060	0.5000	0	94.9	85	115				
Copper	0.50	0.0060	0.5000	0	99.9	85	115				
Lead	0.50	0.0050	0.5000	0	99.3	85	115				
Manganese	0.48	0.0020	0.5000	0	96.9	85	115				
Molybdenum	0.49	0.0080	0.5000	0.002030	98.4	85	115				
Nickel	0.47	0.010	0.5000	0	93.9	85	115				
Silicon	2.6	0.080	2.500	0	104	85	115				
Silver	0.094	0.0050	0.1000	0	94.1	85	115				
Vanadium	0.52	0.050	0.5000	0	104	85	115				
Zinc	0.50	0.010	0.5000	0	101	85	115				

Sample ID	1205193-005EMS	SampType:	MS	TestCode:	EPA Method 200.7: Dissolved Metals						
Client ID:	BatchQC	Batch ID:	R2622	RunNo:	2622						
Prep Date:		Analysis Date:	5/8/2012	SeqNo:	73030		Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2622		RunNo:	2622				
Prep Date:			Analysis Date:	5/8/2012		SeqNo:	73030		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	0.54	0.020	0.5000	0	107	70	130				
Barium	0.52	0.0020	0.5000	0.02182	98.9	70	130				
Zinc	0.54	0.010	0.5000	0.03785	101	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2622		RunNo:	2622				
Prep Date:			Analysis Date:	5/8/2012		SeqNo:	73031		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	0.53	0.020	0.5000	0	106	70	130	1.33	20		
Barium	0.51	0.0020	0.5000	0.02182	97.2	70	130	1.71	20		
Zinc	0.53	0.010	0.5000	0.03785	98.0	70	130	2.48	20		

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74182		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Potassium	56	1.0	50.00	4.808	102	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74183		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Potassium	57	1.0	50.00	4.808	104	70	130	2.44	20		

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74185		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Iron	4.5	0.10	2.500	2.034	99.6	70	130				
Magnesium	390	5.0	250.0	124.9	107	70	130				
Sodium	460	5.0	250.0	192.5	107	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74186		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Iron	4.6	0.10	2.500	2.034	101	70	130	1.03	20		

**Qualifiers:**

\* / X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	1205193-005EMSD	SampType:	MSD	TestCode:	EPA Method 200.7: Dissolved Metals					
Client ID:	BatchQC	Batch ID:	R2670	RunNo:	2670					
Prep Date:		Analysis Date:	5/9/2012	SeqNo:	74186	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Magnesium	390	5.0	250.0	124.9	106	70	130	0.684	20	
Sodium	460	5.0	250.0	192.5	106	70	130	0.966	20	

Sample ID	MB	SampType:	MBLK	TestCode:	EPA Method 200.7: Dissolved Metals					
Client ID:	PBW	Batch ID:	R2670	RunNo:	2670					
Prep Date:	5/9/2012	Analysis Date:	5/9/2012	SeqNo:	74215	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	ND	0.040								
Calcium	ND	1.0								
Iron	ND	0.020								
Magnesium	ND	1.0								
Potassium	ND	1.0								
Sodium	ND	1.0								

Sample ID	LCS	SampType:	LCS	TestCode:	EPA Method 200.7: Dissolved Metals					
Client ID:	LCSW	Batch ID:	R2670	RunNo:	2670					
Prep Date:		Analysis Date:	5/9/2012	SeqNo:	74216	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	0.51	0.040	0.5000	0	101	85	115			
Calcium	54	1.0	50.00	0	107	85	115			
Iron	0.47	0.020	0.5000	0.004190	93.2	85	115			
Magnesium	54	1.0	50.00	0	109	85	115			
Potassium	53	1.0	50.00	0	106	85	115			
Sodium	54	1.0	50.00	0	107	85	115			

**Qualifiers:**

\* / X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit



**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA 200.8: Dissolved Metals</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2629</b>	RunNo:	<b>2629</b>					
Prep Date:		Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73283</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	0.023	0.0010	0.02500	0	92.8	85	115			
Arsenic	0.023	0.0010	0.02500	0	93.1	85	115			
Thallium	0.023	0.0010	0.02500	0	92.9	85	115			

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA 200.8: Dissolved Metals</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>R2629</b>	RunNo:	<b>2629</b>					
Prep Date:		Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73284</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	ND	0.0010								
Arsenic	ND	0.0010								
Thallium	ND	0.0010								

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA 200.8: Dissolved Metals</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2708</b>	RunNo:	<b>2708</b>					
Prep Date:		Analysis Date:	<b>5/10/2012</b>	SeqNo:	<b>75447</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	0.026	0.0010	0.02500	0	104	85	115			
Uranium	0.025	0.0010	0.02500	0	99.2	85	115			

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA 200.8: Dissolved Metals</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>R2708</b>	RunNo:	<b>2708</b>					
Prep Date:		Analysis Date:	<b>5/10/2012</b>	SeqNo:	<b>75448</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	ND	0.0010								
Uranium	ND	0.0010								

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	MB-1862	SampType:	MBLK	TestCode:	EPA Method 245.1: Mercury					
Client ID:	PBW	Batch ID:	1862	RunNo:	2669					
Prep Date:	5/9/2012	Analysis Date:	5/9/2012	SeqNo:	74223	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	ND	0.00020								

Sample ID	LCS-1862	SampType:	LCS	TestCode:	EPA Method 245.1: Mercury					
Client ID:	LCSW	Batch ID:	1862	RunNo:	2669					
Prep Date:	5/9/2012	Analysis Date:	5/9/2012	SeqNo:	74224	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.4	80	120			

Sample ID	1204854-004AMS	SampType:	MS	TestCode:	EPA Method 245.1: Mercury					
Client ID:	BatchQC	Batch ID:	1862	RunNo:	2669					
Prep Date:	5/9/2012	Analysis Date:	5/9/2012	SeqNo:	74226	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.2	75	125			

Sample ID	1204854-004AMSD	SampType:	MSD	TestCode:	EPA Method 245.1: Mercury					
Client ID:	BatchQC	Batch ID:	1862	RunNo:	2669					
Prep Date:	5/9/2012	Analysis Date:	5/9/2012	SeqNo:	74227	Units:	mg/L			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.1	75	125	0.0957	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

Client: New Mexico Copper Corp

Project: Cu Flat

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70797</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	ND	0.10								
Chloride	ND	0.50								
Nitrogen, Nitrite (As N)	ND	0.10								
Nitrogen, Nitrate (As N)	ND	0.10								
Sulfate	ND	0.50								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70798</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.47	0.10	0.5000	0	93.8	90	110			
Chloride	4.6	0.50	5.000	0	92.9	90	110			
Nitrogen, Nitrite (As N)	0.93	0.10	1.000	0	92.9	90	110			
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	97.4	90	110			
Sulfate	9.5	0.50	10.00	0	94.8	90	110			

Sample ID <b>1205075-001BMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70800</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.68	0.10	0.5000	0.1911	98.1	72.9	113			
Nitrogen, Nitrite (As N)	1.0	0.10	1.000	0	101	77.6	111			
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	99.9	82.8	116			

Sample ID <b>1205075-001BMSD</b>	SampType: <b>MSD</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70801</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.65	0.10	0.5000	0.1911	90.9	72.9	113	5.39	20	
Nitrogen, Nitrite (As N)	0.90	0.10	1.000	0	90.2	77.6	111	10.8	20	
Nitrogen, Nitrate (As N)	2.3	0.10	2.500	0	91.3	82.8	116	8.94	20	

Sample ID <b>1205079-001AMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70809</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Nitrogen, Nitrite (As N)	1.3	0.10	1.000	0	127	77.6	111			S
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	97.8	82.8	116			

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076  
14-May-12

Client: New Mexico Copper Corp  
Project: Cu Flat

Sample ID	1205079-001AMSD	SampType:	MSD	TestCode:	EPA Method 300.0: Anions						
Client ID:	BatchQC	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70810	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Nitrogen, Nitrite (As N)	1.2	0.10	1.000	0	122	77.6	111	4.14	20	S	
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	95.5	82.8	116	2.38	20		

Sample ID	MB	SampType:	MBLK	TestCode:	EPA Method 300.0: Anions						
Client ID:	PBW	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70849	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	ND	0.10									
Chloride	ND	0.50									
Nitrogen, Nitrite (As N)	ND	0.10									
Nitrogen, Nitrate (As N)	ND	0.10									
Sulfate	ND	0.50									

Sample ID	LCS	SampType:	LCS	TestCode:	EPA Method 300.0: Anions						
Client ID:	LCSW	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70850	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	0.50	0.10	0.5000	0	99.0	90	110				
Chloride	4.7	0.50	5.000	0	94.2	90	110				
Nitrogen, Nitrite (As N)	0.98	0.10	1.000	0	98.0	90	110				
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	98.3	90	110				
Sulfate	9.6	0.50	10.00	0	95.7	90	110				

Sample ID	1205066-002AMS	SampType:	MS	TestCode:	EPA Method 300.0: Anions						
Client ID:	BatchQC	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70852	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	1.1	0.10	0.5000	0.5616	101	72.9	113				
Nitrogen, Nitrite (As N)	0.93	0.10	1.000	0	92.7	77.6	111				
Nitrogen, Nitrate (As N)	3.3	0.10	2.500	0.5059	111	82.8	116				
Sulfate	48	0.50	10.00	36.66	113	80.5	119				

Sample ID	1205066-002AMSD	SampType:	MSD	TestCode:	EPA Method 300.0: Anions						
Client ID:	BatchQC	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70853	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	1.0	0.10	0.5000	0.5616	93.8	72.9	113	3.52	20		
Nitrogen, Nitrite (As N)	0.79	0.10	1.000	0	78.5	77.6	111	16.5	20		

### Qualifiers:

\* / X Value exceeds Maximum Contaminant Level.  
E Value above quantitation range  
J Analyte detected below quantitation limits  
R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
H Holding times for preparation or analysis exceeded  
ND Not Detected at the Reporting Limit  
RL Reporting Detection Limit

**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	1205066-002AMSD	SampType:	MSD	TestCode:	EPA Method 300.0: Anions						
Client ID:	BatchQC	Batch ID:	R2544	RunNo:	2544						
Prep Date:		Analysis Date:	5/2/2012	SeqNo:	70853	Units:	mg/L				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Nitrogen, Nitrate (As N)	3.0	0.10	2.500	0.5059	98.7	82.8	116	10.2	20		
Sulfate	47	0.50	10.00	36.66	101	80.5	119	2.50	20		

**Qualifiers:**

\* / X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	1205170-001D	SampType:	DUP	TestCode:	EPA 120.1: Specific Conductance					
Client ID:	BatchQC	Batch ID:	R2646	RunNo:	2646					
Prep Date:		Analysis Date:	5/7/2012	SeqNo:	73516	Units:	µmhos/cm			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Conductivity	610	0.010						0	20	

**Qualifiers:**

\* / X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	1205005-001A DUP	SampType:	DUP	TestCode:	SM4500-H+B: pH					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71363					
				Units:	pH units					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	3.92	1.68						0.762		H

Sample ID	1205120-001B DUP	SampType:	DUP	TestCode:	SM4500-H+B: pH					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71373					
				Units:	pH units					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	7.73	1.68						0.645		H

**Qualifiers:**

\*X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	1205005-001A MS	SampType:	MS	TestCode:	SM2320B: Alkalinity					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71221					
				Units:	mg/L CaCO3					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	62.6	110			S

Sample ID	1205005-001A MSD	SampType:	MSD	TestCode:	SM2320B: Alkalinity					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71222					
				Units:	mg/L CaCO3					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	59.9	111	0	10	S

Sample ID	1205120-001B MS	SampType:	MS	TestCode:	SM2320B: Alkalinity					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71242					
				Units:	mg/L CaCO3					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	360	20	80.00	299.4	70.9	62.6	110			

Sample ID	1205120-001B MSD	SampType:	MSD	TestCode:	SM2320B: Alkalinity					
Client ID:	BatchQC	Batch ID:	R2560	RunNo:	2560					
Prep Date:		Analysis Date:	5/3/2012	SeqNo:	71243					
				Units:	mg/L CaCO3					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	350	20	80.00	299.4	67.1	59.9	111	0.869	10	

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit



# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID	<b>MB-1832</b>	SampType:	<b>MBLK</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73329</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	ND	20.0								

Sample ID	<b>LCS-1832</b>	SampType:	<b>LCS</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73330</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	1,020	20.0	1,000	0	102	80	120			

Sample ID	<b>1205078-002GMS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73337</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,890	20.0	1,000	3,791	110	80	120			

Sample ID	<b>1205078-002GMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73338</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,930	20.0	1,000	3,791	114	80	120	0.733	20	

**Qualifiers:**

\* / X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

**QC SUMMARY REPORT**  
**Hall Environmental Analysis Laboratory, Inc.**

WO#: 1205076  
 14-May-12

**Client:** New Mexico Copper Corp  
**Project:** Cu Flat

Sample ID <b>MB-1800</b>	SampType: <b>MBLK</b>	TestCode: <b>SM 2540D: TSS</b>								
Client ID: <b>PBW</b>	Batch ID: <b>1800</b>	RunNo: <b>2570</b>								
Prep Date: <b>5/3/2012</b>	Analysis Date: <b>5/3/2012</b>	SeqNo: <b>71656</b>	Units: <b>mg/L</b>							
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0								

Sample ID <b>LCS-1800</b>	SampType: <b>LCS</b>	TestCode: <b>SM 2540D: TSS</b>								
Client ID: <b>LCSW</b>	Batch ID: <b>1800</b>	RunNo: <b>2570</b>								
Prep Date: <b>5/3/2012</b>	Analysis Date: <b>5/3/2012</b>	SeqNo: <b>71657</b>	Units: <b>mg/L</b>							
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	93	4.0	96.60	0	96.3	82.9	110			

Sample ID <b>1205034-001BDUP</b>	SampType: <b>DUP</b>	TestCode: <b>SM 2540D: TSS</b>								
Client ID: <b>BatchQC</b>	Batch ID: <b>1800</b>	RunNo: <b>2570</b>								
Prep Date: <b>5/3/2012</b>	Analysis Date: <b>5/3/2012</b>	SeqNo: <b>71663</b>	Units: <b>mg/L</b>							
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0						0	15	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit



Hall Environmental Analysis Laboratory  
 4901 Hawkins NE  
 Albuquerque, NM 87105  
 TEL: 505-345-3975 FAX: 505-345-4107  
 Website: www.hallenvironmental.com

# Sample Log-In Check List

Client Name: NEW MEXICO COPPER CORP	Work Order Number: 1205076
Received by/date: <u>AT 05/02/12</u>	
Logged By: Anne Thorne 5/2/2012 7:30:00 AM	<i>Anne Thorne</i>
Completed By: Anne Thorne 5/2/2012	<i>Anne Thorne</i>
Reviewed By: <u>AT 05/02/12</u>	

**Chain of Custody**

1. Were seals intact? Yes  No  Not Present
2. Is Chain of Custody complete? Yes  No  Not Present
3. How was the sample delivered? Client

**Log In**

4. Coolers are present? (see 19. for cooler specific information) Yes  No  NA
5. Was an attempt made to cool the samples? Yes  No  NA
6. Were all samples received at a temperature of >0° C to 8.0°C Yes  No  NA
7. Sample(s) in proper container(s)? Yes  No
8. Sufficient sample volume for indicated test(s)? Yes  No
9. Are samples (except VOA and ONG) properly preserved? Yes  No
10. Was preservative added to bottles? Yes  No  NA
11. VOA vials have zero headspace? Yes  No  No VOA Vials
12. Were any sample containers received broken? Yes  No
13. Does paperwork match bottle labels? (Note discrepancies on chain of custody) Yes  No
14. Are matrices correctly identified on Chain of Custody? Yes  No
15. Is it clear what analyses were requested? Yes  No
16. Were all holding times able to be met? (If no, notify customer for authorization.) Yes  No

# of preserved bottles checked for pH: 2

(2 or 12 unless noted)

Adjusted? \_\_\_\_\_

Checked by AT 05/02/12

**Special Handling (if applicable)**

17. Was client notified of all discrepancies with this order? Yes  No  NA

Person Notified: _____	Date: _____
By Whom: _____	Via: <input type="checkbox"/> eMail <input type="checkbox"/> Phone <input type="checkbox"/> Fax <input type="checkbox"/> In Person
Regarding: _____	
Client Instructions: _____	

18. Additional remarks:

**19. Cooler Information**

Cooler No	Temp °C	Condition	Seal Intact	Seal No	Seal Date	Signed By
1	3.4	Good	Not Present			

# Chain-of-Custody Record

Client: New Mexico Copper Corp

Mailing Address: 2425 San Pedro Dr NE  
Suite 100, AEBQ NM

Phone #: 505-400-7925

email or Fax#:

QA/QC Package:  
 Standard  Level 4 (Full Validation)

Accreditation  
 NELAP  Other \_\_\_\_\_

EDD (Type) \_\_\_\_\_

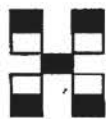
Turn-Around Time:  
 Standard  Rush Need Results by May 11 visened

Project Name: Cu Flat

Project #: Production Well Sampling

Project Manager: Katie Emmer

Sampler: CMC



## HALL ENVIRONMENTAL ANALYSIS LABORATORY

www.hallenvironmental.com  
4901 Hawkins NE - Albuquerque, NM 87109  
Tel. 505-345-3975 Fax 505-345-4107

### Analysis Request

BTEX + MTBE + TMB's (8021)	BTEX + MTBE + TPH (Gas only)	TPH Method 8015B (Gas/Diesel)	TPH (Method 418.1)	EDB (Method 504.1)	8310 (PNA or PAH)	RCRA 8 Metals	Anions (F, Cl, NO <sub>3</sub> , NO <sub>2</sub> , PO <sub>4</sub> , SO <sub>4</sub> )	8081 Pesticides / 8082 PCB's	8260B (VOA)	8270 (Semi-VOA)	see enclosed list	Air Bubbles (Y or N)

Date	Time	Matrix	Sample Request ID	Container Type and #	Preservative Type	Remarks
5/1/12	1400	H <sub>2</sub> O	PW-1	500		-CO
				125	H <sub>2</sub> SO <sub>4</sub>	-CO
				125	HNO <sub>3</sub> + filter	-CO
				500	HNO <sub>3</sub>	-CO
				500	NaOH	-CO

Date: 5/1/12 Time: 14:30 Relinquished by: [Signature]

Date: 5/2/12 Time: 07:30 Relinquished by: [Signature]

Date: May 2012 Time: 14:30 Received by: [Signature]

Date: 5/2/12 Time: 07:30 Received by: [Signature]

Remarks: Please email results to: Katie Emmer  
kemmer@themacresourcesgroup.com  
Please add hardness, per Katie of 5/3

If necessary, samples submitted to Hall Environmental may be subcontracted to other accredited laboratories. This serves as notice of this possibility. Any sub-contracted data will be clearly notated on the analytical report.

NMI Copper  
May 1, 2012

**Table 9-3**  
**Analytical Parameters and Analysis Methods for Groundwater Samples**

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
<b>Anions</b>		
Fluoride	EPA Method 300.0	0.1
Chloride	EPA Method 300.0	0.1
Nitrogen, Nitrite (as N)	EPA Method 300.0	0.1
Nitrogen, Nitrate (as N)	EPA Method 300.0	0.1
Sulfate	EPA Method 300.0	0.5
<b>Dissolved Metals</b>		
Aluminum	EPA Method 200.7	0.02
Antimony	EPA Method 200.8	0.005
Arsenic	EPA Method 200.8	0.02
Barium	EPA Method 200.7	0.002
Beryllium	EPA Method 200.7	0.002
Boron	EPA Method 200.7	0.04
Cadmium	EPA Method 200.7	0.002
Calcium	EPA Method 200.7	0.50
Chromium	EPA Method 200.7	0.006
Cobalt	EPA Method 200.7	0.006
Copper	EPA Method 200.7	0.0003
Iron	EPA Method 200.7	0.02
Lead	EPA Method 200.7	0.005
Magnesium	EPA Method 200.7	0.50
Manganese	EPA Method 200.7	0.002
Mercury	EPA Method 7470 CVAA	0.0002
Molybdenum	EPA Method 200.7	0.008
Nickel	EPA Method 200.7	0.01
Potassium	EPA Method 200.7	1.0
Selenium	EPA Method 200.8	0.02
Silicon	EPA Method 200.7	0.08
Silver	EPA Method 200.7	0.005
Sodium	EPA Method 200.7	0.5

NM Copper  
May 1, 2012

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
Thallium	EPA Method 200.7	0.01
<del>Titanium</del>	<del>EPA Method 200.7</del>	<del>0.005</del>
Uranium	EPA Method 200.8	0.01
Vanadium	EPA Method 200.7	0.005
Zinc	EPA Method 200.7	0.005
Solids		
Total Suspended Solids (TSS)	SM 2540D	1.0 µg/L
Total Dissolved Solids (TDS)	SM 2540C	10
Alkalinity		
Alkalinity, total (as CaCO <sub>3</sub> )	SM 2320B	20
Carbonate	SM 2320B	20
Bicarbonate	SM 2320B	20
Other		
pH	150.1	12.45
Specific Conductance	120.1	0.01 µS/cm
Cyanide	Kelada-01	0.005

Note: NA = not applicable as sample will not be analyzed for a given parameter.





SUSANA MARTINEZ  
Governor  
JOHN A. SANCHEZ  
Lt. Governor

NEW MEXICO  
ENVIRONMENT DEPARTMENT

Harold Runnels Building  
1190 Saint Francis Drive (87505)  
PO Box 5469, Santa Fe, NM 87502-5469  
Phone (505) 827-2990 Fax (505) 827-1628  
[www.env.nm.gov](http://www.env.nm.gov)



BUTCH TONGATE  
Cabinet Secretary  
J.C. BORREGO  
Deputy Secretary

October 23, 2017

VIA E-MAIL

Jaimie Park  
[jpark@nmelec.org](mailto:jpark@nmelec.org)

Re: Request to Inspect Public Records

Dear Ms. Park:

On October 23, 2017, this office received your request for public information. You request information pertaining to: Copper Flat Copper Mine's pending discharge permit application. (See attached request).

I forwarded your request to the bureaus on October 23, 2017. The bureaus will respond by November 6, 2017.

Should you have any questions, please contact the Ground Water Quality Bureau at (505) 827-2919 and the Surface Water Quality Bureau at (505) 827-2819.

Sincerely,

Melissa Y. Mascareñas  
New Mexico Environment Department  
Department Public Records Custodian

cc: Andrew Knight, Assistant General Counsel  
Michelle Hunter, Chief, Ground Water Quality Bureau  
Shelly Lemon, Chief, Surface Water Quality Bureau





**NEW MEXICO ENVIRONMENT DEPARTMENT  
INSPECTION OF PUBLIC RECORD REQUEST FORM**

Please fill out the following information:

1. Date: September 6, 2017
2. Requestor's Name: Jaimie Park, New Mexico Environmental Law Center
3. Requestor's Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505
4. Phone No.: (505) 989-9022
5. Email: jpark@nmelc.org
6. Company Being Represented: New Mexico Environmental Law Center
7. Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505
8. Document or File being requested to be reviewed or copied (please describe the records in sufficient detail to enable Department personnel to reasonably identify & locate the records:

I request that you inform me what documents are available within the scope of this request and when I can inspect those documents. I also request that you not make any copies without first informing me of the number of copies and the cost for the copying that are involved. Finally, I request that if you determine that any documents or portions of documents are exempt from disclosure you inform me of that and provide me with citations to the provisions in the Inspection of Public Records Act that indicate that the documents or portions of documents are exempt from disclosure, describe the type of document being withheld, and identify who sent the document being withheld and who received the document being withheld.

**Records being requested:**

1. NMED determination regarding Copper Flat Copper Mine pit lake being a private water, date range June 28, 2017 through June 19, 2017;
2. Bureau of Land Management request to the Surface Water Quality Bureau to write a letter of concurrence for the Surface Water Quality Bureau's benefit regarding the proposed Copper Flat Copper Mine pit lake and associated patented claims surveys completed and a finding that the pit lake constitutes a "private water" and is not subject to either surface or groundwater quality standards;

3. New Mexico Copper Corporation submittals to NMED pertaining to its discharge permit application since July 17, 2017; and
4. Copper Flat Copper Mine cooperating agency meeting notes for August 2017 meeting (should be August 30, 2017 meeting date).

9. NMED Bureau where Document/File can be found (if known): Surface Water Quality Bureau, Ground Water Quality Bureau

/s/ Jaimie Park

Signature

The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: melissa.mascarenas@state.nm.us**





SUSANA MARTINEZ  
Governor  
JOHN A. SANCHEZ  
Lt. Governor

NEW MEXICO  
ENVIRONMENT DEPARTMENT

Harold Runnels Building  
1190 Saint Francis Drive (87505)  
PO Box 5469, Santa Fe, NM 87502-5469  
Phone (505) 827-2990 Fax (505) 827-1628  
[www.env.nm.gov](http://www.env.nm.gov)



BUTCH TONGATE  
Cabinet Secretary  
J.C. BORREGO  
Deputy Secretary

November 9, 2017

VIA E-MAIL

Jaimie Park  
[jpark@nmelc.org](mailto:jpark@nmelc.org)

Re: Request to Inspect Public Records

Dear Ms. Park:

On November 9, 2017, this office received your request for public information. You request information pertaining to: All drafts of a discharge permit for the Copper Flat Copper Mine and associated staff notes and correspondence pertaining to the drafting of the discharge permit. (See attached request).

I forwarded your request to the bureaus on November 9, 2017. The bureaus will respond by November 27, 2017.

Should you have any questions, please contact the Ground Water Quality Bureau at (505) 827-2919 and the Surface Water Quality Bureau at (505) 827-2819.

Sincerely,

Melissa Y. Mascareñas  
New Mexico Environment Department  
Department Public Records Custodian

cc: Andrew Knight, Assistant General Counsel  
Michelle Hunter, Chief, Ground Water Quality Bureau  
Shelly Lemon, Chief, Surface Water Quality Bureau



**NEW MEXICO ENVIRONMENT DEPARTMENT  
INSPECTION OF PUBLIC RECORD REQUEST FORM**

Please fill out the following information:

1. Date: October 21, 2017
2. Requestor's Name: Jaimie Park, New Mexico Environmental Law Center
3. Requestor's Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505  
\_\_\_\_\_
4. Phone No.: (505) 989-9022
5. Email: jpark@nmelc.org
6. Company Being Represented: New Mexico Environmental Law Center
7. Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505
8. Document or File being requested to be reviewed or copied (please describe the records in sufficient detail to enable Department personnel to reasonably identify & locate the records:

I request that you inform me what documents are available within the scope of this request and when I can inspect those documents. I also request that you not make any copies without first informing me of the number of copies and the cost for the copying that are involved. Finally, I request that if you determine that any documents or portions of documents are exempt from disclosure you inform me of that and provide me with citations to the provisions in the Inspection of Public Records Act that indicate that the documents or portions of documents are exempt from disclosure, describe the type of document being withheld, and identify who sent the document being withheld and who received the document being withheld.

**Records being requested:**

1. NMED determination regarding Copper Flat Copper Mine pit lake being a private water, date range July 2017 through date of this request;
2. Bureau of Land Management letter of concurrence for NMED's/Surface Water Quality Bureau's benefit regarding the proposed Copper Flat Copper Mine pit lake and associated patented claims surveys completed and a finding that the pit lake constitutes a "private water" and is not subject to either surface or groundwater quality standards;

3. New Mexico Copper Corporation submittals to NMED pertaining to its discharge permit application since July 2017;
  4. New Mexico Copper Corporation submittals to NMED pertaining to the pit lake since July 2017;
  5. Copper Flat Copper Mine cooperating agency meeting notes for September and October 2017 meetings; and
  6. NMED's October 2017 determination regarding technical completeness for New Mexico Copper Corporation's discharge permit application.
9. NMED Bureau where Document/File can be found (if known): Surface Water Quality Bureau, Ground Water Quality Bureau

/s/ Jaimie Park  
Signature

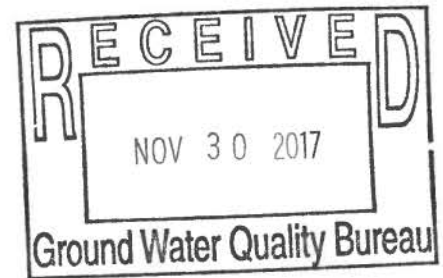
The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: melissa.mascarenas@state.nm.us**



November 30, 2017

Mr. David Ennis  
Reclamation Specialist  
New Mexico Energy, Minerals and Natural Resources Department  
Mining and Minerals Division  
1220 South St. Francis Drive  
Santa Fe, NM 87505



RE: NMMC Response to MMD Additional Technical Comments On:

- Updated Mining Operation and Reclamation Plan, Rev. 1, July 2017;
- Response for Additional Information dated October 13, 2017;

Copper Flat Mine, Sierra County, Permit Tracking No. SI027RN

Dear Mr. Ennis:

The Mining and Minerals Division (MMD) provided New Mexico Copper Corporation (NMCC) with additional technical comments on the Company's proposed Mining Operation and Reclamation Plan in a letter dated November 20, 2017. NMCC has reviewed MMD's comments and a response to each is provided herein.

**MMD Comment 1:**

*NMCC's Item #32 response comment states that rip-rap for reclamation may be sourced from EWRSP-2(A) and EWRSP-2B. NMCC's baseline data report shows that EWRSP-2A and EWRSP-2B can contain high sulfide/low paste pH material, therefore MMD has concerns about the proposal to utilize these piles as a source for rip-rap. A detailed material handling plan describing how clean rip-rap would be tested, identified and segregated from these piles would be required as a future permit condition in order to proceed with this proposal. Alternatively, NMCC may wish to eliminate this proposal from the reclamation plan and source rip-rap from the TSF salvaged material and/or proposed andesite breccia quarry at the WRSP-3 location. Please address.*

**NMCC Response**

NMCC acknowledges MMD's concern and agrees to eliminate EWRSP-2A and EWRSP-2B as sources for rip-rap. NMCC will source rip-rap either from material salvaged during TSF construction or from an andesite breccia quarry located within the footprint of the future WRSP-3.

**MMD Comment 2**

*Plan for reclamation/revegetation of the 4900 expanded catch bench. As proposed, this bench will not be contiguous with other reclaimed areas within the pit. Please address the possibility of*



*performing some additional reclamation to connect the 4900 expanded catch bench to the haul road reclamation.*

**NMCC Response**

NMCC has considered the potential to connect the 4900 expanded catch bench (expanded bench) with the pit haul road as requested by MMD. NMCC notes that the Copper Flat pit design includes a 38-foot wide safety catch bench left in the final pit wall at the 4900 elevation that will physically connect the expanded bench to the haul road above the reformed pit lake waterline and that this safety catch bench will allow wildlife movement between the haul road and the expanded catch bench.

Vehicle access from the haul road to the expanded bench would require construction of a land bridge after mining is complete or widening the safety catch bench along the final pit wall. Construction of a land bridge would require importing approximately 200,000 cubic feet of material into the pit after mining is complete. Re-designing the pit wall to provide a safety catch bench with sufficient width for safe access by personnel would result in significant loss of ore. As a result, NMCC concludes that providing enhanced access to the expanded catch bench by either of these two methods beyond that which will exist as a result of the 38-foot safety catch bench is not practical and is unnecessary.

**MMD Comment 3 (Reference Appendix E, Section 2.5.2):**

- a. *Please identify where the land bridge material will be placed at closure when removed from Grayback arroyo.*

**NMCC Response:**

Material removed from Grayback Arroyo crossings will be used for reclamation grading in the process area. This material will be suitable to meet reclamation needs such as: backfilling foundation excavations and water impoundments; regrading out slopes in the process area; backfilling the tailings pipeline excavation; and backfilling and regrading at the cyclone plant. Excess material, if generated, will be taken and blended into reclamation grading at a mine waste rock stockpiles or at the TSF.

- b. *The majority of the plant area is proposed to be covered with 6 inches of growth media, however the slopes along the perimeter of the plant area will be graded to a slope of 3.0H1V and covered with 36 inches of growth media. Please explain why the slopes are proposed to be covered with 36 inches of growth media while majority of the regraded plant area will be covered with 6 inches of growth media.*

**NMCC Response:**

Bullets 6 and 11 of Section 2.5.2 contain inadvertent typographical errors. The material present in the process area out slopes is non-mine waste that does not require additional cover. The depth of cover discussed therein is intended to be 6 inches to match the proposed cover plan for the general plant area.

**MMD Comment 4:**

*In Appendix E, Table E6, please describe or identify on a map the facility identified as "West Pit Buildup" and what this buildup entails. Is this buildup associated with EWRSP-1 reclamation?*

**NMCC Response:**

MMD's observation is correct. The West Pit Buildup is associated with EWRSP-1 reclamation and represents material required to contour and grade the area to promote positive drainage away from the pit crest and direct stormwater to proposed toe channels TC-2 and TC-3 as shown in Figure C-002 of the Reclamation Plan.

**MMD Comment 5:**

*The structure(s) to the south of the proposed tailings impoundment currently utilized as the Copper Flat mine office do not appear to be included in the MORP Rev 1. Please address the final disposition of these buildings at closure.*

**NMCC Response:**

Three structures are located south of the proposed tailings impoundment: a wooden barn; an adobe house; and a concrete block building. All three structures are located on private property owned by NMCC. The barn is currently used by a local rancher to store ranch materials and supplies. The house structure is not occupied. NMCC occupies the concrete block building and is currently using the building as a site office, shop, and warehouse. NMCC intends to continue to use the block building during operations and during reclamation of the site. At closure, the future of these structures will be determined by their suitability for ongoing ranching operations that are expected to continue post mining and according to their significance as cultural properties as determined by actions contemplated in the Programmatic Agreement between constituent agencies and interested parties.

**MMD Comment 6:**

*The New Mexico Department of Game and Fish has provided comments on NMCC's October 13, 2017 submittal, which is attached and shall be addressed by NMCC.*

*NMDG&F-a. The NM Department of Game and Fish (Department) supports the potential habitat value of pit highwalls as part of the post-mining reclamation plan. However, in order for vertical pit highwalls to provide adequate wildlife habitat they need to contain habitat features that attract wildlife species which utilize cliffs for nesting and shelter. Verticality alone does not create adequate cliff habitat for wildlife. The Department recommends that pit highwalls are enhanced by design features that are of value to wildlife. These would include; creating ledges, niches, alcoves, and horizontal holes on the cliff face for nesting raptors and other bird species that utilize ledges and cavities, and by placing rock piles along the toe of the highwall that mimics talus slopes for small mammals and reptiles.*

**NMCC Response:**

NMCC acknowledges the Department's expertise with developing wildlife habitat and hereby commits to creating ledges, niches, alcoves, and horizontal holes on the pit wall for nesting raptors and other bird species as suggested in the Department's comment. Furthermore, NMCC commits to further enhance the value to wildlife of the future pit habitat by placing rock piles along the highwall toes to mimic talus slopes for small mammals and reptiles. NMCC believes that many of these desired features will manifest themselves naturally in the course of mine development and operations. Nonetheless, NMCC will take action during mining operations to ensure the features described herein are developed and, as noted in our October 13, 2017 submittal, NMCC will actively pursue the assistance of the NMDG&F to maximize those opportunities.

*NMDG&F-b. The Department suggests extending the "Expanded 4900 Catch Bench" as far as it technically feasible, so that it would gently slope into the pit and be submerged after the rapid-fill process is complete. This would create a littoral zone in the pit lake and allow for the establishment and growth of aquatic plants and would further enhance its value as wildlife habitat.*

**NMCC Response:**

Extending the expanded 4900 catch bench would result in significant loss of ore and affect project value and therefore this action is not practical. Additionally, developing a littoral zone around the full perimeter of the expanded catch bench as suggested by the Department would require a significant quantity of material to construct and this action is also not practical. However, as NMCC points out in the October 13 submittal to MMD, reclaiming the expanded catch bench will require construction of an access ramp before rapid fill of the pit. That ramp would be left in place after reclamation activities are complete and therefore would become submerged by the future pit lake, thus creating a section of littoral zone as suggested by the Department.

NMCC appreciates the opportunity to continue to work with the MMD in pursuit of approval of Mining Permit No. SI027RN. Please do not hesitate to contact us if you have any questions or require clarification.

Sincerely,



Jeff Smith  
Chief Operating Officer  
THEMAC Resources Group/New Mexico Copper Corporation  
Telephone: (520) 991-4588

cc: Brad Reid – New Mexico Environment Department



## Reid, Brad, NMENV

---

**From:** Knight, Andrew, NMENV  
**Sent:** Friday, December 01, 2017 11:35 AM  
**To:** Vollbrecht, Kurt, NMENV; Reid, Brad, NMENV  
**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

Absolutely. Tell NMCC that if they have reason to believe that BLM does not have a problem with the surveys, and it is just a matter of their reluctance to put something in writing, I would be glad to contact BLM on behalf of SWQB.

---

**From:** Vollbrecht, Kurt, NMENV  
**Sent:** Friday, December 01, 2017 11:30 AM  
**To:** Knight, Andrew, NMENV <Andrew.Knight@state.nm.us>; Reid, Brad, NMENV <brad.reid@state.nm.us>  
**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

Ok.

We are meeting with NMCC on the 13<sup>th</sup> so we will tell them they need to get BLM to send something that indicates concurrence with the surveys.

Once that is provided NMCC can overlay the current and future pit lake (that is the one that matters) onto the plat.

Thanks Andrew.

Kurt Vollbrecht, Program Manager  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
(505) 827-0195

---

**From:** Knight, Andrew, NMENV  
**Sent:** Friday, December 1, 2017 11:26 AM  
**To:** Vollbrecht, Kurt, NMENV <kurt.vollbrecht@state.nm.us>; Reid, Brad, NMENV <brad.reid@state.nm.us>  
**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

Well,

I have a slightly different take on it. As the adjoining landowner whose boundaries are affected by this survey, BLM does need to acknowledge the validity of the surveys in writing, or at least say that they do not plan on contesting them. If the applicant can't get BLM to say either of those things, it indicates to me that BLM has some kind of problem with the surveys, and if that is the case, we cannot make the determination. It is BLM as the affected landowner, not so much as another government agency here. If NMCC can't get BLM to acknowledge the validity of the surveys, we have a big problem.

---

**From:** Vollbrecht, Kurt, NMENV  
**Sent:** Thursday, November 30, 2017 4:48 PM  
**To:** Reid, Brad, NMENV <brad.reid@state.nm.us>  
**Cc:** Knight, Andrew, NMENV <Andrew.Knight@state.nm.us>  
**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

Ok. I'd suggest that if SWQB wants something from BLM they should ask BLM for that.

That is how we would handle that. We would not tell a permittee (or applicant) to get a concurrence from BLM.

BLM may not want to provide it. And it should not fall on the permittee to convince another government agency to provide something we desire. Seems to me that SWQB should call BLM.

Andrew?

Kurt Vollbrecht, Program Manager  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
(505) 827-0195

---

**From:** Reid, Brad, NMENV  
**Sent:** Thursday, November 30, 2017 4:04 PM  
**To:** Vollbrecht, Kurt, NMENV <kurt.vollbrecht@state.nm.us>  
**Subject:** FW: IPRA Request Copper Flat Copper Mine (October 21, 2017)

See Bryan's comment that I highlighted in yellow below - seems like he is looking for something else from BLM. Also, pasted in directly below is an excerpt from the 10/21/16 SWQB letter to NMCC with their requests. Note that it does ask BLM to "concur" that the water body is on private lands:

**The Department requests that you work with the Bureau of Land Management and document that the land surveys are indeed accurate, and BLM determine if the water body is entirely on private lands. If it is found that the current and/or past surveys are entirely on private lands, the water body would be considered a water body subject to the surface water quality standards codified in 20.6.4**

Here is what BLM says about the matter in their 9/13/17 letter to NMCC:

**We reviewed all the mineral surveys in the area and updated our records. The status of the lands was noted correctly. We have also requested that you determine if this area**

**Enclosed is a revised Master Title Plat. I hope this clarifies the matter. We have also placed the survey completed by your surveyor in the 3809 Plan casefiles.**

**From:** Dail, Bryan, NMENV

**Sent:** Tuesday, October 24, 2017 10:30 AM

**To:** Lemon, Shelly, NMENV <Shelly.Lemon@state.nm.us>

**Cc:** Vollbrecht, Kurt, NMENV <kurt.vollbrecht@state.nm.us>; Reid, Brad, NMENV <brad.reid@state.nm.us>; Knight, Andrew, NMENV <Andrew.Knight@state.nm.us>; Fullam, Jennifer, NMENV <Jennifer.Fullam@state.nm.us>

**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

Dear Andrew:

I address (in red) the aspects of the October 21, 2017 IPRA relevant to the Surface Water Quality Bureau ("SWQB") below:

1. NMED determination regarding Copper Flat Copper Mine pit lake being a private water, date range July 2017 through date of this request;

1A: No such determination has been made as of this writing

2. Bureau of Land Management letter of concurrence for NMED's/Surface Water Quality Bureau's benefit regarding the proposed Copper Flat Copper Mine pit lake and associated patented claims surveys completed and a finding that the pit lake constitutes a "private water" and is not subject to either surface or groundwater quality standards;  
Revised 6/14/12

2A: The Bureau of Land Management ("BLM") sent correspondence to New Mexico Copper Corporation ("NMCC") which was then sent to the SWQB on September 27, 2017 via email with the following attachments:

1. NMCC letter to SLemon\_27Sept2017
2. Copper Flat Boundary Survey Plat, March 2017
3. BLM letter from Ida Viarreal, September 13, 2017
4. BLM Map Township 15 South, Range 7 West

I have attached these to this email, however, I do not consider this BLM letter an unambiguous letter of concurrence, rather, as BLM states; an indication that they have updated their Master Title Plats and intend to update their GIS layer. It would certainly help if there could be an overlay generated of the current and future pit lake maps with the updated GIS layer referenced in the BLM letter dated September 13, 2017. I do not have either sets of maps in a form allowing me to easily do so, or lack the expertise to generate the overlay. Absent that, and concurrence from BLM, NMED is likely not able to make a finding.

3. New Mexico Copper Corporation submittals to NMED pertaining to its discharge permit application since July 2017;

3A: N/A

4. New Mexico Copper Corporation submittals to NMED pertaining to the pit lake since July 2017;

4A: Other than NMCC's correspondence of September 27, the SWQB has no other pit lake related submittals from July to the present.

5. Copper Flat Copper Mine cooperating agency meeting notes for September and October 2017 meetings; and

5A: N/A

6. NMED's October 2017 determination regarding technical completeness for New Mexico Copper Corporation's discharge permit application.

6A: N/A

This, I believe, is a complete listing of all relevant materials from the SWQB.

-Bryan

---

**From:** Lemon, Shelly, NMENV  
**Sent:** Monday, October 23, 2017 4:23 PM  
**To:** Dail, Bryan, NMENV <[Bryan.Dail@state.nm.us](mailto:Bryan.Dail@state.nm.us)>  
**Cc:** Vollbrecht, Kurt, NMENV <[kurt.vollbrecht@state.nm.us](mailto:kurt.vollbrecht@state.nm.us)>; Reid, Brad, NMENV <[brad.reid@state.nm.us](mailto:brad.reid@state.nm.us)>; Knight, Andrew, NMENV <[Andrew.Knight@state.nm.us](mailto:Andrew.Knight@state.nm.us)>; Fullam, Jennifer, NMENV <[Jennifer.Fullam@state.nm.us](mailto:Jennifer.Fullam@state.nm.us)>  
**Subject:** FW: IPRA Request Copper Flat Copper Mine

Bryan,

Please work with Brad, Kurt and Andrew to fulfill this IPRA. Let me know if you need anything.

Thanks!  
Shelly

---

**From:** Mascarenas, Melissa, NMENV  
**Sent:** Monday, October 23, 2017 2:26 PM  
**To:** Knight, Andrew, NMENV <[Andrew.Knight@state.nm.us](mailto:Andrew.Knight@state.nm.us)>; Hunter, Michelle, NMENV <[Michelle.Hunter@state.nm.us](mailto:Michelle.Hunter@state.nm.us)>; Lemon, Shelly, NMENV <[Shelly.Lemon@state.nm.us](mailto:Shelly.Lemon@state.nm.us)>  
**Subject:** IPRA Request Copper Flat Copper Mine

**Please make sure that Andrew Knight reviews all documents prior to release.**

---

**From:** Jaimie Park [<mailto:jpark@nmelc.org>]  
**Sent:** Saturday, October 21, 2017 12:16 PM  
**To:** Mascarenas, Melissa, NMENV <[melissa.mascarenas@state.nm.us](mailto:melissa.mascarenas@state.nm.us)>  
**Cc:** Knight, Andrew, NMENV <[Andrew.Knight@state.nm.us](mailto:Andrew.Knight@state.nm.us)>  
**Subject:** IPRA Request Copper Flat Copper Mine

Dear Ms. Mascarenas,

Please find attached an IPRA request pertaining to the Copper Flat Copper Mine's pending discharge permit application.

Kind Regards,

Jaimie Park





## Reid, Brad, NMENV

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**From:** Knight, Andrew, NMENV  
**Sent:** Monday, December 04, 2017 8:58 AM  
**To:** Vollbrecht, Kurt, NMENV; Reid, Brad, NMENV  
**Cc:** Dail, Bryan, NMENV  
**Subject:** NMCC - BLM Acknowledgement of Validity of Surveys

OK, that's good to hear. I agree that BLM's opinion about where the surface of the future pit lake will be is not necessary to our analysis, so I would not have NMCC ask BLM for that. All we really needed from BLM was agreement that the boundaries between their land and NMCC's are where NMCC claims they are, and now it appears we have that.

Andrew P. Knight  
Assistant General Counsel  
New Mexico Environment Department  
Office: (505) 222-9540  
Cell: (505) 907-8836

---

**From:** Vollbrecht, Kurt, NMENV  
**Sent:** Friday, December 01, 2017 5:01 PM  
**To:** Reid, Brad, NMENV <brad.reid@state.nm.us>; Knight, Andrew, NMENV <Andrew.Knight@state.nm.us>  
**Subject:** RE: IPRA Request Copper Flat Copper Mine (October 21, 2017)

So it appears that BLM updated their "Master Title Plat" to match the survey completed by the registered surveyor, so they have acknowledged the validity of the survey.

The missing piece from these documents is the projected outline of the future pit lake. Since that can't be surveyed, NMCC will need to portray it as accurately as possible. Asking BLM to confirm that projected future pit lake is on private land is a little weird, but I guess they can ask BLM for that.

Brad - as part of the NEPA process and OSE review of the hydrologic model of pit lake drawdown did they also review the expected rebound and final lake level following cessation of pumping? If so, then that piece has already been "approved" as well.

The real question is how accurate is the model of the future lake size/elevation, and we won't know that until the future arrives...

Kurt Vollbrecht, Program Manager  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
(505) 827-0195



December 13, 2017

Mr. David Ennis  
Reclamation Specialist  
New Mexico Energy, Minerals and Natural Resources Department  
Mining and Minerals Division  
1220 South St. Francis Drive  
Santa Fe, NM 87505



RE: Copper Flat Mine, Sierra County, Permit Tracking No. SI027RN  
New Mexico Copper Corporation Document Transmittal

Dear Mr. Ennis:

Enclosed herewith are two reports required for the Copper Flat New Mine Permit Application:

1. Predictive Geochemistry Modeling of the Copper Flat Pit Lake, and
2. Probable Hydrologic Consequences of the Copper Flat Project.

It is our understanding that with this submittal, New Mexico Copper Corporation has fulfilled all requirements for a New Mine Permit Application outlined under 19.10.6.602 NMAC. Please advise if our understanding is not correct.

Furthermore, we feel that, with these two reports, our application package demonstrates compliance with New Mexico Mining Act regulations for Performance and Reclamation Standards for New Mining Operations found in 19.10.6.603 NMAC, and warrants an Agency conclusion that operation and reclamation of the Copper Flat Mine as proposed by New Mexico Copper Corporation will meet the requirements of the Act.

NMCC appreciates the opportunity to continue to work with the MMD in pursuit of approval of Mining Permit No. SI027RN. Please do not hesitate to contact us if you have any questions or require clarification.

Sincerely,

A handwritten signature in black ink that reads "J. Smith".

Jeff Smith  
Chief Operating Officer  
THEMAC Resources Group/New Mexico Copper Corporation  
Telephone: (520) 991-4588

cc: Brad Reid – New Mexico Environment Department



December 13, 2017

Mr. David Ennis, Reclamation Specialist  
Mining and Minerals Division  
New Mexico Energy, Minerals and Natural Resources Department

**Mr. Kurt Vollbrecht, Manager**  
Mining Environmental Compliance Section  
Groundwater Quality Bureau  
New Mexico Environment Department



RE: New Mexico Copper Corporation  
OSE Application to Repair Existing Copper Flat Tailings Dam Reservoir

Enclosed herewith for your information is a copy of an application to OSE for the breach of the splitter dike behind the Copper Flat Tailings Dam. This action will provide a channel to connect the northern and southern cells of the existing tailings storage facility. This activity will not affect the existing Copper Flat Tailings Dam and is fully contained on patented land owned by NMMC. Details regarding the project are contained in an engineer's report provided with the application.

Please do not hesitate to contact us if you have any questions or require clarification.

Sincerely,

A handwritten signature in black ink that reads "Jeff Smith".

Jeff Smith  
Chief Operating Officer  
THEMAC Resources Group/New Mexico Copper Corporation  
Telephone: (520) 991-4588

December 12, 2017

Charles N. Thompson, P.E.  
Office of the State Engineer  
Concha Ortiz Y Pino Building  
P.O. Box 25102  
Santa Fe, NM 87504

Re: OSE File No. D-564  
Copper Flat Tailings Dam  
Application to Breach Splitter Dike

Dear Mr. Thompson:

Submitted herewith is New Mexico Copper Corporation's application to breach the splitter dike contained within the existing Copper Flat Tailings Storage Facility. The application and accompanying design documents have been prepared in response to the OSE Dam Safety Bureau's requirement to breach the facility splitter dike issued in a letter dated September 18, 2017.

NMCC intends to self-perform the work and estimates cost of construction will total approximately \$10,000.

Enclosed with this application is a check in the amount of \$45.00 for payment of the \$25.00 application filing fee and the \$20.00 plan review fee (\$2.00 per \$1,000 of construction cost).

Please contact me if additional information regarding this application is needed.

Sincerely,



Jeff Smith, COO  
New Mexico Copper Corporation

cc: Doug Rappuhn, OSE hydrologist  
Kurt Vollbrecht, NMED  
DJ Ennis, MMD  
Doug Haywood, BLM

**NEW MEXICO OFFICE OF THE STATE ENGINEER  
APPLICATION FOR PERMIT  
TO ALTER OR REPAIR A DAM AND RESERVOIR**

**1. OWNER INFORMATION:**

I, Jeffrey Smith of New Mexico Copper Corporation

County of Bernalillo, State of New Mexico, owner of Copper Flat Tailings Dam, hereby make application for the approval of plans and specifications for the repair of Copper Flat Tailings Dam or reservoir.

If the owner is a corporation, give name and address of president and secretary:

Jeffrey Smith, Chief Operating Officer  
4253 Montgomery Blvd. NE, Suite 130  
Albuquerque, NM 87109

**2. LOCATION:**

A. NE¼ NW¼ SW¼ Section: 31 Township: 15S Range: 6W N.M.P.M.  
in Sierra County.

or X = \_\_\_\_\_ feet, Y = \_\_\_\_\_ feet, N.M. State Plane Coordinate System  
\_\_\_\_\_ Zone Datum of \_\_\_\_\_ in the \_\_\_\_\_ Grant.

B. Latitude in decimal degrees: 32.9576°  
Longitude in decimal degrees: -107.4976°

**3. HAZARD POTENTIAL CLASSIFICATION: Significant**

**4. DESCRIPTION OF PROPOSED WORK:**

A. Type of dam: Earth embankment with internal splitter dike to separate the reservoir into northern and southern cells

B. Description of work contemplated (Use extra sheets or exhibits if necessary.)

Excavate storm water channel through internal splitter dike to allow storm water to flow from northern to southern cells during a flood event. Design details provided in attached technical memo by Golder Associates.



**NEW MEXICO OFFICE OF THE STATE ENGINEER  
APPLICATION FOR PERMIT  
TO ALTER OR REPAIR A DAM AND RESERVOIR**

**5. DESCRIPTION OF DEFICIENCIES OF DAM WITH DESIGN REQUIREMENTS FOR DAMS  
(19.25.12. 11 NMAC):**

Mine tailings deposited in the northern cell have reduced the water holding capacity of the north cell, which may lead to overtopping of the front embankment during a flood event.

6. Work is commenced by estimate Q1 2018 (dependent on OSE approval) and to be completed by 3 months after OSE approval (estimate Q2 2018)

7. Engineer: Golder Associates

8. Contractor: New Mexico Copper Corp. Self-Perform

**9. ACKNOWLEDGEMENT FOR THE DAM OWNER:**

I, Jeffrey Smith affirm that the foregoing statements are true to the best of my knowledge and belief. I fully understand the repair and alteration related to this dam and the responsibility and liability related to dam ownership.

Signed: *Jeffrey Smith* Date: Dec 12, 2017

Subscribed and sworn to before me this 12<sup>th</sup> day of December, 2017

Notary Public: *Katharyn Emmer*

My commission expires: 14 March 2020



**INSTRUCTIONS**

Use this form if the properties of the dam or appurtenant structures will not change as a result of the alteration or repair. This form shall be filed with original signatures and accompanied by construction drawings, specifications, design report, etc. and filing fee of \$25.00 for the application and plan review fee of \$2.00 per \$1000 of construction cost for dam and appurtenances. Owners repairing a dam to address a dam safety deficiency may request a waiver of the plan review fee. This form and supporting documentation shall be delivered to the attention of the OSE Dam Safety Bureau, P.O. Box 25102, Santa Fe, New Mexico.

Hit "F1" key for additional instructions for each cell. Cell sizes are limited.

- Office of the State Engineer Dam File Number required.
- Section 1 Name and address of the representative of the owner making the application. Private owners must enter their name. Public or corporate owners must enter the name of the official authorized to make the application. Name of dam must also be entered.
- Section 2 Location of dam is required.
- Section 3 Hazard potential classification is required. Refer to 19.25.12.10 NMAC.
- Section 4 Items A and B are required.
- Section 5 Describe the dam deficiency as it relates to the design requirements in 19.25.12.11 NMAC
- Section 6 Dates of estimated start and completion of work are required.
- Section 7 Name of Design Engineer.
- Section 8 Name of Contractor selected.
- Section 9 Dam Owner's printed name and notarized signature are required.



## TECHNICAL MEMORANDUM

**Date:** December 11, 2017  
**To:** Jeffrey Smith, PE  
**From:** Sheina Sadza, PE  
David A. Kidd, PE  
**cc:**



**Project No.:** 1789021  
**Company:** THEMAC Resources Group, Ltd

**Email:** [ssadza@golder.com](mailto:ssadza@golder.com)  
[dkidd@golder.com](mailto:dkidd@golder.com)

**RE: COPPER FLAT TAILINGS STORAGE FACILITY – INTERNAL DIKE BREACH DESIGN**

Golder Associates Inc. (Golder) has prepared this Technical Memorandum (TM) for New Mexico Copper Company (NMCC), a wholly owned subsidiary of THEMAC Resources Group Ltd. This TM summarizes the design of a breach to the existing internal splitter dike that divides the North and South Cells of the existing Tailings Storage Facility (TSF) at Copper Flat Mine in Sierra County, New Mexico. In accordance with New Mexico Office of the State Engineer – Dam Safety Bureau (OSE-DSB) guidance, the breach has been designed to safely pass the Probable Maximum Flood (PMF) peak discharge while meeting established freeboard requirements at the existing starter dam along the east side of the TSF.

### 1.0 BACKGROUND

In 2012, Golder prepared a stormwater storage capacity assessment of the impoundment behind the existing TSF starter dam for New Mexico Copper Corporation (NMCC) (Golder 2012). This assessment looked at runoff volumes due to various durations of general Probable Maximum Precipitation (PMP) events and the 1-hour local storm PMP event.

The TSF catchment area is approximately 500 acres and the existing starter dam is located along the east side of TSF at a minimum crest elevation of 5,240 feet above mean sea level (ft-amsl). The TSF was separated into North and South Cells by the construction of an internal splitter dike with a crest elevation varying between 5,240 ft-amsl and 5,242 ft-amsl.

Tailings deposition during mining operations was limited to the North Cell, with placement such that accumulated water would be forced to the west, away from the starter dam face. The tailings deposition reduced the available stormwater storage capacity in the North Cell to approximately 280 acre-feet at the established 4 feet of freeboard.

As the North Cell would not have sufficient storage capacity for the PMP volumes, the 2012 assessment proposed breaching the internal splitter dike between the North and South Cells to allow runoff volumes to equalize between these two areas. It was determined that the combined storage of the North and South Cells (2,106 acre-feet) was sufficient to contain the runoff volume from all the evaluated PMP events and maintain a dry freeboard of 4 feet below the starter dam crest.

x:\tucson\projects\17 pro\1789021 copper flat dike breach\001rev 0\1789021-tm-001-rev0-20171211.docx

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Tucson, AZ 85705 USA  
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Golder Associates: Operations in Africa, Asia, Australasia, Europe, North America and South America

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In September 2017, OSE-DSB requested that the internal splitter dike be breached in accordance with the 2012 Golder assessment as part of NMCC's request for an extension of a maintenance waiver. Any proposed breach of the dike would need to be designed to pass the peak PMP discharge while still providing sufficient freeboard. OSE-DSB design guidance (NMOSE 2008) recommends evaluation of both the 72-hour general storm and the 6-hour local storm for the design of dams and appurtenant structures. The 2012 assessment looked at the longer duration general storm events, including the 72-hour general storm, to verify that there is sufficient combined storage capacity. However, the 6-hour local storm PMP produces higher peak discharges due to the higher rainfall intensity and controls the design of the breach.

The remainder of this document summarizes the design of the proposed dike breach which involves the development of a flood routing study for the 6-hour PMP in support of a breach design that maintains sufficient freeboard in the TSF.

## 2.0 SPLITTER DIKE BREACH DESIGN

Per the previous assessment, the breach should be designed to convey the PMF that reports to the north side of the splitter dike while maintaining 4 feet of dry freeboard below the starter dam crest. As noted in Section 1, the breach section should be designed to pass the peak discharge generated by the 6-hour local storm PMP. The breach design involves two main parts:

- Estimation of the 6-hour local storm PMP and controlling peak discharge
- Development of a breach weir configuration that conveys the peak discharge and meets the 4-foot freeboard requirement

### 2.1 6-Hour Local Storm PMP and Hydrologic Analysis

OSE-DSB's guidance document for the hydrologic analysis of dams (NMOSE 2008) outlines procedures for determining the PMP depths and other basin parameters that contribute to runoff. This document was used as the primary basis for the hydrologic analysis.

The basin areas reporting to the North Cell, North Tailings area and South Cell were verified using existing 2-foot contour interval topography provided by the client in 2011 and review of publicly available aerial imagery. The basin delineations and calculated areas are shown on Figure 1.

Based on the location of the TSF and its total contributing watershed, PMP depths are estimated using Hydrometeorological Report No. 55A (HMR 55A) (US Bureau of Reclamation [USBR] 1988). The estimated 6-hour local storm PMP depth is 16.02 inches. The step-by-step procedures used for the determining this estimate are provided in Attachment 1.

The Hydrologic Engineering Center – Hydrological Modeling System (HEC-HMS, US Army Corps of Engineers [USACE] 2016) software was used for this hydrologic analysis. Using OSE-DSB design guidance

and information from Golder's 2012 assessment, the following assumptions were used to develop the hydrologic model:

- Basin Loss Method: Initial and Constant
  - Initial Loss = 0 inches to reflect saturated soil conditions (NMOSE 2008)
  - Constant Loss = 0.025 inches per hour (tailings); 0.075 inches per hour (natural areas) (Golder 2012), (NMOSE 2008)
- Lag Time coefficient,  $K_n = 0.042$  (desert vegetation) (USBR 1989)
- Rainfall Distributions for Comparison (USBR 1988):
  - United States Army Corps of Engineers (USACE)
  - Hydrometeorological Report No. 5 (HMR)
- Unit Hydrograph: Southwest Desert (USBR 1989)

HMR 55A (USBR 1988) recommends the evaluation of two rainfall distributions for the 6-hour local thunderstorm: one from HMR 5 and the other developed by the USACE. The USACE method produces a "late peaking" distribution, while HMR 5 is a "center peaking" distribution. The distribution that generates the higher peak discharge is typically selected for design. Both methods have been analyzed as part of this study. The results of the hydrologic analysis are summarized in Table 1.

**Table 1: 6-Hour Local PMP Discharge Summary**

Location	HEC-HMS Element ID	Total Area (sq mi)	Peak Discharge (USACE) (cfs)	Peak Discharge (HMR) (cfs)
North Side of Splitter Dike	North-Reservoir	0.438	937	829

Notes:

sq mi = square miles  
cfs = cubic feet per second

Based on the results the controlling peak discharge of 937 cubic feet per second reporting to the north side of the splitter dike results from using the USACE rainfall distribution and is the discharge used to design the breach weir section.

## 2.2 Breach Weir Design

The breach will function similarly to a spillway weir and allow runoff reporting to the north side of the splitter dike to flow into the South Cell. Based on existing topography it is proposed to construct the breach close to the western edge of the North Cell tailings deposition area (See Drawing C-001). This breach location is deemed the most suitable for grading purposes, but also has the added benefit of directing discharges into downstream borrow areas in the South Cell. These borrow areas are isolated from the starter dam face and runoff exiting the North Cell through the breach will flow into the borrow areas and be directed to the west and south, which will help to limit the amount of runoff that could report to the starter dam face.

Golder's 2012 assessment noted that both the splitter dike and the starter dam have elevations that vary between 5,240 ft-amsl and 5,242 ft-amsl; however, the minimum crest elevation of 5,240 ft-amsl was assumed for the capacity assessment and freeboard requirements are also based on that elevation.

The breach design is based on the following assumptions:

- Existing starter dam crest minimum elevation: 5,240 ft-amsl (Golder 2012)
- Maximum water surface elevation to maintain 4-foot dry freeboard: 5,236 ft-amsl
- Stage-storage data for North and South Cells referenced from previous assessment (Golder 2012)
- Weir stage-discharge relationship developed using FlowMaster (Bentley 2009) assuming broad-crested weir configuration

A 40-foot wide breach design was selected to meet conveyance requirements. The stage-discharge relationship for this weir configuration was incorporated into the HEC-HMS model to provide an estimate off the maximum water surface elevation in the North Cell during the 6-hour PMF and to verify that the design meets freeboard requirements. The stage-discharge relationship was developed assuming there is no potential for submergence of the weir. Weir submergence at the outlet can reduce flow conveyed through the structure; however, this was not considered to be a factor in this design as the estimated water surface elevation within the South Cell for the 6-hour PMF is significantly lower than the proposed weir crest elevation even after the volume from the North Cell is added. The design parameters for the proposed breach configuration are summarized in Table 2.

**Table 2: 40-Foot Breach Weir – Design Summary**

Bottom Width of Breach (ft)	40
Bottom Elevation of Breach (ft-amsl)	5,232
Dike Crest Elevation at Breach (ft-amsl)	5,242
Breach Height (ft)	10
Excavation Side Slope (H:V)	2:1
Estimated Excavation Volume (cubic yards)	3,000
Peak Water Surface Elevation in North Cell	5,235.9

Notes:

ft = feet

ft-amsl = feet above mean sea level

Construction details for the proposed breach design are provided on Drawing C-001. The model results are provided in Attachment 2.

### 3.0 CONCLUSIONS

A 40-foot wide breach of the existing internal splitter dike has been designed to equalize stormwater runoff volumes between the North and South Cells of the existing TSF. This configuration will convey the peak PMF discharge while maintaining a minimum of 4 feet of freeboard below the starter dam crest. The breach

is proposed at a location along the splitter dike such that runoff is directed to existing borrow areas within the South Cell and away from the starter dam face. Please contact Golder Associates if there are any questions or concerns regarding this assessment.

#### 4.0 REFERENCES

Bentley Systems Inc. (Bentley). 2009. FlowMaster [software package]. Version V8i. November 2009.

Golder Associates Inc. (Golder). 2012. Revised Stormwater Storage Capacity Assessment, Existing Copper Flat Tailings Dam Revised January 6, 2012 (Letter Report). January 6, 2012.

New Mexico Office of the State Engineer Dam Safety Bureau (NMOSE). 2008. Hydrologic Analysis for Dams (white paper). August 15, 2008.

United States Army Corps of Engineers (USACE). 2016. Hydraulic Engineering Center Hydrologic Modeling System (HEC-HMS) [software package]. Version 4.2. August 2016.

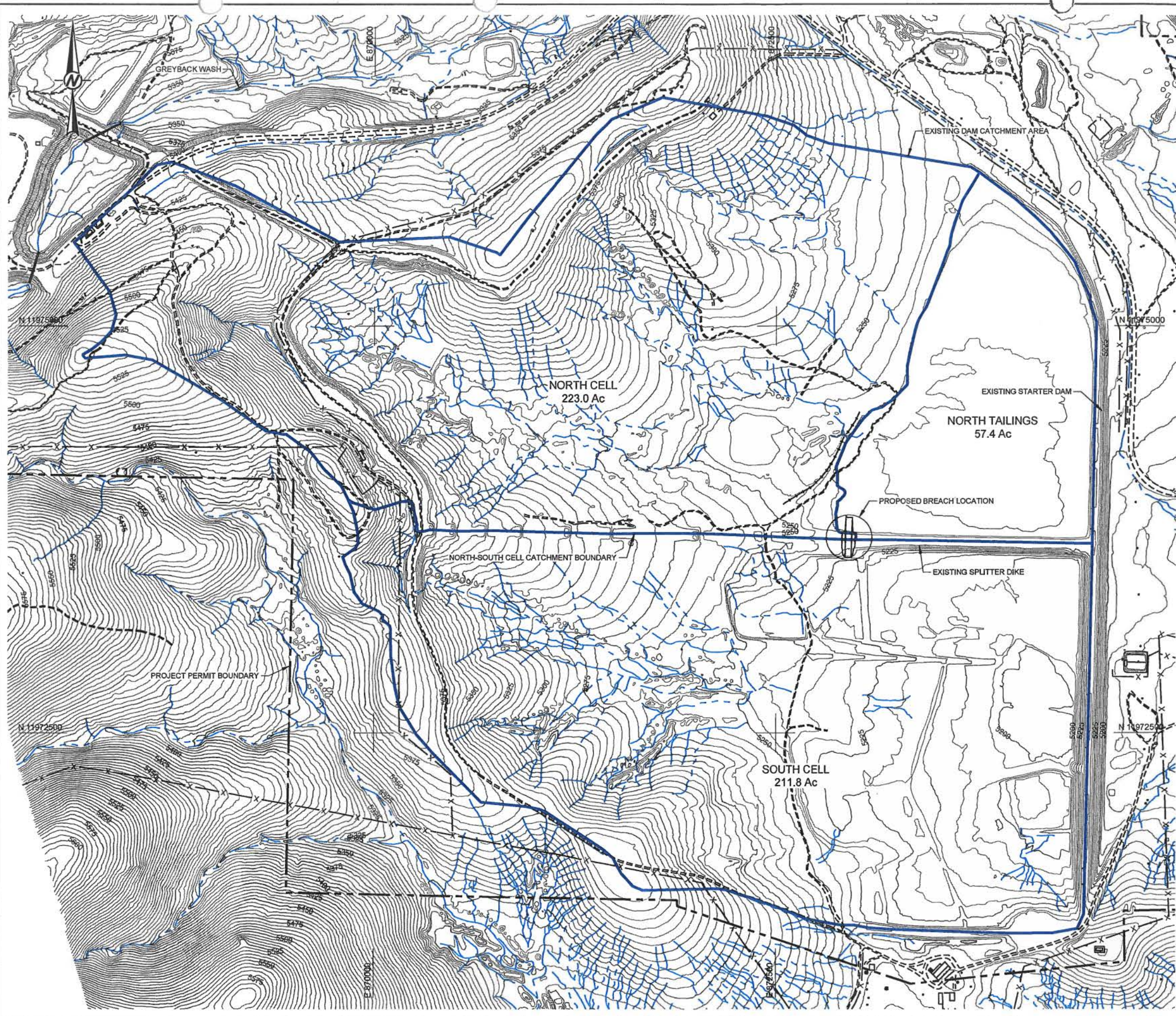
United States Bureau of Reclamation (USBR). 1988. Hydrometeorological Report No. 55A (HMR 55A), Probable Maximum Precipitation Estimates – United States Between Continental Divide and the 103<sup>rd</sup> Meridian. June 1988.

USBR. 1989. Flood Hydrology Manual, First Edition. 1989.

Attachments: Figure 1 – Existing TSF Basin Areas  
Drawing C-001 – Proposed Breach Weir Design  
Attachment 1 – Local PMP Rainfall Development  
Attachment 2 – HEC-HMS Modeling  
Attachment 2-1 – TSF Stage-Storage Information (Referenced from 2012 Golder Analysis)  
Attachment 2-2 – Basin Lag Time Calculation  
Attachment 2-3 – Stage-Discharge Rating  
Attachment 2-4 – HEC-HMS Inputs and Outputs

**FIGURE 1  
EXISTING TSF BASIN AREAS**

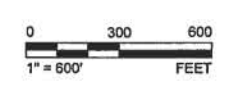
Path: \\usom\download\THEMAC\CopperFlat99\_PROJECT\SI1789021\_1000\_PROD\FIG\_001.dwg | File Name: 1789021\_1000\_PROD\FIG\_001.dwg | Last Edited By: nicolas Date: 2017-12-11 Time: 1:56:54 PM | Printed By: nicolas Date: 2017-12-11 Time: 1:57:48 PM



**LEGEND**

	EXISTING GROUND CONTOURS (ft-MSL)
	EXISTING ROADS
	EXISTING DRAINAGE
	EXISTING FENCELINE
	EXISTING POWER POLE OR POST
	SUB-BASIN BOUNDARY

- REFERENCE(S)**
- 2-FOOT TOPOGRAPHY DEVELOPED BY COOPER AERIAL SURVEY COMPANY BASED ON A JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
  - COORDINATE SYSTEM IS UTM ZONE 13, ON THE NAD83 DATUM, U.S. FOOT.



CLIENT **THEMAC** RESOURCES INC. NEW MEXICO COPPER CORPORATION  
*Environmentally Responsible. Community-Minded. Local Opportunities.*

PROJECT  
 COPPER FLAT PROJECT  
 INTERNAL SPLITTER DIKE BREACH  
 SIERRA COUNTY, NEW MEXICO  
 TITLE  
 EXISTING TSF BASIN AREAS

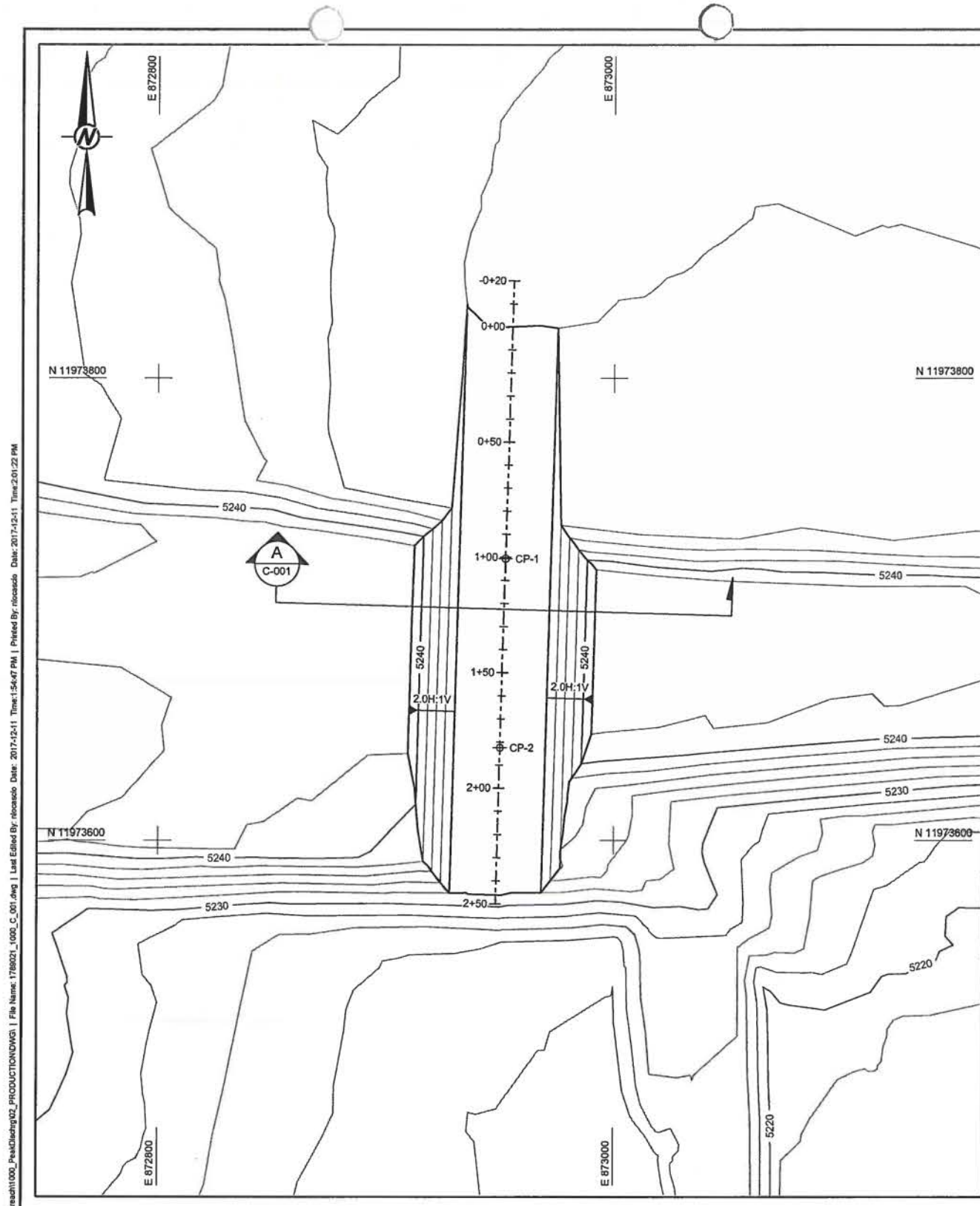
CONSULTANT	YYYY-MM-DD	2017-12-11
	DESIGNED	SPS
	PREPARED	NIL
	REVIEWED	CPB
	APPROVED	DAK

PROJECT NO. 1789021	CONTROL 1000	REV. 0	FIGURE 1
------------------------	-----------------	-----------	-------------

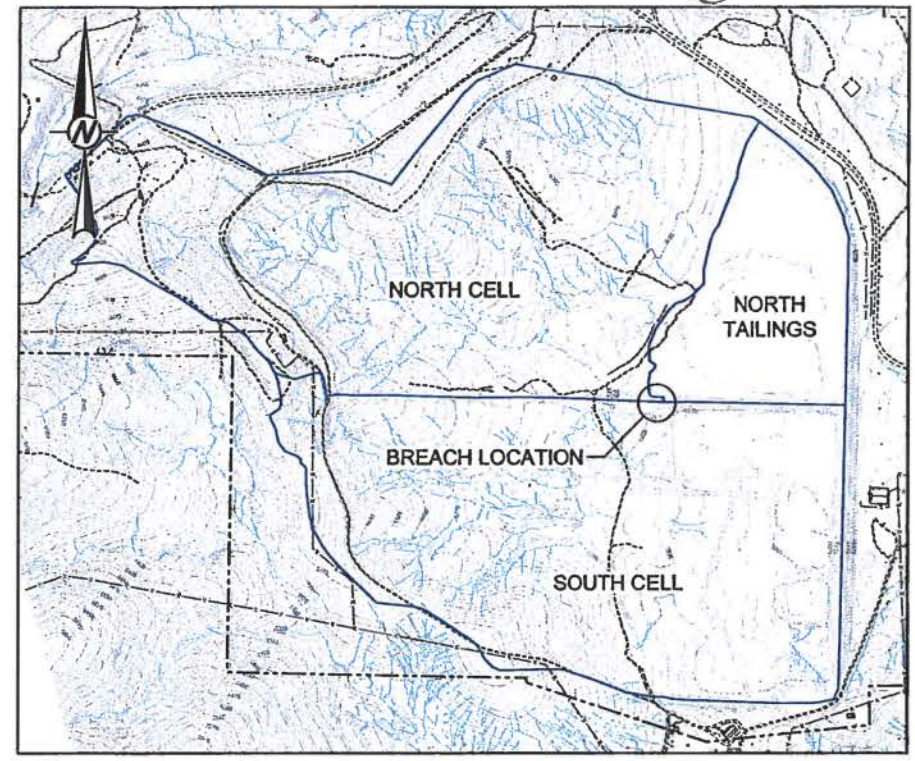
1: IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B



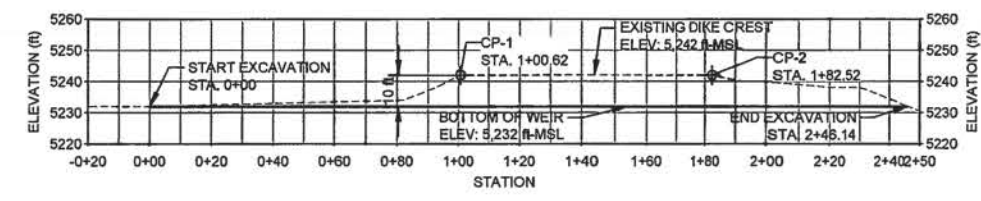
**DRAWING C-001  
PROPOSED BREACH WEIR DESIGN**



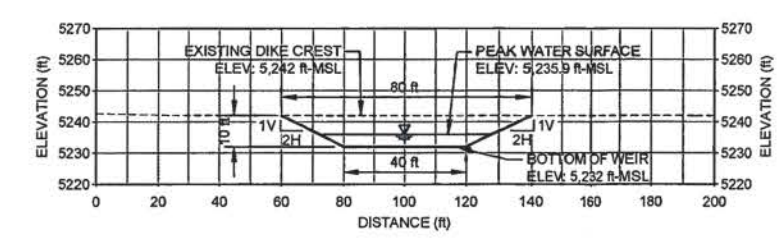
**BREACH WEIR GRADING PLAN**  
SCALE 1" = 30'



**SITE LOCATION MAP**  
N.T.S.



**WEIR EXCAVATION PROFILE**  
SCALE 1" = 30'



**TYPICAL WEIR CROSS-SECTION**  
SCALE 1" = 30' (A C-001)

**SURVEY CONTROL POINT TABLE**

ID	STATION	UTM83-13F NORTHING (ft)	UTM83-13F EASTING (ft)	LATITUDE	LONGITUDE	CREST ELEVATION (ft)
CP-1	1+00.62	11973721.8	872952.3	N032° 57' 34.61"	W107° 30' 09.54"	5242.0
CP-2	1+82.52	11973639.9	872950.0	N032° 57' 33.80"	W107° 30' 09.54"	5242.0

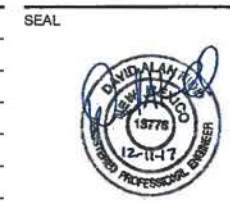


- LEGEND**
- EXISTING GROUND CONTOURS (ft-MSL)
  - FINAL GRADE CONTOURS (ft-MSL)
  - GRADE BREAK
  - CP-1 SURVEY CONTROL POINT
  - 2H:1V or 2H:1V 2 HORIZONTAL TO 1 VERTICAL SLOPE
  - CROSS-SECTION CALLOUT SECTION ID DRAWING SHEET LOCATION

- NOTE(S)**
1. PROPOSED BREACH WEIR CONFIGURATION RESULTS IN APPROXIMATELY 3,000 CY OF EXCAVATION.

- REFERENCE(S)**
1. 2-FOOT TOPOGRAPHY DEVELOPED BY COOPER AERIAL SURVEY COMPANY BASED ON A JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
  2. COORDINATE SYSTEM IS UTM ZONE 13, ON THE NAD83 DATUM, U.S. FOOT.

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
0	2017-12-11	ISSUED FOR CONSTRUCTION	HNL	NIL	SPS	DAK
B	2017-12-07	ISSUED FOR CLIENT REVIEW	HNL	NIL	SPS	DAK
A	2017-12-05	ISSUED FOR INTERNAL REVIEW	HNL	NIL	SPS	DAK



CLIENT  
**THEMAC** RESOURCES INC.  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community-Minded. Local Opportunities.

CONSULTANT  
**Golder Associates**

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4730 N. ORACLE ROAD, SUITE 210  
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www.golder.com

PROJECT  
COPPER FLAT PROJECT  
INTERNAL SPLITTER DIKE BREACH  
SIERRA COUNTY, NEW MEXICO

TITLE  
**BREACH WEIR DESIGN**

PROJECT NO. 1789021 CONTROL 1000 REV. 0 1 of 1 DRAWING C-001

**ATTACHMENT 1  
LOCAL PMP RAINFALL DEVELOPMENT**



## CALCULATIONS

<b>Date:</b>	17-Nov-17	<b>Made by:</b>	SPS
<b>Project No.:</b>	1789021	<b>Checked by:</b>	CPB
<b>Subject:</b>	Local PMP Determination	<b>Reviewed by:</b>	DAK
<b>Project Short Title:</b> COPPER FLAT TSF - INTERNAL SPLITTER DIKE			

### **Objective: Determine 6-Hour Local PMP Thunderstorm Depth using Procedure in HMR 55A**

#### TSF Watershed Properties

Watershed Area: 0.769 square miles  
 Watershed Centroid: 32.9598 Latitude, -107.5071 Longitude  
 Average Basin Elevation: 5,385 feet

#### **Step 1 - Determine 1-hr, 1 mi<sup>2</sup> PMP at 5,000 foot elevation using Plates VIa-c** Project watershed located on Plate VIc

	1 mi <sup>2</sup> PMP (inches)
1-hr	11.75

#### **Step 2 - Adjust Index PMP Value to Mean Watershed Elevation if Elevation is >5,000 feet**

##### **Step 2.1 - Determine Maximum Persisting 12-Hour Dew Point**

Using Figure 4.12 (month of August)

Maximum Dew Point (°F): 78.6

##### **Step 2.2 - Use Maximum Dew Point and Mean Elevation to Determine Adjusted PMP Depth**

Using Figure 14.3

Elevation Adjustment: 1.00%

	Adjusted 1 mi <sup>2</sup> PMP (inches)
1-hr	11.87

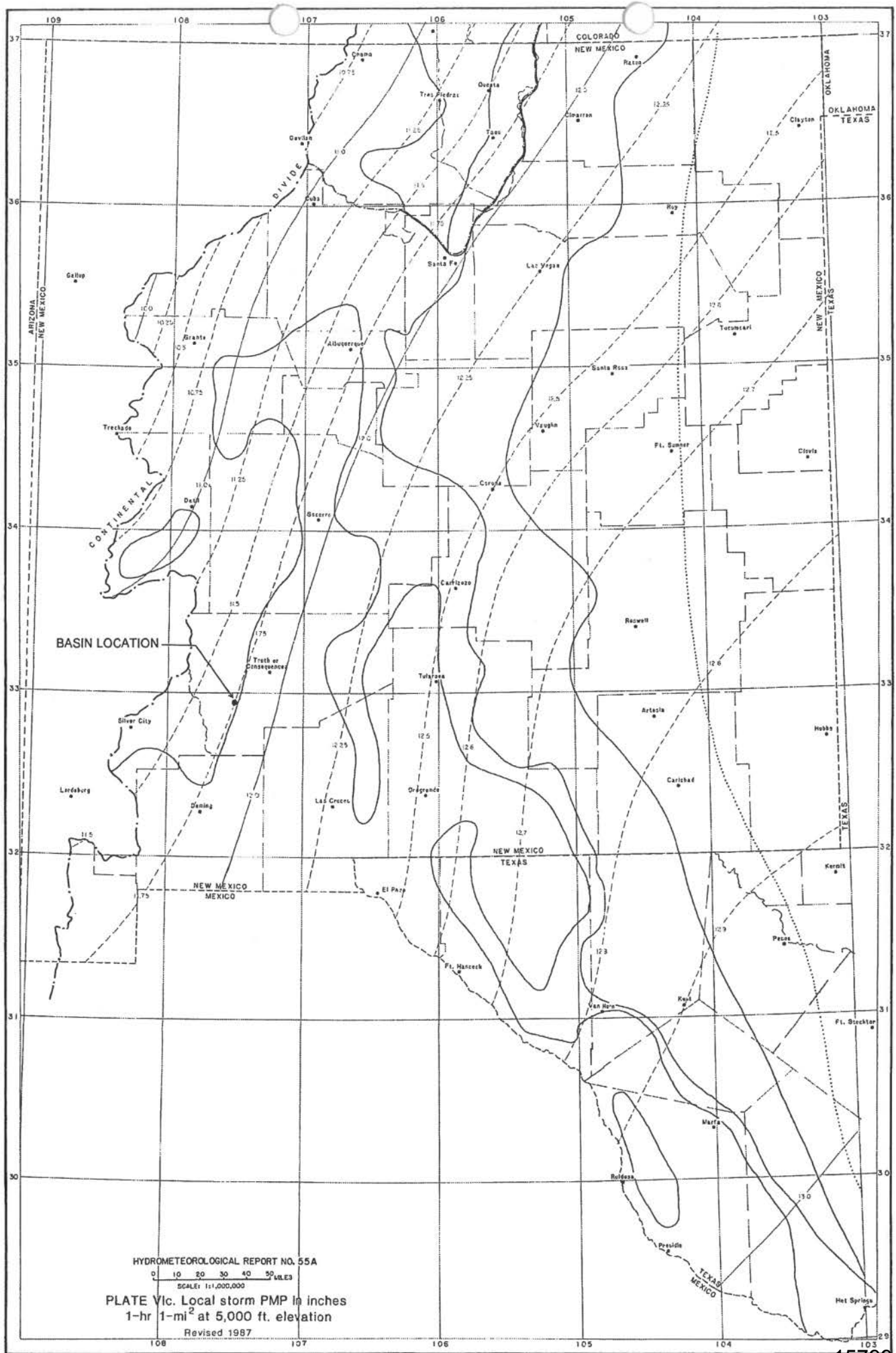
#### **Step 3 - Depth-Duration Relations for 1mi<sup>2</sup> PMP**

Refer to Table 12.4 for depth factors for durations up to 6 hour

Duration	% of 1-Hour Depth	Total Depth (inches)
15 min	0.68	8.07
30 min	0.86	10.21
45 min	0.94	11.16
1 hour	1.00	11.87
2 hour	1.16	13.77
3 hour	1.23	14.60
4 hour	1.28	15.19
5 hour	1.32	15.67
6 hour	1.35	16.02

#### **Step 4 - Apply Any Areal Reductions to PMP Estimate from Step 2**

As the project watershed area is less than 1 square mile, no areal adjustment is required. Use values in Step 3 table.



HYDROMETEOROLOGICAL REPORT NO. 55A

0 10 20 30 40 50 MILES  
SCALE: 1:1,000,000

PLATE VIc. Local storm PMP in inches  
1-hr 1-mi<sup>2</sup> at 5,000 ft. elevation

Revised 1987

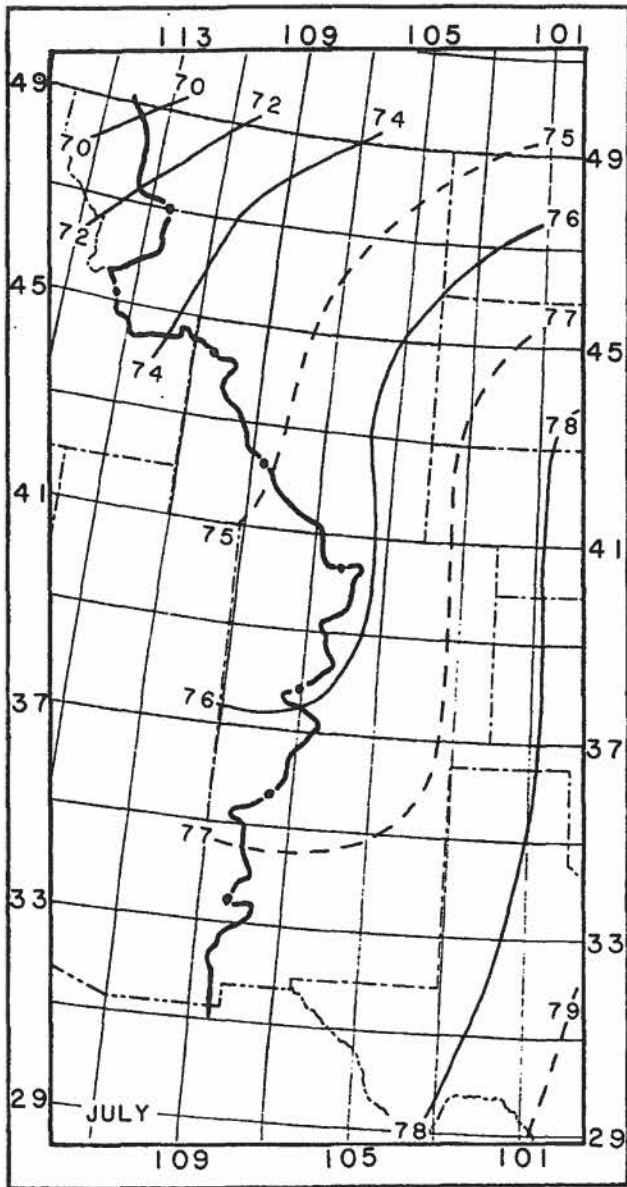


Figure 4.11.--Maximum persisting 12-hr 1000-mb dew points ( $^{\circ}$ F) for July.

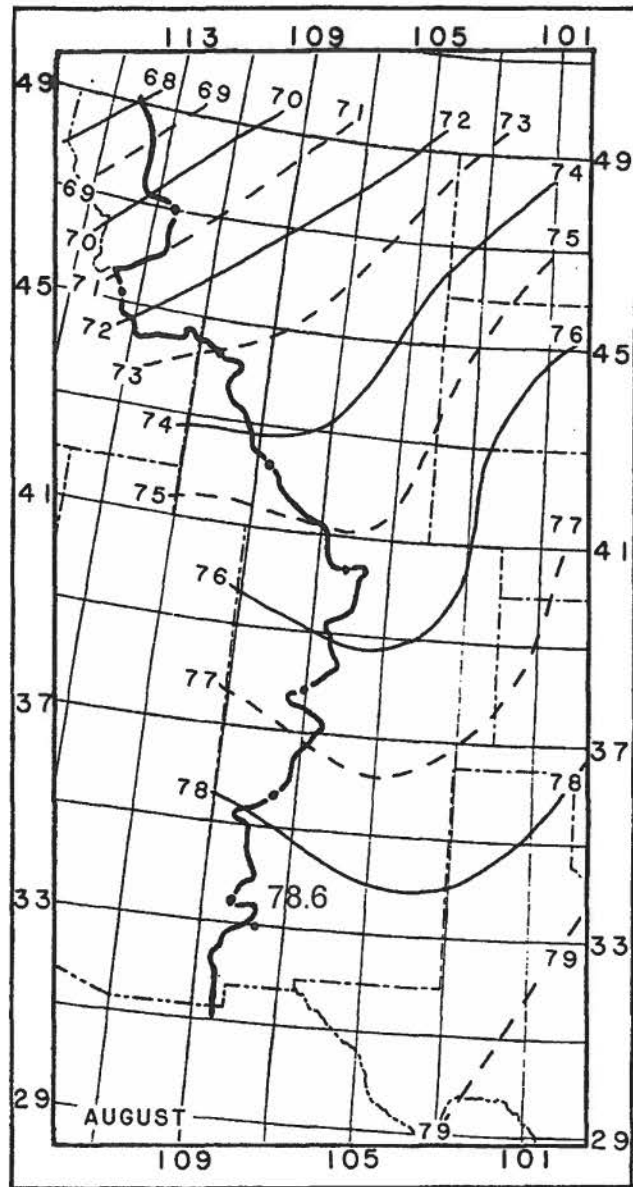


Figure 4.12.--Maximum persisting 12-hr 1000-mb dew points ( $^{\circ}$ F) for August.

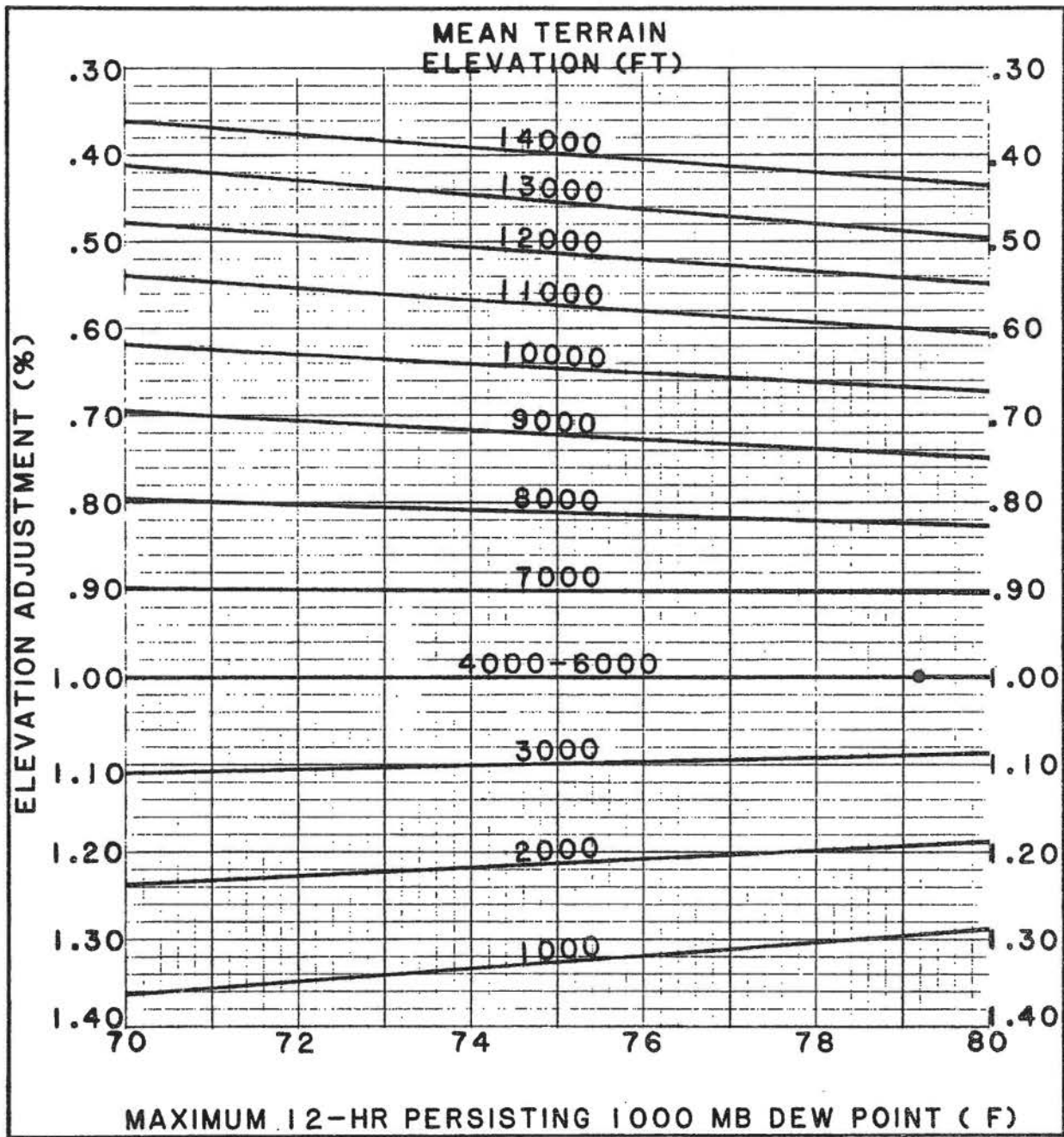


Figure 14.3.--Adjustment for elevation for local-storm PMP based on procedures developed in the report and maximum persisting 12-hr 1000-mb dew point (F).

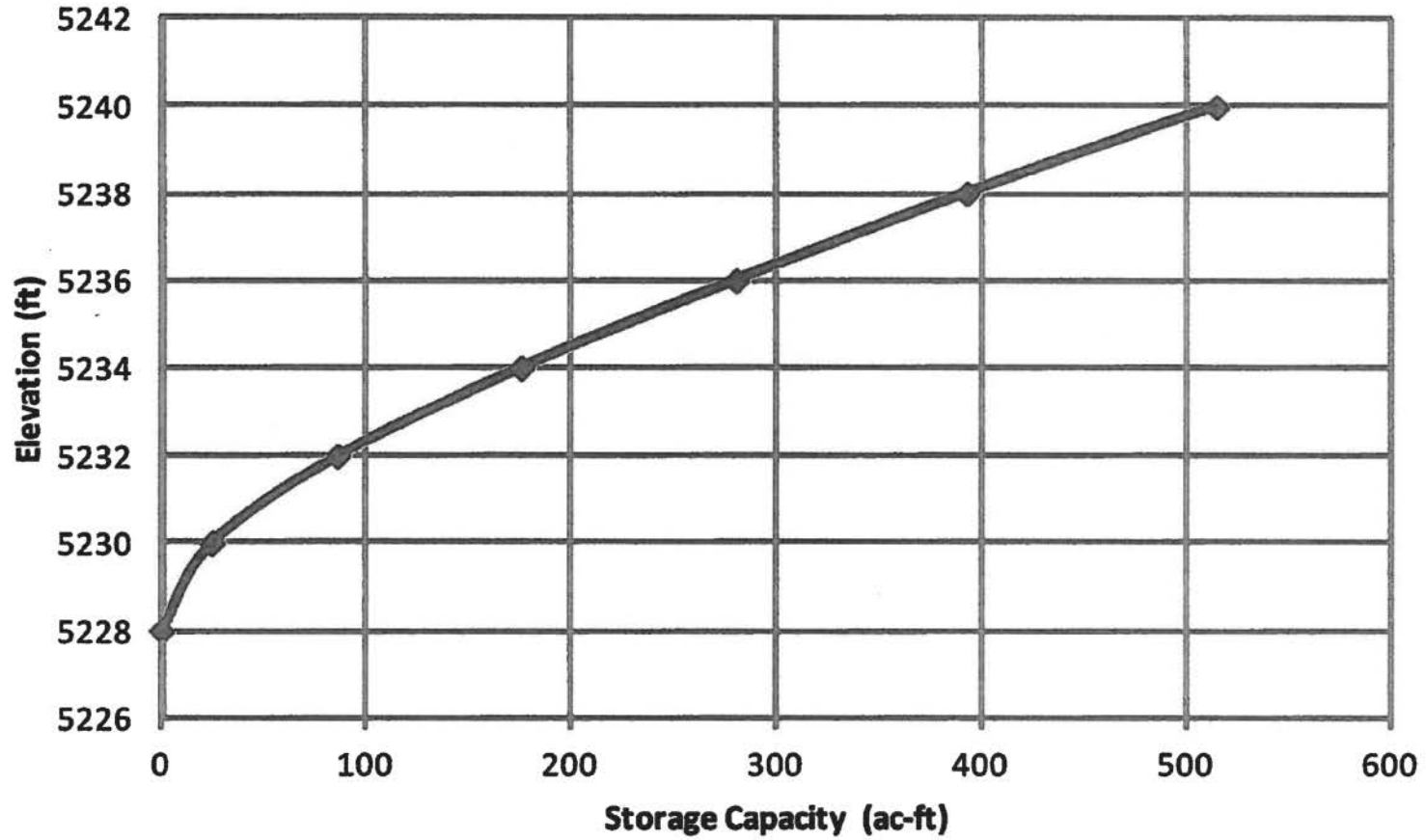
**6-Hour Local Storm Recommended Rainfall Distributions**

<b>Distributed Increments</b>		
<b>Hour</b>	<b>6-Hour Local PMP (COE)</b>	<b>6-Hour Local PMP (HMR)</b>
0	0	0
1	0.36	0.36
2	0.59	0.83
3	1.9	11.87
4	11.87	1.9
5	0.83	0.59
6	0.47	0.47



**ATTACHMENT 2  
HEC-HMS MODELING**

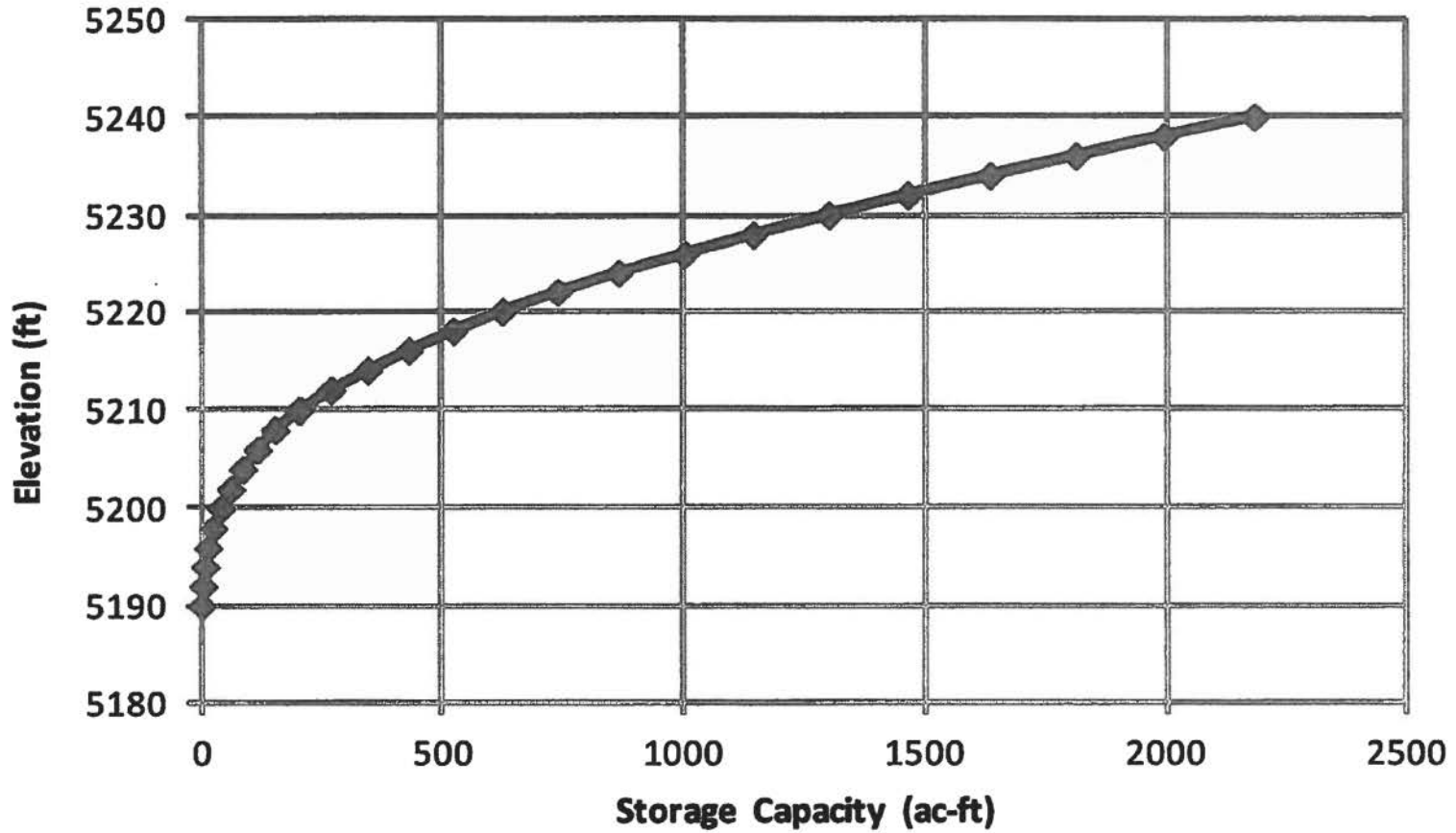
**ATTACHMENT 2-1  
TSF STAGE-STORAGE INFORMATION  
(REFERENCED FROM 2012 GOLDR ANALYSIS)**



PROJECT		COPPER FLAT PROJECT STORMWATER STORAGE CAPACITY ANALYSIS SIERRA COUNTY, NEW MEXICO	
New Mexico Copper Corporation			
TITLE			
EXISTING TAILINGS IMPOUNDMENT NORTH CELL ELEVATION VS. STORMWATER STORAGE CAPACITY			
PROJECT No. 103-92557		FILE No. 10392557E002	
DESIGN	WK	10/18/11	SCALE NOT APPLICABLE
CADD	NL	10/20/11	
CHECK	WK	10/20/11	
REVIEW	GM	10/20/11	



**FIGURE 2**



<b>PROJECT</b>		COPPER FLAT PROJECT STORMWATER STORAGE CAPACITY ANALYSIS SIERRA COUNTY, NEW MEXICO	
New Mexico Copper Corporation			
<b>TITLE</b>			
EXISTING TAILINGS IMPOUNDMENT SOUTH CELL ELEVATION VS. STORMWATER STORAGE CAPACITY			
<b>PROJECT No.</b> 103-92557		<b>FILE No.</b> 10392557E002	
<b>DESIGN</b> WK	10/18/11	<b>SCALE</b>	NOT APPLICABLE
<b>CADD</b> NIL	10/20/11	<b>FIGURE 3</b>	
<b>CHECK</b> WK	10/20/11		
<b>REVIEW</b> GM	10/20/11		



**ATTACHMENT 2-2  
BASIN LAG TIME CALCULATION**



**CALCULATIONS**

Date: 18-Oct-17  
 Project No.: 1789021  
 Subject: Hydrology Input Parameters  
 Project Short Title: COPPER FLAT - INTERNAL SPLITTER DIKE

Made by: SPS  
 Checked by: CPB  
 Reviewed by: DAK

**BASIN LAG TIME CALCULATIONS FOR LOCAL THUNDERSTORM  
 (USBR BASIN CENTROID METHOD)**

Basin	Area (ft <sup>2</sup> )	Area (acres)	Area (m <sup>2</sup> )	Kn	C = 26*Kn	L (ft)	L (mi)	Lca (ft)	Lca (mi)	Δ El. (ft)	S (ft/ft)	S (ft/mi)	N	(L*Lca)/(S <sup>0.5</sup> )	Lg (hr)	Lg (min)
North Cell	9714067	223.0	0.3484	0.042	1.09	6160	1.1667	2455	0.4650	296	0.048	254	0.33	0.03	0.36	21
North Tailings	2499113	57.4	0.0896	0.042	1.09	1955	0.3703	755	0.1430	14	0.007	38	0.33	0.01	0.23	14
South Cell	9225823	211.8	0.3309	0.042	1.09	4895	3.0416	1800	1.1185	262	0.054	283	0.33	0.20	0.64	39

USBR BASIN CENTROID METHOD EQUATION:

$$L_g = C \left( \frac{LL_{ca}}{S^{0.5}} \right)^N$$

Where:

- Lg = Unit hydrograph lag time, in hours
- C = Constant
- L = The length of the longest watercourse from the point of concentration to the boundary of the drainage basin, in miles. The point of concentration is the location on the watercourse where a hydrograph is desired.
- Lca = the length along the longest watercourse from the point of concentration to a point opposite the centroid of the basin, in miles.
- S = The overall slope of the longest watercourse (along L), in feet per mile.
- N= exponent, typically 0.33



**ATTACHMENT 2-3  
STAGE-DISCHARGE RATING**

## North Cell - 40-Ft Wide Breach

### Project Description

Solve For                      Discharge

### Input Data

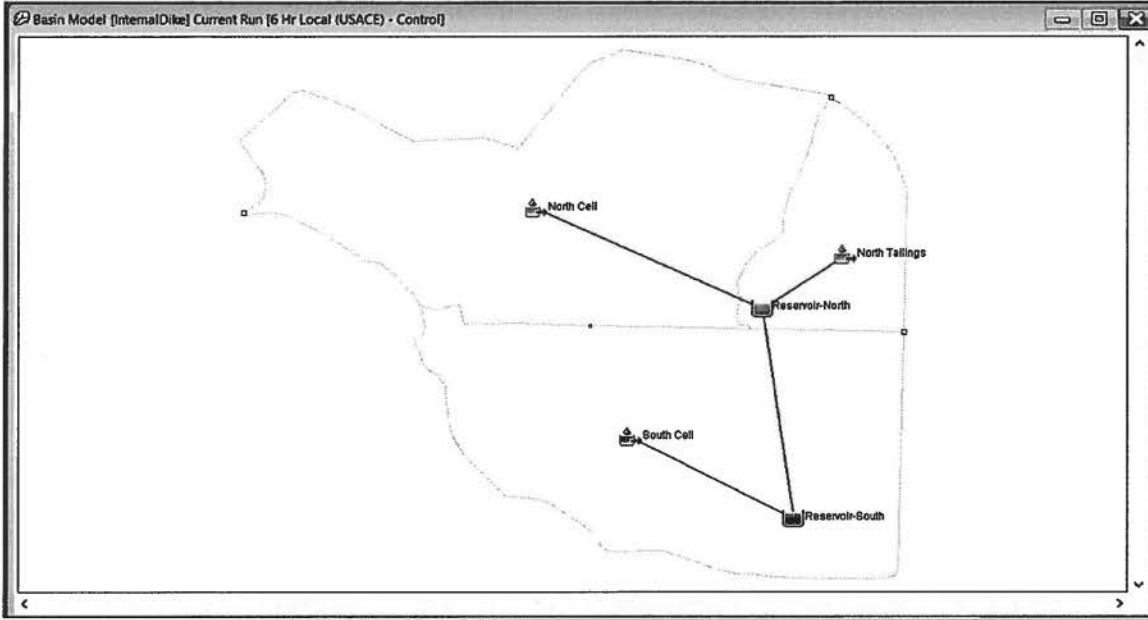
Headwater Elevation		5236.00	ft
Crest Elevation		5232.00	ft
Tailwater Elevation		5228.00	ft
Crest Surface Type	Gravel		
Crest Breadth		82.00	ft
Crest Length		40.00	ft

Headwater Elevation (ft)	Discharge (ft <sup>3</sup> /s)	Velocity (ft/s)
5232.00		
5234.00	336.81	4.21
5236.00	972.80	6.08
5238.00	1787.15	7.45
5240.00	2751.49	8.60



**ATTACHMENT 2-4  
HEC-HMS INPUTS AND OUTPUTS**

HEC-HMS Basin Model Schematic



Sub-Basin Parameters

Subbasin Area [InternalDike]

Show Elements: All Elements      Sorting: Alphabetic

Subbasin	Area (M2)
North Cell	0.3484
North Tailings	0.0896
South Cell	0.3309

Apply    Close

Initial Constant Loss [InternalDike]

Show Elements: All Elements      Sorting: Alphabetic

Subbasin	Initial Loss (I <sub>0</sub> )	Constant Rate (I <sub>0</sub> /HR)	Impervious (%)
North Cell	0	0.075	0.0
North Tailings	0	0.025	0.0
South Cell	0	0.075	0.0

Apply    Close

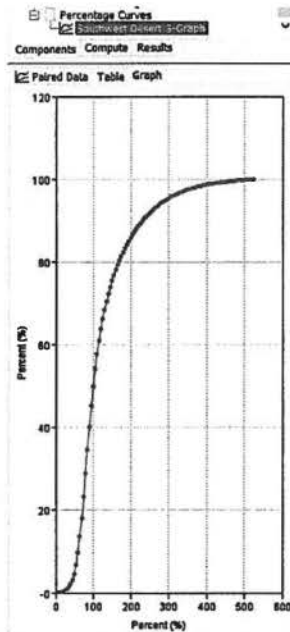
Lag Data and Unit Hydrograph Transform

User Specified S-Graph Transform [InternalDike]

Show Elements: All Elements      Sorting: Alphabetic

Subbasin	S-Graph Table	Lag Time (HR)
North Cell	Southwest Desert S-Graph	0.36
North Tailings	Southwest Desert S-Graph	0.23
South Cell	Southwest Desert S-Graph	0.64

Apply    Close



USACE Rainfall Distribution

Time-Series Gage    Time Window    Table    Graph

Time (ddMMYYYY, HH:MM)    Precipitation (I<sub>0</sub>)

01Jan2050, 12:00	
01Jan2050, 13:00	0.36
01Jan2050, 14:00	0.59
01Jan2050, 15:00	1.90
01Jan2050, 16:00	11.87
01Jan2050, 17:00	0.83
01Jan2050, 18:00	0.47

HMR Rainfall Distribution

Time-Series Gage    Time Window    Table    Graph

Time (ddMMYYYY, HH:MM)    Precipitation (I<sub>0</sub>)

01Jan2050, 12:00	
01Jan2050, 13:00	0.36
01Jan2050, 14:00	0.83
01Jan2050, 15:00	11.87
01Jan2050, 16:00	1.90
01Jan2050, 17:00	0.59
01Jan2050, 18:00	0.47

North Cell - Elevation vs. Storage

Elevation-Storage Functions

North Cell  
South Cell

Components Compute Results

Paired Data Table Graph

Elevation (FT)	Storage (AC-FT)
5228.0	0.0
5230.0	25.0
5232.0	86.0
5234.0	176.0
5236.0	281.0
5238.0	393.0
5240.0	515.0

Storage vs. Discharge - 40-Foot Wide Weir

Storage-Discharge Functions

North Cell-40 ft Weir  
South Cell

Elevation-Storage Functions  
Percentage Curves

Components Compute Results

Paired Data Table Graph

Storage (AC-FT)	Discharge (CFS)
0.0	0.00
25.0	0.00
86.0	0.00
176.0	336.81
281.0	972.80
393.0	1787.15
515.0	2751.49

South Cell - Elevation vs. Storage

Elevation-Storage Functions

North Cell  
South Cell

Components Compute Results

Paired Data Table Graph

Elevation (FT)	Storage (AC-FT)
5190.0	0.0
5192.0	2.9
5194.0	7.1
5196.0	13.3
5198.0	23.9
5200.0	39.9
5202.0	60.7
5204.0	85.8
5206.0	116.0
5208.0	152.6
5210.0	202.6
5212.0	267.9
5214.0	343.3
5216.0	428.4
5218.0	521.7
5220.0	624.4
5222.0	738.2
5224.0	864.4
5226.0	1002.9
5228.0	1144.9
5230.0	1300.9
5232.0	1464.5
5234.0	1635.4
5236.0	1812.8
5238.0	1995.1
5240.0	2182.8

South Cell - Storage vs. Discharge

Paired Data

Storage-Discharge Functions

North Cell-40 ft Weir  
South Cell

Elevation-Storage Functions  
Percentage Curves

Components Compute Results

Paired Data Table Graph

Storage (AC-FT)	Discharge (CFS)
0.0	0.0
2.9	0.0
7.1	0.0
13.3	0.0
23.9	0.0
39.9	0.0
60.7	0.0
85.8	0.0
116.0	0.0
152.6	0.0
202.6	0.0
267.9	0.0
343.3	0.0
428.4	0.0
521.7	0.0
624.4	0.0
738.2	0.0
864.4	0.0
1002.9	0.0
1144.9	0.0
1300.9	0.0
1464.5	0.0
1635.4	0.0
1812.8	0.0
1995.1	0.0
2182.8	0.0

HEC-HMS Global Summary - HMR Rainfall Distribution

Global Summary Results for Run "6Hr Local (HMR)"

Project: CopperFlatTSF Simulation Run: 6Hr Local (HMR)

Start of Run: 01Jan2050, 12:00 Basin Model: InternalDike  
 End of Run: 02Jan2050, 12:00 Meteorologic Model: 6-Hr Local PMP (HMR)  
 Compute Time: 11Dec2017, 10:24:52 Control Specifications: 24hr, 1min

Show Elements:  Volume Units:  IN  AC-FT Sorting:

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
North Cell	0.3484	2528.1	01Jan2050, 15:05	289.3
North Tailings	0.0896	680.0	01Jan2050, 15:01	75.8
Reservoir-North	0.4380	828.9	01Jan2050, 15:44	278.7
Reservoir-South	0.7689	0.0	01Jan2050, 12:00	0.0
South Cell	0.3309	2125.2	01Jan2050, 15:16	274.8

HEC-HMS Global Summary - USACE Rainfall Distribution (Controls - Higher Peak Qs)

Global Summary Results for Run "6 Hr Local (USACE) - Control"

Project: CopperFlatTSF Simulation Run: 6 Hr Local (USACE) - Control

Start of Run: 01Jan2050, 12:00 Basin Model: InternalDike  
 End of Run: 02Jan2050, 12:00 Meteorologic Model: 6-Hr Local PMP (USACE)  
 Compute Time: 11Dec2017, 10:21:55 Control Specifications: 24hr, 1min

Show Elements:  Volume Units:  IN  AC-FT Sorting:

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
North Cell	0.3484	2537.9	01Jan2050, 16:05	289.3
North Tailings	0.0896	680.4	01Jan2050, 16:01	75.8
Reservoir-North	0.4380	936.6	01Jan2050, 16:31	278.6
Reservoir-South	0.7689	0.0	01Jan2050, 12:00	0.0
South Cell	0.3309	2149.7	01Jan2050, 16:15	274.8

Flow Results

Summary Results for Reservoir "Reservoir-North"

Project: CopperFlatTSF Simulation Run: 6 Hr Local (USACE) - Control

Reservoir: Reservoir-North

Start of Run: 01Jan2050, 12:00 Basin Model: InternalDike  
 End of Run: 02Jan2050, 12:00 Meteorologic Model: 6-Hr Local PMP (USACE)  
 Compute Time: 11Dec2017, 10:21:55 Control Specifications: 24hr, 1min

Volume Units:  IN  AC-FT

Computed Results

Peak Inflow: 3213.7 (CFS)	Date/Time of Peak Inflow: 01Jan2050, 16:04
Peak Discharge: 936.6 (CFS)	Date/Time of Peak Discharge: 01Jan2050, 16:31
Inflow Volume: 365.1 (AC-FT)	Peak Storage: 275.0 (AC-FT)
Discharge Volume: 278.6 (AC-FT)	Peak Elevation: 5235.9 (FT)



ENVIRONMENT DEPARTMENT INTERNAL MEMORANDUM

TO: Brad Reid, Mining Environmental Compliance Section  
John Moeny, Surface Water Quality Bureau

FROM: Jeff Lewellin, NMED Mining Act Team Leader

DATE: December 15, 2017

SUBJECT: **Request for Comments, Regular New Mine, New Mexico Copper Corporation, Copper Flat Mine, Review of Documents, Sierra County, MMD Permit No. SI027RN**

---

Attached is a copy of a cover letter received from the Mining Act Reclamation Program (MARP) by the New Mexico Environment Department (NMED) requesting comments regarding the Copper Flat Mine. This is designated as a regular new mine and assigned permit tracking No. SI027RN. Pursuant to §19.10.6.605.C NMAC, NMED has 30 days to provide comment. Please have any comments back to me by January 9, 2017. The documents for review are:

*Probable Hydrologic Consequences of the Copper Flat Project, New Mexico* by John Shoemaker & Associates, Inc. December 2017;

*Predictive Geochemical Modeling of Pit Lake Water Quality, Copper Flat Project, New Mexico* by SRK Consulting, December 11, 2017.

Both documents can be found on the Mining and Minerals Division webpage at:

<http://www.emnrd.state.nm.us/MMD/MARP/PermitSI027RN.html>

Please send all responses to Jeff Lewellin, NMED Mining Act Team Leader.

Attachments:

December 14, 2017, MARP, DJ Ennis, Request for Comments

xc: Richard Goodyear, Chief, AQB  
Shelley Lemon, Acting Chief, SWQB  
Abe Franklin, Program Manager, SWQB  
Michelle Hunter, Chief, GWQB  
Kurt Vollbrecht, Program Manager, GWQB, MECS  
Holland Shepherd, Program Manager, MMD  
DJ Ennis, Lead Staff, MMD

State of New Mexico  
Energy, Minerals and Natural Resources Department

**Susana Martinez**  
Governor

**Ken McQueen**  
Cabinet Secretary

**Matthias Sayer**  
Deputy Cabinet Secretary

**Fernando Martinez, Director**  
Mining and Minerals Division



December 14, 2017

Mr. Jeff Lewellin  
NMED Ground Water Quality Bureau  
P.O. Box 5469  
Santa Fe, NM 87502-5469

**Re: Request for Agency Comments, Copper Flat Mine, Sierra County, New Mexico, Permit Tracking No. SI027RN:**

- **Probable Hydrologic Consequences, Dec. 2017**
- **Predictive Geochemical Modeling of Pit Lake Water Quality, Dec. 2017**

Dear Mr. Lewellin,

On December 13, 2017, the Mining and Minerals Division ("MMD") of the New Mexico Energy, Minerals, and Natural Resources Department received two documents as addendums to the Copper Flat mine application, permit tracking no. SI027RN. The two documents received are:

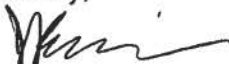
- *Probable Hydrologic Consequences of the Copper Flat Project, Sierra County, New Mexico* by John Shomaker & Associates, Inc., December 2017
- *Predictive Geochemical Modeling of Pit Lake Water Quality, Copper Flat Project, New Mexico* by SRK Consulting, December 11, 2017

These documents are a portion of the overall Permit Application Package being reviewed by MMD in relation to the proposed Copper Flat mine located in Sierra County, New Mexico.

Pursuant to §19.10.6.605.C of the New Mexico Administrative Code, MMD is requesting comments from your agency regarding these two addendums, which can be downloaded from MMD's website at <http://www.emnrd.state.nm.us/mmd/MARP/PermitSI027RN.html>. Please provide any comments your agency may have within 30 days from the date of receipt of this letter.

If you have any questions, please contact me at (505) 476-3434 or by email at [david.ennis@state.nm.us](mailto:david.ennis@state.nm.us).

Sincerely,

  
David J. (DJ) Ennis, P.G., Permit Lead  
Mining Act Reclamation Program (MARP)

cc: Mine File (SI027RN)





## Reid, Brad, NMENV

---

**From:** Lemon, Shelly, NMENV  
**Sent:** Tuesday, December 19, 2017 10:32 AM  
**To:** Vollbrecht, Kurt, NMENV; Dail, Bryan, NMENV  
**Cc:** Reid, Brad, NMENV  
**Subject:** RE: Copper Flat Pit geochem

Thank you!

---

**From:** Vollbrecht, Kurt, NMENV  
**Sent:** Tuesday, December 19, 2017 9:32 AM  
**To:** Lemon, Shelly, NMENV <Shelly.Lemon@state.nm.us>; Dail, Bryan, NMENV <Bryan.Dail@state.nm.us>  
**Cc:** Reid, Brad, NMENV <brad.reid@state.nm.us>  
**Subject:** FW: Copper Flat Pit geochem

FYI-we'll talk to Pat about coordinating with SWQB.

Kurt Vollbrecht, Program Manager  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
(505) 827-0195

---

**From:** Reid, Brad, NMENV  
**Sent:** Tuesday, December 19, 2017 9:00 AM  
**To:** Vollbrecht, Kurt, NMENV <[kurt.vollbrecht@state.nm.us](mailto:kurt.vollbrecht@state.nm.us)>; Lewellin, Jeffrey, NMENV <[Jeffrey.Lewellin@state.nm.us](mailto:Jeffrey.Lewellin@state.nm.us)>  
**Subject:** Copper Flat Pit geochem

I gave Pat the updated SRK model of the Copper Flat Pit Lake - he seems excited. He will bill his hours to Copper Rule and expects to get started next week. I told him we are primarily interested in the model of the reclaimed pit since he has reviewed the other two already.

Brad Reid, Geologist  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
P.O. Box 5469  
Santa Fe, NM 87502  
Phone: [505.827.2963](tel:505.827.2963); Fax: [505.827.2965](tel:505.827.2965)



## Reid, Brad, NMENV

---

**From:** Knight, Andrew, NMENV  
**Sent:** Tuesday, January 02, 2018 1:05 PM  
**To:** Vollbrecht, Kurt, NMENV; Reid, Brad, NMENV; Dail, Bryan, NMENV  
**Subject:** FW: Copper Flat Inter Se Decision Put Out on December 28, 2017  
**Attachments:** Judge Wechsler letter to Parties Copper Flat Inter Se.pdf; Court Findings of Fact and Conclusions of Law 12.28.17.pdf

Fyi,

I guess this is in their water rights case, but looks like it may have implications for the waters of the state determination. I assume THEMAC will appeal this decision.

Andrew P. Knight  
Assistant General Counsel  
New Mexico Environment Department  
Office: (505) 222-9540  
Cell: (505) 907-8836

**From:** Jai Par [mailto:peimiaj@gmail.com]  
**Sent:** Tuesday, January 02, 2018 12:22 PM  
**To:** Knight, Andrew, NMENV <Andrew.Knight@state.nm.us>  
**Subject:** Copper Flat Inter Se Decision Put Out on December 28, 2017

Good morning and Happy New Year, Andrew! I'm not sure if you're aware of the recent decision issued by Judge Wechsler in the Copper Flat Inter Se proceeding, but here it is for your review. Of interest is the Court's finding of fact on page 61 regarding the Copper Flat Copper Mine's open pit: LRG-4652-S-17, the open pit, "is hydrologically connected to groundwater and evaporates at an amount of 34.45 afy", and cites to an exhibit offered by the Office of the State Engineer in support of that fact. Under the private waters exemption, the open pit cannot be determined to be a private water if it is hydrologically connected to groundwater. We now have the Third Judicial District Court upholding the NM Office of the State Engineer's assertion that the open pit is hydrologically connected to groundwater.

Judge Wechsler also went on to find that the mine has only around 900 afy of water rights, which is not enough to operate the mine. Look at pages 70-71 for the amount of water right in each well.

Kind Regards,

Jaimie Park



**From:** [Ennis, David, EMNRD](#)  
**To:** [Lewellin, Jeffrey, NMENV](#)  
**Cc:** [Vollbrecht, Kurt, NMENV](#); [Lemon, Shelly, NMENV](#); [Reid, Brad, NMENV](#)  
**Subject:** RE: Copper Flat Mine - NMED Comments - Request for Extension  
**Date:** Monday, January 08, 2018 8:45:39 AM

---

NMED's request for an extension of time is approved until February 12, 2018.

Thanks  
DJ

---

**From:** Lewellin, Jeffrey, NMENV  
**Sent:** Friday, January 5, 2018 3:50 PM  
**To:** Ennis, David, EMNRD <David.Ennis@state.nm.us>  
**Cc:** Vollbrecht, Kurt, NMENV <kurt.vollbrecht@state.nm.us>; Lemon, Shelly, NMENV <Shelly.Lemon@state.nm.us>; Reid, Brad, NMENV <brad.reid@state.nm.us>  
**Subject:** Copper Flat Mine - NMED Comments - Request for Extension

DJ – We are requesting an extension until February 12, 2018 to provide comment on the two reports sent to NMED on December 14, 2017 related to Copper Flat. Please let us know if the request is granted. Thanks, Jeff

Jeff Lewellin, Mining Act Team Leader  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
(505) 827-1049

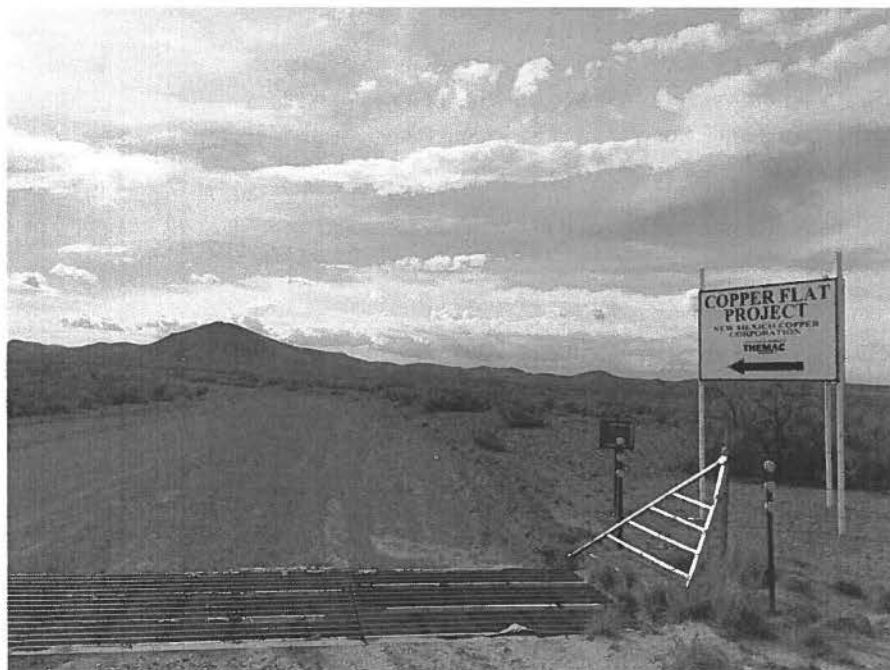


**ENVIRONMENT**

January 8, 2018

# Judge tosses out water rights mining company was banking on for Copper Flat

By Laura Paskus



Laura Paskus

The Copper Flat mine near Hillsboro operated briefly in the early 1980s.

At the end of last year, a state judge chipped away at a company's plans to reopen a long-abandoned copper mine near Hillsboro. On Dec. 28, New Mexico Third Judicial District Court Judge James J. Wechsler found that most of the water rights claimed by the company are not valid.

[New Mexico Copper Corporation \(NMCC\)](http://nmpoliticalreport.com/146649/questions-remain-as-company-seeks-to-re-open-long-defunct-copper-mine/) (<http://nmpoliticalreport.com/146649/questions-remain-as-company-seeks-to-re-open-long-defunct-copper-mine/>), planned to use groundwater rights that two men purchased after operations were abandoned at Copper Flat Mine in 1982. William Frost and Harris Gray, along with NMCC and its attorneys, tried to show that those rights were still valid, even though the water hadn't been put to use over the past four decades—or even when the mine operated.

Under the New Mexico Constitution, water is owned by the public. People or entities must put water to "beneficial use" to maintain their rights, whether to surface water from rivers and streams or water pumped from below the ground.

Wechsler found that the combined water right of Frost, Harris and NMCC is only 861.84 acre feet per year, or about 281 million gallons.

"Certainly, we were disappointed in the outcome, in the judge's decision, and we're considering our options now," said Jeff Smith, chief operating officer of NMCC, a subsidiary of the Canada-based company, THEMAC Resources. Smith said it was premature to



discuss other options for obtaining water rights for the mine, but added he is confident the project will move forward.

Each year, the mine will require about 6,000 acre feet of water, Smith said, and is expected to operate for at least 12 years while employing between 275 and 300 people.

"We have done a tremendous amount of work on planning our operation and planning for and analyzing what potential effects may be of the operation and then putting into place elements for environmental protection or groundwater protection, or offsetting effects that our pumping might have," he said. "We have in place now agreements and plans for other water users in the area that we have discussed with the state to offset any negative effects we might have."

In its 2017 financial report, the company noted it was working with the New Mexico Office of the State Engineer to "incorporate the use of the Jicarilla water into the terms of the State pumping permit for the mine."

That refers to the company's agreement to lease about 3,000 acre-feet of San Juan-Chama water from the Jicarilla Apache Nation in northwestern New Mexico. The tribe has rights to water that is piped in tunnels from the San Juan River and into the Chama River, which flows into the Rio Grande north of Española.

According to that 2015 agreement, that water wouldn't be used at the mine, but would "offset" water depletions in the Lower Rio Grande. That is, it would replace the water that is lost to the river because of groundwater pumping.

## **Decades of plans and deals, just months of operations**

The Copper Flat Mine operated for just a few months before being closed in July 1982. In 1984, when the state was evaluating how much water was being used in the Lower Rio Grande Basin, the company tallied up its water rights for each of its 18 groundwater wells and the open pit and declared it had rights to 6,462 acre feet of water annually.

But during the mine's brief life, the company had put less than 900 acre feet of water to beneficial use for mining, milling, reclamation, dust control, wash water and employee use.

A few years later, the owners tried to sell the water rights to the City of Las Cruces, which declined. At that time, Phelps Dodge, which operated the area's other copper mines near Silver City, also declined to buy its liquidated mine assets. In 1986, Copper Flat Partnership (owned by a Delaware corporation, Quintana and Phibro) and its financial partner, the Canadian Imperial Bank of Commerce, sold all the removable physical assets of the mine to a Papua New Guinea-based mining company. And in 1987, the company notified the state of New Mexico that the mine was permanently closed and would not be restarted.

It was during that time that Frost and Gray, a real estate agent and an accountant, bought the mine's water rights, 6,462 acre feet, for \$20,000. During the 1990s, the mine's new owners—Gold Express and Alta Gold—paid the two men about \$400,000 in total for their consent to use the water rights from previously-drilled groundwater wells.

In 2009, NMCC bought Copper Flats from Hydro Resources Corporation, another new owner, and the following year Frost and Gray entered into an option agreement with the company for the sale of those declared water rights.

But the state pointed out that those water rights hadn't been put to beneficial use for years, and were no longer valid. And now, Wechsler has agreed.

Hillsboro resident Max Yeh, a local opponent of the mine, said he hopes the judge's decision will prompt the owners to reconsider reopening Copper Flat. "There are initial, emotional reactions to this kind of thing, locally," Yeh said. "But it is fairly clear from this decision that in this arid environment, this mine might do great damage."

Yeh and other Hillsboro residents worry about local impacts from the mine—from the mine itself and its tailing [O](#), as well as from dust. But the bigger problem, [O](#)h said, is how groundwater pumping could affect water resources not just in the immediate area, but within the Lower Rio Grande Basin.

And Yeh likely isn't the only one worried about possible impacts to the Lower Rio Grande Basin.

Texas sued New Mexico and Colorado (<http://nmpoliticalreport.com/784640/rio-woes-en/>), over water issues in the Lower Rio Grande, alleging that by allowing farmers in the southern part of the state to pump groundwater, New Mexico violated the Rio Grande Compact and didn't send enough water downstream. The U.S. Bureau of Reclamation also joined the suit, saying that New Mexico is allowing people to use more water than they legally should. If New Mexico loses the case, which is being heard in the U.S. Supreme Court, southern farmers could be forced to curtail groundwater pumping, and the state would likely have to pay out billions of dollars in damages.

## **EIS in the pipeline, other questions remain**

Opening a new mine is no easy task, and the company still must find water and procure federal and state permits.

In 2010, NMCC started working to line up permits and perform baseline studies on wildlife and water. And in 2015, the U.S. Bureau of Land Management released a draft Environmental Impact Statement for how the proposed open pit mine, its mill, waste rock pile, stockpile and other facilities might affect things like local wildlife, water supplies and vegetation.

Hillsboro residents, environmental groups, Turner Ranch Properties—which owns the nearby Ladder Ranch—and New Mexico's two U.S. senators all submitted comments pointing out problems in the analyses from the mine and the federal government. At that time, the New Mexico Interstate Stream Commission noted that BLM didn't adequately consider the project's impacts on New Mexico's ability to meet its Rio Grande water delivery requirements to Texas. And the Elephant Butte Irrigation District and the New Mexico Pecan Growers commented that pumping groundwater for the mine could affect water rights owned by its members along the Rio Grande.

Next, the BLM will issue a final EIS and, eventually, a decision on whether the mine can proceed. Opponents of the mine, meanwhile, asked the BLM to require that the company complete a supplemental EIS, which is required under federal law if a project changes substantially or new information arises.

According to the BLM, the agency will release an answer on that issue as soon as this week.

There were also questions as to whether the mine's pit lake would be on federal or private land, which would affect which water quality regulations would apply to the company. According to the company's plans, the existing pit would be enlarged from 102 to 169 acres.

Last year, the company commissioned surveys to determine the land status of the existing pit, which was believed to cross both federal and private land owned by the company. The BLM agreed with the new survey, concluding that the existing pit lake is on private land, and the company says its design for the future pit will be limited to private lands.

If the New Mexico Environment Department, which did not respond to requests for information for this story, agrees that the pit is "private water" it would not be subject to surface or groundwater quality standards under the federal Clean Water Act.

The company will also need a new mine permit from the New Mexico Mining and Minerals Division.

Regardless of the options NMCC chooses to find water, the company will need state approval. Any groundwater pumping will require a permit from the Office of the State Engineer, for instance. And the agreement to offset water depletions with the Jicarilla Apache Nation's water isn't a done deal yet, either.

Conversations about Copper Flat made their way to the Office of the Governor, as well. In an April 2017 email to State Engineer Tom Blaine requesting a meeting about NMCC's permit application, Deborah Peacock, a board member of NMCC and THEMAC, noted she spoke recently with Gov. Susana Martinez's Chief of Staff Keith Gardner about Copper Flat. Peacock, a patent attorney and regent of New Mexico Tech, copied Gardner on the message, noting she wanted to "keep him in the loop."

For now, however, Wechsler's decision has many wondering how the project can realistically go forward without Frost and Gray's water rights.

In an emailed statement to *NM Political Report*, the attorney for Turner Ranch Properties, Tessa Davidson, said the group was pleased with the judge's decision.


"In the case, New Mexico Copper Corporation took the position that its water rights claims were exempt from New Mexico's constitutional requirement of beneficial use," wrote Davidson on behalf of her client. "The judge's decision confirms that beneficial use is the basis of all rights to use water in the state, and that no particular water user, or type of industry, is exempt from the law."

[New Mexico Copper Corp. water rights decision \(https://www.scribd.com/document/368434685/New-Mexico-Copper-Corp-water-rights-decision#from\\_embed\)](https://www.scribd.com/document/368434685/New-Mexico-Copper-Corp-water-rights-decision#from_embed) by [New Mexico Political Report \(https://www.scribd.com/user/284030527/New-Mexico-Political-Report#from\\_embed\)](https://www.scribd.com/user/284030527/New-Mexico-Political-Report#from_embed) on Scribd


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 (<http://nmpoliticalreport.com/792260/judge-tosses-out-water-rights-mining-company-was-banking-on-for-copper-flats-en/?share=twitter>)

 (<http://nmpoliticalreport.com/792260/judge-tosses-out-water-rights-mining-company-was-banking-on-for-copper-flats-en/?share=email>)

 (<http://nmpoliticalreport.com/792260/judge-tosses-out-water-rights-mining-company-was-banking-on-for-copper-flats-en/?share=google-plus-1>)

 (<http://nmpoliticalreport.com/792260/judge-tosses-out-water-rights-mining-company-was-banking-on-for-copper-flats-en/#print>)

 More (#)

## Related





## Reid, Brad, NMENV

---

**From:** Knight, Andrew, NMENV  
**Sent:** Monday, January 22, 2018 4:53 PM  
**To:** Reid, Brad, NMENV; Vollbrecht, Kurt, NMENV  
**Subject:** FW: IPRA Request Regarding Copper Flat Copper Mine  
**Attachments:** Emails - ELC 01 08 18.pdf; DP-1840 DraftDP.pdf

---

**From:** Knight, Andrew, NMENV  
**Sent:** Monday, January 22, 2018 4:44 PM  
**To:** 'Jaimie Park' <jpark@nmelc.org>  
**Subject:** RE: IPRA Request Regarding Copper Flat Copper Mine

Ms. Park,

This email responds to your IPRA request of January 8, 2018. You requested the following documents:

1. All documents pertaining to a Use Attainability Assessment (“UAA”) for the proposed Copper Flat Copper Mine submitted or generated since December 14, 2017;
2. All documents pertaining to the Copper Flat Mine’s existing uses for its current pit lake;
3. All documents pertaining to Copper Flat cooperating agency meetings held in December 2017, January 2018 or to be held in January 2018 through March 2018;
4. All draft discharge permits for NMCC’s draft discharge permit application; Revised 6/14/12
5. All documents pertaining to NMED’s technical completeness determination regarding NMCC’s discharge permit application;
6. The preliminary final environmental impact statement prepared by BLM and/or its contractor;
7. The draft mining permit prepared by MMD;
8. The preliminary final Biological Assessment prepared by BLM and any comments on the final draft submitted by cooperating agencies.

The attached documents were identified as being responsive to your request. The Department did not have any documents responsive to requests numbered 1, 3, 6, 7, or 8. There are no new UAA documents and there have been no recent cooperating agency meetings. Since there is no date limitation for item #2 this would include documents that have been previously provided through prior ELC IPRA’s regarding pit lake existing uses as it relates to the pit jurisdictional (waters of the state) issue. We have not duplicated documents that were previously provided on this subject. Although the application has not been deemed technically complete, I have included the draft DP-1840 in its current form. Note there may be additional changes to this document before it is released for public notice. Please let me know if you have any questions regarding this response.

Andrew P. Knight  
Assistant General Counsel  
New Mexico Environment Department  
Office: (505) 222-9540  
Cell: (505) 907-8836

---

**From:** Jaimie Park [<mailto:jpark@nmelc.org>]  
**Sent:** Monday, January 08, 2018 5:39 PM

**To:** Mascarenas, Melissa, NMENV <[melissa.mascarenas@state.nm.us](mailto:melissa.mascarenas@state.nm.us)>

**Cc:** Knight, Andrew, NMENV <[Andrew.Knight@state.nm.us](mailto:Andrew.Knight@state.nm.us)>; Reid, Brad, NMENV <[brad.reid@state.nm.us](mailto:brad.reid@state.nm.us)>

**Subject:** IPRA Request Regarding Copper Flat Copper Mine

Dear Ms. Mascarenas,

Please find attached an IPRA request regarding the proposed Copper Flat Copper Mine.

Kind Regards,

Jaimie Park



**NEW MEXICO ENVIRONMENT DEPARTMENT  
INSPECTION OF PUBLIC RECORD REQUEST FORM**

Please fill out the following information:

1. Date: January 8, 2018
2. Requestor's Name: Jaimie Park, New Mexico Environmental Law Center
3. Requestor's Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505  
\_\_\_\_\_
4. Phone No.: (505) 989-9022
5. Email: jpark@nmelc.org
6. Company Being Represented: New Mexico Environmental Law Center
7. Address: 1405 Luisa Street, Suite 5, Santa Fe, NM 87505
8. Document or File being requested to be reviewed or copied (please describe the records in sufficient detail to enable Department personnel to reasonably identify & locate the records:

I request that you inform me what documents are available within the scope of this request and when I can inspect those documents. I also request that you not make any copies without first informing me of the number of copies and the cost for the copying that are involved. Finally, I request that if you determine that any documents or portions of documents are exempt from disclosure you inform me of that and provide me with citations to the provisions in the Inspection of Public Records Act that indicate that the documents or portions of documents are exempt from disclosure, describe the type of document being withheld, and identify who sent the document being withheld and who received the document being withheld.

**Records being requested:**

1. All documents pertaining to a Use Attainability Assessment ("UAA") for the proposed Copper Flat Copper Mine submitted or generated since December 14, 2017;
2. All documents pertaining to the Copper Flat Mine's existing uses for its current pit lake;
3. All documents pertaining to Copper Flat cooperating agency meetings held in December 2017, January 2018 or to be held in January 2018 through March 2018;
4. All draft discharge permits for NMCC's draft discharge permit application;

5. All documents pertaining to NMED's technical completeness determination regarding NMCC's discharge permit application;
6. The preliminary final environmental impact statement prepared by BLM and/or its contractor;
7. The draft mining permit prepared by MMD;
8. The preliminary final Biological Assessment prepared by BLM and any comments on the final draft submitted by cooperating agencies.

For purposes of this request, the term "document" means any record in written, graphic, photographic, or other form kept or memorialized on paper, microfilm, microfiche, or electronic media; and includes each non-identical original or copy of a draft or final record, whether the original or copy is not identical because of notes on the original or copy or otherwise.

For purposes of this request, the term "pertaining to" means addressing, concerning, focusing on, mentioning, relating to, or relevant to in any manner.

For purposes of this request, "existing uses" means "a use actually attained in a surface water of the state on or after November 28, 1975, whether or not it is a designated use". Section 20.6.4.7.E(3) NMAC.

9. NMED Bureau where Document/File can be found (if known): Surface Water Quality Bureau, Ground Water Quality Bureau

---

Signature

The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: [melissa.mascarenas@state.nm.us](mailto:melissa.mascarenas@state.nm.us)**





SUSANA MARTINEZ  
Governor  
JOHN A. SANCHEZ  
Lt. Governor

NEW MEXICO  
ENVIRONMENT DEPARTMENT

Harold Runnels Building  
1190 Saint Francis Drive (87505)  
PO Box 5469, Santa Fe, NM 87502-5469  
Phone (505) 827-2990 Fax (505) 827-1628  
[www.env.nm.gov](http://www.env.nm.gov)



BUTCH TONGATE  
Cabinet Secretary  
J.C. BORREGO  
Deputy Secretary

January 9, 2018

VIA E-MAIL

Jaimie Park  
[jpark@nmelc.org](mailto:jpark@nmelc.org)

Re: Request to Inspect Public Records

Dear Ms. Park:

On January 9, 2018, this office received your request for public information. You request information pertaining to: Copper Flat Copper Mine. (See attached request).

I forwarded your request to the bureaus on January 9, 2018. The bureaus will respond by January 22, 2018.

Should you have any questions, please contact the Ground Water Quality Bureau at (505) 827-2919 and the Surface Water Quality Bureau at (505) 827-2819.

Sincerely,

Melissa Y. Mascareñas  
New Mexico Environment Department  
Department Public Records Custodian

cc: Andrew Knight, Assistant General Counsel  
Michelle Hunter, Chief, Ground Water Quality Bureau  
Shelly Lemon, Chief, Surface Water Quality Bureau



## Reid, Brad, NMENV

---

**From:** Juan Velasquez <jvelasquez@vemsinc.com>  
**Sent:** Monday, January 29, 2018 5:01 PM  
**To:** Reid, Brad, NMENV  
**Cc:** Jeff Smith; 'Katie Emmer'  
**Subject:** Compliance with 20.6.7.22(A)1  
**Attachments:** DP Andesite Plant Figures for Coarse Ore and Run-of- mine Stockpiles.pdf

Brad,

You asked that I provide you with further explanation of the rationale we used in determining how the coarse ore stockpile at Copper Flat will meet the requirements of 20.6.7.22.A(1) which requires that the it be designed such that all materials containing water contaminants that have the potential to migrate to ground water be contained and managed on concrete or low permeability surfaces.

Information contained in NMCC's DP, Revision 1, August 2017 contains the following information that shows how we will meet this requirement. Figure 11K-2a of the application (page 110), a copy of which is attached, is a geologic map of the Copper Flat Mine area. It contains among other things, certain features as explained below, specific to addressing this issue.

1. It depicts the location of the andesite bedrock, quartz monzonite and Santa Fe group formations in relation to the site;
2. It depicts the location of several cross-section lines, the most relevant of which is to this discussion is Cross-section PA-PA'.

I have also attached a couple of other figures important to explaining how NMCC will meet the 22.A.(1) requirement, Figure 11K-4 Hydrogeologic Cross-Section PA-PA', from the DP Application, Revision 1 (page 114) and a figure titled "Plant Area Geology".

The Plant Area Geology figure is not in the DP application. It is an inset that I have added to Figure 11K-2a. This figure shows plant area details including the location of temporary storage of run-of-mine ore (see page 68 of the DP, Rev. 1) and the coarse ore or crushed ore stockpile (see pages 72, 82, 83 and Figure 11J-16 of the DP, Rev. 1).

Figure 11K-4, is included to help orient the more detailed plant area figure to Figure 11K-2a. Figure 11K-4 shows the cross-section line of PA-PA' from the western edge of the site beginning approximately at Well GWQ 96-22 in the andesite across the quartz monzonite (i.e., the pit location) to a point to the east past Well GWQ 96-23, also in the andesite formation. It is important to note from the cross-section that the andesite bedrock formation is over 500 ft. thick, continuing to the east under the plant area.

The presence and location of the andesite bedrock formation is important on these figures. The Plant Area Geology figure shows that the andesite bedrock formation underlies the entire plant site, including the coarse ore and run-of-mine stockpiles. The andesite formation is the same formation that will underlay the waste rock stockpiles. NMCC has determined that the andesite bedrock has a very low permeability of less than  $10^{-6}$  cm/sec. as discussed in the DP, Revision 1, at pages 64 and 65.

Therefore, the requirement of 20.6.7.22.A.(1) is met at these stockpile locations because this low permeability andesite bedrock also underlies them.

I trust that information provides the explanation you requested. Please let me know if you require additional clarification or have other question.

Juan

In printing the figures you might need to choose "document and markups" from the comments and forms drop-down box.

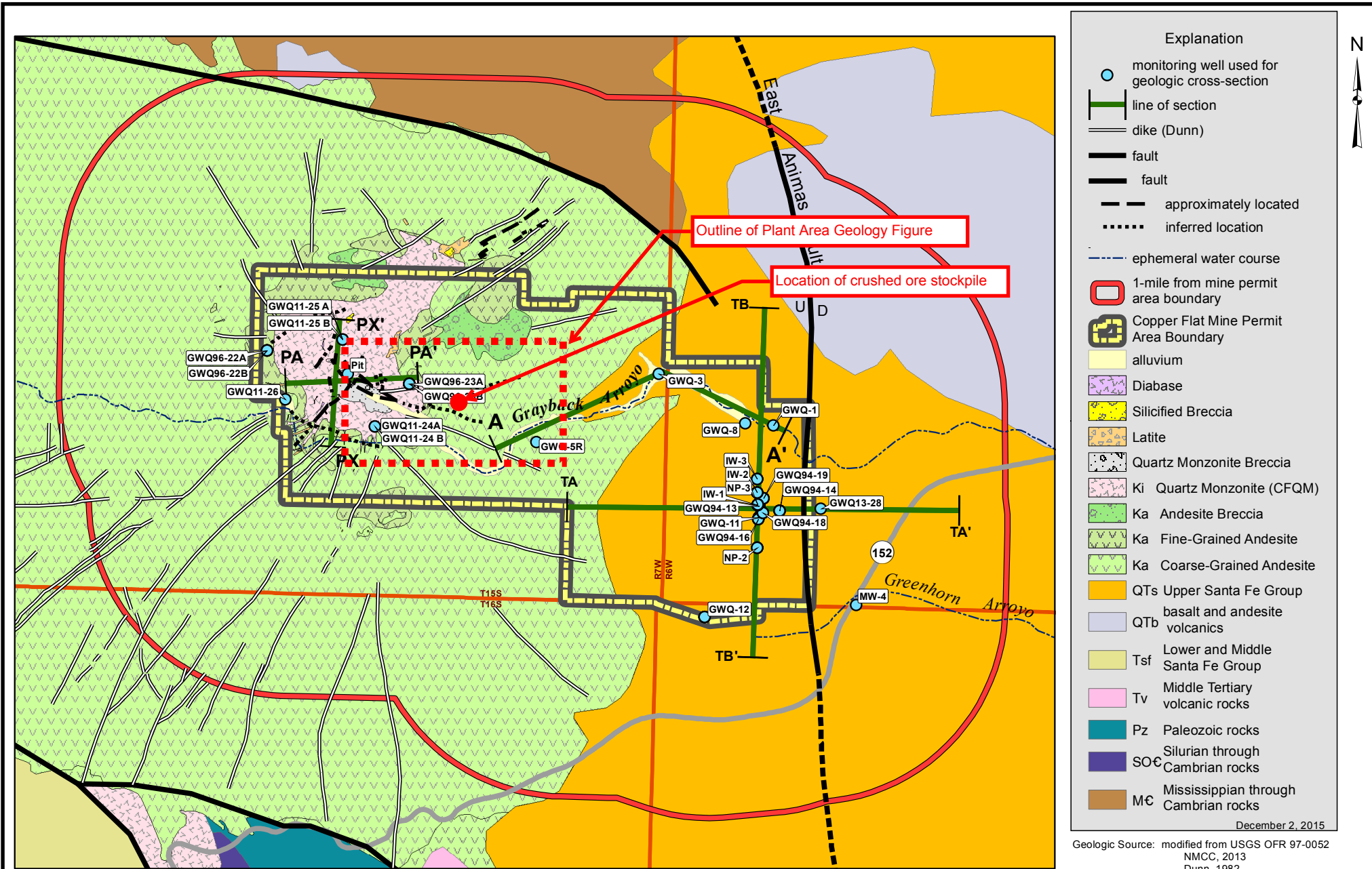


Figure 11K-2a. Geologic map within 1-mile of the Copper Flat Mine Area permit boundary, Sierra County, New Mexico.

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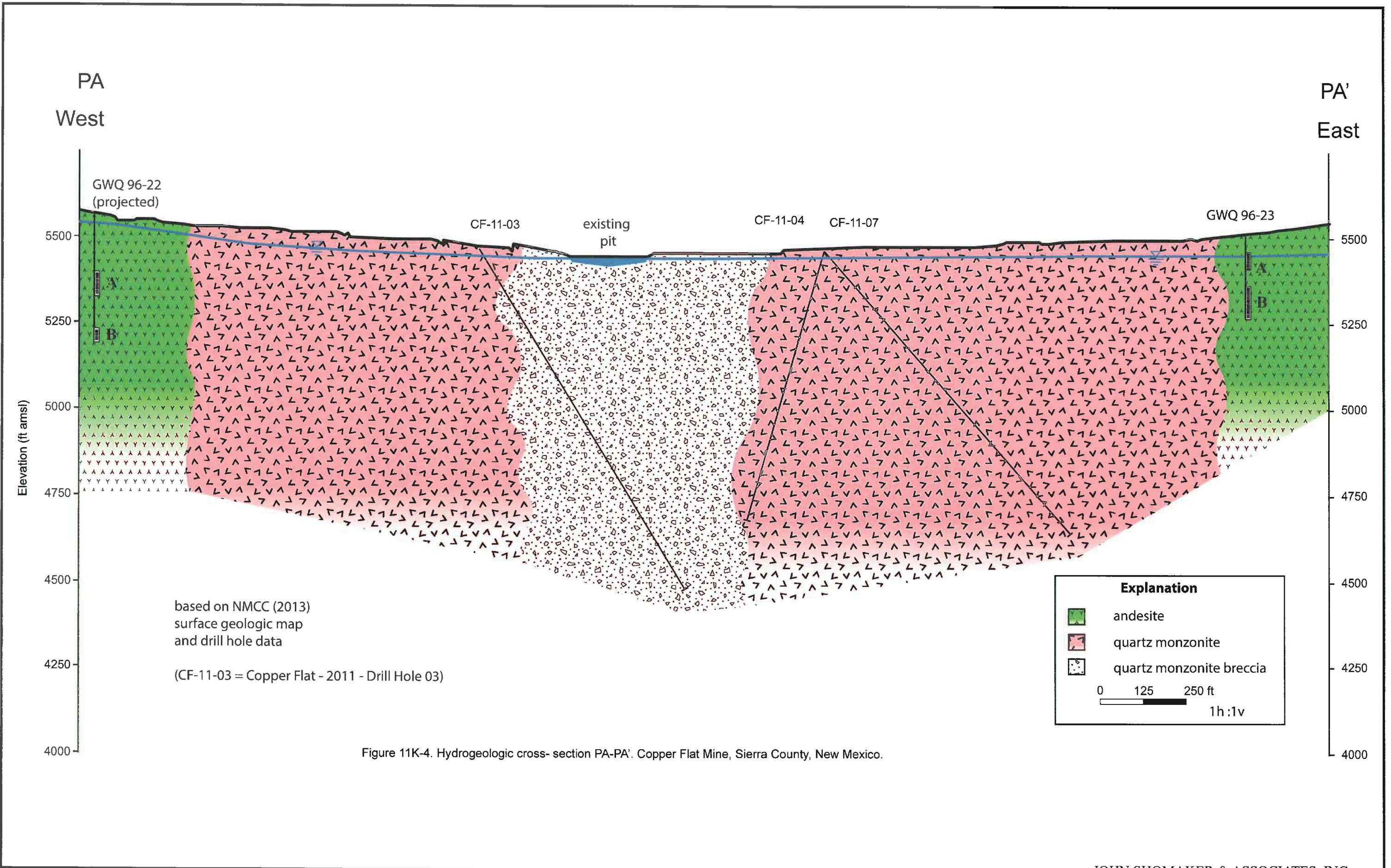


Figure 11K-4. Hydrogeologic cross-section PA-PA'. Copper Flat Mine, Sierra County, New Mexico.

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# Plant Area Geology

**THEMAC**  
RESOURCES  
Copper Flat Project

Figure

December 28, 2017

26

25

35

36

Coarse Ore Stockpile

Run-of-mine stockpile (temporary)

T 15 S R 7 W

## Legend

Proposed Plant Facilities

Sections

Townships

### Joints

Inclined

Vertical

### Faults

Approximate Fault Location

Inferred Fault Location

Fault Zones

### Workings

Adit

Prospect

Shaft

### Contacts

Certain Contact

Approximate Contact

Concealed Contact

Inferred Contact

### Lithology

Extrusive

Diabase

Silicified Breccia

Latite

Quartz Monzonite Breccia

Latite Porphyry

Quartz Monzonite Porphyry

Welded Tuff

Andesite Breccia

Andesite, coarse-grained

Andesite, undifferentiated

Andesite, fine-grained

1 in = 750 feet

0 375 750 1,500 Feet

Coordinate System: NAD\_1983\_UTM\_Zone\_13N  
Projection: Transverse\_Mercator  
False\_Easting: 500000.000000  
False\_Northing: 0.000000  
Central\_Meridian: -105.000000  
Scale\_Factor: 0.999600  
Latitude\_Of\_Origin: 0.000000  
Linear Unit: Meter



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## Reid, Brad, NMENV

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**From:** Reid, Brad, NMENV  
**Sent:** Friday, January 26, 2018 11:07 AM  
**To:** 'Katie Emmer'; Mascarenas, Melissa, NMENV  
**Subject:** RE: NMCC IPRA  
**Attachments:** DP-1840 DraftDP.PDF

Katie,

Here is the draft DP-1840 as of 1/22/18.

Melissa, this IPRA request has been fulfilled.

Brad Reid, Geologist  
Mining Environmental Compliance Section  
Ground Water Quality Bureau  
New Mexico Environment Department  
P.O. Box 5469  
Santa Fe, NM 87502  
Phone: [505.827.2963](tel:505.827.2963); Fax: [505.827.2965](tel:505.827.2965)

---

**From:** Katie Emmer [mailto:[kemmer@themasourcesgroup.com](mailto:kemmer@themasourcesgroup.com)]  
**Sent:** Friday, January 26, 2018 10:43 AM  
**To:** Mascarenas, Melissa, NMENV <[melissa.mascarenas@state.nm.us](mailto:melissa.mascarenas@state.nm.us)>  
**Cc:** Reid, Brad, NMENV <[brad.reid@state.nm.us](mailto:brad.reid@state.nm.us)>  
**Subject:** NMCC IPRA

Greetings Ms. Mascareñas,

Attached please find a request for inspection of public records on behalf of New Mexico Copper Corporation. If you have any questions or need anything further from me, please let me know.

Best regards,

**Katie Emmer | Permitting & Environmental Compliance Manager**

M: +1 505.400.7925 | F: +1 505.881.4616  
A: 4253 Montgomery Blvd. NE, Suite 130, Albuquerque, NM 87109  
W: [themasourcesgroup.com](http://themasourcesgroup.com) | E: [kemmer@themasourcesgroup.com](mailto:kemmer@themasourcesgroup.com)



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**NEW MEXICO ENVIRONMENT DEPARTMENT  
INSPECTION OF PUBLIC RECORD REQUEST FORM**

Please fill out the following information:

1. Date: 26 January 2018
2. Requestor's Name: Katie Emmer
3. Requestor's Address: 4253 Montgomery Blvd, NE, Suite 130  
Albuquerque, NM 87109
4. Phone No.: (505 )400-7925
5. Email: kemmer@themacresourcesgroup.com
6. Company Being Represented: New Mexico Copper Corporation
7. Address: See above
8. Document or File being requested to be reviewed or copied (please describe the records in sufficient detail to enable Department personnel to reasonably identify & locate the records):  
Draft Discharge Permit 1840 for Copper Flat - Brad Reid Permit Lead
9. NMED Bureau where Document/File can be found (if known): GWQB

*Katie Emmer*

Signature

The cost for copying by NMED is as indicated on Attachment A. Please send this request to:

**Melissa Y. Mascareñas**  
**Inspection of Public Records Officer**  
**1190 St. Francis Drive, Ste. N-4050**  
**Santa Fe, New Mexico 87505**  
**fax: (505) 827-1628 or**  
**email: melissa.mascarenas@state.nm.us**



## Reid, Brad, NMENV

---

**From:** Katie Emmer <kemmer@themacresourcesgroup.com>  
**Sent:** Monday, January 29, 2018 5:19 PM  
**To:** Reid, Brad, NMENV  
**Cc:** Juan Velasquez; Jeffrey Smith  
**Subject:** Copper Flat Monitoring Plan Figure 2  
**Attachments:** MonitoringPlan\_Fig2\_28Jan2018.pdf

Greetings Brad,

Per our discussion, a revised version of Figure 2 for Appendix E Rev 1, *Water-Quality Monitoring Plan for the Copper Flat Mine Discharge Permit Pursuant to 20.6.7.11.R and 20.6.7.28 NMAC*, June 2016 by JSAI, Part of NMCC's Discharge Permit Application, is attached.

This revised Figure 2 includes surface-water sampling locations.

Best regards,

**Katie Emmer | Permitting & Environmental Compliance Manager**

M: +1 505.400.7925 | F: +1 505.881.4616

A: 4253 Montgomery Blvd. NE, Suite 130, Albuquerque, NM 87109

W: [themacresourcesgroup.com](http://themacresourcesgroup.com) | E: [kemmer@themacresourcesgroup.com](mailto:kemmer@themacresourcesgroup.com)



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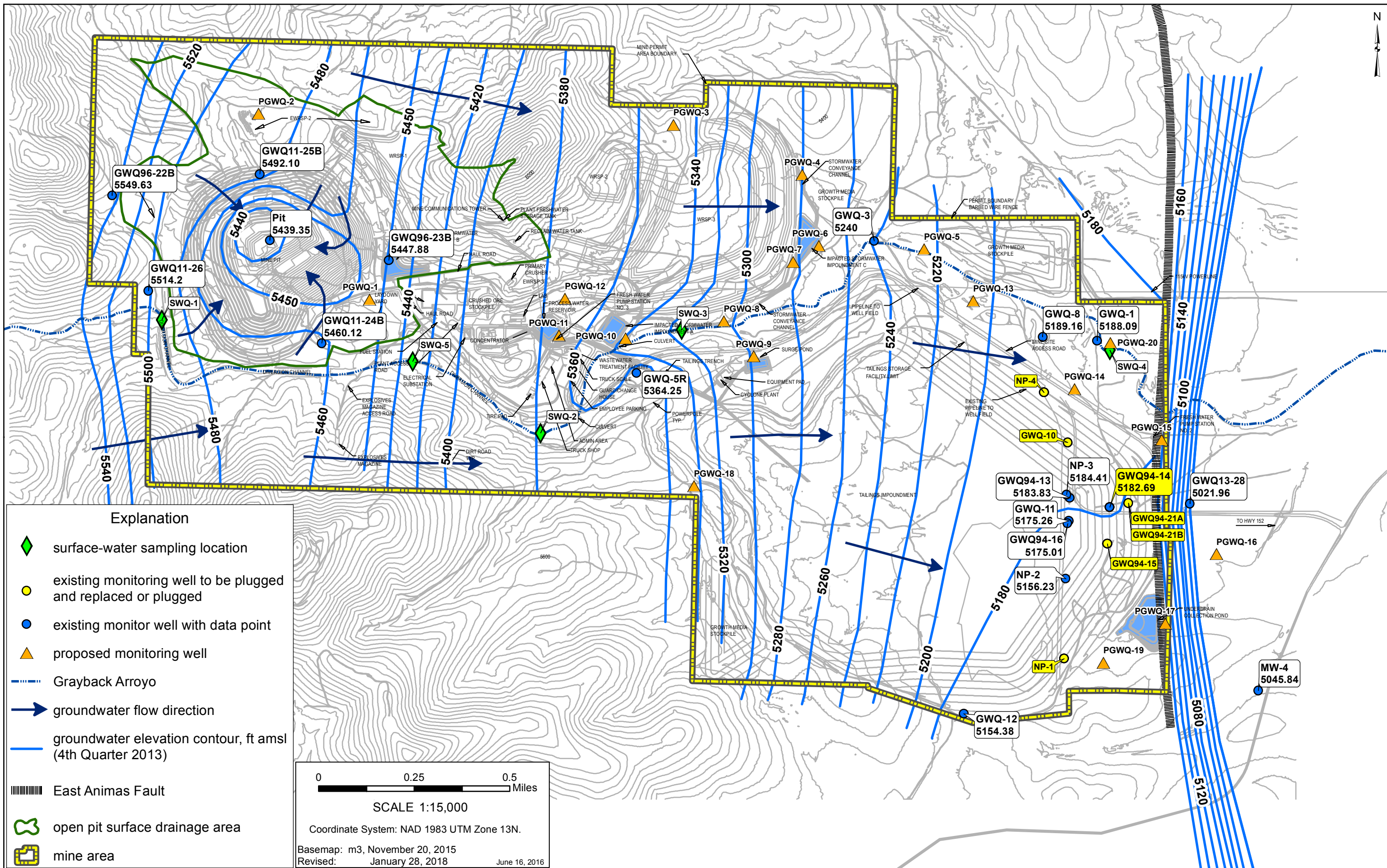
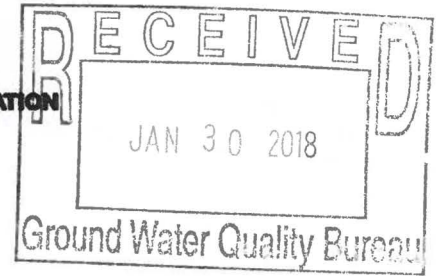


Figure 2. Groundwater flow direction.





NEW  
MEXICO  
COPPER  
CORPORATION



January 25, 2018

Mr. Bryan Dail  
Surface Water Quality Bureau  
New Mexico Environment Department  
P.O. Box 5469  
Santa Fe, NM 87502

**Re: Private Land Status of Open Pit Lake  
New Mexico Copper Corporation**

Dear Mr. Dail,

This letter provides additional information regarding the location of New Mexico Copper property at Copper Flat.

As you may recall, NMCC identified an error with a property boundary on land status maps being used by State and Federal agencies; specifically maps showing private property lines in Sections 25, 26, 35, and 36, Township 15S, Range 7W, N.M.P.M. As I researched this matter, I learned that the maps in question are based on GIS records maintained by the BLM. As described in my September 27, 2017 letter to Ms. Shelley Lemon, Chief of the Surface Water Quality Bureau, the maps using BLM GIS information for this area erroneously show the NMCC property line crossing the existing pit lake. The correct location of the property line is approximately 300 feet west of the existing pit lake, which places the existing pit lake completely within the NMCC owned property.

This matter was raised with BLM in early 2017 in a meeting at BLM's State Office in Santa Fe, which was attended by representatives from BLM cadastral, land legal, and GIS departments, NMCC's registered land survey consultant, and NMCC. During this meeting, and in subsequent discussions, BLM pointed out that maps sourced through their GIS department are not precise and advised that property owners are responsible to prepare and document surveys to establish correct locations for property boundaries. As further evidence to the possibility for error, the following disclaimer appears when accessing BLM GIS information online:

*"The geographic coordinates and their associated products are NOT legal land survey records. These coordinates can NOT be used as a substitute for a legal land survey."*

Following our discussions with BLM, NMCC completed a boundary plat of NMCC property in the aforementioned area. The field work and plat were prepared according to New Mexico Land Survey Statutes and BLM standards, which require physical location and tagging of section and

**Private Land Status of Open Pit Lake**  
**New Mexico Copper Corporation**  
**January 25, 2018**

property corners by a registered land surveyor. The plat was sealed by a New Mexico registered land surveyor, recorded with Sierra County, and submitted to the BLM Cadastral Survey Department. A copy of the sealed and recorded plat was provided to your office with my September 2017 letter. The plat shows the outline of the current pit water body and shows the existing water body is fully confined to private property owned by NMCC. Figure 1 attached to this letter is page 1 of the boundary plat with NMCC property and the existing pit lake highlighted for clarity.

Figure 2 was developed by Mr. Stephen Beyerlein, a land surveyor in BLM's cadastral survey office in Santa Fe. To create this figure, Mr. Beyerlein overlaid the NMCC property plat onto BLM property maps that are maintained by the BLM cadastral survey department. This figure confirms that the private property boundaries mapped by NMCC align with BLM survey information. Mr. Beyerlein states in his October 12 email (provided with figure 2) that "this [overlay] should be evidence for the State or anyone else that you are in the proper location." On December 12, Mr. Beyerlain further clarified that regarding this figure "it was our (i.e. BLM's) intention to show that the claims are on private land."

Figure 3 shows the future pit design described in our Mine Reclamation and Closure Plan with NMCC property lines. This figure clearly shows the proposed future pit lake is fully contained within the boundaries of private land owned by NMCC.

NMCC believes that this additional information provides further confirmation to demonstrate that the future NMCC pit lake will be located on private land and that the BLM concurs with this conclusion. Please do not hesitate to contact me should you require any additional information.

Sincerely,



Jeff Smith  
Chief Operating Officer  
New Mexico Copper Corporation

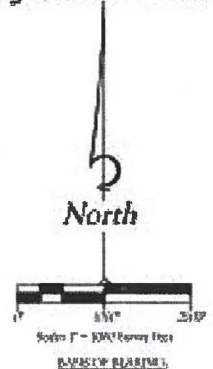
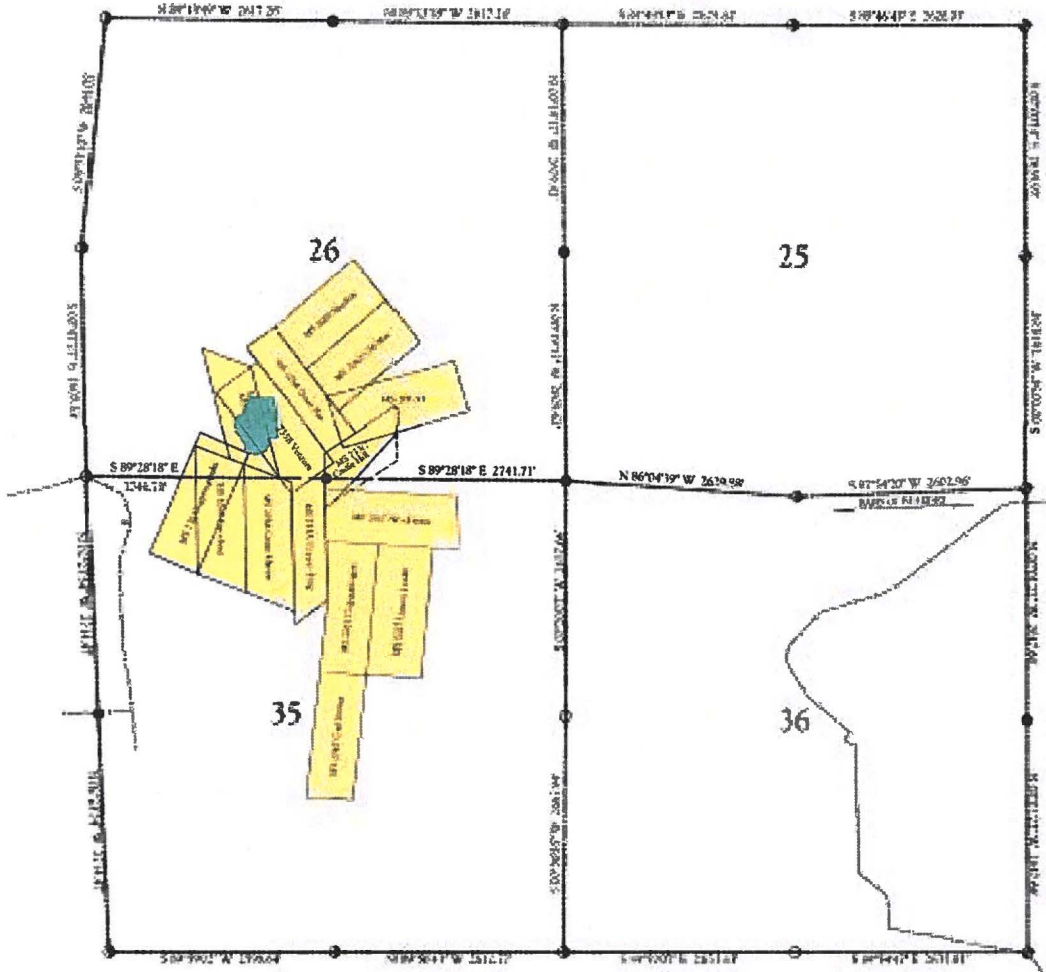
cc: Leighandra Keeven, BLM Geologist  
David Henney, Solv, LLC, Project Manager  
Brad Reid, NMED Ground Water Bureau Permit Lead  
David Ennis, MMD Permit Lead

**Figure 1. NMCC Boundary Plat**  
w/ NMCC Property and Existing Pit Lake Highlighted

# BOUNDARY SURVEY PLAT

Sections 25, 26, 35, 36 and Mineral Surveys 733A Copper King, 733B Ventura, 733C Castle Hill, 736 Copperopolis  
806 83, 2066 Craze Martin, Sternberg, Allhutzen, 2067 Stockholm, Coppenhagen, Union Leader, Carl Sextus, 2068  
Grass Flat, Shadow and Old man, Township 15 South, Range 7 West, of the New Mexico Principal Meridian  
Sierra County, New Mexico

Book 127  
Page 4193



The South line of the SW 1/4 of sections 25, 26, 35, & 36, N.M.P.M. 733A, 733B, 733C, 736, 806 83, 2066 Craze Martin, Sternberg, Allhutzen, 2067 Stockholm, Coppenhagen, Union Leader, Carl Sextus, 2068 Grass Flat, Shadow and Old man, Township 15 South, Range 7 West, of the New Mexico Principal Meridian, Sierra County, New Mexico, is a measured line and is shown as such on this plat.

- LEGEND**
- Found 1/2\"/>

**GENERAL NOTE**

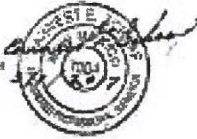
All lines shown herein are for graphical purposes only. This plat may be used as evidence in any court of law.

This survey is a preliminary survey and does not constitute a final plat. It is subject to change and should not be used as a basis for any legal action.

Note:  
The Mineral Surveys and original plat is shown as filed. This plat is the reproduction of the original plat as filed by the surveyor and is not a copy of the original plat.

**STATEMENT OF SURVEYOR**

I, Bruce E. Schind, New Mexico Professional Land Surveyor No. 1094, do hereby certify that this BOUNDARY SURVEY PLAT and the actual survey on the ground are a true and correct copy of the original plat as filed with me and that I am responsible for the survey and the accuracy of the bearings and distances shown thereon.

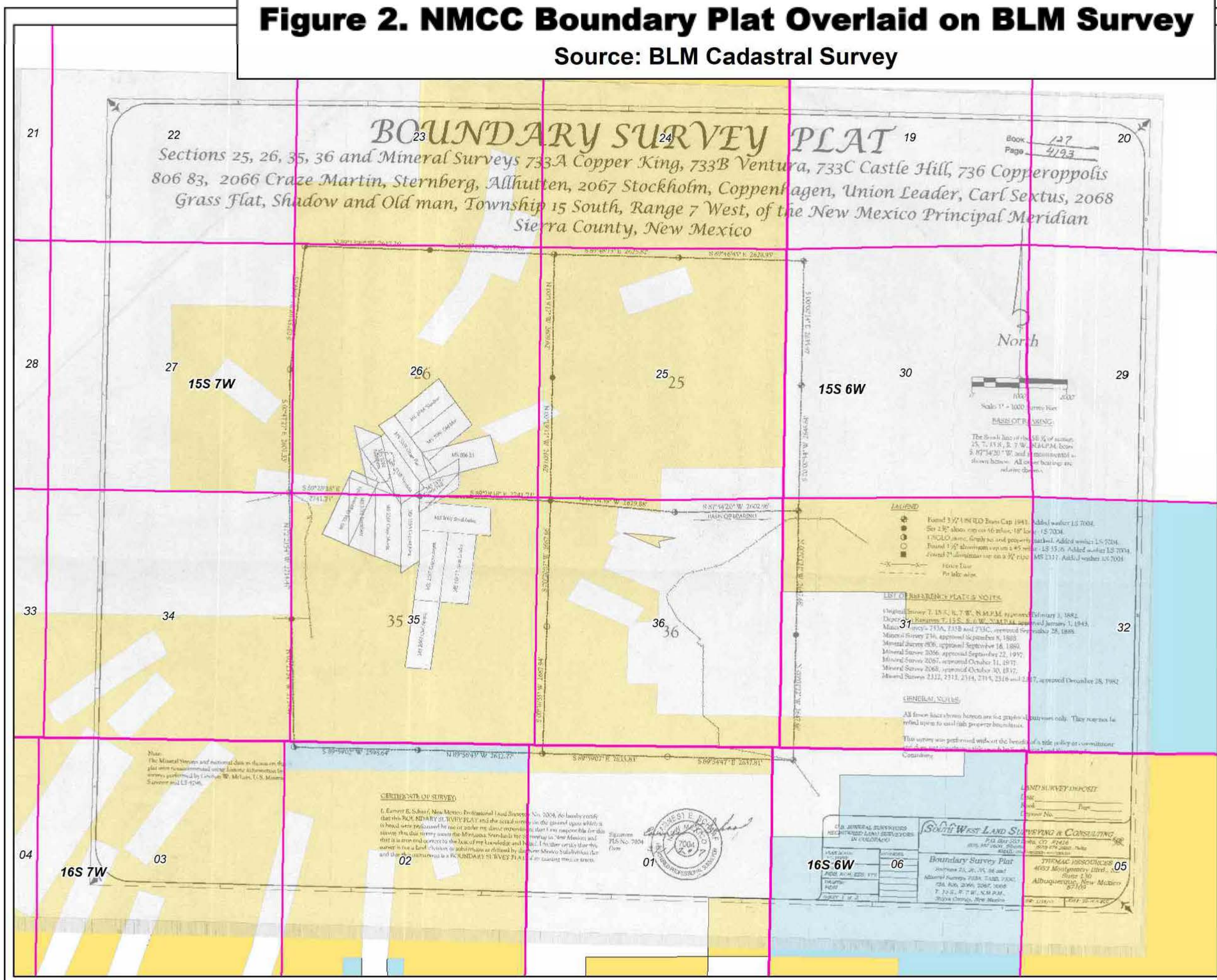


1/4 SECTION SURVEYED ACCORDING TO PLAT IS CORRECTED	<b>BOUNDARY SURVEY PLAT</b> Sections 25, 26, 35, 36 and Mineral Surveys 733A, 733B, 733C, 736, 806 83, 2066 Craze Martin, Sternberg, Allhutzen, 2067 Stockholm, Coppenhagen, Union Leader, Carl Sextus, 2068 Grass Flat, Shadow and Old man, Township 15 South, Range 7 West, of the New Mexico Principal Meridian, Sierra County, New Mexico.	<b>PROPERTY RESOURCES</b> 4053 Mineral Survey Plat, No. 1094 State of New Mexico Albuquerque, New Mexico 87102
---	---	--



**Figure 2. NMCC Boundary Plat Overlaid on BLM Survey**  
 Source: BLM Cadastral Survey

Portion of  
 Township 15 South - Range 7 West  
 Showing Scanned/GeoReferenced  
 Boundary Survey Plat of  
 Sections 25, 26, 35 and 36  
 and Mineral Surveys



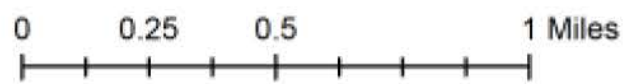
**Legend**

**Public Land Survey System**

- Township Line
- Section Line

**Surface Ownership**

- Bureau of Land Management
- Private
- State



No warranty is made by the Bureau of Land Management as to the accuracy, reliability, or completeness of these data for individual use or aggregate use with other data, or for purposes not intended by BLM. Spatial information may not meet National Map Accuracy Standards. This information may be updated without notification.

## Jeffrey Smith

---

**From:** Beyerlein, Stephen <[sbeyerle@blm.gov](mailto:sbeyerle@blm.gov)>  
**Sent:** Tuesday, December 12, 2017 7:46 AM  
**To:** Jeffrey Smith  
**Subject:** Re: Copper Flat Boundary Survey Plat

Good morning Jeff. To clarify, it was our intention to show that the claims were made on BLM land and are now private. Many are surrounded by BLM and are not in conflict with the State lands. Hoop this helps.

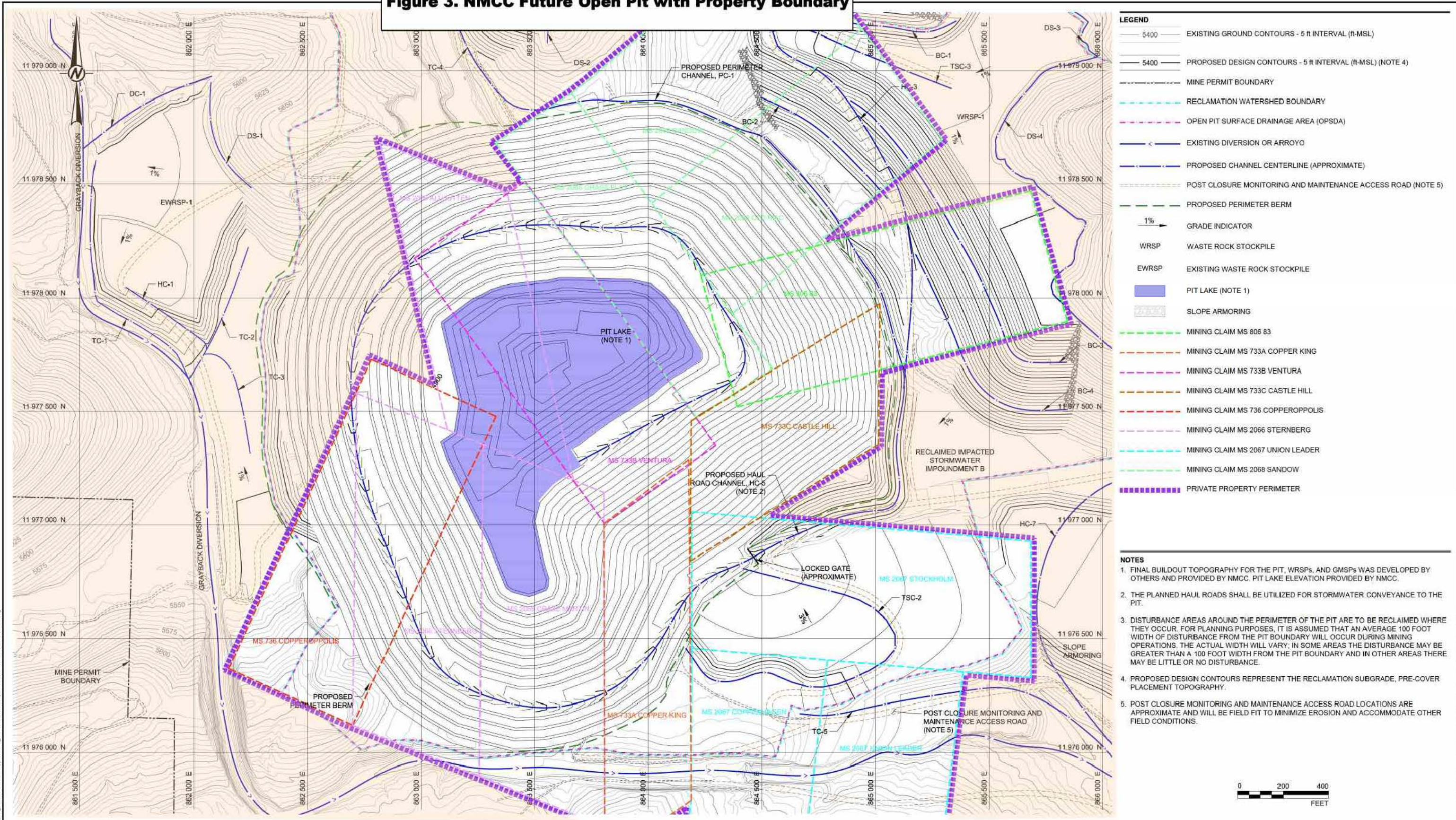
Steve

**From:** Beyerlein, Stephen [<mailto:sbeyerle@blm.gov>]  
**Sent:** Thursday, October 12, 2017 12:05 PM  
**To:** Jeffrey Smith  
**Subject:** Re: Copper Flat Boundary Survey Plat

Jeff. Attached is our GIS overlay showing our survey, your survey and the land status. It shows your claims are all on BLM lands. This should be evidence for the State or anyone else that you are in the proper location. Let us know if we can be of further help.

Steve

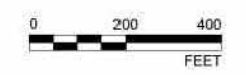
**Figure 3. NMCC Future Open Pit with Property Boundary**



**LEGEND**

- 5400 EXISTING GROUND CONTOURS - 5 ft INTERVAL (ft-MSL)
- 5400 PROPOSED DESIGN CONTOURS - 5 ft INTERVAL (ft-MSL) (NOTE 4)
- MINE PERMIT BOUNDARY
- RECLAMATION WATERSHED BOUNDARY
- OPEN PIT SURFACE DRAINAGE AREA (OPSDA)
- EXISTING DIVERSION OR ARROYO
- PROPOSED CHANNEL CENTERLINE (APPROXIMATE)
- POST CLOSURE MONITORING AND MAINTENANCE ACCESS ROAD (NOTE 5)
- PROPOSED PERIMETER BERM
- 1% GRADE INDICATOR
- WRSP WASTE ROCK STOCKPILE
- EWRSW EXISTING WASTE ROCK STOCKPILE
- PIT LAKE (NOTE 1)
- SLOPE ARMORING
- MINING CLAIM MS 806 B3
- MINING CLAIM MS 733A COPPER KING
- MINING CLAIM MS 733B VENTURA
- MINING CLAIM MS 733C CASTLE HILL
- MINING CLAIM MS 736 COPPEROPPOLIS
- MINING CLAIM MS 2066 STERNBERG
- MINING CLAIM MS 2067 UNION LEADER
- MINING CLAIM MS 2068 SANDOW
- PRIVATE PROPERTY PERIMETER

- NOTES**
1. FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NMCC. PIT LAKE ELEVATION PROVIDED BY NMCC.
  2. THE PLANNED HAUL ROADS SHALL BE UTILIZED FOR STORMWATER CONVEYANCE TO THE PIT.
  3. DISTURBANCE AREAS AROUND THE PERIMETER OF THE PIT ARE TO BE RECLAIMED WHERE THEY OCCUR. FOR PLANNING PURPOSES, IT IS ASSUMED THAT AN AVERAGE 100 FOOT WIDTH OF DISTURBANCE FROM THE PIT BOUNDARY WILL OCCUR DURING MINING OPERATIONS. THE ACTUAL WIDTH WILL VARY; IN SOME AREAS THE DISTURBANCE MAY BE GREATER THAN A 100 FOOT WIDTH FROM THE PIT BOUNDARY AND IN OTHER AREAS THERE MAY BE LITTLE OR NO DISTURBANCE.
  4. PROPOSED DESIGN CONTOURS REPRESENT THE RECLAMATION SUBGRADE, PRE-COVER PLACEMENT TOPOGRAPHY.
  5. POST CLOSURE MONITORING AND MAINTENANCE ACCESS ROAD LOCATIONS ARE APPROXIMATE AND WILL BE FIELD FIT TO MINIMIZE EROSION AND ACCOMMODATE OTHER FIELD CONDITIONS.



SEAL	CLIENT	<p><b>PRELIMINARY</b> FOR AGENCY REVIEW</p>	
	CONSULTANT	<p><b>THEMAC</b> RESOURCES NEW MEXICO COPPER CORPORATION Environmentally Responsible. Community-Minded. Local Opportunities.</p>	
		<p>TUCSON OFFICE 4730 N. ORACLE ROAD, SUITE 210 TUCSON, ARIZONA UNITED STATES OF AMERICA (+1) (520) 888 8818 www.golder.com</p>	
		<p><b>Golder Associates</b></p>	
		<p>PROJECT: COPPER FLAT PROJECT MINE RECLAMATION AND CLOSURE PERMIT</p>	
		<p>TITLE: NMCC PRIVATE PROPERTY IN MINE PIT AREA (PATENTED MINING CLAIMS)</p>	
		PROJECT NO. 1531453	REV. 1 of 1 A
		FIGURE 1	
A	2017-12-07	ISSUED FOR CLIENT REVIEW	HNL HNL TS TS
REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED PREPARED REVIEWED APPROVED

Path: \\nas01cvs01\NMCC\Copper Flat\03\_TheMAC\Reclam\Supp\0300\_Reclam\Permit\1531453\MCC\MCC1.dwg  
 File Name: 1531453\MCC\MCC1.dwg

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN INCORPORATED FROM ANSD





NEW MEXICO ENVIRONMENT DEPARTMENT  
GROUND WATER QUALITY BUREAU



GROUND WATER DISCHARGE PERMIT  
APPLICATION

Instructions for completing the application are included in the form itself and in the Supplemental Instructions found at the back of the application. You may fill out the application manually, or a Microsoft Word version may be downloaded from [www.env.nm.gov](http://www.env.nm.gov) (Ground Water Quality) and filled out electronically. Timely processing of this application is contingent upon the technical completeness of the submission. Failure to provide all of the information pursuant to Section 20.6.2.3106 NMAC, following notice of technical deficiency, may result in denial of the application.

**Send two complete paper copies AND one electronic copy of this application, with the filing fee to:**

Program Manager  
Ground Water Pollution Prevention Section  
New Mexico Environment Department  
P.O. Box 5469  
Santa Fe, NM 87502

**Introduction**

Facility Name: Copper Flat Mine

**For Existing Discharge Permits:**

DP Number: DP-01

Expiration Date: N/A

**Type of Discharge (check one):**

- Domestic
- Industrial
- Agricultural
- Mining

<b><u>GWQB – Date of Receipt</u></b> (Department use only)
---

**Type of Application (check appropriate box)**

- New – new facility
- New – existing (unpermitted) facility
- Renewal only
- Modification only  
*“modification” includes a change in the location of a discharge, and/or increase in the quantity of the discharge, and/or a change in the quality of the discharge.*
- Renewal and Modification

Part I only  
per NMED Instruction  
Pages 1 through 9

If this application is to *modify* or *renew and modify* a Discharge Permit, what is the reason for modification of the Discharge Permit? Describe the proposed changes that would result in modification, meaning a change in the location of a discharge, and/or an increase in the quantity of the discharge, and/or a change in the quality of the discharge.

The reason for modification of the Discharge permit is that NMCC proposes to recommence operation of the Copper Flat mine. The mine has not operated since 1982 and all of the mine facility will be reconstructed per NMCC's application. The proposed changes are described in detail in the attached application. Briefly, NMCC proposes to rebuild all of the processing facilities, recommence the mining operation and construct a new tailings disposal facility and waste rock stockpiles. While the location of the discharges is generally the same, the quantity of the discharge will change

**Fees Included with Application**

All applicants are required to submit a **\$100 Application Filing Fee**. An additional fee will be assessed prior to permit issuance. Permit fees are listed in section 20.6.2.3114 NMAC. **Make checks payable to: NMED-Ground Water Quality Bureau**

**Application Checklist**

The following checklist has been provided to assist in ensuring that the application is complete prior to submission (*check all that apply*):

<input type="checkbox"/>	<p>Part I. Administrative Completeness</p> <ul style="list-style-type: none"> <li><input type="checkbox"/> \$100 Application Filing Fee \$1,000 fee per Copper Rule</li> <li><input checked="" type="checkbox"/> A. General Information</li> <li><input checked="" type="checkbox"/> B. Public Notice Information</li> <li><input checked="" type="checkbox"/> C. Public Notice Preparation</li> </ul>
<input checked="" type="checkbox"/>	<p>Part II. Technical Completeness</p> <ul style="list-style-type: none"> <li><input checked="" type="checkbox"/> A. Discharge Volume and Description</li> <li><input checked="" type="checkbox"/> B. Identification and Physical Description of Facility</li> <li><input checked="" type="checkbox"/> C. Flow Metering</li> <li><input checked="" type="checkbox"/> D. Ground Water Monitoring</li> <li><input checked="" type="checkbox"/> E. Engineering and Surveying (electronic copies)</li> <li><input type="checkbox"/> F. Land Application Area</li> </ul>
<input checked="" type="checkbox"/>	<p>Part III. Site-Specific Proposals</p>
<input checked="" type="checkbox"/>	<p>Part IV. Electronic (PDF) format of Maps and Logs is required (additional paper copies of maps and logs are optional and may be requested by the Department if required for review)</p> <ul style="list-style-type: none"> <li><input checked="" type="checkbox"/> A. Surface Soil Survey and Vadose Zone Geology</li> <li><input checked="" type="checkbox"/> B. Location Map</li> <li><input checked="" type="checkbox"/> C. Flood Zone Map</li> </ul>

**Copies of Application**

An applicant applying for a Discharge Permit shall submit **two paper copies of the signed application, and an electronic copy of the signed application including all supporting documentation**, to the address listed below.

- Two paper copies – completed and signed
- Electronic copy in portable document format (PDF) of the signed application and all supporting documentation (designs, maps, logs), on the following media (*choose one*):
  - Compact disc (CD)/DVD
  - Flash drive

**Send application and fees to the following address:**

Program Manager  
Ground Water Pollution Prevention Section  
New Mexico Environment Department  
P.O. Box 5469  
Santa Fe, NM 87502

**Applicant's Signature**

Signature must be that of the person listed as the legally responsible party on this application (Part I, 2a).

*I, the applicant, attest under penalty of law to the truth of the information and supporting documentation contained in this application for a Ground Water Discharge Permit.*

Signature:	<u>Jeff Smith</u>	Date:	<u>12/9/2015</u>
Printed Name:	<u>Jeff Smith</u>	Title:	<u>COO</u>

# Part I. Administrative Completeness

## General Information

### 1. Facility Information

See Supplemental Instructions to determine what constitutes a “facility.” The physical address must be provided. If the facility does not have an address, the location can be described by road intersections, mile posts, or landmarks, as appropriate. See Supplemental Instructions for additional information.

Facility Name	New Mexico Copper Corporation Copper Flat Mine
Discharge Permit #	DP-01
Physical Address	85 Copper Rock Rd., Hillsboro NM 88042
County	Sierra
Type of Facility	Open pit copper mine and concentrate production facility
Driving Directions	South from Albuquerque on Interstate 25 165 miles to Exit 63 onto to NM State Highway 152 toward Hillsboro, west on NM 152 for 10.3 miles, turn right on Gold Dust Road (marked with a Copper Flat Project sign) to enter the mine site. Follow signs to the mine project office.

### 2. Contact Information

**a) Applicant Information** The applicant is the person or entity (e.g., corporation, partnership, organization, *municipality*, etc.) legally responsible for the discharge and for complying with the terms of the Discharge Permit. If the applicant is an entity, then the name and title of a contact person must be provided. This application must be signed by the applicant or contact person named here.

Applicant Name	Jeff Smith	Title	Chief Operating Officer
Mailing Address	4253 Montgomery Blvd. NE		
	City	Albuquerque	State NM Zip 87109
Contact Person	Jeff Smith	Title	Chief Operating Officer
	Office Number	505-382-5770	Fax Number
Contact Information	Cell Number	520-991-4588	E-mail jsmith@themacresourcegroup.com

**b) Facility Operator/Manager Information** Provide the contact information for the facility operator or manager below. If the facility is required to have an operator certified by the State of New Mexico, please include the certification level of the operator named here.

Name	Same as above	Title
Mailing Address		
	City	State Zip
Contact Information	Office Number	Fax Number
	Cell Number	E-mail



Cell Number \_\_\_\_\_ E-mail \_\_\_\_\_

Certification Level \_\_\_\_\_  
(if applicable)

**c) Consultant's Information (if applicable)** If the consultant is a company or organization, then the name and title of a contact person must be provided here.

Company Name (1) Velasquez Environmental Management Services, Inc.

Company Contact Juan R. Velasquez

Mailing Address 12912 Sand Cherry Pl. NE

City Albuquerque State NM Zip 87111

Contact Information Office Number 505-239-3728 Fax Number \_\_\_\_\_

Cell Number 505-239-3728 E-mail jvelasquez@vemsinc.com

Company Name (2) \_\_\_\_\_

Company Contact \_\_\_\_\_

Mailing Address \_\_\_\_\_

City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Contact Information Office Number \_\_\_\_\_ Fax Number \_\_\_\_\_

Cell Number \_\_\_\_\_ E-mail \_\_\_\_\_

**d) Permit Contact Information (if applicable)** If someone other than the contacts listed above is a primary contact for this application and/or facility, list here.

Name Same as Applicant Title \_\_\_\_\_

Mailing Address \_\_\_\_\_

City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Contact Information Office Number \_\_\_\_\_ Fax Number \_\_\_\_\_

Cell Number \_\_\_\_\_ E-mail \_\_\_\_\_

Facility Affiliation \_\_\_\_\_

**3. Ownership and Real Property Agreements** [20.6.2.7HH NMAC]

The applicant owns (check as appropriate):

- The facility
- All discharge sites
- Some discharge sites

If someone other than the applicant owns the facility or any of the discharge sites, provide ownership information below. For any portion of the facility where the applicant is not the owner of record, the applicant shall submit a copy of any lease agreement or other agreement which authorizes the use of the

real property for the duration of the term of the requested permit (typically five years). Lease prices or other prices may be redacted.

- If more than one person has ownership interest, or a partnership exists, list all persons with an ownership interest.
- If a corporate entity holds an ownership interest, provide the name of the corporate entity and the entity's registered agent as filed with the New Mexico Public Regulation Commission.

Name \_\_\_\_\_ See Information provided in \_\_\_\_\_ Title \_\_\_\_\_  
 Application Section 20.6.7.11.C  
 Ownership of the Facility is mixed.  
 Some land is fee land owned by the  
 applicant and other land is either  
 patented land and/or unpatented land  
 owned by the federal government and  
 administered by the US BLM

Mailing Address \_\_\_\_\_  
 City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Contact Office Number \_\_\_\_\_ Fax Number \_\_\_\_\_  
 Information Cell Number \_\_\_\_\_ E-mail \_\_\_\_\_

Owns  The facility  A discharge site  
 Attached – lease (or other authorized use) agreement

Name \_\_\_\_\_ Title \_\_\_\_\_

Mailing Address \_\_\_\_\_  
 City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Contact Office Number \_\_\_\_\_ Fax Number \_\_\_\_\_  
 Information Cell Number \_\_\_\_\_ E-mail \_\_\_\_\_

Owns  The facility  A discharge site  
 Attached – lease (or other authorized use) agreement

**4. Public Notice Information**

**a) Proposed Maximum Daily Discharge Volume:** 25,264,000 gallons per day

*Note: Use the information from Part II.A.2 following its completion.*

**b) Depth-to-Most-Shallow Ground Water:** 42 feet

*Note: Use the information from Part II.A.2 following its completion.*

**c) Pre-Discharge Total Dissolved Solids Concentration in Ground Water**

[Subsection C of 20.6.2.3106 NMAC]

Provide the concentration of total dissolved solids (TDS) in ground water prior to discharging from the facility. *Note: This information is likely the same as that submitted in the first application for a Discharge Permit for this facility.*

- Pre-discharge TDS concentration in ground water: 500-800 (See Section.6.7.11.G of application) mg/L (ppm)
  - Attached – Copy of laboratory analysis report (if available)
- From what source was the sample collected (e.g., upgradient monitoring well, on-site supply well, nearest well within a one-mile radius of the facility)?  
See Section 6.7.11.G of application

**5. Facility Location**

In the table below, describe the location for the entire facility by listing the Township, Range, and Section, and/or latitude and longitude for the locations of all components of the processing, treatment, storage, and/or disposal system. See Supplemental Instructions for additional information. [Paragraph (2) and (5) of Subsection C of 20.6.2.3106 NMAC]

Component <sup>1</sup> ID	Township	Range	Section(s)	Latitude	Longitude
See Section 20.6.7.11.E of application					

<sup>1</sup> Components include: septic tanks, impoundments, treatment systems, irrigation sites, leachfields, monitoring wells, mine stockpiles, etc. Additional examples are listed in the Supplemental Instructions. Each component should have a unique ID, for example septic tank-1, monitoring well-3, etc.

**6. Processing, Treatment, Storage, and Disposal System**

Briefly describe how wastewater, sludge, etc. is processed, treated, stored, and/or disposed of at your facility. Include each component listed in the table above.

Process water, storm water, tailings will be stored and/or disposed of in lined impoundments as described in the attached application

**7. Public Notice Preparation** [20.6.2.3108 NMAC]

Once NMED has determined that your application is administratively complete, you must complete the applicant's public notice requirements of Section 20.6.2.3108 NMAC. Language for notifications will be mailed to you with an administratively complete determination. Note: Guidance and instructions for completion of applicant's public notice can also be found at the following link: <https://www.env.nm.gov/gwb/NMED-GWQB-PublicNotice.htm>. The information requested below will be used by NMED to approve or reject the proposed public notice newspaper and signage posting locations in accordance with Subsection A of 20.6.2.3108 NMAC. Note: Other requirements of Section 20.6.2.3108 NMAC not listed here, such as certified mailings to nearby landowners, may also apply.

**a) Public Notice Posting Locations**

Select the type of application you are submitting and provide the requested information. Language to be used in the required notifications will be included in the administratively complete packet.

Renewal Application

1. Following receipt of an administrative completeness determination from NMED, the applicant is required to provide public notice of this application by placing a 2 inch by 3 inch display ad (classified or legal sections are not acceptable) in a newspaper of general circulation in the location of the proposed discharge. Indicate the newspaper in which you intend to place the ad. [Subsection C of 20.6.2.3108 NMAC]

Newspaper: \_\_\_\_\_

New Application, Modification Application, or Renewal with Modification Application

1. Following receipt of an administrative completeness determination from NMED, the applicant is required to provide public notice of this application by placing a display ad (classified or legal sections are not acceptable) in a newspaper of general circulation in the location of the proposed discharge. Indicate the newspaper in which you intend to place the ad. [Paragraph (4) of Subsection B of 20.6.2.3108 NMAC]

Newspaper: Sierra County Sentinel

2. Following receipt of an administrative completeness determination from NMED, the applicant is required to post a sign(s) (2 feet x 3 feet in size) for 30 days in a location conspicuous to the public at or near the facility. One sign must be posted for each 640 contiguous acres or less. NMED may require additional postings for facilities of more than 640 acres or when the discharge site(s) is not located on contiguous properties. Indicate the location(s) where you intend to display the sign(s). [Paragraph (1) of Subsection B of 20.6.2.3108 NMAC]

*Note: Conspicuous location means a location where the sign is visible and legible to the public and the public has access (e.g., at facility entrance on public road).*

- o Is the entire facility (including all components and discharge sites) contained within **less than 640 acres**, and is the acreage contiguous?

- Yes - Indicate a sign location below.
- No – Indicate **two** sign locations below.

Sign Location(s): To be placed at the road pull-out on the southeast side of NM Highway 152 near Geronimo Trail Scenic Highway sign and another at a location to be determined, if necessary.

- 3. Following receipt of an administrative completeness determination from NMED, the applicant is required to post an additional notice (a flyer 8.5” X 11” or larger) for 30 days at an off-site location conspicuous to the public (e.g., public library). Indicate the location where you intend to display the flyer. [Paragraph (1) of Subsection B of 20.6.2.3108 NMAC]

*Note: The U.S. Postal Service no longer allows the posting of flyers in post offices.*

Flyer Location: Hillsboro Community Center

**b) Mailing Instructions**

a) The administrative completeness determination letter, including public notice instructions, should be sent to:

- Applicant
- Consultant

**Copper Flat Mine  
Discharge Permit Application  
Pursuant to  
20.6.7 NMAC**

**Prepared for:  
New Mexico Environment Department  
Ground Water Quality Bureau**

**And**

**New Mexico Copper Corporation**



**Prepared by:**



**Velasquez Environmental Management Services, Inc.**

**December 2015**

**Revision 1  
August 2017**



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2. Appendix B, Impoundment Design Report, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corp. November, 2015
3. Appendix C, Process Facility Containment Report, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corp., November, 2015
4. Appendix D, Site Diversion Analysis, Copper Flat Project, M3-PN120085 Revision 0, M3 Engineering & Technology Corporation, November, 2015, Revised June 2016
5. Appendix E, Water Quality Monitoring Plan for the Copper Flat Mine Discharge Permit Pursuant to 20.6.7.11R and 20.6.7.28 NMAC, John Shomaker Associates, Inc., November, 2015, Revised June 2016



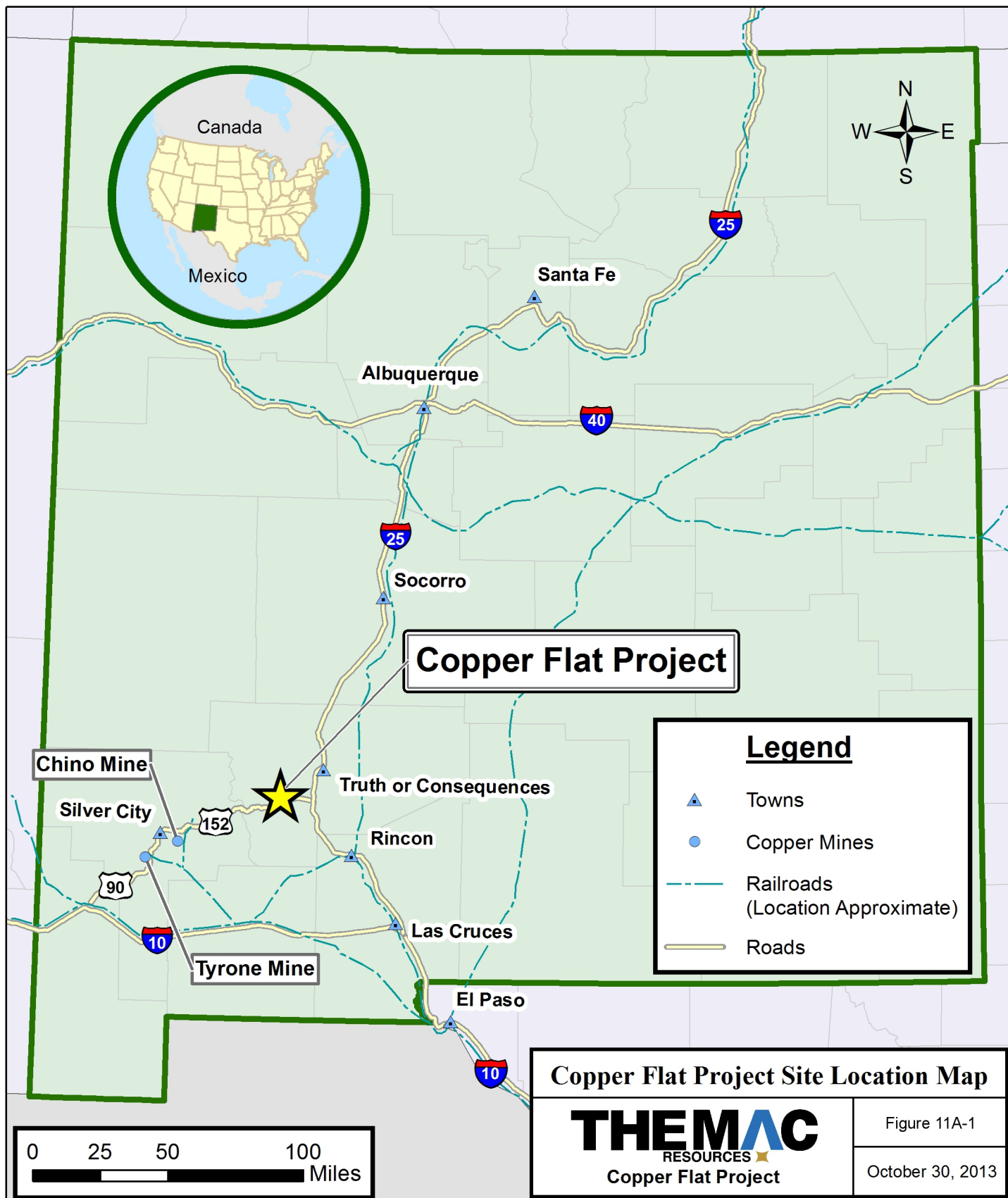
## 20.6.7.11.A INTRODUCTION

*An application for a new discharge permit or a renewal of an existing discharge permit shall include the applicable information in this section. An application for a modification of an existing discharge permit shall include the information in this section relevant to the proposed modification but need not include information listed in this section if the information was submitted to the department in the prior discharge permit application and the information has not changed since the discharge permit was issued. The department may require separate operational and closure discharge permits, or may combine operational and closure requirements in the same permit.*

New Mexico Copper Corporation (NMCC) is developing the Copper Flat Mine located approximately 150 miles south of Albuquerque, New Mexico and 20 miles southwest of Truth or Consequences, NM, north of NM state highway 152 between the communities of Caballo to the east and Hillsboro to the west in Sierra County, as shown on Figure 11A-1.

This Discharge Permit application provides the specific information required by Section **20.6.7.11 NMAC** of the New Mexico Ground Water Protection Regulations and is organized in a manner that presents all pertinent sections of the regulations followed by a discussion of the manner in which NMCC proposes to comply with the regulation. NMED has determined that this Discharge Permit Application for the Copper Flat project is an application for a new Discharge Plan and has assigned it the identification no. of DP-1840. NMCC anticipates that approval of this Discharge Plan will address the issues pending with the NMED regarding abatement of certain existing site conditions that occurred as a result of previous operation of this facility. Further, NMCC recognizes its obligations to address abatement and closure of the site in accordance with the NMED ground water regulations regardless of the outcome of this application. It is NMCC's desire to proceed with future operation of the Copper Flat mine in a manner proposed herein while incorporating mitigation procedures that will ultimately address existing conditions at the site.

NMCC proposes to mine approximately 125 million tons of copper ore including low-grade ore. Over the life of the mine it will produce approximately 113 million tons of ore, 33 million tons of waste rock and 12 million tons of low-grade ore, defined by NMCC as mined material containing less than 0.20 percent copper. The low-grade ore maybe processed during operations during the operating life of the mine. Low grade material will be stockpiled along with the other waste rock produced until such time it is suitable for milling and processing. Annually, the mining operation will supply approximately 11 million tons of copper ore (an average of approximately 30,000 tons per day) to the mill for processing. Waste rock production is estimated to average approximately 3 million tons per year (ranging from 0 to 7 million tons annually). The mine life for the current reserve is estimated at 11 to 12 years.



**Copper Flat Project**

**Legend**

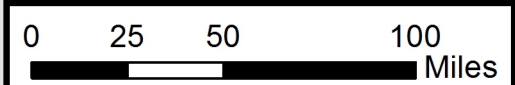
- ▲ Towns
- Copper Mines
- - - Railroads (Location Approximate)
- Roads

**Copper Flat Project Site Location Map**

**THEMAC**  
RESOURCES  
Copper Flat Project

Figure 11A-1

October 30, 2013





**20.6.7.11.B CONTACT INFORMATION**

*An application shall include:*

- (1)** Applicant's name, title and affiliation with the copper mine facility, mailing address, and telephone number.

Jeff Smith  
Chief Operating Officer  
New Mexico Copper Corporation  
4253 Montgomery Blvd. NE  
Suite 130  
Albuquerque, NM 87109  
(505) 382-5770

- (2)** The name, mailing address and telephone number of each owner and operator of the copper mine facility.

The owner and operator of the mine is New Mexico Copper Corporation. The address is that stated above.

- (3)** If different than the applicant, the application preparer's name, title and affiliation with the copper mine facility, mailing address, telephone number and signature.

Juan R. Velasquez, President of Velasquez Environmental Management Services, Inc. has prepared the application under a consulting contract with NMCC. VEMS Inc.'s address is:

12912 Sand Cherry Pl. NE  
Albuquerque, NM 87111  
505-239-3728

 Dec 9, 2015  
Juan R. Velasquez Date

- (4)** The mailing address and telephone number of any independent contractor authorized to assist the copper mine facility with compliance with the Water Quality Act and 20.6.2 NMAC and 20.6.7 NMAC; and

NMCC has not authorized any independent contractor(s) to assist with compliance with the Water Quality Act and 20.6.2 and 20.6.7.



**(5)** *If the person submitting the application is not the owner or operator of the copper mine facility, a certification that the person is duly authorized to submit the application on behalf of the owner or operator.*

NMCC is the owner of the mine and is the person submitting the application.





## 20.6.7.11.C OWNERSHIP AND REAL PROPERTY AGREEMENTS

- (1)** *An application shall include the copper mine facility owner's name, title, mailing address and phone number.*
- a)** *If more than one person has an ownership interest in the copper mine facility or a partnership exists, then the applicant shall list all persons having an ownership interest in the copper mine facility, including their names, titles, mailing addresses and telephone numbers.*
  - b)** *If any corporate entity holds an ownership interest in the copper mine facility, the applicant shall also list the name(s), as filed with the New Mexico public regulation commission, of the corporate entity, and the corporate entity's registered agent's name and address.*

The Copper Flat facility is owned by New Mexico Copper Corporation (NMCC), a wholly-owned subsidiary of THEMAC Resources Group Limited, (THEMAC) a Canadian corporation.

NMCC's mailing address and telephone number are:

4253 Montgomery Blvd. NE  
Suite 130  
Albuquerque, NM 87109  
505-382-5770

NMCC's registered agent is:

Mark Adams, Esq.  
P.O. Box 1357  
Santa Fe, NM 87504

THEMAC's mailing address and telephone number are:

700-510 West Hastings Street  
Vancouver, British Columbia, Canada V6B, 1L8  
(+1) 604-495-6723



*(2) If the applicant is not the owner of the real property upon which the copper mine facility is or will be situated, or upon which the discharge will occur, the applicant shall submit the name, address and telephone number of the owner(s), and a notarized statement from the owner which authorizes the use of the real property for the duration of the term of the requested permit. In the event the property is under federal or state ownership the applicant shall provide other evidence of authorization to enter public lands for mining.*

NMCC owns and/or controls all of the real property upon which the mine facility will be situated through a combination of fee ownership, and federal government patented and unpatented mining claims administered by the Bureau of Land Management (BLM) managed by the Las Cruces District Office of the BLM located at:

1800 Marquess Street  
Las Cruces, NM 88005-3370  
505-525-4300

The proposed mining will be conducted on unpatented lode, placer, and mill-site claims owned and controlled by NMCC. Claim names and BLM serial numbers are provided in Table 11C-1.

The Copper Flat mine will be situated within a 2190 acre parcel of property identified in the various maps and figures contained herein as the Mine Permit Area Boundary. While the NMED groundwater regulations are not structured in terms of a site boundary, Discharge Plan approval is one of several permissions that must be obtained as part of the New Mexico Mining Act permitting process and much of the information required by the Mining Act with respect to groundwater protection is contained in the Discharge Permit documentation. The mine permit requires designation of a permit area boundary. As such, the designated mine permit area boundary is incorporated into much of this document.



<b>TABLE 11C-1</b> <b>NMCC Mining Claims and Fee Lands</b>	
<b>Unpatented Mining Claims</b>	
<b>Claim Name</b>	<b>BLM Serial Number</b>
Graveyard Placer	NMMC 60021
Old Cabin Placer	NMMC 60022
Rainey Season Placer	NMMC 60027
Desert Gold Placer	NMMC 60043
Gray Back Placer	NMMC 60044
Black Sand Group 9 No. 1 Placer (amended)	NMMC 60045
Black Sand Group 10 No. 3 Placer (amended)	NMMC 60046
Surprise No. 1 Lode	NMMC 60052
Surprise No. 2 Lode	NMMC 60053
Dutch-1 Lode	NMMC 60054
Olympia (amended)	NMMC 60057
Gluck Auf	NMMC 60058
Taurus (amended)	NMMC 60059
Hercules	NMMC 60060
El Oro No. 3	NMMC 60063
Saint Louis Republic	NMMC 60069
Dolores, aka Delores	NMMC 60070
Highland No. 1	NMMC 60071
Highland No. 2	NMMC 60072
Highland No. 3	NMMC 60073
The Wellington	NMMC 60074
Three Boys No. 1 (amended)	NMMC 60080
Blue Moon (amended)	NMMC 60081
The Leone	NMMC 60082
Dolores Placer (amended)	NMMC 60083
Jones Hill Placer	NMMC 60084
Duke No. 1	NMMC 60085
Duke No. 2	NMMC 60086
Renew No. 1	NMMC 106464
Renew No. 2	NMMC 106465
M.S. #1	NMMC 60093
M.S. #2	NMMC 60094
M.S. #3	NMMC 60095



<b>TABLE 11C-1 NMCC Mining Claims and Fee Lands</b>	
M.S. #4	NMMC 60096
M.S. #5	NMMC 60097
M.S. #6	NMMC 60098
M.S. #8	NMMC 60099
M.S. #10	NMMC 60101
M.S. #11	NMMC 60102
M.S. #12 (amended)	NMMC 60103
M.S. #13 (amended)	NMMC 60104
M.S. #14	NMMC 60105
M.S. #15	NMMC 60106
M.S. #16	NMMC 60107
M.S. #17	NMMC 60108
M.S. #18	NMMC 60109
M.S. #20	NMMC 60110
M.S. #21	NMMC 60111
M.S. #22	NMMC 60112
M.S. #23	NMMC 60113
M.S. #25	NMMC 60114
M.S. #26	NMMC 60115
M.S. #29	NMMC 60118
M.S. #33	NMMC 60122
M.S. #38	NMMC 60123
M.S. #48 (amended)	NMMC 60129
M.S. #49	NMMC 60130
M.S. #53 (amended)	NMMC 60131
M.S. #102	NMMC 60138
M.S. #104 (amended)	NMMC 60139
M.S. #105	NMMC 60140
M.S. #106	NMMC 60141
M.S. #107	NMMC 60142
M.S. 222 (amended)	NMMC 60170
M.S. 223 (amended)	NMMC 60171
M.S. 224 (amended)	NMMC 60172
M.S. 225 (amended)	NMMC 60173
M.S. 228 (amended)	NMMC 60176
M.S. 264 (amended)	NMMC 60194
M.S. 282 (amended)	NMMC 60210



<b>TABLE 11C-1</b> <b>NMCC Mining Claims and Fee Lands</b>	
M.S. 288 (amended)	NMMC 60216
M.S. 289 (amended)	NMMC 60217
M.S. 290 (amended)	NMMC 60218
M.S. 291 (amended)	NMMC 60219
M.S. 292 (amended)	NMMC 60220
M.S. 293 (amended)	NMMC 60221
M.S. 316 (amended)	NMMC 60240
M.S. 320 (amended)	NMMC 60244
M.S. 322 (amended)	NMMC 60246
M.S. 329 (amended)	NMMC 60253
M.S. 330 (amended)	NMMC 60254
M.S. 331 (amended)	NMMC 60255
M.S. 337 (amended)	NMMC 60261
M.S. 338 (amended)	NMMC 60262
M.S. 339 (amended)	NMMC 60263
M.S. 340 (amended)	NMMC 60264
M.S. 341 (amended)	NMMC 60265
M.S. 342 (amended)	NMMC 60266
M.S. 345 (amended)	NMMC 60267
M.S. 346 (amended)	NMMC 60268
MS. 347 (amended)	NMMC 60269
M.S. 438	NMMC 60312
M.S. 439	NMMC 60313
M.S. 440	NMMC 60314
M.S. 441	NMMC 60315
M.S. 452	NMMC 60318
M.S. 453	NMMC 60319
M.S. 454	NMMC 60320
M.S. 455	NMMC 60321
M.S. 456	NMMC 60322
M.S. 458	NMMC 60324
M.S. 460	NMMC 60326
M.S. 461	NMMC 60327
M.S. 462	NMMC 60328
M.S. 463	NMMC 60329
M.S. 464	NMMC 60330
M.S. 465	NMMC 60331



<b>TABLE 11C-1 NMCC Mining Claims and Fee Lands</b>	
M.S. 467	NMMC 60333
M.S. 468	NMMC 60334
M.S. 469	NMMC 60335
M.S. 470	NMMC 60336
M.S. 471	NMMC 60337
M.S. 472	NMMC 60338
M.S. 473	NMMC 60339
M.S. 474	NMMC 60340
M.S. 475	NMMC 163361
M.S. 476	NMMC 163362
M.S. 477	NMMC 163363
M.S. 478	NMMC 163364
Animas #1 Placer	NMMC 60341
Animas #2 Placer	NMMC 60342
The Betsy Ross	NMMC 60344
Wicks Extension No. 1 (amended)	NMMC 60346
Anderson Extension No. 2	NMMC 60348
Crescent 101	NMMC 60349
Wicks Extension 100	NMMC 60350
Betsy Ross 101	NMMC 60351
Portland 101	NMMC 60352
Ready Pay Apex 100	NMMC 60353
Anderson Extension 101	NMMC 60354
Greer No. 2	NMMC 72821
Chatfield	NMMC 72822
Chatfield No. 3	NMMC 72823
Chatfield No. 4	NMMC 72824
Chatfield No. 5	NMMC 72825
Chatfield No. 6	NMMC 72826
Chatfield No. 9	NMMC 81353
Chatfield No. 10	NMMC 81354
Chatfield No. 25	NMMC 100695
Golden 1	NMMC 190838
Golden 2	NMMC 190839
Golden 3	NMMC 190840
Golden 4	NMMC 190841
Golden 5	NMMC 190842



<b>TABLE 11C-1 NMCC Mining Claims and Fee Lands</b>	
Golden 6	NMMC 190843
Golden 7	NMMC 190844
Golden 8	NMMC 190845
Golden 9	NMMC 191032
Golden 10	NMMC 191039
Golden 11	NMMC 191033
Golden 12	NMMC 191034
Golden 13	NMMC 191035
Golden 14	NMMC 191036
Golden 15	NMMC 191037
Golden 16	NMMC 191038
CU 1	NMMC 189246
CU 2	NMMC 189247
CU 3	NMMC 189248
CU 4	NMMC 189249
CU 5	NMMC 189250
CU 6	NMMC 189251
CU 7	NMMC 189252
CU 8	NMMC 189253
CU 9	NMMC 189254
CU 10	NMMC 189255
CU 11	NMMC 189256
CU 12	NMMC 189257
CU 13	NMMC 189258
CU 14	NMMC 189259
CU 15	NMMC 189260
CU 16	NMMC 189261
CU 17	NMMC 189262
CU 18 (amended)	NMMC 189263
CU 19 (amended)	NMMC 189264
CU 20 (amended)	NMMC 189265
CU 21 (amended)	NMMC 189266
CU 22	NMMC 189267
CU 23	NMMC 189268
CU 24	NMMC 189269
CU 25	NMMC 189270
CU 26	NMMC 189271



CU 27	NMMC 189272
CU 28	NMMC 189273
CU 29	NMMC 189274
CU 30	NMMC 189275
CU 31	NMMC 189276
CU 32	NMMC 189277
CU 33	NMMC 189278
CU 34	NMMC 189279
CU 35	NMMC 189280
CU 36	NMMC 189281
CU 37	NMMC 189282
CU 38	NMMC 189283
CU 39	NMMC 189284
CU 40	NMMC 189285
CU 41	NMMC 189286
CU 42	NMMC 189287
CU 43	NMMC 189288
CU 44	NMMC 189289
CU 45	NMMC 191058
CU 46	NMMC 191059
CU 47	NMMC 191060
CU 48	NMMC 191061
CU 49	NMMC 191062
CU 50	NMMC 191063
CU 51	NMMC 191064
CU 52	NMMC 191065
CU 53	NMMC 191066
CU 54	NMMC 191076
CU 55	NMMC 191077
CU 56	NMMC 191078
CU 57	NMMC 191079
CU 58	NMMC 191080
CU 59	NMMC 191081
CU 60	NMMC 191082
CU 61	NMMC 191083
CU 62	NMMC 191084
CU 63	NMMC 191085





<b>TABLE 11C-1 NMCC Mining Claims and Fee Lands</b>	
CU 64	NMMC 191086
CU 65	NMMC 191087
CU 66	NMMC 194880
CU 67	NMMC 194881
CU 68	NMMC 194882
CU 69	NMMC 194883
CU 70	NMMC 194884
CU 71	NMMC 194885
CU 72	NMMC 194886
CU 73	NMMC 194887
CU 74	NMMC 194888
CU 75	NMMC 194889
CU 76	NMMC 194890
CU 77	NMMC 194891
CU 78	NMMC 194892
CU 79	NMMC 194893
CU 80	NMMC 194894
CU 81	NMMC 194895
CU 82	NMMC 194896
CU 83	NMMC 194897
CU 84	NMMC 194898
CU 85	NMMC 194899
<b>End Unpatented Mining Claims</b>	



TABLE 11C-1 NMCC Mining Claims and Fee Lands	
Patented Mining Claims	
Claim Name	Mineral Survey
Feeder	M.S. 943C
Chance	M.S. 945A
Xmas	M.S. 945B
Extension	M.S. 945D
Smokey Jones	M.S. 1024
Little Jewess	M.S. 1715
Wisconsin	Lot No. 805
Copper King	Lot No. 733A
Ventura	Lot No. 733B
Castle Hill	Lot No. 733C
Copperopolis	Lot No. 736
83	Lot No. 806
Soudan	Lot No. 807
Stenberg	M.S. 2066
Allhutten	M.S. 2066
Craze Martin	M.S. 2066
Copenhagen	M.S. 2067
Carl Sextus	M.S. 2067
Union Leader	M.S. 2067
Stockholm	M.S. 2067
Grass Flat	M.S. 2068
Sadow	M.S. 2068
Old Mac	M.S. 2068
End Patented Mining Claims	



<b>TABLE 11C-1 NMCC Mining Claims and Fee Lands</b>	
<b>Fee Lands</b>	
<b>Township, Range, Section</b>	<b>Lot</b>
<b>Township 15 South, Range 7 West</b>	
Section 36	Part of Lot 1 (Parcel N)
Section 36	Part of Lot 4 (Parcel M)
Section 36	Part of Lot 6 (Parcel J)
Section 36	Lot 10 (Parcel L)
Section 36	Lot 11 (Parcel K)
Section 36	Part of N1/2SE1/4 (Parcel I)
Section 36	Part of N1/2S1/2SE1/4 (Parcel H)
<b>Township 15 South, Range 6 West</b>	
Section 31	Lot 3 (Parcel D)
Section 31	Lot 6 (Parcel G)
Section 31	Lot 7 (Parcel C)
Section 31	Part of NE1/4SW1/4 (Parcel E)
Section 31	N1/2SE1/4SW1/4 (Parcel B)
Section 31	Part of S1/2SE1/4NW1/4 (Parcel F)
Section 31	Part of SE1/4 (Parcel A)
<b>Township 16 South, Range 6 West</b>	
Section 6	Part of Lot 3 (Parcel P)
Section 6	Part of Lot 4 (Parcel O)
<b>End Fee Lands</b>	



## **20.6.7.11.D SETBACKS**

*An application for a new copper mine facility shall include a scaled map of the proposed copper mine facility layout demonstrating that the copper mine facility meets the setback requirements of 20.6.7.19 NMAC.*

### **20.6.7.19 SETBACK REQUIREMENTS FOR A COPPER MINES FACILITY APPLYING FOR A DISCHARGE PERMIT**

*20.6.7.19.E.(1) requires that leach stockpiles, waste rock stockpiles, tailings impoundments, process water impoundments or impacted stormwater impoundments shall be located;*

- (a) greater than 500 feet from a private domestic water well or spring that supplies water for human consumption; and*
- (b) greater than 1000 feet from any water well or spring that supplies water for a public water system as defined by 20.7.10 NMAC, unless a wellhead protection program established by the public water system requires a greater distance.*

*20.6.7.19.E.(4) requires that setback distances shall be measured from the toe of the outer edge of a leach stockpile, waste rock stockpile, tailing impoundment, process water impoundment or impacted stormwater impoundment as its final design build out.*

NMCC proposes to construct new waste rock stockpiles, a new tailings impoundment, new process water impoundments and impacted storm water impoundments. NMCC does not propose any leach stockpiles at the Copper Flat project. Figure 11D-1 is a scaled map of that shows the footprint of NMCC's proposed Copper Flat Mine Project Mine Permit Area Boundary, as defined by NMCC pursuant to the New Mexico Mining Act regulations for the New Mexico Mining and Minerals Division (MMD). All of the Copper Flat facilities, with the exception of the company's water supply wells and the pipeline that provides fresh water to the site, are otherwise located within the mine permit area boundary. Figure 11D-1 shows the location of private domestic water wells for human consumption and water wells that supply water for a public water system as defined by **20.7.10 NMAC**, in relation to the facility boundary. It shows that the closest private and public water supply wells are all located over three miles from the mine permit area boundary. Figure 11D-1 also shows the location of springs in the area of the project. However, none of the springs are sources of water for human consumption.

As shown on Figure 11D-1, the Smith Well, located along Percha Creek 3.5 miles southeast of NMCC's facilities, and the Chatfield Well, located along Las Animas Creek 3.8 miles northeast of NMCC's facilities, are the closest known private domestic water wells that supply water for human consumption. Figure 11D-1 also shows the location of the town of Hillsboro relative to the NMCC facilities, 3.2 miles from the mine site.

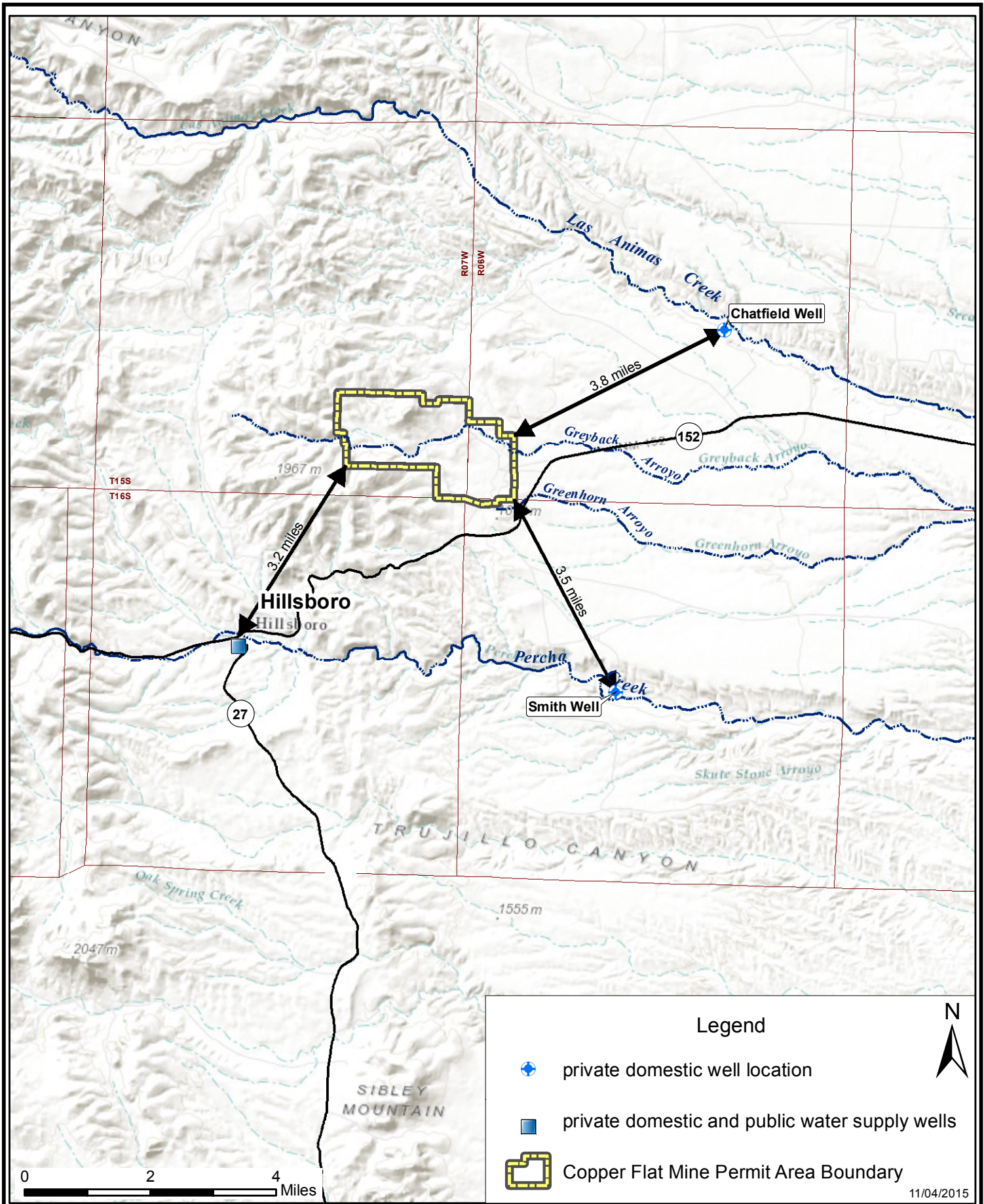


Figure 11D-1. Domestic and Public Water Supply Well Locations



There are known to be a number of private water supply wells in the vicinity of Hillsboro, some of which are assumed to provide water for human consumption, although none have been specifically identified.

Figure 11D-1 demonstrates that the proposed NMCC mine facility meets the setback requirements of **20.6.7.19.E.(1) through (4)** of the Copper Rules. All of NMCC's proposed waste rock stockpiles, its tailings impoundment, process water impoundment and impacted storm water impoundments are located within the Mine Permit Area Boundary.

NMCC has not identified any springs within 500 ft. of the facility that supply water for human consumption nor any springs within 1000 ft. of the facility that supply water for a public water supply.

With respect to **20.6.7.19.E.(1).(b)** of the Copper Rules, as shown on Figure 11D-1, that the town of Hillsboro has a water well, or group of water wells, that supply water for a public water system. The system well(s) are over 3 miles from the southwestern edge of the NMCC facility Mine Permit Area Boundary. NMCC has not identified any springs within its area of data collection that supply water for a public water system.

With respect to **20.6.7.19.E.(4)** of the Copper Rules, all of the Copper Flat facilities are located within the mine permit area boundary and the toe of the outer edge of the proposed waste rock stockpiles, tailings impoundment, process water impoundments, and impacted storm water impoundments are all within the mine permit area boundary identified by NMCC for the MMD. All of the wells subject to the setback requirements are further than 500 ft. and 1000 ft. from the boundary. Therefore, the setback requirements are met. There are, of course, a number of other wells located throughout the site as well as others in the vicinity of the site. However, all of these wells serve a purpose other than to supply water for domestic and/or public consumption. Some are water quality monitoring wells. Others are livestock wells. As such, they have not been included herein. Information on those wells can be found in documentation previously submitted to the NMED by NMCC, such as the Baseline Data Report, and others.



## **20.6.7.11.E COPPER MINE FACILITY INFORMATION AND LOCATION**

*An application shall include:*

**(1)** *the copper mine facility name, physical address and county;*

The facility name is the Copper Flat mine. Its physical address is:

85 Copper Rock Rd.  
Hillsboro, New Mexico 88042

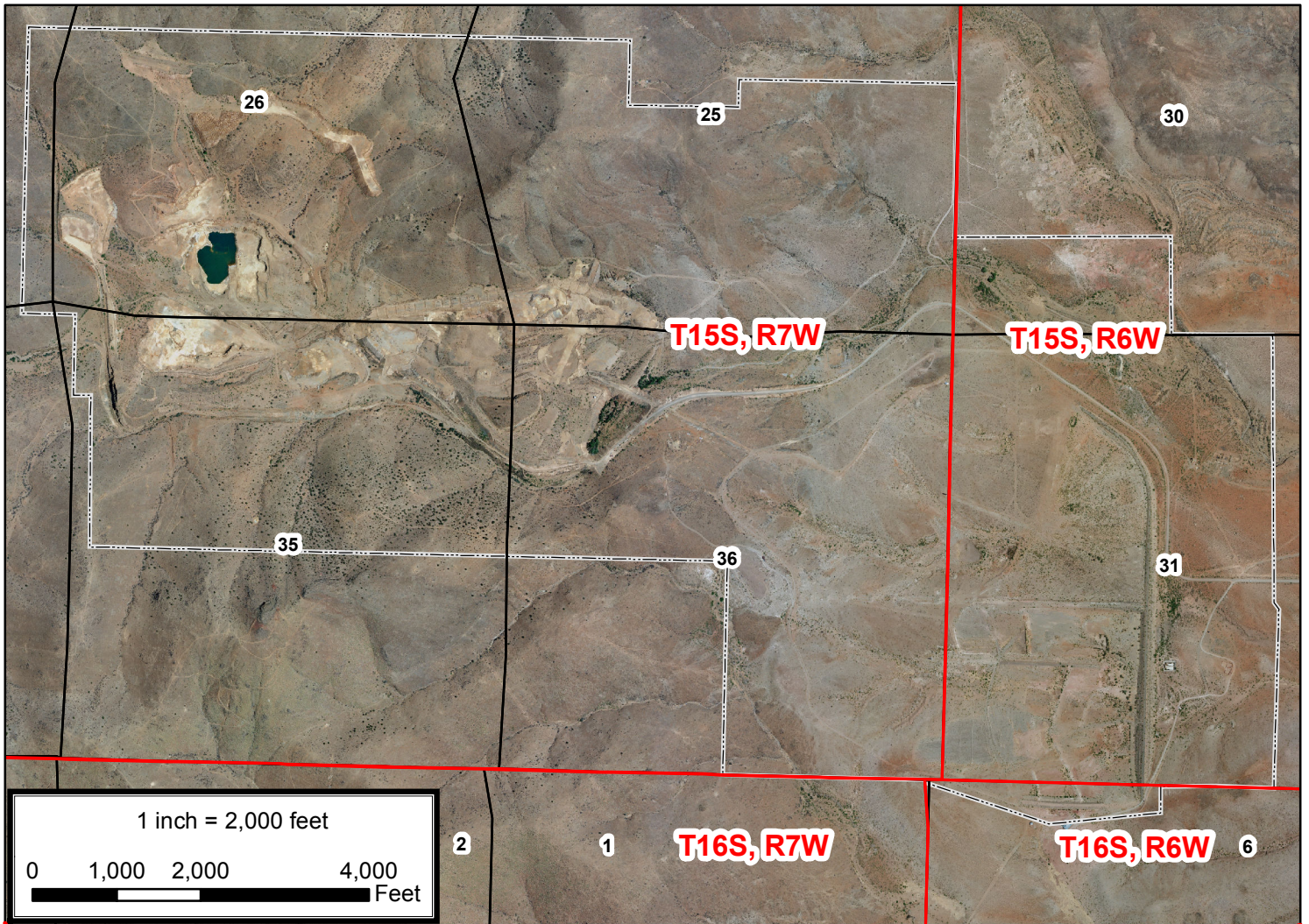
It is located north of NM state highway 152 between the communities of Caballo to the east and Hillsboro to the west in Sierra County, NM as shown on Figure 11E-1.

**(2)** *the township, range and section for the entire copper mine facility; and*

The facility is located in all or parts of Sections 30 and 31, Township 15 South, Range 6 West; Sections 25, 26, 35 and 36, Township 15 South, Range 7 West, New Mexico Principal Meridian, and Section 6, Township 16 South, Range 6 West, as shown on Figure 11E-1.

**(3)** *the total acreage of the copper mine facility.*

The total number of acres within the facility area is 2190.



Permit Boundary Area: 2,190 US Acres

Copper Flat Mine Address:  
85 Copper Rock Rd.,  
Hillsboro, Sierra County,  
New Mexico 88042



### Legend

- Populated Areas
- Interstates
- Highway
- Copper Flat Mine Permit Area Boundary
- Township Boundary
- Section Boundary
- T16S** Township
- R6W** Range
- 31** Section

<b>Copper Flat Mine Facility Information and Location</b>	
<b>THEMAC</b> RESOURCES Copper Flat Project	Figure 11E-1
	August 6, 2015





## **20.6.7.11.F PUBLIC NOTICE PREPARATION**

- (1) An application for a new, modified or renewed and modified discharge permit shall include the name of a newspaper of general circulation in the location of the copper mine facility for the display advertisement publication, the proposed public location(s) for posting of the 2-foot by 3-foot sign, and the proposed off-site public location for posting of the additional notice, as required by Subsection B of 20.6.2.3108 NMAC.*
- (2) An application for a renewed discharge permit that does not seek a discharge permit modification shall include the name of a newspaper of general circulation in the location of the copper mine facility for the future display advertisement publication as required by Subsection C of 20.6.2.3108 NMAC.*

Public notice will be provided in the Sierra County Sentinel. The required 2-foot by 3-foot sign will be placed at the road pull-out on the southeast side of New Mexico Highway 152 near the Geronimo Trail Scenic Highway sign and at a second location to be determined, if necessary. An off-site public notice flyer will be posted at the Hillsboro Community Center.



**20.6.7.11.G PRE-DISCHARGE TOTAL DISSOLVED SOLIDS CONCENTRATIONS IN GROUND WATER**

*An application shall include the pre-discharge total dissolved solids concentration, or range of concentration, from analytical results of ground water obtained from on-site test data from the aquifer(s) that may be affected by discharges from the copper mine facility. A copy of the laboratory analysis stating the pre-discharge total dissolved solids concentration shall be submitted with the application.*

NMCC has identified three aquifers and associated sub-aquifers that may be affected by discharge from the Copper Flat mine facility. They are the;

1. Quaternary Alluvial aquifer associated with Grayback Arroyo and the alluvial fan and fluvial deposits in the Santa Fe Group
2. Santa Fe Group aquifer
3. Crystalline bedrock aquifer including the andesite and quartz monzonite

The range of pre-discharge total dissolved solids (TDS) concentrations in groundwater for these aquifers at the mine site is presented below. The data was summarized from several reports prepared for Quintana Minerals Corporation and New Mexico Copper Corporation as shown in Table 11G-1. All of these reports have been previously submitted to NMED. The laboratory reports associated with this data are incorporated into the reports. The data indicates that the results are generally consistent with the preoperational results provided to NMED by Quintana.

<b>TABLE 11G-1 Source Reports Used For Pre-Discharge TDS Analysis</b>		
<b>Report Title</b>	<b>Report Prepared By</b>	<b>Date</b>
Tailings Dam and Disposal Area, Quintana Mineral Corporation, Copper Flat Project, Gold Dust, New Mexico	Sargent, Hauskins, and Beckwith (SHB 1980)	October 1980
Geohydrologic Evaluation For Submission of Discharge Plan, Copper Flat Project, Quintana Mineral Corporation, Sierra County, New Mexico	SHB (SHB 1981)	June 1981
Baseline Data Characterization Report, Copper Flat Mine, Sierra County, New Mexico	Intera et al. (Inter 2012)	June 2012
Results from the First Year of the Stage 1 Abatement Investigation at the Copper Flat Mine Site Near Hillsboro, New Mexico	John Shomaker and Associates (JSAI May 2014b)	May 2014



Table 11G-2 provides a summary of the pre-discharge TDS concentrations in groundwater for each aquifer.

TABLE 11G-2 Pre-Discharge TDS Concentrations					
Aquifer	Sub-Aquifer	Pre-Discharge Concentration (mg/l)	Wells Sampled	Sample Date (m/yr)	Well Locations
Quaternary Alluvial	Grayback Alluvial Up-gradient of Ore	317-905	GWQ11-26	4/2013-10/2013	Up-gradient of the ore body
	Grayback Alluvial Down-gradient of Ore	868-1,260	GWQ-3 <sup>1</sup> GWQ-5 <sup>2</sup>	9/1976-2/1982	Down-Gradient of the ore body
	Alluvial Fan and Fluvial deposits in the Upper Santa Fe Group	354-840	SHB-27 <sup>3</sup> SHB-28 <sup>3</sup> SHB-29 <sup>3</sup> SHB-30 <sup>3</sup> NP-5	9/1976-2/1982	In the vicinity of the current TSF
Santa Fe Group	NA	350-650	GWQ-1 GWQ-2 GWQ-7 GWQ-8 GWQ-9 GWQ-10 GWQ-11 NP-1 NP-2 NP-3	6/1976-2/1982	In the vicinity of the current TSF down-gradient of the ore body
Crystalline Bedrock	Andesite	500-798	GWQ96-22A GWQ96-22B GWQ-4	6/1981-1/2013	Up-gradient of the ore body;
	Andesite	496-920	GWQ-5R GWQ96-23A GWQ96-23B	7/1996-10/2013 <sup>6</sup>	Down-gradient of ore body
	Quartz Monzonite <sup>4,5</sup>	2,280-4,400	GWQ11-24A GWQ11-24B GWQ11-25B	1/2010-10/2013 <sup>6</sup>	

1. GWQ-3 is a 33 ft. deep concrete lined well, 40 inches by 43 inches perforated from 10 to 33 feet.
2. GWQ-5 is a 20 ft. deep rock lined well located in Grayback Arroyo downstream of the Quintana Plant Site. Well was destroyed during construction of the Quintana Mine.
3. SHB 1981 SHB (1980) indicates that water samples were collected from geotechnical borings SHB-29, 30, 31, 33, and 34 and the water samples were submitted to "Controls for Environmental Pollution, Inc." for analysis. SHB (1981) describes these as "wells" SHB-27, 28, 29, 30, and 34. The relationship between soil borings and "wells" is not documented in either report. However, using the laboratory data reports from SHB-1980 and SHB 1981, the relationship between borings and wells can be established.
4. Quartz Monzonite TDS data from Well GWQ11-25A was not included as representative of the Quartz Monzonite because JSAI (2013) describes the groundwater chemistry samples from that well as "completely different from all other samples in the pit area". This well is completed in a localized zone of sulfide mineralization and the water source is suspected to be from oxygenated water infiltrating through sulfide bearing fractures with limited storage. TDS samples from GWQ-25A range from 11,300 to 27,700 mg/L.
5. Quartz Monzonite hosts the ore body
6. No pre-Quintana mining data exists

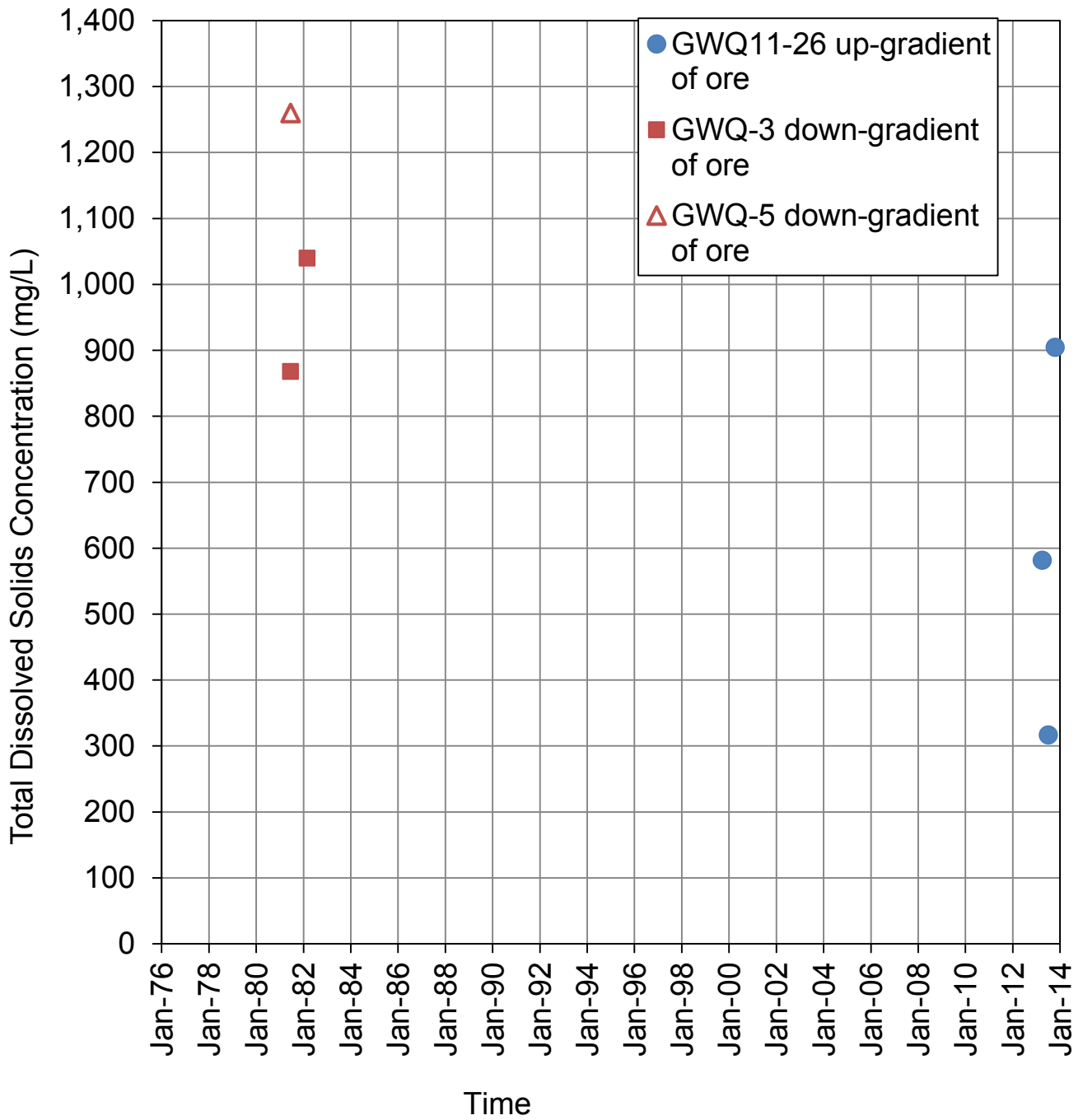


Figures 11G-1 through 4 are scatter-plot diagrams of the data represented in Table 11G-2. Figure 11G-1 shows the pre-discharge TDS concentrations in the Grayback Alluvial Aquifer up gradient and down-gradient of the Copper Flat ore body. Even though monitoring well GWQ11-26 was recently installed in 2011, it is upgradient of the mine and the ore body; therefore, it is representative of pre-discharge conditions in that area. The TDS concentration in GWQ11-26 has been analyzed to be 905 mg/L or less. TDS samples collected from wells GWQ-3 and GWQ-5 in 1981 and 1982 represent pre-discharge TDS concentrations downgradient of the ore body. TDS concentration from GWQ-3 and GWQ-5 range from 868 mg/L to 1,260 mg/L suggesting that the ore body is naturally influencing TDS concentrations in the Grayback Alluvial Aquifer downstream.

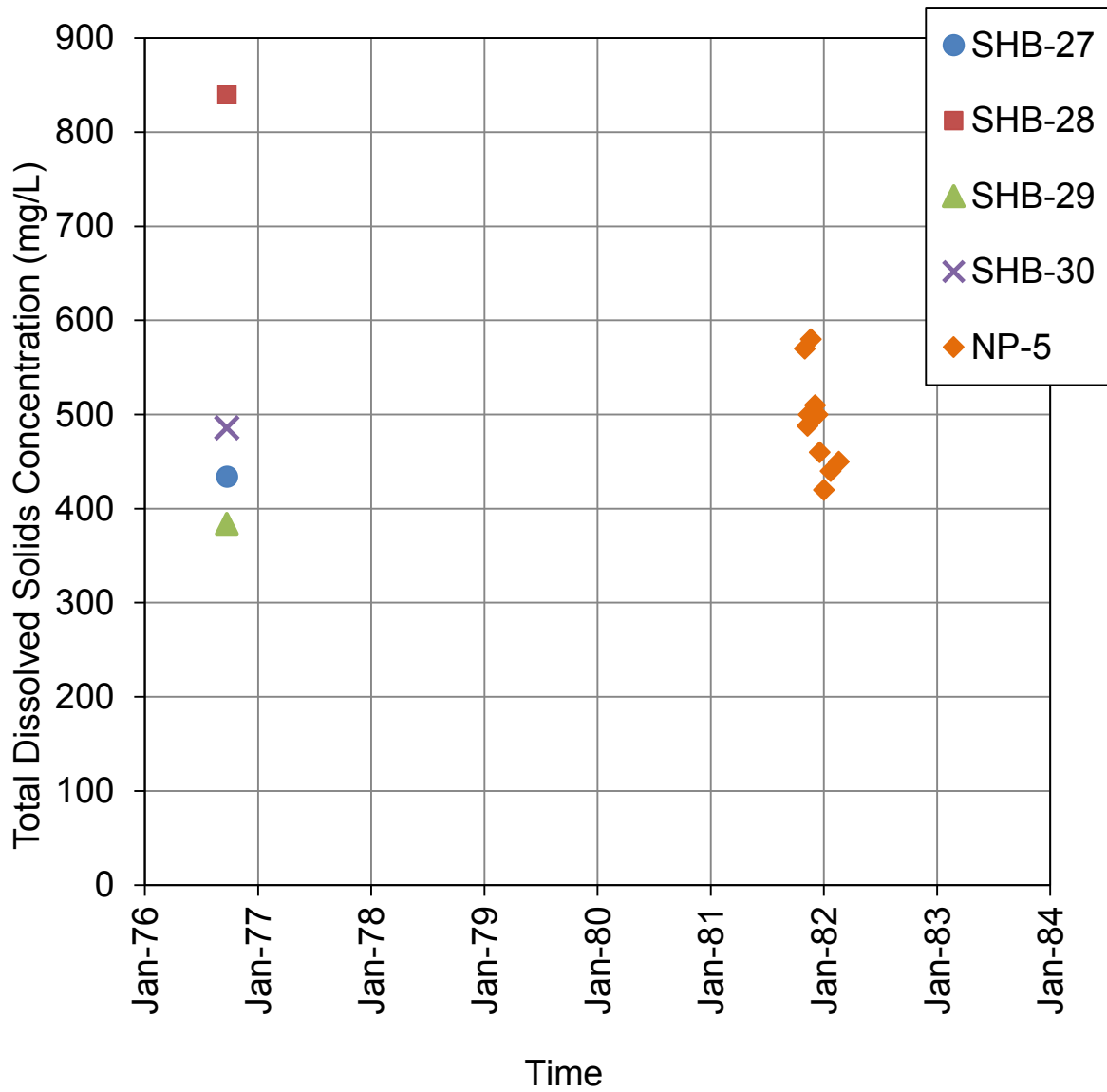
Figure 11G-2 shows consistent pre-discharge TDS concentrations for the Quaternary Alluvial and Fluvial deposits in the Upper Santa Fe Group Aquifer. More than 90 percent of the available pre-discharge data shows a consistent TDS concentration between about 380 and 580 mg. One outlier shows a TDS concentration of 840 mg/L.

Figure 11G-3 shows consistent pre-discharge TDS concentrations for the Santa Fe Group Aquifer. The TDS concentrations are consistently between 350 and 650 mg/L.

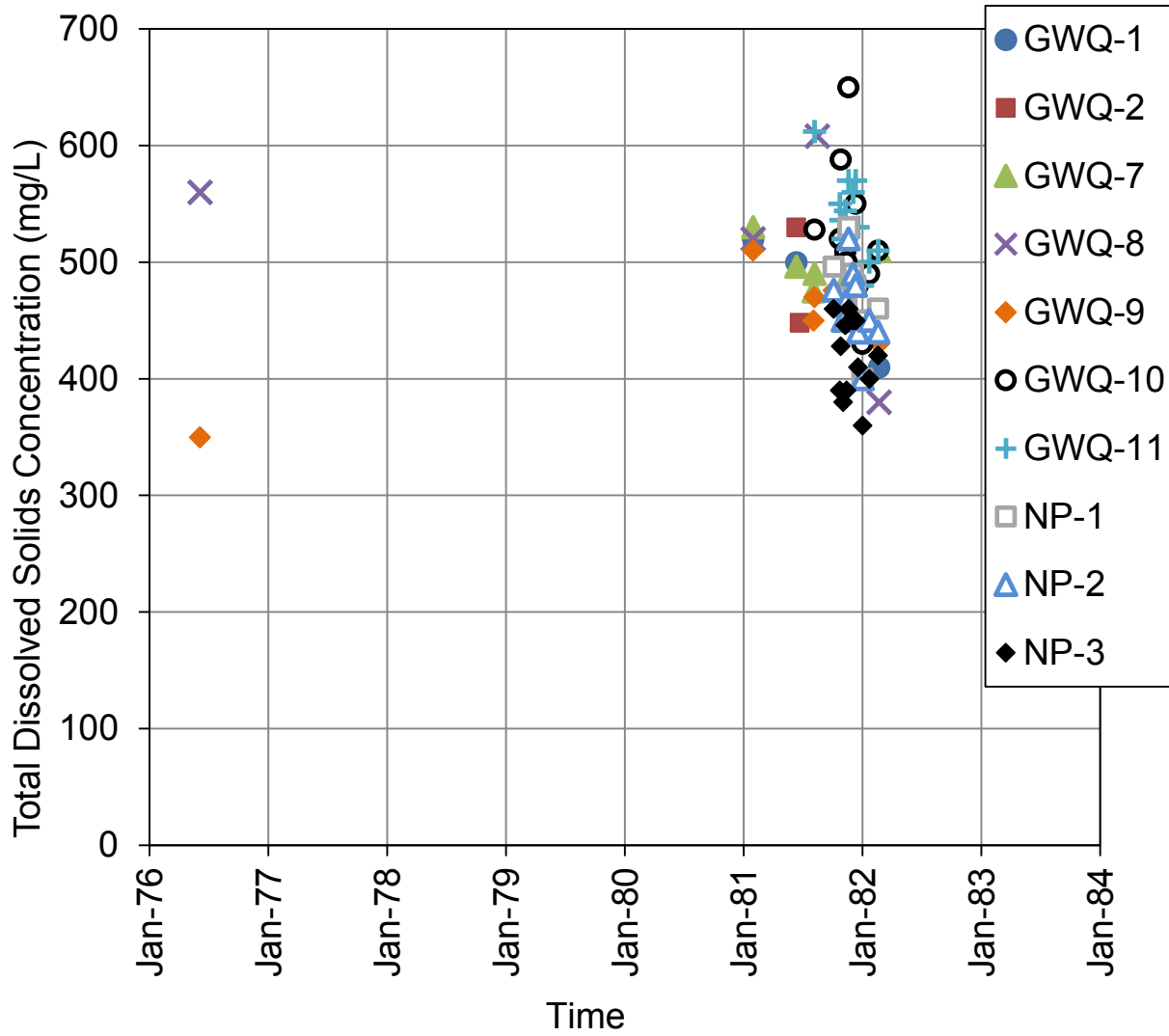
Figure 11G-4 shows that, in general, the TDS concentration in the Crystalline Bedrock Aquifer ranges from 500 mg/L to 920 mg/L. This is the case for groundwater within the hydrologic pit created by the pit and down-gradient of the pit. TDS concentration in groundwater sampled from wells within the ore body ranges from 2,280 to 4,400 mg/L. This illustrates the influence that the sulfide ore body has on the local groundwater quality.



**Figure 11G-1**  
**Quaternary Grayback Alluvial Aquifer TDS Concentration**



**Figure 11G-2**  
**Quaternary Alluvial & Fluvial Deposits in Upper Santa Fe Group TDS Concentration**



**Figure 11G-3**  
**Santa Fe Group Aquifer TDS Concentration**

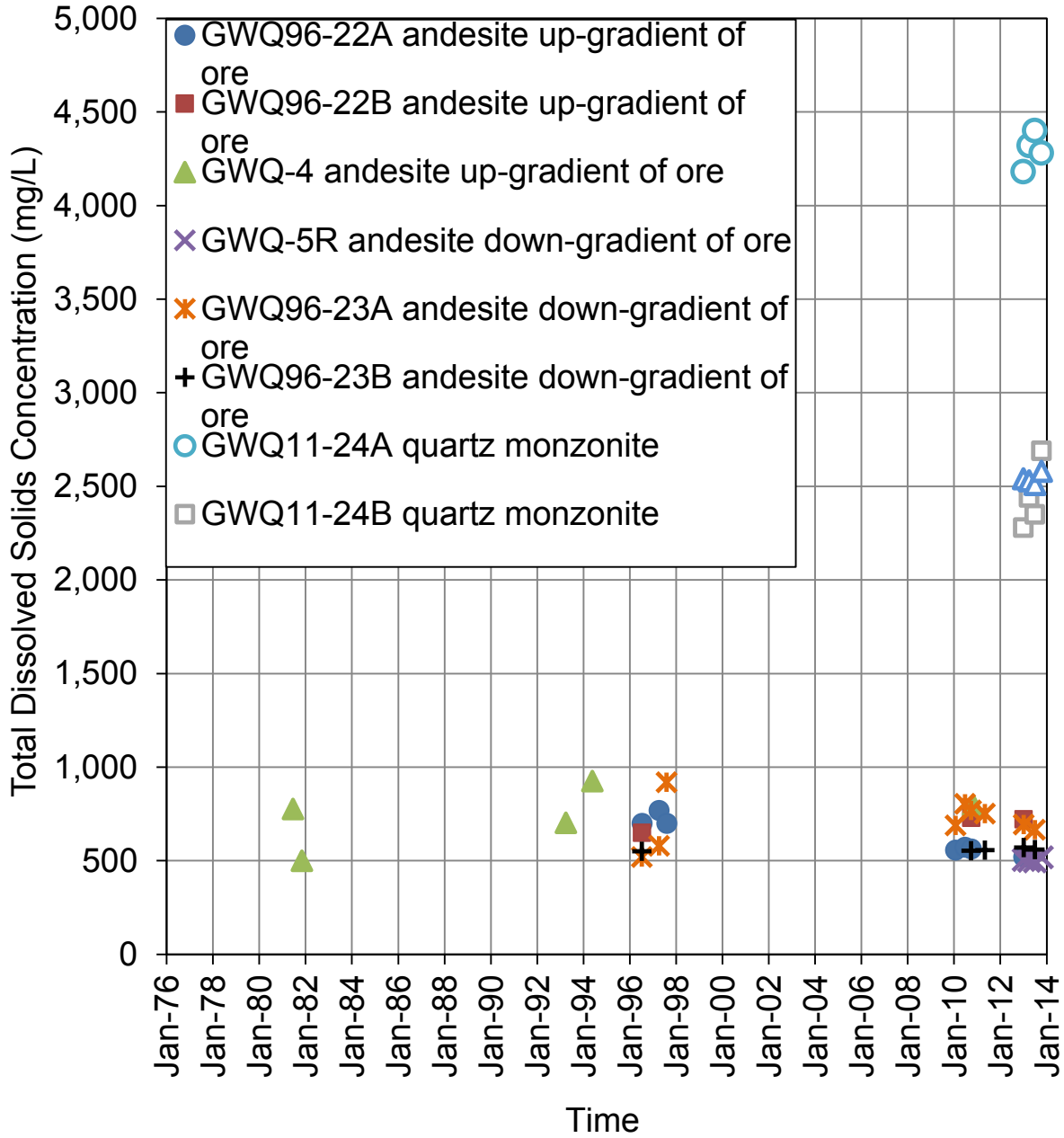


Figure 11G-4  
Crystalline Bedrock Aquifer TDS Concentration





**20.6.7.11.H DETERMINATION OF MAXIMUM DAILY DISCHARGE VOLUME**

An application shall include the following information:

**20.6.7.11.H.(1)** The proposed maximum daily discharge volume of process water and tailings for each discharge location and a description of the discharge locations and the methods and calculations used to determine that volume.

Table 11H-1 presents the anticipated maximum daily discharge volume of process water and tailings for the various discharge locations containing mill process and tailings effluents.

<b>TABLE 11H-1 Maximum Daily Discharge Volume of Process Water and Tailings</b>			
<b>Location</b>	<b>Point of Measure</b>	<b>Process Water Gallons per Day</b>	<b>Tailings Tons per Day</b>
Concentrator Tailings	Concentrator Tailings Sump	25,264,000	38,000
Tailings Storage Facility Water Recycle System	Recycle Water Return to Process Water Reservoir	21,236,000	0
Process Water Reservoir	Reservoir Return to Concentrator	24,300,000	0
Pit Bottom	Pit Water Sump	34,560	0

The basis for these estimates is as follows:

1. Process engineering and flow sheets prepared for NMCC by M3 Engineering and Technology Corp (M3) for the Copper Flat Feasibility Study;
2. Concentrator processing rate increased 20% over plan average to provide for operating peaks (plan average provided below in Subsection 20.6.7.11.H.(3));
3. Concentrator processes 1,600 tons of ore per hour;
4. Concentrator operates at 100% utilization (average 24 hours per day);
5. Design maximum flow rate of tailings under-drain collection system (see Appendix A); and
6. Pit discharge volume based on anticipated seepage rate into pit of 24 gpm.

**20.6.7.11.H.(2)** The identification of all sources of process water and tailings

Table 11H-2 summarizes the sources of process water and tailings at the Copper Flat Mine.



<b>TABLE 11H-2 Sources of Process Water and Tailings</b>			
<b>Location</b>	<b>Point of Measure</b>	<b>Process Water</b>	<b>Tailings</b>
Concentrator Tailings	Concentrator Tailings Sump	Yes	Yes
Tailings Storage Facility	Reclaim Water Return to Process Water Reservoir	Yes	No
Process Water Reservoir	Reservoir Return to Concentrator	Yes	No
Pit Water	Pit Water Sump	Yes	No

Mineral recovery at Copper Flat will be accomplished at the Copper Flat concentrator facility by standard crushing, grinding, and flotation processes only. There are no plans for leaching mine rock stockpile or ore heaps at Copper Flat.

Concentrator tailings will be piped from the concentrator as a slurry comprised of process water and flotation tailings, i.e., whole tailings, to the Copper Flat tailings storage facility. In route to the tailings storage facility, the slurry will pass through a cyclone classification plant to separate the whole tailings into coarse and fine fractions to facilitate ongoing construction of the impoundment and distribution of tailings within the storage facility. When the cyclone plant is not operating, concentrator tailings will bypass the plant for direct deposit at the tailings storage facility. In addition, tailings may be diverted to the surge water pond directly from the whole tailings pipeline or from the cyclone process plant in the event of upset conditions, not under normal operating conditions.

Process water will be collected from two locations at the tailings storage facility, i.e., the supernatant pond that will form on top of the stored tailings impoundment and the under-drain collection pond constructed at the base of the tailings impoundment. Water from these ponds will be combined into a single pipeline and returned to the process water storage reservoir for reuse in the mineral recovery process. The volume of process water shown in Tables 11H-1 and 11H-2 for the Tailings Storage Facility reflects the total volume from both of these sources.

The process water reservoir will hold water recycled from tailings facility and in-process dewatering points and will also serve as the entry point for fresh water addition to the mineral recovery process. The reservoir will be located in the vicinity of the concentrator facility. Water from the process water storage reservoir will be pumped to a process water head tank for gravity distribution to the concentrator.

There are no plans for further treatment of the mineral concentrate at Copper Flat.



As described in subsection **20.6.7.11.J.(4)**, water from all concentrate produced at the mine will be removed using pressure filters before packaging and the concentrate will be transported off-site for smelting and refining at a separate location.

Water produced from seepage into the open pit will be used for dust control within the open pit surface drainage area. NMCC anticipates that approximately 24 gallons per minute will be generated from seepage into the pit. All of this water will be easily consumed in the operations activities.

**20.6.7.11.H.(3)** *The estimated daily volume of process water and tailings generated*

Table 11H-3 summarizes estimated daily volumes of process water and tailings for each of the sources identified in Subsection **20.6.7.11.H.(2)**, above.

<b>TABLE 11H-3</b>			
<b>Daily Volume of Process Water and Tailings – Average</b>			
<b>Location</b>	<b>Point of Measure</b>	<b>Process Water Gallons per Day</b>	<b>Tailings Tons per Day</b>
Concentrator Tailings	Concentrator Tailings Sump	19,475,000	30,000
Tailings Storage Facility	Recycle Water Return to Process Water Reservoir	16,370,000	0
Process Water Reservoir	Reservoir Return to Concentrator	18,731,000	0
Pit Bottom	Pit Water Sump	34,560	0

The Basis of these estimates is as follows:

1. Process engineering and flow sheets prepared by M3 Engineering;
2. Concentrator processes 1,333 tons of ore per hour;
3. Concentrator operates at 92.5% utilization (average 22.2 hours per day); and
4. Pit discharge volume based on anticipated seepage rate into pit of 24 gpm.

**20.6.7.11.H.(4)** *Information regarding other waste discharges (i.e., domestic or industrial) at the copper mine facility. Permit identification numbers shall be submitted for those discharges that are already permitted.*

Disposal of domestic wastes generated at the Copper Flat Mine will be accomplished through a combination of a single packaged wastewater treatment plant serving the majority of employees and visitors at the mine and individual portable toilet facilities for outlying areas of the operation. The packaged wastewater treatment plant will receive and treat domestic wastes from buildings located in the administration, concentrator, and



mine shop areas. The packaged system is sized for a load based on the number of mine employees and visitors expected at the mine during a 24-hour period and applying an average water use of 50 gallons per day per person.

Breaking down the employee headcount for the mine by the planned rotation schedule indicates 160 employees per day will be using facilities connected to the package plant. In addition to mine employees, an additional 40 persons per day are assumed to account for visitors and contractors. Based on these figures, a 10,000 gallon per day plant has been selected for Copper Flat Mine. The design criteria for this facility will be revisited during final design to ensure that the packaged plant capacity is appropriate for the size of the facility.

The plant will be located on a pre-existing concrete slab near the main gate. The plant will generate effluent treated to secondary treatment levels. Treated effluent from the plant will be piped to the tailings storage facility for reuse as process water. System specifications and installation will conform to State and local regulations. Manufacturer information describing a type of packaged system that may be used at the Copper Flat Mine is attached as an example; final specifications and selection will be developed as the project advances to construction.

Individual portable toilet facilities will be provided for employees working in outlying areas of the operation that will not be connected to the packaged wastewater treatment plant (the pit, mine stockpile areas, the primary crusher, and the TSF). The portable toilets will be maintained by a licensed contractor on a regular basis.



### **20.6.7.11.I PROCESS WATER AND TAILINGS QUALITY**

*An application shall include estimated concentrations of process water and tailings slurry quality for the constituents identified in 20.6.2.3103 NMAC including the basis for these estimations.*

An estimate of the concentrations of dissolved constituents identified in **20.6.2.3103 NMAC** in process water and tailing slurry pore water per **20.6.7.11.I NMAC** are provided in Table 11I-1. The basis used to develop the table is largely on information provided by SRK Consulting U.S. Inc., as presented in their “Geochemical Characterization Report for the Copper Flat Project New Mexico, May 2013” (SRK 2013). This report has been previously submitted to NMED and is included in this Discharge Plan application by reference. It is also based on an understanding of the chemicals used in the milling process and the geochemical processes and water-rock reactions expected to occur as part of the management of the tailing slurry and associated process waters. SRK (2013) provides solution constituent concentrations that were utilized to estimate anticipated pore water and process water chemistries as input to the geochemical modeling of the tailing storage facility at the proposed Copper Flat Project. The sources of this data included information from:

- averaged analytical data of the analyses of pore water extracts from the tailing humidity cell tests (HCT) of samples prepared in the laboratory;
- analyses of composite of extracted samples of tailing pore water from the flotation tails analyzed during operation of the Quintana facility between 1981 and 1982; and
- analyses of pore water extracted during meteoric water mobility testing of a single weathered tailing grab sample collected from the historic tailing facility.

While SRK’s results are meant to be representative of anticipated tailing and process water chemistry, they are not an exact duplicate of what will actually be encountered during operations and should be viewed as such. Table 11I-1 presents an estimated range of concentrations based on SRK’s data.

<b>TABLE 11I-1</b>	
<b>Estimated Constituent Concentrations in Process Water &amp; Tailings</b>	
<b>20.6.2.3103 NMAC Constituent <sup>a</sup></b>	<b>Estimated Concentration Range (mg/l <sup>a</sup>)</b>
Arsenic	Less than 0.01 to 0.005
Barium	Less than 0.05 to 0.2
Cadmium	Less than 0.001 to 0.005
Chromium	Less than 0.005 to 0.005
Cyanide	Not analyzed for by SRK
Fluoride	Less than 1.96 to 2.0
Lead	Less than 0.002 to 0.02
Total Mercury	Less 0.0001 to 0.001
Nitrate (as N)	Less than 1 to 5.49
Selenium	Less than 0.005 to 0.01
Silver	Less than 0.01 to 0.02
Uranium	Less than 0.04 to 0.19
Radioactivity: combined Ra-226 & Ra-228	Not analyzed for by SRK
Volatile Aromatic and Chlorinated Hydrocarbons, PAHs and PCBs in items (14) through (33) 20.6.2.3103	Not analyzed for by SRK as these compounds are not expect to be used at or produced as part of the ore processing or tailing and water management processes
Chloride	Less than 5.25 to 28
Copper	Less than 0.05 to 0.58
Iron	Less than 0.01 to 0.04
Manganese	Less than 0.03 to 0.18
Phenols	Not analyzed by SRK as these compounds are not expect to be used or produced as part of the ore processing or tailing management processes
Sulfate	Less than 75 to 2400
Total Dissolved Solids	Less than 270 to 3,700
Zinc	Less than 0.01 to 0.05
pH	7.5 to 8.1 (std. units)
Aluminum	Less than 0.01 to 0.06
Boron	Less than 0.1 to 0.11
Cobalt	Less than 0.01 to 0.02
Molybdenum	Less than 0.06 to 3.5
Nickel	Less than 0.01 to 0.05

a. except as noted



## **20.6.7.11.J IDENTIFICATION AND PHYSICAL DESCRIPTION OF THE COPPER MINE FACILITY**

*An applicant shall provide the following information;*

The following subsections **20.6.7.11.J.(2) through (11)** provide the information required. They describe the proposed Copper Flat mine facility in detail showing the location and features of the facility as required by the Copper Rules, including the impoundments and waste rock stockpiles, the open pit mine facility, sumps,, tanks, pipelines and wash units, storm water management, water wells and monitoring wells, flow meters, surface waters, sampling locations, and waste treatment facility.

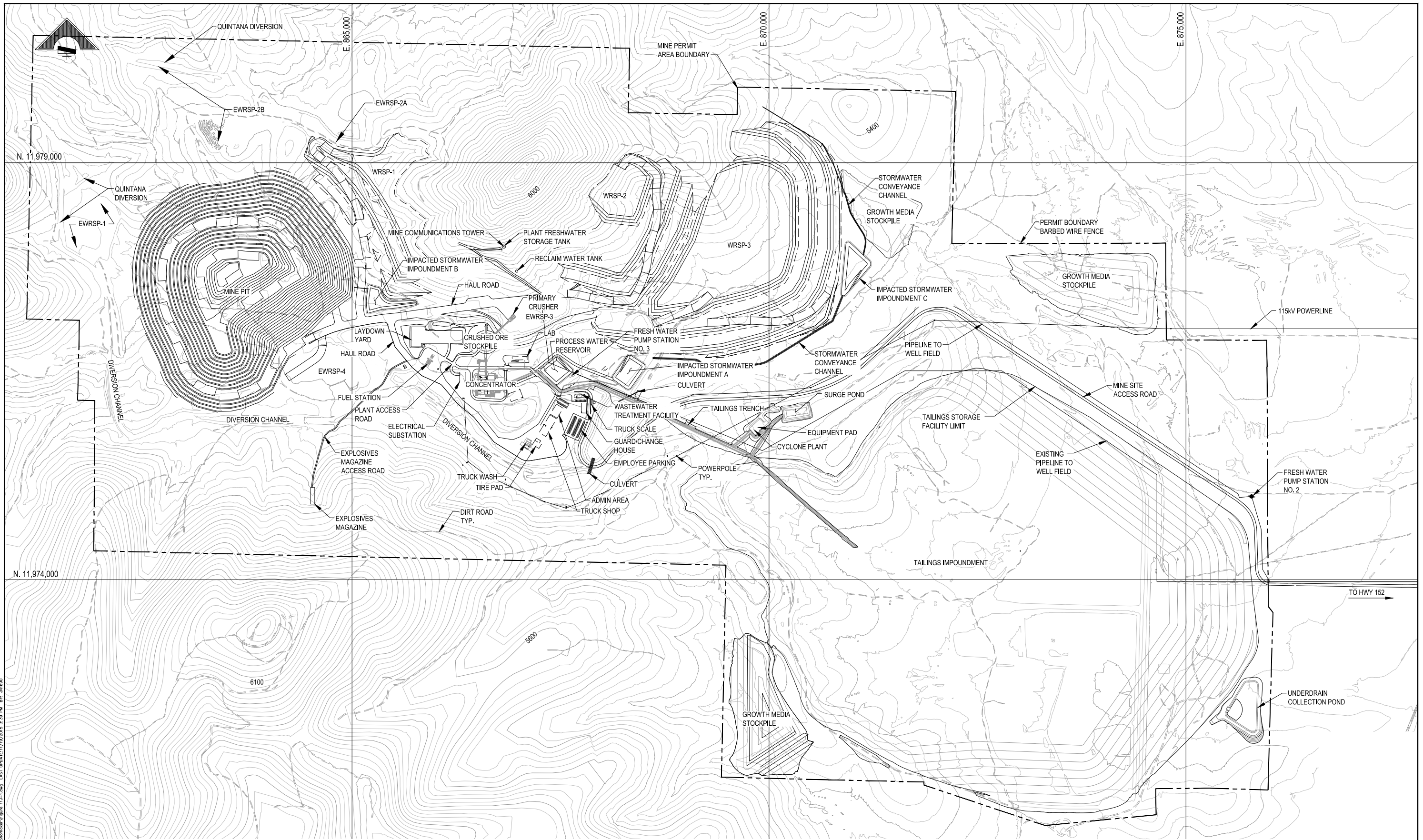
The infrastructure supporting the project is discussed in various places in this application. For example, under normal operating conditions electrical power is provided to the site by the Sierra Electric Cooperative, a reliable provider of residential and commercial electrical power throughout the region. However, recognizing the need to ensure that emergency power is available for critical operating systems for the protection of personnel safety, as well as protection of human health and the environment, the Copper Flat facilities will be equipped with sufficient backup power systems to provide emergency power. An on-site diesel powered generator designed to provide sufficient emergency power to the facility in the event of a power failure will be installed. All critical systems, including pumps, sumps, process areas, tailings impoundment pipelines and other areas that have dedicated process water handling equipment that must remain operational during disruption of the normal power supply, will be tied into the site emergency power grid to ensure that unauthorized discharges to ground water do not occur. The emergency generator will start automatically whenever power a disruption is detected and will be tested monthly to ensure dependable response and operation.



**20.6.7.11.J.(1)** *a scaled map of the entire existing or proposed copper mine facility showing the location of all features identified in Paragraphs (2) through (11) of this subsection; the map shall be clear and legible, and drawn to a scale that all necessary information is plainly shown and identified; the map shall show the scale in feet or metric measure, graphical scale, a north arrow, and the effective date of the map; multiple maps showing different portions of the copper mine facility may be provided using different scales as appropriate; documentation identifying the means used to locate the mapped objects (i.e., global positioning system (GPS), land survey, digital map interpolation, etc.) and the relative accuracy of the data (i.e., within a specified distance expressed in feet or meters) shall be included with the map; any objects that cannot be directly shown due to its location inside of existing structures, or because it is buried without surface identification shall be identified on the map in a schematic format and identified as such;*

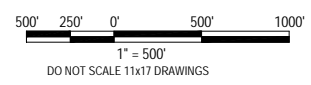
Figure 11J-1 provides a map of the entire site facility showing the location of the various features identified in paragraphs (2) through (11) of this subsection. Figure 11J-2 is a map of the site as it currently exists, showing the various existing facilities as constructed by Quintana when it operated the mine in the early 1980's. This map is provided so that the reviewer can differentiate between what currently exists as a disturbance on-site as compared to NMCC's proposed activities. Figure 11J-3 is a map of the site depicting the various watersheds that will be developed at the mine site in order to manage surface water runoff. These three maps contain the essential information required by **20.6.7.11.J.(1)**. There are also additional maps included within each subsection as each feature is described in more detail.





EWRSP = EXISTING WASTE ROCK STOCKPILE  
 WRSP = WASTE ROCK STOCKPILE

**SITE PLAN**  
 SCALE: 1:500



**PRELIMINARY**  
 FOR AGENCY REVIEW



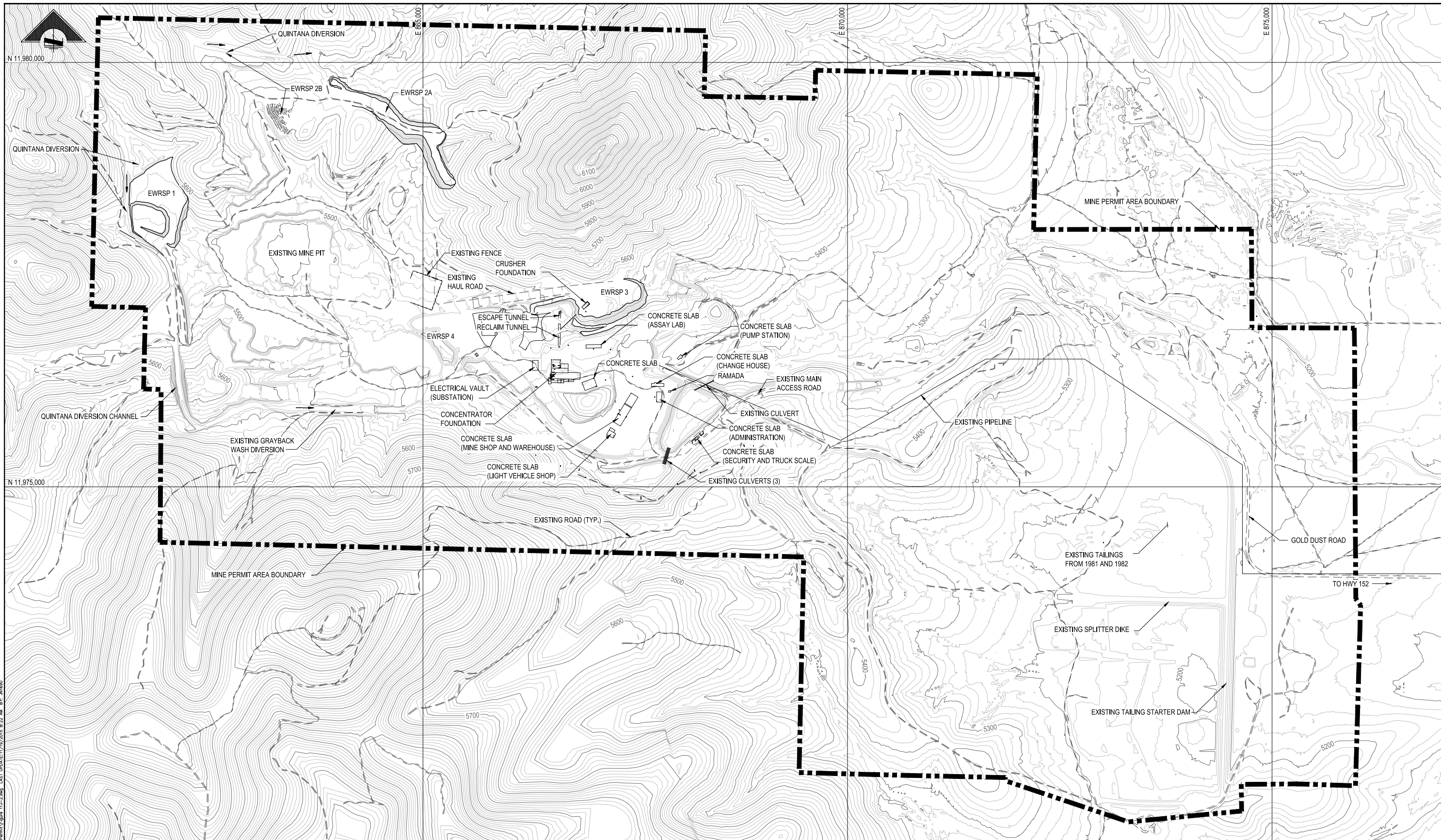
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DESIGNED BY: SAM	DATE: DEC12
DRAWN BY: SAM	DATE: JAN13
CHECKED BY: TDL	
PROJECT MGR: RKZ	
CLIENT APPR:	

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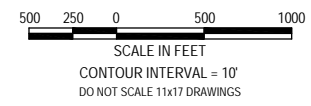
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 JOB NO. M3 PN-120085  
**SITE GENERAL CIVIL PROJECT AREA PROPOSED SITE PLAN**  
**FIGURE 11J-1**  
 REV NO. P18 DATE 16 NOV 15

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**NOTES:**  
 CONCRETE FOUNDATIONS SHOWN REFLECT PLANS  
 BY QUINTANA MINERALS CORPORATION FROM 1981  
 AND MOST RECENT SURVEY CONDUCTED.

**SITE PLAN**  
 SCALE: 1" = 500'



**PRELIMINARY**  
 FOR AGENCY REVIEW



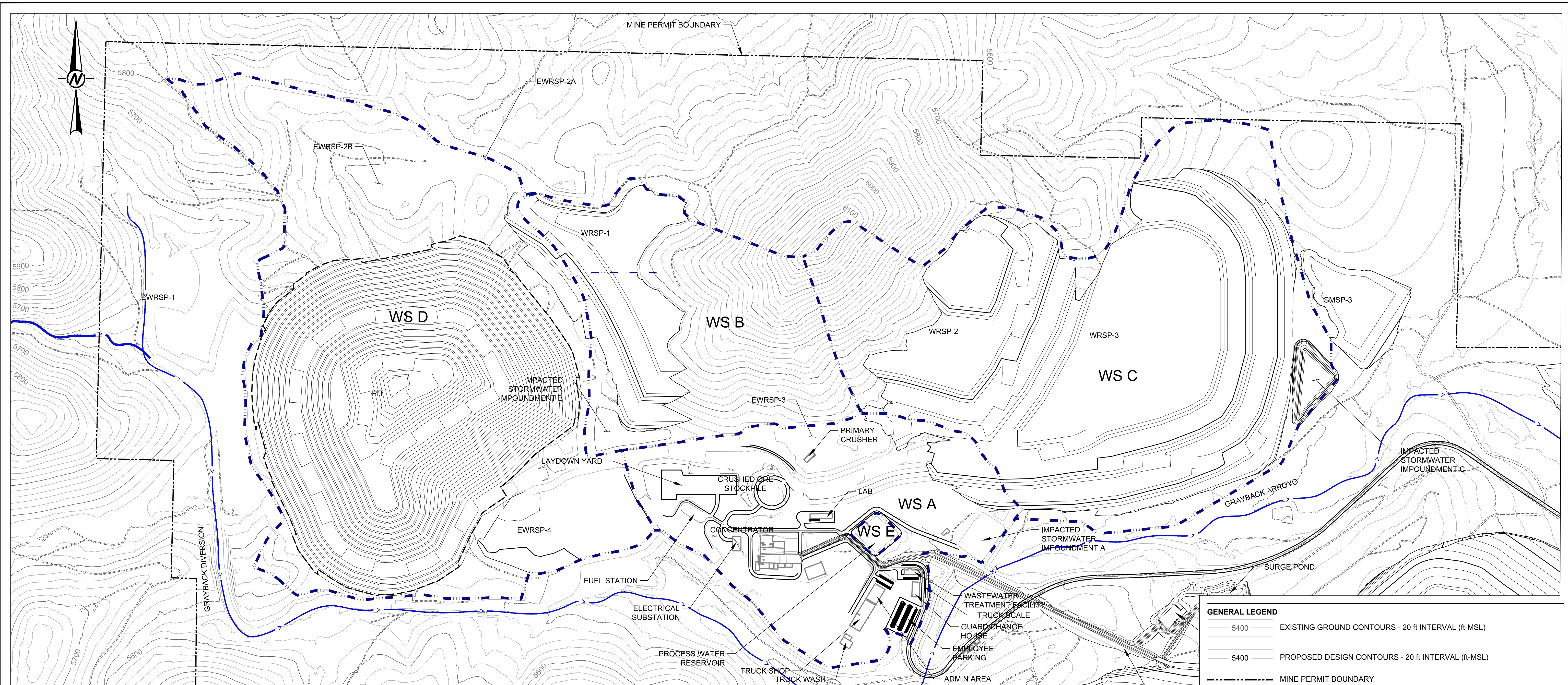
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SCALE: 1" = 500'	DATE:
DESIGNED BY: JPN	OCT 15
DRAWN BY: JPN	OCT 15
CHECKED BY:	
PROJECT MGR: RKZ	
CLIENT APPR:	

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**COPPER FLAT PROJECT**  
 JOB NO. M3 PN-12085  
**SITE GENERAL CIVIL**  
**MINE SITE**  
**EXISTING CONDITIONS**  
 DWG NO. **FIGURE 11J-2**  
 REV NO. P2 DATE 20 NOV 15

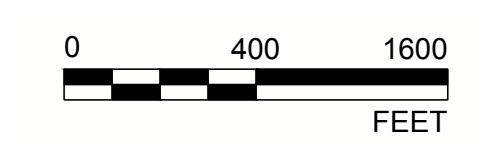
File: P:\2015\20085\CFL (344)\344.2\_Dept\Figures for Permit\Figure 11J-2.dwg LAST UPDATE: 11/19/2015 8:22 AM BY: JN600



**GENERAL LEGEND**

	EXISTING GROUND CONTOURS - 20 ft INTERVAL (ft-MSL)
	PROPOSED DESIGN CONTOURS - 20 ft INTERVAL (ft-MSL)
	MINE PERMIT BOUNDARY
	WATERSHED BOUNDARY (BY OTHERS)
	EXISTING DIVERSION OR ARROYO
	EXISTING ROAD
	WRSP WASTE ROCK STOCKPILE
	EWRSP EXISTING WASTE ROCK STOCKPILE
	TSF TAILING STORAGE FACILITY
	GMSP GROWTH MEDIA STOCKPILE
	WS WATERSHED

- NOTES**
- EXISTING GROUND TOPOGRAPHY WAS DEVELOPED BY COOPER AERIAL SURVEYS BASED ON JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
  - FINAL BUILDOUT TOPOGRAPHY FOR THE PIT, WRSPs, AND GMSPs WAS DEVELOPED BY OTHERS AND PROVIDED BY NEW MEXICO COPPER CORPORATION (NMCC).
  - WS D IS THE SURFACE WATERSHED EXPRESSION OF THE OPEN PIT SURFACE DRAINAGE AREA (OPSDA) DURING OPERATIONS, AND WS B PLUS WS D IS THE SURFACE WATERSHED EXPRESSION OF THE OPSDA AFTER RECLAMATION.



Path: \\ussmcc\usmcc\Copper Flat\02\_Projects\1531453\_1531463\_THEMACPermitSupport\0500\_RepairFigures\ File Name: 1531453-0500-FIG2-14.dwg

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A4S D

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
C	2017-07-10	ISSUED FOR AGENCY REVIEW	HNL	HNL	TS	JC
B	2017-06-21	ISSUED FOR CLIENT REVIEW	HNL	HNL	TS	TS
A	2017-06-21	ISSUED FOR INTERNAL REVIEW	HNL	HNL	TS	TS

SEAL

**PRELIMINARY FOR AGENCY REVIEW**

CLIENT

**THEMAC RESOURCES** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community-Minded. Local Opportunities.

CONSULTANT

**Golder Associates**

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4730 N. ORACLE ROAD, SUITE 210  
TUCSON, ARIZONA  
UNITED STATES OF AMERICA  
[+1] (520) 888 8818  
www.golder.com

PROJECT  
COPPER FLAT PROJECT

TITLE  
**DEVELOPED WATERSHED AREAS**

PROJECT NO. 1531453	CONTROL 0500	REV. C	1 of 1	FIGURE 11J-3
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**20.6.7.11.J.(2)** a description of each existing or proposed tailing impoundment, leach stockpile, process water and impacted stormwater impoundment, waste rock stockpile, and slag including information about its location, purpose, liner material, storage or disposal capacity, and the methods proposed or used to prevent pollution of ground water;

**I. 20.6.7.11.J.(2) DESCRIPTION OF THE PROPOSED TAILINGS IMPOUNDMENT, UNDERDRAIN COLLECTION SYSTEM, AND WATER RECLAIM SYSTEM**

NMCC proposes to construct a new tailings storage facility (TSF) which will include a lined tailings impoundment with an associated underdrain collection system and underdrain collection pond for the impoundment and the dam as well as a water reclaim or recycle system to maximize water reuse. The TSF also includes a cyclone plant to separate the tailings coarse and fine fractions and a surge pond to handle potential upset conditions at its Copper Flat project.

Appendix A, Feasibility Level Design, 30,000 TPD Tailings Storage Facility and Tailings Distribution and Water Reclaim Systems, Copper Flat Project, Sierra County, New Mexico, November, 2015, Revised June 2016, prepared by Golder Associates, Inc., provides the technical design detail for the TSF. This document is made part of NMCC's Discharge Permit application by reference herein.

**Location**

The location of the proposed new TSF is shown in Figure 11J-1 provided herein in response to **20.6.7.11.J.(1)**.

**Purpose**

The purpose of the TSF and the tailings impoundment is to handle, store and contain the tailings materials produced from processing of copper ore.

The purpose of the underdrain collection system and pond is to collect and store water that is captured by the blanket drain constructed under the tailings dam and the underflow drain system constructed under the tailings impoundment. The dam blanket drain will provide the mechanism that will drain water within the dam out of the structure and allows the dam to become consolidated and stable as it is continually constructed during operations as the sand fraction of tailings is emplaced on the dam. The tailings impoundment underdrain system will allow free water in the impoundment to be drained out of the impoundment from underneath the impoundment and be recycled to the processing facility. The underdrain, together with the impoundment synthetic liner, will provide significant mitigation against the potential for seepage from the impoundment. It will also contribute to the ability to recycle water from the system to the process facility while contributing to the stability of the TSF. The TSF underdrain collection pond will also serve to capture surface water runoff routed from the downstream face of the tailings dam via runoff control ditches to the pond.



The purpose of the water reclaim system is to provide a mechanism by which as much water as possible can be pumped from the inner surface of tailings impoundment and recycled back to the processing facility. The water reclaim system will consist of a floating barge located within the tailings impoundment containing pumps to remove water from the impoundment, a pipeline to transport the reclaimed water to the process water reservoir, and the reservoir itself. The reservoir will be located at the process facility areas shown in Figure 11J-1 and 11J-3. Pumping water out of the tailings impoundment will also provide the capacity needed for tailings disposal.

The TSF will also have an associated surge pond that will be part of the cyclone plant located at the TSF. The cyclone plant will receive whole tailings slurry from the process circuit and separate the sand fraction from the slimes fraction. The purpose of the surge pond, located at the cyclone plant, will be to capture and temporarily retain tailings materials in the event of a temporary upset in the cyclone plant. It will also provide temporary storage in the event that an upset occurs in the process circuit.

### **Liner material**

The tailings impoundment will be lined with an 80-mil high density polyethylene (HDPE), or equivalent liner, placed on a minimum 12-inch thick liner bedding fill material. In the initial phases of construction the bedding material will consist of recovered sand from the old Quintana starter dam. Later phases of construction will require the use of selected crushed and screened native materials or selected local soil be utilized.

The TSF underdrain collection pond will be double-lined with a 60 mil, or equivalent, HDPE geomembrane liner. An HDPE geonet will be placed between the liners to serve as the leak collection and recovery system (LCRS) and to minimize pressure on the lower pond liner.

The surge pond liner will consist of a compacted liner bedding fill layer, overlain with a 60 mil HDPE geomembrane or equivalent. The liner bedding will be a minimum of six inches of sand or fine soil.

### **Storage or disposal capacity**

The tailings impoundment is designed to store 113 million tons of tailings produced over approximately 11 years. Tailings deposition will occur at a rate of approximately 32,000 tons per day (tpd).

The TSF Underdrain collection pond will be sized to contain 24 hours of underdrain flow at maximum estimated drainage rates from the dam and impoundment underdrains, as well as runoff from the 100-year, 24-hour storm event of 3.73 inches incident on the downstream dam face. The pond capacity will be approximately 12.22 million gallons with 2 feet of freeboard.



The surge pond is designed to retain tailings from the cyclone plant and other process water from the plant site not diverted directly to the tailings impoundment in the event of temporary upset conditions. Under normal operating conditions the surge pond will be empty. The surge pond is designed with a feasibility level design capacity of 1.6 million gallons, sufficient to handle the volume of upset conditions plus direct precipitation from a 100-year 24-hour precipitation event with at least 2 feet of freeboard. The design capacity conservatively assumes that an upset would occur during a maximum precipitation event and that the processing facility is running at maximum design rates. Given these assumes, the pond would reach its capacity in approximately one half hour. The pond will be equipped with dedicated pumps to automatically begin pumping materials to the TSF when the fluids in the pond reach a predetermined level. These pumps will be tied into the site's emergency power grid described earlier herein. The process control room will be equipped with emergency alarms that notify the operator of an upset condition allowing the operator to make necessary adjustments in the process, as needed. Section 7.4 of Appendix A describes the management of flows in more detail.

### **Methods proposed to prevent pollution of groundwater**

The primary method to prevent pollution of groundwater at the tailings impoundment, the surge pond, the TSF underdrain collection pond and the process water reservoir will be through the use of engineered systems to control and manage water. These systems include the use of liners, installation of a tailings dam and impoundment underdrain system, where appropriate, installation of a leak collection system, and construction of appropriate run-off and run-on control structures. Storm water will be diverted away from and around these facilities to the maximum extent possible to minimize the potential for contact of storm water with materials that have the potential to impact ground water. Run-on to these facilities will be minimized to the maximum extent possible so as to reduce the volume of water that enters these facilities, thus mitigating the potential for impact to groundwater.

The tailings dam and impoundment will be constructed with underdrain systems that will collect water from the dam and the impoundment and capture it in the underdrain collection pond. The tailings impoundment will be lined with an 80 mil HDPE geomembrane, or equivalent, material.

The underdrain collection pond will be double-lined with a 60 mil HDPE geomembranes, or equivalent, and be equipped with a leak collection system to protect groundwater. In addition, the water in the pond will be continually evacuated from collection pond to the process water reservoir where it will be recycled into the process circuit.

Storm water that falls onto the down-stream face of the tailings dam will be captured in run-off control ditches. The ditches will be constructed at the outside toe of the dam and routed to the underdrain collection pond. The surge pond will also be lined with a 60 mil HDPE geomembrane, or equivalent, to protect groundwater.



In addition to the mitigation measures identified above to protect groundwater, all of the pipelines that transport tailings, impacted storm water or reclaimed water will be conveyed within lined channels designed to direct leaks and spills to impoundments so as to protect ground water. The ground surface surrounding the surge pond will be graded in a manner to direct surface runoff away from the pond.

NMCC's significant proactive approach to maximally and proactively managing and recycling water, in and of itself, provides a means of protecting groundwater.

## **II. 20.6.7.11.J.(2) DESCRIPTION OF PROPOSED LEACH STOCKPILES**

NMCC does not propose to construct or operate any leach stockpiles at the Copper Flat project.

## **III. 20.6.7.11.J.(2) DESCRIPTION OF PROCESS WATER IMPACTED STORM WATER IMPOUNDMENTS**

NMCC proposes to construct a process water reservoir and three (3) storm water impoundments. The process water reservoir will hold water from several sources including the reclaimed water from the TSF, captured storm water, and fresh make-up water from the off-site well field. The storm water impoundments will capture surface water runoff from the waste rock stockpile areas and the plant area. Appendix B, Impoundment Design Report, Copper Flat Project, November 2015 (M3 2015), prepared by M3 Engineering & Technology Corporation, provides the technical design details for the impoundments and the process water reservoir. This document is made part of NMCC's Discharge Permit application by reference herein.

NMCC has determined that it will address the three storm water impoundments as if they were impacted storm water impoundments as defined in Section **20.6.7.7.B.(29) and (30)** of the Copper Rule, although there is some uncertainty as to whether or not one or more, in fact, meet the definition because of the quality of the water that they are intended to capture. NMCC will work with the NMED as the detailed designs become more definitive to determine the applicability of the impacted storm water requirements to each of these impoundments.

Figure 11J-3 shows the various watershed areas that will be developed on-site by grading and contouring the areas to control and capture surface water runoff. The developed watershed areas shown are as follows:

- Watershed area A (WS A) wherein the process facilities and ancillary plant areas, including the ore stockpile, will be located. An existing waste rock stockpile, EWRSP-3, as shown in Figure 11J-1, 2 and 3, also called the low-grade ore stockpile in earlier documents, is also located within WS A;



- Watershed area B (WS B) is a portion of the open pit surface drainage area wherein the proposed new Waste Rock Stockpile no. 1 (WRSP-1) will be located. An existing waste rock stockpile, EWRSP-2A, as shown in Figure 11J-1, 2 and 3, (also called the north waste rock disposal facility in earlier documents) is located at the northern edge of WS B;
- Watershed area C (WS C), wherein the proposed new Waste Rock Stockpiles no. 2 and 3 (WRSP-2 and WRSP-3) will be located; and
- Watershed area D (WS D) is a portion of the open pit surface drainage area wherein the mine pit is located. Existing waste rock stockpiles EWRSP-1 (also called the west waste rock disposal facility in earlier documents), EWRSP-2B (also called the north waste rock stockpile in earlier documents), and EWRSP-4 (also called the south waste rock stockpile in earlier documents) shown in figures 11J-1, 2 and 3 are also located within WS D. EWRSP-1 will no longer be part of the WS D after it is reclaimed.

As noted above, developed WS B and D are sub-watersheds of the larger open pit surface drainage area. The entire area naturally drains to the mine pit. NMCC has opted to develop these two sub-watersheds separately in order to provide control of the amount of surface water that will report to the mine pit under normal operating conditions. Surface water runoff from WS D will flow directly to the bottom of the pit as shown in Figure 11J-3. Surface runoff from WS B will be diverted to flow into Impacted Storm water Impoundment B as shown on Figures 11J-1 and 3.

As described in more detail in Appendix B, Impacted Storm water Impoundment B will be constructed at the lower southwestern corner of developed WS B to capture runoff from the proposed new WRSP-1 under normal operating conditions. However, should overflow from this impoundment occur as a result of an extraordinary precipitation event, it will flow over the spillway and into the open pit via a culvert, as shown in Figure 11J-3 and discussed in Appendix B. This will allow NMCC to control the flow of surface runoff into the pit while maximizing the harvesting of water for use as process water.

In addition to the developed watershed areas described above, there is also a small developed watershed area E (WS E), shown in Figure 11J-3. WS E represents the footprint of the process water reservoir, the impoundment that holds process water prior to it being introduced into the process circuit. WS E is depicted as a watershed to indicate that the reservoir will not collect any storm water runoff from the plant site. Only precipitation that falls directly on the reservoir will be collected therein. The details of design of the process water reservoir are contained in Appendix B.

Developed watersheds A, and C will each have a storm water impoundment associated with them, as shown in Figures 11J-1 and 3. The purpose of these impoundments will be to capture and manage storm water runoff. Appendix B provides the technical design details for these impoundments.





## Location

The location of the process water reservoir and the impoundments is shown in Figures 11J-1 and 11J-3, provided herein in response to **20.6.7.11.J.(1)**. Impacted Storm water impoundment A will be located at the southeastern edge of developed WS A wherein the process facilities, administrative building and associated mine infrastructure will be located and where EWRSP-3 is located. Storm water impoundment B will be located at the bottom of developed WS B at the southwestern corner of watershed. Storm water impoundment C will be located at the southeastern corner of developed WS C wherein WRSP 2 and 3 will be located.

The process water reservoir is designed to hold all of the water recycled from the tailings storage facility, water transferred from the impacted storm water impoundments and any required fresh make-up water from the off-site well field prior to introduction into the process circuit. Impacted storm water impoundment A is designed to capture and manage surface water runoff from WS A, i.e., the plant area. Impacted storm water impoundment B is designed to capture and manage storm water runoff from WS B which contains WRSP 1. Impacted storm water impoundment C is designed to capture and manage storm water runoff from WS C which contains WRSP 2 and WRSP-3.

## Purpose

The purpose of the Impacted Storm water Impoundments is to capture and manage storm water run-off from watershed (WS) areas WS A, B and C. The water captured in these impoundments will be transported to the process water reservoir within 30 days for use as an additional source of make-up process water.

NMCC recognizes, as the NMED has noted, that there is an area along the western edge of developed WS D, south of EWRSP-1 that could drain into the Grayback Arroyo instead of the mine pit. As discussed later herein, NMCC has corrected this condition by repairing the breach to re-establish the integrity of the open pit surface drainage area so that all surface water runoff in WS D will flow into the mine pit. EWRSP-1 will be reclaimed early in the operation such that upon reclamation this area will drain to Grayback Arroyo

Similarly, NMCC has determined that the northernmost edge of EWRSP-2A, shown on Figure 11J-2, is at the extreme northern limit of the open pit surface drainage area in developed WS B. The natural topography of the area in that location forms a natural hydrologic surface divide forming the northern edge of the open pit surface drainage area so that surface water runoff would naturally flow south. However, there is the possibility that the topographic configuration of the existing waste rock stockpile placed in that area by the previous operation is such that limited surface water runoff could flow north, out of the open pit surface drainage area. Recognizing that condition, as discussed in more detail in subsection **20.6.7.11.J.(2)**, Description of Waste Rock Stockpiles, below, NMCC's design of proposed new Waste Rock Stockpile no. 1 (WRSP-1), will correct this situation. That is, construction of proposed new WRSP-1 will envelop and subsume EWRSP-2A during operations at Copper Flat. Any existing waste rock identified to potentially be located



outside of the open pit surface drainage area will be move back into the open pit surface drainage area. WRSP-1 will be constructed over the top of EWRSP-2A early in the operations phase of the project. The design of WRSP-1, as described below, will ensure that both EWRSP-2A and WRSP-1 remain entirely within the open pit surface drainage area.

The purpose of the process water reservoir is to provide a holding structure for recycled water from the TSF, storm water from the storm water impoundments, and fresh make-up water from the off-site wells for use as process water in the concentrator.

**Liner material**

The impacted storm water impoundment liners will consist of a compacted liner bedding fill layer, overlain with a 60 mil HDPE geomembrane or equivalent, as specified in Appendix B. The liner bedding will be a minimum of six inches of sand or fine soil.

The process water reservoir will be double-lined with a lower 60 mil, or equivalent, HDPE geomembrane and a 60 mil, or equivalent, upper HDPE geomembrane liner. An HDPE geonet will be placed between the liners to serve as the seepage collection pond leak collection and recovery system (LCRS) and to minimize pressure on the lower pond liner.

**Storage or disposal capacity**

The design storage capacity for each of the impacted storm water impoundments is driven by the size of the watershed. Table 11J-1 provides the storage capacity for each storm water impoundment and the process water reservoir.

TABLE 11J-1 Impoundment Storage Capacity		
Impoundment	Size (Acres)	Capacity (Gal)
Impacted storm water impoundment A	1.98	7,306,464
Impacted storm water impoundment B	2.12	5,513,140
Impacted storm water impoundment C	6.37	10,513,140
Process Water Reservoir	1.8	5,433,472

Design capacity of the impoundments is based on anticipated normal operating conditions at the site plus prevention of overflow resulting from a 100-year, 24-hour return interval storm event while maintaining two feet of freeboard. The process water reservoir is sized to contain the water that will be pumped from the water reclaim system and the underdrain collection pond at the TSF plus additional capacity for adding makeup water for the process from other sources including the freshwater off-site well field and the impacted storm water impoundments.



### **Methods proposed to prevent pollution of groundwater**

The primary method to prevent pollution of groundwater at the storm water impoundments and the process water reservoir is through the use of engineered systems to control and manage surface water runoff in combination with the use of liners. Surface water runoff up-gradient from the mine site location was diverted around the site during previous operations. Existing diversion structures will be kept in place as they eliminate the potential for contact of storm water runoff up-gradient from the site with materials on-site that may have the potential to impact ground water.

The process area and the proposed waste rock stockpiles will be graded and contoured to route storm water runoff through storm water conveyances such as drainage ditches, storm drains, culverts and swales to direct and capture storm water in the impacted storm water Impoundments. The impoundments are designed to store the captured water from a 100 year storm with 2 feet of freeboard. As described above, the impoundments will be lined with a 60 mil HPDE liner or equivalent. Captured water will be held in the impacted storm water impoundments for less than thirty days and will be pumped to the process water reservoir for use as process make-up water. Each impacted storm water impoundment will be designed to have a spillway per the requirements of Section **20.6.7.17.D.(7)** of the Copper Rule.

The process water reservoir will be designed such that only precipitation that falls directly upon the footprint of the reservoir will be captured by it. Surface water runoff from the area will be routed to Impacted Storm Water Impoundment A. The process water reservoir will be double-lined with a 60 mil HDPE geomembranes, or equivalent, and be equipped with a leak collection and recovery system, to protect groundwater.

In addition to the mitigation measures identified above to protect groundwater, all of the pipelines that transport tailings, impacted storm water or reclaimed water will be conveyed within lined channels designed to direct leaks and spills to impoundments so as to protect ground water.

NMCC's significant proactive approach to maximally and proactively managing and recycling water, in and of itself, provides a means of protecting groundwater. Maximally recycling water from the TSF and harvesting water from waste rock stockpiles lessens the pressure on the hydrologic cycle by conserving as much water as possible.

#### **IV. 20.6.7.11.J.(2) DESCRIPTION OF WASTE ROCK STORAGE PILES**

NMCC proposes to construct three new waste rock stockpiles (WRSP) in conjunction with operation of its Copper Flat project. The material contained in these stockpiles will meet the definition of "waste rock" as stated in Section **20.6.7.B.(65)**, i.e., "all material excavated from a mine facility that is not ore or clean topsoil."



Ore is an economic term. That is, mineralized material only becomes ore when it can be economically processed to extract the commodity to sell at a profit. As such, the proposed new waste rock stockpiles, i.e., material not deemed to be ore by NMCC, will be constructed as units segregated by the grade of copper contained in the waste rock material so as to maximize its potential to be processed as ore in the future. New proposed Waste Rock Stockpile no. 1 (WRSP 1) will contain the highest non-ore grade material. New proposed Waste Rock Stockpile no. 2 (WRSP 2) will contain the next highest non-grade material. New proposed Waste Rock Stockpile no. 3 (WRSP 3) will contain all the remaining material.

The proposed waste rock stockpiles will be built generally to a configuration of 3 horizontal to 1 vertical slope angles (18.4 degrees). This configuration will help facilitate reclamation at the end of the mine life as provided for in Section **20.6.7.33.C.(3)**. Each lift within the stockpile will be maximally 75 ft. high and be placed at angle of repose (35.54 degrees) with 120 ft. setbacks left between lifts to maintain the 3 to 1 overall angle for the stockpile. Surface water run-off collection trenches will be constructed, as needed, to collect and route run-off and/or flowing seeps from the proposed stockpiles to the storm water impoundments describe above. These trenches will be constructed in a manner to maximizing positive flow while minimizing the potential for ponding and erosion.

As shown in Figure 11J-2, there are also four existing waste rock stockpiles from previous operations at the site. They are;

- EWRSP-1 located at the western end of the site within the open pit surface drainage area. EWRSP-1 is identified in previous reports as the “existing west waste rock disposal facility” (WRDF);
- EWRSP-2A and 2B located at the northwest side of the site within the open pit surface drainage area. EWRSP-2A and B are identified in previous reports as the “existing north waste rock disposal facility” (WRDF);
- EWRSP-3 located next to the primary crusher. EWRSP-3 is identified in previous documents as the “low grade ore stockpile”; and
- EWRSP-4 located southeast of the mine. EWRSP-4 is identified in previous documents alternatively as the “lean ore stockpile” and the “south waste disposal facility”.

EWRSP-1 and EWRSP-2B will be reclaimed as discussed in more detail later herein in conformance with an approved MORP and Closure Plan. EWRSP-2A will be incorporated into the larger new proposed WRSP-1 constructed during operations. It will be enveloped and covered over by the new stockpile as WRSP-2 being developed over time.

EWRSP-3 is located to the east of the primary crusher as shown on Figure 11J-2. Some of this material will be fed into the processing circuit in the early stages of process operations to “condition” the circuit. “Conditioning” refers to the process of feeding new equipment, first with water, followed by some waste rock and then to “line” the new machinery and



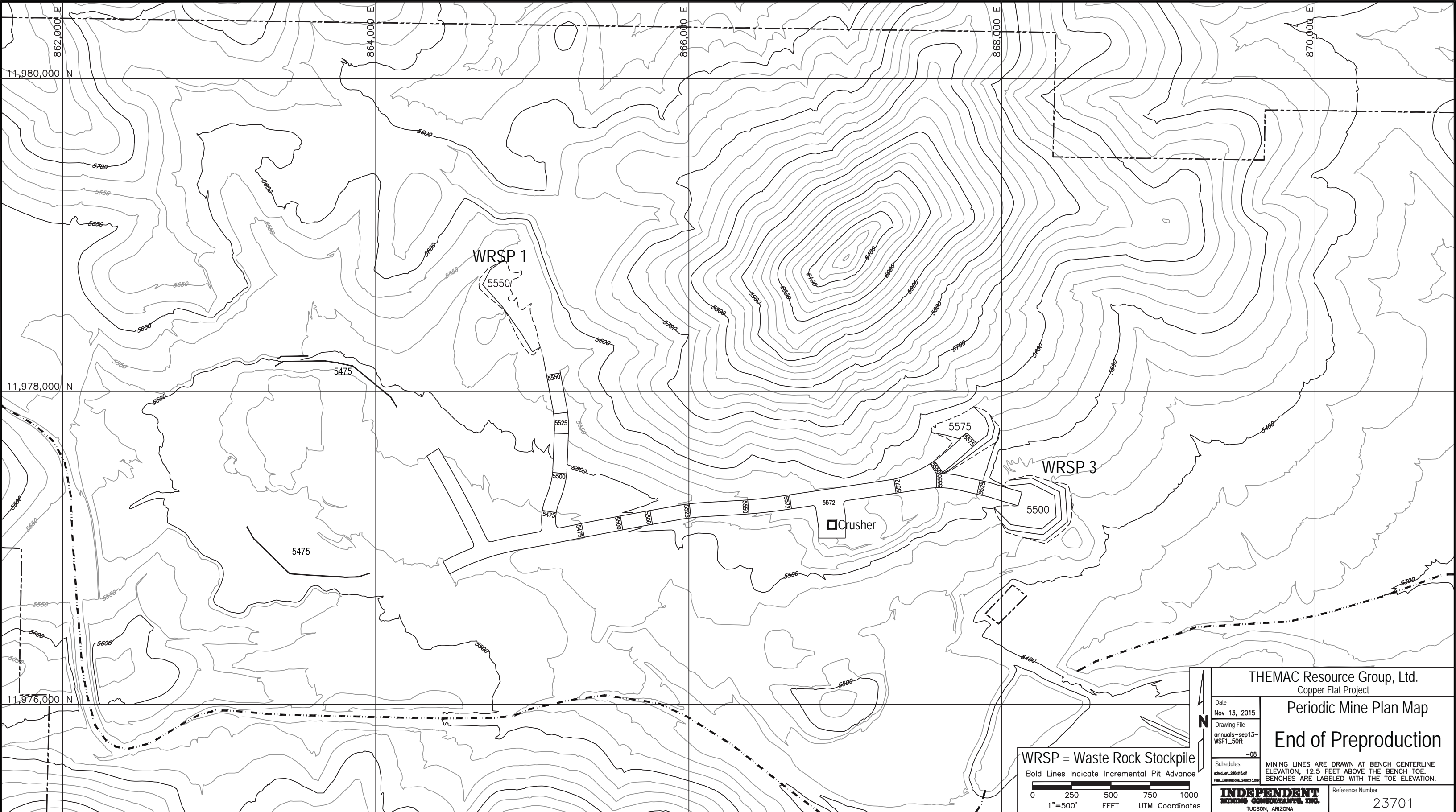
prepare it for full operations. The material at EWRSP-3 may also be blended over time into the process if economic conditions warrant. The area containing EWRSP-3 will be the staging or holding area for rock that is too large to be fed directly into the crusher that has to be broken down with a pneumatic hammer.

EWRSP-4 will be graded and re-contoured as discussed later herein to direct surface water flow to the mine pit. The southern edge of the stockpile will be reclaimed in conformance with the approved MORP and Closure Plan during operations to protect against potential surface water impacts to Grayback Arroyo from the EWRSP. The graded and contoured area will be utilized during operations as an additional equipment storage and lay-down area and will be reclaimed at the end of the mine life in accordance with the reclamation and closure plan.

This section, together with section **20.6.7.11.N.(1), Engineering, Design, Construction and Surveying, Waste Rock Stockpiles, 20.6.7.11.O, Material Characterization and Material Handling Plan, and 20.6.7.11.P, Hydrologic Conceptual Model**, of NMCC's Discharge Permit application, contain the design plans for the proposed new waste rock stockpiles (WRSPs) located outside the open pit surface drainage area pursuant to **20.6.7.21.B.(1).(d)**. The proposed areal extent and configuration of these new proposed WRSPs and the topography of the site where they will be located is shown in Figures 11J-4 through 11J-15. The geology of the site is described in detail later in this application in section **20.6.7.11.K, Soils, Geology and Hydrology**.

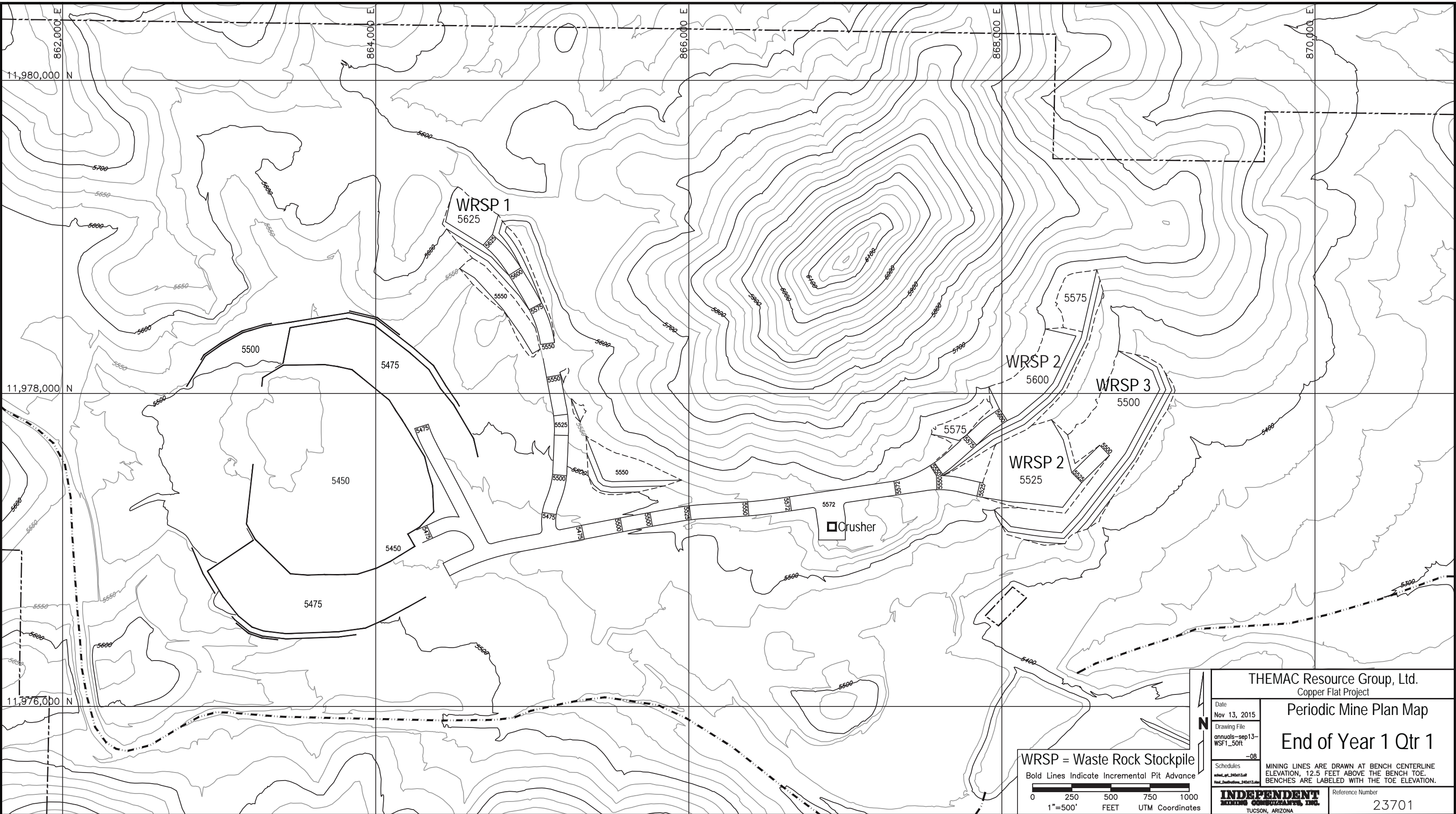
Figures 11J-4 through 11J-15 show the design and construction sequencing for the mine pit and the new proposed WRSPs beginning at the preproduction stage through the life of the mine. Their design and configuration is based on consideration of site-specific conditions as discussed in more detail below and in the sections of the application cited above. For example, the material characterization work performed by NMCC as described in section **20.6.7.11.O** that follows provides the basis for controlling surface run-off and seepage from the stockpiles using run-off collection trenches to capture it and routing it to impacted surface water impoundments. Similarly, the information presented in sections **20.6.7.11.K and P** regarding site geology and hydrology conditions provides the basis for placing the waste rock stockpiles over low permeability andesite bedrock.

Section **20.6.7.11.J.(6)** of this application provides a discussion and analysis of the storm water diversion structures present at the site that will minimize the contact between storm water run-on and the waste rock material.



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13-WSF1_50ft	End of Preproduction
Schedules mining-annuals-sep13-WSF1_50ft	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

**Figure 11J-4**

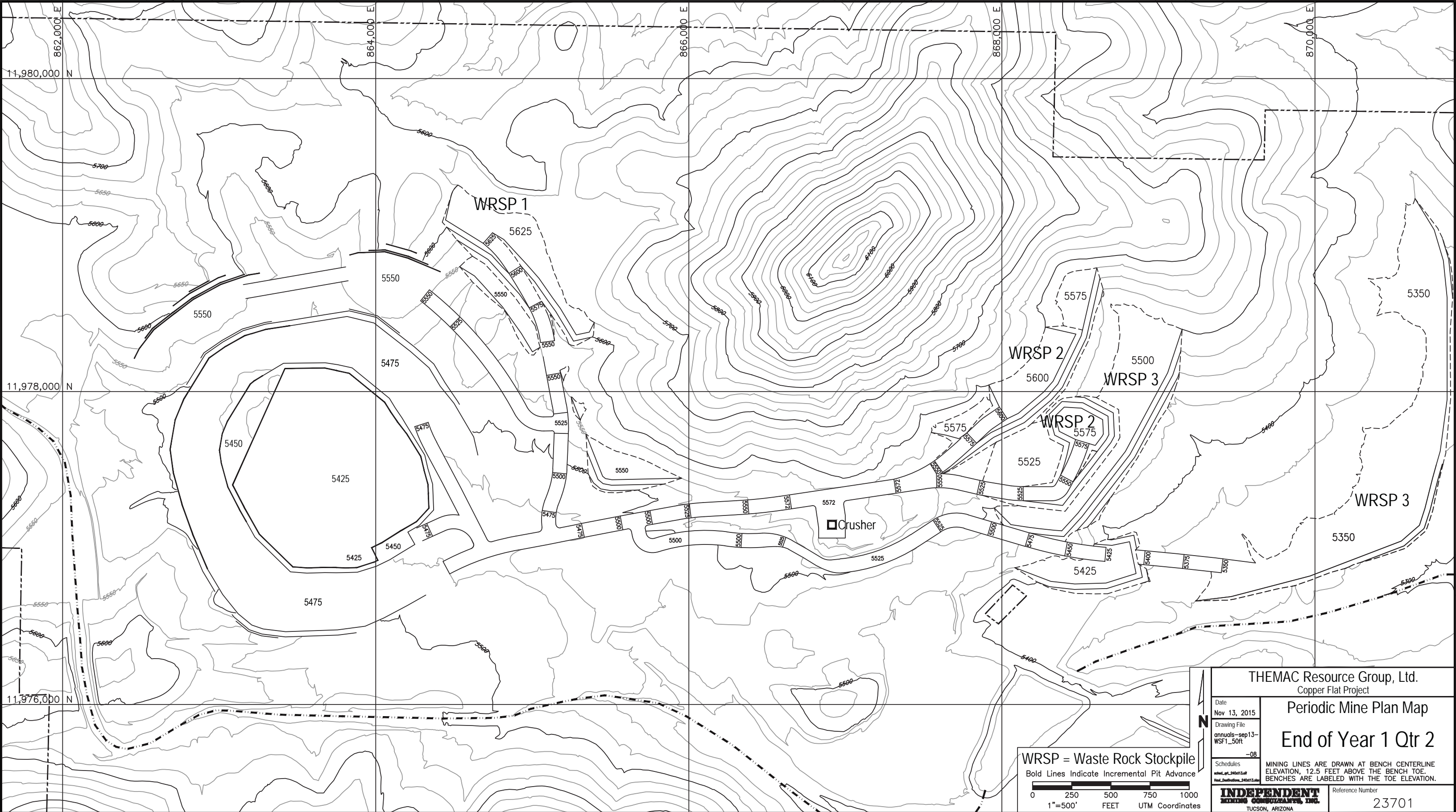


WRSP = Waste Rock Stockpile  
Bold Lines Indicate Incremental Pit Advance



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 1 Qtr 1
Schedules mining-comp13- mining-comp13- mining-comp13- mining-comp13-	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

**Figure 11J-5** Page 51



THEMAC Resource Group, Ltd.  
Copper Flat Project

Periodic Mine Plan Map  
End of Year 1 Qtr 2

Date  
Nov 13, 2015  
Drawing File  
annuals-sep13-  
WSF1\_50ft

Schedules  
mining-comp13-  
TUCSON, ARIZONA

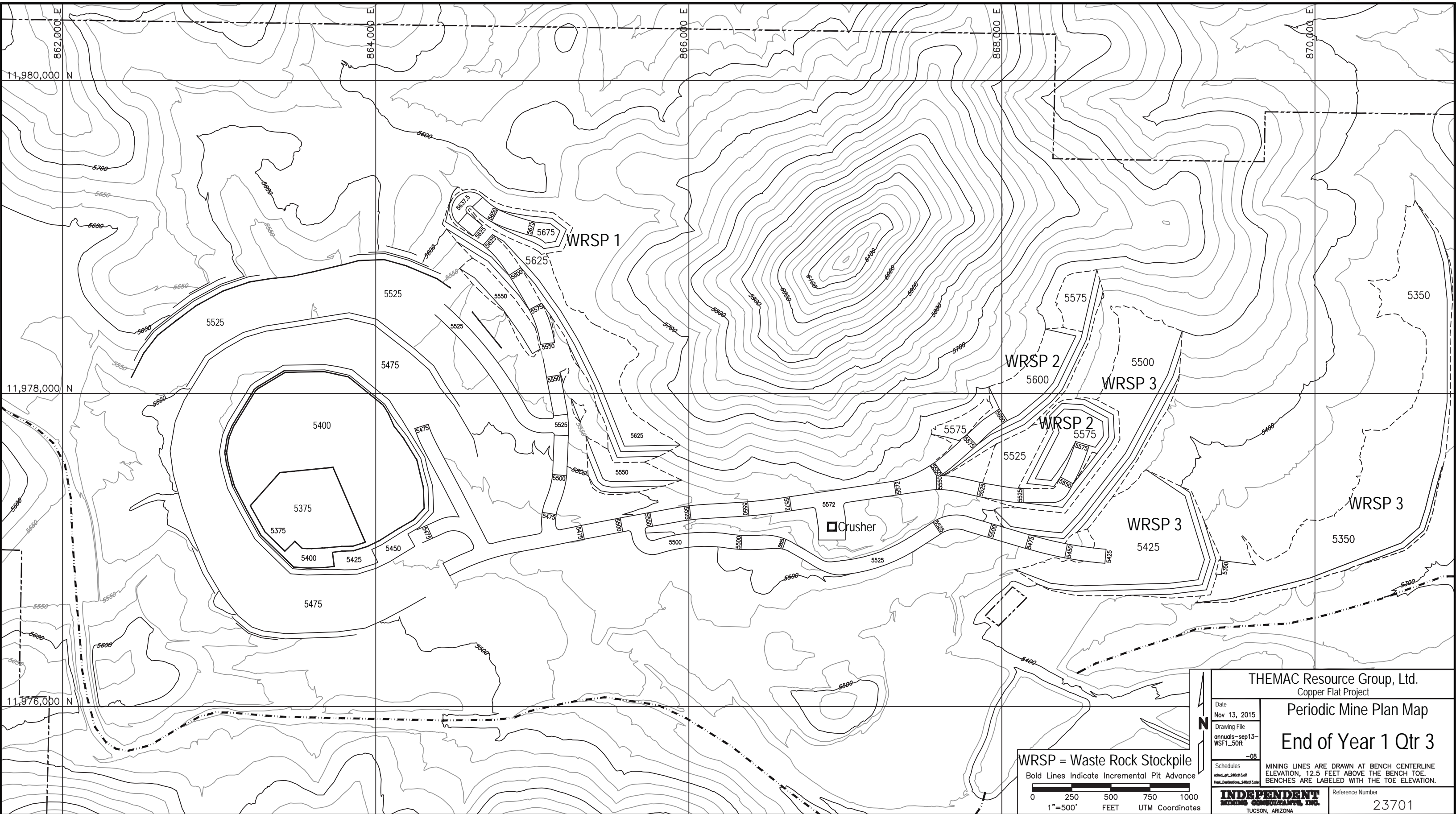
MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.

**INDEPENDENT**  
MINING CONSULTANTS, INC.  
TUCSON, ARIZONA

Reference Number  
23701

Figure 11J-6



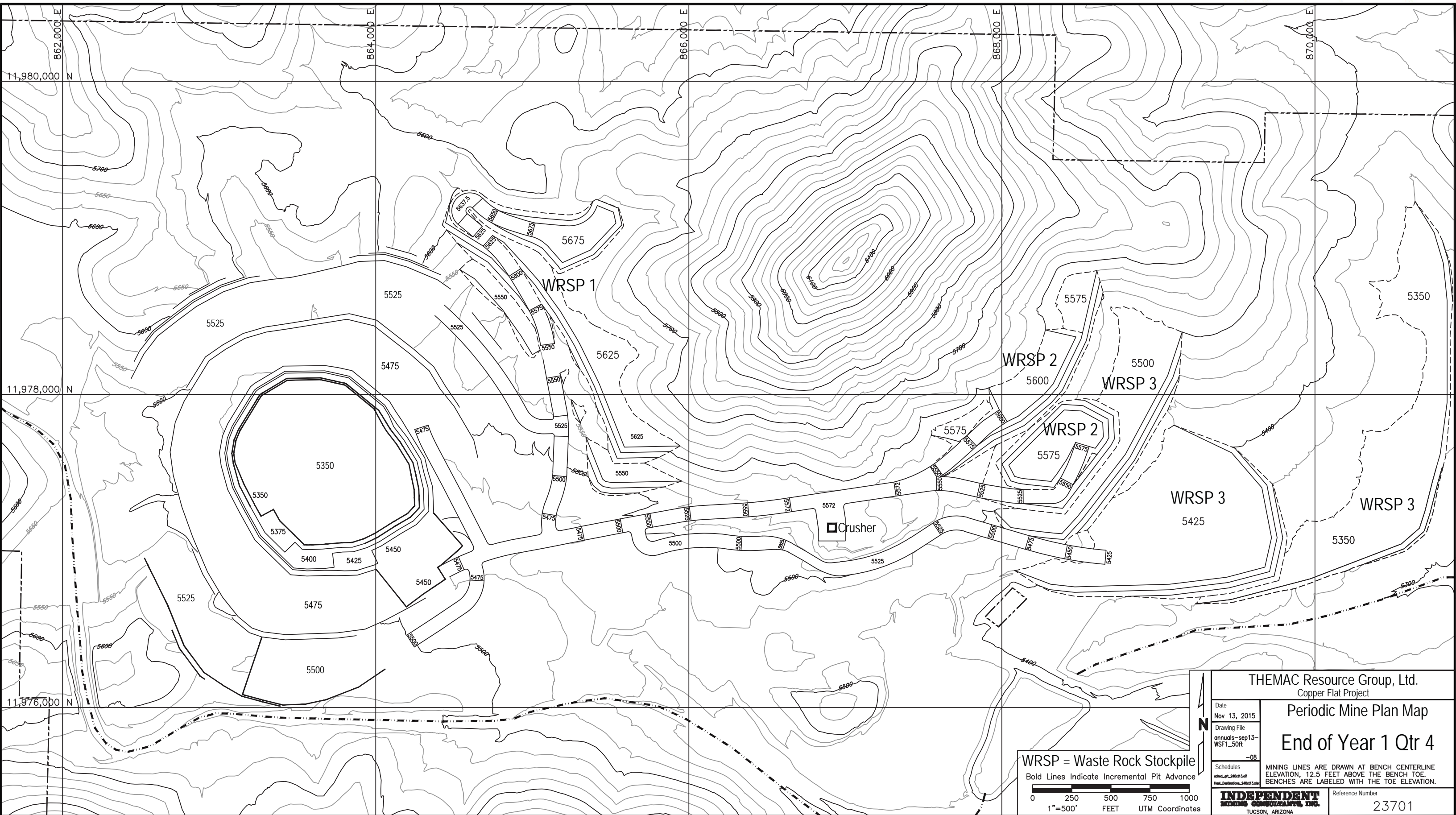


WRSP = Waste Rock Stockpile  
 Bold Lines Indicate Incremental Pit Advance



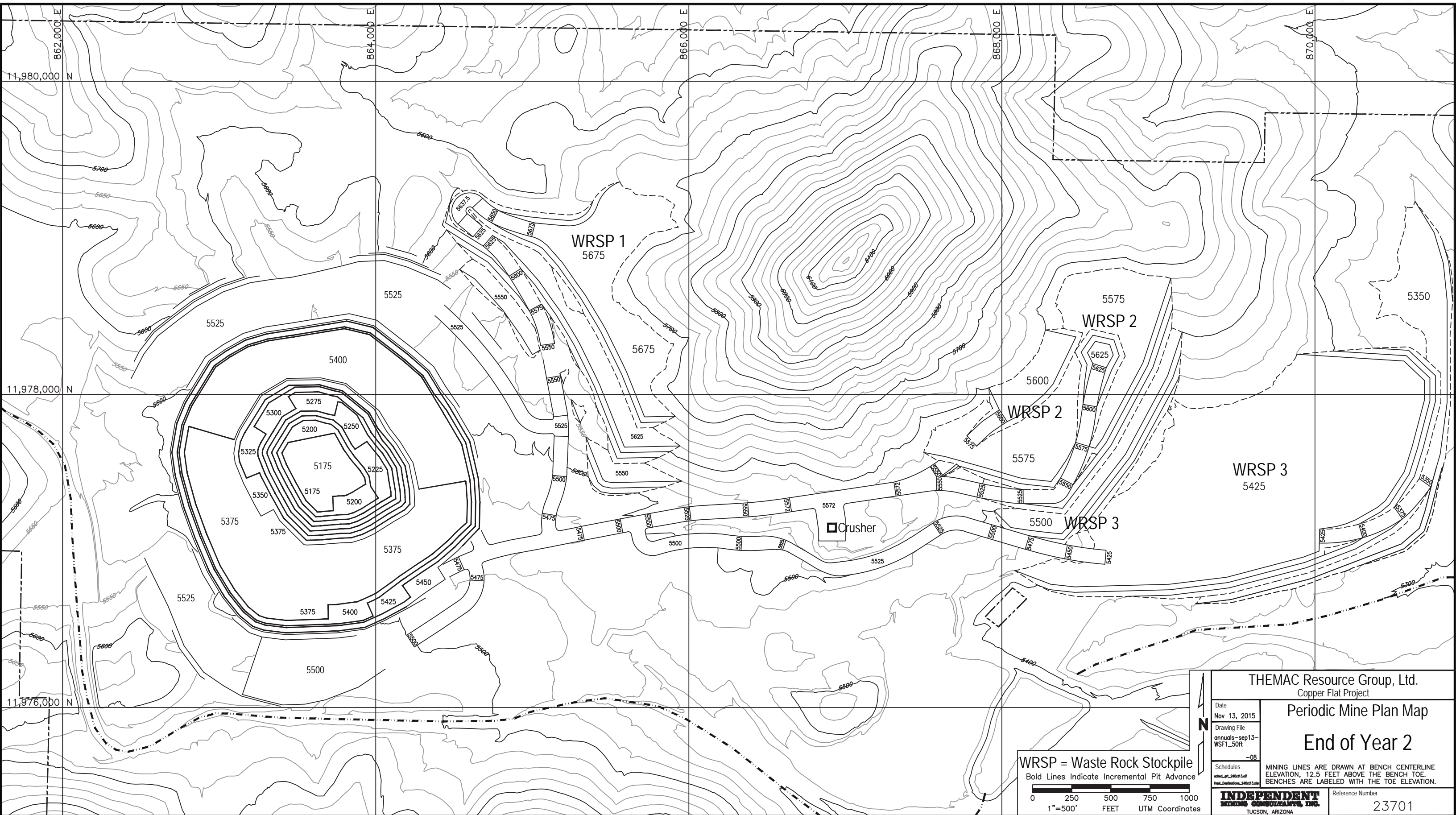
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map End of Year 1 Qtr 3
Drawing File annuals-sep13- WSF1_50ft	
Schedules mining-compliance_2015	Mining Lines are drawn at bench centerline elevation, 12.5 feet above the bench toe. Benches are labeled with the toe elevation.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

Figure 11J-7



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 1 Qtr 4
Schedules mining-compliance-2013	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

**Figure 11J-8** Page 54

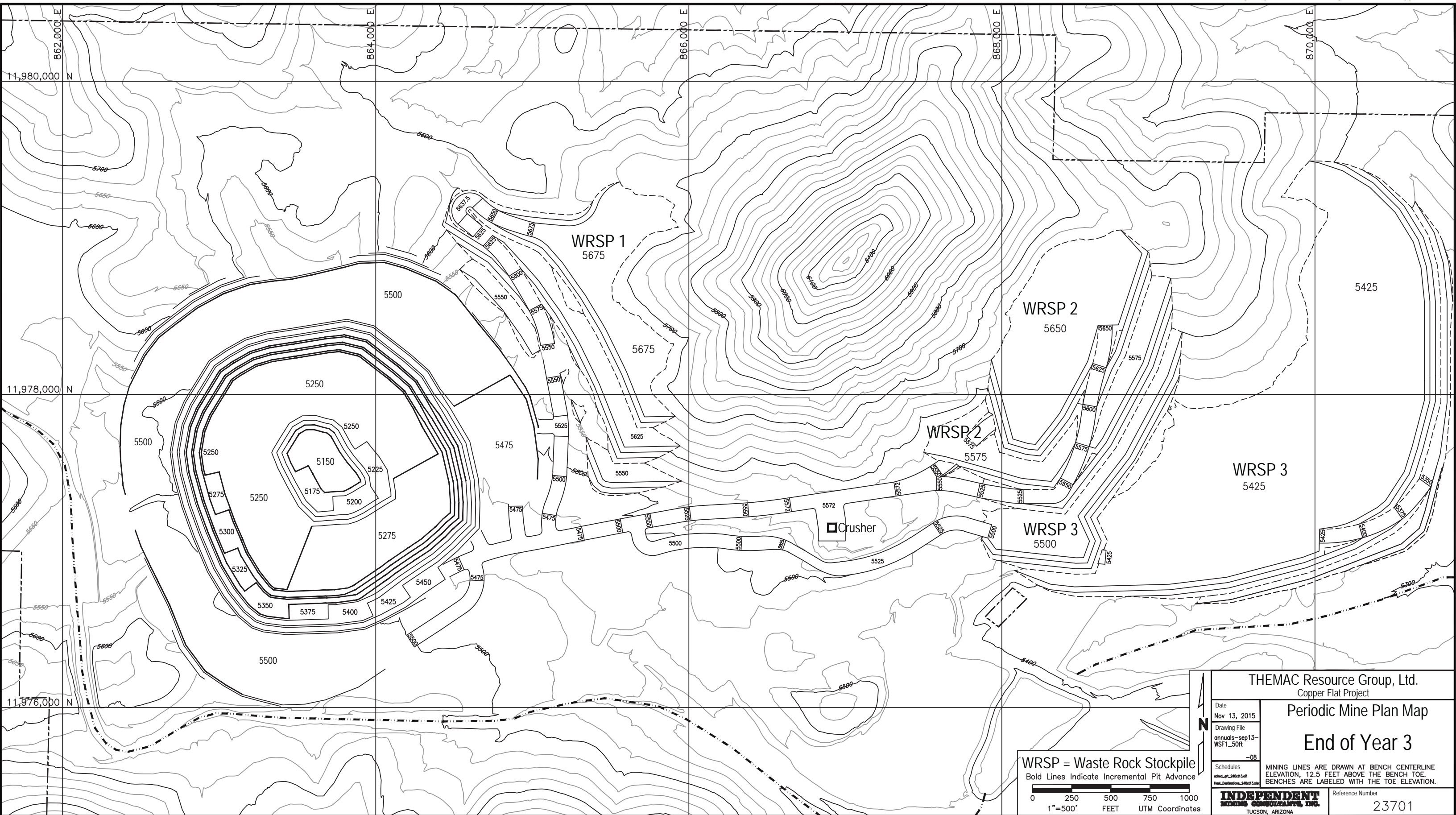


WRSP = Waste Rock Stockpile  
 Bold Lines Indicate Incremental Pit Advance

0 250 500 750 1000  
 1"=500' FEET UTM Coordinates

THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	<b>Periodic Mine Plan Map</b>  <b>End of Year 2</b>
Drawing File annuals-sep13-WSF1_50ft	
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number <b>23701</b>	

**Figure 11J-9** Page 55

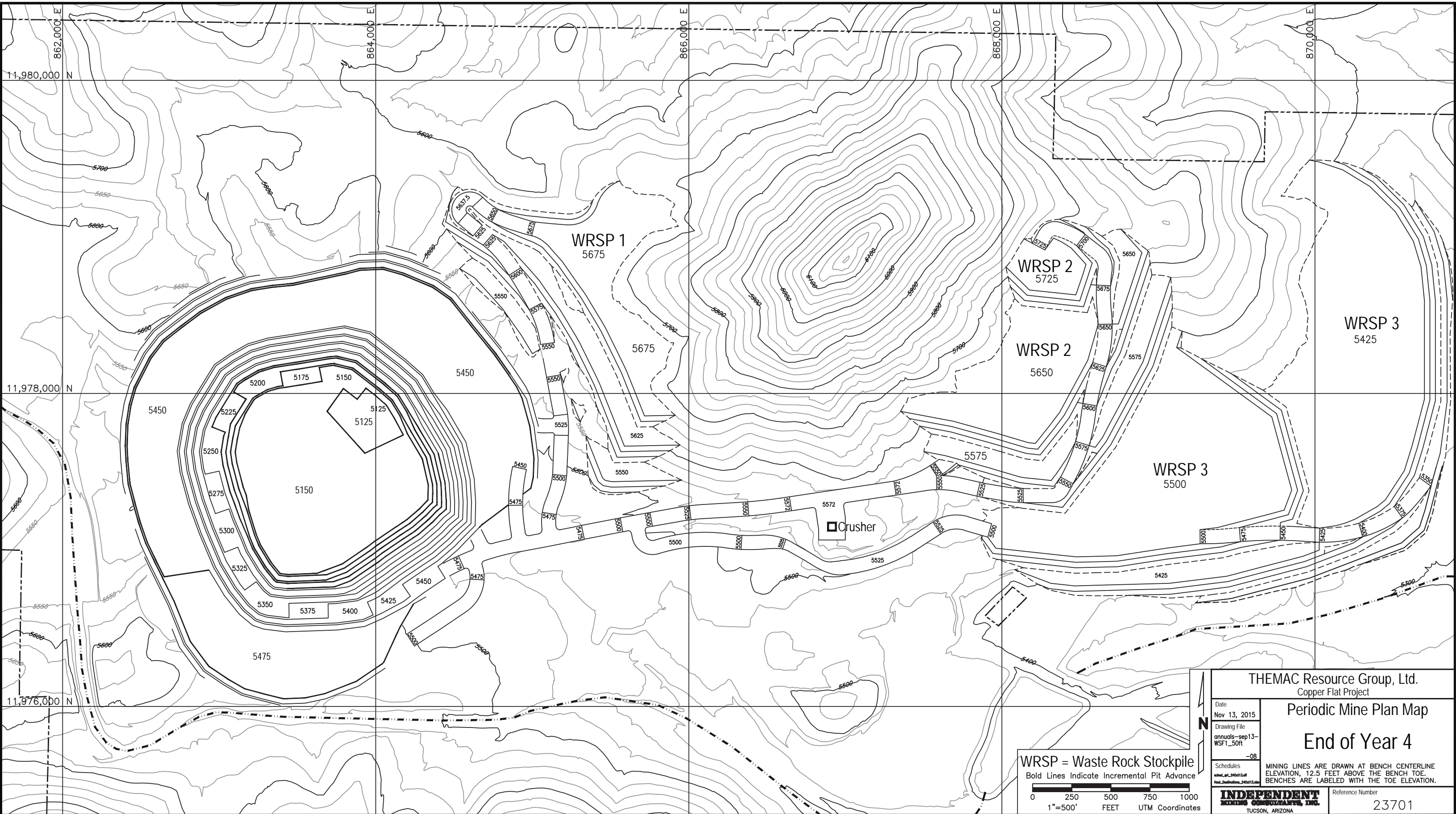


WRSP = Waste Rock Stockpile  
Bold Lines Indicate Incremental Pit Advance

0 250 500 750 1000  
1"=500' FEET UTM Coordinates

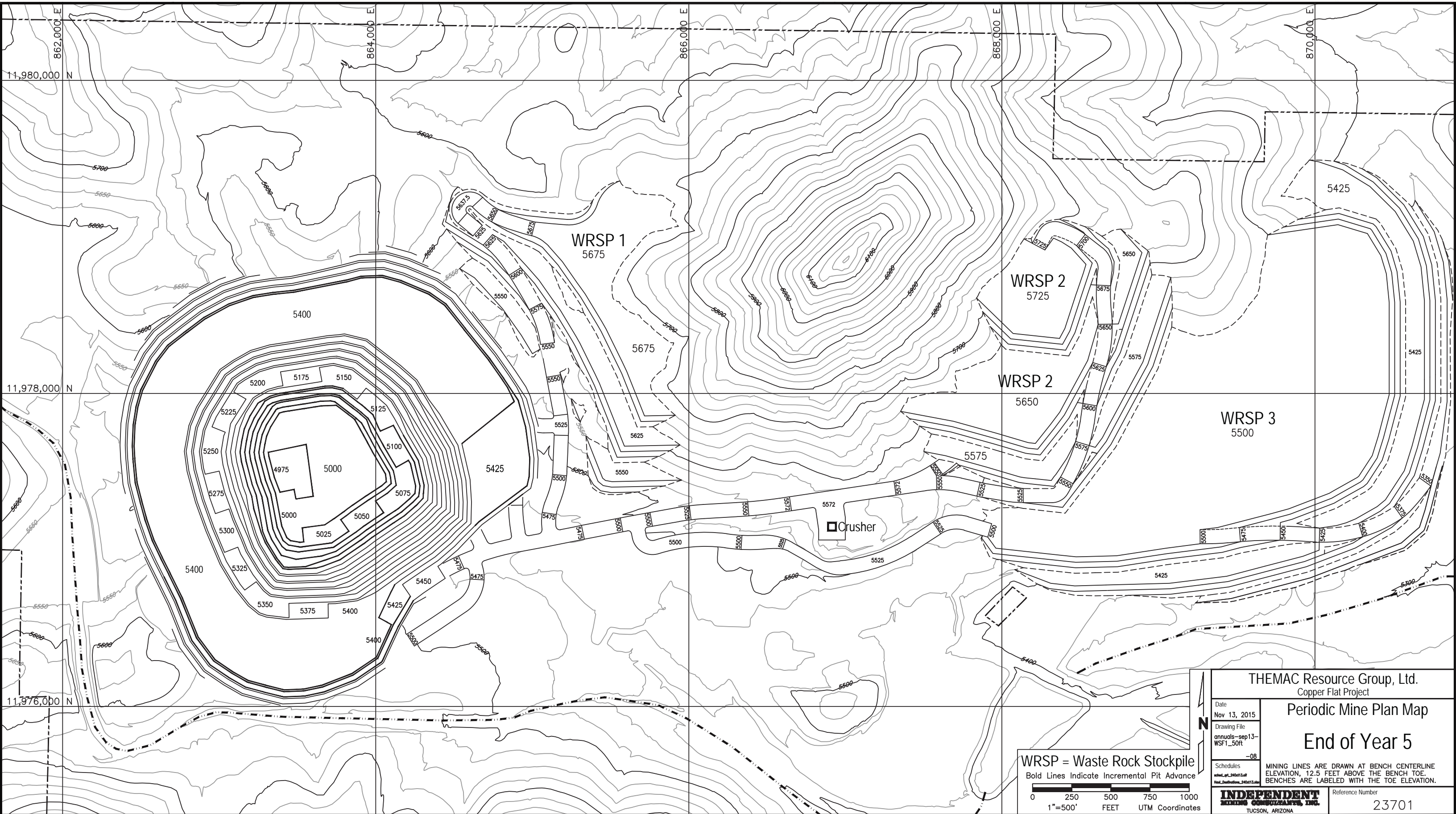
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 3
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number 23701

**Figure 11J-10** Page 56



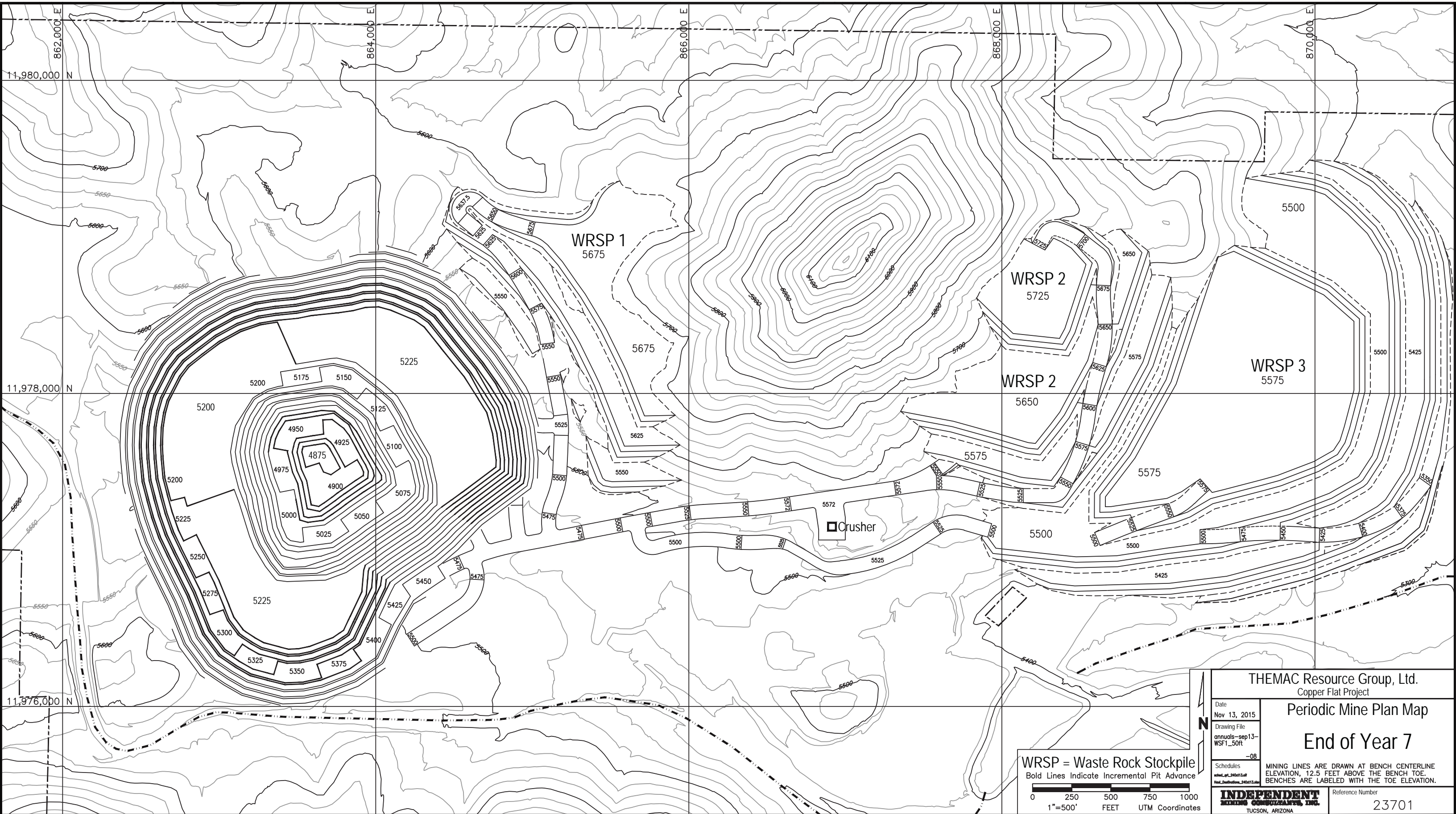
THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 4
Schedules mining-comp13- 13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number <b>23701</b>

**Figure 11J-11** Page 57



THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map
Drawing File annuals-sep13- WSF1_50ft	End of Year 5
Schedules mining-comp13- 2015	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	Reference Number <b>23701</b>

**Figure 11J-12** Page 58



THEMAC Resource Group, Ltd.  
Copper Flat Project

Periodic Mine Plan Map

End of Year 7

Date  
Nov 13, 2015

Drawing File  
annuals-sep13-  
WSF1\_50ft

Schedules  
-08

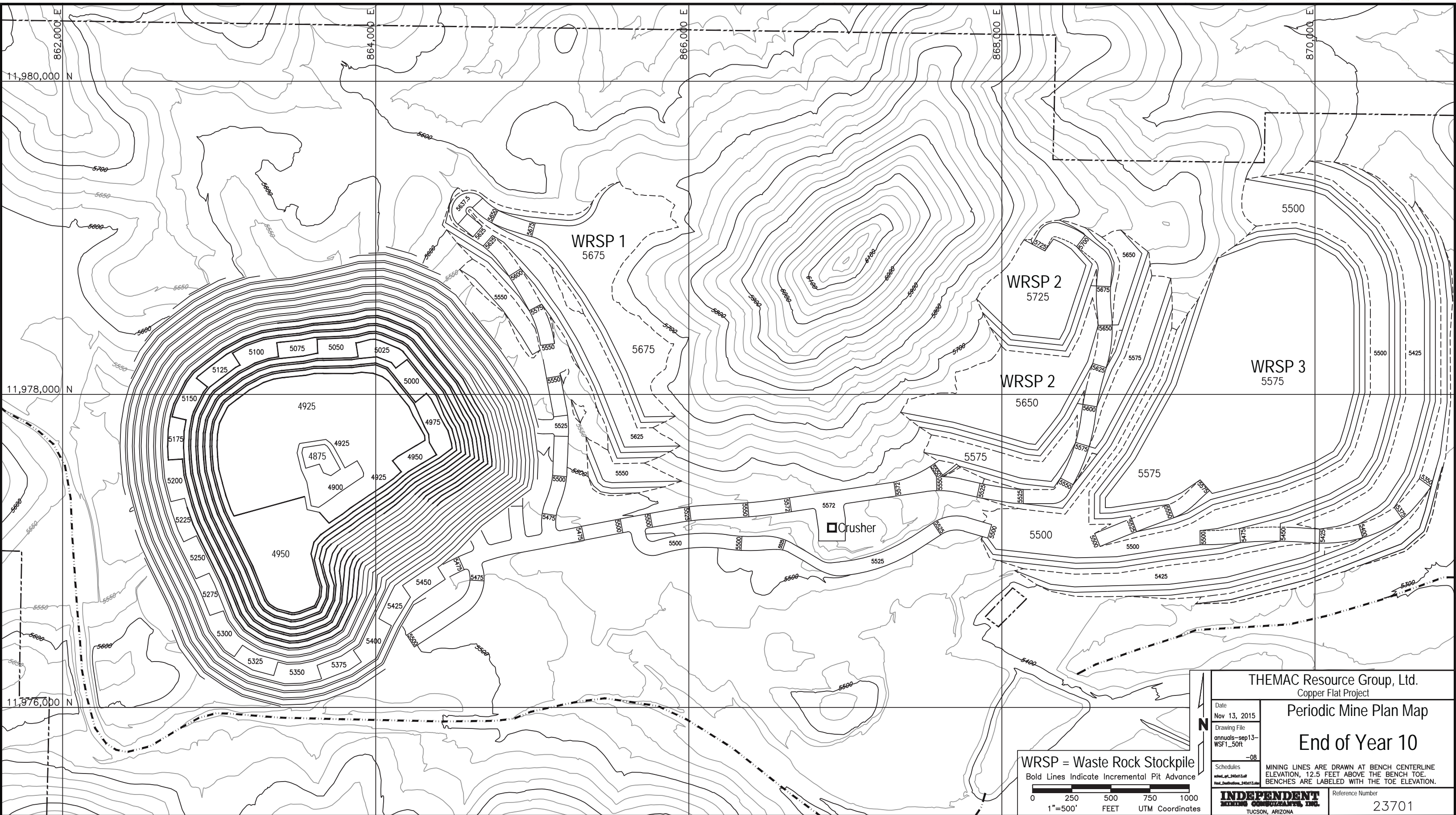
MINING LINES ARE DRAWN AT BENCH CENTERLINE  
ELEVATION, 12.5 FEET ABOVE THE BENCH TOE.  
BENCHES ARE LABELED WITH THE TOE ELEVATION.

**INDEPENDENT**  
MINING CONSULTANTS, INC.  
TUCSON, ARIZONA

Reference Number  
23701

WRSP = Waste Rock Stockpile  
Bold Lines Indicate Incremental Pit Advance





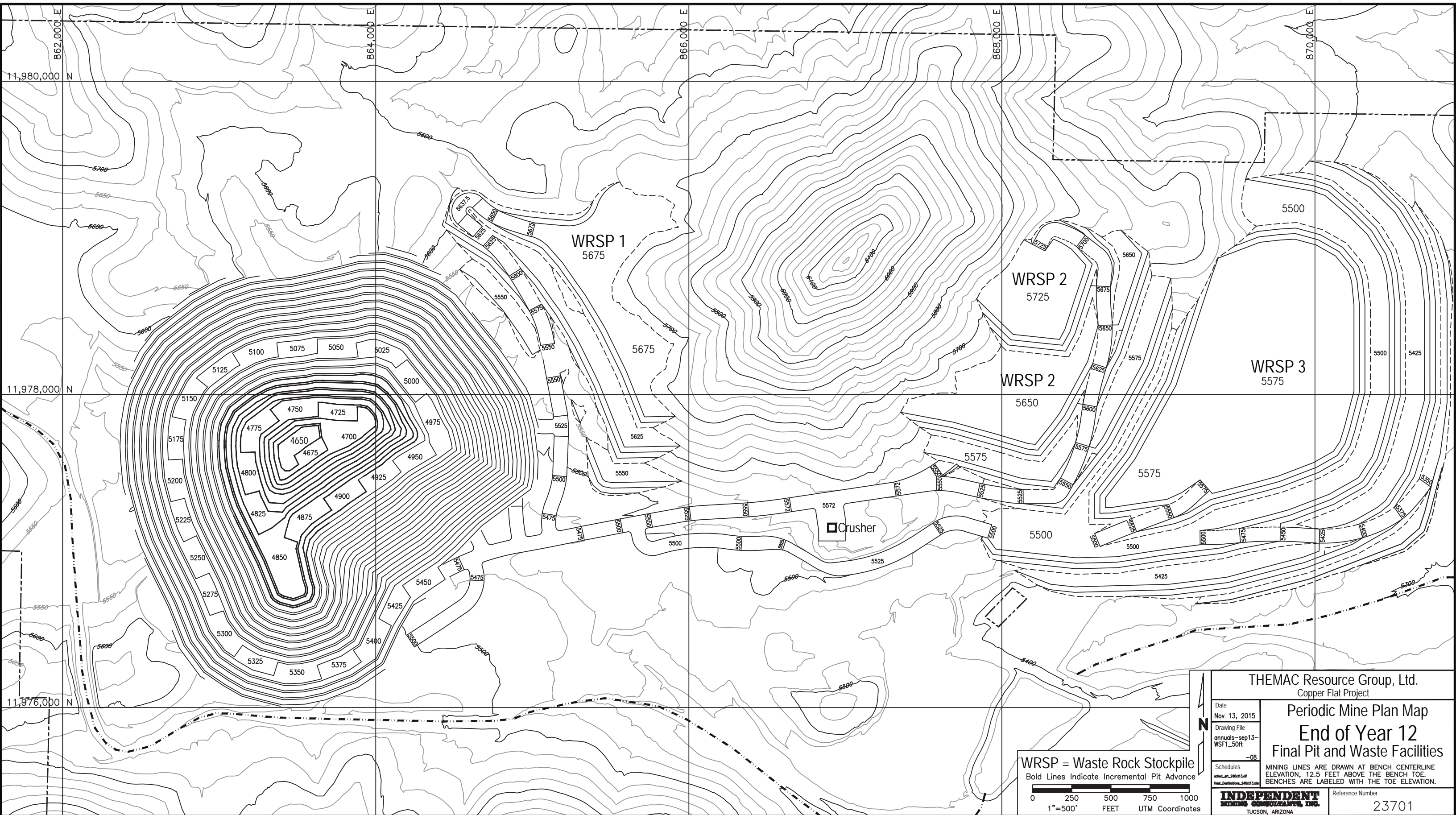
WRSP = Waste Rock Stockpile  
 Bold Lines Indicate Incremental Pit Advance

0 250 500 750 1000  
 1"=500' FEET UTM Coordinates

THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	Periodic Mine Plan Map  End of Year 10
Drawing File annuals-sep13- WSF1_50ft	
Schedules mining-comp13-08	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
INDEPENDENT MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number 23701	

**Figure 11J-14** Page 60





THEMAC Resource Group, Ltd. Copper Flat Project	
Date Nov 13, 2015	<b>Periodic Mine Plan Map</b> <b>End of Year 12</b> <b>Final Pit and Waste Facilities</b>
Drawing File annuals-sep13- WSF1_50ft	
Schedules mining-comp12a13.dwg mining-comp12a13.dwg	MINING LINES ARE DRAWN AT BENCH CENTERLINE ELEVATION, 12.5 FEET ABOVE THE BENCH TOE. BENCHES ARE LABELED WITH THE TOE ELEVATION.
<b>INDEPENDENT</b> MINING CONSULTANTS, INC. TUCSON, ARIZONA	
Reference Number	23701

**Figure 11J-15** Page 61



## Location

The location of the existing and proposed new waste rock stockpiles is shown in Figure 11J-1 provided herein in response to **20.6.7.11.J.(1)** and Figures 11J-2 and 3. Figures 11J-4 through 15 provide additional design detail of how the proposed new WRSP's will be constructed over time.

- Proposed new WRSP 1 will be located along the western edge of developed watershed (WS) B;
- Proposed new WRSP 2 will be located in the western third of developed WS C;
- Proposed new WRSP 3 will be located in the remainder of developed WS C;
- EWRSP-1 is located at the western end of the site within the open pit surface drainage area. EWRSP-1 is identified in previous reports as the “existing west waste rock disposal facility” (WRDF);
- EWRSP-2A and 2B are located at the northwest side of the site within the open pit surface drainage area. EWRSP-2A and B are identified in previous reports as the “existing north waste rock disposal facility” (WRDF);
- EWRSP-3 is located next to the primary crusher in WS A. EWRSP-3 is identified in previous documents as the low grade or stockpile; and
- EWRSP-4 located southeast of the mine pit. EWRSP-4 is identified in previous documents alternatively as the “lean ore stockpile” and the south waste disposal facility.

## Purpose

The purpose of the waste rock stockpiles is to store the all of material excavated from the mine that is not ore or clean topsoil, i.e., growth media, in conformance with the definition of “waste rock” in **20.6.7.7.B.(65)**.

## Liner material

The proposed new stockpiles will be constructed over andesite bedrock, a very low permeability formation that provides a natural liner protective of groundwater. Andesite at the site has a permeability of less than  $10^{-6}$  centimeters per second (cm/sec) (SRK, May 2013). Alluvial fans may exist, such as may be the case underlying a portion of proposed WRSP-3, as indicated in the New Mexico Bureau of Geology Draft Open-File Map 242 (Jochems, et al., 2014). The stormwater collection channels constructed along the toe of the new proposed stockpiles will also serve as collection galleries for potential seepage that may occur along the interface between the alluvial material and the andesite. Alluvial materials that may exist along the toe of the stockpiles will be removed and the collection channels constructed into the andesite. The channels will follow the land surface contours for positive drainage to the lined impacted stormwater impoundments. NMCC also proposes monitoring wells directly down-gradient of the channels in the drainages as discussed in Appendix E of the Discharge Plan application to monitor for any potential discharges.



Draft Open-file Map 242 indicates fan deposits (Qaf1 and Qaf2) exist in the area of proposed WRSP-3 at an estimated maximal thickness of approximately 3 to 4 meters. However, there are no drill data or measured sections to support these estimates. As such, a field reconnaissance was performed by JSAI on May 25, 2016 to correlate the information mapped in Draft Open-file Map 242 and site conditions. JSAI concluded, based on visual observation, that the thickness of the alluvial cover in that area is a minimal thin veneer overlying the andesite, not a wedge-shaped mass of alluvium thickening towards Grayback Arroyo as might be assumed from simply interpreting the map. Looking at the location of proposed WRSP-3 from the entrance road along the south side of Grayback Arroyo to the north toward the location, JSAI observed outcrops of andesite visible on the north side of Grayback Arroyo that were not mapped by Jochems et al (2014). Exposure of andesite was also seen in a small drainage channel near or at the toe of the proposed WRSP-3, also not mapped by Jochems et al. (2014). These observed andesite outcrops in the area of mapped units Qaf1 and Qaf2 (Jochems et al., 2014) indicate that the alluvial fans are much thinner than the map tends to indicate.

SRK Consulting (U.S.), Inc. conducted a mine waste characterization program for the Copper Flat project. SRK's waste characterization investigations were undertaken in the midst of formulation of the Copper Rules and the report was submitted in advance of their promulgation in December 2013. Nonetheless, every effort was made to conduct the investigation in a manner consistent with Section **20.6.7.21.A.(1).(a), (b), (c) and (d)** of the Copper Rules. Further, the results of the testing demonstrated that the waste rock produced at Copper Flat will not be acid generating or generate a leachate containing water contaminants. The results of SRK's investigations have been previously presented to NMED (SRK May 2013). Notwithstanding SRK's conclusions that the waste materials at Copper Flat are acid generating, SRK's report also concluded that migration of seepage from the waste rock stockpiles is not expected as the stockpiles will be placed on low permeability ( $<10^{-6}$  cm/sec) andesite bedrock which will function as a liner (SRK, May 2013, page 79). NMCC believes that this low permeability condition constitutes a "natural liner system" underlying materials that have been and will be placed outside of the open pit drainage area.

NMED and NMCC have worked closely in the intervening time through meetings, written comments, responses, and comment resolution to ensure that the characterization conducted by SRK satisfies those requirements. As such, this Geochemical Characterization Report, the Geochemical Modeling Report of Pit Lake Water Quality and associated documents for the Copper Flat project are incorporated into this Discharge Plan application by Reference. In correspondence in this regard in a February 23, 2015 letter from NMED to NMCC wherein NMED recommends that the results of the geochemical characterization work be incorporated into the Discharge Permit application (NMED 2105).

The geochemical testing of mine waste rock provided the characterization required to evaluate the potential for it to generate acid and to release contaminants in excess of the standards of **20.6.2.3103** NMAC. This information, in turn, allowed for a quantitative risk



assessment and evaluation of the options for design, construction and closure of the tailings and waste rock stockpiles.

### **Storage or disposal capacity**

The storage or disposal capacity of the proposed new waste rock stockpiles is as follows;

- Proposed new WRSP-1 is designed to stockpile up to 3.16 million tons of material over the life of the mine.
- Proposed new WRSP-2 is designed to stockpile up to 8.64 million tons of material over the life of the mine.
- Proposed new WRSP-3 is designed to stockpile up to 32.89 million tons of material over the life of the mine.

The existing waste rock stockpiles have the following approximate amount of material associated with them;

- EWRSP-1 contains approximately 512,000 tons of material.
- EWRSP-2A and 2B contain approximately 913,000 tons of material. About half of this material, i.e., EWRSP-2A, will be enveloped into proposed new WRSP-1.
- EWRSP-3 contains approximately 523,000 tons of material. Approximately 123,000 tons of this material consists of unprocessed ore remaining on-site at the end of Quintana's operations. Of this, 24,000 tons is the small amount of unprocessed run-of-mine ore, 44,000 tons of crushed ore stockpiled around the semi-autogenous (SAG) grinding mill and 55,000 tons utilized to backfill the process building foundations.
- EWRSP-4 contains approximately 1.2 million tons of material.

### **Methods proposed to prevent pollution of groundwater**

NMCC's proposed method to prevent pollution of groundwater from the proposed new and existing waste rock stockpile involves the use of natural site conditions, i.e., the low permeability of the andesite bedrock and the open pit surface drainage area in combination with the use of engineered systems to control and manage water. The location of the stockpiles themselves allows the use natural conditions to protect groundwater from the waste rock materials. The use of liners in the storm water impoundments for surface water runoff control and management at the proposed new stockpile areas during the operational life of the mine will provide a significant measure of protection of groundwater in these areas. Prevention of pollution of groundwater from the waste rock stockpiles in the long-term will be provided in the reclamation and closure phase after the end of the life of the mine by re-grading, contouring and covering the waste rock stockpiles with sufficient cover material to minimize the potential for infiltration of precipitation once the facility is reclaimed.

As indicated above, the existing waste rock stockpiles are placed over the andesite bedrock and the proposed new waste rock stockpiles will also be placed on andesite

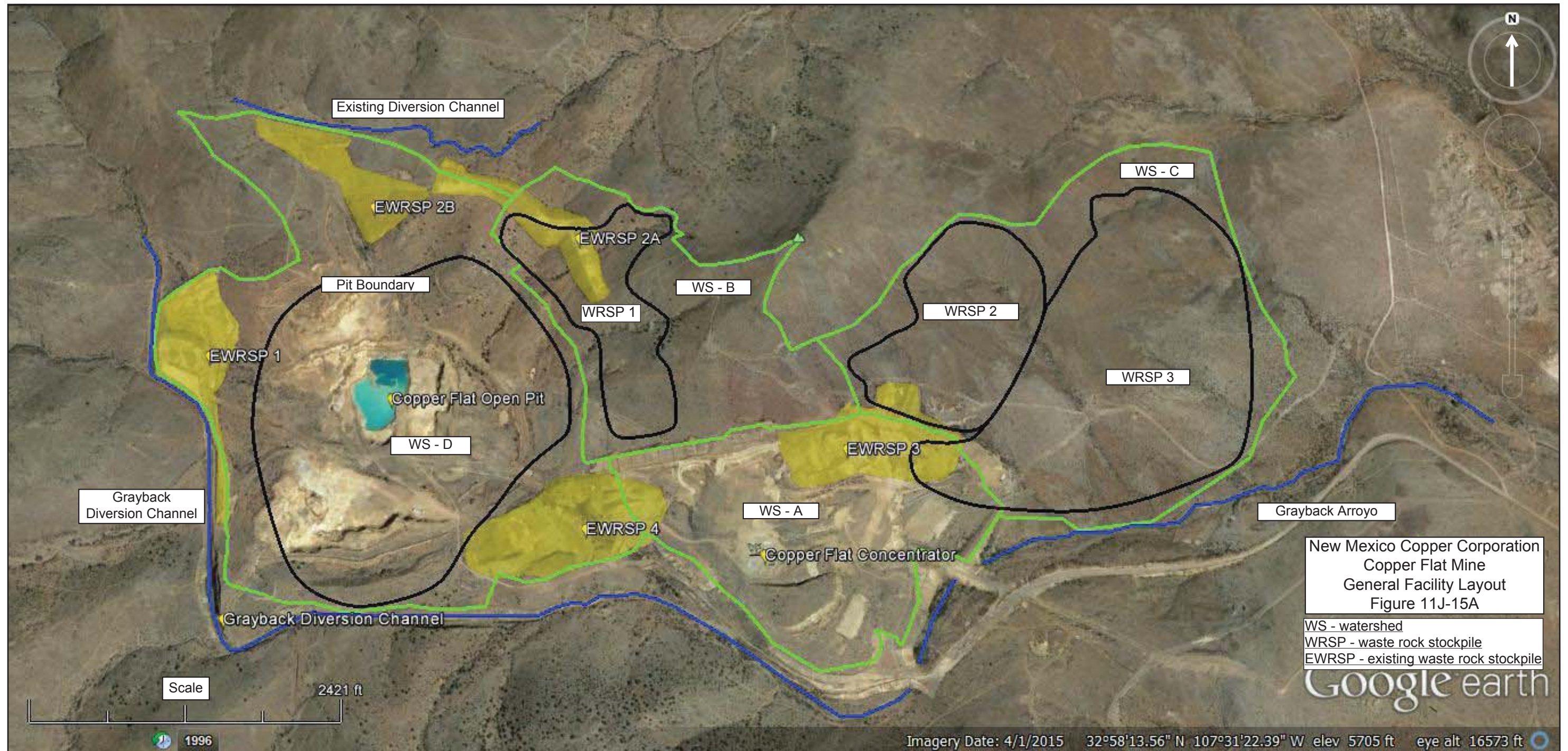


bedrock. Andesite has been demonstrated to have a very low permeability, thus providing protection of groundwater. The conclusions that SRK made in their report (SRK, May 2013) were based on the 2012 conceptual hydrologic model report prepared for NMCC by John Shoemaker and Associates, Inc., Conceptual Model of Groundwater Flow in the Animas and Palomas Basin, Copper Flat Project, Sierra County, New Mexico, May, 2012, (JSAI 2012) which has previously been submitted to NMED. NMCC has since that time also submitted to NMED two comprehensive follow-on documents titled “Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico, August 22, 2013”, and “Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico, March, 2014”. These reports confirm that the permeability of the andesite bedrock is less than  $10^{-6}$  cm/sec. and the mine pit is a hydrologic sink. The above cited documents are incorporated into NMCC’s Discharge Plan Application by reference.

Surface water runoff will be diverted away from and around the proposed new waste rock stockpiles and processing area to the maximum extent possible to minimize the potential for contact of storm water with materials that have the potential to impact ground water. Precipitation directly onto these stockpiles will be controlled and managed through construction of runoff control and conveyance structures leading to impacted storm water impoundments. As described above, Appendix B provides a detailed description of the design and construction of the impoundments and conveyance structures. The impoundments will be lined per the requirements of **20.6.7.17.D**. The impoundments are designed to capture all of the water that runs off from the WRSP from a 100 year return interval precipitation event with a minimum 2 ft. of freeboard. The water captured will be retained in the impoundments less than thirty days and transported to the process water reservoir for use in the process circuit. As described above, the process water reservoir will be double-lined and equipped with a leak collection and recovery system.

NMCC recognizes NMED’s concerns regarding the existing waste rock stockpiles (EWRSPs) at the site. Section **20.6.7.21.C.(2)** allows the existing stockpiles to continue to “operate” as previously permitted. Figures 11J-3 and 11J-15A show that EWRSP-1, EWRSP-2A and EWRSP-2B are located within the open pit surface drainage area. The mine pit acts as a natural drainage sink for surface and ground water at this location. EWRSP-3 is located at the north end of developed watershed A (WS A) within the plant site. EWRSP-4 is located southeast of the mine pit. NMCC proposes the following plan to address how each of the existing waste rock stockpiles will be managed during operations. The details of the plan are described in the Updated MORP submittal and are generally discussed herein. NMCC will conduct some interim maintenance and reclamation at these locations to ensure that all surface water drainage at the EWRSP’s is appropriately managed.

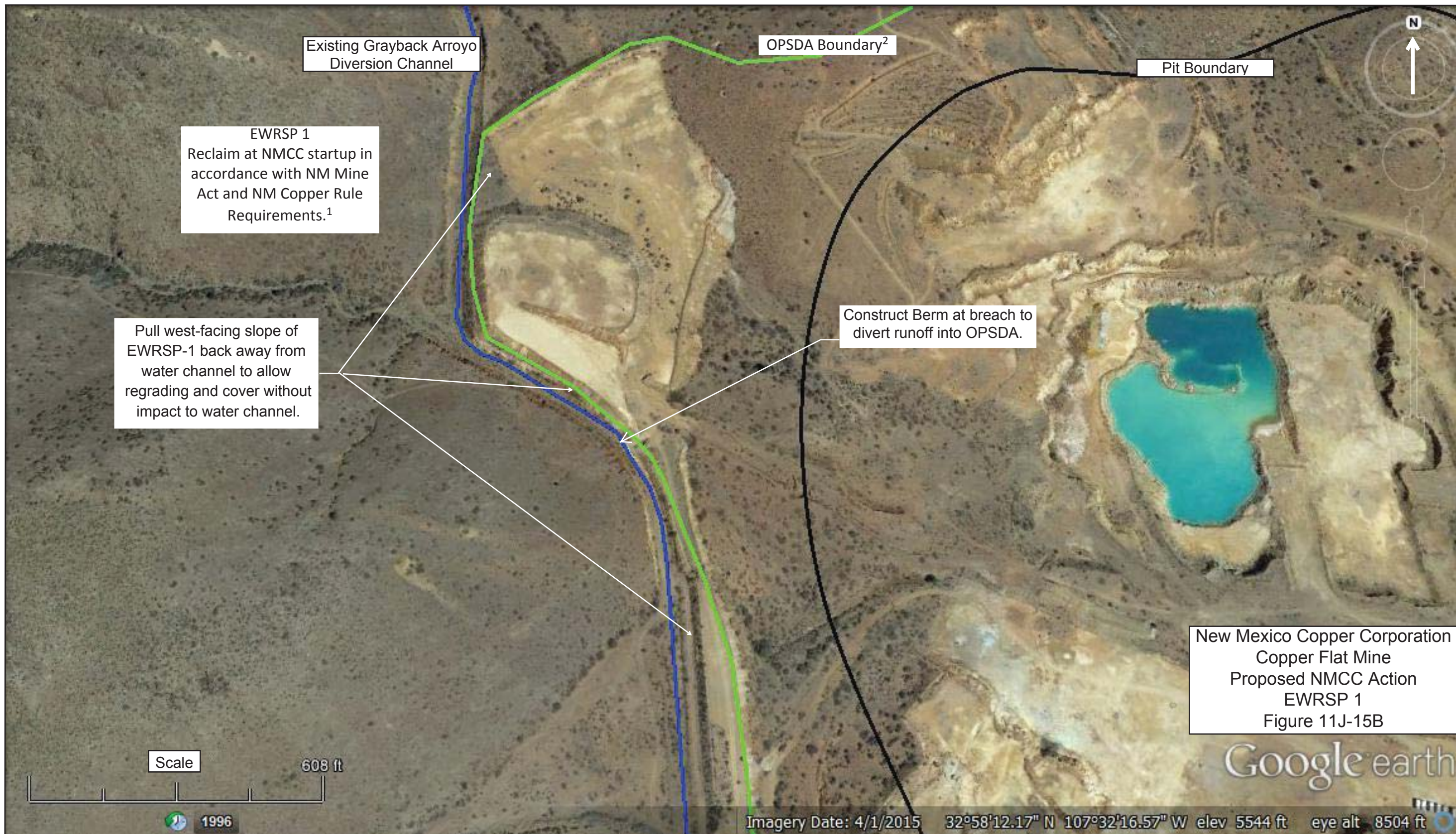
EWRSP-1 and EWRSP-2B will be reclaimed during the operations phase of the project. This will allow NMCC to establish test-plots to test a variety of vegetation scenarios, thus aiding NMCC in determining successful reclamation and revegetation alternatives to implement at closure. At EWRSP-1, as shown in Figure 11J-15B, a berm was constructed immediately



New Mexico Copper Corporation  
 Copper Flat Mine  
 General Facility Layout  
 Figure 11J-15A

WS - watershed  
 WRSP - waste rock stockpile  
 EWRSP - existing waste rock stockpile





1. See detail in Updated MORP Reclamation and Closure Plan  
 2. See Figure 11J-3 and Updated MORP for OPSDA boundary after reclamation complete



downstream of the location of the breach that currently exists to divert surface water drainage back into the OPSDA so that it no longer enters Grayback Arroyo. This work was performed in the summer of 2016. In addition, during operations, NMCC will reclaim EWRSP-1 as described in the approved MORP. At reclamation, surface run-off will be routed to Grayback Arroyo diversion. This will result in the reclaimed EWRSP-1 to be located outside of the OPSDA. Detailed plans for reclaiming EWRSP-1 are provided in Section 2.1 of the Mine Reclamation and Closure Plan of the Updated MORP. In summary, waste rock adjacent to the diversion channel on the west side of EWRSP-1 will be moved onto the waste rock pile to separate the east bank of the channel from the reclaimed surface. Otslopes will be graded to a 3H:1V interbench slope and the top graded at minimum 1% slope to drain. Storm water controls will be constructed for erosion control.

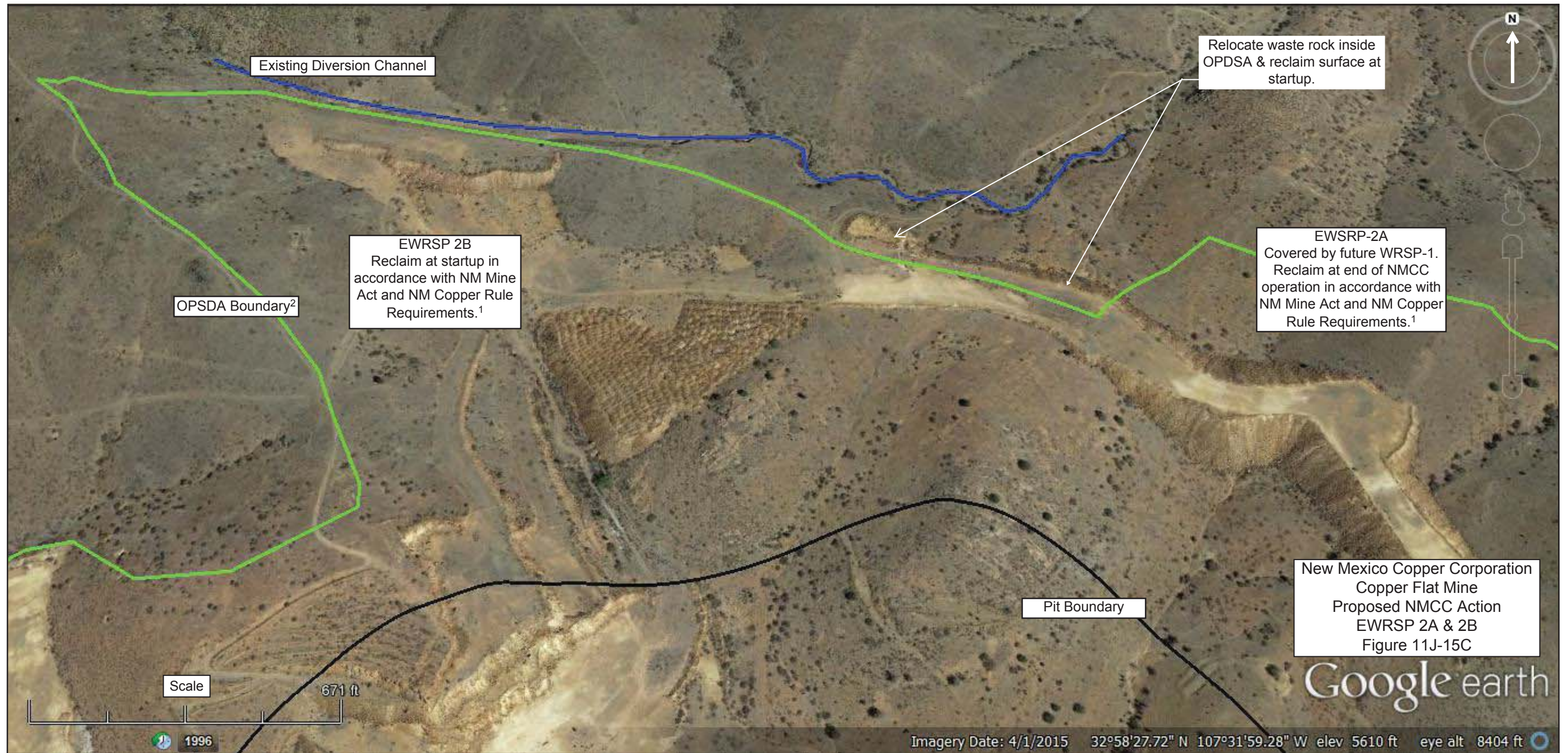
At EWRSP-2B, as shown in Figure 11J-15C, the wastes will be re-graded and reclaimed during operations in accordance with the approved MORP. At EWRSP-2A, a small amount of waste at the northernmost edge of the stockpile may be just outside of the OPSDA as is shown on Figure 11J-15C. NMCC will remove and redeposit that waste onto the portion of EWRSP-2A located within the OPSDA, as discussed in Section 2.1 of the Updated MORP, during the site preparation phase of the project prior to the commencement of construction of Waste Rock Stockpile 1 (WRSP-1). As shown Figures 11J-4 through 15, EWRSP-2B will be covered over time with the waste rock deposited in WRSP-1 during operations and will simply become part of WRSP-1 over the life of the mine.

NMCC has chosen to manage surface water runoff from the eastern one-third of the OPSDA by developing a sub-watershed, i.e. developed WS B (see Figure 11J-3). The purpose of this sub-watershed is to manage runoff from the proposed new waste rock stockpile WRSP-1 to be constructed within the watershed. Impacted storm water impoundment B will be constructed at the southwest corner of developed WS B as a surface water runoff control measure to manage surface water inflow to the mine pit. The captured surface water runoff will be utilized as process make-up water.

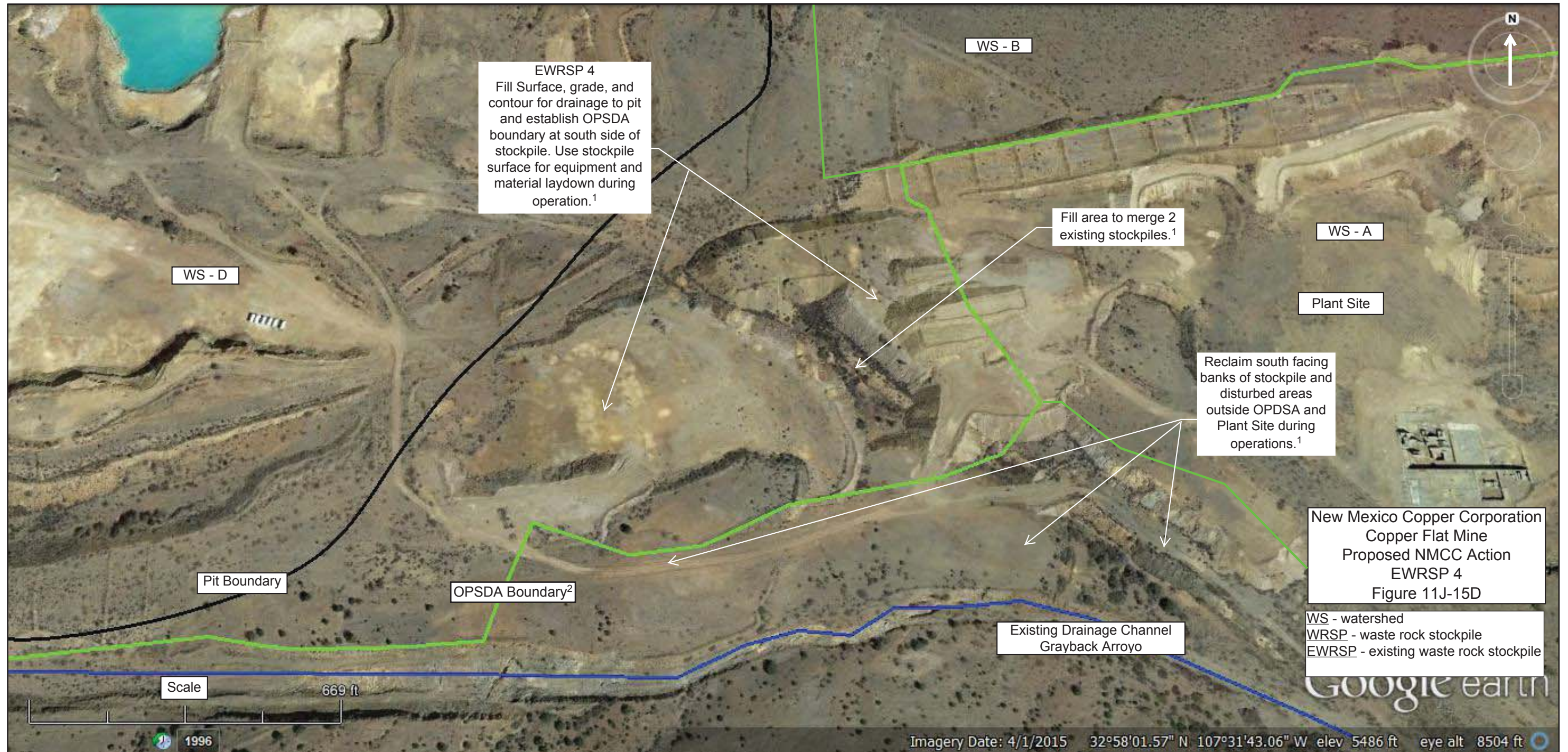
EWRSP-3 represents the last vestiges of ore and low grade materials left from the Quintana operation at shut-down. As discussed earlier herein, early in the operation some of this material will be processed through the new processing equipment as a circuit “conditioning” activity. During subsequent operations some of this waste rock stockpile may also be blended with run-of-mine ore and fed into the process if economic conditions warrant. The EWRSP-3 area is part of the processing site (see Figure 11J-3 and 11J-15A). During operations it will be utilized as the area of temporary storage of run-of-mine ore transported from the mine not immediately fed into the primary crusher. During operations storm water runoff from this area will be captured in impacted storm water impoundment A.

EWRSP-4 will be re-graded and contoured during the site preparation and construction phase of the project for use as an equipment storage and laydown area during operations as shown in Figure 11J-15D. Detailed grading and reclamation plans for EWRSP-4 are





1. See detail in Updated MORP Reclamation and Closure Plan
2. See Figure 11J-3 and Updated MORP for OPDSA boundary after reclamation complete



1. See detail in Updated MORP Reclamation and Closure Plan  
2. See Figure 11J-3 and Updated MORP for OPSDA boundary after reclamation complete



provided in Section 2.1 of Appendix E, Mine Reclamation and Closure Plan the Updated MORP. Impacted stormwater runoff from the area will be managed by grading the top surface such that runoff from the area will be routed into the open pit away from Grayback Arroyo. Fill material will be added to the top surface for grading to minimum 1% slope. The southern facing outslope of the stockpile will be reclaimed by grading to a 3H:1V interbench slope, covering with 3 feet of growth media material and revegetating. Storm water controls will be constructed for erosion control. Final reclamation of the area will be completed at the end of operations.

The cost of reclamation of all the existing waste stockpiles will be included in the financial surety calculations provided in the MORP. At the end of the life of the mine, the Copper Flat facility will be reclaimed in accordance with an approved reclamation plan/closure plan. All of the waste rock stockpiles will be reclaimed by contouring and grading the piles to prevent run-on, control run-off and minimize infiltration of precipitation. Three feet of soil cover will be placed over the stockpiles and out-slopes. The cover will promote evapotranspiration, further reduce infiltration and facilitate re-vegetation. NMCC believes that the steps described above contribute significantly to protection of groundwater.

**V. 20.6.7.11J.(2) DESCRIPTION OF PROPOSED SLAG**

New Mexico Copper does not propose to produce slag at the Copper Flat project inasmuch as the proposed facility does not include a smelter.



**20.6.7.11.J.(3)** a description of each existing or proposed open pit and underground mine within the proposed copper mine facility and information about its location, depth, size, and acreage;

### **DESCRIPTION OF THE OPEN PIT MINE FACILITY**

NMCC proposes to construct an open pit mine at its Copper Flat project. This new facility will entail the expansion of an existing open pit previously developed and operated for a short time in 1982 by Quintana Minerals. A portion of the ore body at Copper Flat is exposed at and near the surface and will be mined by conventional truck and shovel open pit methods.

#### **Existing Open Pit**

NMCC's Copper Flat mine facility will expand the existing open pit shown in Figure 11J-2. Quintana Minerals created this existing open pit in 1982 when they brought the property into production as an open pit mine and mineral processing plant. The initial mine excavation needed to expose the ore body occurred during the four- to six-month period immediately preceding startup of the mineral processing plant. Following startup of processing, the open pit and processing plant were in commercial production for three and a half months. At that time, all operations were halted due to a significant decline in copper prices. Approximately 3 million tons of overburden material and 1.2 million tons of ore were mined from the open pit by Quintana. No mining has occurred at this open pit since 1982.

The location of the open pit is shown on Figures 11J-1 and 11J-3, provided herein in response to **20.6.7.11.J.(1)**. The floor of the existing pit is 5,400 feet above sea level, which is approximately 100 feet beneath the original pre-mining ground surface. The existing open pit encompasses approximately 102 acres. A 5.2-acre lake is located in the existing pit. The depth of the pit lake water is approximately 35 - 40 feet. The existing pit lake contains approximately 20 to 28 million gallons (61 to 86 AF) of water.

#### **Proposed Open Pit**

NMCC's proposed open pit will be created through the expansion of the existing open pit. A multiple bench, open pit mining method will be used to mine the Copper Flat ore body. Figures 11J-4 through 11J-15 depict the expansion of the pit over time. Over the 11-year life of the proposed project, approximately 125 million tons of copper ore and 33 million tons of waste rock will be mined and removed from the open pit. The proposed mining activities will enlarge the open pit over time to a diameter of approximately 2,800 feet. In accordance with **20.6.7.24.A, REQUIREMENTS FOR OPEN PITS**, the open pit will remain within the area identified in the discharge permit. The floor of the proposed open pit will reach a depth of approximately 4,650 feet above sea level, which will be approximately 900 feet beneath the original pre-mining ground surface. The area of the pit will be expanded to approximately 161 acres. The existing diversions of Grayback Arroyo constructed by Quintana during its operation of the mine will not be altered by the proposed pit expansion. No underground mining is proposed.



Ore material from the pit will be drilled and blasted, loaded, and hauled to the primary crusher and then conveyed to the mill as described below. Waste rock will be placed in designated stockpile areas as described earlier herein.

#### **MINE OPERATION WATER MANAGEMENT PLAN**

This Mine Operation Water Management Plan is submitted in accordance with **20.6.7.24.C** of the Copper Rule. Water use and water conservation are amongst the most important aspects of operating a copper mine after employee safety and protection of human health and the environment. The water demands for the Copper Flat operation are such that it is very important that all water conservation measures possible are taken to maximize water harvesting and recycling of water wherever possible.

On average, approximately 13,000 gpm of water will be required to operate the facility. The source of water will come largely from recycling approximately 9,200 gpm from the tailings storage facility. The remaining made-up water will come from an off-site well field and whenever possible, make-up water provided from capture and management of surface water runoff.

Appendix A contains the detail design of the Copper Flat tailings storage facility water recycling system which includes a water reclamation barge and an underdrain collection system. The water reclaim barge located within the water pond of the tailings impoundment will be equipped with pumps that will pump free water back to the process circuit for reuse in the process. The barge pumps will operate 24 hours per day continuously providing the greatest majority of the water required by the process at a design capacity of 13,000 gpm, though as noted above, the average rate of pumping will be 9,200 gpm.

In addition, as indicated, the tailings facility will be equipped with an underdrain collection system that will allow water to drain from bottom of the tailings impoundment and from underneath the dam. This underflow water will be captured in an underdrain collection pond located at the outside toe of the tailings impoundment. In addition to the water harvested by the tailings underdrain system, precipitation that falls on the outer surface of the tailings dam will also captured by lined runoff collection galleries or trenches that will divert the water to the underdrain collection pond. The underdrain collection pond will be equipped with a pump station capable of pumping a design capacity of 4,000 gpm, though the average rate of pumping is anticipated to be approximately 1,700 gpm.

The water from the TSF water recycling system will be transported via a pipeline to the process water reservoir where it will be held for introduction to the process. The pipeline from the TSF recycling system will be placed within a trench that is lined for secondary containment such that any leaks that may occur in the line will be transported back either to the underdrain collection pond or the tailings impoundment, thus minimizing potential loss of water.

In addition to the water tailings storage facility recycling system, NMCC will harvest as much runoff water as possible. Section **20.6.7.11.J.(2)**, above, describes the various



impacted storm water collection impoundments proposed at Copper Flat. These impoundments are designed to maximize the capture and retention of runoff through trenches and ponds. Water captured in these impoundments will be retained for less than thirty day before being evacuated to the process water reservoir. While the amount of water harvested from these areas is entirely dependent upon the amount of precipitation received, any water captured and utilized in the process represents an amount less that has to be produced from the freshwater wells.

Water that is captured within the crushing, grinding and process area sumps will all be routed back to the into the process as described in more detail in section **20.6.7.11.J.(5)**. Water produced from the packaged wastewater treatment plant will be piped to the tailings storage facility and become subject to recycling at the point.

Approximately 73 percent of the water required for processing ore will be provided by recycling water back from the TSF through the designed water collection and recycle system described above. Approximately 23 percent of the water used for processing ore will remain entrained within the tailings. The remaining 4 percent will be lost to evaporation or as moisture in the product concentrates. The amount of water in the concentrates will be less than 1 percent of the total water used for processing ore. Table 11J-2 lists the amount of water required for processing ore as well as the amount that would result from collection and recycle from the TSF.

Water Use	Acre-Ft per Year			Percent of Total
	Recycled	Non-recycled	Total	
Ore Processing:				
Reclaimable TSF water	15,504	0	15,504	73%
Water retained in tailings	0	4,973	4,973	23%
Evaporation	0	752	752	4%
Concentrates	0	13	13	<1%
<b>Ore Processing Total</b>	<b>15,504</b>	<b>5,738</b>	<b>21,242</b>	<b>100%</b>

It is NMCC’s goal to maximize the use of recycled and harvested water and minimize its waste. As such all of the water impoundments, except the mine pit, including the tailings storage facility, underdrain collection pond, storm water impoundments, process water reservoir, and the surge pond will be lined to minimize the loss of water. However, even while maximizing water recycle, there will be some water loss, mainly to entrained water in the tailings themselves, some to evaporation, and a small amount entrained in the product. As such, there will be some make-up water required to be provided by the off-site well field. NMCC hopes to keep the use of freshwater to a minimum.



### **Pit dewatering**

Dewatering of the mine pit and the pit lake will be necessary prior to mining and continuously throughout the life of the mine. The water contained in the pit lake prior to operations will be used for dust control during construction and operations. NMCC understands that use of the water produced from the mine pit must meet NMCC water quality standards if it is to be used for dust control purposes at locations outside of the OPSDA. NMCC will utilize all of the water produced from the open pit during operations for dust suppression on the haul roads, working areas, and waste rock stockpiles only within the OPSDA. NMCC will also utilize excess water from the OPSDA as an additional source of process water whenever possible. NMCC will utilize water produced from the mine pit for dust suppression outside of the OPSDA only if the quality of water meets limits placed on the discharge permit.

NMCC anticipates that during operations, groundwater will continue to seep into the pit at an annual average rate of approximately 24 gpm (39 AFY). In addition, storm water runoff will contribute an average of approximately 68 AFY to the pit. Water removal from the pit will continue over the operational life of the mine through a sump or series of sumps located within the pit. Water removal will end once mining of the pit is completed.



**20.6.7.11.J.(4)** a description of each existing or proposed material handling and processing unit including crushing, milling, concentrating, smelting and SX/EW units within the copper mine facility, and information about its location and proposed methods of process water handling and disposal;

## **DESCRIPTION OF PROPOSED MATERIAL HANDLING AND PROCESSING**

The ore processing facilities will be constructed at the site of the original Quintana Minerals processing plant site which is located southeast of the existing open pit as shown in Figure 11J-1, provided herein in response to **20.6.7.11.J.(1)**. Ore processing will consist of a conventional sulfide flotation plant to extract copper, a molybdenum processing circuit, and a gravity gold recovery circuit. No smelting, refining or SX/EW operations will be conducted at the Copper Flat site. The plant will produce copper and molybdenum concentrates as well as a small amount of coarse gold concentrate. Figure 11J-16 is a conceptual flow diagram of the process. Figures 11J-17, 18, 19 and 20 are preliminary isometric drawings that provide an overview of the process area to aid the reader in visualizing how the ore is processed. As discussed in more detail in Section **20.6.7.11.J.(5)**, all of the processing units are in containment designed to capture operational spills.

The ore will be crushed and ground to a fine particle size and then processed through mineral flotation circuits. Ore processing activities will continue 24 hours per day, seven days per week, 365 days per year. The plant will process approximately 11 million tons per year at an average rate of 30,000 tons per day or over the life of the project. The major equipment for the mineral processing plant will consist of the following:

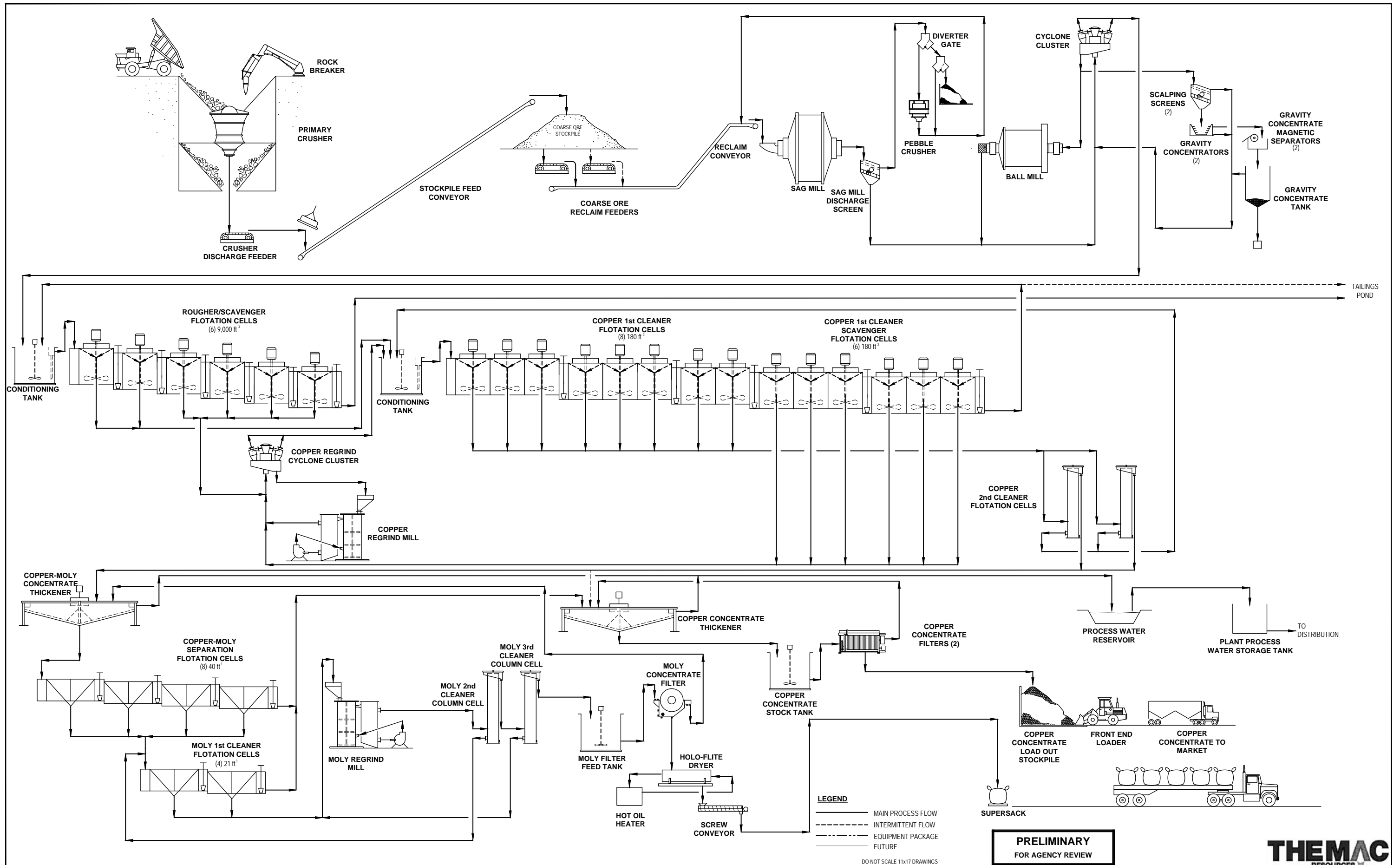
### **Primary Crushing and Coarse Ore Stockpile:**

- One 42- x 65-inch gyratory crusher;
- One crusher discharge feeder;
- One 48-inch x 454-foot-long stockpile feed conveyor with stacker;
- Two coarse ore reclaim feeders; and
- One 48 inch X 470-foot-long reclaim conveyor.

### **Crushing and Grinding:**

- One 32-foot-diameter x 14-foot-long SAG mill, 11,000 horsepower;
- One 12X16-foot double deck SAG mill discharge vibrating screen;
- One 4.5-foot cone crusher, 300 horsepower (pebble crusher);
- One 24-foot-diameter X 35 foot-long ball mill, 1500 horsepower;
- One primary cyclone cluster with eight 33-inch-diameter cyclones; and
- Two gravity gold concentrators with scalping screens.





**PRELIMINARY**  
FOR AGENCY REVIEW



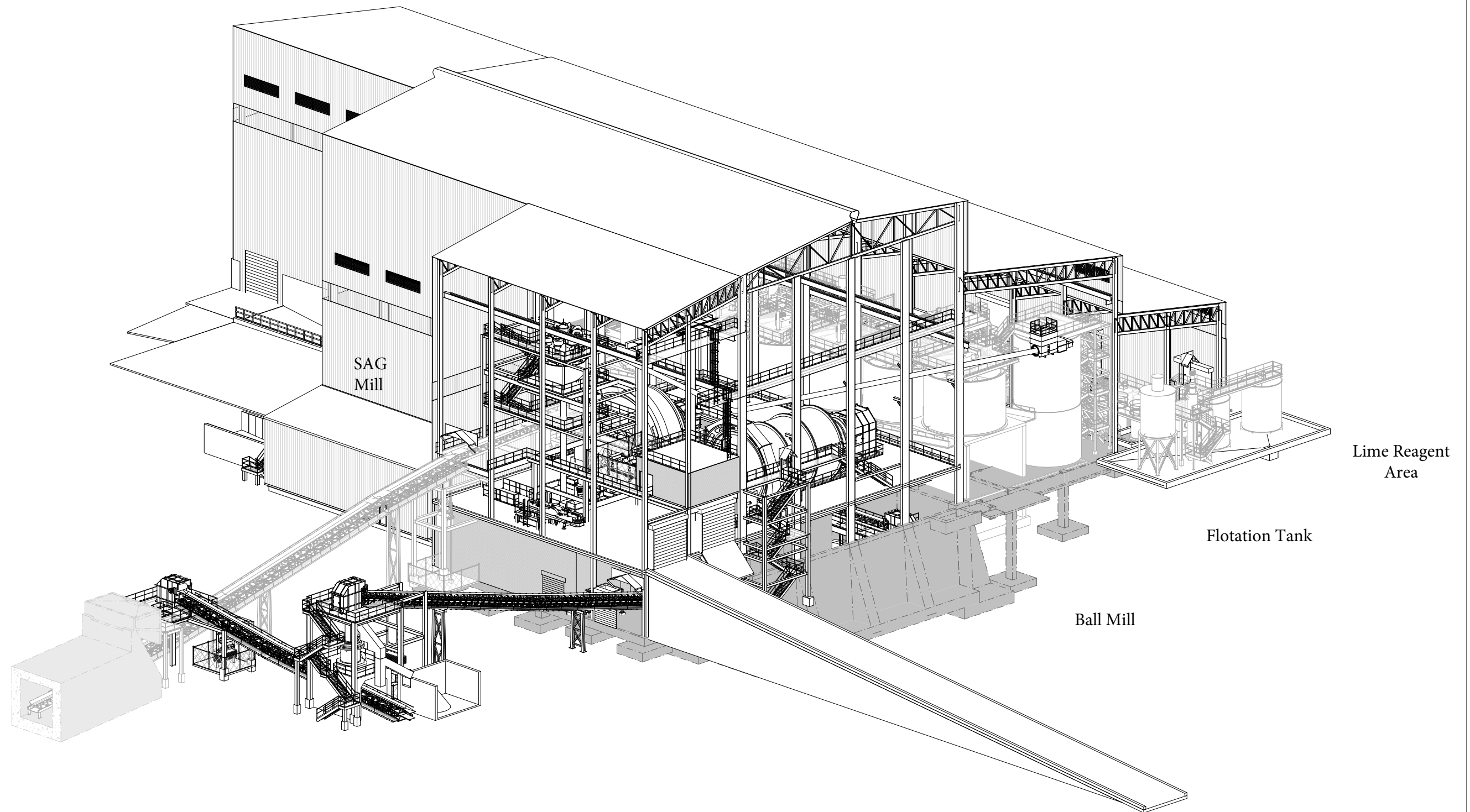
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DESIGNED BY: EA	DATE: JUL 11
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CHECKED BY:	
PROJECT MGR:	
CLIENT APPR:	

**COPPER FLAT PROJECT**

PROCESSING PLANT  
OVERALL FLOW SHEET  
30,000 TPD  
CONVENTIONAL TAILINGS

JOB NO. M3 PH-120085  
DWG. NO. **FIGURE 11J-16**  
REV. NO. P6 DATE 18 MAR 13



DO NOT SCALE 11x17 DRAWINGS

**THEMAC**  
RESOURCES

REFERENCES		REFERENCES		REVISIONS						REVISIONS					
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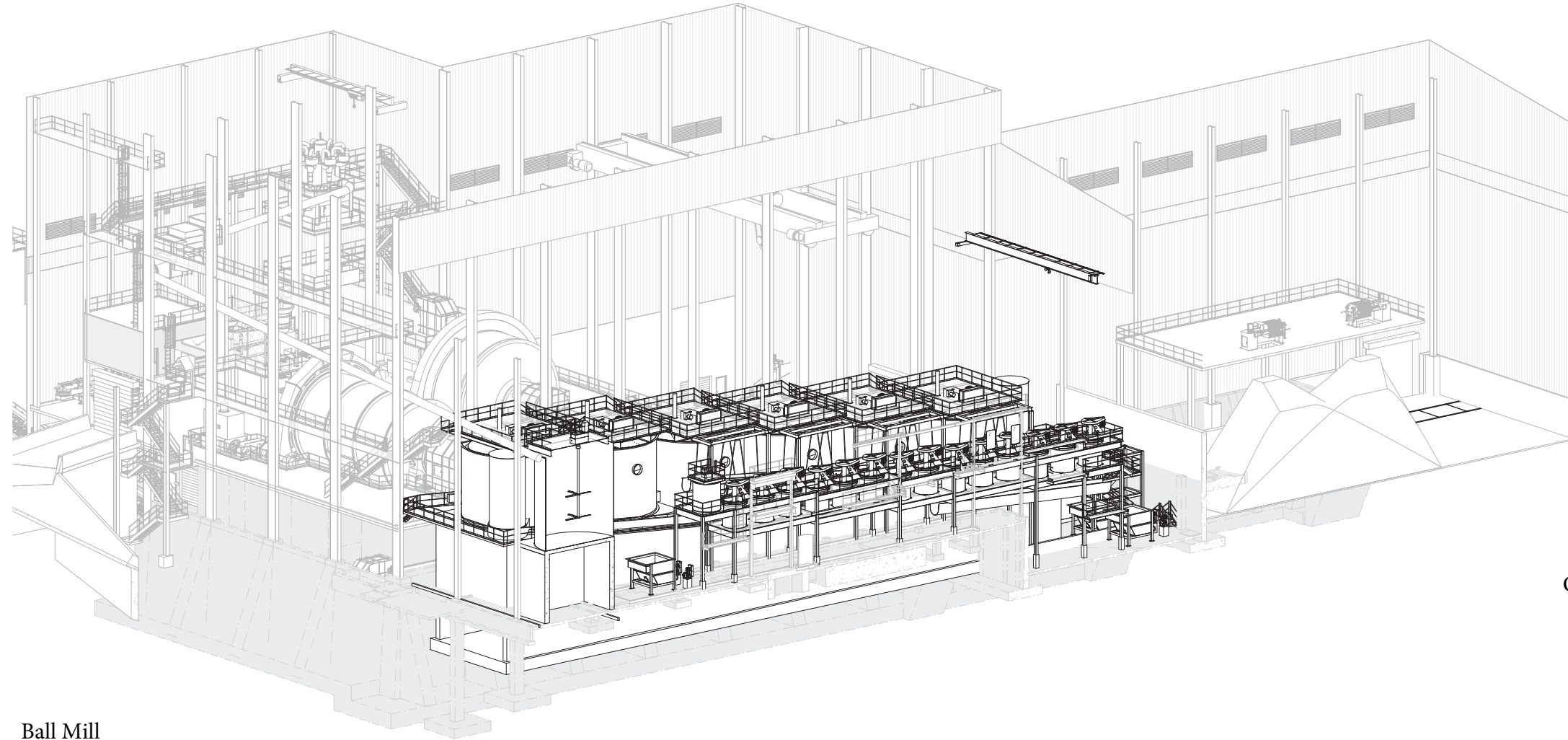
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DRAWN BY MWM	FEB 13
CHECKED BY	
PROJECT MGR	
CLIENT APPR	



<b>COPPER FLAT PROJECT</b>		PROJECT NO. M3-PN120085
		DWG NO. <b>3015-GA-000</b>
REV NO. P2	DATE 26 MAR 13	

Figure 11J-17 Page 78 15948 Jun 2016

SAG  
Mill



Copper Concentrate  
Pressure Filters

Copper Concentrate  
Product

Ball Mill

Copper Flotation  
Area

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RESOURCES

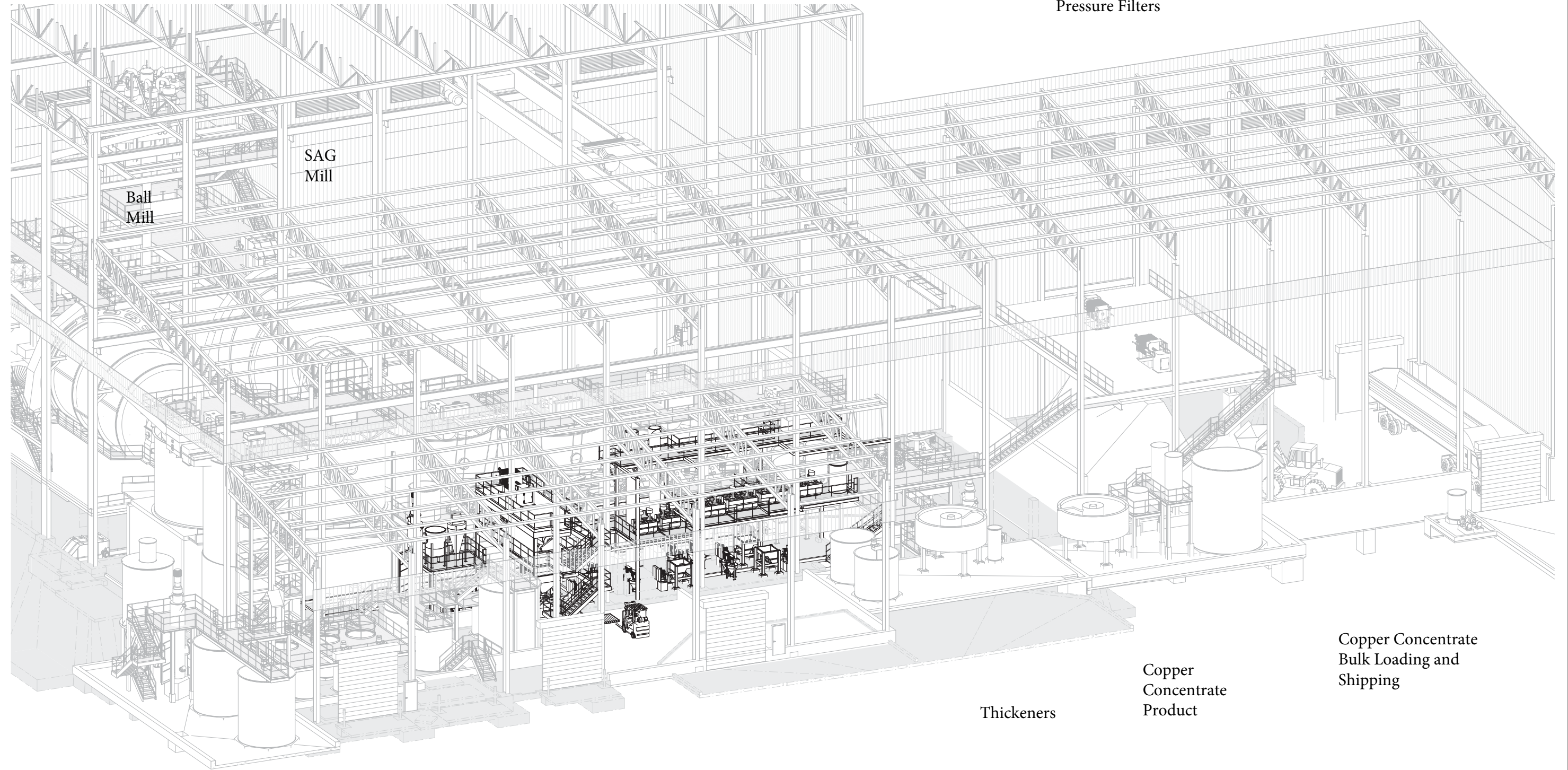
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SCALE: AS NOTED	DATE
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PROJECT MGR	
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<b>COPPER FLAT PROJECT</b>	
PROJECT NO. M3-PN120085	
DWG NO. <b>3016-GA-000</b>	
REV NO. P3	DATE 06 MAY 13

Copper Concentrate  
Pressure Filters



Copper Concentrate  
Bulk Loading and  
Shipping

Copper  
Concentrate  
Product

Thickeners

Moly Flotation  
Area

Reagent Storage  
Area

DO NOT SCALE 11x17 DRAWINGS

**THEMAC**  
RESOURCES

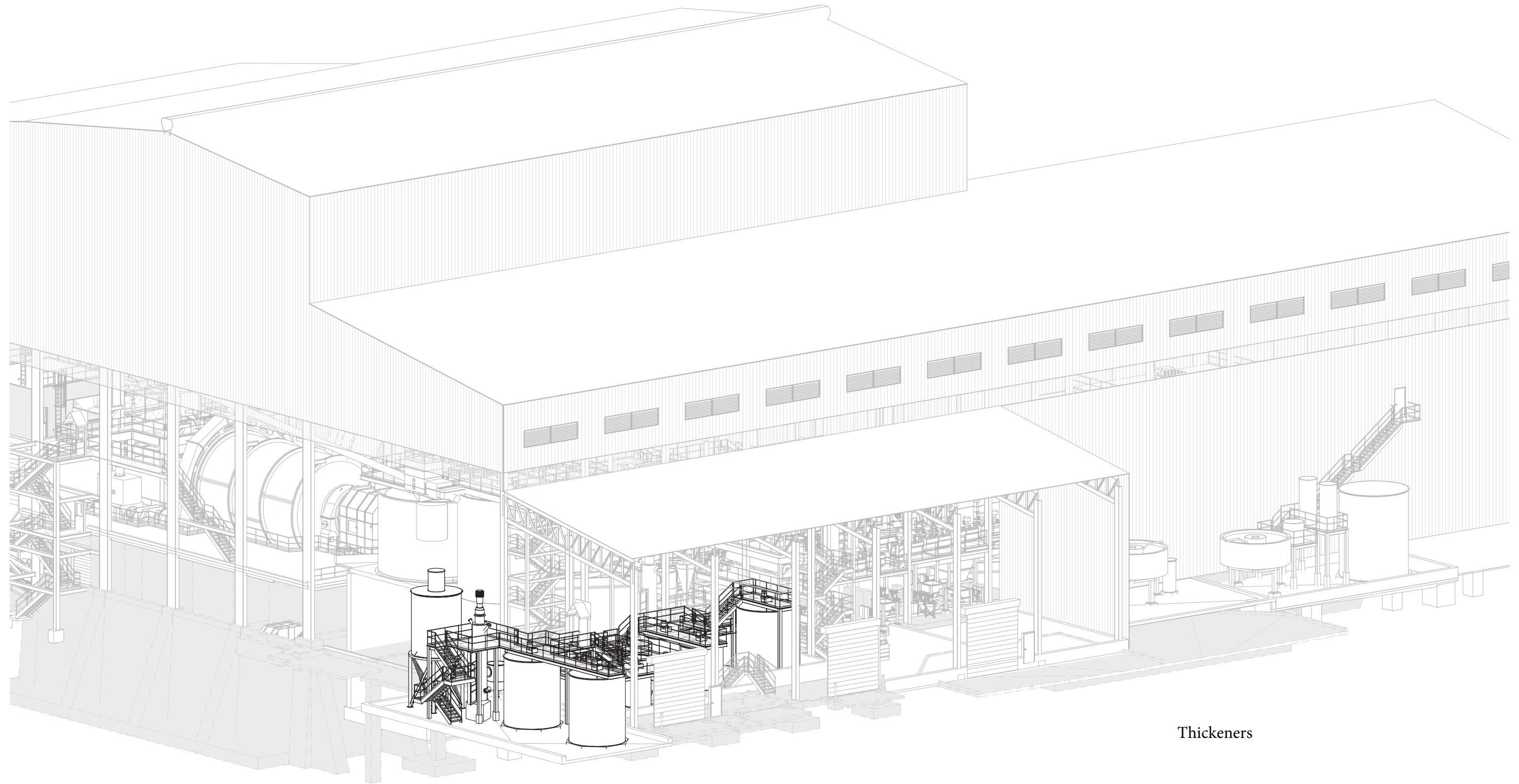
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SCALE: AS NOTED	DATE
DESIGNED BY: AF	JAN 13
DRAWN BY: TRR	JAN 13
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<b>COPPER FLAT PROJECT</b>	
<b>MOLY PROCESS GENERAL ARRANGEMENT CONCENTRATOR ISOMETRIC</b>	
PROJECT NO. M3-PN120085	DWG NO. 3017-GA-000
REV NO. P2	DATE 26 MAR 13

Figure 11J-19 Page 80 of 15950 Jun 2016



Ball Mill

Reagent Storage Area

Thickeners

DO NOT SCALE 11x17 DRAWINGS

**THEMAC**  
RESOURCES

REFERENCES		REFERENCES		REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT NO.	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT

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PROJECT MGR	
CLIENT APPR	



<b>COPPER FLAT PROJECT</b>		PROJECT NO. M3-PN120085
		DWG NO. <b>3014-GA-000</b>
REV NO. P2	DATE 26 MAR 13	



#### Flotation and Concentration:

- Six 9,000-ft<sup>3</sup> bulk rougher flotation machines (copper/moly);
- Fourteen 180-ft<sup>3</sup> cleaner flotation machines (copper);
- Two 800-ft<sup>3</sup> column flotation machines (copper);
- Eight 25-ft<sup>3</sup> separation flotation machines (moly);
- Four 10-ft<sup>3</sup> cleaner flotation machines (moly);
- Two 40-ft<sup>3</sup> column flotation machines (moly);
- One 16-foot-diameter bulk concentrate high rate thickener (copper/moly);
- One 16-foot-diameter concentrate high rate thickener (copper);
- Two automatic filter presses (copper); and
- One 4-dstph disk filter (moly).

#### **Primary Crushing and Coarse Ore Stockpile Facilities**

As shown on Figure 11J-16, ore will be hauled from open pit in trucks and fed to the primary crusher for the first stage of crushing. The crusher will size the run-of-mine rock to a nominal 8 inches in diameter or less. The crushed rock will be fed by an apron feeder onto the stockpile feed belt conveyor for transport to the coarse ore stockpile where it will be temporary stored prior to being fed into to the grinding mill. The belt conveyor will include a stacker for placing the coarse ore into the stockpile as seen on Figure 11J-16. The coarse ore stockpile will have a maximum capacity of 75,000 tons.

Ore will be fed from the coarse ore stockpile through the reclaim tunnel located beneath the stockpile onto a conveyor system to feed the SAG mill in the grinding circuit. The conveyor system will be equipped with two variable speed apron feeders that will feed the reclaim conveyor.

Ore handled through this part of the process will be relatively dry. Water associated with this part of the process will include:

- Moisture associated with the ore;
- Water spray used to control dust within the primary crusher pocket and at the stockpile feed stacker; and
- Water used for housekeeping purposes.

The primary crusher and the coarse ore reclaim equipment will be located below ground level in reinforced concrete structures. These concrete structures are existing structures from the Quintana operation and have concrete sumps built into the structures to contain excess water. Water collected in these sumps will be reused within the ore processing circuit via a pumping and recycling system that will be installed during the construction phase of the project. Water used for housekeeping purposes will be confined to use within the concrete structures where it will be contained and recycled via the sump collection and recycle system.



Water spray used for dust control at the stockpile feed stacker will be exterior to the concrete structures. The sprays will only be used when necessary to control dust and will be controlled to minimize excess moisture. During normal operations, water from these sprays will evaporate once the ore reaches the coarse ore stockpile. In the event of an upset condition, any excess water from these sprays will be contained similarly to storm water runoff from this area.

### **Crushing and Grinding**

Ore from the coarse ore stockpile fed through the reclaim tunnel as shown on Figure 11J-17 to the SAG mill will be ground up as a result of impact between the rock entering the mill and five-inch steel grinding balls fed to the mill along with the rock. The walls of the building have been removed in this drawing to allow a view of the equipment and interior.

Material handling from this point in the process will become a wet process as water and various reagents will be added to the SAG mill feed to start the conditioning of the ore pulp for subsequent stages of treatment. Rock from the SAG mill will discharge onto the double-deck SAG mill discharge vibrating screen for sizing.

The double-deck vibrating screen will size the rock in the following manner:

- Rock staying on top of the upper screen deck (oversize) will be taken by belt conveyor to the cone crusher (pebble crusher) where it will be crushed to less than 0.75-inch diameter and returned by belt conveyor to the SAG mill.
- Rock passing through the upper screen deck but not passing through the bottom screen deck will return directly to the SAG mill by conveyors for further crushing and grinding.
- Rock passing through both screen decks will travel to the cyclone feed sump for additional sizing.

Rock collected at the cyclone feed sump will be pumped to the primary cyclone cluster where the eight cyclones will be used to separate the finer ground material (overflow slurry) from the more coarse material (underflow slurry). The overflow slurry will be sent to the first stage of flotation. Most of the underflow slurry will return to the ball mill for further grinding, but a portion of the underflow will be taken through a Knelson-type gravity concentrator circuit to collect gravity recoverable gold. The gravity separation circuit will consist of two concentrators each of which will have an upstream scalping screen to remove oversize material. The gravity concentrates will pass through magnetic separators for removal of tramp iron, i.e., steel introduced from worn liners, and broken grinding media. The reject from the gravity concentrators will be pumped back to the cyclone feed sump.

The majority of this part of the process will be constructed on an existing Quintana foundation. This reinforced concrete foundation is constructed with stem walls and sloped to drain to internal sumps that will contain spills. Finer material and all solutions will be



collected in the internal sumps and pumped to the cyclone feed sump for further processing. The cyclone feed sump will be a large reinforced concrete tank built on and attached to the foundation of the building. Coarser material will be collected in piles on the foundation floor and allowed to drain into the sump so that there is no free water associated with this material. Mobile equipment will be used to place the drained material in the coarse ore stockpile for reprocessing.

The pebble crusher will be located outside and adjacent to the existing Quintana foundation. The feed material for the pebble crusher will be large, free-draining, pebble-like material that will be free of water except for small amounts of residual surface moisture. This pebble crusher feed will be collected off the upper screen of the SAG mill discharge vibrating screen. This upper screen will have screen openings of approximately 0.75 inches or larger ensuring the dewatering of this material. Multiple inclined belt conveyors will be used to transport the feed from the SAG mill discharge screen to the pebble crusher.

### **Flotation and Concentration**

Cyclone overflow slurry, which is the final product of the grinding circuit, will flow by gravity to the rougher flotation conditioning tank located ahead of the rougher flotation cells. The overflow slurry will be sampled and analyzed for metallurgical control prior to flotation. The flotation process begins when the cyclone overflow enters from the conditioning tank to the first in a series of six flotation cells (see Figure 11J-17 and 18). Flotation reagents will be added to the overflow and the tanks agitated, creating forth or bubbles, which will allow the reagents to react with the ore particles. Molybdenum and Copper-bearing sulfide mineral particles will concentrate and adhere to bubbles created by the induced air and frothing agents. The copper/molybdenum concentrate will be floated off and concentrated through a series of flotation cells. Cyclones and a regrind mill will be used for further cleaning and concentration.

The final underflow from the flotation process will be a tailings and water mixture, i.e., whole tailings that will be transported to the TSF via pipeline. The final overflow of the flotation process will be a wet copper/molybdenum concentrate mixture that will report to the copper/molybdenum concentrate thickener. The copper/molybdenum concentrate thickener will be used to dewater the concentrate prior to separating the molybdenum concentrate from the copper concentrate. The overflow of the copper/molybdenum concentrate thickener (primarily process water) will be pumped via pipeline to the process water reservoir located near the plant. The underflow (a dewatered copper/molybdenum concentrate mixture) will report to the molybdenum plant.

Flotation cells within the molybdenum plant will be used to separate the molybdenum concentrate from the copper concentrate. Reagents will be added to cause the copper minerals to remain in solution while the molybdenum minerals will adhere to bubbles and float to the top of the flotation solution. Once floated, the wet molybdenum concentrate will be collected and further cleaned and concentrated using a regrind mill and column





flotation cells. The molybdenum concentrate will then be dewatered, dried, and packaged. Dewatering will be accomplished with disk filters. Filtrate from the filters will be returned to the copper/molybdenum concentrate thickener. Drying will be accomplished with a hydro-flite dryer, which will use heat to evaporate excess water in the molybdenum concentrate. The molybdenum concentrate will be packaged for transport to off-site facilities for further processing. The packaging area is located generally in the area shown on Figure 11J-19 in front of the forklift shown in the figure. The molybdenum concentrate will be approximately 50% molybdenum sulfide.

The underflow from the molybdenum flotation plant will be a wet copper concentrate. The copper concentrate will be dewatered in the copper concentrate thickener. Overflow from the copper concentrate thickener (primarily process water) will be pumped to the process water reservoir. The underflow from the copper concentrate thickener (dewatered copper concentrate) will report to pressure filters for further dewatering. Filtrate from the pressure filters will be collected and returned to the copper concentrate thickener. Once pressure filtered, the copper concentrate will be stored for transport to off-site smelting and refining facilities. The copper concentrate will average approximately 28% copper.

The flotation and concentration processes will be contained within a structure utilizing a reinforced concrete foundation and floor with sumps incorporated into the floor. Spillage caused by any upset event will be contained within the foundation area, collected in sumps and reintroduced into the process.

### **Process Water Handling and Disposal**

All water used in the processing of ore will be either be contained within the ore processing circuit, discharged into a lined impoundment (TSF), or be in the copper and molybdenum concentrates as moisture content. The Mine Operation Water Management Plan provided above in Section **20.6.7.11.J.(3)** provides more detailed on water handling and disposal.

The plant water system will consist of a lined process water reservoir and a plant process water storage tank. Both will be located near the plant site. Water will be delivered to the reservoir via pipelines. Water reporting to the reservoir includes the following:

- Recycled process water from the TSF;
- Copper/molybdenum concentrate thickener and copper concentrate thickener overflows;
- Storm water from the Impacted storm water impoundments; and
- Makeup water from the fresh water tank (water from the well field).

Water from the process water reservoir will be pumped to the plant process water tank. The tank will deliver water the processing areas for use as needed a via gravity-flow pipeline. As indicated above, in Table 11J-1, approximately 73 percent of the water



required for processing ore will be provided by recycling water back from the TSF through the designed water collection and recycle system described above. Approximately 23 percent of the water used for processing ore will remain entrained within the tailings. The remaining 4 percent will be lost to evaporation or as moisture in the concentrates. The amount of water in the concentrates will be less than 1 percent of the total water used for processing ore.



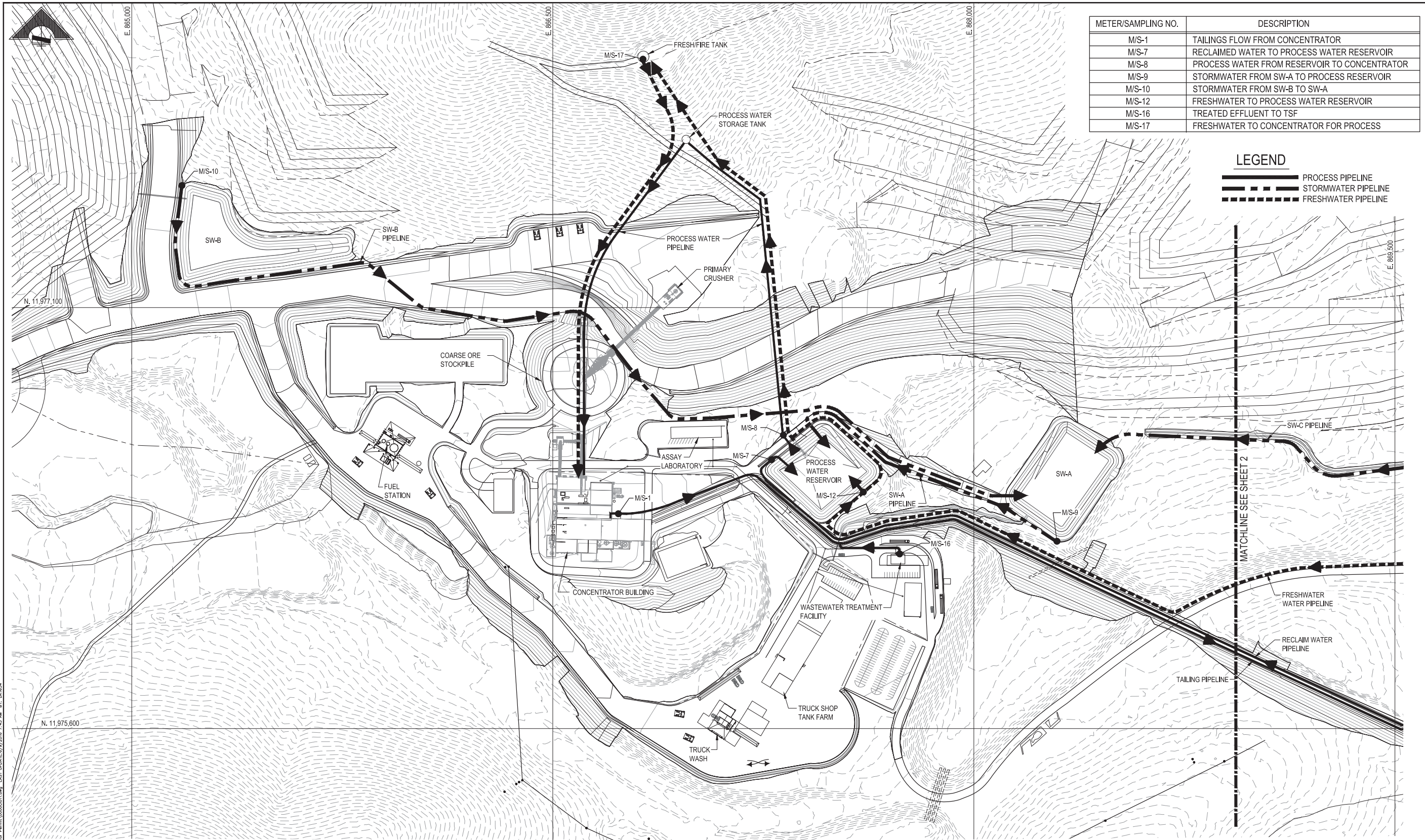
**20.6.7.11.J.(5)** *a description of existing or proposed sumps, tanks, pipelines and truck and equipment wash units, including information for each unit regarding its location, purpose, construction material, dimensions and capacity; for portable tanks or pipelines or those subject to periodic relocation, identify the areas within which they may be used;*

#### **DESCRIPTION OF SUMPS, TANKS, PIPELINES and TRUCK and EQUIPMENT WASH UNITS**

Figures 11J-20A and 11J-20B are scaled maps that display the location of proposed pipelines. Table 11J-5 provides information describing the location of each pipeline and providing information regarding the purpose, construction materials, dimension and capacity of each pipeline. This information supplements the Process Facility Containment Report prepared by M3 that provides the sump, tank, pipeline and truck and equipment wash unit information requested. The report is included in this DP application as Appendix C. The purpose of this report is, in part, to identify and describe the proposed sumps, tanks, pipelines and truck and wash units, including their purpose, construction material, dimensions and capacity. This report also provides information regarding the form and design of the containment structures that will be incorporated into the process to contain and manage materials containing water contaminants that have the potential to migrate to groundwater and cause and exceedance of applicable groundwater standards and meet the requirements of **20.6.7.22, 23 and 26 NMAC**.

Drawing no. 0000-CI-008 in Appendix C is a scaled map of the location of the various process facility containment areas. Drawing no. 0000-GA-050 is a scaled map of the concentrator area identifying the containment arrangement for all of the process tanks, including the locations of the sumps and tanks. Drawing no. 1010-AR-012 is a scaled map of the truck shop tank farm showing the location of the tanks and sump. Drawing no. 1010-GA-010 is a scaled map of the fuel station showing the location of the tanks and sumps. Drawing no. 1010-GA-001 is a scaled drawing showing the location of the Truck Wash and its sumps or settling tanks.

With respect to the requirements of **20.6.7.23.C.(6)** for a pipeline evaluation plan for existing pipelines, NMCC will not be utilizing any existing pipelines at the Copper Flat mine with the exception of the freshwater pipeline providing water from the off-site well field. As such not pipeline evaluation plan is necessary.



METER/SAMPLING NO.	DESCRIPTION
M/S-1	TAILINGS FLOW FROM CONCENTRATOR
M/S-7	RECLAIMED WATER TO PROCESS WATER RESERVOIR
M/S-8	PROCESS WATER FROM RESERVOIR TO CONCENTRATOR
M/S-9	STORMWATER FROM SW-A TO PROCESS RESERVOIR
M/S-10	STORMWATER FROM SW-B TO SW-A
M/S-12	FRESHWATER TO PROCESS WATER RESERVOIR
M/S-16	TREATED EFFLUENT TO TSF
M/S-17	FRESHWATER TO CONCENTRATOR FOR PROCESS

**LEGEND**

	PROCESS PIPELINE
	STORMWATER PIPELINE
	FRESHWATER PIPELINE

**SITE PLAN**

150 75 0 150 300  
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**PRELIMINARY**  
FOR AGENCY REVIEW



**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL PIPELINES AND FLOWMETER LOCATIONS SHEET 1**

JOB NO. M3 PN-120085  
DWG NO. **Figure 11J-20A**  
REV. NO. P2 DATE 09 JUN 16

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																TDL	FEB 13
																RKZ	FEB 13



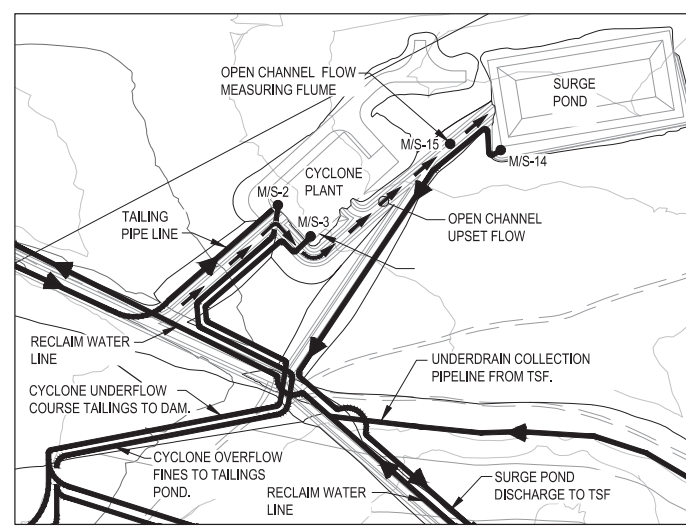
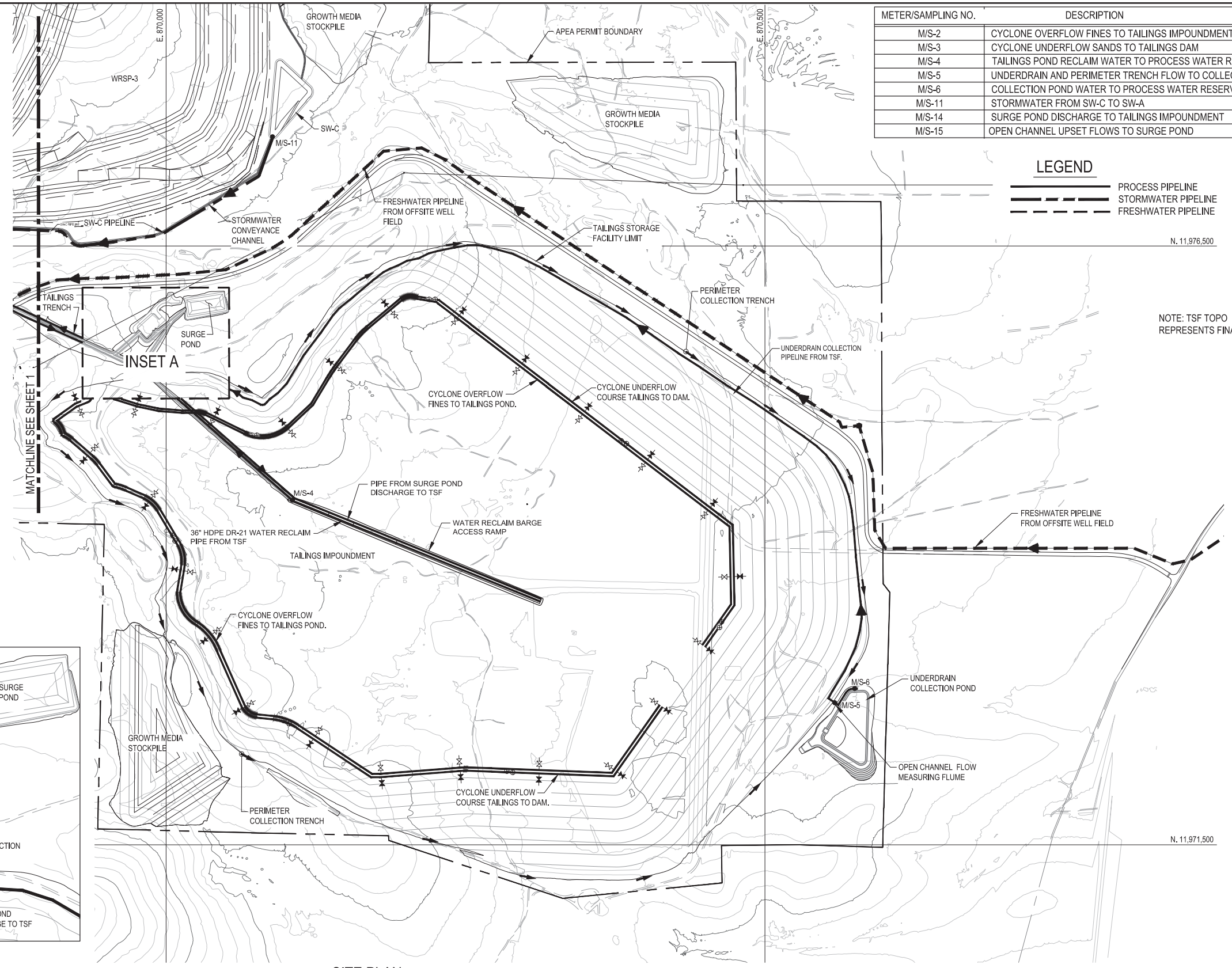
METER/SAMPLING NO.	DESCRIPTION
M/S-2	CYCLONE OVERFLOW FINES TO TAILINGS IMPOUNDMENT
M/S-3	CYCLONE UNDERFLOW SANDS TO TAILINGS DAM
M/S-4	TAILINGS POND RECLAIM WATER TO PROCESS WATER RESERVOIR
M/S-5	UNDERDRAIN AND PERIMETER TRENCH FLOW TO COLLECTION POND
M/S-6	COLLECTION POND WATER TO PROCESS WATER RESERVOIR
M/S-11	STORMWATER FROM SW-C TO SW-A
M/S-14	SURGE POND DISCHARGE TO TAILINGS IMPOUNDMENT
M/S-15	OPEN CHANNEL UPSET FLOWS TO SURGE POND

**LEGEND**

	PROCESS PIPELINE
	STORMWATER PIPELINE
	FRESHWATER PIPELINE

N. 11,976,500

NOTE: TSF TOPO REPRESENTS FINAL PHASE



**INSET A**  
SCALE IN FEET  
150 75 0 150 300

**SITE PLAN**  
SCALE IN FEET  
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**PRELIMINARY**  
FOR AGENCY REVIEW



**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL**  
**PIPELINES AND FLOWMETER LOCATIONS**  
**SHEET 2**

JOB NO. M3 PN-12085  
DWG NO. **Figure 11J-20B**  
REV. NO. P2 DATE 09 JUN 16

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DRAWN BY	SAM	FEB 13
CHECKED BY	TDL	FEB 13
PROJECT MGR	RKZ	FEB 13
CLIENT APPR.		

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**TABLE 11J-3  
PROCESS & IMPACTED STORM WATER PIPELINE DETAILS**

Description	Location	Purpose	Construction Material	Pipe Diameter	Pipe Capacity (gpm)
Whole Tailings Transport	Concentrator Bldg to Cyclone Plant	Transport tailings to TSF via Cyclone Plant	HDPE SDR 17	30 in.	15,000
TSF Barge Water Reclaim	TSF Impoundment to Process Water Reservoir	Transport reclaimed water for re-use in process	HDPE SDR 21	36 in.	13,000
Underdrain collection to reclaim	Underdrain collection pond to intersect and join TSF barge reclaim pipeline	Transport reclaimed water for re-use in process	HDPE SDR 21	20 in.	4,000
Cyclone Overflow	Cyclone Plant to TSF Impoundment	Transport tailings fines to TSF impoundment	HDPE SDR 17	30 in.	13,100
Cyclone Underflow	Cyclone Plant to TSF Dam	Transport tailings sands to TSF dam	HDPE SDR 9	12 in.	1,990
Surge Pond	Surge pond to TSF	Transport upset condition fluids to TSF impoundment	HDPE SDR 17	12 in.	2,000
Impacted Stormwater Impoundment C	Impoundment C to Impoundment A	Transport runoff water from WRSP-2 and 3 to Impoundment A	HDPE SDR 21	10 in.	2,000
Impacted Stormwater Impoundment B	Impoundment B to Impoundment A	Transport runoff water from WRSP-1 to Impoundment A	HDPE SDR-21	6 in.	700
Impacted stormwater impoundment A	Impoundment A to Process Water Reservoir	Transport runoff water from plant area and impoundments B and C to Process water reservoir	HDPE SDR 21	12 in.	3,500
Process Water Reservoir	Process water reservoir to process water tank to concentrator bldg	Transport re-use water to process water tank	HDPE SDR 21	24 in.	15,000
Process Water Tank	Process Water Tank to Concentrator bldg..	Transport re-use water to Concentrator bldg..	HDPE SDR 21	30 in.	50,000
Wastewater treatment	Wastewater treatment plant to intersect with and tap into Tailings Transport pipeline	Transport and dispose of waste water from treatment plant	HDPE SDR 17	2 in.	50

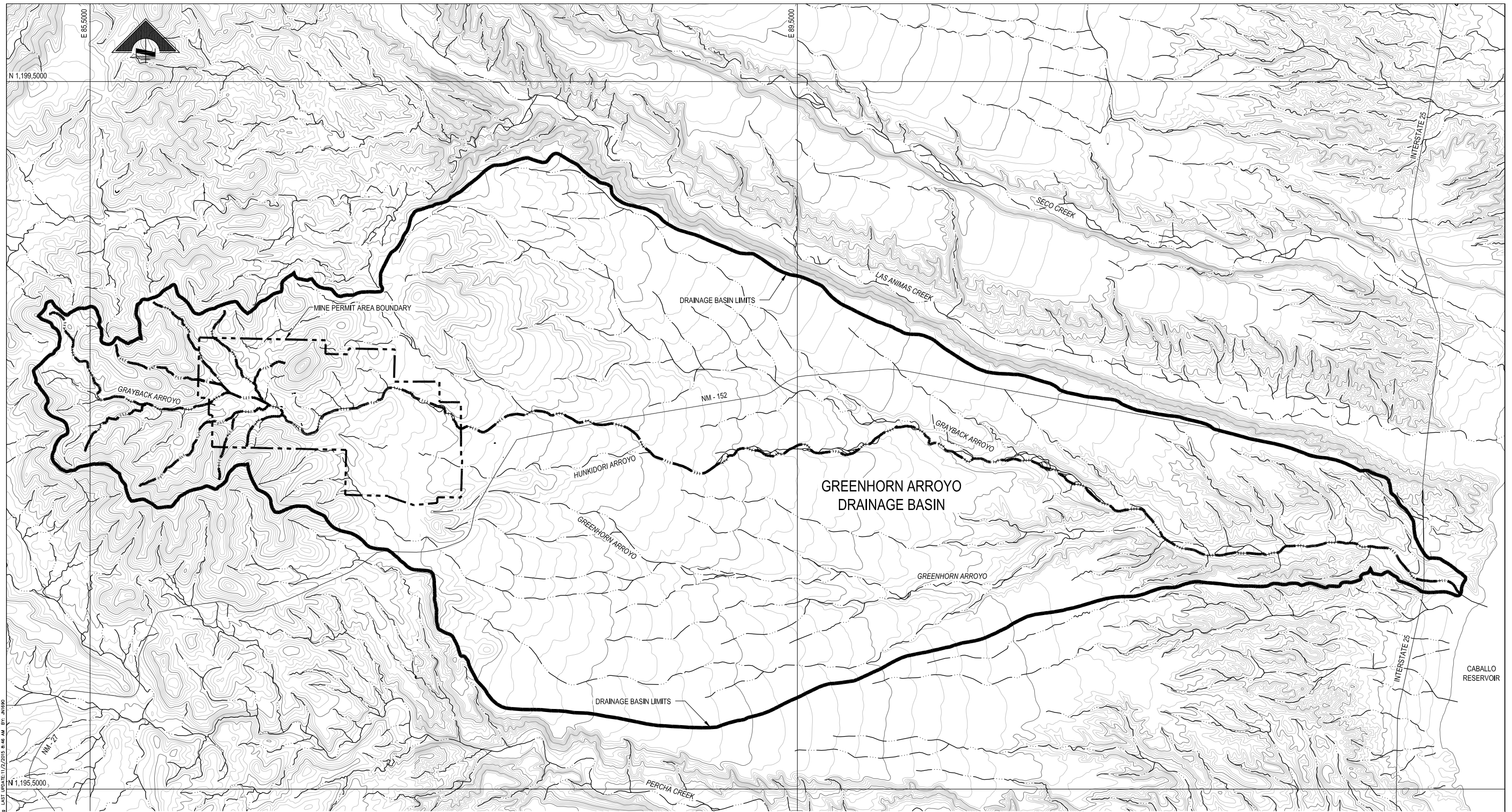


**20.6.7.11.J.(6)** *a description of the proposed method(s) to manage stormwater runoff and run-on to minimize leachate that may be discharged;*

NMCC proposes to utilize some existing water diversion structures as discussed in more detail below and construct other storm water management structures to manage run-on and run-off at the Copper Flat mine to minimize the potential for the discharge of leachate. M3 performed a peak discharge and volume analysis of the drainage areas contributing to Grayback Arroyo at the site. The purpose of the analysis was to evaluate the existing diversions and water conveyance features existing at the Copper Flat site as their adequacy in conveying flows from storm events and protecting the site from flooding. The return periods analyzed were the 100-year, 200-year, and 500-year, 24 hour storms. The capability of certain existing culverts within Grayback Arroyo to safely pass those storms without overtopping the proposed facility roadway or pipeline corridor was also analyzed. Appendix D contains the results of the analysis. Following is a synopsis of the results.

Figure 11J-21 shows the location of the Copper Flat mine permit area boundary within the Greenhorn Arroyo drainage basin watershed. It shows the Greenhorn Arroyo watershed as it existed prior to any mining. As shown in Figure 11J-21, the site is located in the head of the basin. Figure 11J-22 provides a closer view of the headwater drainage of the watershed in relation to the mine permit area boundary. Grayback Arroyo and its tributaries naturally begin as the headwaters of the drainage basin and converge at the western side of the site, transecting the mine permit area draining from west to east. The main-stem of Grayback Arroyo enters the western boundary of the site and several small unnamed arroyos, tributary to Grayback Arroyo, enter the site at the northwest and southwest corners of the site. As seen on Figure 11J-21, Hunkidori Gulch begins at the western edge of the site down-gradient of the operations, and drains to the east away from the site. Greenhorn Arroyo begins off of the southeast corner of the site down-gradient from the operations and also drains east. Hunkidori and Greenhorn Arroyos have no impact upon the Copper Flat facility as they are located down-gradient of the mine. Only Grayback Arroyo and its unnamed arroyos, which enter the site from the west, need be the subject of consideration with respect to surface water run-on and runoff management.

Pre-production site preparation activities conducted by Quintana Minerals in the early 1980's included the construction of diversion structures to Grayback Arroyo and unnamed arroyos to divert drainage around the site. These structures are shown in Figure 11J-23. Diversion structures were constructed to divert the headwaters of Grayback arroyo and its western tributaries as they entered the western site boundary to the south and east around the mine pit and the processing area. Another diversion structure, similar in purpose but smaller in size, was constructed at the northwest corner of the site to divert drainage from two small tributaries to Grayback Arroyo, diverting them to the east around Animas Peak away from the site into a sub-watershed that joins Grayback Arroyo east of the of the site boundary.



**LEGEND**

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

**SITE PLAN**

SCALE: 1" = 2500'



SCALE IN FEET  
CONTOUR INTERVAL = 50'  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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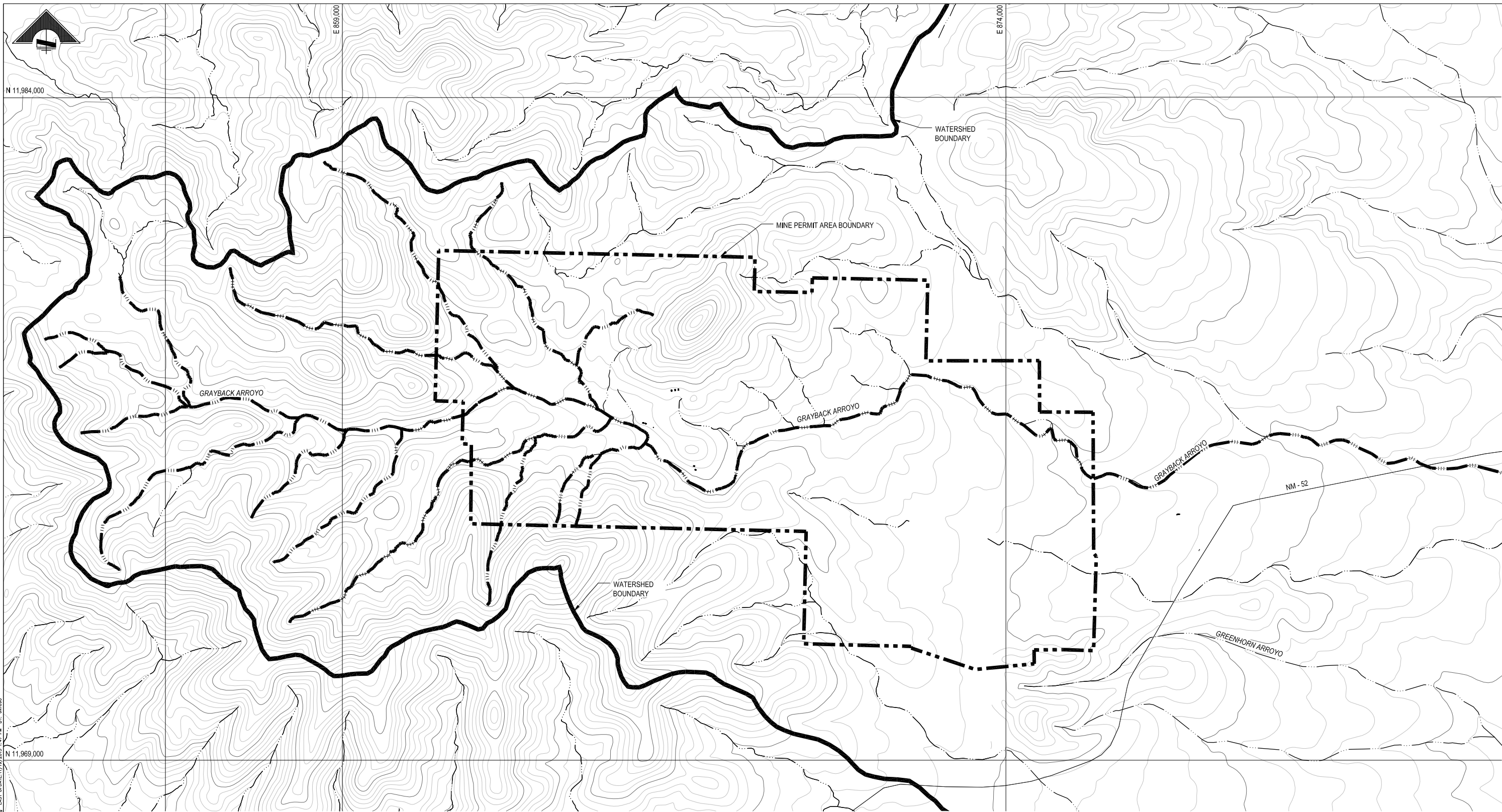
**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**GREENHORN ARROYO**  
**PRE-MINING WATERSHED BASIN**

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-100**  
REV. NO. P2 DATE 20 NOV 15

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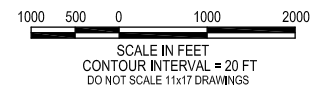




**LEGEND**

DRAINAGE BASIN BOUNDARY	
MINE PROPERTY BOUNDARY	
GRAYBACK ARROYO	
FLOWLINE	

**SITE PLAN**  
SCALE: 1" = 1000'



**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS				SCALE: 1" = 1000'	
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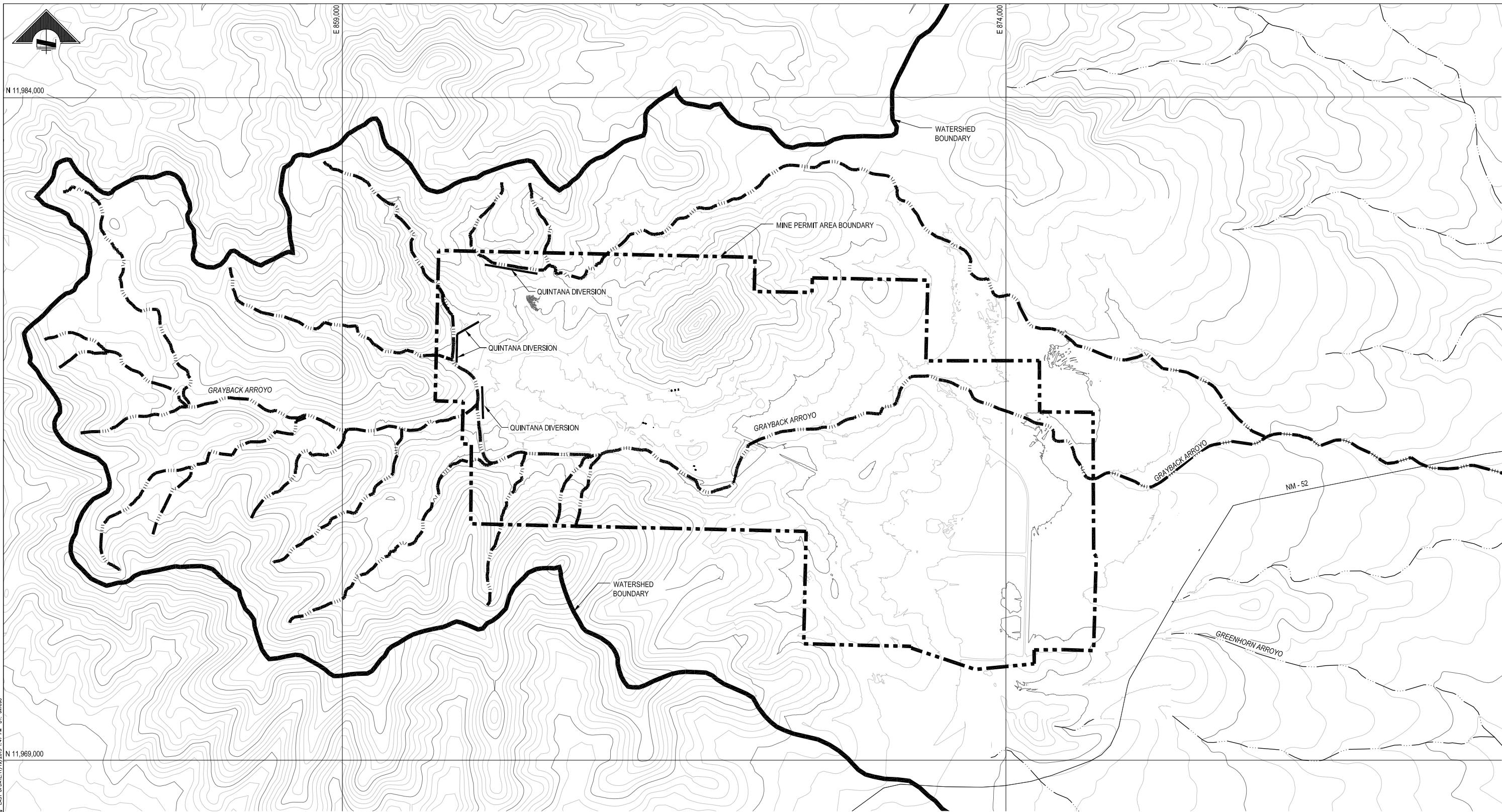
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**COPPER FLAT PROJECT**

GENERAL SITE  
COPPER FLAT SITE  
EXISTING HYDROLOGY  
PRE-QUINTANA MINING

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-101**  
REV. NO. P1  
DATE 20 NOV 15

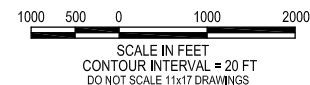
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**LEGEND**

DRAINAGE BASIN BOUNDARY	
MINE PROPERTY BOUNDARY	
GRAYBACK ARROYO	
FLOWLINE	

**SITE PLAN**  
SCALE: 1" = 1000'



**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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SCALE:	1" = 1000'	DATE	OCT 15
DESIGNED BY	JPN	CHECKED BY	JPN
DRAWN BY	JPN	PROJECT MGR	
CHECKED BY		CLIENT APPR.	

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**COPPER FLAT PROJECT**

**GENERAL SITE  
COPPER FLAT SITE  
EXISTING HYDROLOGY  
POST QUINTANA MINING**

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-102**  
REV. NO. P1 DATE 20 NOV 15

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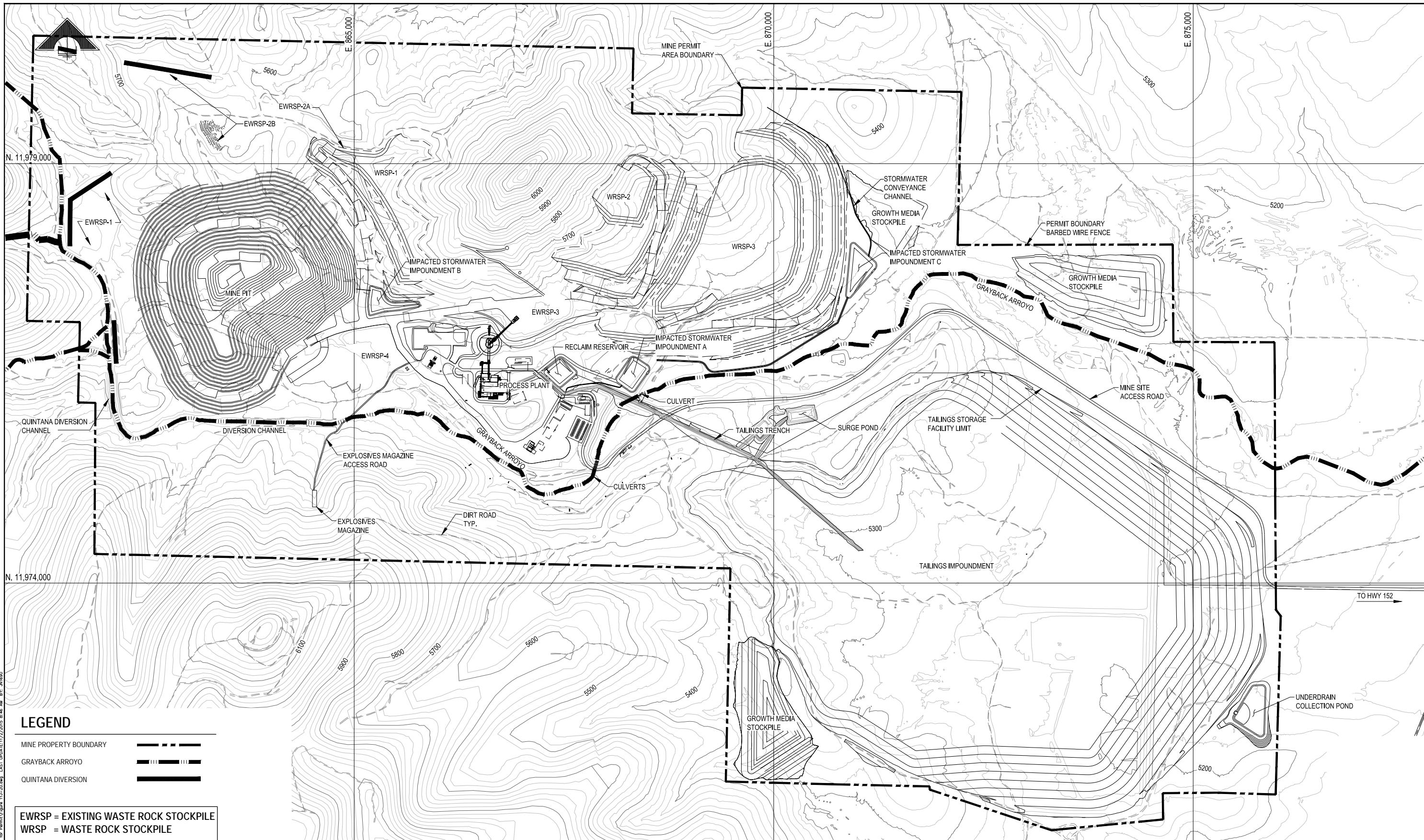
In addition to the diversion structures, Quintana installed large diameter culverts, as shown on Figure 11J-24, where the tailings transport pipeline and where the access road cross-over Grayback Arroyo. These structures are still in place and will be used to control storm water passing through the site. Figure 11J-24 also shows these structures in relation to NMCC's proposed Copper Flat mine facilities. The pre-mine development natural topography and drainage pattern of the site is shown in Figure 11J-25. Sixteen sub-basin watersheds naturally contributed to Grayback Arroyo. The upstream drainages merged in the central portion of the current Copper Flat Project area and passed through to the eastern boundary via Grayback Arroyo.

M3's analysis evaluated the diversions and culverts to determine their adequacy in conveying flows from storm events and protecting the site from flooding. Peak discharge and volume analysis for a 100-year, 200-year and 500 year 24-hour storm event was performed for drainage areas contributing to Grayback Arroyo located within the Copper Flat site area. Culvert and channel capacity analysis for the two culvert crossings was conducted for Grayback to determine water surface elevations during the design storm events. Peak flows were analyzed for each sub-basin contributing flow upstream of the site. Figure 11J-26 shows the upstream sub-basins in relation to the site.

Diversion of surface drainages away from the mining area was accomplished by Quintana when they developed the site by constructing the diversions described above. Future development of the site by NMCC will result in watershed changes to those shown on Figure 11J-25. Watershed areas no. 15 and 16 will be completely within open pit surface drainage area and will be eliminated as tributaries to Grayback Arroyo. Portions of watersheds 1, 2, 3 and 14 will be incorporated into the site storm water control area of the site and will no longer contribute directly to Grayback Arroyo. These differences can be seen by comparing Figure 11J-25 to 11J-26.

M3 evaluated the storm flows in the Grayback Arroyo drainage for the 100-year, 200-year, and 500-year 24-hour storm events for the pre-Quintana, i.e., natural conditions in comparison to Post-Quintana NMCC proposed site conditions. The results demonstrate that the existing diversion structures and culverts provide appropriate surface water management of the drainage that is protective of the site.

As NMCC has indicated earlier, it recognizes that there is an area along the western edge of developed WS D, and south of EWRSP-1, that could drain into the diversion of Grayback Arroyo instead of the mine pit. NMCC has corrected this condition in June 2016 by repairing the breach by constructing a rock and earth berm across the top of the breach and an upstream diversion swale to direct stormwater back to the open pit, to re-establish the integrity of the open pit surface drainage area. Otherwise, this diversion remains in place and functioning today. In order to manage run-on of storm water at the site, NMCC will continue to maintain the integrity of the diversion structure during and operating life of the mine and take it into consideration as part of reclamation and closure of the project.



**LEGEND**

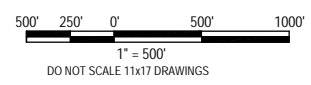
MINE PROPERTY BOUNDARY

GRAYBACK ARROYO

QUINTANA DIVERSION

EWRSP = EXISTING WASTE ROCK STOCKPILE  
WRSP = WASTE ROCK STOCKPILE

**SITE PLAN**  
SCALE: 1:500



**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES		REFERENCES		REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT

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DESIGNED BY: JPN	OCT 15
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CLIENT APPR:	

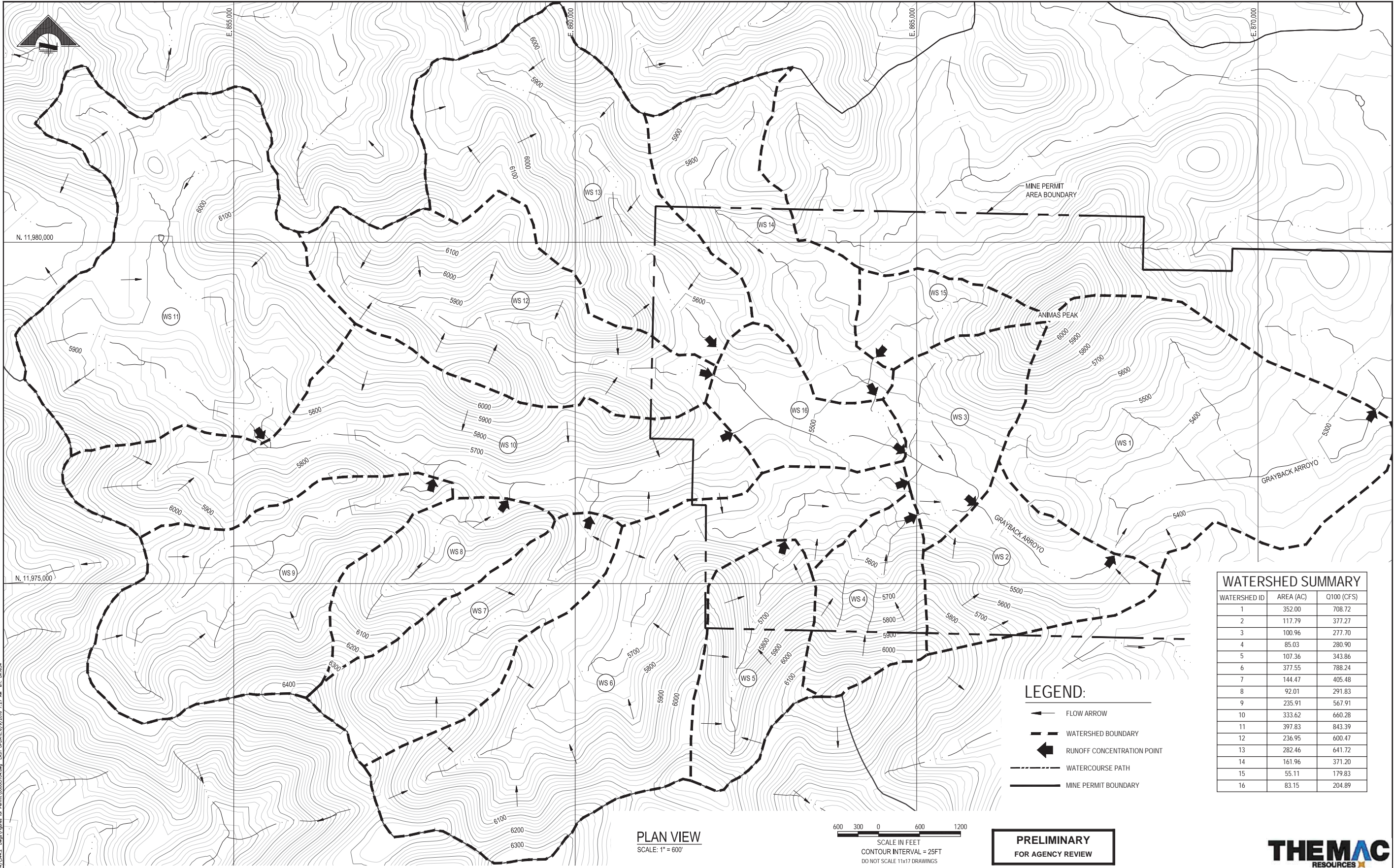
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**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL**  
**GRAYBACK ARROYO DIVERSION THROUGH NMCC PROJECT SITE**

JOB NO. M3 PN-12085  
DWG. NO. **000-CI-103**  
REV. NO. P1 DATE 20 NOV 15

File: P:\2015\20085\Civil (044)\44.2 Draw\Figures for Permit\Figure 11J-20.dwg, LAST UPDATE: 11/2/2015 8:46 AM BY: JN1600



WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
1	352.00	708.72
2	117.79	377.27
3	100.96	277.70
4	85.03	280.90
5	107.36	343.86
6	377.55	788.24
7	144.47	405.48
8	92.01	291.83
9	235.91	567.91
10	333.62	660.28
11	397.83	843.39
12	236.95	600.47
13	282.46	641.72
14	161.96	371.20
15	55.11	179.83
16	83.15	204.89

- LEGEND:**
- FLOW ARROW
  - - - WATERSHED BOUNDARY
  - ◀ RUNOFF CONCENTRATION POINT
  - · - · - WATERCOURSE PATH
  - MINE PERMIT BOUNDARY

**PLAN VIEW**  
SCALE: 1" = 600'

SCALE IN FEET  
CONTOUR INTERVAL = 25FT  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: 1" = 600'			
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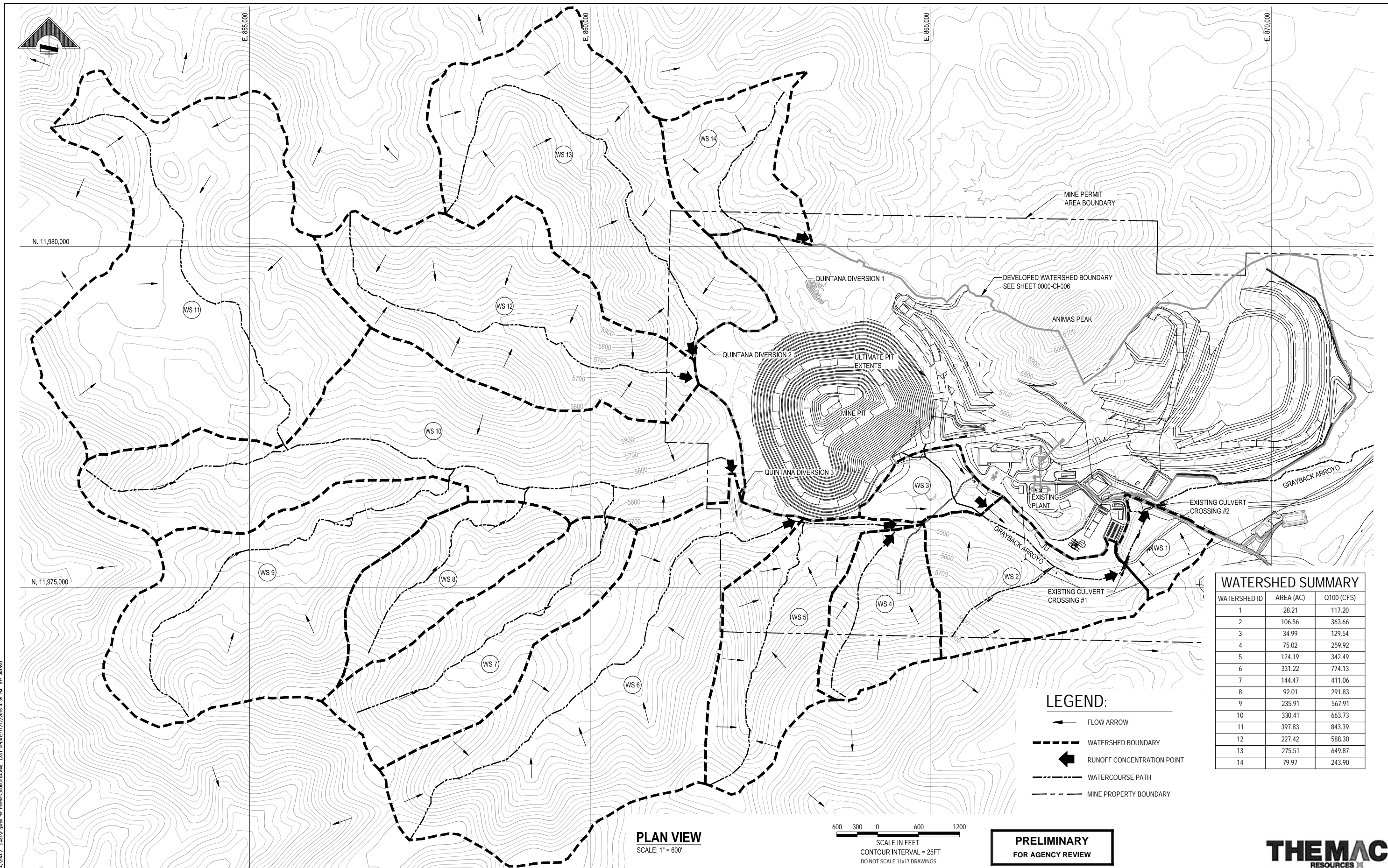
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**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL PRE QUINTANA EXISTING WATERSHED AREAS**

JOB NO. M3 PN-120885  
DWG. NO. **Figure 11J-25**  
REV. NO. P2 DATE 09 JUN 16

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WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
1	28.21	117.20
2	106.56	363.66
3	34.99	129.54
4	75.02	259.92
5	124.19	342.49
6	331.22	774.13
7	144.47	411.06
8	92.01	291.83
9	235.91	567.91
10	330.41	663.73
11	397.83	843.39
12	227.42	588.30
13	275.51	649.87
14	79.97	243.90

- LEGEND:**
- ← FLOW ARROW
  - WATERSHED BOUNDARY
  - ⬇️ RUNOFF CONCENTRATION POINT
  - - - WATERCOURSE PATH
  - - - MINE PROPERTY BOUNDARY

**PLAN VIEW**  
SCALE: 1" = 600'

600 300 0 600 1200  
SCALE IN FEET  
CONTOUR INTERVAL = 25FT  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES		REFERENCES		REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD.	DATE	CLIENT
0000-CI-006	DEVELOPED WATERSHED AREAS														

SCALE: 1" = 600'	DATE:
DESIGNED BY: AJE	OCT 15
DRAWN BY: AJE	OCT 15
CHECKED BY:	
PROJECT MGR:	
CLIENT APPR:	

**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**POST QUINTANA**  
**EXISTING WATERSHED AREAS**

JOB NO. M3 PN-12085  
DWG NO. **0000-CI-104**  
REV NO. P1  
DATE 20 NOV 15

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Also, as indicated in the Conclusions section of M3's Appendix D report, NMCC will repair the upstream inlets to the culverts and to remove vegetation at the culvert inlets and outlets to ensure safe passage of water.

Storm water runoff from the various areas at the site where it need be considered will be managed at the Copper Flat mine in a manner appropriate for that location. At Copper Flat there are four areas of the site that require storm water management;

- open pit mine
- process area, administration and buildings ancillary facilities, parking areas
- waste rock stockpile areas
- tailings storage facility

### **Open Pit Mine**

The open pit mine is located at the top of the watershed area for Grayback Arroyo. The small amount of watershed area located up-gradient from the pit is diverted around and away so that it cannot contribute any run-on into the mine. Only precipitation that falls directly onto the open pit surface drainage area contributes run-on to the pit. As discussed previously, a portion of the open pit drainage area, i.e., the surface water from the open pit surface drainage area where WRSP-1 will be located, will be managed by diverting runoff into an impoundment in order to limit the amount of surface water entering the mine pit and to assist in water harvesting.

Water collected in the mine pit during operation will be pumped out and either utilized for dust control or in the process circuit. At closure, the water will be allowed to collect in the pit and contribute to the volume of the pit lake as described in the Reclamation and Closure Plans.

### **Processing Facility**

The processing facility, including crushing and grinding, milling, flotation, concentrating, drying and packaging, administration, parking and all other ancillary support facilities will be located as shown on Figure 11J-1. This area will be graded and berms and other retainment structures constructed to eliminate storm water run-on such that there will be no run-off from this watershed area released. The area will be graded and contoured to collect all precipitation in storm water impoundment A. As discussed previously, details of the design of this impoundment are contained in Appendix B. Water captured in the impoundment will be transferred to the process water reservoir and utilized as make-up water in the process.

At closure, the impoundment will be filled, re-contoured, re-graded and covered as described in the Reclamation and Closure Plans so that water is no longer retained.

### **Waste Rock Stockpiles**

The new proposed waste rock stockpiles will be located as shown in Figure 11J-1 and 11J-3. Berms and drain ditches will be constructed around the toe of the stockpile to eliminate



storm water run-on through the stockpile area. As the areal extent of the stockpile expands over time the location of the berms and ditches will be moved to continue to provide run-on control. The surface of each waste rock stockpile will be contoured and graded in a manner to control and divert run-off from each stockpile to an impacted storm water impoundment. Storm water control ditches will be constructed, as necessary, to control run-off and minimize the potential for ponding and erosion while allowing water to move quickly and easily to the impoundment thus minimizing the opportunity for infiltration and leaching. Water captured in the impoundments will be transferred to the process water reservoir and utilized as make-up water in the process. At closure, the waste rock stockpiles and the impoundments will be re-contoured, re-graded and covered as described in the Reclamation and Closure Plans.

### **Tailings Storage Facility**

The Tailings Storage Facility will be located as shown in Figure 11J-1. The area is naturally protected from storm water runoff passing through Grayback Arroyo by the natural topography. As such, there will only be run-on and runoff from precipitation that falls directly on the footprint of the TSF to manage.

The tailings dam will be constructed in phases as discussed in detail in Appendix A. Surface water run-on will be controlled during operations by grading and contouring the area to minimize the amount of water contributed into the tailings impoundment. Several run-on diversion ditches will be constructed at the up-gradient perimeter of the tailings impoundment to divert run-on around it per the requirements of the NM Office of the State Engineer. These diversion ditches will be reconstructed for each phase of dam construction, as needed, as the dam expands in size until it reaches its ultimate height.

Runoff from the exterior face of the dam will be managed by constructing drainage ditches to control and direct runoff into the underdrain collection pond located at the southeastern toe of the dam. These drainage ditches will be constructed in a manner that minimizes the opportunity for ponding yet moves water in a controlled fashion to control erosion. The ditches on the face of the dam will enter a lined water conveyance ditch located at the toe of the dam. That ditch will also contain the water recycle pipeline leading from the underdrain collection pond to the process facility. Runoff water from the downstream face of the dam will join the water collected from the tailings underdrains in the pond and will be transported to the process water reservoir for use as make-up water in the process.

The surface area of the cyclone plant, immediately adjacent to the TSF, as shown on Figure 11J-1, will be contoured and graded to route runoff from the area into the lined water conveyance ditch at the toe of the dam. It will drain to the underdrain collection pond.

At closure the TSF and its attendant runoff diversion structures will be re-contoured, re-graded, filled and a cover will be placed over the entire area in accordance with the Reclamation and Closure Plans.





**20.6.7.11.J.(7)** *a description of water wells and monitoring wells, including information for each well regarding its location, construction material, dimensions and capacity;*

NMCC has prepared and submitted a number of documents and reports containing the information requested regarding a description of water wells which include information regarding location, construction material, dimensions and capacity, to the extent possible. These documents include;

- Baseline Data Report, June 2012 and Supplements, (Intera 2012)
- Discharge Permit Application and Abatement Plan, March 2011 (Intera 2011)
- Abatement Plan Amendment, October 2011, (JSAI 2011)
- Results From 1<sup>st</sup> year Stage 1 Abatement Investigation, (JSAI 2014b)
- Mine Plan of Operations, December 2010 and amendments, (NMCC 2010)
- Model of Groundwater Flow, August 2014, (JSAI 2014c)

These documents have been incorporated into this Discharge Permit Application by reference and are available at NMED.

With regard to monitoring wells, the Baseline Data Report, Abatement Plan and Model of Groundwater Flow all contain monitoring data that was compiled at the site and surrounding environs. In addition, NMCC has developed a proposed groundwater Quality Monitoring Plan, as required by **20.6.7.11.R and 20.6.7.28 NMAC** that incorporates some of the wells contained in these reports in combination with proposed installation of others to monitor site conditions prior to and during operation of the mine. The details of this plan, including the location, construction material (existing and proposed), dimensions, and capacity, are included in Appendix E of this application.



**20.6.7.11.J.(8)** *a description of flow meters required pursuant to the copper mine rule or a discharge permit and fixed pumps for discharge of process water, tailings and impacted stormwater;*

Subsection **20.6.7.11.S** of this application as required by the Copper Rule contains the information requested in this subsection.



**20.6.7.11.J.(9)** *a description of any surface water(s) of the state and any other springs, seeps, ditch irrigation systems, acequias, and irrigation canals and drains located within the boundary of the copper mine facility;*

The only surface water that exists within the boundary of the Copper Flat mine permit area is Grayback Arroyo. Grayback is an ephemeral arroyo that transects the mine permit area from the west to the east, roughly through the middle of the area as shown on Figure 11J-21 through 26 and 11K-8.

There is one seep that occasionally generates water in response to significant precipitation located on the sidewall of the existing open pit. The pit lake itself may or may not be considered a water of the state. However, it will be completely dewatered during the operating life of the mine.

There are no springs, ditch irrigation systems, acequias, irrigation canals or drains located within the boundary of the Copper Flat mine permit area.



**20.6.7.11.J.(10)** *a description of proposed sampling locations;*

The proposed sampling locations for tailings and process water will be coincident with the locations identified in Subsection **2.6.7.11.S** for the location of the flow metering devices. The other sampling locations are identified in the Water Quality Monitoring Plan required by **20.6.7.11.R** and **20.6.7.28 NMAC** of the Copper Rule.



**20.6.7.11.J.(11)** *a description of all septic tanks and leach fields used for the disposal of domestic wastes;*

NMCC will not use a septic tank and leach field treatment systems for disposal of domestic wastes at the Copper Flat Mine. Disposal of domestic wastes generated will be accomplished by installing a single packaged wastewater treatment plant to serve the majority of employees and visitors at the mine. Individual portable toilet facilities for outlying areas of the operation will also be utilized, as needed.

The packaged wastewater treatment plant will receive and treat domestic wastes from buildings located in the administration, concentrator, and mine shop areas. The packaged system will be sized for a load based on the number of mine employees and visitors expected at the mine during a 24-hour period and applying an average water use of 50 gallons per day per person. Breaking down the employee headcount for the mine by the planned rotation schedule indicates 160 employees per day will be using facilities connected to the package plant. In addition to mine employees, an additional 40 persons per day are assumed to account for visitors and contractors. Based on these figures, a 10,000 gallon per day plant has been selected for Copper Flat Mine.

The plant will be located on a pre-existing concrete slab near the main gate as shown in Figure 11J-1. The plant will generate effluent treated to secondary treatment levels. Treated effluent from the plant will be piped to the tailings storage facility for disposal in the impoundment and recycling to the process water reservoir. System specifications and installation will conform to State and local regulations.

Individual portable toilet facilities will be provided for employees working in outlying areas of the operation such the pit, mine stockpile areas, the primary crusher, and the TSF. The portable toilets will be maintained by a licensed contractor on a regular basis.

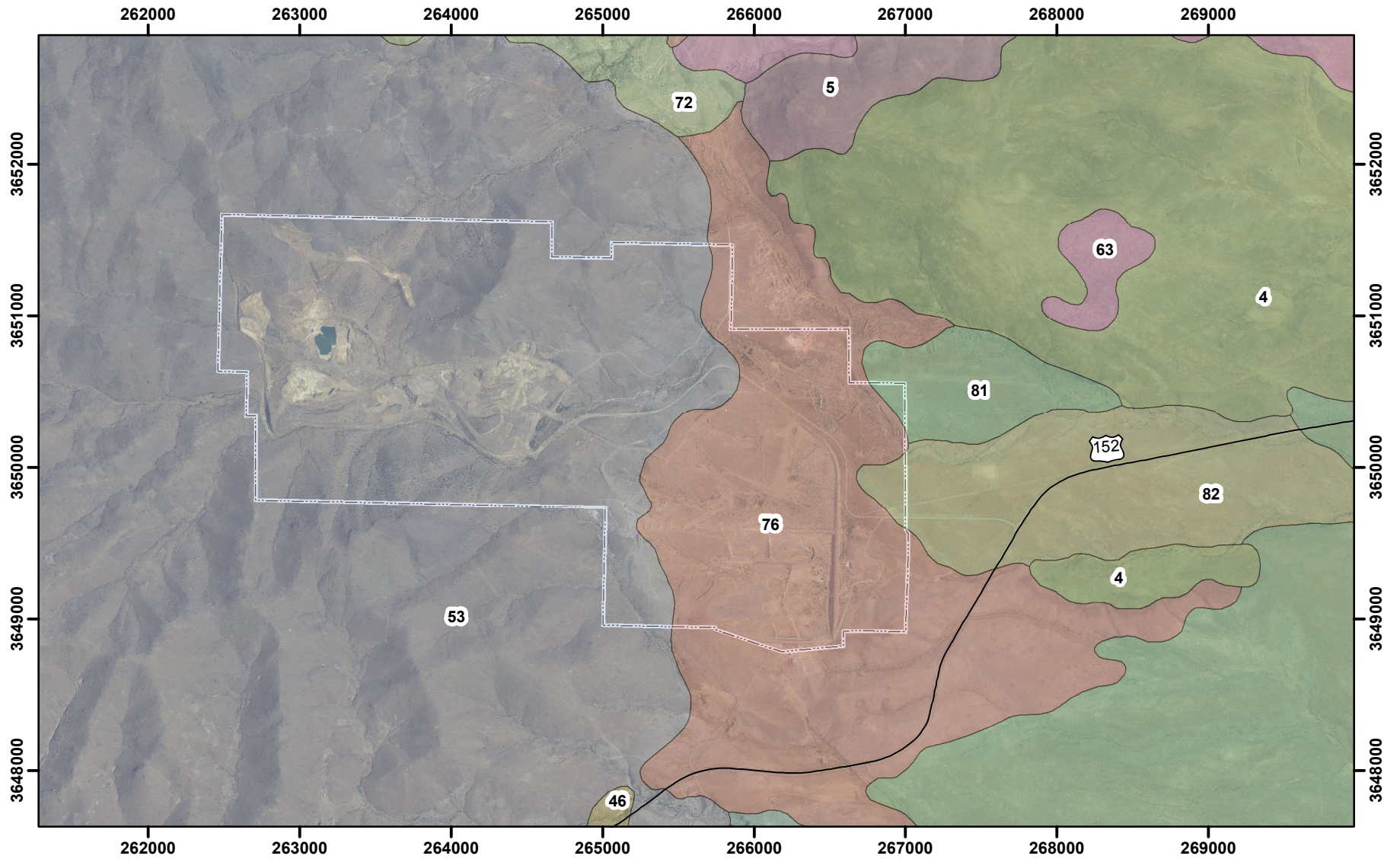


## **20.6.7.11.K SURFACE SOIL SURVEY, GEOLOGY, AND HYDROLOGY**

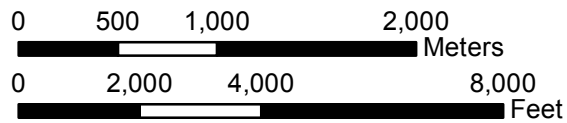
*An application shall include:*

**20.6.7.11.K.(1)** *the most recent regional soil survey map and associated descriptions identifying surface soil type(s);*

Figure 11K-1 is a soil survey map of soils in the mine permit and surrounding area. The map includes a description identifying the surface soil types present. In addition, Section 6.0 of the Baseline Data Report (Intera 2012) contains more detailed information regarding soils in the area.



Map Scale: 1:38,000 If printed on 8.5" x 11" sheet



**Legend**

- Roads
- ⬜ Copper Flat Mine Permit Area Boundary
- 4 Akela very gravelly loam, moderately rolling
- 5 Akela-Rock outcrop association, very steep
- 46 Ildefonso-Scholle association, hilly
- 53 Luzena-Rock outcrop association, very steep
- 63 Nickel-Chamberino association, gently sloping
- 72 Rock outcrop-Deama association, extremely steep
- 76 Scholle-Ildefonso association, moderately rolling
- 81 Tres Hermanos gravelly fine sandy loam, gently sloping
- 82 Tres Hermanos-Hap association, gently sloping

<b>Soil Survey</b>		Figure 11K-1
Environmentally Responsible. Community-Minded. Local Opportunities.		June 24, 2015

Source: Baseline Data Characterization Report for Copper Flat Mine, June 2012, prepared by Intera et al., and Stetson Engineers.



**20.6.7.11.K.(2)** *a geologic map covering the area within a one-mile radius of the copper mine facility and geologic and lithological information which provides a geologic profile of the subsurface conditions beneath the copper mine site, including the thickness of each geologic unit, identification of which geologic units are water bearing, cross sectional diagrams and sources of all such information; and*

Figure 11K-2a is a geologic map of the area within a one-mile radius of the Copper Flat mine permit area boundary. A prominent feature on this map, as well as other geologic maps prepared by JSAI on behalf of NMCC, is the East Animas Fault. Some discussion has occurred between NMCC, its' consultant JSAI, and the regulatory agencies with respect to the most accurate location of this feature. A correct interpretation of the data generated over time is necessary in order to have a correct understanding of the geology of this area. NMCC offers the following interpretation, which it believes represents the most accurate information available.

The East Animas Fault Trend is a north-south normal fault that forms boundary a between the Animas Uplift and Palomas Basin. Figure 11K-2b shows the various locations the East Animas Fault has been mapped as presented in various published documents. All previous researchers identify the East Animas Fault as trending south to north and down thrown on east side. The difference in its location near the Copper Flat mine permit boundary has been with respect to position in longitude, i.e., east to west position.

Key references for the East Animas Fault include Kelley et al. (1979), Seager et al. (1982), Harrison et al.(1993), Beaumont (2011), Hawley (2012), JSAI (2013), JSAI (2014), JSAI (2014a), and Koning et al. (2015). Kelly mapped the fault further to the east than more recent mapping has shown it to be. As such the Kelly location is not depicted on Figure 11K-2b. The East Animas Fault was mapped as an inferred fault in slightly different longitude by Seager et al. (1982) than by Hawley (2012) as shown on Figure 11K-2b.

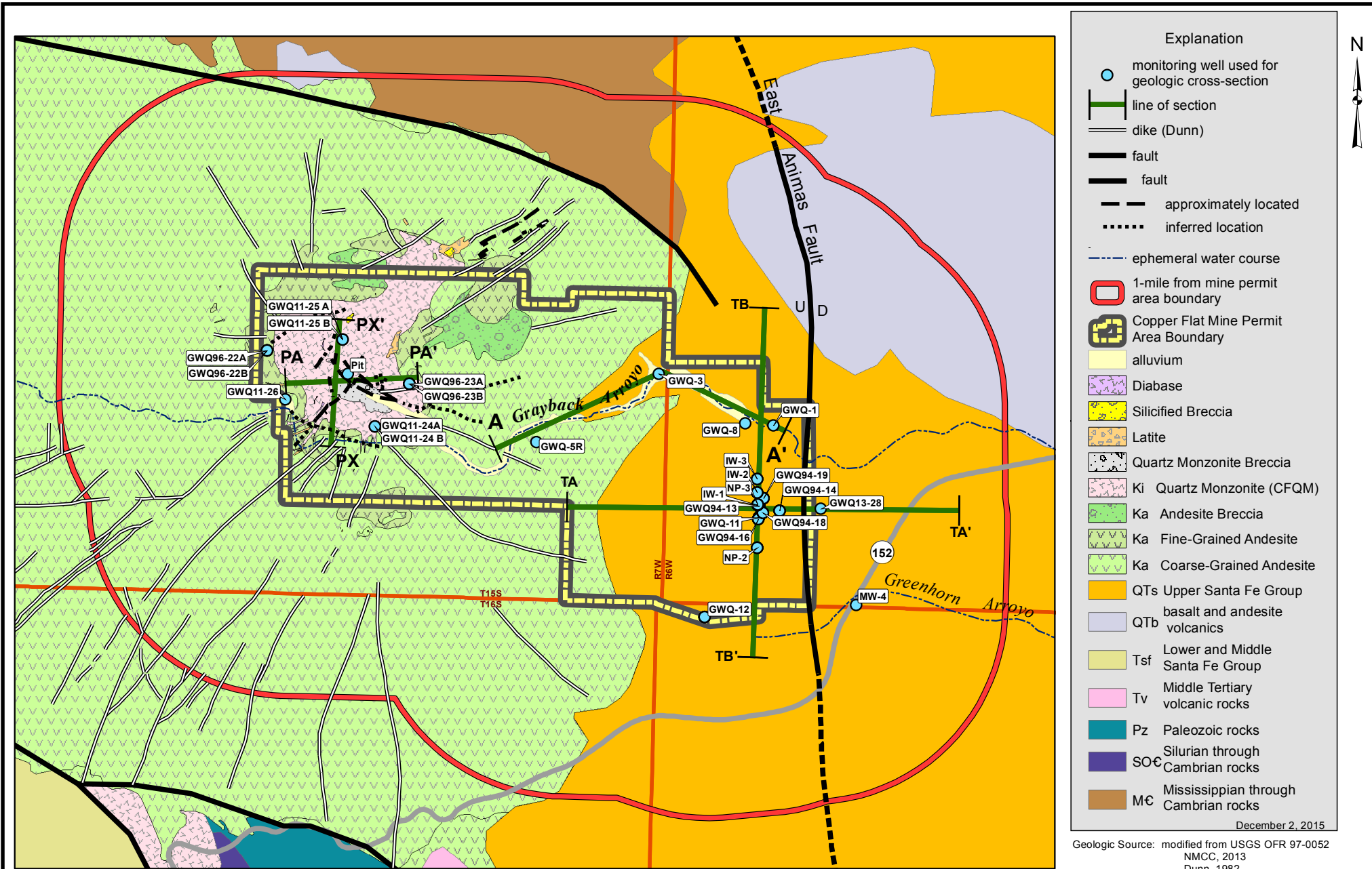
Work performed by Beaumont (2011) and JSAI (2013) was based on analysis of well logs and lineaments identified from aerial photographs. During the 2013 evaluation, the East Animas Fault was known to exist between wells GWQ94-21(A,B) and MW-4, which narrowed the longitude position to an approximate 1,800 ft. wide zone. In 2013, monitoring well GWQ13-28 was drilled as part of Stage 1 abatement Investigation (JSAI, 2014a) to further define the longitude position of the East Animas Fault. GWQ13-28 was determined to be located on the east side of the fault, as determined by the lithologic differences between GWQ94-21(A,B) and GWQ13-28 and the 150 ft. drop in water levels between GWQ94-21(A,B) and GWQ13-28. The longitude position of the East Animas Fault provided by Beaumont (2011) was slightly east of GWQ13-28, so JSAI adjusted the longitude position to half way between GWQ94-21 and GWQ13-28 in JSAI (2014) and JSAI (2014a) to more accurately reflect the new data. Koning et al. (2015) consulted with JSAI on the most current well drilling data and geologic analysis of faults for the Skute Stone Quadrangle map, and at the time were given the East Animas Fault as mapped by Beaumont (2011) and presented in JSAI (2013). Koning et al. (2015) has not updated the





longitude location of the East Animas Fault to reflect the results of GWQ13-28 as reported in JSAI (2014) and JSAI (2014a).

Figures 11K-3a and 3b through 11K-7a and 7b provide lithological information showing a profile of the subsurface conditions beneath the site at cross-sections identified in Figure 11K-2a. These cross sections provide information regarding the thickness of each geologic unit, identification of which geologic units are water bearing, as well as the source of the information. Figures 11K-3a, 6a and 7a depict the cross-sections in vertical exaggeration to provide the reviewer with more detail. Figures 11K-3b, 6b and 7b provide a 1:1 vertical to horizontal depiction of the same cross-sections.



**Explanation**

- monitoring well used for geologic cross-section
- line of section
- dike (Dunn)
- fault
- fault
- approximately located
- inferred location
- ephemeral water course
- 1-mile from mine permit area boundary
- Copper Flat Mine Permit Area Boundary
- alluvium
- Diabase
- Silicified Breccia
- Latite
- Quartz Monzonite Breccia
- Ki Quartz Monzonite (CFQM)
- Ka Andesite Breccia
- Ka Fine-Grained Andesite
- Ka Coarse-Grained Andesite
- QTs Upper Santa Fe Group
- basalt and andesite volcanics
- Tsf Lower and Middle Santa Fe Group
- Tv Middle Tertiary volcanic rocks
- Pz Paleozoic rocks
- SOc Silurian through Cambrian rocks
- Mc Mississippian through Cambrian rocks

December 2, 2015

Geologic Source: modified from USGS OFR 97-0052  
 NMCC, 2013  
 Dunn, 1982

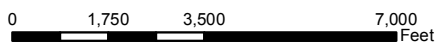


Figure 11K-2a. Geologic map within 1-mile of the Copper Flat Mine Area permit boundary, Sierra County, New Mexico.

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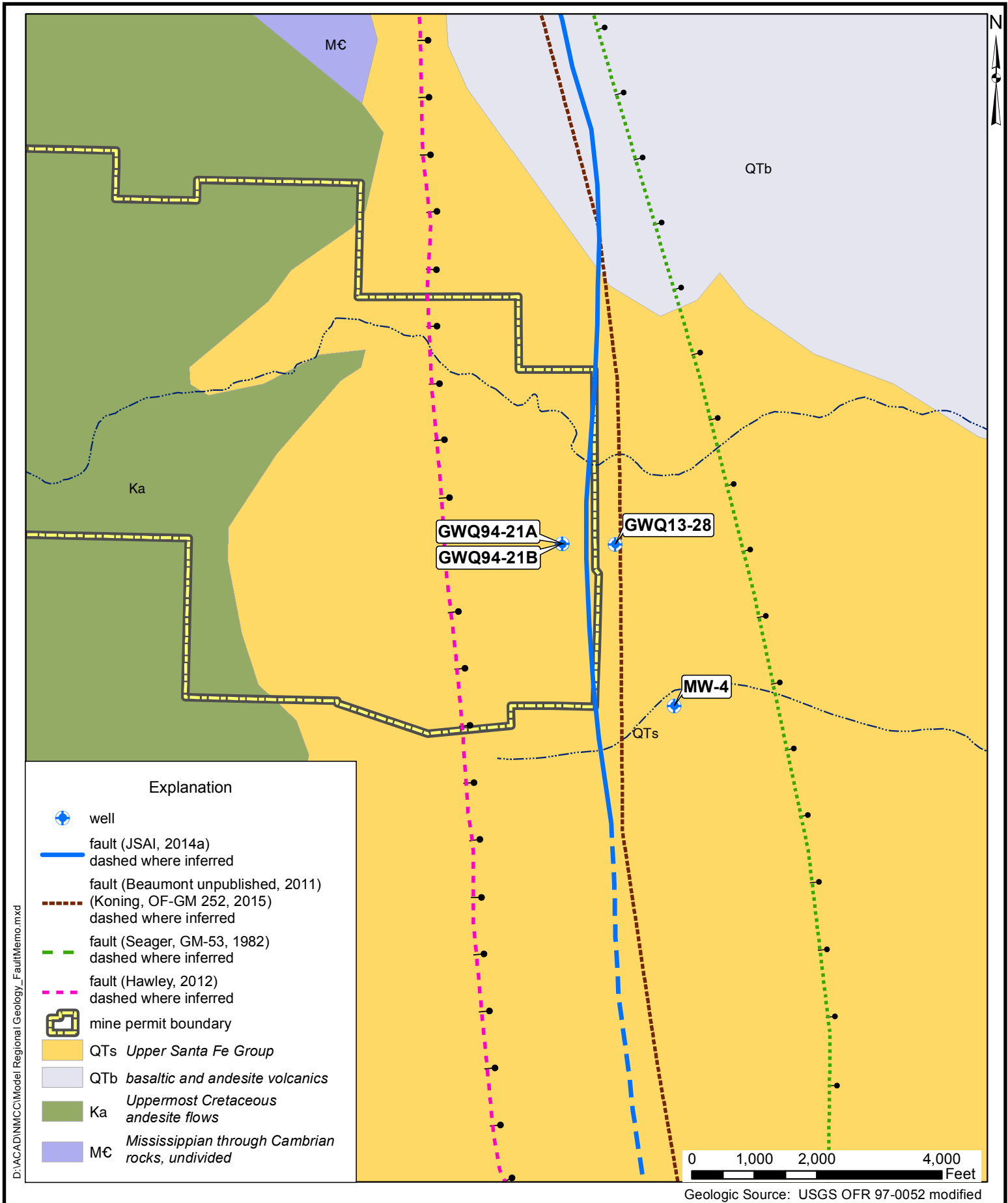


Figure 11K-2b. Regional surface geology and location of East Animas Fault as reported by various investigations.

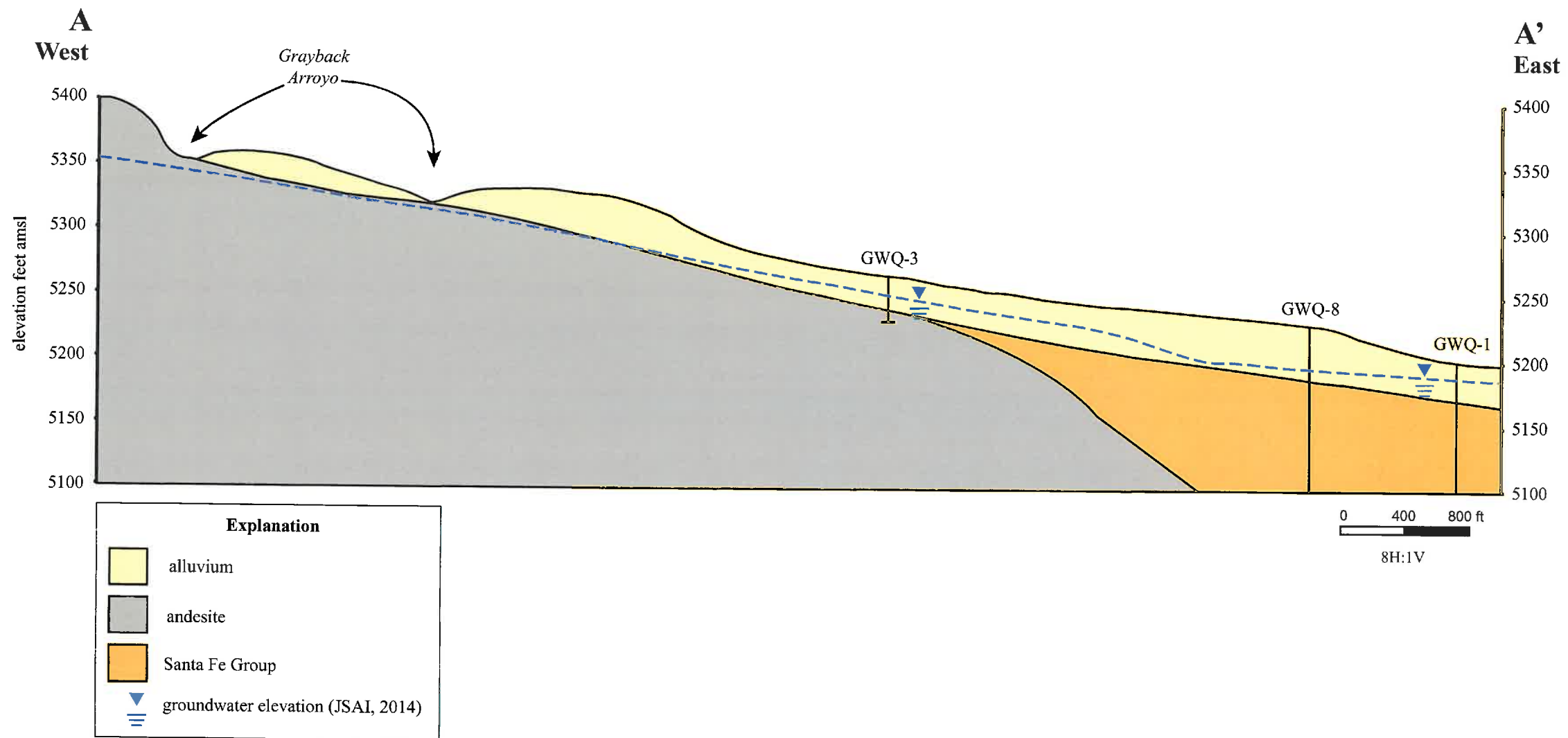


Figure 11K-3a. West to east geologic cross-section along Grayback Arroyo, Copper Flat Mine, Sierra County, New Mexico.

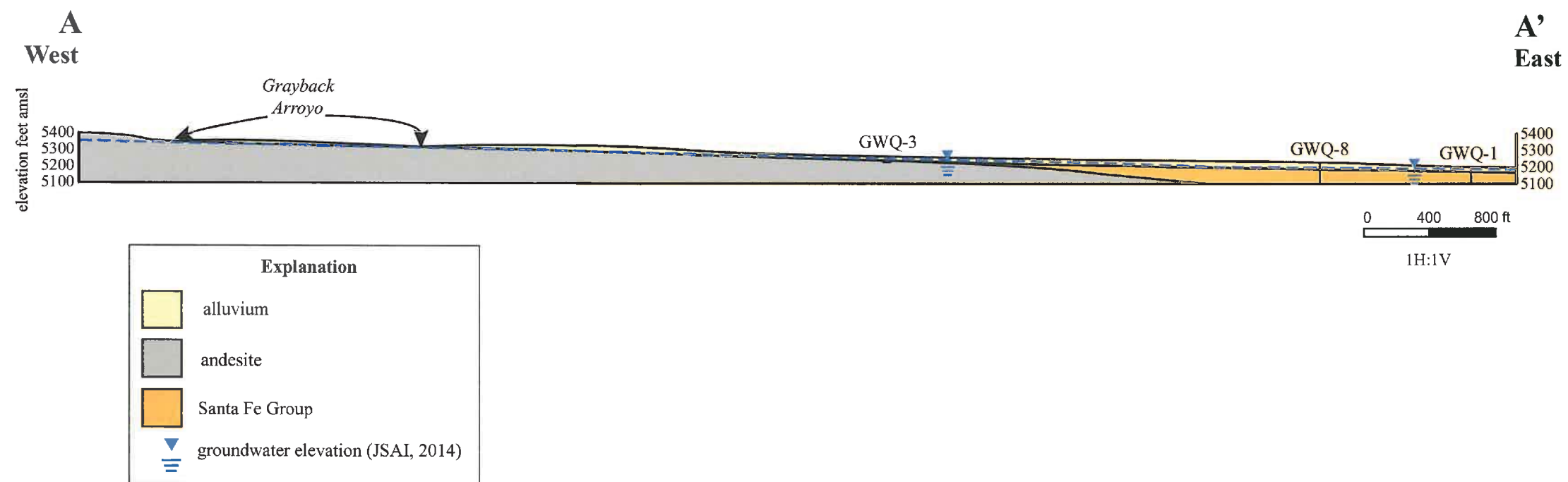


Figure 11K-3b. West to east geologic cross-section along Grayback Arroyo, Copper Flat Mine, Sierra County, New Mexico.

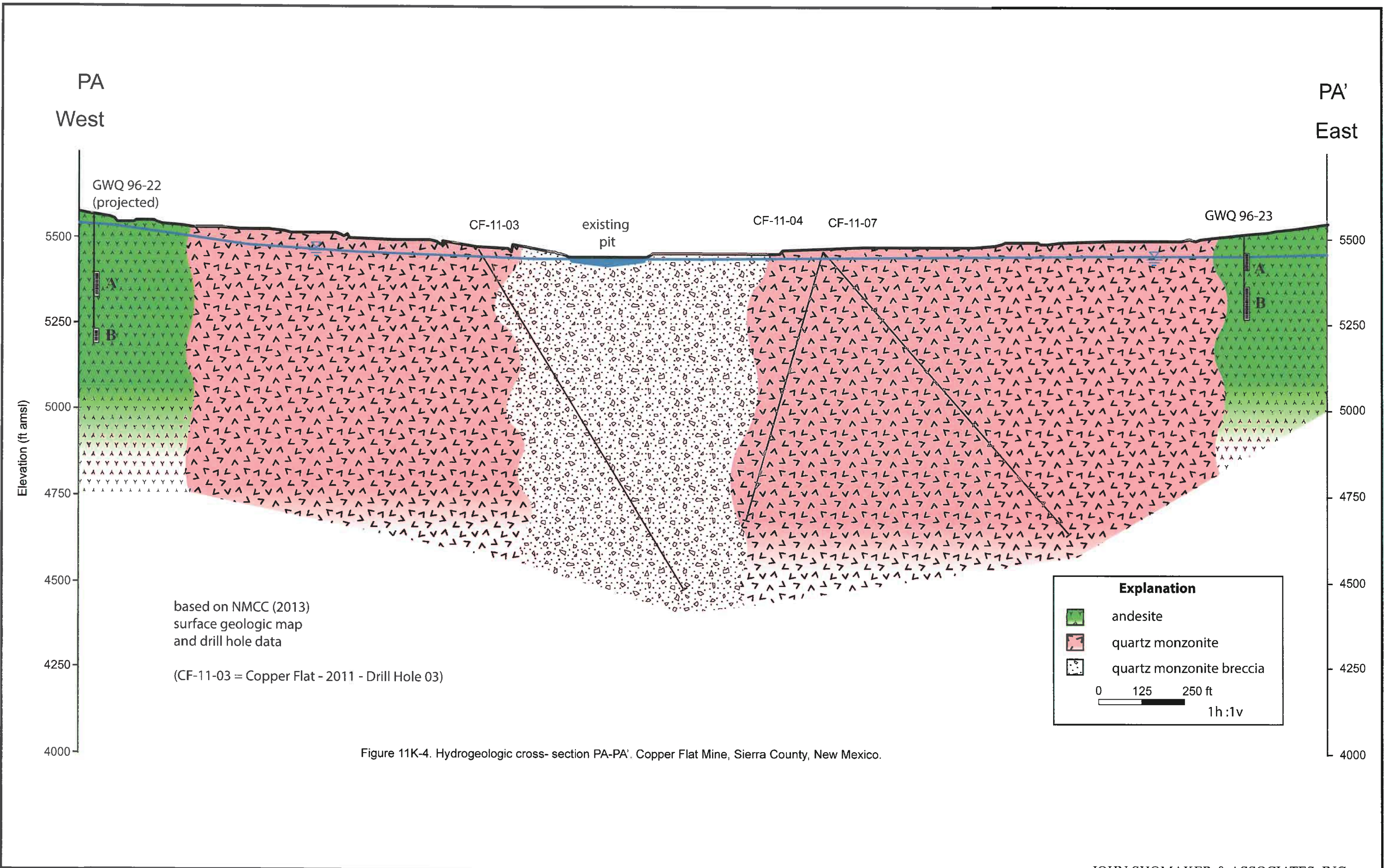
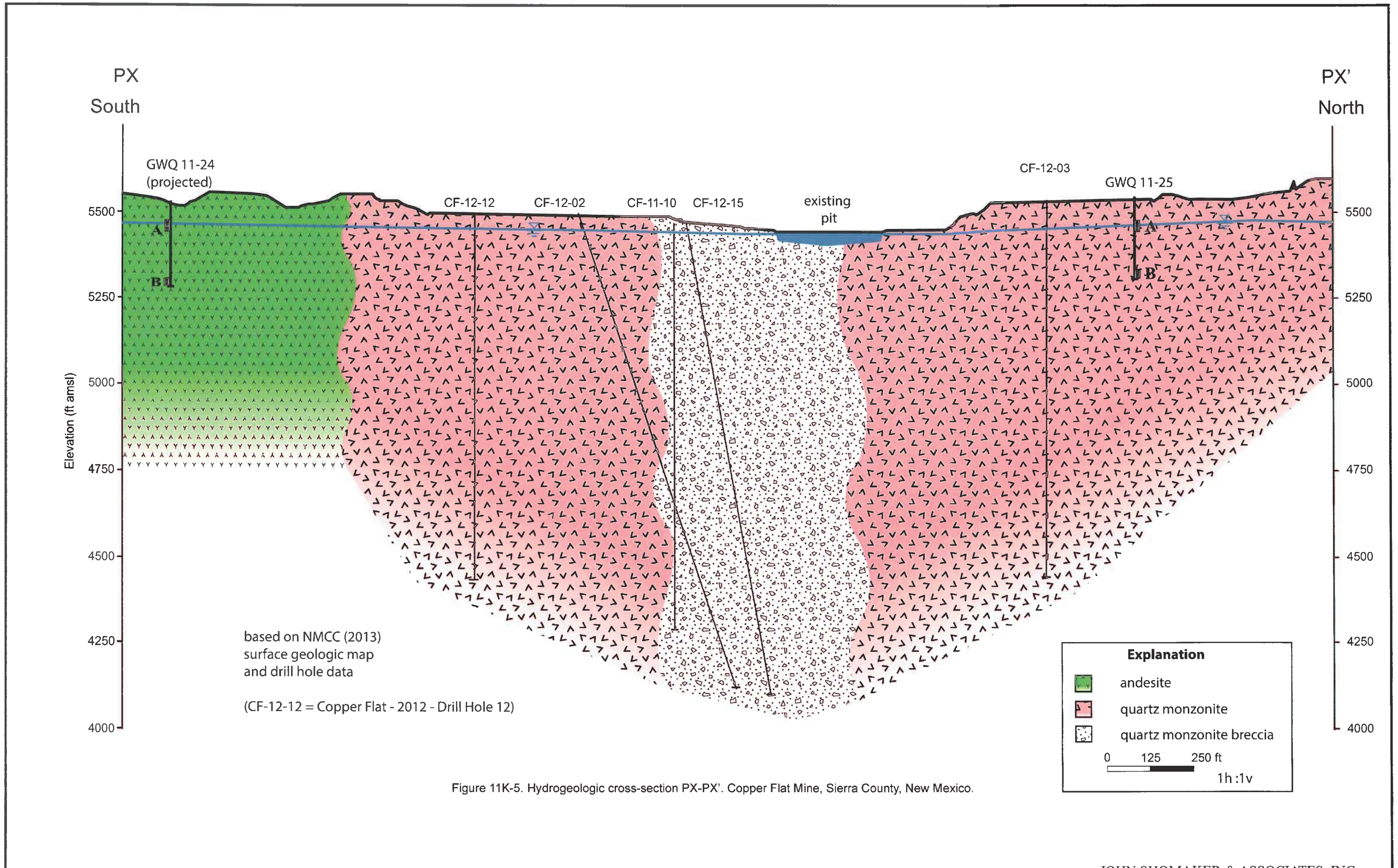


Figure 11K-4. Hydrogeologic cross-section PA-PA'. Copper Flat Mine, Sierra County, New Mexico.



based on NMCC (2013)  
 surface geologic map  
 and drill hole data  
  
 (CF-12-12 = Copper Flat - 2012 - Drill Hole 12)

Figure 11K-5. Hydrogeologic cross-section PX-PX'. Copper Flat Mine, Sierra County, New Mexico.

Explanation	
	andesite
	quartz monzonite
	quartz monzonite breccia
0    125    250 ft 	
1h:1v	

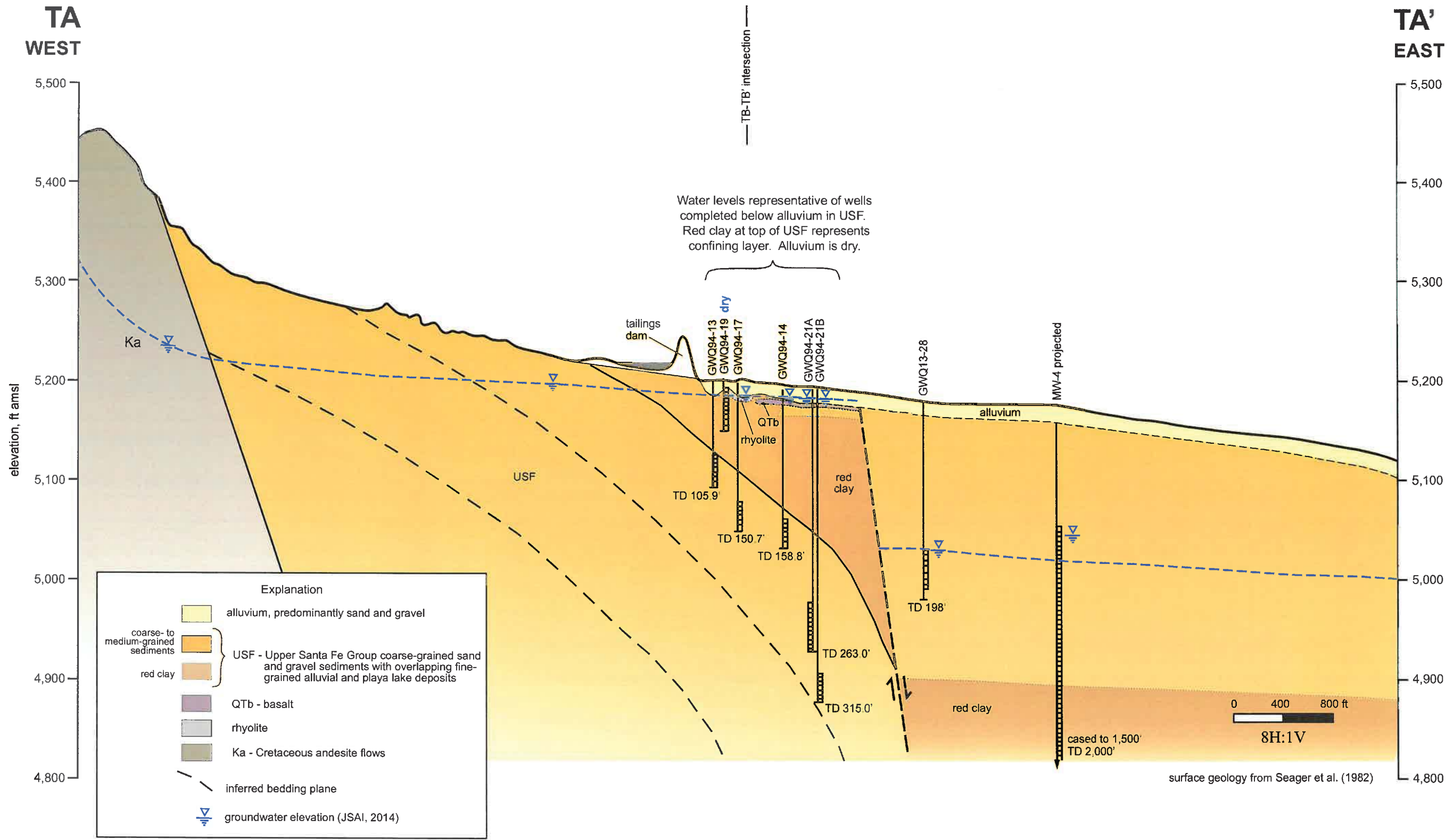


Figure 11K-6a. West to east geologic cross-section TA-TA' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico.

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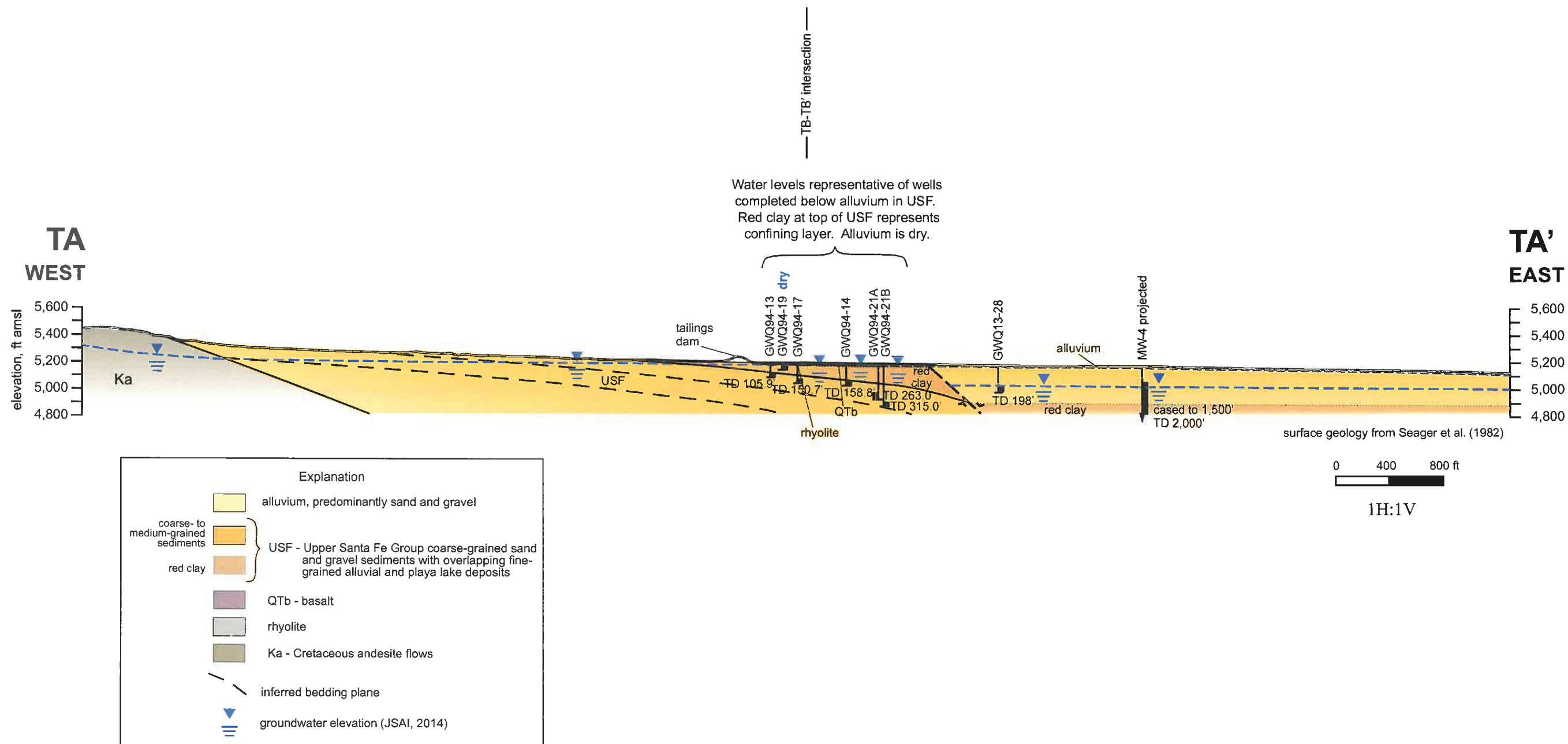


Figure 11K-6b. West to east geologic cross-section TA-TA' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico.

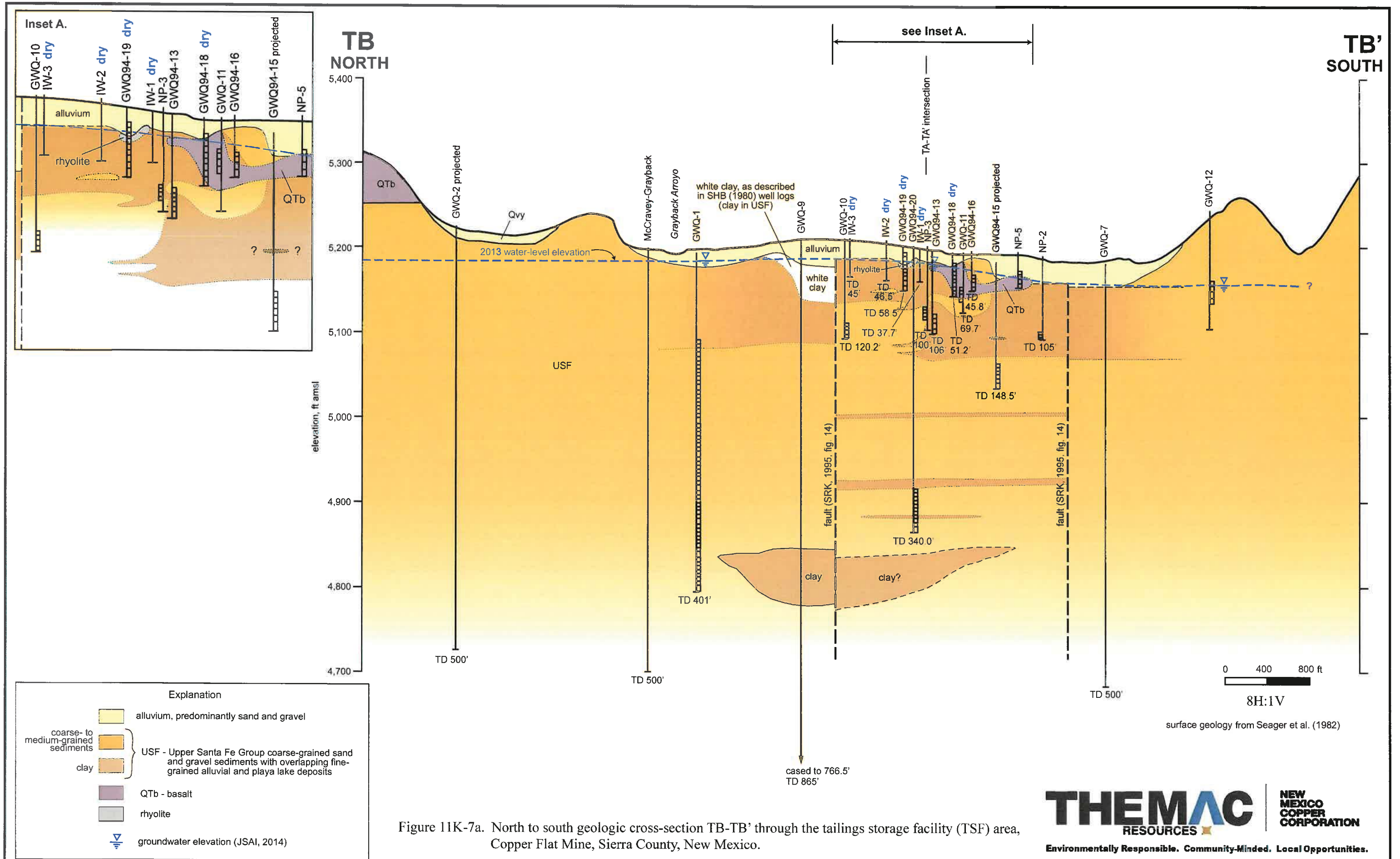


Figure 11K-7a. North to south geologic cross-section TB-TB' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico.

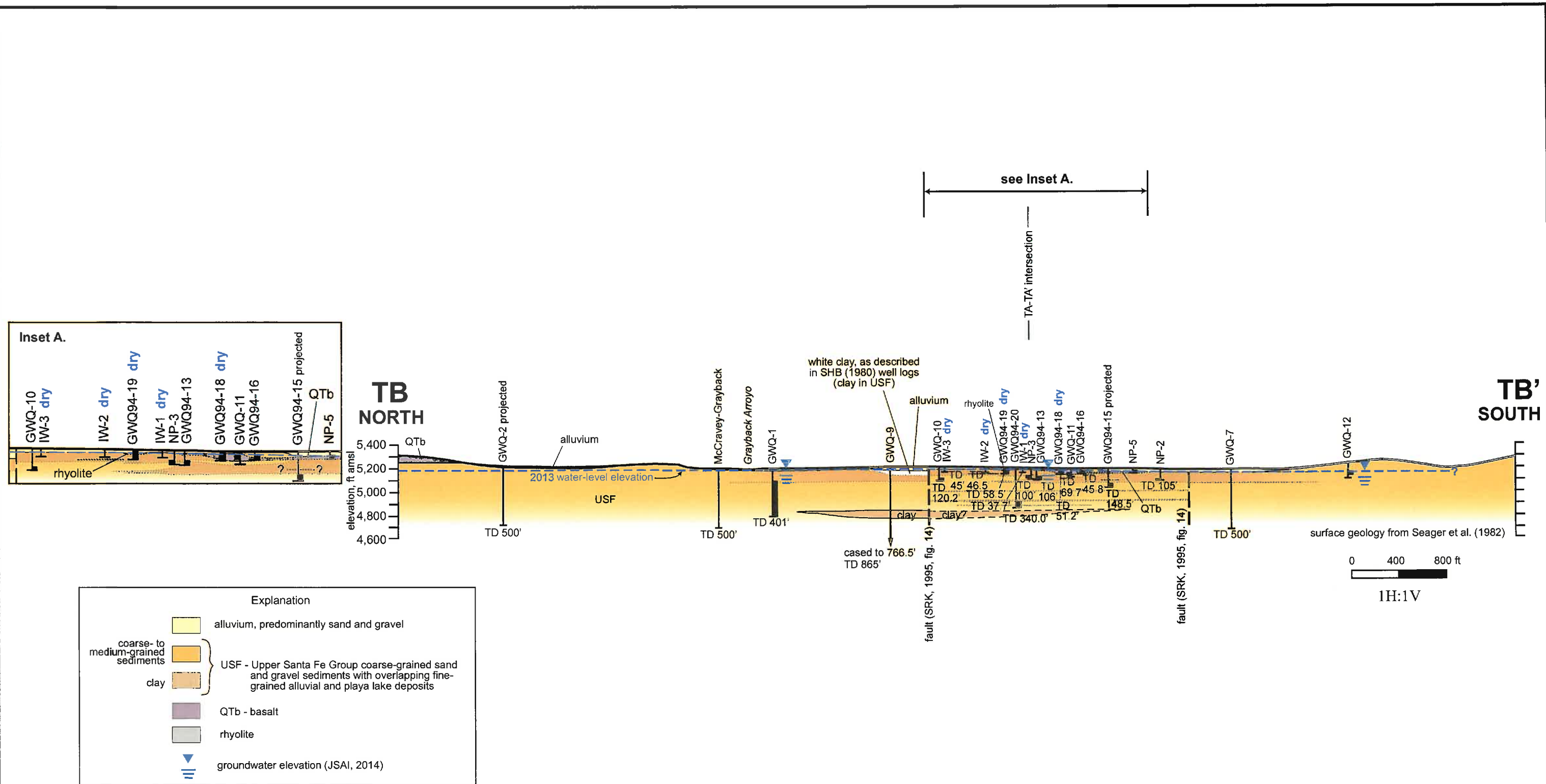


Figure 11K-7b. North to south geologic cross-section TB-TB' through the tailings storage facility (TSF) area, Copper Flat Mine, Sierra County, New Mexico.



**20.6.7.11.K.(3)** hydrologic information on any surface waters of the state within one-half mile of the boundary of the copper mine facility, and of subsurface conditions for all water bearing zones beneath the copper mine facility including maximum and minimum depths to ground water, direction of ground water flow, hydrologic gradients shown by potentiometric maps, transmissivity and storativity, and ground water quality; the sources of all such information shall be provided with the application.

NMCC has been actively engaged in water related baseline data studies since 2010 and Stage 1 Abatement activities since 2011. All of the hydrologic information submitted previously to NMED in this regard is incorporated into this application by reference. Relevant information from those activities has been reviewed by NMCC and its consultants to provide the following summary hydrologic information. Source reports consulted in the preparation of this hydrologic summary are presented in Table 11K-1.

Table 11K-1 Source Reports Cited		
Report Title	Report Prepared By	Date
Baseline Data Characterization Report, Copper Flat Mine, Sierra County, New Mexico	Intera et al.	June 2012
Results from the First Year of the Stage 1 Abatement Investigation at the Copper Flat Mine Site Near Hillsboro, New Mexico	John Shomaker and Associates	May 2014
Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico	John Shomaker and Associates	August 2014

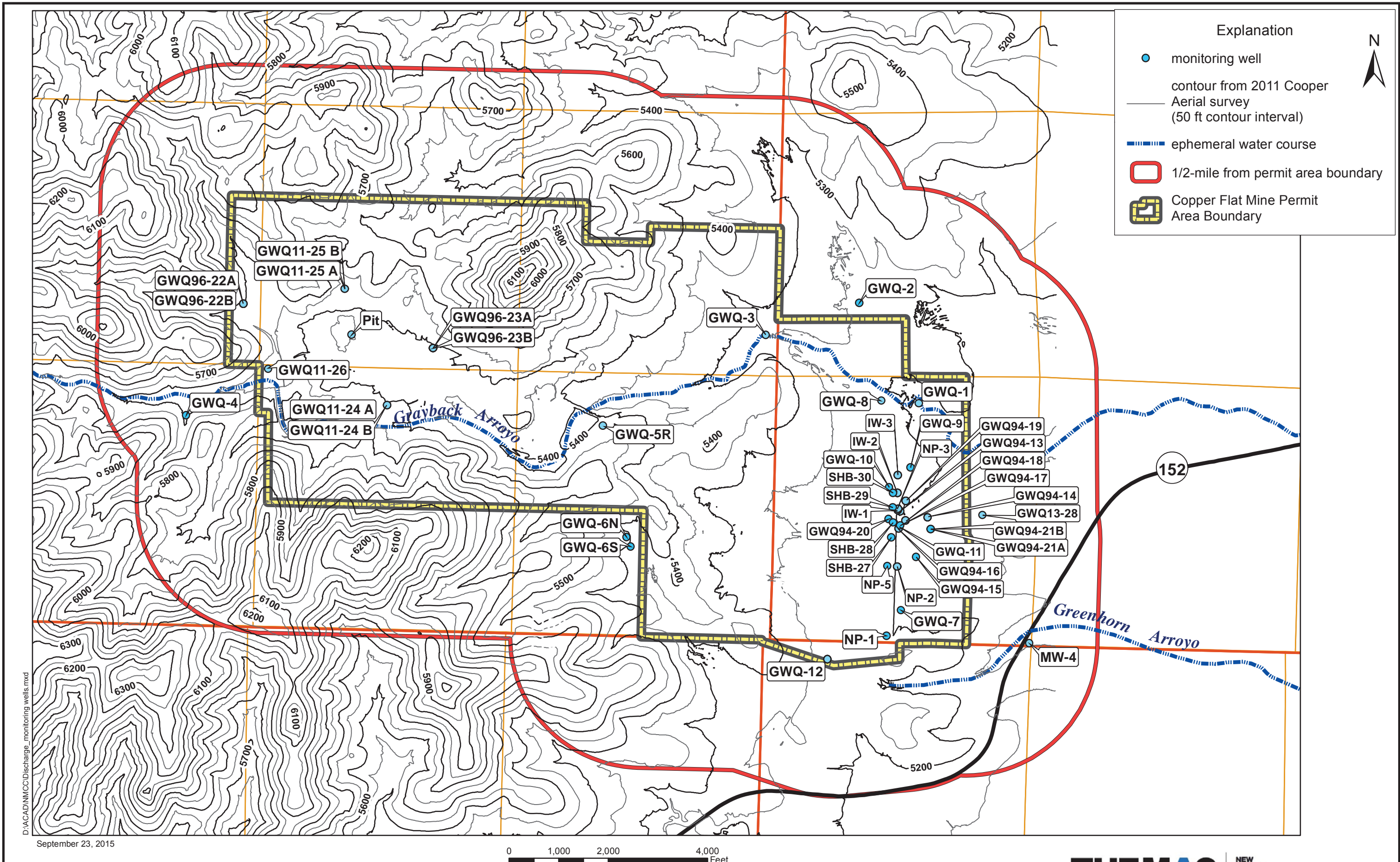
**Surface waters of the state**

Figure 11K-8 is a map that shows the surface waters of the state within one-half mile of the Copper Flat Mine Permit Area Boundary. Grayback Arroyo is an ephemeral arroyo that enters the area from the west and exits to the east.

**Water bearing zones beneath the facility**

NMCC has identified three water bearing zones or aquifers beneath the Copper Flat Mine facility. They are the:

1. Quaternary alluvium associated with Grayback Arroyo and the alluvial fan and fluvial sediments in the Upper Santa Fe Group
2. The Santa Fe Group
3. Crystalline bedrock including the Andesite volcanic flows and the Quartz Monzonite intrusive



D:\ACAD\MCC\Discharge\_monitoring\_wells.mxd

September 23, 2015

Figure 11K-8. Surface water within 1/2-mile from Copper Flat Mine Permit Area Boundary.



JOHN SHOMAKER & ASSOCIATES, INC.



Figure 11K-2a, which shows the geology of the area in plan-view, and Figures 11K-3a and 3b through 11K-7a and 7b, which show the cross sections, demonstrate the sectional spatial relationships of these water bearing units beneath the mine permit area.

The crystalline bedrock is beneath the western portion of the mine permit area. The crystalline bedrock is composed of Cretaceous age volcanic andesite flows, quartz monzonite intrusive porphyry and quartz monzonite breccia. The Copper Flat ore body is within the quartz monzonite porphyry and breccia. The quartz monzonite porphyry intruded the vent of a strato-volcano, then dikes of and mineralized veins radiated out from the porphyry into fault and fracture controlled zones in the surrounding andesite. The depth of the crystalline bedrock is over 3,000 feet deep (Intera, et. al, 2012).

The Santa Fe Group is present in the eastern portions of the mine permit area and is over 500 feet deep below the existing tailings facility. The Santa Fe group is composed of stratified sediments with a variety of grain sizes consisting of silts, clays, sand and gravel. The strata dip generally to the east (Intera et. al, 2012).

The Quaternary alluvial water bearing zones are sediments to the east of the andesite or deposited in the Grayback Arroyo. These sediments are composed of Alluvial fan deposits and fluvial sands of the Santa Fe Group and saturated alluvium in the drainages. Alluvium is found in Grayback Arroyo and is primarily composed of sand and gravel that ranges in thickness from 5 to 50 feet (Intera et. al, 2012).

The most significant water bearing zones are the Crystalline Bedrock and the Santa Fe Group that underlie the east and west portions of the Copper Flat Mine Permit Area. The Quaternary alluvial fan and fluvial water bearing units are located along the east side of andesite-upper Santa Fe Group contact in the vicinity of the existing tailings disposal facility. This shallow water bearing zone lies on top of a low permeability red clay and does not extend east of the large normal fault located east of the Mine Permit Area Boundary as shown on the Geologic Map (Figure 11K-2a) and the east-west Geologic Cross-section TA-TA' (see Figures 11K-6a and 6b).

Generally, the wells in this water bearing zone are screened to 60 feet or less (Intera et al., 2012, JSAI 2014b). In the alluvial sediments of the Grayback Arroyo overlying the crystalline bedrock, groundwater flow follows the stream channel. East of the andesite, the groundwater in the Grayback arroyo seeps into and recharges the Santa Fe Group. Within the Mine Permit Area, two wells are screened into the Grayback Alluvium, GWQ11-26 west of the pit and GWQ-3 downstream of the pit. The locations of both wells are shown on Figure 11K-2a.

### **Maximum and minimum depth to groundwater**

Table 11K-2 provides information regarding the minimum and maximum depths to water for each water-bearing zone. It identifies the well from which the water level was collected and the date that the water level was collected. Figure 11K-8 shows the location



of the wells measured. The source of this data is from recent groundwater measurements taken during the Stage 1 Abatement Investigations conducted 2013 at the site.

Water Bearing Zone	Minimum Depth (ft.)	Well/Date	Maximum Depth (ft.)	Well /Date
Crystalline Bedrock	19.0	GWQ11-25A/Oct 2013	75.34	GWQ11-25B/Jul 2013
Santa Fe Group	7.26	GWQ-1/Jan 2013	156.20	GWQ13-28/Oct 2013
Quaternary Alluvium, Upper Santa Fe Group	22.40	GWQ94-16/Oct 2013	46.50	IW-1/Jul 2013
Quaternary Alluvium, Grayback Arroyo Alluvium	12.60	GWQ-3/Oct 2013	41.58	GWQ11-26/Jul 2013

### **Aquifer Transmissivity and Storativity**

Tested hydraulic conductivities are summarized in Table 11K-3. Transmissivity is conductivity multiplied by thickness, which is dependent on the saturated screened interval, which varies.

Water Bearing Zone	Range of Hydraulic Conductivity (ft/day)	Range of Transmissivity (ft <sup>2</sup> /day)	Storativity	Wells included in Test Range	Source of Information
Crystalline Bedrock Aquifer (Andesite)	0 to 0.0027	NA <sup>(1)</sup>	NA <sup>(2)</sup>	GWQ96-22, GWQ96-23, GWQ-5R	JSAI May 2014, JSAI August 2014
Crystalline Bedrock Aquifer (Quartz Monzonite)	0.02 to 0.14	NA <sup>(1)</sup>	NA <sup>(2)</sup>	GWQ11-24, GWQ11-25	JSAI May 2014, JSAI August 2014
Santa Fe Group	1.0 to 4.7	187 to 1,710	2.5X10 <sup>-4</sup> <sup>(3)</sup>	GWQ-1, GWQ-7, GWQ-9, GWQ94-17, GWQ13-28,	JSAI May 2014, JSAI August 2014
Quaternary Alluvial Aquifer	3.8	87	NA <sup>(4)</sup>	GWQ94-16	JSAI May 2014, JSAI August 2014

<sup>(1)</sup>Transmissivity is more appropriate for aquifers with fairly uniform inter-granular permeability and less appropriate for fractured rocks because of spatial variability and limited spatial influence (JSAI, Personal Communication July 30, 2015).

<sup>(2)</sup>Storage coefficient for a system of fractures in an otherwise nearly impermeable crystalline rock is not approachable with a short term pressure injection test because of the lack of rock volume information into which water is injected (JSAI, Personal Communication July 30, 2015).

<sup>(3)</sup>Transmissivity and storage coefficient was derived from a 78 hour pumping test on GWQ94-17 by Adrian Brown Consultants in 1994 (Appendix C of JSAI August 2014).

<sup>(4)</sup>There is no pumping test data for the alluvial aquifer system, hydraulic conductivities were estimated from water level measurement during pumping for sample collection (JSAI, May 2014).



### **Direction of Groundwater Flow**

The general direction of groundwater flow is from the west to the east. The exception to this is in the vicinity of the existing mine pit, which acts as a hydraulic sink. Groundwater flow direction and hydraulic gradients are shown on Figure 11K-9. The water level elevation contours in shown are derived from data from wells around the perimeter of the mine pit.

Figure 11K-10 provides hydrographs that illustrate water levels in the existing pit water body and from monitoring wells installed around the pit from 2009 through 2013. The water levels show that water in the pit has been consistently lower than the water levels in the surrounding wells, indicating that the pit is and has been a hydrologic sink. The water-level elevation in the pit after mining operations have ceased is projected to be at approximately 4900 ft. While the water levels surrounding the mine pit may be lower after mining operations have ceased than shown in Figure 11K-10, they would be expected to be higher than the pit.

NMCC provided the BLM and its contractor a response to an inquiry as to the possibility of water flowing from the pit lake into groundwater after large short-term precipitation events which can occur during the summer. In the response of June 25, 2015 JSAI explained that projected water levels in the pit and downstream derived from the JSAI groundwater model indicates that the water level down-gradient of the pit will always above 5,100 ft. elevation. Filling the pit from 4,900 ft. elevation to 5,100 ft. elevation would require about 250 inches of water over the entire 327-acre pit catchment and watershed or about 6,800 acre-feet of water in order to create a flow-through system and overcome the sink that the mine pit creates. The wettest year on record in the Hillsboro area is 21 inches of water over an entire year, i.e., 1941. That would have generated about 82 acre-feet of precipitation and runoff to the pit (JSAI 2015). As such, it is reasonable to conclude that the mine pit will continue to be a hydrologic sink.



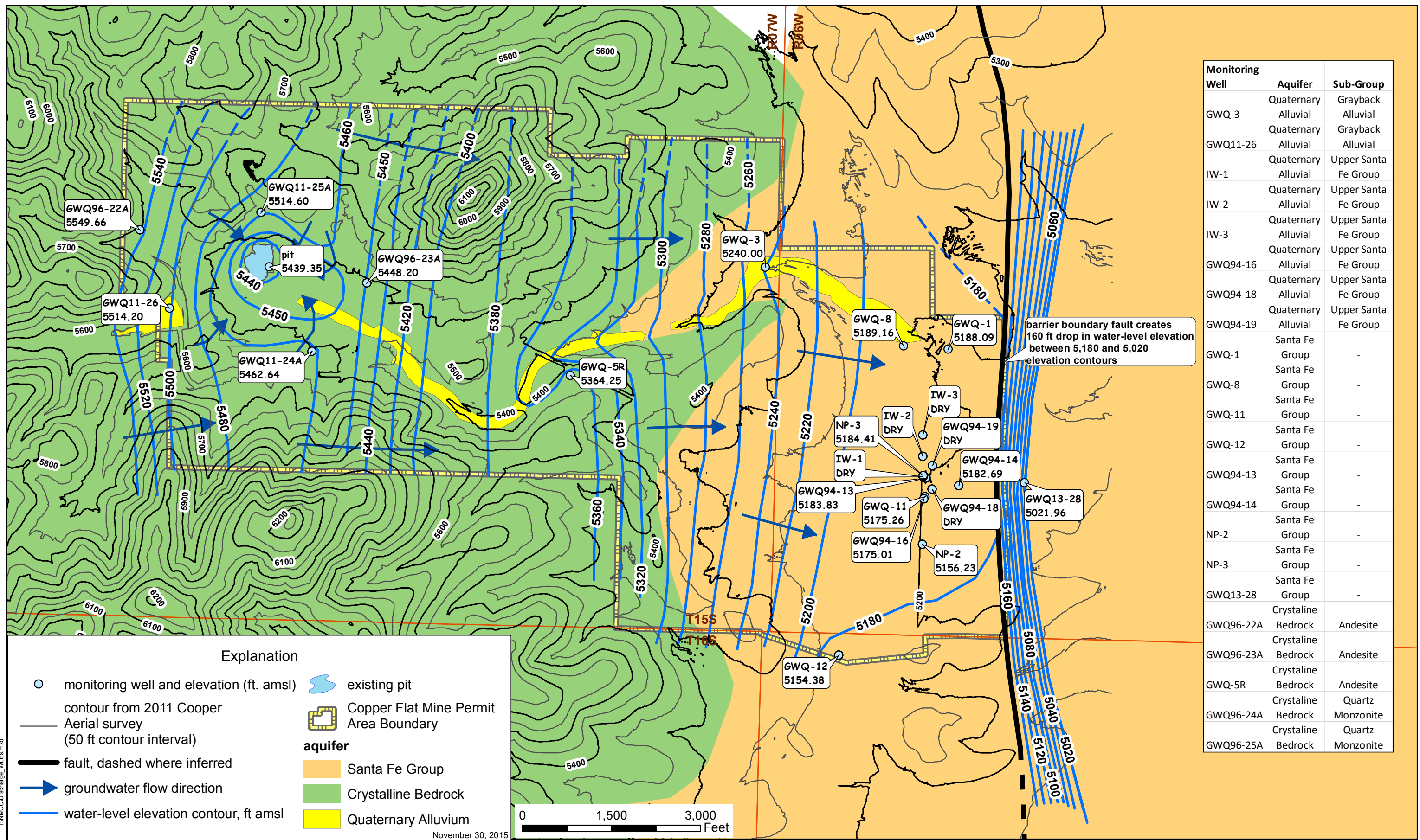


Figure 11K-9. Geologic map showing water level elevations from 4th Quarter 2013, Copper Flat Mine, Sierra County New Mexico.

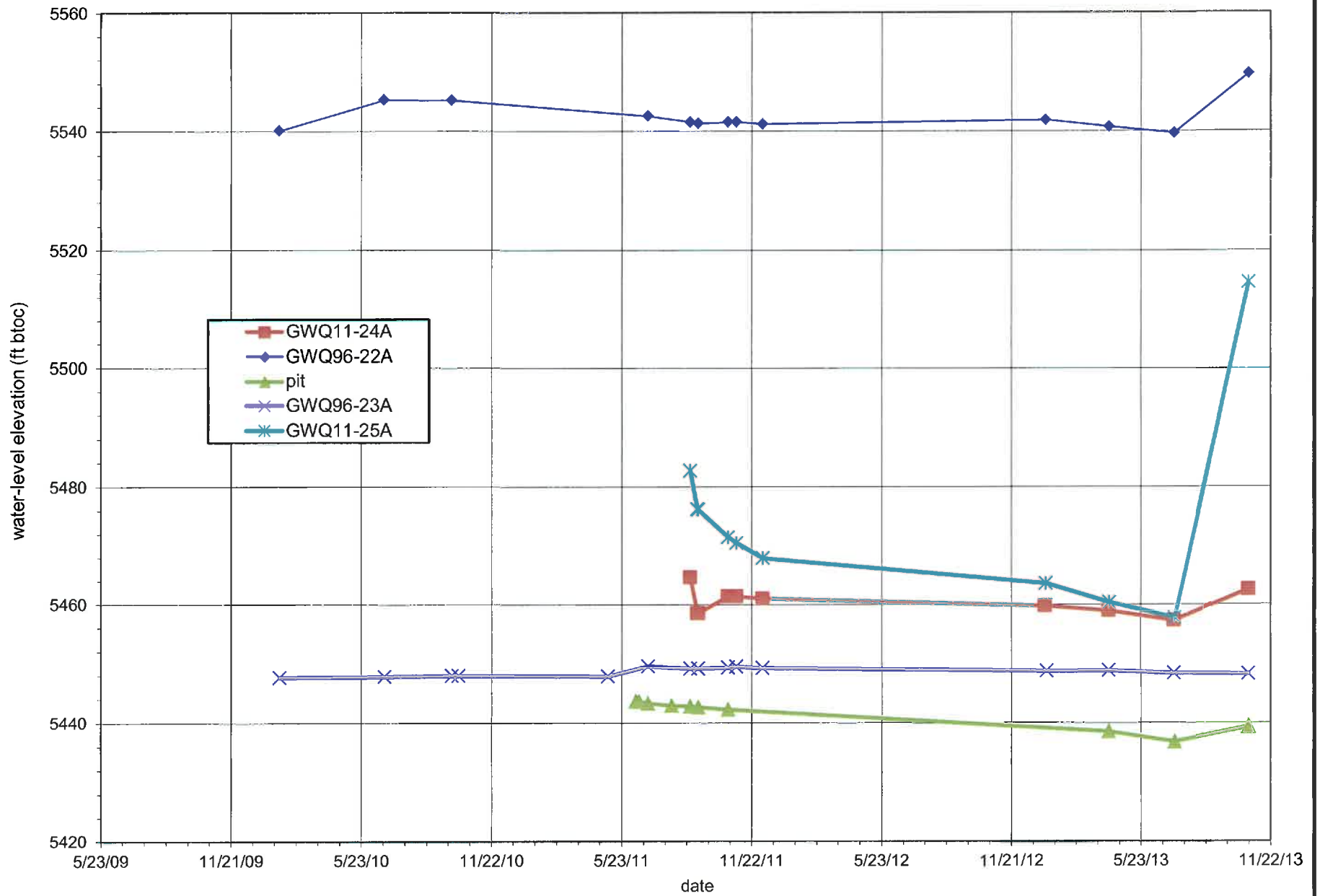


Figure 11K-10. Hydrograph of pit area wells and pit water levels.



**Groundwater Quality**

This section presents groundwater quality for the water bearing zones at the Copper Flat Mine project. The data was summarized from several reports prepared for Quintana Minerals Corporation and New Mexico Copper Corporation. Table 11K-4 presents the reports that were cited in this analysis. The laboratory reports associated with this data are incorporated into the cited reports in Table 11K-4.

<b>TABLE 11K-4</b>		
<b>Source Reports Cited in Groundwater Quality Analysis</b>		
<b>Report Title</b>	<b>Report Prepared By</b>	<b>Date</b>
Tailings Dam and Disposal Area, Quintana Mineral Corporation, Copper Flat Project, Gold Dust, New Mexico	Sargent, Hauskins, and Beckwith (SHB)	October 1980
Geohydrologic Evaluation For Submission of Discharge Plan, Copper Flat Project, Quintana Mineral Corporation, Sierra County, New Mexico	SHB	June 1981
Baseline Data Characterization Report, Copper Flat Mine, Sierra County, New Mexico	Intera et al.	June 2012
Results from the First Year of the Stage 1 Abatement Investigation at the Copper Flat Mine Site Near Hillsboro, New Mexico	John Shomaker and Associates (JSAI)	May 2014

Table 11K-5 presents groundwater results from samples collected prior the initiation of mineral processing by Quintana Minerals, which initiated in March, 1982. These water quality results represent the pre-discharge conditions at the Copper Flat Mine based on for some constituents the data available from that time. The results have been categorized by the wells the samples that were collected from the aquifers or water bearing units that the wells were screened in. The well locations are presented on Figure 11K-8.

Table 11K-6 presents groundwater data collected by NMCC and its consultants during baseline data characterization and Stage 1 Abatement monitoring activities conducted from 2010 to 2013. The results from this table represent the current water quality conditions beneath the Copper Flat Mine Area Permit Boundary. For the Quaternary Alluvial Aquifer in Grayback Arroyo and the Crystalline Bedrock Aquifer, the wells screened in the up-gradient and down-gradient of the Copper Flat ore body have been segregated. In Grayback Arroyo, well GWQ11-26 is up-gradient of the ore body and is also representative of a pre-discharge condition. In the Crystalline Bedrock Aquifer, wells GWQ96-22A and B, GWQ-24A and B, and GWQ-25B have all been completed since the existing Copper Flat Mine pit was excavated; therefore, these wells have been within the influence of the evaporative hydraulic sink caused by the pit. Given these wells are within the cone of depression caused by the pit, they are effectively up-gradient and are representative of pre-discharge conditions. This is not the case for the Quaternary Alluvium (upper Santa Fe Group) Aquifer and the Santa Fe Group Aquifer as these aquifers are both down-gradient of the Copper Flat ore body. Groundwater quality sample results from Well GWQ11-25A was not included as representative of the Crystalline Bedrock



**TABLE 11K-5  
Water Quality Samples Collected Prior to 1982**

<b>Water Bearing Units beneath the Copper Flat Mine Area Permit Boundary</b>				
<b>Constituent</b>	<b>Quaternary Alluvial Aquifer (Grayback) (mg/L)</b>	<b>Quaternary Alluvium (Upper SFG) (mg/L)</b>	<b>Santa Fe Group (mg/L)</b>	<b>Crystalline Bedrock (mg/L)</b>
	<b>Wells Sampled</b>	<b>Wells Sampled</b>	<b>Wells Sampled</b>	<b>Wells Sampled</b>
	<b>GWQ-3<sup>1</sup>, GWQ-5<sup>2</sup></b>	<b>NP-5, SHB-27<sup>3</sup>, SHB-28<sup>3</sup>, SHB-29<sup>3</sup>, SHB-30<sup>3</sup></b>	<b>GWQ-1, GWQ-2, GWQ-7, GWQ-8, GWQ-9, GWQ-10, GWQ-11, NP-1, NP-2, NP-3</b>	<b>GWQ-4, GWQ-6<sup>4</sup></b>
Arsenic	0.004 to <0.01	<0.005 to 0.02	<0.002 to 0.024	<0.01
Barium	<0.2	<0.2 to 0.218	<0.02 to 0.25	<0.2
Cadmium	<0.005	<0.001 to <0.005	<0.001 to 0.006	<0.005
Chromium	<0.01	0.002 to <0.02	<0.005 to <0.05	<0.01
Cyanide	<0.01	0.001 to <0.01	<0.001 to 0.36	<0.01
Fluoride	0.6 to 1.03	0.77 to 1.3	0.3 to 1.9	0.68 to 1.1
Lead	<0.02 to 0.073	<0.001 to <0.02	<0.005 to 0.033	<0.02
Total Mercury	<0.001	<0.0004 to <0.001	<0.0005 to 0.0064	<0.001
Nitrate	0.1 to 5.5	<0.1 to 4.1	<0.05 to 60	0.5 to 3.3
Selenium	0.0037 to 0.0062	<0.005 to <0.02	<0.0005 to 0.029	0.0046 to <0.005
Silver	<0.02	<0.001 to <0.02	<0.001 to 0.023	<0.02
Uranium	NA	NA	NA	NA
Chloride	32 to 56	21 to 51	17 to 100	22 to 102
Copper	<0.05	0.002 to <0.1	<0.02 to 0.48	<0.02 to <0.05
Iron	<0.1	0.007 to 0.52	<0.01 to 3.8	<0.1
Manganese	<0.05	0.036 to 0.42	0.001 to 1.4	<0.05 to 0.11
Sulfate	383 to 575	145 to 353	34 to 220	41 to 270
TDS	868 to 1,260	384 to 840	350 to 650	400 to 810
Zinc	0.064 to 0.32	0.004 to 0.4	<0.05 to 5.3	<0.025 to 0.28
pH	7.0 to 7.9	7.6 to 8.0	7.0 to 8.6	7.2 to 8.3
Aluminum	<0.01	<0.01 to 0.239	<0.01 to 10.2	<0.01
Boron	<0.1 to 0.108	0.07 to 0.1	<0.004 to 0.77	<0.1 to 0.135
Cobalt	<0.05	<0.001 to <0.02	<0.02 to <0.05	<0.02 to <0.05
Molybdenum	<0.05	0.002 to <0.1	<0.01 to 0.26	<0.05
Nickel	<0.05	0.019 to <0.05	<0.01 to <0.05	<0.05

<sup>1</sup>GWQ-3 33 ft. deep concrete line well, 40 inches by 43 inches perforated from 10 to 33 feet.

<sup>2</sup>GWQ5 20 ft. deep rock lined well located in Grayback Arroyo downstream of the Quintana Plant Site. Well was destroyed during construction of the Quintana Mine.

<sup>3</sup>SHB 1981 SHB (1980) indicates that water samples were collected from geotechnical borings SHB-29, 30, 31, 33, and 34 and the water samples were submitted to "Controls for Environmental Pollution, Inc." for analysis. SHB (1981) describes these as "wells", SHB-27, 28, 29, 30, and 34. The data from SHB-34 was not used because the concentration data was not realistic; which are more representative of a QA blank sample. The relationship between soil borings and "wells" is not documented in either report. However, using the laboratory data reports from SHB-1980 and SHB 1981, the relationship between borings and wells can be established.

<sup>4</sup>GWQ-6 Old well located near an old windmill site west of the existing tailings facility. No well construction details are available and depth is unknown. Subsurface conditions in this area are andesite flows and/or volcanic debris flows. The well is currently unusable. GWQ-6 corresponds to GWQ-6N on NMCC current survey files.



**TABLE 11K-6**  
**Water Quality Samples from 2010 to 2013**

**Water Bearing Units Beneath the Copper Flat Mine Area Permit Boundary**

Constituent	Quaternary Alluvial Aquifer (Grayback) (mg/L)		Quaternary Alluvium (Upper SFG) (mg/L)	Santa Fe Group (mg/L)	Crystalline Bedrock (mg/L)	
	Wells Sampled	Wells Sampled	Wells Sampled	Wells Sampled	Wells Sampled	Wells Sampled
	GWQ-3 (down-gradient of ore body)	GWQ11-26 (up-gradient of ore body)	NP-5, IW-2, GWQ94-16	GWQ-1, GWQ-8, GWQ-11, GWQ-12, GWQ13-28, GWQ94-13, GWQ94-14, GWQ94-15, GWQ94-17, NP-1, NP-2, NP-3, NP-4	GWQ-5R, GWQ96-23A, GWQ96-23B (down-gradient of ore body)	GWQ11-24A, GWQ11-24B, GWQ11-25B, GWQ-4, GWQ96-22A, GWQ96-22B (up-gradient of ore body)
Arsenic	NA	NA	<0.001 to 0.0092	<0.001 to 0.0042	<0.001 to 0.0027	<0.001 to 0.0057
Barium	NA	NA	0.018 to 0.039	0.03 to 0.059	0.078 to 0.13	0.057 to 0.11
Cadmium	NA	<0.002	<0.002	<0.002	<0.0012 to <0.002	<0.0012 to 0.256
Chromium	NA	NA	<0.006	<0.006	<0.006	<0.006
Cyanide	NA	NA	<0.005 to <0.01	<0.005 to 0.012	<0.005	<0.005 to <0.01
Fluoride	NA	0.4 to <1.0	0.6 to 0.7	<0.1 to 0.6	1.3 to 2.1	0.73 to 24.4
Lead	NA	NA	<0.005	<0.005	<0.005	<0.005
Total Mercury	NA	NA	<0.0002 to 0.00048	<0.0002 to 0.00026	8.94E-07 to <0.0002	8.94E-07 to <0.0002
Nitrate	NA	NA	1.7 to 4.1	1.4 to 7.5	<0.1 to <1.0	<1.0 to 2.1
Selenium	NA	0.0015 to 0.0062	0.0021 to 0.037	<0.001 to 0.028	<0.001 to 0.0016	<0.001 to 0.0566
Silver	NA	NA	<0.005	<0.005	<0.005	<0.005
Uranium	NA	NA	0.0013 to 0.0062	0.0013 to 0.0023	<0.001 to 0.0037	<0.001 to 0.0037
Chloride	63 to 75	14 to 32	79 to 600	20 to 290	12 to 19	26 to 110
Copper	NA	0.00265 to <0.006	<0.001 to <0.006	<0.001 to <0.006	0.00087 to <0.006	<0.001 to 137
Iron	NA	NA	<0.02 to 1.3	<0.02 to 0.10	0.04 to 1.4	0.02 to 9.3
Manganese	NA	0.0168 to 0.0437	<0.002 to 3.6	<0.002 to 0.19	0.29 to 0.63	0.029 to 13.7
Sulfate	1,210 to 1,750	96.5 to 179	170 to 1,200	5 to 830	1.4 to 140	6.2 to 2,730
TDS	2,410 to 3,060	317 to 905	623 to 2,770	360 to 1,740	496 to 804	521 to 4,400
Zinc	NA	<0.01 to 0.013	<0.01 to 0.29	<0.01 to 1.85	0.0065 to 0.07	<0.01 to 8.65
pH	7.3 to 7.5	6.8 to 7.5	7.0 to 8.0	6.4 to 8.8	6.9 to 8.2	3.7 to 8.0
Aluminum	NA	<0.02 to 0.153	<0.02 to 0.13	<0.02 to 0.14	<0.0053 to 0.0314	0.0097 to 56.6
Boron	NA	NA	<0.04 to 0.081	<0.04 to 0.04	0.068 to 0.14	<0.04 to 0.28
Cobalt	NA	<0.006	<0.006 to 0.017	<0.006	0.0018 to <0.006	0.002 to 0.439
Molybdenum	NA	NA	<0.008 to 0.024	<0.008	<0.008	<0.008
Nickel	NA	NA	<0.01	<0.01	<0.01	<0.01



Aquifer because JSAI (2014) describes the groundwater chemistry samples from that well as "completely different from all other samples in the pit area...." This well is completed in a localized zone of sulfide mineralization and the water source is suspected to be from oxygenated water infiltrating through sulfide bearing fractures with limited storage.

The well locations for this time period are also presented on Figure 11K-8. Groundwater elevations from the 4<sup>th</sup> Quarter 2013 are presented on Figure 11K-9.



## 20.6.7.11.L LOCATION MAP

*An application shall include a location map with topographic surface contours identifying all of the following features within a one-mile radius of the copper mine facility:*

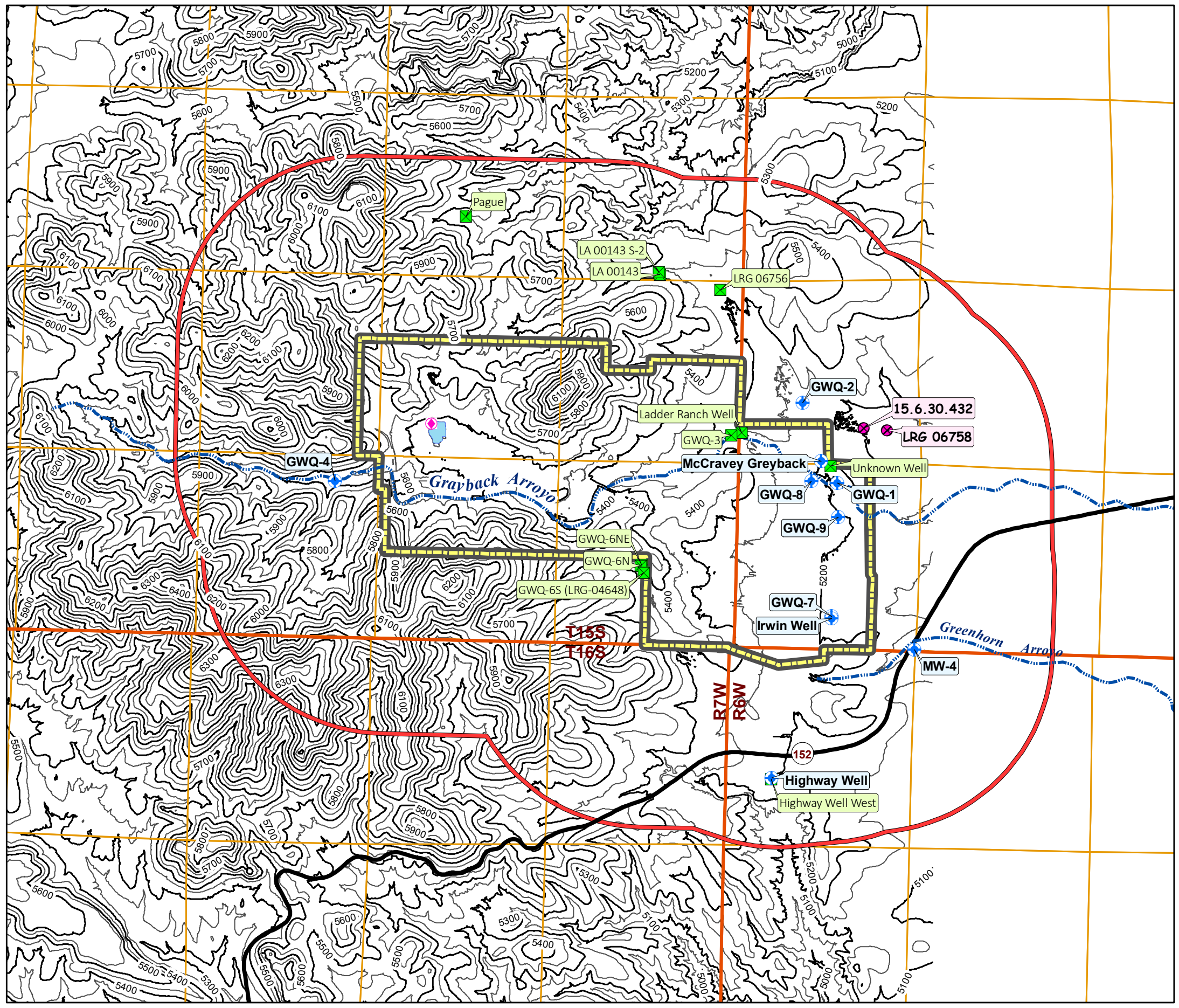
- (1) watercourses, lakebeds, sinkholes, playa lakes, seeps and springs (springs used to provide water for human consumption shall be denoted);*
- (2) wells supplying water for a public water system and private domestic water wells;*
- (3) irrigation and other water supply wells; and*
- (4) ditch irrigation systems, acequias, irrigation canals and drains.*

Figure 11L-1 provides the information requested. With respect to water courses, all are ephemeral, flowing only in response to significant precipitation events. Grayback Arroyo transects the site from west to east, Greenhorn Arroyo begins at the southeast corner just outside of the mine permit boundary area and runs east, and Hunkidori Gulch begins at the eastern edge just outside of the mine permit area boundary and runs east. The only lakebed in the area requested is the pit lake attendant to the existing open pit on the site. There are no playa lakes within a one-mile radius of the facility. One seep exists on the high wall of the existing open pit. There are no springs located within a one-mile radius of the facility.

With respect to water supply wells for a public system or domestic water wells, there are none within a one-mile radius of the facility.

With respect to irrigation and other water supply wells, there are several wells within a one-mile radius of the facility. Some are useable. Others are not currently useable. And other are known to exist but could not be verified as useable or not useable because access by NMCC to these wells has been denied.

With respect to ditch irrigation systems, acequias, irrigation canals and drains, there none within a one-mile radius of the facility.



Explanation	
	intermittent seep
	other well verified useable
	other well verified not currently useable
	well status not verified due to lack of access to private land
	ephemeral water course
	contour from 2011 Cooper Aerial survey (50 ft contour interval)
	pit
	1-mile from mine permit area boundary
	Copper Flat Mine Permit Area Boundary

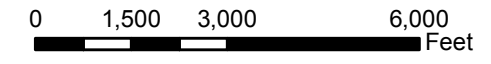


Figure 11L-1. Copper Flat Mine Location Map







## **20.6.7.11.M FLOOD ZONE MAP**

*An application shall include, if available, the most recent 100-year flood zone map developed by the federal emergency management administration (FEMA), flood insurance rate map or other flood boundary and floodway map with the copper mine clearly identified along with all 100-year frequency flood zones for the copper mine facility, and a description of any engineered measures used for flood protection.*

Figure 11M-1 is a 100-year flood insurance rate map (FIRM) flood zone map for the Copper Flat project area. All of the project site area is located in Zone C, Areas of Minimal Flooding. There are no engineered measures used in the project site area used for flood protection.

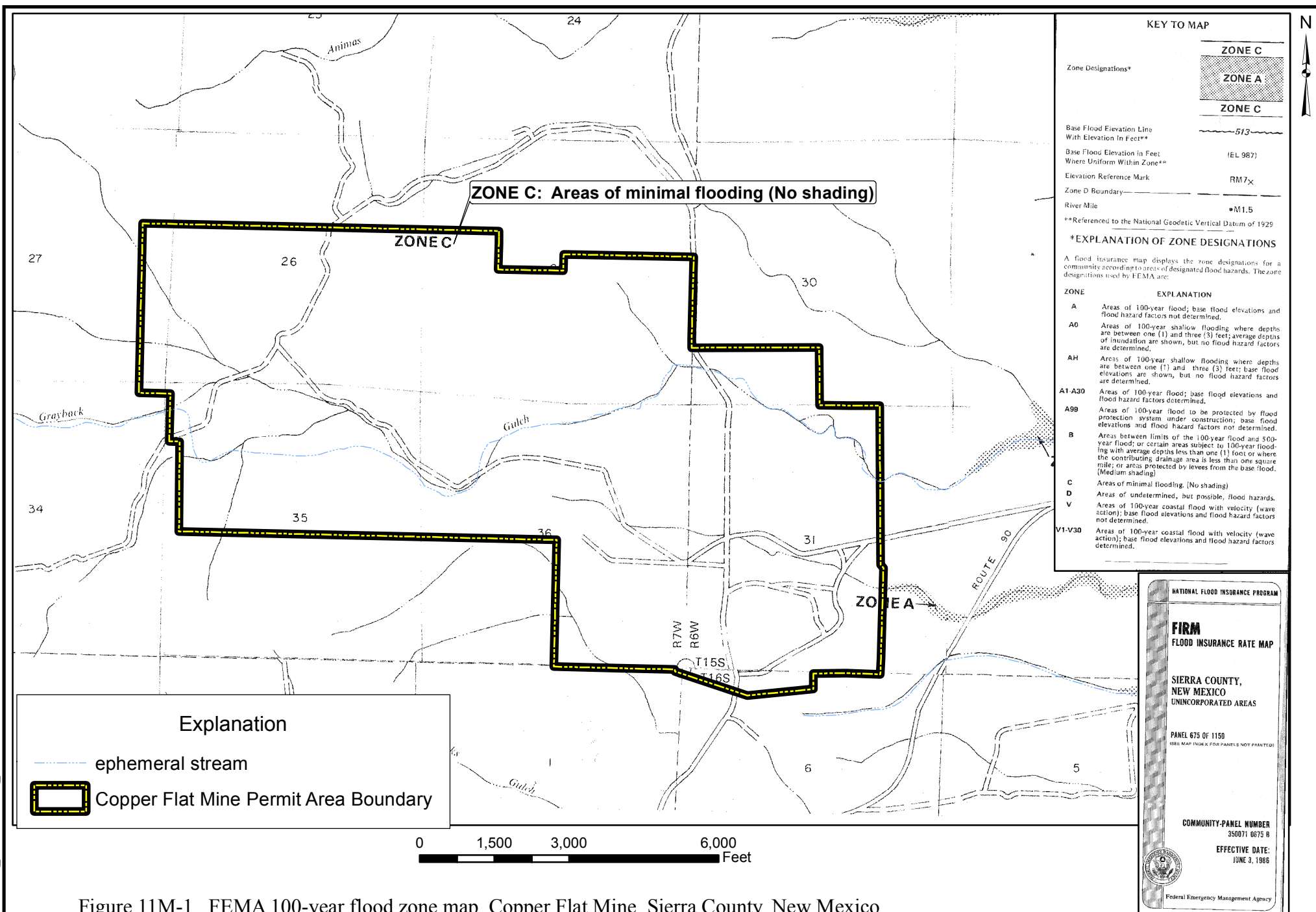


Figure 11M-1. FEMA 100-year flood zone map, Copper Flat Mine, Sierra County, New Mexico.



## **20.6.7.11.N ENGINEERING DESIGN, CONSTRUCTION AND SURVEYING**

*Pursuant to 20.6.7.17, 20.6.7.18, 20.6.7.20, 20.6.7.21, 20.6.7.22, 20.6.7.23 and 20.6.7.26 NMAC an application shall include:*

- (1) plans and specifications for proposed new or modified tailings impoundments, leach stockpiles, waste rock stockpiles, and process water and impacted stormwater impoundments and associated liners;*
- (2) plans and specifications for proposed new or modified tanks, pipelines, truck and equipment wash units and other containment systems; and*
- (3) a stormwater management plan.*

### **20.6.7.11.N.(1) TAILINGS STORAGE FACILITY**

Plans and specifications for Copper Flat project new tailings storage facility, including associated liners are contained in Appendix A. These plans and specifications are prepared at the Feasibility grade level. NMCC believes that the information contained therein is sufficient to allow the NMED to evaluate the liner design and construction for these facilities pursuant to the Copper Rules.

Sections 1 through 5 of the design document present the various parameters and site condition considerations that were analyzed in preparing the design, including site topography, climate, subsurface geology and other site observations. The design document discusses the results of the extensive site-specific soils and geotechnical exploration and testing programs that were conducted in the field as well as the tailings test and materials classification and consolidation tests conducted in the laboratory. It also presents consolidation modeling results for the tested tailings materials.

Section 6, Feasibility Level Design, presents the design of the tailings impoundment, the seepage collection pond and related facilities, i.e., the TSF, beginning with Table 6 which summarizes the key design criteria for the design of the TSF including regional design factors, impoundment-specific design factors, water storage and storm water diversion design factors.

Sections 7 and 8 present tailings delivery and distribution system and the water reclaim system designs and their related liner systems.

### **20.6.7.11.N.(1) WASTE ROCK STOCKPILES**

The plans and specifications for the waste rock stockpiles are also at a feasibility level of design. In general, the stockpiles will be built to a configuration of 3 horizontal to 1 vertical slope angles (18.4 degrees). This configuration will help facilitate reclamation at the end of the mine life as provided for in Section **20.6.7.33.C.(3)**. Each lift within the stockpile will be maximally 75 ft. high and be placed at angle of repose (35.54 degrees) with 120 ft. setbacks left between lifts to maintain the 3 to 1 overall angle for the stockpile.



#### **20.6.7.11.N.(1) PROCESS WATER AND IMPACTED STORM WATER IMPOUNDMENTS**

The Plans and Specifications for the process water and impacted storm water impoundments and associated liners are contained in Appendix B. As with the plans and specifications for the TSF, these plans and specifications are also prepared at the Feasibility grade level. NMCC believes that the information contained therein is sufficient to allow the NMED to evaluate the liner design and construction for these facilities pursuant to the Copper Rules.

#### **20.6.11.N.(2) TANKS, PIPELINES, TRUCK AND EQUIPMENT WASH UNITS**

As indicated above, M3 has prepared a Process Facility Containment Report that provides the plans and specifications for the tanks, pipelines and truck and equipment wash and other containment units. The report is included herein as Appendix C. As with the plans and specifications for the TSF, these plans and specifications are also prepared at the Feasibility grade level. NMCC believes that the information contained therein is sufficient to allow the NMED to evaluate the liner design and construction for these facilities pursuant to the Copper Rules.

The report identifies and describes the proposed sumps, tanks, pipelines and truck and wash and other containment units, including their purpose, construction material, dimensions and capacity. This report also provides information regarding the form and design of the containment structures that will be incorporated into the process to contain and manage materials containing water contaminants that have the potential to migrate to groundwater and cause and exceedance of applicable groundwater standards and meet the requirements of **20.6.7.22, 23 and 26 NMAC**.

Certain foundations, sumps and other containments originally constructed by Quintana for previous operation of the Copper Flat mine exist and may be utilized, where appropriate. Specifically, the primary crusher, the coarse ore stockpile, the reclaim tunnel and the concentrator processing area are areas that have the certain structures that may be utilized by NMCC's operations. Sumps and containment structures exist in certain areas of the existing footprint that may be located and may be of sufficient size to be utilized in the NMCC operations. If they can be utilized, each of these existing sumps and containment areas will be tested to ensure that they do not leak. They will be sealed with approved water stops, coatings or liners, and/or will be reconstructed, as necessary, to meet the requirements of **20.6.7.22, 23, and 26 NMAC** prior to use. All new sumps and containments will be constructed to meet the requirements of **20.6.7.22.B.(1) NMAC**.

Sumps that and other existing containment structures that cannot be utilized in NMCC's operations will be decommissioned in a manner to prevent them from becoming conduits for potential release of contaminants to surface or groundwater. Unutilized sumps will be filled with grout and/or concrete. Unutilized containment areas will be removed so that they cannot retain water.



NMCC proposes to install new tanks and pipelines for its operations, with the exception of the freshwater pipeline that will provide water to the operation from the off-site well field. Therefore, with respect to the requirements of **20.6.7.23.C.(6) NMAC** for a pipeline evaluation plan for existing pipelines, NMCC believes that no pipeline evaluation plan is necessary. All new pipelines and tanks will be constructed in accordance with the requirements of **20.6.7.23.B.(1) NMAC**. All containment system structures, pipelines, tanks and secondary containment structures will be inspected at least monthly per **20.6.7.22.C.(3).b** and **20.6.7.23.(C).(2) NMAC**, respectively.

NMCC proposes to have two (2) areas specifically designed for equipment and truck washing. One area will be located within the concentrator area, designated as the “Wheel Wash Area”. The purpose of this area is to provide a location where the wheels of the located with the copper concentrate will be washed to remove any concentrate product that adheres to the truck wheels prior to leaving the concentrator area. This activity will be conducted in a containment area as described in Section 4.1.9 of Appendix C. The second area, designated as the “Truck & Equipment Washing Unit” will be constructed in the plant site along the haulage road from the mine to the truck shop as described in more detail in Section 4.4 of Appendix C. Both of these units have been designed and will be constructed and operated in conformance with **20.6.7.26 NMAC** as discussed in Appendix C.

#### **20.6.7.11.N.(3) STORM WATER MANAGEMENT PLAN**

Storm water will be managed using a series of engineering and best management practices to ensure that storm water is controlled in a manner in conformance with applicable state and federal requirements. The purpose of the plan is to control surface storm water flows to minimize contact with mine disturbances and activities so as to prevent impacts to surface water and ground water quality.

NMCC’s overall storm water management and impact mitigation strategies will include actions such as;

- Grading and contouring all site facilities to maximize the opportunities to route surface drainage around and away from the mining activity;
- Eliminating and/or minimizing run-on and run-through routes;
- Controlling run-off velocity to control erosion;
- Constructing run-off control ditches wherever needed to divert run-off and control and capture impacted storm water;
- Collecting run-off water that has been potentially impacted by mining activities in the mine pit and/or lined impoundments as necessary and minimizing potential for release;
- Reusing captured water as make-up process water whenever possible.

### **Drainage Basin Management**



As described in Section **20.6.7.11.J.(6)**, NMCC proposes to construct several storm water management structures to manage run-on and run-off at the Copper Flat mine. Figure 11J-21 shows that the Copper Flat mine permit area boundary lies within the Greenhorn Arroyo drainage basin. The site is located in the top or head of the basin. As such, storm water drainage at and through the site is limited. Grayback Arroyo, an ephemeral watercourse, and a small unnamed arroyo tributary to Grayback Arroyo are the only drainage-ways that enter the site. Grayback Arroyo transects the site from west to east beginning at the top of the basin. The unnamed arroyo enters the site at the northwest corner of the site. Mining activities undertaken by Quintana Minerals in the early 1980's resulted in construction of diversion structures to Grayback Arroyo and the unnamed arroyo to divert drainage around the site. NMCC has conducted an analysis of the capability of the structures to safely pass a 100- year 24 hour storm event through the site. This analysis has determined that Grayback Arroyo will safely handle such an event while keeping all of the water within its banks. As such, a primary feature of NMCC's storm water management strategy incorporates the existing diversion structures to eliminate run-on from all areas of the site. These structures have worked very well over time and remain in place and functioning today. NMCC will continue to maintain the integrity of the diversion structure during and operating life of the mine as part of its storm water management plan during operations and as part of reclamation and closure of the project.

### **Site Drainage Management**

Storm water runoff from the various areas at the site will be managed at the Copper Flat mine in a manner appropriate for that location. The Copper Flat site has four areas where storm water management will be especially important;

- Open pit mine
- Process area, administration building, parking and other ancillary facilities
- Waste rock stockpile areas, and
- Tailings storage facility

NMCC will develop watershed areas within the site for each of these four areas by grading and contouring the areas and constructing the structures in a manner to control storm water. Figure 11J-3 identifies the developed watersheds in association with surface water drainage management.

### **Mine Pit Management**

As describe previously, developed watershed D (WS D) will be located at the westernmost edge of the site. It contains the mine pit and existing waste rock stockpiles (EWRSP-1 and EWRSP-2A) constructed by the Quintana Mining operation. All precipitation that falls onto the footprint of the developed watershed D will be captured in the mine pit. Drainage ditches will be constructed, as necessary, to move run-off as efficiently as possible while taking care not to cause erosion or to allow ponding and infiltration. Water collected in the mine pit during operation will be pumped out and utilized for dust control within the open pit surface drainage area or as make-up water in the process. When applying water for dust control, care will be taken to not over-water to prevent ponding or erosion from



occurring. At closure, the water will be allowed to collect in the pit and contribute to the volume of the pit lake as described in the Reclamation and Closure Plans.

### **Plant Area Management**

Developed watershed A (WS A) will be located in the south-central portion of the site as shown in Figure 11J-3. It will control storm water from the processing facility, including crushing and grinding, milling, flotation, concentrating, drying and packaging, the administration building, parking and other ancillary support facilities. WS A also contains an area which includes an existing waste rock stockpile (EWRSP-3). As described earlier, a portion of this existing stockpile includes the small volume of ore that was not processed by Quintana before shutting down. This ore material will be utilized by NMCC to feed the process circuit early in the operations phase. The remaining existing waste rock stockpile will be utilized as a temporary holding area to store oversized ore rock that cannot be fed directly into the crusher and must be reduced in size using pneumatic hammers.

Watershed A will be graded and berms constructed, as necessary, to eliminate storm water run-on. The area will be graded and contoured to direct and collect precipitation to a lined storm water impoundment as shown in Figure 11J-3. Details of the design of this impoundment are contained in Section **20.6.7.11.J.(2)** of this application and Appendix B. The impoundment will be designed to capture the water produced from a 100-year 24 hour storm event and will have a minimum of two feet of free-board. Captured water will be transferred to the process water reservoir, also located within WS A (see Figure 11J-3), and utilized as make-up water in the process.

The storm water impoundment will be constructed with a spillway designed to safely pass overflow water in the event of the occurrence of an extraordinary storm event that exceeds NMED design specifications. Overflow water from the spillway will be routed Grayback Arroyo.

The process water reservoir will also be located in within developed watershed A. The reservoir is designed such that no surface water runoff from the watershed will enter it. Only precipitation that falls directly on the footprint of the reservoir will be retained therein. The process water reservoir will not have a spillway, per **20.6.7.17.D.(7) NMAC**, that empties onto the ground surface. In the unlikely event of overtopping of the reservoir as a result of a significant storm event larger than the design storm (which has been designed per NMED requirements) the water will be directed through a weir into a lined conveyance ditch designed to route water to the tailings storage facility. The primary purpose of this conveyance ditch will be to function as the secondary containment structure for the pipeline from the process circuit to the tailings facility and the recycled water pipeline from the tailings facility to the process water reservoir. However, it is also designed to provide a means whereby overflow from the process water reservoir can be managed in conformance with **20.6.7.17.D.(7) NMAC**.

At closure, these impoundments will be re-contoured, re-graded and covered as described in the Reclamation and Closure Plans so that water is no longer retained.



### **Waste Rock Stockpile Management**

Developed watersheds B and C will be located at the north-central portion of the site as shown in Figure 11J-3. Developed watershed B is the area that will contain proposed new waste rock stockpile (WRSP-1) described in section **20.6.7.11.J.(2)**. This area also contains existing waste rock stockpile EWRSP-2A. This existing stockpile will be enveloped and covered by construction of proposed new stockpile WRSP-1. Developed watershed C will contain proposed new waste rock stockpiles WRSP-2 and WRSP-3.

These developed watersheds and proposed waste rock stockpiles will be constructed, graded and contoured in a manner to direct precipitation runoff to lined storm water impoundments as shown in Figure 11J-3. Berms, swales and other conveyance structures will be constructed in the area and on the stockpiles, as needed, to provide controlled conveyance of storm water while preventing erosion and minimizing ponding.

The storm water impoundments will be designed and constructed to safely retain the volume of water generated from a 100-year 24-hour storm event and have a minimum 2 feet of freeboard. Details of the design of the impoundments are contained in section **20.6.7.11.J.(2)** and Appendix B.

Captured water from these storm water impoundments will be transferred to the process water reservoir in the process area for use as make-up water in the process. The storm water impoundments will be designed with a spillway structure to safely pass overflow water from an extraordinary event in excess of NMED design specifications.

Developed WS B is part of the open pit surface drainage area as shown on Figure 11J-3 and discussed in more detail above. As such, water released from the spillway at storm water impoundment B will pass via a culvert to the mine pit. In the case of storm water impoundment C, water released from the spillway will be released to the land down-gradient and adjacent to the impoundment. The water may find its way to Grayback Arroyo.

At closure, the waste rock stockpiles and the impoundments will be re-contoured, re-graded and covered as described in the Reclamation and Closure Plans.

### **Tailings Storage Facility Management**

The Tailings Storage Facility will be located as shown in Figure 11J-1. The area is naturally protected from storm water runoff passing through Grayback Arroyo by the natural topography of the site. As such there is only runoff from precipitation that falls directly on the TSF location to manage. The tailings dam will be constructed in phases as discussed in detail in Appendix A. Surface water run-on will be controlled during operations by grading and contouring the area to minimize the amount of water contributed into the tailings impoundment. Several run-on diversion ditches will be constructed at the up-gradient perimeter of the tailings impoundment to divert run-on around it per the requirements of the NM Office of the State Engineer. These diversion ditches will be reconstructed in each subsequent phase, as needed, as the dam expands in size until it reaches its ultimate height.





Runoff from the exterior face of the dam will be managed by constructing drainage ditches to control and direct runoff into the underdrain collection pond located at the southeastern toe of the dam (See Figure 11J-1). These drainage ditches will be constructed in a manner that minimizes the opportunity for ponding yet moves water in a controlled fashion to control erosion. The ditches on the exterior face of the dam will enter the lined water conveyance ditch that also holds the water recycle pipeline leading from the underdrain collection pond to the process facility. Runoff water will join the water collected from the tailings underdrains in the pond and will be transported to the process water reservoir for use as make-up water in the process.

The surface area of the cyclone plant, located immediately adjacent to the TSF as seen on Figure 11J-1, will also be contoured and graded to route runoff from the area into the lined water conveyance ditch and drained to the underdrain collection pond.

At closure the TSF and its attendant runoff diversion structures will be re-contoured, re-graded, filled and a cover will be placed over the entire area in accordance with the Reclamation and Closure Plans.



## **20.6.7.11.O MATERIAL CHARACTERIZATION AND MATERIAL HANDLING PLAN**

*An application shall include a material characterization plan and, if applicable, a material handling plan for all waste rock excavated at the copper mine facility pursuant to Subsection A of 20.6.7.21 NMAC.*

NMMC submitted a proposed Mine Plan of Operations (MPO) for the Copper Flat mine in December, 2011 to the Las Cruces, NM office of the Bureau of Land Management (NMCC 2010). This document, revised in June, 2011, was also provided to the NMED for review and comment as part of the process of preparation of an Environmental Impact Statement (EIS) by the BLM. Appendix C of the MPO contains a Mine Waste Management Plan for the waste rock which included a plan for waste characterization and handling. This plan provides the information as required by **20.6.7.11.O and 20.6.7.21.A NMAC**. Appendix C also contains results of waste characterization work performed by SRK in 1997 in support of an EIS that was being prepared for mining activities proposed by Alta Gold Corporation.

In the intervening time since the MPO was first proposed by NMCC in 2010, the waste characterization portion of the plan was implemented. The results of this work has been submitted and discussed with the NMED and is utilized in various sections of this DP application. NMCC will initiate the materials handling plan contained in Appendix C as contained in the MPO assuming that it is ultimately approved by the BLM, NMED and other constituent agencies.

### **MATERIAL CHARACTERIZATION**

As cited previously herein, SRK has performed extensive geochemical characterization studies in support of NMCC's proposed Copper Flat mine (SRK 2013). The resulting report and additional documentation requested by NMED upon review of SRK's report represent NMCC's material characterization efforts to date and are incorporated into this application by reference. As such, NMCC believes that its material characterization program and material handling plan are in conformance with the requirements of **20.6.7.21 NMAC**.

In brief, SRK conducted a mine waste characterization program for the Copper Flat project. The geochemical testing of mine waste materials provided the characterization required to determine the potential for Acid Rock Drainage and Metal Leaching (ARDML) from mining facilities. This provided the basis for a quantitative risk assessment and evaluation of the options for design, construction and closure of the tailings and waste rock disposal facilities.

The Copper Flat mine waste characterization program was designed to investigate the potential for ARDML due to exposure and oxidation of sulfide minerals, such as pyrite, that are unstable under atmospheric conditions. Upon exposure to oxygen and water, sulfide minerals will oxidize, releasing metals, acidity and sulfate. SRK's geochemical characterization investigated the potential for rock that will be exposed in the Copper Flat waste rock disposal facility and pit walls to generate acid and leach when exposed to the



atmosphere. The results of the characterization program were used in quantitative numerical predictions to assess the potential future leachate chemistry associated with the mine facilities, specifically the waste rock stockpiles and the TSF.

SRK's investigation concluded that with respect to waste rock, acid generation is not anticipated to occur for most of the un-weathered waste rock materials during operations. SRK concluded that the acid generating potential of the Copper Flat materials is largely dependent on the sulfide mineral content and that the sulfide concentrations in the material varied from less than analytical detection limits to a maximum of 2.52 weight percent (wt%) that was highest in the transitional waste material. Transitional material is the oxidized and partially oxidized surface rock material that overlies the fresh rock material below the surface. Where oxidation of this overlying material is complete, waste rock produced from it will be inert with respect to acid generation. Partially oxidized material occurs in a transition zone beneath the oxidized cover and the underlying un-oxidized material. This transitional material has been exposed to oxidizing conditions over geologic time. Such material will typically exhibit a low paste pH and high paste conductivity, and will be generally acid generating. Examples of this condition can be found in the exposed pit walls and where transitional waste material was deposited on the existing waste rock stockpiles.

SRK determined that 96% of waste rock that will be produced at Copper Flat will consist of sulfide, non-oxidized Quartz Monzonite/Breccia waste, which typically exhibited either non-acid forming characteristics or a low potential for acid generation. However, samples collected from the surface of the existing waste rock stockpiles and pit walls indicated that there is potential for acid generation from material mined by previous mining operations and exposed to natural weathering conditions.

With respect to tailings, SRK concluded that, based on tailings samples collected as part of the characterization program, that there is generally low potential for ARDML generation.

### **MATERIAL HANDLING PLAN**

NMCC's proposed material handling plan is based, in part, on SRK's recommendations to minimize the potential for acid leaching from the waste rock stockpiles and tailings. Based on SRK's findings the materials that will be generated will have only a low potential for acid leaching, NMCC anticipates that it will, generally, not be necessary segregate waste rock. As described in Appendix C of the MPO, most of the waste rock produced by the operation will be low and high sulfide material with only small amounts of oxide and transitional material produced near the surface of the proposed pit expansion. Additionally, it should be noted that the terms "high sulfide" and "low sulfide" are terms relative to specific conditions at Copper Flat and, therefore, to each other. That is, at Copper Flat low sulfide material is defined as material having less than 0.5% sulfide and high sulfide material will have 0.5% sulfide or more.



The waste will be primarily quartz monzonite and andesite and the ore will be primarily breccia. NMCC will implement a waste material classification program as described in Section 2.5 of the MPO. It is anticipated that the waste rock generated will oxidize very slowly and may potentially produce acid over some period of time. The ARD potential associated with un-oxidized waste rock is relative in the long-term. SRK determined that the waste rock can be considered as being inert with respect to ARD for a timeframe in the order approximately 20 years. Therefore, the vast majority of the waste material, i.e., about 96%, that will be generated will generally have a low ARD potential. As indicated above, the transitional materials that have been exposed to oxidizing conditions over time have the most potential for acid generation.

### **WASTE MATERIAL CLASSIFICATION**

Section 2.6.2 of the MPO generally describes the general waste classification approach that will be implemented at Copper Flat. The subsequent work conducted by SRK and presented in its May, 2013 Geochemical Characterization Report confirms the approach to be utilized. However, while the approach is generally the same as described in Appendix C of the MPO, there will be some aspects of material handling that may differ from the information provided therein as supported by the later findings of SRK's material characterization studies. The overarching approach to waste rock material handling to control ARD will be to control the movement of water through the waste rock stockpiles in combination with continual diligent monitoring and characterization of the waste materials produced to confirm SRK's conclusion that the majority of the material has a low ARD potential. As discussed in more detail below, NMCC believes that depositing the small amounts of material with high ARD potential waste materials, i.e., the transitional waste, when encountered, along with the large amounts of non-ARD materials will further reduce the potential for the ARD materials to create acid. The buffering capacity of the large volume non-transitional waste will neutralize the small volume transitional waste. Should field characterization reveal that more ARD materials than anticipated are being generated the materials handling plan will be adjusted to consider isolation, encapsulation and other means of treatment to mitigate the potential for acid generation. The non-transitional waste will be used as base material in any areas where it has been determined in the field that transitional material should be segregated. However, as a practical matter, under normal circumstances large volumes of non-transitional waste will typically be placed below, above and all around the transitional waste produced.

SRK recommended in its May 2013 report that, during proposed operations, specific controls would be needed to collect storm water runoff from the waste rock stockpiles and that storm-water diversions would be required to prevent run-on. SRK also recommended that covering the waste rock stockpiles with a re-vegetated 36-inch cover at the end of mine life would reduce infiltration of water and flux of oxygen into the facility, and thus, limit oxidation of sulfide minerals. SRK also noted that migration of seepage from the waste rock stockpiles into the underlying bedrock would be anticipated to be very small, or nil, because of the low permeability of the andesite underlying the area. These recommendations have been incorporated into NMCC's design of its waste rock stockpiles.



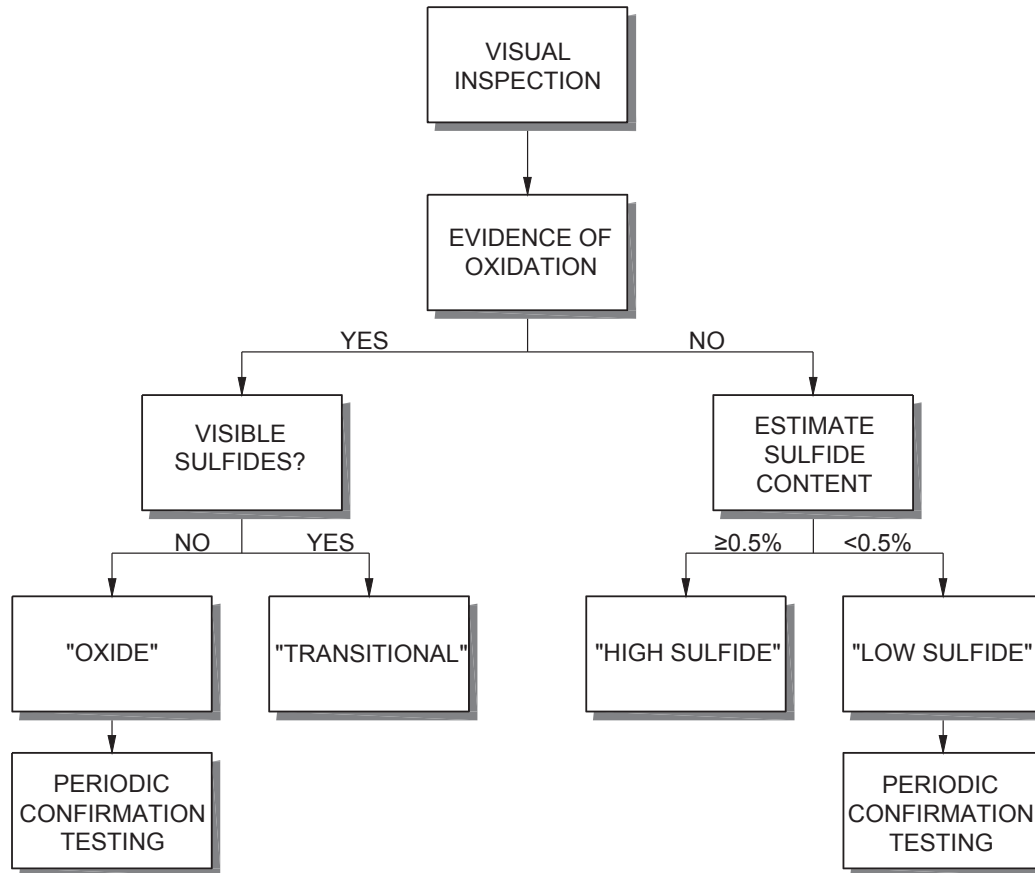
Figure 11O-1 describes process that will be utilized in the NMCC's proposed waste classification program during operations. Following the evaluation path shown in Figure 11O-1, a determination will be made as to classification, and periodic confirmation testing will be conducted. Confirmation testing will include a field testing program for representative samples of the cuttings, as determined by the qualified geologist or technician. The tests conducted will include paste pH, saturated paste conductivity, and acid base accounting testing (total sulfur, sulfate sulfur, and NP testing).

As described in more detail in Section 2.6.2.5 of Appendix C of the MPO, prior to blasting active benches in the open pit, the drill cuttings from each drill blast hole will be inspected. Blast-hole drill cuttings will be visually inspected by a qualified geologist or trained technician prior to blasting and removal of the material from the pit. The rock type, color, degree of oxidation, sulfide content and other pertinent features will be noted and transferred to the bench plan maps. All materials characterized as oxide or low sulfide waste will be sampled for confirmation testing. Material classified as low sulfide waste rock will be subject to periodic confirmation testing at a frequency initially of one confirmation test for each five blastholes designated as oxide waste rock. This frequency will be adjusted as ongoing testing and field observation continues to demonstrate consistent reproducible results supporting visual waste classification. NMCC anticipates a frequency of confirmation testing in the longer term to be one test for every 20 holes. Confirmation testing will be performed on-site using a Leco Furnace to evaluate the classification determinations made.

### **WASTE ROCK FLAGGING AND ROUTING**

Waste rock from the open pit will be examined as benches are mined to identify sulfide bearing transition material that may represent ARD potential. Specific procedures will be established by the mine operations team when preparing for start-up; however it is anticipated that the procedures employed by the operation will be similar to the following description, which follows standard mine geology practices and methods. The operations team will develop full details after the team is assembled to begin startup of the mine. As requested by NMED, NMCC will meet with NMED staff as plans develop to discuss the plan and receive input prior to implementation.

The mine waste rock identification, flagging, and routing process will be similar to mine ore control procedures and the two processes will be completed simultaneously. Identification, digging plan design, field identification for operations, determination of destination and placement, and routing of ore and waste materials will be the responsibility of the mine technical services team, which typically includes geology engineering, and surveying disciplines. The mining and ore/waste classification cycle begins with blasthole drilling. At Copper Flat, all benches mined will require blasting; therefore all areas mined will be drilled on a regular grid pattern across the full width and length of each bench. Each blasthole will be assigned a unique identification number (ID) for data tracking, and each will be surveyed and plotted onto a blasthole location map that is spatially tied to a three dimensional (3D) model of the mine geology. As holes are



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 COPPER FLAT PROJECT  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**OPERATIONAL WASTE CLASSIFICATION FLOWCHART**

PROJECT No. 153-1453	PHASE 0006	Rev. 1	FIGURE 110-1
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drilled and surveyed, the ID and survey coordinates of each blasthole will be logged into a blast hole data base. After drilling, and before blasting, the geology of the bench surfaces and exposed mine faces will be examined for key geologic parameters and the geology mapped to the same scale as the blast hole maps. The blasthole cuttings will be visually examined to determine rock type, sulfide content, and oxidation level and samples taken for laboratory analysis. Data from field examinations will be logged into the blasthole data base and plotted onto the blasthole map, with the analytical results added to the maps and the database upon receipt from the laboratory. When the geologic and analytical data for a specific area is complete, the technical services team will develop ore and waste zone boundaries and identify material types that are subject to a specific routing plan, such as for ore transport or transport of potential ARD material to the WRSP. The boundaries and material designations will be transferred back to the blasthole maps and survey coordinates produced for identifying material boundary lines in the field.

After blasting, and before excavation begins, the material boundary lines will be established on the top of the broken rock by survey, and the broken rock visually examined again to determine if any field adjustment in classification is needed; field adjustments will be transferred to the blasthole maps for record keeping. Even after having been blasted, the area will be closed to excavation and material removal until all data is received and the “dig plan” is finalized by the technical services team. When an area is opened for excavation of material, the specific ore and waste boundary lines will be visually identified on the top of the broken rock and specific material types designated with color coded flagging. Copies of the blasthole maps showing the corresponding material types and boundaries will be provided to the mine equipment operators for reference during material removal.

As excavation proceeds, the loader operators will selectively excavate specific ore or waste material types following the dig plan established by the technical services team. Each haul truck will be loaded with only the one type of material designated. After loading, the loader operator will communicate the material type to the truck operator through an established signal system. The material type loaded into the truck will designate a pre-determined destination for the load. The truck operator will track loads by material type and destination and the load information will be compiled and maintained by mine staff for reporting.

#### **TRANSITIONAL WASTE MATERIAL DISPOSAL**

Section 2.6.2.2 of Appendix C of the MPO provides that material classified as “transition waste” will be isolated in a waste rock stockpile area and covered with a minimum of six feet of “non-transition” material. SRK’s investigation determined that the transitional material to be excavated will be less than 4% of the total volume of waste produced from the operation. SRK’s analysis was performed prior to development of the most recent mine plan for Copper Flat which is reflected in this DP application. Even so, the most recent mine plan indicates that transitional material will still be produced at the same relative ratio as compared to non- transition waste. NMCC anticipates that the transitional



waste will be produced in the first 8 years of operation (approximately 5.4 million tons), with about half of it produced in the first 2 years. Some of this material will be disposed of in WRSP-1, which is located in the OSPDA. The remainder will be disposed of in WRSP-2 and 3. During the same two years as much as 5.2 million tons of non-transitional acid neutralizing waste material, will also be produced. Some of this acid neutralizing material will be used as neutralizing material for those areas where transitional material may be deposited. NMCC will lay a minimum 10 ft. of base of non-transitional waste underlying the area where transitional material will be deposited in the WRSPs and ensure that at least 10 feet of non-transitional waste surrounds the transitional waste in such a manner that the transitional waste is not exposed to oxidation.

The remaining approximate 2.6 million tons of transitional material will be produced over years 3 through 8 at an average rate of approximately 433 thousand tons per year while at the same time about 27.6 million tons, an average of 4.6 million tons of acid neutralizing non-transitional waste will be produced. As such, the greatest volume of waste material generated, by far, will be classified as un-oxidized high sulfide and/or un-oxidized low sulfide waste. As confirmed by SRK's waste rock characterization investigations, this material poses negligible short-term risk of ARD. While NMCC considers it unnecessary, and perhaps to some extent, contrary to the desire to minimize potential acid generation, to isolate and concentrate that material in one area, NMCC will continue to identify potential ARD generating waste during operations and take steps to establish disposal areas within the WRSPs for this material ensuring that a minimum of 10 ft. of non-transitional acid-neutralizing waste surrounds the transitional waste within the WRSPs where the potential ARD generating material will be deposited. As a practical matter, NMCC will ensure that non-transitional material is placed below, above and all around the transitional material wherever possible, providing a thick neutralizing "blanket" around the transitional material.

Quality assurance testing will also be performed in addition to the daily field sampling. Up to 10 archived blast-hole samples will be randomly selected and subjected to paste pH, saturated paste conductivity, and acid base accounting testing (total sulfur, sulfate sulfur, and NP testing). The testing will be performed by a third-party independent state approved laboratory. The samples will be classified with respect to ARD potential on the basis of NP/AP ratio. The samples will be located on the appropriate bench plan maps and the quality assurance test classifications will be compared to the operational waste classification designations.

At the end of the mine life the waste rock stockpiles will be reclaimed in accordance with the approved reclamation and closure plan. The waste rock stockpiles will be covered with a minimum of 3 ft. of soil completely mitigate the potential for acid generation and impacts to groundwater.

SRK has performed significant kinetic testing of waste materials since the time of submittal of the MPO. Humidity cell testing results were first reported to NMED in SRK's May, 2013





report. Continued and more extensive humidity cell results were reported to NMED in February, 2014 in an SRK report titled “Humidity Cell Termination Report for the Copper Flat Project, New Mexico”. These documents are included in this Discharge Plan application by reference. As indicated before, these documents provide the basis for NMCC’s mine waste characterization and handling plans.



## **20.6.7.11.P HYDROLOGIC CONCEPTUAL MODEL**

*An application for a discharge permit for a new copper mine facility shall include a site hydrologic conceptual model providing:*

**20.6.7.11.P.(1)** *a description of the hydrogeologic setting at the copper mine facility including ground water potentiometric maps, surface water drainages and flows, types of ground water and surface water recharge and its distribution, and hydrologic boundary conditions and divides;*

NMCC and its water resource consultant, John Shoemaker & Associates (JSAI) have conducted extensive hydrologic investigation of the Copper Flat site and the surrounding area in support of NMCC's permitting activities for the project. These activities include supplementing the NMCC Abatement Plan previously submitted to NMED and the BDR submitted to NMED and NM MMD, providing detailed analysis for the BLM Environmental Impact Statement (EIS) and supporting various requests for information by the NM Office of the State Engineer (OSE).

NMCC has previously submitted to the various agencies, including NMED, JSAI's document titled "Model of Groundwater Flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico, August 2014" (JSAI 2014c). This document, and the subsequent review documentation provided by NMCC and JSAI, is therefore, incorporated into this DP application by reference. The following information is a synopsis of the volumes of information already provided in the groundwater model documentation in an effort to provide specifics as required by the Copper Rule.

### **Hydrogeologic Setting**

The Copper Flat facility will be located in area that includes the following water-bearing formations within the Animas Uplift: crystalline bedrock (andesite and quartz monzonite) in the western part of the mine permit area, Santa Fe Group sedimentary deposits in the eastern part of the mine permit area, and alluvium of upper Grayback Arroyo overlying the crystalline bedrock and Santa Fe Group in the mine permit area. Figure 11K-9 Provided in subsection **20.6.7.K.(3)** presents the water-bearing formations at the facility, potentiometric surface contours and direction of groundwater flow, selected monitoring wells, and topography. Figure 11K-2a, provided in subsection **20.6.7.K.(2)** of the application, presents a geologic map of the facility and surrounding area, and transects of hydrogeologic cross-sections.

Groundwater flow is mainly from west to east in the mine permit area, with groundwater discharging from the crystalline bedrock as subsurface flow across the contact with the Santa Fe Group, and as evaporation from the open pit. The potentiometric surface contours shown in Figure 11K-9 in the vicinity of the existing open pit indicate that the open pit is a hydraulic sink. Cross-sections PA-PA' and PX-PX' presented in Figures 11K-2a, 11K-4 and 11K-5 show groundwater flow in the crystalline bedrock in the vicinity of the open pit.



Monitoring wells designated as “dry” in Figure 11K-9 are shallow wells installed to depths of less than 60 ft. in the Upper Santa Fe Group. Hydrogeologic cross-sections presented as Figures 11K-6a and 6b and 11K-7a and 7b depict the Upper Santa Fe Group in the mine permit area.

The crystalline bedrock at the facility is relatively impermeable and groundwater recharge from local precipitation to the crystalline bedrock is limited by low hydraulic conductivity. The groundwater system at the facility conducts little water, and the eastern mine permit boundary generally coincides with the East Animas Fault, shown in Figures 11K-2a and b and 11K-9, which acts as a barrier to groundwater flow.

A portion (approximately 230 acres) of the original Grayback Arroyo watershed within the mine permit area now drains to and includes the open pit. Grayback Arroyo is an ephemeral drainage in the mine permit area. However, groundwater levels are close to the surface, and there can be base flow discharge to Grayback Arroyo following wet periods. The cross-section A-A', presented in Figures 11K-2a and 11K-3a and 3b, show the alluvium of upper Grayback Arroyo overlying the crystalline bedrock and Santa Fe Group in the mine permit area.

**20.6.7.11.P.(2)** *the site hydrogeologic setting relative to both local and regional hydrology and geology including appropriate cross-sectional diagrams depicting major geologic formations and structures, aquifers, and ground water depths;*

The Copper Flat porphyry copper-molybdenum deposit is hosted by the Cretaceous quartz monzonite in the western part of the mine permit area (Fig. 11K-2a). The quartz monzonite intruded andesite of similar age. Faults to the north and south of the mine permit boundary juxtapose the andesite with older, Paleozoic sedimentary rocks. The eastern mine permit boundary generally coincides with the East Animas Fault, which defines the eastern edge of the Animas Uplift. The Santa Fe Group deposits in the mine permit area are located west of the East Animas Fault. As indicated in the section above, Figures 11K-3a and 3b through 11K-7a and 7b present hydrogeologic cross-sections showing geologic formations, water-bearing formations, and groundwater depths.

Most of the precipitation that recharges the groundwater system at the facility originates in the upper part of the watersheds to the west of the mine permit area. The main groundwater systems in the region are found in Santa Fe Group sedimentary deposits downstream of the mine permit area, with groundwater conveyed through more permeable Paleozoic sedimentary rocks located to the north and south of the facility. Runoff from Grayback Arroyo infiltrates the Santa Fe Group sedimentary deposits downstream of the mine permit area.



**20.6.7.11.P.(3)** potential sources of water constituents including discharge types and their locations;

Table 11P-1 presents a summary of the potential sources of water constituents, discharge types, and locations. Figures 11J-1 and 11J-3 show the locations of the various potential sources.

<b>TABLE 11P-1 Potential Sources of Water Constituents, Discharge Types, and Locations</b>		
<b>Potential Source</b>	<b>Discharge Type</b>	<b>Source Location</b>
Tailings Storage Facility (TSF)	Tailings, process water and impacted storm water	Southeast area of site
TSF cyclone plant surge pond	Tailings	Southeast area north of TSF
Tailings slurry pipeline conveyance	Tailings	Process Area to TSF Permit Area Boundary
TSF water recycle system-under-drain collection pond	process water and impacted storm water	Eastern edge of site
TSF water recycle system pipeline conveyances	Process water and impacted storm water	TSF area to process water reservoir
Process water reservoir	Process water and impacted storm water	East-central area of plant site
Impacted storm water impoundment A	Impacted storm water and process water	Plant site Area
Impacted storm water impoundment B	Impacted storm water	Southwest corner of WS B
Impacted storm water impoundment C	Impacted storm water	Southeast corner of WS C
Waste Rock Stockpile (WRSP)-1	Impacted storm water	Western side of Watershed (WS) A
WRSP-2	Impacted storm water	Western portion of WS B
WRSP-3	Impacted storm water	Eastern portion of WS B
Open Pit	Impacted storm water, mine water	Western side of site
Material handling and processing-primary crushing	Process water	Central portion of site within WS A
Material handling and processing-crushing and grinding	Process water	Central portion of site within WS A
Material handling and processing-flotation and concentration	Process water	Central portion of site within WS A
Plant site sumps, tanks, pipelines and truck and equipment wash units	Process water	Central portion of site within WS A
Packaged water treatment plant	Influent sanitary waste, treated effluent water to the TSF	Central portion of site within WS A
Mobile Equipment Fuel Station	Petroleum Products-Diesel, Gasoline, Oil	Central portion of site within WS A



**20.6.7.11.P.(4)** *potential pathways for migration of water constituents to ground water and surface water;*

The potential pathways for migration of constituents to ground water from the sources identified in Table 11P-1 could be either from direct infiltration into the water bearing formations, release of fluids to the ground surface, or run-on and runoff of precipitation through and off of the site.

All of the impoundments, ponds, and reservoirs will be lined with synthetic liner materials as described in more detail above. Similarly, the conveyance ditches for the tailings to the TSF and convey process water from the TSF and other impoundments for reuse in the process will also be lined. As such, the potential for these structures being a pathway for migration is minimal and would only occur in the event of unanticipated events such as improper installation or operation, faulty materials, or exceedance of the design limits of each structure. All of the impoundments, ponds, and reservoirs and conveyance ditches are all designed to eliminate run-on and capture run-off to the maximum extent possible and contain storm events in accordance with the requirements of the Copper Rule. Therefore, the potential for these sources be sources of migration of constituents to ground water or surface water is further reduced.

As discussed in more detail above, the waste rock stockpiles will be constructed over bedrock that has a permeability of  $10^{-6}$  cm/sec. As such, these areas will not require lining. While the waste rock stockpiles represent a potential source of migration on constituents to ground water, that potential is significantly mitigated by the natural barrier to migration that exists.

The waste rock stockpiles also represent sources of potential pathways to migration of constituents to surface water. However, as discussed in detail above, each of the waste rock stockpile areas will be constructed within a developed watershed designed to eliminate run-on, control and capture runoff in a lined impoundment. Surface water diversion ditches and swales will be constructed within the waste rock stockpile areas to divert water to the impoundments at a controlled rate while reducing the potential for ponding and infiltration.

The process area also represents a source of potential migration of constituents to ground water and surface water. With respect to ground water, the potential pathways to migration exist largely in the form of process water handling within the process, in particular at sumps that hold and transfer water from one part of the process to another. The sumps in the process area, including the crushing and grinding and flotation and concentrating areas, are designed and will be constructed using techniques such as water-stops and single monolithic pours to reduce the likelihood of release.

With respect to the potential for release to surface water, the process area will be constructed within a developed watershed that will be graded such that run-on will be



eliminated and runoff will be controlled and captured in a lined impoundment. Water from all of the process facilities, material handling areas, parking areas, storage areas, chemical and fuel inventory areas and pipeline surface areas will be routed to the impoundment.

The open pit mine also represents a source of potential migration of constituents to ground water and surface water. With respect to ground water, the mine pit will receive nominal amounts of ground water inflow because the mine is a hydrologic sink, as discussed in more detail in section 20.6.7.11.K.(3), above. Because it is a sink migration will all be inward to the mine and not away from the mine.

With respect to surface water, the mine pit will be constructed in a manner within a watershed area such that all surface water within the watershed will run into the bottom of the mine pit. The water will be pumped out of the pit and used for dust control on the site and as process water whenever possible. The water will be applied in a manner that controls dust but minimizes the potential for infiltration and ponding.

***20.5.7.11.P.(5) any surface waters of the state that are gaining because of inflow of ground water that may be affected by water constituents discharged from the copper mine facility.***

Grayback Arroyo does not flow perennially in the mine permit area, but groundwater levels are close to the surface, and there can be occasional base-flow of ground water to Grayback Arroyo following wet periods. This occasional surface water has the potential to be affected by water constituents discharged from the facility.



### **20.6.7.11.Q WASTE MINIMIZATION PLAN**

*An application shall include a waste minimization plan to implement, as practicable, best management practices for minimization and recycling of process water and wastes generated at the copper mine facility to reduce the potential for impacts to ground water.*

NMCC will implement best management practices with respect to the handling of water at the Copper Flat mine in a manner that maximizes the opportunity to recycle process water wherever possible and minimize the generation of wastes and reduce the potential impacts to ground water. The largest volume of waste generated at Copper Flat, by far, will be from the open pit mine and from the processing of ore. NMCC anticipates producing approximately 33 million tons of waste rock over the life of the mine to be stockpiled as described above. Processing of ore will result in the disposal of approximately 113 million tons of tailings at the Tailing Disposal facility.

The opportunity for waste minimization at the waste rock stockpiles is limited by the amount of non-ore material that must be removed from the mine in order to maximize the amount of ore mined and processed. Similarly, the opportunity for waste minimization of the tailings is limited by the amount of copper, molybdenum, and other produced that can be economically removed from the ore. However, there exists significant opportunity to minimize the amount of water that will be utilized at the facility by capturing and recycling maximal amounts of water from the stockpile areas and the tailings disposal facility.

As described in more detail above, NMCC has designed the Copper Flat facility to maximize water use and reuse. The tailings disposal facility is designed with a liner system and underdrain system that will remove water from the bottom of the tailings impoundment, capture it in a lined underdrain collection pond, and return it to the process facility for reuse. The tailing dam itself is also designed with a dam underdrain system that will capture water from the sand fraction of the tailings used to construct the dam, route it to the underdrain collection pond and then to the process for reuse. The outside face of the dam will also be designed so that water run-off from the dam will be captured, directed to the underdrain pond and then to the process for reuse.

In addition, the tailings facility will be equipped with a “water reclaim barge”, a system of pumps attached to a floating barge within the surface of the ponded water in the interior of the tailings disposal facility. Free water from the impoundment will be pumped from the impoundment to a process water reservoir located in the process area in order to maximally reuse water from the tailings facility in the process. The pipelines through which the water from the reclaim barge and the underdrain collection pond will be transported will lie in a conveyance trench that is also lined to prevent possible releases from impacting groundwater. Similarly, the run-off conveyance structures will be lined as they are routed to the pond.

The average water recovery rate from these systems is anticipated to be approximately 9200 gallons per minute which represents about 73% of the water needed for the process. This represents a huge reduction in the potential to impact ground water.



In addition to this significant amount of water recycle, as described in more detail above, the waste rock stockpiles will be designed such that water run-off from them will be routed through lined trenches into lined impacted storm water impoundments. The captured water will be pumped to the process water reservoir for use in the process. Similarly, the process area will be contoured and graded to route run-off into a lined impacted storm water impoundment that will, in turn, be pumped to the process water reservoir for use in the process. All of these systems are designed to minimize the production of waste and recycle of water to reduce the potential for impacts to ground water.

Other waste management and minimization activities to reduce the potential for impacts to groundwater include the installation of a packaged wastewater treatment plant to capture and treat domestic wastes generated at the site. Treated water from the plant will be piped to the tailings storage facility to join the water in the impoundment and ultimately recycled along with the other water in the impoundment.

Mine operations will result in the generation of small amounts of non-hazardous and hazardous waste materials. With respect to non-hazardous wastes generated, including such items as waste paper, wood, scrap metal and other domestic trash, NMCC does not expect that these types of wastes pose potential impacts to ground water. However, NMCC will recycle such materials to the maximum extent possible or be disposed of in a permitted on-site Class III land fill.

With respect to hazardous and other chemical wastes, NMCC anticipates that the Copper Flat mine will be classified as a “small quantity generator” of such wastes. All such wastes will be managed and transported off-site by a licensed contractor for disposal in accordance with state and federal regulations.





### **20.6.7.11.R GROUNDWATER MONITORING WELLS**

*An application shall include the location of all existing and proposed ground water monitoring wells pursuant to 20.6.7.28 NMAC.*

Appendix E, "Water Quality Monitoring Plan for the Copper Flat Mine Discharge Permit, Pursuant to 20.6.7.11.R and 20.6.7.28 NMAC", November 2015, prepared by JSAI provides the information requested.



### **20.6.7.11.S FLOW METERING SYSTEM**

*An application shall describe a copper mine facility's flow metering system pursuant to Paragraph (5) of Subsection C of 20.6.7.17 NMAC, Subsection E of 20.6.7.18 NMAC, and Subsections C and E of 20.6.7.29 NMAC, including:*

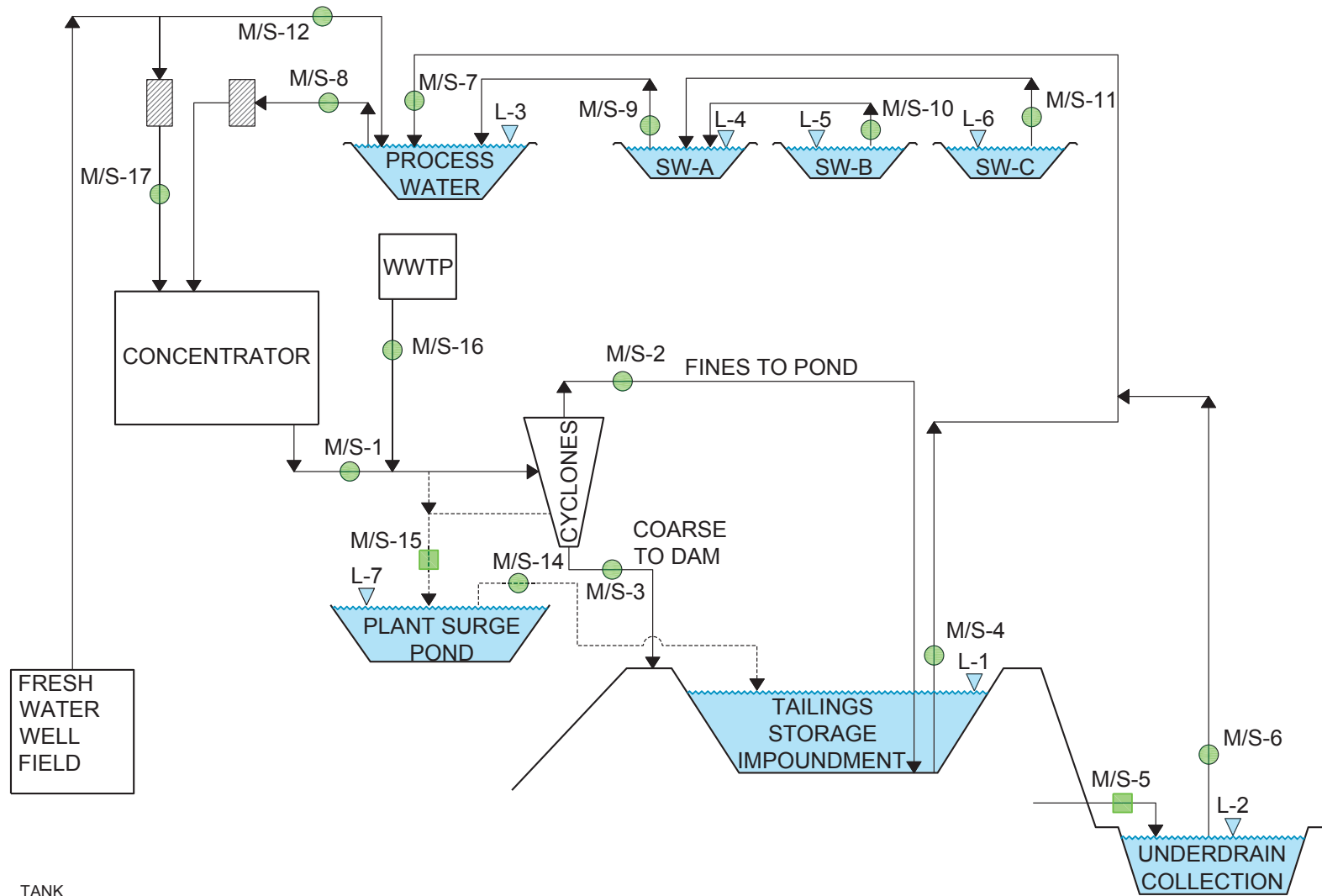
- (1) the method(s) (i.e., pumped versus gravity flow) of process water discharge and stormwater transfer and handling;*
- (2) the proposed flow measurement devices for each flow method and information about its type and capacity; and*
- (3) the location of all existing and proposed flow meters required pursuant to the copper mine rule or a discharge permit.*






The flow of process water at the Copper Flat facility is a closed loop. Process water and tailings will be handled using a combination of transfer methods, including gravity flow and mechanical pumps. Figure 11S-1 is a schematic diagram that shows the sources of water and the metering locations. Figures 11J-20A and 11J-20B (see Section 20.6.7.11.J.(5) above) are scaled maps that identify the locations of the flow meters and fixed pumps. The source of water to the process will be from the fresh water well field and the process water reservoir which will contain a combination of water pumped from the off-site fresh water well field, recycled water pumped from the tailings impoundment and captured storm water, pumped to the process water reservoir, when available.

As shown on Figure 11S-1, fresh intake water will be pumped from an off-site well field on-site to the freshwater tank and/or the process water reservoir and metered at these locations. Process make-up water will be pumped and metered from the process water reservoir into the process facility through the process water tank. Maximal utilization of recycled water will minimize the need for freshwater to the extent possible.

Mine ore fed to the SAG mill will be ground and pulped for the concentration and flotation process. The flotation overflow materials will be directed to the remaining steps of the product concentration process. The underflow material from the flotation circuit will be discharged from the sump and transported as whole tailings materials via gravity flow to the cyclones at the tailings storage facility. Outflow from the flotation sump will be metered as shown on Figure 11S-1.

The cyclones will separate the sand or coarse fraction from the slimes or fines fraction of the tailings. The cyclone underflow or sand fraction will be pumped and metered to the tailings area where it will be used as the material to construct the dam. The cyclone overflow or slimes fraction will be pumped and metered to the interior of the tailings storage facility behind the dam where it will be deposited and allowed to separate as free water which will form a pool of water, and sludge which will separate from the free water and line the inside of the impoundment.



-  TANK
-  WEIR BOX/SAMPLING
-  FLOW METER/SAMPLING
-  WATER LEVEL
-  FLOW DIRECTION
- WWTP WASTE WATER TREATMENT PLANT
- SW-A STORMWATER POND A

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 COPPER FLAT PROJECT  
 SIERRA COUNTY, NEW MEXICO  
 TITLE  
**FLOW METER AND SAMPLING LOCATIONS**

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The free water will be pumped off of the surface of the impoundment using a floating pump barge located within tailings pond. The water will be pumped and metered at the barge outflow back to the process area, through a recycle water pipeline to the process water reservoir for reuse back in the process.

An underflow drain system will be installed beneath the dam and at the bottom of the lined tailings impoundment. The underflow drain system will collect water from the bottom of the impoundment and the bottom of the dam. This water will be directed to an underdrain collection pond constructed immediately down-gradient from the tailings impoundment. The underdrain collection pond will also be utilized as the impacted storm water impoundment for storm water runoff captured off of the downstream face of the dam. Water produced from the underdrain collection gallery and the storm water collection ditches will be measured at a weir box installed at the inflow point of the ditch into the underdrain collection pond. The water in the underdrain pond will be pumped and metered at the pond pump discharge into the underdrain recycle water pipeline which will join the recycle water pipeline and pumped to the process water reservoir where it, too, will be utilized as process make-up water. Recycle water from the tailings impoundment and underdrain water from the underdrain collection pond in the pipeline will also be metered upstream of union between the barge recycle water stream and the underdrain pond water stream as shown in Figure 11S-1.

NMCC will also construct three impacted storm water impoundments to capture runoff from the Copper Flat site facilities. The water collected therein will also be pumped metered at each impoundment pump outlet to the process water reservoir as shown in Figure 11S-1 a used as process make-up water.

As indicated in Subsection **20.6.7.11.J.(11)** of this application, NMCC will utilize a packaged wastewater treatment plant for domestic wastes. The treated water will be pumped and metered as it joins the tailings slurry that is pumped to the TSF.

The processing facility will be designed with contingency mechanisms that will allow diversion of process water from the process area and the cyclone plant directly to the surge pond in the event of upset conditions. Transport of these flows will be accomplished using the pumps in the process and by gravity in the secondary containment ditch which is designed to route upset flows to the surge pond. The contingency pipelines will also be metered as shown in Figure 11S-1. A weir box will be installed at the secondary containment ditch entrance to the surge pond to measure inflow.



### **20.6.7.11.T CLOSURE PLAN**

*An application shall include a closure plan for all portions of a copper mine facility pursuant to Subsection A of 20.6.7.18 NMAC, 20.6.7.33 NMAC, 20.6.7.34 NMAC and 20.6.7.35 NMAC unless closure of the copper mine facility is covered, or will be covered, by a separate closure discharge permit.*

This section describes NMCC's Closure Plan per the requirements of the Copper Rule. In November 2012 NMCC submitted its Mine Operation and Reclamation Plan (MORP) (NMCC 2012) to the New Mexico Mining and Minerals Division. In 2010 NMCC had also submitted its MPO to the BLM. These documents are currently under review by the respective agencies and will be incorporated into this Discharge Plan upon approval. The 2012 MORP, including the reclamation plan, was reviewed by MMD, was determined administratively complete and underwent technical review by MMD and other agencies, including the NMED. A series of comments and requests for additional information and NMCC responses followed, including submittal of various documents that have since modified the original 2012 submittal. All of this documentation is considered as part of the application documentation for the MORP, and, therefore, for this DP application as well.

An Updated MORP was prepared by NMCC in late 2016 to provide more detailed information regarding NMCC's proposed mine operation and reclamation plans. The Updated MORP was submitted to the MMD and NMED in October 2016. Comments were received from the MMD in April 2017 and responded to by NMCC in July 2017. Revisions to the Updated MORP are included herein, where applicable.

The Reclamation and Closure Plan component of the Updated MORP contains information required by the Discharge Plan application. In effect, the approved MORP and the approved Discharge Permit will be one and the same as they relate to groundwater protection and closure as one cannot be approved without obtaining approval of the other. NMCC's MORP documentation contains an Appendix E, Mine Reclamation and Closure Plan, which fulfills the requirements of subsection **20.6.7.11.T, Closure Plan**, of the Copper Rule. The information provided in this Discharge Permit Application more generally addresses the closure requirements. However, the Reclamation and Closure Plan, in Appendix E of the Updated MORP, contains the detailed reclamation/closure design and implementation required.

### **20.6.7.18 General Operational Requirements**

In accordance with subsection **20.2.6.7.18.A** of the Copper Rule, Planning for closure, NMCC has designed and will operate its mine facility units in a manner that considers implementation of its Closure Plan.

In accordance with subsection **20.6.7.18.A.(1)** NMCC has identified material on-site that is suitable for use to construct the necessary covers. The suitability of soils for reclamation purposes will relate to physical or chemical parameters. Steep slopes limit harvesting



topdressing materials due to increased erosion potential and the difficulty in establishing vegetation to re-stabilize the slope, whereas chemical parameters can affect how well a soil provides nutrients to plants during the re-vegetation process. Soil texture, the relative amount of sand, silt, and clay, affects water available to plants (available water holding capacity, AWHC), rate of water movement into and through the soil, and seed-soil contact during germination. The relative size of soil particles can also affect the total porosity and water holding capability of the soil. Larger cobbles and stones can effectively decrease porosity and thus the water holding capability of the soil.

Chemical components of soils, such as the amount of calcium carbonate present, the pH, sodium, and salinity can impact various aspects of the soil. Calcium carbonate can affect the availability of nutrients, affect the hydraulic properties of the soil, and can limit root growth in high concentrations. The pH can affect the availability of nutrients. Salinity will compete with plant roots for water in the soil, and sodium, although it has little effect on plants, can act as a dispersing cation and degrade soil structure which can alter water movement into and through the soil.

Topdressing, for the purposes of reclamation, refers to soil and/or geological material used as growth media or soil cover that will sustain vegetation and reduce infiltration of precipitation through the underlying material. The topdressing or cover materials required to support re-vegetation and reclamation efforts will be obtained from within the footprint of the new TSF, WRSP-2 and WRSP-3. The amount of growth media to be removed has been estimated on the basis of the soil survey included in the BDR. In response to agency comments NMCC submitted an addendum to the BDR (THEMAC 2013), which contains a Technical Memorandum (Insert C), prepared by Golder Associates which, includes a Supplemental Soils Investigation Report providing additional characterization and suitability assessment. The report provides a detailed description of the soil resources at the site to support reclamation planning. A borrow investigation was performed to assess the range of available soil materials. The suitability criteria provided in the original BDR document were revised based on the new information. QA/QC salvage procedures will be implemented in the field to identify and segregate suitable growth media, including performing soil water characteristic curve analyses to achieve the water holding capacity as required by **20.6.7.33 NMAC**. Section 3.4 of Golder's Supplemental Soils Investigation report discusses the suitability of the soils for use as cover material. Section 4.0 provides estimates of the volume of cover material available on-site and the amount that will be required to meet the cover requirements. Reclamation cover is further discussed in Section 4.5 of the Updated MORP and Sections 3.1 and 5.5 of the Reclamation and Closure Plan. In general, the soil materials identified by Golder are suitable for use in the primary or secondary root zone and acceptable for use as soil cover. The Updated MORP further refines these volume estimates.

Salvaged topdressing materials will be stored in the growth media stockpiles shown in Figure 11J-1. It will be stockpiled so as not to be disturbed by mining operations. The side-slopes of these stockpiles will be shaped with overall slopes of 3H:1V or shallower to minimize soil loss. To further minimize erosion and the establishment of undesirable



weeds, the stockpiles will be seeded with an interim seed mix. Diversion ditches will be constructed up-gradient of the stockpiles, where necessary, to minimize run-on erosion. BMPs such as silt fences or staked straw bales will be used as necessary to capture sediment and reduce soil loss.

In accordance with subsection **20.6.7.18.A.(2) NMAC**, NMCC has considered the grading and drainage plans in its design of the facility, including its waste rock stockpiles and tailings impoundment (See subsection **20.6.711.J.(2)** above).

### **20.6.7.33 Closure Requirements for Copper Mine Facilities**

In accordance with **20.6.7.33.A NMAC** all permanent storm water conveyances, ditches, channels and diversions have been designed to convey the peak flow generated by the 100-year return storm event. NMCC has used the same techniques in designing the water management aspects of the Closure Plan, including selection of the duration and maximum peak flow generated from the 100-year storm, as it has for the operation design components of the mine.

With regard to slope stability at closure, the tailings storage impoundment is regulated by the New Mexico OSE. As such, subsection **20.6.7.33.B** of the Copper Rule does not apply to the TSF. NMCC's proposed new waste rock stockpiles have been designed and will be constructed in a manner that promotes long-term stability at closure. There are no critical or non-critical structures as defined in **20.6.7.7.B.(16) NMAC** at the Copper Flat project. At closure an analysis will be conducted of those structures or units to which **20.6.7.33.B** applies that will include an evaluation for static and seismic liquefaction.

#### **Tailings Storage Facility**

Surface re-grading of the tailings impoundment at closure will be performed that will provide a stable configuration to minimize ponding and promote conveyance of surface water per subsection **20.6.7.33.C NMAC**. The final grade of the top surface of the re-contoured impoundment will be at least 0.5% after accounting for the magnitude and location of large-scale settlement of tailings.

Re-grading and contouring of the TSF to its post-mining configuration will begin at the end of the project life only after it has been determined that sufficient water has been removed from the tailings impoundment by the underdrain system. Because the Copper Flat TSF will be constructed with an engineered underdrain system, prior to conducting final grading activities NMCC will ensure that adequate drainage of the impoundment has occurred to ensure that large-scale settlement following grading is minimized.

Consolidation drainage into the underdrain system is anticipated to continue at declining rates for an indefinite period following the cessation of tailings discharge operations. Underdrain water collected will be pumped from the underdrain collection pond and applied to the surface of the TSF using water sprays or other evaporation enhancement systems to facilitate evaporation. When underflow is reduced to an acceptably low rate, the underdrain pipes beneath the embankment will be sealed with grout and the underdrain collection pond will be decommissioned.



The embankment out-slopes of the TSF will be will be graded to establish erosion controls and control water surface drainage. The out-slope inter-bench slopes will be 3H:1V and the out-slope angle for the composite, including benches will be about 3.22H:1V. The maximum bench width will be 23 ft. and the maximum inter-bench slope length will be 100 ft. The top surface slope will be not less than 0.5 percent and the bench cross and bench longitudinal slopes will be 2 percent and 2 percent, respectively. Cross bench drainages will be trapezoidal and constructed to safely convey storm water off reclaimed slopes for a 100-year precipitation event that results in the peak discharge.

A 36-inch topdressing soil cover will be placed on the top surfaces of the tailings impoundment and embankment out-slopes unless NMCC can demonstrate a thinner cover will resist erosion, sustain vegetation and be equally protective of groundwater considering site-specific reclamation plans for the facility. The cover area will then be seeded. Riprap and other erosion control structures will be placed as necessary in the drainage channels.

### **Waste Rock Stockpiles**

The waste rock stockpiles will be re-graded and reclaimed to blend into the surrounding topography to the extent practicable. Waste rock will be managed based on NMCC's operations material characterization program and predictive geochemical modeling. Concurrent reclamation to the extent practicable, materials management and surface water control measures will be used to maximize runoff, reduce infiltration, and reduce contact with the air, thus minimizing the potential for acid generation. The top surfaces of the waste rock stockpiles will be designed and constructed to a minimum final grade of 1%. The potential for ponding on the final surface will be reduced by careful contouring of the surface.

During operations the waste rock stockpiles will be constructed to facilitate re-grading during reclamation such that inter-bench slope faces will be 3H:1V or flatter and shaped to enhance run-off and prevent infiltration and ponding. Inter-bench slopes lengths will be no longer than 200 ft. The composite overall slope, which includes the inter-bench slopes and benches, will be 3H:1V or flatter.

The waste rock stockpiles will be reclaimed at the end of the life of the main by re-grading re-contoured, as necessary and a the 36-inch soil cover will be placed over the stockpiles, unless NMCC can demonstrate a thinner cover will be resist erosion, sustain vegetation and be equally protective of groundwater considering site-specific reclamation plans for the facility. The cover area will be seeded thereafter as soon as practicable.

The top surfaces of the stockpiles will be graded to promote positive drainage to storm water conveyance channels. These channels, in conjunction with perimeter berms, and hydraulic structures, will be designed to control erosion on the top surfaces and out-slopes and safely convey storm-water off of the stockpile areas. Cross bench drainages will be trapezoidal and constructed to safely convey storm-water off reclaimed slopes for a 100-year precipitation event that results in the peak discharge. Longitudinal slopes for these





drainages will be 1 to 5 percent. Energy-dissipation structures will be constructed at channel outlets to reduce erosive velocities where necessary. Where possible, channels will be constructed to incorporate existing topography, grade controls and exposed inert bedrock, which will promote long-term integrity of the structures. The final designs will be adjusted for local conditions.

Temporary erosion control measures will be provided during the reclamation construction and early vegetation establishment periods. These control measures include mulch, straw bales, silt fences and minor corrective re-grading, as necessary.

### **Open Pit**

The documents cited herein provide the detailed information required in subsection **20.6.7.33.D.(1) and (2) NMAC** that the open pit is currently, and is expected to remain a hydrologic sink capturing groundwater flowing from all directions during operations and post-closure (JSAI 2011, JSAI 2012, JSAI 2013, JSAI 2014, JSAI 2014b, JSAI 2014c, JSAI 2015). These documents have been previously submitted to the NMED and are incorporated into the Discharge Plan application by reference.

The water level elevation of the pit lake once mine dewatering and operations have ceased is anticipated to reach an average steady-state condition of approximately 4895 ft. above MSL over the long-term. NMCC will conduct rapid filling of the mine pit with water provided from the off-site well field. This will provide a source of good quality water and provide a mechanism by which the mineralized rock walls of the pit will be more quickly submerged under water, thus limiting mineral oxidation. Rapid refill with approximately 2200 acre-feet of water will bring the pit water to a steady-state in about 6 months.

Stable pit walls will be left in place. Any unstable pit walls will be stabilized by blasting or using other safe methods to ensure stability of the walls. In areas that can be safely accessed, benches above the projected 4900 water level elevation will be graded, sufficient soil cover placed on the benches and seeded to promote vegetation and limit erosion. Roads will be ripped and water bars constructed to control surface water runoff. Disturbed areas around and adjacent to the pit will be graded, soil materials placed and seeded to promote vegetation. Access to the mine site will be limited with a locked gate access roads blocked as appropriate, with physical barriers to discourage trespass. The access ramp into the pit will be graded and other ramps constructed, as necessary to provide escape routes for wildlife. The pit area and high walls will be barricaded with physical barriers and fences and posted in accordance with state and federal requirements.

Reclamation of disturbed areas in the open pit surface drainage areas surrounding the mine pit will be performed to minimize infiltration and promote vegetative growth. This will create a store and release cover, minimize infiltration of storm water around the pit perimeter, and limit water/rock interaction in the upper pit walls.



Limited open pit haul road reclamation within the pit will be accomplished by removing loose materials, installing a storm water conveyance channel into the pit, placing soil as needed to promote vegetation, and seeding. Vehicle access will be limited as much as possible during the rapid fill process and ultimately restricted after rapid fill has been completed through the construction of berms and other such physical barriers.

### **Surface Water Management**

Section **20.6.7.11.J**, above, contains a detailed discussion of the surface water control systems that NMCC has designed and will construct at the Copper Flat mine facility to manage surface water. All of those same structures and best management practices will be maintained during the reclamation and closure phase of the project to ensure compliance with subsection **20.6.7.33.E NMAC**.

In brief, there are five surface watershed areas within the mine facility that will be managed; the open pit area, the two waste rock stockpile areas outside of the open pit surface drainage area, the plant site and the tailings storage facility area. In addition, the entire surface area of the mine facility footprint is managed through surface water diversion structures that were installed by previous operations at the site, i.e., Quintana. These existing diversion channels will remain in place during and after reclamation and closure to ensure the long-term integrity of the reclaimed mine site.

Each of the surface water management areas or watersheds noted above has an engineered water retention structure associated with it to provide capture of run-off. During operation of the mine, surface water run-off from within the open pit surface drainage area watershed area will be captured in the mine pit, except that surface water from proposed new WRSP-1, also in the open pit drainage area, will be captured in a storm water impoundment. Surface water run-off from the plant site area and the two waste rock stockpile watershed areas outside of the open pit surface drainage area will be captured in storm water impoundments. Water that falls directly onto the tailings storage impoundment will be captured within the impoundment. Run-off from out-slopes of the tailings dam will be captured in the underdrain collection pond.

All of these surface water management structures will remain in place and functioning during the reclamation and closure phase of the project until they are no longer needed. Each of the impacted storm water impoundments will be decommissioned, re-contoured and re-graded so as to no longer hold water as each of the coincident areas are reclaimed. The open pit surface drainage area watershed will be re-graded to ensure that run-off water is routed to the pit. Impacted Storm Water Impoundment B will be removed and the area regraded to ensure that surface drainage from developed watershed B, which is naturally part of the open surface drainage area, will report to the mine pit after reclamation.

The underdrain collection pond will be incorporated into the passive evaporation system of the TSF described in the Reclamation and Closure Plan (Appendix E) of the Updated MORP.



## Cover System

NMCC has identified the waste rock stockpiles and the tailings storage impoundment as the units at the Copper Flat mine that will require the cover system required by subsection **20.6.7.33.F NMAC**. As indicated above, the reclaimed tailings storage impoundment and the waste rock stockpiles will be covered with 36 inches of soil unless NMCC can demonstrate a thinner cover will resist erosion, sustain vegetation and be equally protective of groundwater considering site-specific reclamation plans for the facility. The source of this soil will be the stockpiled top-dressing or growth media material inventoried and stored during construction of the mine facilities. The cover system will be a store and release/evapotranspiration cover designed to provide erosion control, sustain vegetation and reduce infiltration of storm water through the underlying materials. Other areas of the site will also receive a cover of growth media materials varying in thickness, but on less than 6 inches, to promote vegetation growth and minimize the potential for erosion as part of the site reclamation process.

In order to properly place the cover materials it will first be necessary to re-grade and re-contour the areas that will receive the cover. Grading will be performed in order to achieve positive drainage, optimize constructability, provide efficient conveyance of water, limit slope length and gradient and minimize soil erosion. An aesthetic component of grading is also to blend disturbed areas, to the extent possible, with the natural topography.

Prior to cover placement, top surfaces will require minor grading to fill rills, enable the construction of surface water control features and ensure that the final grade is between 1 and 5 percent. More extensive grading will be required on the slopes to achieve the desired slope configuration, smooth the bed materials and accommodate surface water control features. Areas that have become compaction will be ripped or disked to a depth sufficient to ensure good contact between the soil cover materials and sub-soils.

Once facilities are re-graded to an acceptable configuration, cover materials will be hauled from growth media stockpiles and placed on the top surface and slopes using a variety of equipment including scrapers or haul trucks. Bulldozers and motor graders will be used to facilitate the cover placement. Cover material will be left in a somewhat “roughened” condition in order to provide small micro sites where seed and moisture can collect thereby lending itself to better re-vegetation establishment. Required cover material estimates are contained in Appendix E of the Updated MORP.

The demand for process water will lessen as production slows. As such, the quantity of process water in storage will far exceed the process water in circulation at the end of production. The water in circulation at the end of production will be essentially that water used to flush out and clean the process units. That water will be disposed of in the tailings impoundment. However, until all of the water has been placed into the tailings facility, the operational water management units, i.e., the impacted storm water impoundments (one at the plant site and two at the waste rock stockpile areas), the process water reservoir, the surge pond, and the underdrain collection pond will remain functional, providing water



storage capacity. At the end of production each of these units will retain differing amounts of process water in inventory.

The impacted storm water impoundments will likely have only limited amounts of water inventoried as they are designed to hold water for less than thirty days. Any water inventoried in these ponds will be pumped to the process water reservoir or directly to the tailings pond during their use in the reclamation phase of the project. Each impoundment will eventually be decommissioned as the area they service is reclaimed.

At the end of production the process water reservoir inventory will be limited as its purpose will no longer be to retain water recycled from the tailings impoundment for use in the process. All water in inventory in this impoundment will be quickly utilized in flushing operations at the plant. Fresh water from the well field will quickly replace recycle water as the source of water used for flushing and cleaning the processing plant. All of that water will be routed to tailings impoundment.

The surge pond, located near the cycle plant at the tailing storage facility, under normal conditions remains empty and only retains process water as a result of upset conditions during operations. In addition, the surge pond is designed such that it retains any water that it receives for less than thirty days before being transferred to the tailings impoundment. At the end of production the surge pond will likely not have any inventory of process water. To the extent it does, that water will be evacuated into the tailings impoundment.

The underdrain collection pond is an integral part of the process water recycling system as described earlier herein in Section **20.6.7.11.J**. It is designed to capture water that is drained from the underdrain system installed in the tailings impoundment and the dam as well as storm water run-off from the exterior face of the dam. At the end of production, the underdrain collection pond will contain process water and will continue to do so during all phases of reclamation and closure as it will continue to be an operating component of the closure water management system discussed below.

NMCC does not propose any significant modifications to any of the water management system constructed during operations in order to create a more efficient process water reduction. Each unit will simply continue to function as designed and constructed and will be decommissioned when it is no longer needed as part of the reclamation and closure phase.

### **Closure Water Management and Water Treatment Plan**

The Closure water management and treatment plan as required in subsection **20.6.7.33.H NMAC** for the Copper Flat mine has three fundamental aspects to it that make it somewhat unique for this site and for the State of New Mexico copper mining projects. First with respect to the water in the open pit, NMCC has demonstrated that the open pit is a hydrologic sink (see JSAI, 2011; JSAI, 2014). Second, closure water management as it pertains to the waste rock stockpiles and the plant site area involves the continued use of the impacted storm water impoundments to capture run-off from those areas while



reclamation actions are being undertaken and ensuring that water from those impoundments is removed from them in less than thirty days. The water may be utilized for dust suppression or simply may be transferred into the tailings storage impoundment.

Third, closure water management of the water inventoried in the tailing impoundment actually begins at design and approval of NMCC's proposed water recycling and underdrain system. A fundamental aspect of the water management plan for closure is design and implementation of an efficient water recycling program at the tailings storage impoundment during operations. A water recycling barge and underdrain system as described in detail in section **2.6.7.11.J** of this application and in Appendix A is proposed for the TSF that maximizes use of water during operations. Over 70% of the process water utilized in the mill operations will be provided by recycled water from the TSF. As such less water will have to be drained from the TSF before the TSF can be permanently reclaimed and closed.

At the end of production some water will remain in the TSF impoundment and embankment. At end of operation the TSF will enter a "drying out" or "draining" period. Section 6.5.2, of Appendix A indicates that the maximum down-drain flow rate at final buildout of the dam is anticipated to be approximately 448 gpm from the dam underdrain and 66 gpm from the impoundment underdrain. As such, the dam will drain more quickly than the impoundment and is, therefore, anticipated to undergo reclamation sooner than the impoundment surface. The underdrain systems will continue to operate after cessation of operations. An "active" underdrain water management program will commence thereafter, including pumping captured water from the underdrain collection pond back to the impoundment surface and using forced evaporation equipment to reduce the volume of water. The TSF embankment is expected to drain quickly, allowing its reclamation to begin within 3 to 5 years after ceasing operations. It is anticipated that some reclamation of the impoundment may begin within 3 to 5 years of ceasing operation as the impoundment continues to drain and dry, allowing construction equipment to be utilized to commence cover placement. The duration of continued operation of the "active" water management system will be driven by the volume of water that continues to drain from the impoundment. NMCC has planned for 5 to 10 years of operation of the active program followed by a longer period of "passive" drain-down water management. After decommissioning of the active program and full reclamation of the TSF, any water that may continue to drain from the TSF (at ever decreasing rates for an estimated 20 years) will be captured in an evaporation cell that will be constructed below the toe of the TSF. The underdrain collection pond will be incorporated into the cell. The details of its design, operation and reclamation, the TSF drain-down assumptions and calculations are discussed in Attachment E2 of the Updated MORP Reclamation and Closure Plan. Estimates for drain-down flow are based on industry experience and comparative drain-down curves from industry sources. The costs for this system will be included in the financial surety calculations.



## Impoundments

All of the impoundments at the Copper Flat mine will be closed in accordance with the requirements of subsection **20.6.7.33.I NMAC**. The three impacted storm water impoundments and the surge pond will all have a 60 mil HPDE liner, or equivalent. All of these impoundments will hold water for less than thirty days. Captured water will be transferred to the tailings storage impoundment for disposal. As such, the likelihood of leaks occurring at those locations is small. Sediments contained in the impoundments will also be disposed of in the tailings impoundment. The liners will be removed and disposed of in the tailings impoundment. The impoundment berms will be pushed into the impoundment using earth-moving equipment contoured and graded to create positive drainage. A 36-inch soil cover will be placed over them, depending on their location, and subsequently seeded.

The process water reservoir and the underdrain collection pond will be constructed with a double liner and leak collection system. Upon completion of the closure and reclamation activities at their respective locations any water remaining in these impoundments will be transferred to the tailings storage impoundment for disposal. Sediments will, similarly, be removed and transferred to the tailings storage impoundment for disposal. The liners will also be removed and disposed of in the tailings disposal facility.

Because these two impoundments will have been equipped with a leak collection system, it will be possible to determine based on the operating records of the facility to determine whether or not any leaks occurred during operation. If it is determined that a leak occurred during operations that was not remediated during operations, the materials below the liner will be characterized to determine if remediation actions are required. If so, those actions will be initiated in consultation with NMED to implement the appropriate actions.

The process water impoundment is located in an area of the plant site where NMCC anticipates placing a minimum 6-inch soil cover. The underdrain collection pond is part of the tailings storage facility the will receive a 36-inch soil cover.

## Pipelines, Tanks and Sumps

As discussed above, at the end of production the entire processing facility, including the attendant pipelines, tanks and sumps will be flushed with clean water and disposed of in the tailings storage facility in accordance with subsection **20.6.7.33.J NMAC**. Pipelines will be either be removed and disposed of or cleaned and buried in place, as appropriate. Sumps will either be removed for disposal or cleaned and broken up and buried in place, as appropriate. The Pipeline, sump and tank areas will be inspected at the time of decommissioning for evidence of spills not detected and remediated during operations. Corrective actions pursuant to **20.6.2.1203 NMAC** will be pursued, as appropriate. NMCC has no plans to use bedding material that has the potential to generate acid. Therefore, no such removal actions will be required at closure.



### **Crushing, Milling, Concentrating and Smelting**

At closure, all surface facilities, including crushing, milling, and concentrating areas, equipment, and buildings will be removed from the area in a manner in conformance with subsection **20.6.7.33.K NMAC**. All concrete foundations footings and slabs will be broken up and buried in place or alternatively, excavated and disposed of in the tailing storage impoundment or at some other agency authorized location. The crushing, milling and concentrating areas will be characterized to determine if there are any potential water contaminants caused by the processing Copper Flats processing activities that could result in an exceedance of applicable standard in groundwater. All fuel tanks and reagent storage facilities and their contents will be removed from the site and disposed of in accordance with applicable federal and state laws. The plant site area will then be graded and contoured for surface drainage control and covered with a minimum of 6 inches of soil cover to conform to the surrounding topography to the extent practicable and seeded.

### **Closure Monitoring and Maintenance**

During closure NMCC will continue to conduct its operational monitoring program as required by subsection **20.6.7.33.L NMAC**. The monitoring plan will be modified, as appropriate, to reflect the need to monitor conditions associated with closure. All such modifications will be made only after obtaining approval from NMED.

### **Exceptions to Design Criteria**

NMCC will seek approval from the NMED for any modifications it may propose to make to the closure design criteria. NMCC acknowledges the role and jurisdiction of the New Mexico State Engineer with respect to jurisdictional impoundments.



### **20.6.7.11.U FINANCIAL ASSURANCE**

*An application shall include a proposal for financial assurance for those portions of a copper mine facility to be reclaimed in accordance with a closure plan submitted pursuant to Subsection A of 20.6.7.18 NMAC, 20.6.7.33 NMAC, 20.6.7.34 NMAC and 20.6.7.35 NMAC.*

Financial assurance will be required by the BLM and State of New Mexico to guarantee the completion of Project reclamation. Following regulatory review and approval of the NM MMD Mine Operations Plan (MORP), the Discharge Plan (DP) and the Mine Plan of Operations (MPO) and the reclamation techniques presented therein, NMCC will prepare a detailed estimate of cost to fully reclaim the operations as required by the **19.10.12 NMAC and the CFR § 3809.552**.

The reclamation plan will be consistent with the requirements of **Subsection A of 20.6.7.18 NMAC, 20.6.7.33 NMAC, 20.6.7.34 NMAC and 20.6.7.35 NMAC** and will be administered by the New Mexico Energy, Minerals and Natural Resources Department, Mining and Minerals Division (MMD) and the New Mexico Environment Department, Mining Environmental Compliance Section.

The estimated costs are based on projected third-party costs to reclaim the site assuming that all reclamation will occur following operations without the benefit of contemporaneous reclamation. Prior to commencement of operations, financial assurance in the amount of the estimated costs to reclaim the disturbance which would occur during the first five years of operations will be posted with the MMD.

The Nevada Standardized Reclamation Cost Estimator (SRCE) software that was developed in a cooperative effort between the Nevada Division of Environmental Protection, Bureau of Mining Regulation and Reclamation, the U.S. Department of Interior, Bureau of Land Management (BLM) and the Nevada Mining Association. This approach facilitates accuracy, completeness and consistency in the calculation of costs for mine site reclamation.

The estimate utilized current, i.e., 2013, feasibility economic model labor rates and equipment rates except where specific equipment were required, in which case rental rates were utilized and mobilization/demobilization costs were included.

Studies completed to date identified both the need for longer-term, post-closure water management for the tailings impoundment as well as the potential to develop pit lake chemistry that may not meet surface water limitations for wildlife. The cost estimate, therefore, incorporated costs for management of long-term tailings solution management, including an active and passive phase of management. In addition, estimated costs for pit water treatment have been included to account for closure liabilities that will exist until regulatory approval of the pit lake water at closure can be obtained and/or mitigation strategies are developed and closure alternatives can be identified. To allow closure cost calculation at this stage of the project, 30 years of active pit water management to arrive at





a regulatory acceptable solution was assumed. The 30-year timeframe for post-closure pit lake management was selected to represent a realistic time frame for demonstration that acceptable pit-lake water quality (i.e., that satisfies NMED post-closure chemistry requirements), could be passively achieved (i.e., after cessation of treatment). This would be achieved by evaluating periods of treatment followed by periods of non-treatment of the water column.

The un-inflated and un-discounted direct closure costs, excluding the contingency costs for the Copper Flat Project range from \$30.3 to \$42.5 million, including all physical reclamation activities, pit water and tailings solution management. With indirect costs for engineering and permitting, project and construction management, procurement and insurances, the estimated total reclamation and closure cost is anticipated to be as much as \$44.5 million. This amount is assumed to be spent over a 4-year period.



## **20.6.7.11.V VARIANCES**

*An application shall identify any issued or proposed variances for the copper mine facility pursuant to 20.6.2.1210 NMAC and the sections of the copper mine rule affected by the variance(s).*

NMCC does not propose to request variances pursuant to **20.6.1210 NMAC** for any of activities proposed at the Copper Flat project.



### 20.6.7.11.W METEOROLOGICAL DATA

An application shall include a plan to measure meteorological data at sites throughout the copper mine facility including precipitation, temperature, relative humidity, solar radiation, wind speed and wind direction.

NMCC established its on-site meteorological monitoring station and installed a 10-meter meteorological tower in August 2, 2010, with full data collection beginning September 1, 2010. NMCC submitted a Sampling and Analysis Plan (SAP) for the proposed Copper Flat Mine to the New Mexico Mining and Minerals Division (MMD) on September 3, 2010 (Intera, September 2010). The SAP was prepared to develop the BDR required by the MMD as part of the permitting process for the proposed mine. The SAP contained a description of the meteorological monitoring station installed by NMCC and outlined the proposed monitoring activities. The MMD approved NMCC’s SAP on January 11, 2011.

Subsequently, NMCC submitted the BDR to the MMD meteorological monitoring results including precipitation, temperature, relative humidity, solar radiation, wind speed and wind direction as required by this subsection. The SAP and BDR submitted by NMCC are incorporated into this Discharge Permit application by reference.

Table 11W-1 presents the meteorological data collected and instrumentation used. The location of the meteorological monitoring station is shown on Figure 11W-1. Its’ location was selected prior to development of the many of the subsequent site development plans. As such, NMCC anticipates that the tower may be moved in the future as its current location may conflict with the constructed site facilities.

TABLE 11W-1 Meteorological Data Collected				
Parameter	Tower Level (meters above ground surface)			Equipment Manufacturer and Model
	0	2	10	
Horizontal Wind Direction			X	Climatronics F460
Horizontal Wind Speed			X	Climatronics F460
Ambient Temperature		X	X	Climatronics 100093 Motor Aspirated
Temperature Lapse (2-10 m)		X	X	Climatronics 100093 Motor Aspirated
Pan Evaporation	X			NovaLynx
Relative Humidity		X		Climatronics 100098 Motor Aspirated
Net Radiation		X		Kipp and Zonen
Precipitation	X			Climatronix Tipping Bucket
Barometric Pressure		X		Setra

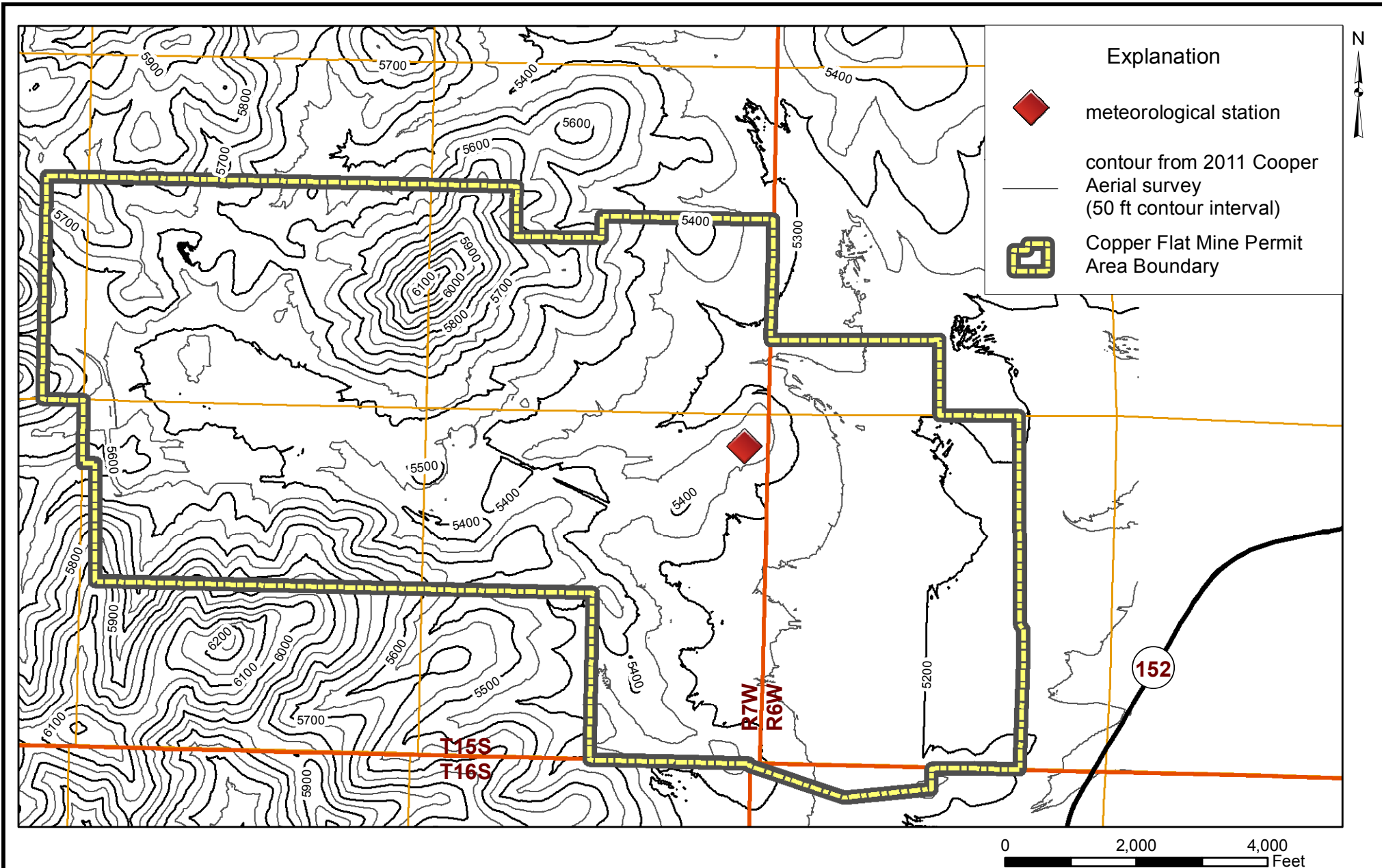


Figure 11W-1. Copper Flat Meteorological Station Location.



The meteorological monitoring program operated for one year to collect baseline climatological data representative of the Site. The meteorological data provided input to characterize the climatological factors listed in Table 11W-1 on a quarterly and annual basis. NMCC has continued to operate the meteorological station for some time beyond the one-year BDR collection period for the purpose of supplementing the long-term record. NMCC will reinstate operation of the station as part of its operational monitoring program for the facility.



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**Appendix A**

**Feasibility Level Design, 30,000 TPD Tailings Storage Facility  
And  
Tailings Distribution and Water Reclaim Systems**

**Copper Flat Project**

**Sierra County, New Mexico**

**Golder Associates Inc.,**

**Revised, June 2016  
Revised, November 2016**



Appendix A is contained in a separate volume binder as it is too voluminous to be included herein.

**Appendix B**

**Impoundment Design Report**

**M3 Engineering & Technology Corp.**

**November, 2015**

**Appendix C**

**Copper Flat**

**Process Facility Containment Report**

**M3 Engineering & Technology Corporation**

**December, 2015**

**Appendix D**

**Copper Flat**

**Site Diversion Analysis**

**M3 Engineering & Technology Corporation**

**Revised, June 2016**

Appendix D is contained in a separate volume binder as it is too voluminous to be included herein.

**Appendix E**

**Water Quality Monitoring Plan  
For the  
Copper Flat Mine Discharge Permit  
Pursuant to 20.6.7.11.R and 20.6.7.28**

**John Shomaker Associates, Inc.**

**Revised, June 2016**

**Appendix A**

**Feasibility Level Design, 30,000 TPD Tailings Storage Facility  
And  
Tailings Distribution and Water Reclaim Systems**

**Copper Flat Project**

**Sierra County, New Mexico**

**Golder Associates Inc.,**

**Revised, June 2016**

**Revised, November 2016**



# FEASIBILITY LEVEL DESIGN REPORT

## FEASIBILITY LEVEL DESIGN, 30,000 TPD TAILINGS STORAGE FACILITY

COPPER FLAT PROJECT  
SIERRA COUNTY, NEW MEXICO

**Submitted To:** New Mexico Copper Corporation  
THEMAC Resources Group Ltd.  
4253 Montgomery Blvd NE, Suite 130  
Albuquerque, NM 87110

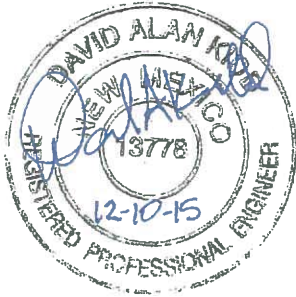
**Submitted By:** Golder Associates Inc.  
5200 Pasadena Ave. NE, Suite C  
Albuquerque, NM 87113

November 30, 2015





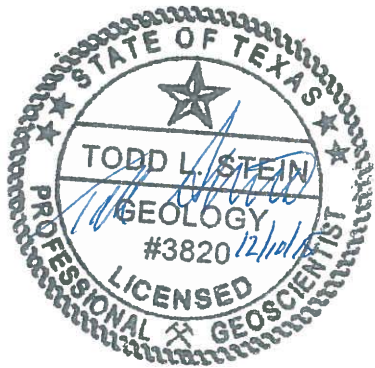
This report documents the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The design included herein was developed at a level consistent for agency review. Development of this report and associated TSF design was conducted under the oversight of the following Golder staff:



\_\_\_\_\_  
David A. Kidd, PE  
(Feasibility Design Drawings and Associated Engineering Calculations)

12-10-2015

Date



\_\_\_\_\_  
Todd Stein, PG  
(Preparation of Feasibility Level Design,  
30,000 TPD Tailings Storage Facility Report)

12-10-15

Date



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## List of Acronyms

1D	one dimensional
3D	three dimensional
ASTM	American Society of Testing and Materials
cm/sec	centimeters per second
F	Fahrenheit
FEMA	Federal Emergency Management Agency
FOS	factor of safety
ft-bgs	feet below the ground surface
ft-msl	feet above mean sea level
gpm	gallon per minute
HDPE	high density polyethylene
HSA	hollow stem auger
kg	kilograms
LCRS	leakage collection and recover system
MAG	Mineral Advisory Group
MDE	Maximum Design Earthquake
mm	millimeter
NMCC	New Mexico Copper Corporation
NMDSB	Dam Safety Bureau
NMED	New Mexico Environment Department
NOAA	National Oceanic and Atmospheric Administration
OSE	New Mexico Office of the State Engineer
PCPE	polyethylene pipe
pcf	pounds per cubic foot
PGA	peak ground acceleration
PI	plasticity index
PMP	probable maximum precipitation
PSD	particle size distribution curve
psi	pound per square inch
SPT	standard penetration test
TPD	tons per day
TPH	tons per hour
TPY	tons per year
TSF	tailing storage facility
USCS	Universal Soil Classification System
wt%	percent by total weight
yd <sup>3</sup>	cubic yard



## 1.0 INTRODUCTION

### 1.1 Scope

Golder Associates Inc. (Golder) has been contracted by the New Mexico Copper Corporation (NMCC) and its parent company THEMAC Resources Group Ltd (THEMAC), to complete the feasibility level design of the tailings storage facility (TSF) for the Copper Flat Project, located near Hillsboro, New Mexico in Sierra County. The TSF design presented herein has been completed in support of an overall Copper Flat project feasibility study as well as to support the various regulatory processes leading to permit approval of NMCC's project.

The TSF feasibility study report addresses geotechnical aspects of the project and presents the feasibility-level design of the TSF and tailings distribution and water reclaim systems. The individual components of the TSF feasibility study include: (1) the TSF design; (2) whole tailings delivery and distribution systems from the process plant and cyclone plant areas; (3) tailings delivery systems on the TSF; (4) underdrain collection system beneath the TSF; (5) TSF underdrain collection pond designs; (6) surge pond designs; (7) tailings reclaim water collection and delivery systems; and (8) systems for handling potential upset flow conditions (including secondary containment).

The location of the proposed facility is shown on Drawing 1 (Appendix J). Copper Flat is a proposed porphyry copper mining operation at a property that was briefly operated by Quintana Resources in 1981 and 1982. During the former mining operation, open pit pre-stripping was completed and a TSF with a design capacity of approximately 60 million tons was constructed and operated. Shortly after mining operations started, the mine was closed due to adverse economic conditions and depressed copper prices. In Drawing 2, which shows existing site conditions, the remains of the starter dam and splitter dike from the Quintana mining operation can be seen.

A new TSF will be constructed at Copper Flat in the same location as the former (old) Quintana Resources facility. The new TSF will extend approximately 1,000 feet to the east of the old starter dam (the tailings expansion area) as shown on Drawing 2.

A centerline construction method using cycloned tailings sand (cyclone underflow) for tailings dam construction will be utilized. A starter dam will be constructed using borrow material to provide initial storage capacity and to provide a location for initial discharge of tailings. The centerline approach allows construction of a stable, drained tailings dam using the cyclone underflow (i.e., tailings sand), while reducing the quantity of fill material required for dam construction. Tailings slimes (cyclone overflow) will be discharged into the interior of the TSF impoundment. The use of sand tailings for dam construction are such that the cyclone plant will be operated continually to produce the construction material.



The new TSF design will comply with the design and dam-safety guidelines and regulations of the New Mexico Office of the State Engineer (OSE) Dam Safety Bureau (NMDSB, 2010). Stormwater that cannot be diverted will be accommodated inside the impoundment by maintaining a dam crest elevation that provides adequate freeboard for containment of direct precipitation and run on.

The Mining and Environmental Compliance Section of the Ground Water Quality Bureau of the New Mexico Environment Department (NMED) will be the permitting authority for the groundwater discharge permit. NMED has provided guidance on anticipated design requirements for the TSF's liner system, which have been incorporated in this feasibility level design. Golder has also provided the feasibility level design of the tailings distribution and water reclaim systems.

Design drawings (Appendix J) to be read in conjunction with this report include the following:

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## 1.2 Terminology/Definitions

In this report, the TSF is described as consisting of the impoundment and the tailings embankment or dam. The impoundment refers to the interior tailings storage area located upstream of the embankment or dam. Tailings with different gradations will be placed inside the impoundment and on the dam.





Whole tailings refer to the process tailings delivered to the cyclone plant from the flotation plant outlet. At the cyclone plant, the tailings will be separated into cyclone underflow and cyclone overflow. The cyclone underflow, which consists primarily of the coarse sand fraction of the tailings, will be used to construct the dam. The cyclone overflow, which consists primarily of the fine fraction of the tailings, will be placed in the impoundment. Each of these tailings products will be transported and discharged as a slurry, with varying concentrations of tailings solids suspended in process water.

The cyclone overflow discharged into the impoundment will form a surface that gently slopes away from the point of discharge. The beach refers to the area near the point of discharge where the coarsest particles in the cyclone overflow will tend to settle. The slimes refer to the finer fraction of the cyclone overflow, which will flow down the beach with the majority of the process water to the distal portion of the impoundment surface and settle in the vicinity of the free water pond.

The TSF will have two separate underdrain systems. The dam underdrain will underlie the dam and overlie the dam liner and collect drainage from the cyclone underflow. The impoundment underdrain will overlie the impoundment liner and will collect drainage from the tailings beach and slimes (cyclone overflow).



## 2.0 SITE DESCRIPTION

### 2.1 Topography

Existing surface conditions at Copper Flat are shown on Drawing 2. The starter dam, splitter dike, and approximately 1.2 million tons of tailings from the Quintana Resources operation remain on the property.

The TSF site consists of a broad, shallow basin located at the head of a natural drainage that discharges to Grayback Arroyo. Elevation ranges from 5,170 to 5,435 feet above mean sea level (ft-msl) within the proposed TSF footprint. Topography is gently sloping over most of the site with the steepest slopes located around the west and southwest periphery of the facility. Because the site is near the head of a natural basin, requirements to divert water from upstream catchment areas will be minimal. Surface water diversion is discussed in Section 6.6.

### 2.2 Climate

The property is located within an arid, high desert area in the Basin and Range physiographic province subject to hot summers and relatively mild winters. Maximum summer temperatures can exceed 100 degrees Fahrenheit (F) while the average maximum daily temperature during winter months is approximately 40 degrees F. Average annual rainfall is approximately 13 inches and the property receives snow periodically.

Most rainfall occurs in July through September and is associated with high intensity, short duration, convective storms and moisture from the Gulf of Mexico. Winter precipitation is associated with west to east moving Pacific frontal storms. These storms typically produce less intense precipitation over a longer duration.

Based on the National Oceanic Atmospheric Administration Atlas 14 (NOAA, 2006), the 100-year, 24-hour storm event is estimated to be approximately 3.73 inches. Hydrometeorological Report 55A (US Department of Commerce, 1988), which provides probable maximum precipitation (PMP) estimates for areas located between the continental divide and the 103<sup>rd</sup> meridian, indicates a 72-hour PMP depth of 26 inches.

### 2.3 TSF Area Subsurface Conditions

#### 2.3.1 Geology

The proposed TSF site is located in Sierra County, within the southern Basin and Range physiographic province (Parsons, 1995). The Basin and Range province is described as a broad, highly extended terrain that extends from Canada, through the western United States, and across much of Mexico (Parsons, 1995). The name is derived from the type of extensional block-faulting that left the characteristic pattern of alternating basins and ranges across the province.



The site lies near the eastern edge of the Black Range, on the Piedmont slopes of the Palomas Basin. The Black Range is a Late Cretaceous to early Tertiary (Laramide) volcanic-plutonic arc (Oniell et al, 2002). The Basin and Range province in this region is cut by the Rio Grande Rift zone. The property sits in the westernmost basin of the Rio Grande rift zone, which is made up of three parallel north-trending basins separated by intra-rift horsts (Chapin, 1971).

The Palomas Valley is a relatively narrow flood plain flanked on the east by the piedmont slopes of the Caballo Mountains and on the west by the long gentle slopes ascending to the base of the Hillsboro-Animas Hills (Hawley, 1965).

Detrital material derived from the erosion of the adjacent slopes fills the Rio Grande valley to a depth of up to 9,000 feet (Harley, 1934). Much of this material is gravel, which consists of boulders and pebbles of quartzite, limestone, granite, rhyolite, andesite, and basalt, derived from the surrounding rock complex. They are coarser along the valley sides, grading to finer material toward the middle of the basins. In the higher regions, as in the sides of the valley between Fairview and Hillsboro and at Hermosa and elsewhere, alluvial deposits of Quaternary age are comprised of boulders and pebbles in a finer matrix of the same material, loosely cemented into a firm hard conglomerate known as Palomas gravel (Harley, 1934). Younger sand and gravel deposits, which form some of the intermediate terraces, represent a filling of channels eroded in the older deposits of Santa Fe and Palomas age (Harley, 1934).

The principal fan in the district is that formed by the drainage toward the east out of Copper Flat, principally through Grayback Arroyo, but in part through Dutch Gulch. The basal part of the original fan is composed of fine to coarse fragments of rhyolite derived from the late flows that once covered the Animas Hills. The intermediate part of the fan is composed principally of andesite and latite fragments. The topmost portion of the fan is composed of basalt, andesite, and latite fragments (Harley, 1934).

The Animas Hills consists predominantly of andesite flows and breccias of Late Cretaceous age with minor interbeds of sandstone. The andesites and sandstone are intruded by a stock of quartz monzonite of Late Cretaceous age centered at Copper Flat, and by quartz latite dikes radial to the stock (Seegerstrom et al, 1975).

Upper Cenozoic basin fill in the Rio Grande rift is generally referred to as the Santa Fe Formation or Group. The Santa Fe Group (middle Pleistocene to uppermost Miocene), is comprised of multiple formations including the Camp Rice, Fort Hancock, Palomas, Sierra Ladrones, Arroyo Ojito, Ancho, Puye, and Alamosa Formations.

The Santa Fe consists of basal conglomerate and interbedded sand and clay beds. The cobbles and boulders are mainly andesite. Thickness is variable but generally in tens of feet in the subject area. Two facies in the study area include: 1) the piedmont facies consisting of brown, poorly sorted, weakly



stratified conglomerates and fan gravels and brown sandy silt, conglomerate and clay; and 2) the axial-river facies of cross-stratified sandstone, sand, pebble conglomerate, gravel, and clay lenses. The piedmont facies comprises the major portion of the Palomas Formation. It rests in angular unconformity on bedrock of varying lithologies along the major uplifts bordering the basins. However, in many central basin areas the piedmont facies appears to grade downward into coarse-grained deposits of the lower Santa Fe Group (Lozinsky, 1986).

### **2.3.2 Site Observations**

The site geotechnical investigation program details are shown on Drawing 3. Drawings 4 through 9 contain TSF area geologic cross-sections developed on the basis of the recent drill holes and test pits, supplemented with subsurface information reported by Sergeant, Hauskins, and Beckwith (SHB, 1980).

Drill hole logs indicate that the foundation in the tailings area consists primarily of alluvial deposits that includes silt, sand and gravel underlain by clay. In the northwestern waste rock stockpile area, borings indicated the presence of gravelly silts and sands overlying conglomerate consisting primarily of andesite. The conglomerate is underlain by unweathered andesite.

Silts, sands, and gravels that occur in the proposed TSF area have been identified as piedmont alluvium and the older deposits of the Santa Fe Group (SHB, 1980) on which the piedmont alluvium was deposited. The Santa Fe Group is reported to consist of interfingering alluvial fan (gravel) and clay facies. Basalt flows are reported to occur in channels and arroyos cut into the piedmont and Santa Fe sediments. Basalt outcrops have been identified in an arroyo in the center of the TSF and locally around the site.

Drilling logs and geologic cross-sections indicate a high degree of variability in near surface materials both vertically and laterally, within the impoundment area and beneath the proposed dam. Silty/clayey horizons alternating with gravelly sand layers could potentially represent either the interfingering of the Santa Fe Group facies, or the more recent effects of local erosion and deposition.

In general, the interior of the impoundment is underlain by silty, clayey and gravelly sand, and cemented gravelly sand with a near surface layer of silty, wind-blown material. Eastward, toward the future dam site, interbedded clays and silts occur at depths typically greater than 20 feet; however, the composition of the foundation remains highly variable with interfingering, silty, sandy and clayey gravel units.

Groundwater is typically encountered at depths greater than 50 feet below ground surface (ft-bgs) in the vicinity of the TSF. A small zone of perched water has been identified in the vicinity of the old Quintana dam; however, recent drill holes completed in the perched water area to a depth of 50 ft-bgs did not intercept water. Groundwater and local perched water are not anticipated to impact the design and operation of the TSF.



## 3.0 SITE INVESTIGATIONS

### 3.1 Previous Site Studies

Portions of the TSF site were investigated by Sergeant, Hauskins, and Beckwith (SHB, 1980) prior to construction of the Quintana Resources TSF. The SHB investigation focused on the dam alignment for a 60 million ton facility, which does not coincide with the alignment currently proposed for the over 1 million ton facility. SHB coverage of the potential borrow areas in the impoundment interior was relatively extensive.

### 3.2 Site Geotechnical Investigation Program Description

Golder conducted a site geotechnical investigation between December 2012 and January 2013 to expand the coverage of the former SHB site investigation to include the new dam alignment. The field investigation consisted of 31 test pit excavations and 28 drill holes. Drawing 3 illustrates test pit and drill hole locations, and the location of geologic cross-sections developed to show the geology of the dam foundation.

Test pits were excavated with a Case CX210B or Terex 7606 hydraulic backhoe to depths up to 20 feet. Test pits were logged and photographed. Soil samples collected from test pits included bulk 5-gallon bucket and bag grab samples. Bulk samples are suitable for geotechnical soil classification, strength, consolidation, compaction and permeability testing. Bag samples are suitable for soil classification and moisture content testing. Test pit logs are contained in Appendix A.1.

Most of the drilling was completed with a track mounted CME 75 drill rig using hollow stem augers (HSA). The rig was equipped for conversion to diamond core drilling if bedrock was encountered. The drill hole target depth was 50 ft-bgs. Due to the presence of cemented gravels that were not amenable to drilling by either HSA or diamond core methods, down-hole percussion (Tubex) equipment was also used.

Standard penetration testing (SPT) was carried out at 5-foot intervals during HSA drilling. When percussion drilling was required, SPT test frequency was reduced to about 10 feet. Samples collected during drilling included bulk 5-gallon bucket cuttings samples from auger drilling, bagged Tubex rig cuttings samples, and bagged samples recovered from the SPT split-spoon sampler. Drill hole logs are contained in Appendix A.2.

### 3.3 Geotechnical Testing

Bulk and bag samples collected during site exploration were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for soil classification, compaction, permeability, consolidation, and strength testing. Laboratory reports for the various tests are contained in Appendix A.3. Soils were classified according to the Universal Soil Classification System (USCS). All geotechnical tests were completed in



accordance with applicable American Society of Testing and Materials (ASTM) standards. No undisturbed samples were collected during the site exploration. Geotechnical strength, consolidation, and permeability tests were conducted on remolded samples.

### **3.3.1 Soil Classification**

#### **3.3.1.1 Tailings Impoundment Interior**

Within the interior, central and western portions of the impoundment area, site soils consist predominantly of clayey sand with gravel (SC), well-graded silty sand with gravel (SW-SM) with lesser clayey and well-graded silty gravel (GC, GW-GM). Silty and clayey soils (CL-ML) also occur locally. The fine fraction (finer than 75 microns or the material passing the No. 200 standard sieve) in sandy samples from the impoundment interior ranges from 8 to 29 percent and averages 20 percent. Plasticity indices (PI) average 17 percent. The specific gravity of interior area soils ranged from 2.67 to 2.75.

Four composite samples of near surface sandy, gravelly soils were prepared to evaluate materials potentially available for the construction of processed (crushed and screened) drainage material. Each composite sample was composed of two to three 5-gallon bucket samples of near surface materials. Composite samples were initially crushed to 100 percent finer than 1 inch to simulate material that would be suitable for placement against a geomembrane liner. Three of the four composites were classified as clayey sand (SC), while the other was classified as well graded silty gravel (GW-GM). The fine fraction in the crushed composite samples ranged from 8 to 20 percent and the average PI was 17 percent. It is anticipated that screening will be required to reduce the fines content of this material if it is to be used for highly permeable, manufactured drain fill adjacent to critical drainage pipes.

#### **3.3.1.2 North Cell Tailings**

The north cell of the old Quintana TSF contains tailings from mining conducted in the early 1980s. Old tailings samples were classified as silty sand (SM) and low plasticity silt (ML). The sample classified as sand was located adjacent to the old dam and presumably, near the point of discharge. It contained a minus 75-micron fine fraction of 49 percent and was non plastic. The silty sample was obtained from the center of the north cell and had a minus 75-micron fraction of 71 percent and a PI of 5 percent. Differences in old tailings properties are presumed to be the result of tailings segregation on the former tailings beach. Moisture content in the old tailings samples ranged from 6.0 to 11.3 percent.

#### **3.3.1.3 Tailings Dam Footprint**

Soils encountered in the footprint of the proposed dam are highly variable. Clayey sand and gravel (SC,GC) generally occur at shallow depth with interbedded high and low plasticity clays (CH,CL) and silts (ML,MH) occurring at depths below 20 ft-bgs. Clay intercepts indicate that the clay occurs in



discontinuous lenses or in eastward dipping strata. The fine fraction in the sandy and gravelly soils ranges from 17 to 39 percent and the PI averages 17 percent.

### 3.3.2 Foundation Strength Testing

Foundation samples were subjected to consolidated undrained triaxial testing with pore pressure measurement to determine effective shear strength for use in supporting stability analyses. Confining pressures were selected to represent anticipated foundation pressures associated with tailings embankment construction. Granular foundation materials are assumed to be cohesionless. Strength data reported below represent the effective internal friction angle. Triaxial test reports are contained in Appendix A.3.

It is anticipated that the old tailings will be placed beneath the new TSF geomembrane liner as a liner bedding fill layer. A 12-inch by 12-inch direct shear test was conducted to evaluate the interface friction at the TSF liner interface. The direct shear set-up included a layer of compacted old tailings from Test Pit 10 in the center of the north cell, a textured 80-mil high density polyethylene (HDPE) geomembrane, and a layer of material from the drainage composite samples 1 through 4. Tailings underlying the liner were placed at optimum moisture content based on ASTM D698 compaction testing and maintained in an unsaturated state during shearing. The over-liner drainage materials were wetted prior to testing but maintained in an unsaturated state during the test. The test was conducted in a manner that allows the failure through the lower strength interface (i.e., liner against old tailings or liner against drain fill). Direct shear test results are contained in Appendix A.3.

Table 1 summarizes the strength test samples, test objectives and strength test results.

**Table 1: Summary of Strength Test Results**

Sample	Material, Objective, Test Method	Internal Friction Angle
TP-10-3-13	Old tailings, strength of liner bedding triaxial CU <sup>(1)</sup>	34 degrees
TP-10-3-13	Old tailings, liner interface shear strength, direct shear <sup>(2)</sup>	26.5 degrees
Composite 1-4	TSF Interior borrow for structural fill and drain material, triaxial CU	40 degrees
BH-16-0-8.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	28 degrees <sup>(3)</sup>
BH-25-0-12.5	Clayey Gravel, structural fill borrow and embankment foundation strength, triaxial CU	43.5 degrees
BH-10-0-14.5	Clayey sand, structural fill and embankment foundation strength, triaxial CU	32 degrees

**Notes:**

<sup>(1)</sup> CU = consolidated, undrained

<sup>(2)</sup> Liner interface testing included old tailings and drain material in contact with textured, 80-mil HDPE geomembrane.

<sup>(3)</sup> Sample was 91 percent finer than ¾ inch. Represents a matrix shear strength and does not account for interlocking of the coarse fraction.

No undisturbed samples were collected during site exploration. Tests were conducted on remolded samples.



### **3.3.3 Drainage Material Permeability Testing**

Drainage material permeability testing was performed to support the design of the impoundment and dam underdrains. Testing was completed on composite samples 1 through 4, which represent the near surface materials that will be available for preparation of drainage material. Equal volumes of composite samples 1 through 4 were blended to prepare a sample for permeability testing. Samples were tested in a 10-inch diameter rigid wall cell with a 150-pound-per-square-inch (psi) load applied to simulate loading under field conditions. Permeability test reports are contained in Appendix A.3.

Two drainage materials are considered in the TSF design. These include crushed and screened materials with reduced fines content for the dam underdrain and primary drains in the impoundment interior (primary drain fill), and material obtained from selective borrowing of sandy soils within the TSF footprint (select native drain fill). Processing of the select native drain fill is anticipated to be limited to crushing to reduce the maximum particle size. No washing to reduce the fines content is anticipated to be required to produce suitable drainage materials.

#### **3.3.3.1 Primary Drain Fill**

A primary drain fill sample was prepared for testing the performance of the dam underdrain. The minus No. 40 standard sieve fraction of the sample was removed to simulate a prepared drainage material with low fines content that is filter compatible with the cyclone underflow sand. The hydraulic conductivity of the primary drain fill sample was  $9.1 \times 10^{-2}$  centimeters per second (cm/sec).

#### **3.3.3.2 Select Native Drain Fill**

The composite sample was tested without additional modification to estimate the permeability of the sandy site soils. As tested, the minus 75-micron fraction in the composite samples was 9 percent. The hydraulic conductivity of the select native drain fill sample was  $3.8 \times 10^{-5}$  cm/sec.

### **3.3.4 Foundation Sample Consolidation Testing**

Selected foundation samples were subjected to conventional one-dimensional consolidation testing to support estimation of settlement potential in the proposed dam foundation. Samples were selected to evaluate silty and clayey horizons where changes in loading conditions could result in additional consolidation. Samples were remolded to dry densities ranging from 88 to 95 pounds per cubic foot (pcf) at natural moisture content to reflect in-situ density estimated from standard penetration tests. Foundation sample consolidation test reports are contained in Appendix A.3. Foundation settlement potential is discussed in Section 11.0.





## 4.0 TAILINGS TESTING

### 4.1 Program Description

The TSF embankment proposed in this feasibility study is to be constructed by the centerline raise method using cyclone underflow. Whole tailings from the flotation plant will be routed to a cyclone plant where the tailings will be separated into underflow (sand) and overflow (slimes) fractions. The cyclone underflow will be routed to the dam centerline and used to construct the tailings dam. The overflow will be discharged into the impoundment interior.

Primary considerations for effective centerline sand dam construction include adequate drainage and compaction of the cyclone underflow sand. Drainage requirements are typically met by:

- Producing a relatively free draining sand material. This is usually achieved when the minus 75-micron fraction of the underflow does not exceed 20 percent by weight.
- Having the hydraulic conductivity of the material placed in the dam two orders of magnitude greater than the material placed in the impoundment.

These two conditions generally result in a well-drained structure. A fixed cyclone station maintains optimum and consistent conditions at all cyclones and facilitates meeting gradation objectives. A blanket drain will be placed beneath the embankment to facilitate collection of cyclone sand drainage and minimize saturated conditions at the base of the embankment.

Industry experience at operating mines utilizing cyclone sand for dam construction indicates that hydraulic placement and self-weight compaction is generally sufficient to minimize liquefaction potential in cyclone sand dams located in regions with low seismic risk, such as at Copper Flat. Where it is required, compaction to a relative density of 60 percent (equivalent to approximately 90 percent of ASTM D698 maximum dry density) will result in low potential for liquefaction under static and seismic loading conditions (CANMET, 1977). At Copper Flat, tailings placed on the dam crest will be compacted. Some compaction of tailings placed on the dam out-slope will occur as a result of dozer spreading operations.

An initial cyclone test was conducted on a 55-kilogram (kg) tailings sample to determine the quantity and quality of sand available for dam construction, and produce samples of future tailings products for geotechnical testing. The initial cyclone test sample was produced by Metcon Research in September 2011. The sample was run through a 4-inch cyclone at the FL Smidth-Krebs (Krebs) facility in Tucson, Arizona. On the basis of the initial 4-inch cyclone test, Krebs, utilizing proprietary software, predicted that a gMAX15U-20 (15-inch) cyclone could recover 46 percent of the whole tailings stream with a minus 75-micron fraction of less than 20 percent. The quantity and quality of the cyclone underflow predicted on the basis of the 4-inch cyclone test were consistent with industry guidelines for sand dam construction. The quantity of sand recovered met anticipated construction requirements.



An additional pilot scale metallurgical study was conducted by the Mineral Advisory Group (MAG). Approximately 255 kg of tailings solids (whole tailings) were provided by MAG in five sealed 55-gallon steel drums. Drums were delivered to Krebs in July 2012 and the tailings were passed through a gMAX15U-20 cyclone in the Krebs laboratory on August 4, 2012. The sand recovery was 41 percent and the minus 75-micron fraction was under 16 percent. Cyclone underflow, and cyclone overflow produced during the test, and residual whole tailings were shipped to Golder's geotechnical laboratory in Lakewood, Colorado for testing.

Appendix B.1 contains Krebs analysis of the August 2012 cyclone run and the simulation (prediction) of full field scale cyclone performance. The cyclone plant is estimated to have a recovery of 45 percent with a minus 75-micron fraction of 18 percent.

## 4.2 Tailings Test Program Description

Table 2 contains a test matrix for the tailings products produced during the cyclone test. Gradation analyses were completed on the cyclone underflow, cyclone overflow and whole tailings. The cyclone overflow and whole tailings were subjected to flume testing to simulate discharge into the impoundment. Flume testing involves the discharge of tailings slurry at the field anticipated solids content into a 12-inch flume at low velocity. The tailings flow down and settle in the flume. Samples are collected at various flume locations to evaluate changes in gradation and solids content. Gradation and slurry consolidation tests were conducted on samples from the head and tail of the flume to evaluate the characteristics of tailings found on the beach and in the interior of the impoundment (slimes). Laboratory data sheets for tailings tests are contained in Appendix B.

**Table 2: Test Matrix for Cyclone Underflow, Cyclone Overflow and Whole Tailings**

Test Method	Underflow	Overflow	Whole Tailings
Gradation and Atterburg Limits (ASTM D4221, D4318)	1	2 <sup>(1)</sup>	2 <sup>(2)</sup>
Specific Gravity (ASTM D854)			1
Compaction Test (ASTM D-698)	1		
One-Dimensional Consolidation (ASTM D2435)	1		
Flume Test (flumes samples taken from head and tail of flume to evaluate segregation and settling) (specialty test)		1	1
Column Settling Test (single drain) (specialty test)		1	1
Slurry Consolidation Test (specialty test)		2 <sup>(1)</sup>	2 <sup>(2)</sup>
Permeability (ASTM D2434)	1		
Triaxial Shear Strength, Consolidated-Undrained (ASTM D2850)	1	1	

**Notes:**

- (1) Testing completed on head and tail section samples from the flume to simulate properties of the beach and slimes fraction of the cyclone overflow.
- (2) Testing completed on head and tail section samples from flume to simulate properties of the beach and slimes fraction of the whole tailings.



## 4.3 Test Results

### 4.3.1 Material Classification

Table 3 summarizes the characteristics of the tailings prior to and following cyclone separation. Gradation test results for the whole tailings cyclone feed, cyclone underflow and cyclone overflow are contained in Appendix B.2.

**Table 3: Summary of Tailings Properties, Pre- and Post-Cyclone Separation**

Material	P <sub>80</sub> (microns) <sup>1</sup>	Minus 75-micron Fraction (percent) <sup>2</sup>	USCS Classification
Whole Tailings (feed)	110	59	SM (silty sand)
Underflow (sand)	150	17.9	SM (silty sand)
Overflow (slimes)	5.7	90	ML (low plasticity silt)

**Notes:**

<sup>1</sup>P<sub>80</sub> is the particle size for which 80 percent of the material is finer

<sup>2</sup>The minus 75-micron fraction is the percentage of clay and silt sized particles

Based on the gradation test results, the whole tailings sample produced in the MAG metallurgical study is slightly coarser than the flotation tailings that will be produced during operations (P<sub>80</sub>=110 microns versus the design P<sub>80</sub> of 105 microns).

### 4.3.2 Cyclone Underflow Testing

Cyclone underflow sand will be delivered to the dam at a solids content of approximately 70 percent based on cyclone test results and cyclone performance predictions. Cyclone underflow sand discharged on the dam crest will be spread and compacted. Moisture density testing (ASTM D698) indicates a cyclone underflow maximum dry density of 97 pcf and an optimum moisture content of 16.8 percent. The moist weight of the compacted cyclone underflow will be on the order of 110 pcf. Underflow compaction test data are contained in Appendix B.2.

Cyclone underflow samples were subjected to one-dimensional consolidation and consolidated-undrained triaxial shear strength testing. A cyclone underflow sample compacted to within 5 percent of maximum dry density exhibited an effective internal friction angle of 40 degrees. Consolidation and triaxial test reports are contained in Appendix B.3.

### 4.3.3 Cyclone Overflow and Whole Tailings Testing

Flume tests were conducted on cyclone overflow and whole tailings in a 12-inch wide by 24-foot long test flume. The primary purpose of the flume tests is to provide data to support estimation of post deposition density. Samples were collected from the head and tail sections of the flume for gradation, settling and slurry consolidation testing. Test data were used to develop input parameters for one-dimension numerical consolidation modeling and impoundment filling rate studies. Flume test gradation, settling and



slurry consolidation test reports are contained in Appendix B.4. Tailings consolidation modeling is discussed in Section 5.0.

To support stability analyses, a sample of cyclone overflow collected from the head of the test flume (beach material) was subjected to consolidated-undrained triaxial shear strength testing. The sample was tested at a dry density of approximately 94 pcf and a moisture content of 27 percent. The measured effective internal friction angle was 37 degrees. The triaxial test report is contained in Appendix B.4.



## 5.0 TAILINGS CONSOLIDATION ANALYSES

### 5.1 Approach

Consolidation calculations were performed using the computer program FSConsol (GWP Software, 1999). FSConsol performs a one-dimensional (1D), large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967). For modeling purposes, the non-linear relationships used to express permeability and compressibility are those proposed by Abu-Hejleh and Znidarcic (1994 and 1996), and defined by Equations 5.1 and 5.2, which are used in the consolidation and desiccation numerical model.

$$k = C e^D \quad \text{Equation 5.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 5.2}$$

When using FSConsol model, Equation 5.1 remains the same; however, Equation 5.2 is rewritten by the modified power law form to represent compressibility, as shown in Equation 5.3.

$$e = A(\sigma')^B + M \quad \text{Equation 5.3}$$

Where:

e = void ratio of the tailings

$\sigma'$  = the effective confining stress

k = the hydraulic conductivity of the tailings

A, B, M (or Z), C, and D are material parameters determined from laboratory slurry consolidation and column settling tests

Five material parameters, *A*, *B*, *C*, *D*, and *M*, were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 and 2. Fitted parameters are shown in Tables 4 and 5. Data are based on slurry consolidation testing of cyclone overflow samples derived from the head and tail sections of the test flume discussed in Section 4.3.3.

**Table 4: Permeability Input Parameters for Cyclone Overflow**

Sample	C (centimeters per second)	D (dimensionless) <sup>1</sup>
Slimes	$1.380 \times 10^{-7}$	3.353
Overflow Beach	$1.523 \times 10^{-6}$	3.035

**Table 5: Compressibility Input Parameters for Cyclone Overflow**

Sample	A (1/kilopascals) <sup>B</sup>	B (dimensionless) <sup>1</sup>	M (dimensionless) <sup>1</sup>
Slimes	3.144	-0.1952	-0.1424
Overflow Beach	1.787	-0.2983	0.5224

**Note:**

<sup>1</sup> B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second units



The tailings impoundment was modeled by running analyses on single tailings columns or volumes filled with materials representing either the beach or slimes components of cyclone overflow. Under the proposed mining plan, the operator will be required to maximize use of the cyclone plant to generate sufficient sand to construct the dam. Therefore, whole tailings discharge will have little impact on the filling rate of the TSF or the characteristics of tailings placed in the TSF.

In each model run, the modeled component is assumed to represent 100 percent of the inflow to the model storage volume. Models were run until the modeled storage volume reached capacity, or the entire mass of tailings was input. The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities (adjusted for errors as discussed in Section 5.3) from the final void ratio profile of each tailings component to obtain an overall dry density that is weighted based on laboratory grain size distribution mass balance results, as discussed in Section 5.2.

## 5.2 Tailings Beach versus Tailings Slimes Split

Grain size distribution tests were performed by the Golder soils laboratory in the Denver, Colorado, on beach and slimes samples from laboratory flume tests and on a cyclone overflow head sample discharged into the test flume. From the grain size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the beach and slimes that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the beach and slimes samples should equate to the percentage of sand and fines in the original cyclone overflow head sample. Table 6 depicts the results of the laboratory gradation tests and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes) by weight or mass.

**Table 6: Laboratory Grain Size Distribution and Beach to Slimes Split Calculation**

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids Recovered	% Sand recovered from overflow	% Fines recovered from overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head Sample	9.63	90.37	100	9.63	90.37

## 5.3 Consolidation Modeling Results

FSConsol model output and a detailed description of modeling procedures and interpretation are contained in Appendix C. If 100 percent of the inflow into the TSF impoundment is represented by the tail section flume sample (i.e., the finest fraction of the tailings), FSConsol predicts that the final dry density of the slimes fraction over the modeled profile will average approximately 33.5 pcf. With 100 percent of the



inflow representative of tailings overflow beach materials, FSConsol predicts a final average tailings dry density of 78.6 pcf

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall bowl shaped geometry of the TSF, and the fact that the mass and volume quantities are larger towards the top than at the bottom of the TSF. The average dry density of each tailings component is corrected based on the average of the calculated percent error between 1D checks of the height of solids, the volume of solids, and the mass of solids. Based on the TSF geometry, calculated material properties, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1 to 14 percent with an average error ranging between 2.7 and 5.4 percent. Based on Golder's experience, 1D analyses resulting in less than a 15 percent average error reasonably depict expected consolidation in the field; therefore, the three-dimensional (3D) impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error calculated for the following checks: height of solids, volume of solids, and mass of solids.

In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components. After accounting for 3D characteristics of the impoundment geometry and errors, the predicted dry density of the beach and slimes components are 74.6 and 31.7 pcf, respectively.

#### 5.4 TSF Capacity

Based on model results and corrections based on error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow slimes and beach at the end of filling. The capacity of the TSF interior is 96.9 million cubic yards (yd<sup>3</sup>) with a crest elevation of 5,460 ft-msl and a maximum tailings surface elevation of 5,450 ft-msl. While the beach and slimes components are predicted to represent 60 and 40 percent, respectively, of the mass of cyclone overflow to be discharged into the TSF interior based on the mass balance as discussed in Section 5.2, these components will represent approximately 40 and 60 percent, respectively, of the storage volume. At the estimated densities predicted with FSConsol, the required storage capacity is 93.4 million yd<sup>3</sup>. This assumes near constant operation of the cyclone plant. With approximately 96.5 percent utilization of the cyclone plant, available storage capacity will equal required storage capacity.

Without consideration of managed deposition effects, there is a small excess in available storage volume. The estimated weighted dry density of tailings within the TSF impoundment is 48.4 pcf, which is low in comparison to copper industry experience. Managed deposition such as cycling discharge locations to promote desiccation and controlling the size of the free water pond can be expected to increase the post deposition dry density and reduce storage volume requirements. In addition, the FSConsol modeling runs for slimes inflow were run for a time period of 7.9 years. The slimes can reasonably be expected to



continue to consolidate and increase in dry density through the remainder of the 11.1-year mine life. Only a slight increase in the dry density of the slimes over the remainder of the mine life will result in an increase in storage capacity.

The inflow rate for cyclone overflow assumes near constant operation of the cyclone plant to produce sufficient sand to construct the dam to the elevation of 5,460 ft-msl. As such, the mill should not be operated unless the cyclone plant is operating, and maintenance of the cyclone plant should be performed concurrently with mill maintenance. The cyclone overflow distribution system has two operating legs that will facilitate near constant operation. During operations, filling rates and tailings post deposition dry density should be regularly monitored to evaluate consolidation characteristics. If the rate of consolidation is better than predicted, utilization of the cyclone plant could potentially be reduced, the final dam crest could be lowered, and a corresponding reduction in the amount of sand required to construct the dam could be realized.





## 6.0 FEASIBILITY LEVEL DESIGN

Table 6 summarizes key design criteria assumed in feasibility level design of the new TSF.

**Table 6: Feasibility Study Design Criteria**

Regional Design Factors	
Precipitation/ Evaporation	Based on NOAA weather data for Hillsboro and Caballo Dam, New Mexico
Design Storm Events	100 percent of the 72-hour general storm probable maximum precipitation (PMP), 26 inches
Stability FOS	Minimum 1.5 for static conditions and 1.1 for seismic loading conditions
Seismicity PGA	USGS MDE, 2475-year return period, 0.13 times gravitational acceleration (0.13g)
TSF Design Factors	
Storage Capacity	112 million tons (THEMAC)
Production/Delivery Schedule	1,333 tons per hour (TPH) net tailings to the TSF year 1-5, 1,222 TPH years 6 to 11.1, 125,000 tons per year (TPY) (THEMAC), post concentrate recovery
Mill utilization	92.5 percent (M3)
Operating Life	11.1 years (THEMAC)
Tailings Specific Gravity	2.64 (Golder test)
Tailings Solids Content (wt%)	29.2 percent solids by weight (whole tailings to cyclone plant). Tailings diluted in outlet sump as needed to optimize cyclone performance.
Production Rate	Varies, Net tailings to the TSF from 9,182 to 10,704 kiltons per year (25,156 to 29,326 tons per day) (NMCC)
Tailings post- deposition dry density	31.7 and 74.6 pounds per cubic foot (pcf) dry weight assumed for post-deposition cyclone overflow slimes and beach, respectively, 92 pcf dry weight for the cyclone underflow fraction. (Golder estimate)
Embankment Construction	Phase 1 earthen starter dam to an elevation of 5,250 ft-msl. Post Phase 1 peripheral earthen dam extension constructed to 10 feet above grade. Centerline raise construction using cyclone underflow sand. Cyclone underflow on dam crest compacted to minimum of 90 percent of American Society for Testing and Materials (ASTM) D698, relative density > 60 percent
Liner System	From bottom to top: Prepared foundation, 12-inch liner bedding fill, 80-mil HDPE geomembrane, overliner drainage collection layer with internal drainage pipe network beneath the tailings embankment and continuous beneath impoundment



<b>TSF Design Factors (cont.)</b>	
Earthworks Slopes (assumed)	Soil cut slopes = 1.5H:1V (1.5 horizontal to 1 vertical) Rock cut slopes = 1H:1V Fill slopes = 2H:1V Lined slopes = 3H:1V to 2.5H:1V max Embankment out-slope = 3H:1V nominal Starter Dam, 2.5H:1V inner, 2H:1V outer
Drainage/TSF Underdrain Collection Pond	Double-lined pond with LCRS to contain dam and impoundment under drainage and surface water runoff. Pond to be constructed as an OSE non-jurisdictional facility.
Collection Pond Reclaim	Submersible turbine pumps with 4,000-gallon per minute (gpm) capacity
Collection Pond Capacity	Normal inventory, 24 hours reserve capacity for underdrain for reclaim pump system upset, 100-year, 24-hour event (3.73 inches) stormwater storage capacity for runoff contributing areas
Tailings Management	Tailings routed through eighteen 15-inch cyclones at 83 TPH feed rate per cyclone. 45.2 to 45.6 percent underflow solids recovery (Krebs), 18.2 to 18.4 minus 200 fraction in underflow (Krebs). Cyclone overflow discharged from the dam crest into the impoundment interior.
Supernatant Reclaim	Floating barge with 12,978-gpm capacity
<b>TSF Water Storage and Stormwater Diversion Design Factors</b>	
Dam Safety Hazard Ranking	Significant, due to environment risks associated with a release of tailings (OSE)
TSF Pond Design Freeboard	As required to accommodate wave run-up and provide minimum freeboard for design storm
TSF Pond Required Stormwater Storage	Contain flows from 1.0 times the 72-hour PMP storm event plus normal inventory of supernatant water
Hydrology Runoff Curve Numbers	100 - Impounded tailings and lined areas 50 - Tailings embankment sand shell 92 - Native ground surfaces
Stormwater Diversion	Divert runoff from undeveloped areas inside ultimate footprint where feasible. Divert exterior area runoff where feasible.
Underdrain System	Continuous underdrain layer beneath dam and TSF interior. Collected water will be returned to the process via TSF underdrain collection pond reclaim pump system
TSF Water Pond Surface Area	40 percent of tailings impoundment interior or a maximum of 40 acres assumed for feasibility level water balance calculations
TSF Water Pond Surface Evaporation	75 percent of average Pan evaporation
Tailings Surface Evaporation	50 percent of average Pan evaporation

**Notes:**

FOS = factor of safety

MDE = maximum design earthquake

PGA = peak ground acceleration

HDPE = High density polyethylene



The following sections describe the various construction components of the TSF feasibility level design.

## 6.1 Earthworks

TSF construction activities will require borrowed structural fill for starter dam and toe berm construction, drain fill for constructing underdrain layers, and liner bedding fill material. Due to permit boundary and land ownership conditions, the majority of the construction materials must be derived from within the TSF footprint. Meeting fill, reclamation topdressing salvage, and reclamation cover material requirements for construction will necessitate stockpiling selected materials early in the life of the TSF because the borrow sources for these materials will be buried as the TSF footprint expands. Initial Phase 1 grading and liner installation will cover approximately 60 percent of the ultimate TSF footprint and much of the area available for borrowing construction material and reclamation cover.

The Phase 1 liner bedding fill material will be derived from the existing tailings produced during the Quintana Resources operation. All existing tailings lie within the Phase 1 construction footprint. Liner bedding fill needed for Phase 2 through Phase 5 construction will be derived from soil borrow areas.

Drain fill material will be produced by crushing and screening native soils and gravels. Drain fill material will be placed in contact with geomembrane liners. To meet drainage and liner compatibility requirements, a minus 1-inch gradation is assumed. The fine rejects (undersized) from drain fill material production will be suitable for liner bedding fill. Because Phase 1 liner bedding fill requirements will be met by using the existing tailings, the undersized materials produced in Phase 1 will be stockpiled for use as liner bedding fill in Phases 2 and 3. Liner bedding fill stockpiled in Phase 1 will be supplemented with additional material produced during construction. Approximately 100,000 cy<sup>3</sup> of undersized material will be stockpiled in Phase 1.

### 6.1.1 Site Grading

The TSF grading plan for Phases 1 through 5 is illustrated on Drawing 10. The topographic surface shown on the grading plan reflects the over-excavation/removal of borrow materials required for cover material stockpiling and TSF construction. The approximate construction limits by phase are also indicated on Drawing 10. Drawing 11 illustrates the Phase 1 grading plan and Phase 1 construction. Site grading will include removal of the old starter dam and splitter dike for use as structural fill in the new starter dam and toe berm. Additional structural fill and drainage material will be borrowed from within the TSF footprint. In general, gravelly sands suitable for drain material lie on the interior impoundment slopes to the west of the dam. Materials suitable for structural fill are exposed on the surface over most of the TSF footprint. Borrow areas developed during phased construction will extend across construction phase limits but will lie within the ultimate TSF footprint.



## 6.2 Toe Berm and Starter Dam

The Phase 1 toe berm and starter dam are illustrated on Drawing 11. The TSF at final build-out is shown on Drawing 12, and the toe berm and dam sections and details are shown on Drawings 13 and 14.

A temporary toe berm will be constructed around the downslope TSF periphery in Phase 1. The temporary toe berm will be removed and reconstructed as the liner is extended outward and downslope in Phases 2 through 4. In Phase 5, a permanent toe berm will be constructed.

The primary purpose of the temporary toe berm is to contain runoff and sediment from the dam face, and direct dam drainage to the underdrain collection system and then to the TSF underdrain collection pond. Both the temporary toe berms and permanent toe berm will be constructed with structural fill and a geomembrane “flap” draped over the perimeter berm. The temporary berm liner flap will be anchored in a temporary perimeter anchor trench on the top of the berm. To relocate the temporary toe berm, the geomembrane flap will be folded inward over sandbags to divert drainage away from the temporary berm while it is removed. Once the temporary toe berm is removed, the liner extension will be installed and the original liner will be laid back over the liner extension. The seam between the two liners will then be extrusion welded in accordance with industry standards. This method will ensure that the liner seam is located in the downgradient flow direction for tailing drainage. At final build-out, a permanent toe berm will be constructed to contain runoff and sediment, buttress the dam toe and establish the limit for reclamation cover placement. The return water pipeline from the TSF underdrain collection pond will also run along the upstream side of the toe berm and above the geomembrane liner. The return water pipeline will be relocated in conjunction with the reconstruction of the temporary toe berms during Phases 2 through 4. In Phase 5, the return water pipeline will be placed in its final location along the upstream side of the permanent toe berm and above the geomembrane liner.

The Phase 1 starter dam will be constructed to an elevation of 5,250 ft-msl, with a 2.5H:1V inner slope and a 2H:1V outer slope. The purpose of the starter dam is to provide initial containment of tailing material, and to aid in tailings distribution from the dam crest. In the early stages of the operation, impounded water may periodically come in contact with the upstream face of the starter dam. The upstream face of the Phase 1 starter dam will be lined with an extension of the TSF geomembrane liner. The purpose of the liner extension is to prevent tailing drainage into the starter dam fill. The liner extension is illustrated on Drawing 13.

The starter dam will be constructed over the impoundment liner and underdrain collection systems. In Phases 2 through 5, starter dam extensions will be constructed to a height of 10 feet over the liner surface along the south, west and north boundaries.



### 6.3 TSF Liner System

Liner system details are shown on Drawing 14. The liner will consist of an 80-mil HPDE liner placed on a 12-inch thick liner bedding fill layer. In Phase 1, the liner bedding fill will consist of a minimum of 12 inches of tailings recovered from the north cell of the old starter dam. After Phase 1, liner bedding fill will consist of a 12-inch layer of crushed and screened native material, or selected local soil.

### 6.4 Tailings Drainage

Drainage from future tailings will be collected in two separate underdrain systems and transported to the TSF underdrain collection pond. Drainage from the TSF impoundment interior will be collected in a continuous underdrain (impoundment underdrain) constructed over the geomembrane liner. A separate blanket drain will underlie the tailings dam (dam underdrain). The layout of the underdrain systems is shown on Drawing 15. Underdrain details are shown on Drawings 16 and 17.

#### 6.4.1 Drain Description

The impoundment underdrain system will consist of a system of primary 10-inch diameter drainage pipes placed in drainage channels, and a system of 4-inch diameter lateral drain pipes that cover the remainder of the TSF interior floor. Two types of drain fill will be used for the impoundment underdrain. These include primary drain fill placed as an envelope around primary drain pipes, and a continuous minimum 18-inch thick layer of selected native drain material that covers the impoundment liner and contains the lateral pipe network. The primary drain fill will be produced by processing native gravelly sand to reduce its content of fine sand, silt and clay sized particles. The native drain fill material will consist of selected site soils (gravelly sand).

Scour protection will be placed at points of cyclone overflow discharge to protect the impoundment underdrain system. The scour protection will consist of locally derived coarse material cover over the underdrain, or the incorporation of energy dissipation measures on discharge spigots. Scour protection details are provided in Drawing 29. The specific number and type of scour protection required will be determined based on estimated cyclone overflow discharge volumes and flow velocities.

The dam underdrain system will consist of a minimum 18-inch thick layer of primary drain fill material and a network of 4-inch diameter internal drainage pipes.

##### 6.4.1.1 Impoundment Underdrain

Pipes in the impoundment and dam underdrains will be placed at a spacing that maintains minimum hydraulic head on the geomembrane liner and reduces the potential for leakage through the geomembrane liner. Pipe spacing is a function of the rate at which tailings drainage reports to the underdrain and the hydraulic conductivity of the drain fill.



Tailings placed in contact with the impoundment underdrain can be expected to rapidly consolidate and form a low permeability layer over the drain fill. Slurry consolidation tests indicate that the cyclone overflow that will be deposited in the impoundment interior will exhibit post-consolidation hydraulic conductivities ranging from  $5 \times 10^{-7}$  cm/sec for material deposited on the tailings beach, to  $5 \times 10^{-8}$  cm/sec for cyclone overflow slimes. Assuming a unit hydraulic gradient and an average tailings beach and slimes hydraulic conductivity of  $2.75 \times 10^{-7}$  cm/sec ( $9 \times 10^{-9}$  feet/second), the rate of drainage through the tailings and into the drain layer will be on the order of  $7.8 \times 10^{-4}$  feet/day/ft<sup>2</sup>. At final build-out with an impoundment floor area of 321 acres, total drainage collected in the impoundment underdrain will be on the order of 66 gallons per minute (gpm).

The drain pipe spacing is set to maintain a drain layer water depth and liner head that is less than the drain layer thickness of 1.5 feet. Because drainage into the impoundment underdrain will occur at a very low rate, the drain layer fill hydraulic conductivity can be relatively low and still maintain drainage and low liner head. Using the mound equation (Masada, 1988) and a hydraulic conductivity of  $3.8 \times 10^{-5}$  cm/sec for the native material drain fill layer, a spacing of 35 feet between impoundment lateral drain pipes will result in a maximum liner head of 1.33 feet. The spacing calculation assumes a 1 percent grade between drain pipes, which is the minimum grade on the TSF floor. Steeper slopes between drain pipes will reduce the head on the liner. Drainage mound and pipe spacing calculations are contained in Appendix D.1.

#### 6.4.1.2 Dam Underdrain

The dam underdrain constructed beneath the cyclone underflow sand fill will be subject to different conditions. The sand will be relatively permeable and the drainage rate will be variable because sand deposition locations will change frequently. The water in the cyclone underflow will be delivered to the dam at an average flow rate of approximately 1,042 gpm. In order to determine dam underdrain pipe spacing, the following assumptions were used to estimate drain inflow rates:

- Approximately 42 percent of the water deposited with the cyclone overflow will be retained in the tailings pore space. The remainder (approximately 58 percent) will either drain through the sand dam and report to the drain, or be lost to evaporation. Approximately 15 percent of the underflow water is assumed to be lost to evaporation. The resulting maximum flow of approximately 448 gpm will report to the dam underdrain.
- Dam construction is assumed to occur over an area of approximately 100 by 600 feet (60,000 square ft<sup>2</sup>).
- The slope between drain pipes is one percent.
- The dam underdrain fill (primary drain fill) will be a relatively clean fill, with approximately 20 percent finer than the No. 4 standard sieve, produced by crushing and/or screening of native gravelly sand. It is assumed to have a hydraulic conductivity of  $1 \times 10^{-1}$  cm/sec. A permeability test of prepared drain fill material with 50 percent finer than the No. 4 sieve exhibited a hydraulic conductivity of  $9.1 \times 10^{-2}$  cm/sec.



Based on these assumptions, the rate of application of cyclone drainage to the dam underdrain will be approximately  $5.42 \times 10^{-4}$  cm/sec (1.5 feet/day/ft<sup>2</sup>). Using the mound equation, a pipe spacing of 45 feet will result in a maximum head of 1.41 feet on the geomembrane liner. Drain layer thickness will be a minimum of 1.5 feet.

## 6.4.2 Drain Fill and Tailings Filter Compatibility

### 6.4.2.1 General Requirements for Drain Fill

Drain fill materials shall meet hydraulic conductivity and stability requirements. The dam underdrain fill (primary drain fill) shall be capable of retaining the cyclone underflow sand while allowing the transfer of drainage, i.e., without clogging. Inside the impoundment, the primary drain fill shall also be compatible with the underdrain pipe slot size and shall be capable of retaining the select native drain fill material while allowing tailing drainage to pass.

The select native drain fill layer shall meet a number of conditions, including:

- It shall be erosion resistant to control the potential for scour while temporarily exposed on impoundment slopes.
- It shall be compatible with the drain pipe slot size.
- It shall be capable of retaining the cyclone overflow slimes.
- It shall be retained by the primary drain fill.

### 6.4.2.2 Dam Underdrain Fill/Primary Drain Fill

Figure 3 shows the particle size distribution curve (PSD) for the cyclone underflow that will be used to construct the TSF embankment and will be in contact with the dam underdrain. The cyclone underflow filter envelope defines the range of drain fill gradations that are filter compatible with the cyclone underflow, i.e., materials that will restrict the migration of the cyclone underflow sand into the drain fill. It is assumed that this material will be prepared on site by reducing (screening out) a portion of the fine fraction of the native gravelly sand. A drain material with a minus 150-micron (No. 100 standard sieve) fraction of less than 10 percent and a minus 75-micron (No. 200 standard sieve) fraction of less than 5 percent is anticipated.

Figure 3 shows the estimated average PSD for the native soil composite samples following removal of the fine fraction with approximately 10 percent finer 425 microns (approximately 10 percent passing the No. 40 sieve). The modified gradation falls within the filter envelope for the cyclone underflow.

Type N-12 dual wall perforated, corrugated pipes have 3-millimeter (mm) wide slots for pipe diameters up to 10 inches. For broadly graded drain fill, Federal Emergency Management Agency (FEMA) (2007) recommends that the ratio of the D<sub>85</sub> particle size (the particle size for which 85 percent is finer) to the drain pipe slot width be greater than 4. To satisfy this condition, a D<sub>85</sub> of at least 12 mm is recommended.



Gravelly sand composite samples that were collected to represent materials available for drain construction meet the  $D_{85}$  requirement. When screened to remove fine particles, the  $D_{85}$  particle size will increase and the material will continue to meet the  $D_{85}$  size recommendation.

The primary drain fill that will be placed as an envelope around the primary drain pipes in the TSF interior will meet the same  $D_{85}$  size requirements for compatibility with the drain pipe slot size. The gravelly sand processed for the dam underdrain will also meet the requirements for primary drain fill.

For field production of the dam drain fill and primary drain fill during TSF construction, the minus 0.19 inch fraction (material finer than the No. 4 sieve) will be screened out to produce a granular drainage material. Table 7 presents the specification for the primary drain fill.

**Table 7: Primary Drain Fill Specification**

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	100-40
No. 4	0-20
No. 100	<10
No. 200	<5

Based on review of native soil gradation test results, a primary drain fill recovery rate of 40 to 60 percent is estimated during processing of available gravelly-sand soils. Fine rejects will be suitable for liner bedding fill material.

#### 6.4.2.3 Select Native Drain Fill

The cyclone overflow slimes represent the finest material that will be discharged into the impoundment and placed in contact with the impoundment underdrain. Figure 4 shows the PSD of slimes and the gradation of the filter envelope required to retain the slimes. The SD of the composite gravelly sand samples that are anticipated to be used for the select native material drain fill are also shown. The PSD intercept the slimes filter band in the critical  $D_{15}$  range. The select native drain fills will be capable of retaining the slimes. Some local clogging could be anticipated because the native material fines content is higher than required; however, the material is expected to function as needed because the drainage rate into the filter layer will be very low. Table 8 presents the gradation specifications for the select native drain fill.



**Table 8: Select Native Drain Fill Specification**

Particle Size (inch) or Sieve Size	Percent Passing
1	100
3/8	90-60
No. 4	80-40
No. 10	70-30
No. 100	10-30
No. 200	>10

If the native soils are crushed to minus 1 inch, the recovery of select native drain fill should be 100 percent during field processing for construction.

#### 6.4.2.4 Primary and Select Native Drain Fill Compatibility

The primary drain fill shall be capable of retaining the select native drain fill, while allowing tailing drainage to pass from the select native fill into the primary drains. Figure 5 shows the filter envelope for the select native drain fill and the estimated PSD for the primary drain. The primary drain fill in the critical  $D_{15}$  to  $D_{50}$  range of the PSD falls within the select native drain material filter envelope and the two drain materials are expected to be filter compactible, i.e., the select native drain fill will not migrate into the primary drain fill.

#### **6.4.3 Drain Piping**

The primary drainage pipes in the impoundment underdrain will be 10-inch diameter Type N-12 dual wall perforated, corrugated polyethylene pipe (PCPE). The 10-inch PCPE pipe will be plain-end and joined with soil tight split couplings.

The lateral drain piping in the impoundment and dam underdrain will be 4-inch diameter Type N-12 PCPE pipe with plain ends and soil tight split couplings.

#### **6.4.4 Pipe Placement**

The primary drain pipes will be placed in a constructed channel or drainage swale that is a minimum of 16 inches deep and inside an envelope of primary drain fill as shown in Drawings 16 and 17. After the primary drain fill is placed, the lateral drain pipes will be placed on the liner surface and oriented to drain into the primary drain pipe channel. In the primary pipe channel, lateral pipes will lie in select native fill and run parallel to the primary drain fill envelope for a length of at least 4 feet. The select native drain material fill will be placed over and around the primary drain fill and lateral drainage pipes. The final cover of primary and select native drain fill placed over the primary pipes will be a minimum of 24 inches thick. Pipe placement is such that collapse or damage of an individual lateral drain pipe will not result in the transfer of tailings or select native drain fill into the primary drain pipes. The cover of select native drain fill



over the primary drain fill will prevent the transfer of soil or tailings from the lateral to primary pipe network.

Dam underdrain pipes will be placed within the drain fill and oriented to discharge outside the toe of the dam in the dam underdrain and runoff collection channel. As the dam is raised, the dam underdrain and runoff collection channel will be relocated and the dam underdrain pipes will be extended to the new perimeter collection channel.

#### 6.4.5 Drain Pipe Deflection

Deflection analyses were conducted to evaluate the performance of the underdrain pipes under the loads imposed by a tailings cover of approximately 240 feet. The Type N-12, dual walled PCPE pipes used for the underdrain collection system are considered to be flexible and can resist damage by distorting sufficiently to shed overburden loads to the surrounding underdrain fill.

As pressure on the top of the pipe is increased through an increase in tailings height, an increasing proportion of the vertical pressure on the pipe is transferred to the surrounding fill. This process is commonly called bridging. Therefore, the key parameter in assessing deflection of the underdrain collection system is the stiffness (modulus) of the fill in contact with the pipe. This bridging phenomenon was first accurately modeled by Burns and Richards in the paper *Attenuation of Stress for Buried Cylinders* (Burns and Richards, 1964). Golder has analyzed the pipe stresses and deformations based on the work of Burns and Richards and Hoeg (1968), with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression.

Golder analyzed the worst-case scenario with an entire column of tailings underflow cyclone sand of maximum height placed over the underdrain collection system. Based on Golder laboratory test results for the tailings underflow cyclone sand, a friction angle of 39 degrees and soil density of 120 pcf was used to model the tailings properties used in the analysis. The supporting fill was assumed to consist of the select native drain fill material with a similar stress versus strain relationship for a silty sand compacted to 80 percent of maximum dry density. The primary drain fill used beneath the dam and in the impoundment primary drains will have a reduced fines content compared to the select native drain fill, and will exhibit a strength that is at least that of the select native drain fill. Modeling deflection based on the select native drain fill is conservative.

Drain fill properties, along with pipe dimension and properties, were used to determine the maximum deformation that may be expected for the pipes. The maximum vertical pipe deflections are estimated to be between 11 and 14 percent. Golder's observations have been that pipe deflections greater than 15 to 20 percent often result in plastic deformation of the pipe at the springline. A vertical deformation of 15 percent is assumed as the maximum limiting deflection for flexible pipe. Therefore, the estimated



deformations of 11 to 14 percent are within the acceptable performance criteria established by Golder. Detailed data worksheets and calculations are provided in Appendix D.2.

#### **6.4.6 Drainage Outlet Works**

Tailing drainage from the impoundment underdrain will be routed beneath the tailings starter dam and cyclone sand dam to the TSF underdrain collection pond. Drainage outlet works details are shown on Drawings 16 and 17. The primary drain pipe network in the impoundment underdrain will be reduced to three pipes to transmit drainage to the outlet works. Three primary drainage pipes will be routed into a 12-inch diameter Type N-12 PCPE manifold at the upstream toe of the starter dam, which will in turn be connected to a 14-inch diameter, Schedule 80 carbon steel drain pipe. The steel drain pipe will be routed from the upstream toe of the starter dam to the TSF underdrain collection pond in a 42-inch deep by 42-inch wide ditch that will be backfilled with concrete.

The steel outlet pipe will pass through a valve vault. The valve vault will consist of a 72-inch diameter prefabricated concrete manhole base unit placed on an 18-inch thick reinforced concrete foundation mat. The manhole base unit will be fabricated with inlet and outlet openings for the steel drain pipe. Prefabricated manhole riser sections will be used to extend the valve vault vertically to a maximum height of 100 feet, to an elevation of approximately 5,300 ft-msl.

A 14-inch diameter, hydraulically actuated knife gate valve will be installed on the steel outlet pipe in the valve vault. The hydraulic actuator lines will be 100 feet long and will be routed up through the riser sections to the top surface of the dam for connection to a portable hydraulic power pack. As the manhole riser sections are added, the outlet valve hydraulic lines will be secured to the man-way ladder steps cast into the manhole riser sections. This arrangement will enable valve operation without manhole entry.

The purpose of the outlet valve is to prevent the drainage of excess water into the TSF underdrain collection pond in the early stages of impoundment operation (i.e., before the impoundment underdrain is covered with cyclone overflow). As positioned, the valve will be upstream of the main body of the dam, and its use will not result in pressurization of the underdrain pipe inside the dam. Once the underdrain is covered with cyclone overflow, the flow into the impoundment underdrain will be limited by the low hydraulic conductivity of the tailings slimes overlying the drain. When the dam reaches a height of 100 feet (approximately 5,300 ft-msl), the outlet valve will be fully opened and the valve vault will be backfilled with cement grout and granular fill materials.

### **6.5 TSF Underdrain Collection Pond**

#### **6.5.1 Pond Description**

The location of the TSF underdrain collection pond is shown on Drawings 2, 10, 11, and 12. TSF underdrain collection pond details and sections are shown on Drawings 13, 18, 19 and 20. Figure 6



illustrates total TSF underdrain collection pond capacity, the maximum operating water required to preserve upset and stormwater storage capacity, and the maximum stormwater storage level.

The TSF underdrain collection pond will be double-lined with minimum 60-mil HDPE geomembrane liners. An HDPE geonet will be placed between the liners to serve as the collection pond leakage collection and recovery system (LCRS) and minimize the head on the lower pond liner. The pond will be fitted with a primary drain material filled sump and LCRS pump to recover any leakage through the upper geomembrane.

TSF underdrain collection pond reclaim pumps will be submersible, vertical turbine pumps supported in a reinforced concrete sump and headwall structure. The sump will allow the water level in the pond to be drained to the pond floor level and no dead storage will be required. The use of submersible turbine pumps mounted in a concrete sump will eliminate the potential for liner damage associated with a barge mounted pump coming to rest on the pond floor. The reinforced concrete sump is shown in cross section on Drawing 19.

Impoundment underdrain flows will be transported to the pond via a buried steel pipe. Runoff and dam underdrainage will be routed to the pond via an HDPE lined open ditch constructed at the toe of the dam.

### **6.5.2 TSF Underdrain Collection Pond Sizing**

The TSF underdrain collection pond will contain drainage water from the TSF impoundment and dam underdrains, as well as runoff from the downstream face of the tailings dam. The pond is sized to contain 24 hours of tailing drainage flow at maximum estimated drainage rates, runoff from the 100-year, 24-hour storm event of 3.73 inches (National Oceanic and Atmospheric Administration [NOAA] 2006) incident on the downstream dam face, and an additional minimum 2-feet of freeboard. Underdrain flow rate calculations and runoff estimates are contained in Appendix E.

Underdrain flow rate estimates are based on the assumption that materials representative of the consolidated, cyclone overflow will be in contact with the impoundment underdrain and will control the rate of tailing drainage reporting to the TSF underdrain collection pond. The hydraulic conductivity of materials representative of beach and slimes samples are  $5.0 \times 10^{-7}$  and  $5.0 \times 10^{-8}$  cm/sec, respectively. If it is assumed that the more permeable beach-like material cover 60 percent of the impoundment underdrain at final build-out, the maximum underdrain flow rate will be on the order of 66 gpm.

Approximately 1,042 gpm of water will be delivered to the dam in cyclone underflow with a moisture content of approximately 30 percent. An estimated 42 percent of the water will be permanently bound or entrained within the pore space of the sand fill, and an additional 15 percent is estimated to be lost to evaporation. The remaining 28 percent (448 gpm) is assumed to report to the dam underdrain and TSF underdrain collection pond.



A storage allowance is provided for potential inflows associated with the free water pond coming in direct contact with the impoundment underdrain system. In this case, the drainage rate will be controlled by the hydraulic conductivity of the select native drain fill material that will cover the impoundment floor and drain pipe network. Permeability testing of a representative sample of select native drain fill indicated a hydraulic conductivity of  $3.8 \times 10^{-5}$  cm/sec. If it is assumed that a 20-acre area of impoundment drain will be inundated to an average depth of 2.5 feet, the estimated drainage rate will be on the order of 1,220 gpm.

The maximum contribution of stormwater runoff to the TSF underdrain collection pond will be from the combination of dam out-slope area and exposed toe area liner and underdrain occurring in Phase 4. The 100-year, 24-hour storm event incident on this area is estimated to produce a runoff volume of 3.94 million gallons.

Table 9 summarizes the TSF underdrain collection pond storage capacity requirements. The pond capacity is approximately 12.24 million gallons with 2 feet of dry freeboard below the crest of the pond (top of pond liner). The pond has the capacity to store up to approximately 5.8 million gallons of process water for facilitating process water make-up or storage of extra water during wet periods. A maximum water surface elevation of 5,157 should be maintained in order to provide sufficient storage for stormwater associated with the 100-year, 24-hour storm event and a coincident 24-hour upset period.

**Table 9: TSF Underdrain Collection Pond Storage Capacity Requirements**

Source	Type of Inflow	Volume (gal)
Dam Face Runoff	Storm Event Runoff	3,942,528
Dam Underdrainage	24-hour upset volume	645,206
Impoundment Underdrainage	24-hour upset volume	95,074
Free water pond direct drainage	24-hour upset volume	1,754,857
<b>Total</b>		<b>6,437,666</b>

## 6.6 Surface Water Management

### 6.6.1 Control of Impoundment Runon

The TSF will be required to contain inflows and direct precipitation associated with the 72-hour PMP of 26 inches. Diversion ditches constructed for impoundment runon control have been sized to carry the peak discharge associated with the prescribed PMP event using a rainfall intensity versus time distribution defined in Hydrometeorological Report 55A (US Department of Commerce, 1998). Runoff estimation and ditch sizing calculations are contained in Appendix F.1.

Diversion ditches will be constructed to divert runon away from the impoundment where possible. Peripheral catchment and runoff contributory areas are limited because the TSF lies in the head of a hydrologic catchment area. The Phase 1 grading plan (Drawing 11) indicates the location of diversion ditches. The Phase 1 ditches are located outside the Phase 2 construction area and will be functional



during Phases 1 and 2. In Phase 3, a permanent diversion ditch will be constructed on the west periphery of the TSF as shown on Drawing 11. Table 10 summarizes peak discharge estimates for the Phase 1 and 3 diversion ditches.

**Table 10: Summary of Impoundment Runoff Diversion Ditch Capacity and Size Requirements**

Phase/Location	Peak Discharge (cfs)	Ditch Width (ft)	Ditch Depth (ft) <sup>(3)</sup>
Phase 1, TSF northeast	525	10 <sup>(1)</sup>	5.5
Phase 1, TSF southwest	340	10 <sup>(2)</sup>	5.1
Phase 3/ TSF southwest periphery	205	10 <sup>(2)</sup>	4.2

**Notes**

<sup>(1)</sup> 2H:1V side slopes assumes on the downslope side, slope on upstream side varies

<sup>(2)</sup> 2H:1V slopes

<sup>(3)</sup> Depth at the lowest channel slope, includes 1 foot of freeboard.

### 6.6.2 Dam and TSF Underdrain Collection Pond Surface Water Management

Surface water management facilities other than impoundment diversions are designed to contain and transport flows associated with the appropriate 100-year storm event. Hydrologic calculations for the dam and TSF underdrain collection pond surface water management facilities are contained in Appendix F.2.

Runoff from the downstream face of the dam will be routed to the TSF underdrain collection pond. The time of concentration ( $T_c$ ) for the toe ditch catchment area is estimated to be three hours. The dam underdrain and runoff collection channel at the toe of the dam has been sized to carry the peak discharge associated with the 100-year, 3-hour storm. The 100-year, 3-hour storm will produce the peak 100-year storm runoff of 71 cubic feet per second. The flow depth at peak discharge is estimated to be a maximum of 0.5 feet. The perimeter toe berm height will be 3 feet (temporary berms) to 4 feet (permanent berms) high and will provide 2.5 to 3.5 feet of dry freeboard in the toe area dam underdrain and runoff collection channel.

### 6.7 Cyclone Plant Area

Excavation and site preparation will be required for the cyclone plant pad, the pump equipment pad and the surge pond. The cyclone plant general arrangement plan and site grading plan are shown on Drawing 21. The purpose of the cyclone plant is to separate whole tailings into sand and slimes fractions. Its' design and purpose are described in more detail in Section 7.0.

Surge pond cross sections and details are shown on Drawing 22. The purpose of the surge pond is to contain discharges (tailings, process, and reclaim water) from various processing locations under upset conditions, due to a pipe failure or shutdown of the cyclone plant. Upset flows from the cyclone plant will discharge by gravity to the surge pond within a secondary containment ditch lined with a minimum 60-mil HDPE geomembrane liner placed over 6 inches of liner bedding fill. Further details of the secondary containment ditch are provided below in Section 7.4.



## 7.0 TAILINGS DELIVERY AND DISTRIBUTION SYSTEM DESIGN

### 7.1 General System Description

The tailings delivery and distribution system design consists of pipeline system that delivers whole tailings from the processing plant to the tailings storage facility. Whole tailings will be separated into fine material and sand material in the cyclone plant. The sand fraction will be transported to the TSF and used for dam construction while fine material will be deposited into the TSF. The tailings surge system is designed for tailings management in case of unanticipated shutdown of any of the tailings stations or surges or overflows from station sumps. Return or reclaim water will be collected from the TSF surface pond and TSF underdrain water collection pond and transported back to the process plant. A general process flow diagram for the tailing delivery and distribution system is provided on Drawing 23.

Process equipment for the tailings delivery and distribution system will be located in four main stations as listed below:

- Cyclone Station: including the cyclone cluster, slurry pumps, slurry transfer sumps, gland seal water system, and electrical equipment;
- Surge Discharge System: including the surge pond evacuation pumps and lined secondary containment ditches;
- TSF Return Water Pond Barge Station: including a floating barge and barge mounted vertical turbine pumps and electrical equipment; and
- TSF Underdrain Collection Pond Pump Station: including vertical turbine pumps in a permanent structure and electrical equipment.

Tailings distribution will include whole tailings transport from the process area to the cyclone station and sand and fine tailings transport to the TSF. Return water will include tailing drainage water and TSF return water transported to the process plant. The major pipelines are listed below, and their interactions are shown in the overall system process flow diagram on Drawing 23.

- Cyclone Feed Line
- Cyclone Overflow Line
- Cyclone Underflow Line
- Cyclone Whole Tailings Bypass Line
- TSF Return Water Line
- TSF Underdrain Collection Return Water Line
- Main Surge Discharge Line

The major pipelines will be installed within secondary containment ditches lined with a minimum 60-mil HDPE geomembrane liner placed over six inches of liner bedding fill. The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23



NMAC, and will include secondary containment. Further details of the secondary containment ditches are provided below in Section 7.4.

The arrangement of the major components of the tailings delivery and distribution system is shown on Drawings 21 and 24. Drawings 25 and 26 present the tailings delivery and distribution system plan and profile. Whole tailings produced at the flotation plant will be transported via a 30-inch HDPE DR17 pipeline to the cyclone plant at the northwest side of the TSF. The cyclone plant will separate the sands fraction, which represents approximately 45 percent of the whole tailings stream, from the slimes fraction, which represents approximately 55 percent of the whole tailings stream. The sands fraction or the “underflow” of the cyclone plant will produce an underflow slurry which will be transported to the TSF in a separate 12-inch HDPE DR9 pipeline and discharged on the dam. The cyclone underflow sand placed on the dam crest will be then be spread, graded and compacted, as necessary to push sand down the dam out-slope and continually build the dam.

The cyclone overflow (slimes) from the cyclone plant will be routed to the TSF in a separate 30-inch HDPE DR17 pipeline and discharged into the impoundment. The cyclone overflow water will be returned to the process water reservoir at a rate of up to 13,000 gpm. The cyclone plant will operate continuously to produce the sand material needed for continuous construction of the dam. In the event of upset conditions when the cyclone plant is not in operation, whole tailings will be discharged via gravity into the surge pond through a lined secondary containment ditch (see Section 7.4).

## 7.2 Tailings Delivery

### 7.2.1 Underflow Sand

The cyclone underflow pipeline will deliver sand to the top of the embankment for tailings dam construction. Two underflow pipelines will be used. The east leg will be routed around the north side of the TSF, and the south leg will be routed around the south side of the TSF as shown on Drawing 24. Each leg is sized to transport 100 percent of the cyclone underflow at up to 45.6 percent sand recovery. This allows for 100 percent availability of sand delivery to the dam.

Cyclone underflow will be discharged through 4-inch spigots placed every 333 feet. Each spigot will include one 4-inch manual pinch valve. The underflow pipelines will also have in-line knife-gate isolation valves every 2,000 feet to allow for isolation and relocation of the pipe as the dam rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pick-up truck.

The north and south cyclone underflow pipelines will be operated independently. When one is in operation, the other can be serviced or broken down and relocated. Cyclone underflow pipes will be flanged each 500 feet to facilitate breakdown and relocation.





## 7.2.2 Cyclone Overflow

Two cyclone overflow delivery pipelines, one leg to the north side and one leg to the south side of the TSF, will transport the cyclone overflow to the TSF interior (impoundment interior) as shown on Drawing 24. The cyclone overflow will be discharged via spigots placed every 667 feet. Each spigot will include a manual pinch valve. Each pipe is sized to carry 100 percent of the cyclone overflow to permit pipeline relocation without interrupting operation as the TSF elevation rises. One leg will remain active while the other is serviced or relocated.

The cyclone overflow pipelines will also have knife-gate isolation valves placed every 2,000 feet to allow for isolation and relocation of the pipe as the impoundment rises. The knife-gate isolation valves will be quick-disconnect with hydraulic actuators powered by a mobile hydraulic power unit mounted on a pickup truck. The cyclone overflow delivery pipelines will be flanged every 500 feet to allow for breaking down and relocating the pipe.

## 7.3 Deposition Management

### 7.3.1 Dam Construction

Figure 7 illustrates height versus capacity and surface area relationships and rate of rise for the tailings impoundment. Near continuous operation of the cyclone plant will be required to produce sufficient sand to construct the dam to the ultimate elevation of 5,460 ft-msl. The difference in elevation between the dam crest and the head of the cyclone overflow impoundment beach will be maintained at 10 feet. Maintenance of this elevation difference will place the transition between the cyclone underflow sand dam fill and the interior cyclone overflow a distance of approximately 30 feet upstream of the inside dam crest. The elevation differential of 10 feet will be maintained in order to maintain adequate freeboard for stormwater storage. Maintaining this elevation differential also allows for maximum storage capacity for cyclone overflow, and thus maximum production of sand needed for dam construction.

In the early stages of operation, there will be more sand available than needed to maintain the elevation differential between the dam crest and the head of the beach. Excess sand will be pushed down the out-slope of the dam and used to construct the dam base. In the later stages of operation, the sand previously used to construct the dam base will reduce the sand requirements for raising the dam crest, and facilitate maintaining the crest to beach elevation differential.

### 7.3.2 Cyclone Overflow Discharge

The storage volume for the cyclone overflow will be maximized through managing deposition and practicing sub-area, thin lift deposition. Discharge spigot locations will be frequently cycled so that a thin lift of tailings is placed on the tailings beach. Exposure to evaporation prior to burial with a subsequent lift of tailings will allow the tailings to desiccate and consolidate. The degree to which thin lifts can be placed



and consolidation can occur under managed deposition is primarily a function of rate of tailings rise. It is also influenced by tailings properties, climatic conditions, surface water management, and operator effort.

#### 7.4 Management of Upset Flows

Potential upset flows from the process area, cyclone plant, and TSF will be controlled through a series of secondary containment ditches, the surge pond, and the TSF underdrain collection pond (see Section 6.5). The secondary containment ditches and associated pipelines will be constructed in accordance with the requirements listed in 20.6.7.23 NMAC. The secondary containment ditches will run from the process area to the TSF (the main ditch), from the main ditch to the cyclone area, and from the cyclone area to the surge pond. The secondary containment ditches are designed to contain and transport flows via gravity that are related to potential upset conditions and direct precipitation onto the ditches associated with the 25-year 24-hour storm event (2.88 inches). Maximum upset flow conditions would be associated with overtopping of the process water reservoir (as estimated by M3, the design contractor for the process water reservoir). This maximum upset flow was assumed to be 18,000 gpm over a 30-minute period, at which point the process area pumps would be shut down. The secondary containment ditches are designed for these maximum upset flows, direct precipitation, and an additional 2 feet of freeboard. The main ditch is designed to flow to the TSF by gravity for the first six years. After year six, gravity flow to the TSF is no longer possible because of the increased height of the TSF and upset flows will then discharge to the surge pond via gravity in a lined ditch through year 11.1. The alignment of the secondary containment ditches is shown on Drawings 2, 3, 10, 11, 12, 21, and 24 through 26. Details of the secondary containment ditches are provided in Drawing 29.

Surge pond cross sections and details are shown on Drawing 22. The surge pond liner system will consist of a liner bedding fill layer overlain with a minimum 60-mil HDPE geomembrane liner. The surge pond is located at an elevation of 5,340 feet and is sized for a surge retention time of half an hour with an additional reserve capacity of over one million gallons. The pond is sized for the retention of approximately 1,610,000 gallons of slurry with an additional 2 feet of freeboard. The use of the surge pond will be intermittent and temporary and the pond will be empty under normal operating conditions. The pond will be equipped with dedicated hard-wired pumps that will automatically evacuate its contents. Emergency power for the pumps will be provided by the emergency diesel power generation system located on-site in the event of a power outage. The process facility control room will be equipped with emergency alarms that notify the operator of an upset condition allowing the operator to make necessary adjustments in the process, as needed. The pumps at the surge pond will be automatically activated upon the pond reaching a predetermined level. Water and solids collected from the surge pond will be discharged through a 12-inch HDPE DR17 pipeline to the top of the TSF. The solids handling pump is designed to evacuate the surge pond within 12 hours.



## 8.0 WATER RECLAIM SYSTEM DESIGN

The water reclaim system is a significant part of NMCC's water conservation program. It will provide approximately 75 percent of the water used in the process in the form of recycled water. The purpose of the water reclaim system is to recycle supernatant water stored in the TSF and water captured in the underdrain collection gallery and stored in the TSF underdrain collection pond. The TSF water reclaim system will recover water released as the cyclone overflow consolidates within the TSF. The underdrain water collection system will recover water from the bottom of the tailings impoundment through the TSF underdrains into the TSF underdrain collection pond. The underdrain pond will also store water captured by the dam underdrains from the downstream side of the dam as sand is deposited and compacted as well as precipitation run-off water from the face of the dam. All of this water will be transported from the TSF and TSF underdrain collection pond to the process water reservoir located at the plant via a 20-inch HDPE variable DR return water pipeline. Water from the TSF underdrain collection pond may also be pumped back directly to the TSF. The major components of the water reclaim system are shown on Drawing 27. Water reclaim system details are shown on Drawing 28.

The TSF reclaim system will be a barge-mounted pump station in the impoundment equipped with four pumps (three operating and one spare) with a design flow from this station of 13,000 gpm. This is equal to the maximum design rate at which water will be delivered to the TSF from the cyclone plant during normal operation.

The TSF underdrain collection pond system will be a pump station of two pumps (one operating and one spare) installed in a sump within the pond. The design flow from this station will be 4,000 gpm. This is the maximum design flow of the TSF underdrain collection gallery. It is anticipated that up 4000 gpm of flow will be captured by the underdrains in the initial stages of operation of the TSF. This flow rate will become less over time as the tailings impoundment fills and the underdrains become overlain with tailings materials.

The water reclaim barge in the TSF impoundment will be accessed from a ramp constructed over the impoundment liner as shown on Drawing 28. The ramp will be approximately 35 feet wide and initially constructed to a height of 10 feet above the impoundment floor. As the tailings level rises during operations, the position of the barge will migrate up the ramp northwestward along the reclaim pipeline alignment. In each construction phase, ramp construction will be completed with borrowed structural fill material.



## 9.0 WATER BALANCE

Water balance calculations are included in Appendix G. Figure 8 summarizes the results of a water balance analysis of the proposed TSF for average rainfall conditions. The water balance model incorporates water input from slurry water inflow, direct precipitation on the impoundment surface and runoff from un-diverted upgradient areas. On-site meteorological data collection at Copper Flat was initiated in August 2010. Due to the short duration of record keeping and recording gaps, NOAA data from Hillsboro and Caballo Dam, New Mexico, were used in conjunction with Copper Flat data to estimate monthly precipitation and evaporation rates. The ratio of site evaporation to Hillsboro evaporation for months where data are available from both sites was used to estimate site evaporation for months where data were not collected at the site.

Water balance model losses include entrainment of water within the tailings solids, evaporation from the TSF supernatant pool, evaporation from the exposed tailings beach, and evaporation of water from the dam. Entrainment represents the most significant water loss and is calculated on the basis of the estimated final post-deposition dry density of the cyclone underflow and cyclone overflow. An average post-deposition dry density of 57 pcf is assumed for the cyclone overflow. Over the life of the facility, approximately up to 49 million tons of cyclone underflow will be produced assuming near constant operation of the cyclone plant and mill.

The water balance model does not identify reclaim rates from specific locations because operation of the impoundment will impact where water accumulates. Water that is not lost to evaporation or bound within the tailings is assumed to be recovered from either the impoundment free water pond or the TSF underdrain collection pond.

The impoundment underdrain will be equipped with a shutoff valve at its inlet during the initial years of operation so that when the water level in the TSF underdrain collection pond exceeds the normal operating level, the underdrain will be closed to utilize the TSF supernatant pool for storage and the TSF undergrain collection pond will be pumped down.

A total process water inflow of 12,978 gpm is estimated based on average operating conditions. This inflow includes water contained in the cyclone overflow slurry and water delivered to the dam with the cyclone underflow. As shown in Figure 8, the estimated process water reclaim rate averages 9,215 gpm. The average make-up water requirement, calculated as the difference between the process water delivered and the water reclaimed, is estimated to be 3,169 gpm or approximately 152 gallons per ton of tailings placed in the TSF. The maximum estimated make-up water rate is 3,676 gpm.

The water balance examines water reclaim rates for average rainfall conditions. If the site experiences precipitation that is less than or exceeds average conditions, water reclaim rates and make-up water



requirements can also be expected to vary. The water reclaim system is capable of recovering water at a rate adequate to account for all water in the tailings slurry discharged from the flotation plant. Maximum reclamation of water following storm events can temporarily reduce demand on external water sources. The water balance does not consider additions from the open pit or waste rock stockpile stormwater ponds. Water available from other on-site sources is not expected to be significant.



## 10.0 TAILINGS DAM STABILITY ANALYSIS

### 10.1 Methods

Slope stability analyses were performed in support of the feasibility design of the Copper Flat TSF with an ultimate crest elevation of 5,435 ft-msl. The analyses were performed using limit-equilibrium slope stability software and Spencer's (Spencer, 1967) method of slices to compute the theoretical factors of safety (FOS) for various potential failure surfaces. Material properties used in the stability calculations for native soil and tailings materials were based on laboratory geotechnical testing performed in Golder's in Denver, Colorado laboratory and discussed previously in Sections 3.3 and 4.3.

Slope stability analyses were conducted to determine the FOS against failure for the critical stability section along the highest embankment section and most adverse subsurface topography on the downstream slope. The critical factors of safety assumed for stability analysis are 1.5 for static conditions and 1.1 for pseudo-static conditions, according to NMAC 19.25.12.11.12. Stability analyses were performed for static (steady-state) and dynamic (pseudo-static) loading conditions. Steady-state loading conditions represent the long-term stability of the TSF and pseudo-static loading conditions represent the stability of the TSF during the design earthquake loading event.

The computer package SLIDE™ (Version 6.021) was used to conduct the stability analyses (Rocscience, 2013). An arcuate (circular) failure mode was used to analyze the critical section with the shear surface failing through the tailings and/or foundation materials. A block failure mode was used to analyze the critical section with a potential failure at the liner interface. Both methods are based on the principle of limit equilibrium, i.e., the method calculates the shear strengths that would be required to maintain equilibrium, and then calculates the FOS by dividing the available shear strength by the shear strength required to maintain stability.

Analyses were limited to the investigation of global failures that can affect the full height of the embankment, are deeper seated, and may result in lowering the embankment crest and loss of containment. Local stability analyses associated with shallow slope failures were not investigated.

A pseudo-static analysis approach was used for the seismic loading case. With this method, a lateral force is added to a potential failure mass, with magnitude equal to some fraction of the weight of the sliding mass. The fraction is defined in the form of a pseudo-static coefficient and is expressed as a percentage of gravity. Selection of the pseudo-static coefficient is discussed below. Stability analysis supporting data and computer-generated outputs are contained in Appendix H.



## 10.2 Seismic Design Criteria

According to the regulations set by the NMDSB, the TSF can be classified as having a significant hazard potential. Dams assigned the significant hazard potential classification are those where failure results in no probable loss of human life but can cause economic loss, environmental damage, and/or disruption of lifeline facilities. The NMDSB requires that structures such as the Copper Flat TSF be designed to withstand the seismic loading from the Maximum Design Earthquake (MDE) with a 2 percent probability of exceedance in 50 years (approximately 2,475-year return frequency). The peak ground acceleration (PGA) for the Copper Flat property was obtained using the US Seismic “Design Maps” Web Application developed by the United States Geological Survey (USGS) Geologic Hazards Science Center (USGS, 2011). Considering the 2009 National Earthquake Hazards Reduction Program provisions for a Site Class C and a site location of 32.96° North latitude and 107.5° West longitude, the resulting PGA for the 2,475-year return MDE is approximately 0.13 times gravitational acceleration (0.13g).

The method developed by Hynes-Griffin and Franklin (1984) and Jansen (1985) was used to evaluate pseudo-static loading conditions, as outlined by the NMDSB guidelines (2010). This method recommends that the pseudo-static coefficient selected for analysis must be at least 50 percent of the predicted PGA, but not less than 0.05g, and the FOS under pseudo-static analysis should be 1.1 or greater. A coefficient of 0.087g, corresponding to two-thirds of the design PGA, was conservatively used for the analyses.

The results of the previous seismic liquefaction potential evaluation of the Copper Flat TSF by SHB (1980) indicated that the probability of liquefaction affecting the Quintana TSF was extremely remote based on the seismic hazard potential of the site and empirical data derived from case histories of tailings dams and natural, saturated, loose sandy deposits subject to earthquake-induced ground motions. Golder anticipates that a drained response to seismic loading will dominate the pore water conditions in the TSF during and following its active life based on the material permeabilities, boundary drainage conditions, and construction practices. Given the seismic hazard potential of the site and the proposed TSF construction and operating practices, the liquefaction potential of the new TSF is also considered low.

## 10.3 Tailing Drainage and Phreatic Conditions

The primary source of tailing drainage is drain-down water associated with the centerline-constructed cycloned underflow tailings portion of the embankment, with a minor contribution from consolidation and drainage of the impounded cyclone overflow upstream of the embankment crest. The drainage control system consists of a continuous granular fill blanket drain beneath the cyclone underflow sand embankment and toe area, with lateral underdrain pipes spaced at 45-foot centers across the embankment footprint. Lateral drain pipes will connect to an open channel, which gravity drains to the HDPE-lined TSF underdrain collection pond.



The phreatic surface was assigned to be coincident with the drainage control system at the base of the final embankment, based on performance of similar structures. Because of the large difference in the hydraulic conductivity of the cyclone underflow and cyclone overflow, a near vertical phreatic surface is assumed at the dam/beach interface. This condition will result in the main body of the dam being well-drained and unsaturated.

#### 10.4 Pore Pressure Conditions and Liquefaction Potential

The response of tailings material to loading can be either drained or undrained, and is associated with the development of pore water pressures. For stability analyses, a phreatic surface was assumed at the beach material surface level upstream of cycloned underflow tailings embankment and an undrained response was evaluated. Undrained analyses were performed by applying an undrained strength to the cyclone overflow on and beneath the beach.

Susceptibility to liquefaction potential is assumed to be limited to the saturated beach areas upstream of the dam. Steady-state (residual) undrained strength was applied to the tailings beach upstream of the cyclone sand dam fill to evaluate post-liquefaction stability.

The risk of static and seismic liquefaction triggering will likely be low if appropriate control over embankment construction and the phreatic-surface elevation is exercised, and the cyclone underflow sand behaves as anticipated. Sand placed on the dam crest will be spread and compacted. Operating experience at mine sites utilizing cyclone underflow fill indicates that self-weight compaction of sand on the tailings out-slope is typically adequate for minimizing liquefaction potential. At Copper Flat, some compaction will be realized on the dam out-slope as a result of dozer spreading operations.

#### 10.5 Material Properties

The material properties used in the stability analyses are based on a review of the properties used in previous site studies (SHB 1980), new data derived from testing of samples collected during the site geotechnical investigation, and from tests conducted on tailings characterization study samples. The components of the slope stability model include:

- Foundation Materials
- Liner Interface Zone
- Cyclone Underflow sand
- Cyclone overflow (beach material)
- Structural Fill

Table 11 summarizes the strength parameters used in the preliminary slope stability analyses.



**Table 11: Summary of Properties Used in Stability Analyses**

Component	Unit Weight (pcf)	Drained Strength		Undrained Strength ( $S_u/\sigma'_v$ )
		Cohesion (psf)	$\phi'$ (degrees)	
Foundation Materials	120	150	29	NA
Liner Interface Zone	120	0	26.5	NA
Cyclone Underflow	113	0	39	NA
Cyclone overflow	108	NA	NA	0.05
Structural Fill	120	0	29	NA

**Notes:**

pcf = pounds per cubic foot

psf = pounds per square foot

 $\phi'$  = Effective stress friction angle $S_u/\sigma'_v$  = Residual undrained shear strength normalized by the effective overburden stress

NA = Not applicable

**10.5.1 Foundation Materials**

The foundation underlying the proposed TSF embankment has been characterized as an alluvial deposit comprised of predominately silty and clayey sands and gravels. Foundation material strength test results ranged from 28 to 32 degrees (effective friction angle). Based on the results of consolidated-undrained triaxial tests completed by Golder, an effective stress friction angle ( $\phi'$ ) of 29 degrees and effective cohesion of 150 pounds per square foot were applied for the shear strength of the foundation materials.

**10.5.2 Liner Interface Zone**

The shear strength of the cycloned underflow tailings/geomembrane liner/liner bedding interface will be the controlling factor for possible sliding block-type failure surfaces along the base of the TSF embankment. The interface shear strength was evaluated in a direct shear test performed by Golder. Old (Quintana) tailings, and drain fill materials were placed in contact with a sample of textured, 80-mil HDPE geomembrane. The composition of the test sample is representative of the Phase 1 interface, when the old tailings will be utilized for liner bedding fill. The use of coarser, higher strength materials (crushed and screened native gravelly sand) is anticipated in later construction phases. The direct shear test indicated an interface friction of 26.5 degrees. This strength was assigned to the interface zone comprised of liner bedding, geomembrane and drainage materials.

**10.5.3 Cyclone Underflow**

The sloping, cyclone underflow embankment was considered to be fully drained and cohesionless because of the anticipated low phreatic-surface elevation and the high permeability of the cyclone underflow in the embankment, and the effect of the drainage control system. Mittal and Morganstern (1975) evaluated the  $\phi'$  for cycloned copper tailings sands. The effects of particle crushing and sand dilatancy were most pronounced at stresses up to 40 psi, while at higher stresses  $\phi'$  remained relatively constant and approximately equal to 34 degrees. Volpe (1975) reported  $\phi'$  values for copper sands and slimes between 33 to 37 degrees. A consolidated-undrained triaxial test conducted by Golder on a



cyclone underflow sample indicated an internal friction angle of 40 degrees. An effective friction angle of 39 degrees was assumed in stability analyses.

#### **10.5.4 Cyclone Overflow**

Cyclone overflow in the impoundment is assumed to exhibit a drained response for most loading conditions except for the conditions occurring after static or seismic liquefaction. However; for conservatism, an undrained shear strength behavior was assumed for the cyclone overflow upstream of the embankment.

Undrained shear strength implicitly accounts for the effects of shear-induced pore pressures. Based on experience from similar mining projects and published data (Mittal and Morganstern 1975, Vick 1990), the peak undrained shear strength normalized by the effective overburden stress ( $S_u/\sigma'_{vo}$ ) was estimated to be 0.20 for the impoundment area extending upstream from the embankment crest.

Residual undrained shear strengths were used in a static analysis to evaluate embankment stability following liquefaction in the beach area. Similar to that for peak shear strength, the residual undrained shear strength normalized by the effective overburden stress ( $S_u/\sigma'_{vo}$ ) was estimated to be 0.05 for the tailings upstream of the cyclone underflow dam.

#### **10.5.5 Structural Fill**

Strength parameters for the starter dam structural fill are based on testing of representative materials recovered from the TSF borrow areas during the site exploration. Foundation material strength test results ranged from 28 to 32 degrees. The material was conservatively classified as cohesionless with a  $\phi'$  of 29 degrees with a moist unit weight of 120 pcf.

#### **10.5.6 Liner Material**

The liner interface direct shear test was conducted with a sample of textured geomembrane. The use of un-textured geomembrane may be feasible, but testing of interface strength with un-textured liner has not been performed. To evaluate the potential for use of smooth liner, the interface strength was varied to find the interface friction angle required to meet the minimum pseudostatic FOS of 1.1. This analysis was performed for the block failure mode.

#### **10.5.7 Fissured Clay Foundation Analysis**

Stability analyses presented in Section 10.6 for the maximum embankment section address the embankment, the cyclone underflow beach and the liner interface zone on a maximum height embankment section. Additional sections corresponding to geology sections B-B' and D-D' were evaluated to determine the effects of clay foundation soils on the stability of the TSF.



Drill holes in the vicinity of these sections intercepted a clay layer that appears to be dipping to the eastward based on the first high plasticity clay intercepts identified during drilling. At several locations in the TSF expansion area, the top of this clay layer exhibited characteristics of a softened clay, with locally high moisture content and corrected SPT blow counts, in the range of 12 to 25, that were lower than those in overlying and underlying soils.

Stark and Eid (1977) state that fissured clays in first time slides (or first time slope failures) may exhibit a mobilized shear strength that is lower than the strength of fully softened clay, and suggest the use of the average of the fully softened and residual (large strain) clay strength in evaluating stability. Clay shear strength was estimated based on an empirical method presented by Mesri and Shahien (2003) that relates shear strength under varying normal stress to plasticity index. The method provides a non-linear shear strength envelope used to estimate the fully softened, residual and average strength of hard, fissured clays. The resulting shear strength versus normal stress envelope is used for clay strength input in the slope stability model.

The highest plasticity index ( $I_p$ ) of 42 percent in Copper Flat samples, which was associated with a softened clay sample recovered from drill hole BH-18 at a depth of 43 feet, was assumed for estimating the strength of the high plasticity clay layers. In section B-B', clay interbedded clay and granular soil layers were modeled. In Section D-D', the clay layer was assumed to extend from the first high plasticity clay intercept to the base of the model section.

## 10.6 Stability Analysis Results

The results of stability analyses for static and pseudo-static loading conditions are summarized in Table 12.

**Table 12: Calculated Slope Stability Factors of Safety**

Failure Mode	Method	Static FOS (Global)	Pseudostatic FOS (Global)
Maximum Section Circular	Spencer	2.53	1.92
Maximum Section Block	Spencer	2.24	1.69
Maximum Section Circular, Post Liquefaction	Spencer	2.53	NA
Maximum Section Block, min required interface strength = 13.6 degrees	Spencer	1.53	1.1
Fissured Clay Section B-B' Circular	Spencer	1.56	1.12
Fissured Clay Section B-B' Block	Spencer	2.50	1.90
Fissured Clay Section D-D' Circular	Spencer	1.53	1.13
Fissured Clay Section D-D' Block	Spencer	2.48	1.87

**Notes:**

FOS = factor of safety

NA = not applicable

Min = minimum



The conservative assumptions applied in the stability analyses suggest that the Copper Flat TSF will be stable. All factors of safety meet or exceed the minimum NMDSB requirements of 1.1 and 1.5 for static and pseudostatic conditions. The residual strength analysis suggests if liquefaction of saturated tailings upstream of the dam occurs, the embankment will remain stable. The evaluation of the sensitivity of pseudostatic stability to the friction angle of the liner interface indicates a relatively low interface friction angle is required to maintain stability, and that the required interface strength is likely to be achievable with an un-textured geomembrane liner.



## 11.0 TAILINGS DAM FOUNDATION SETTLEMENT POTENTIAL

### 11.1 Analysis Approach

The TSF will consist of an earthen starter dam constructed to a height of approximately 50 feet with the remainder of the dam constructed with sand recovered from the cyclone plant. A geotechnical investigation was performed in the embankment footprint, which included standard penetration testing and sample collection from the surface to a depth of 50 feet. Drilling indicated that in general, the tailings embankment foundation consists primarily of alluvial deposits that include silt, sand and gravel, which are underlain by clay.

Representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Selected samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because settlement of the embankment will be adequately mitigated by continuous fill placement during ongoing embankment construction. Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc., for the analysis of settlement and consolidation under foundations and embankments.

A detailed description of the settlement potential investigation, settlement calculations and supporting information are contained in Appendix I.1. Drill holes and the location of cross-sections used to evaluate subsurface conditions are shown on Drawing 3. Drawings 5 and 7 present geologic cross sections B-B' and D-D', respectively, which were developed to evaluate settlement perpendicular to the dam axis. The cross-sections also include information derived from the former geotechnical study conducted on behalf of Quintana by Sargent Hauskins and Beckwith (SHB, 1980). Drill hole logs are contained in Appendix A.2.

A differential settlement and geomembrane strain analysis was subsequently conducted by Golder and is included in Appendix I.2. Cross sections were developed to intercept the various geologic materials underlying the TSF site. The engineering properties of the foundation materials were derived from the 1980 Sargent, Hauskins and Beckwith (SHB) geotechnical study, the geotechnical investigation conducted as part of the TSF design report and experience with similar foundation materials.

### 11.2 Settlement Potential Analysis Results

Laboratory consolidation testing was conducted on remolded specimens of the fine fraction of samples recovered from the embankment foundation. As such, the settlement prediction does not account for the presence of the coarse fraction in the foundation soils, and associated inter-particle contact and support of



foundation loads. Settlement predictions based on the laboratory consolidation tests are therefore conservative.

Results of the settlement potential analysis are shown graphically on geologic sections B-B' and D-D'. The maximum calculated settlement beneath the embankment is approximately 2.1 feet in the area of the maximum dam (and tailings beach) foundation loads. Settlement decreases at a relatively uniform rate as the weight of post-construction loading decreases towards the outer toe of the embankment.

Settlement prediction based on the laboratory consolidation testing of the fine fraction of foundation samples is conservative. SPT testing conducted during drilling showed the foundation strata to generally be very dense to hard. On the basis of SPT test results, actual post-construction consolidation settlement of less than 1 foot is anticipated.

Dam construction will be more or less continuous during the life of the facility. The effects of foundation settlement include the potential for the loss of dry freeboard for stormwater storage. The potential loss of freeboard can be mitigated by elevating the dam crest with managed/targeted placement of cyclone underflow sand.

The analyses did not indicate the potential for differential settlement that could impact the integrity of the TSF geomembrane liner. Sections B-B' and D-D' indicate predicted settlement varies uniformly across areas subject to changing foundation loads.

The impoundment underdrain will pass beneath the dam in a steel pipe placed in a ditch backfilled with concrete near section F-F' (Drawing 9). The settlement will not adversely impact the impoundment underdrain outlet pipe. There is adequate grade and elevation change along the outlet pipe alignment to accommodate predicted settlement.

A basalt outcrop identified by SHB (SHB, 1980) may lie beneath or in the vicinity of the impoundment underdrain pipe inlet near the upstream toe of the dam. The outcrop occurred in an area that was disturbed during Quintana dam construction activities, and was not observed during the recent site exploration. If the inlet to the underdrain pipe bears on basalt, local differential settlement could occur along the pipe alignment, which could induce stress on the outlet pipe. If, during construction, a basalt outcrop is identified at the location of the inlet, an alignment change may be warranted to avoid the pipe bearing on basalt.

It should be noted that the settlement potential investigation was performed for a previously completed design study, and evaluated an embankment geometry that differs from that presented in this report. The new embankment is higher and the depth of embankment fill overlying the foundation is greater for this 30,000 tons per day design; however, the original analyses assumed a higher, more conservative embankment moist unit weight of 130 pcf. Tailings testing completed after the settlement potential study



was conducted indicates a post embankment fill placement moist unit weight of approximately 113 pcf. The foundation loads imposed by the higher embankment fill, when corrected for the moist unit weight determined by laboratory testing, are lower than those used in the settlement potential analysis. Therefore, the results of the settlement investigation presented above are conservative relative to the current design. As part of future detailed engineering studies, settlement calculations will be updated for final design conditions; however, the conclusions are anticipated to be consistent with those presented herein.

The results of the differential settlement and geomembrane strain analysis indicates that, in general, settlement potential across the TSF is predicted to be limited. As such, the potential for tearing of the HDPE liner due to potential differential settlement within the entire area of the TSF is considered to be low. The maximum settlement is estimated to be 0.72 feet, while the maximum tensile strain on the HDPE liner due to differential settlement is estimated to be 0.02 percent. The allowable tensile strain on an 80 mil HDPE geomembrane liner is 10 percent and the predicted tensile strain is well within acceptable conditions. Therefore, Golder does not expect tearing of the HDPE liner due to differential settlement to be an issue.



## 12.0 USE OF THIS REPORT

This feasibility level design report has been prepared by Golder exclusively for the use of THEMAC and NMCC. No third-party engineer or consultant shall be entitled to rely on any of the information, conclusions, or opinions contained in this report without the written approval of Golder, THEMAC or NMCC.

The conclusions and recommendations in this report have been prepared in a manner consistent with the level of care and skill ordinarily exercised by engineering professionals practicing under similar conditions, subject to the time limits and financial and physical constraints imposed on or otherwise applicable to the work.





### 13.0 REFERENCES

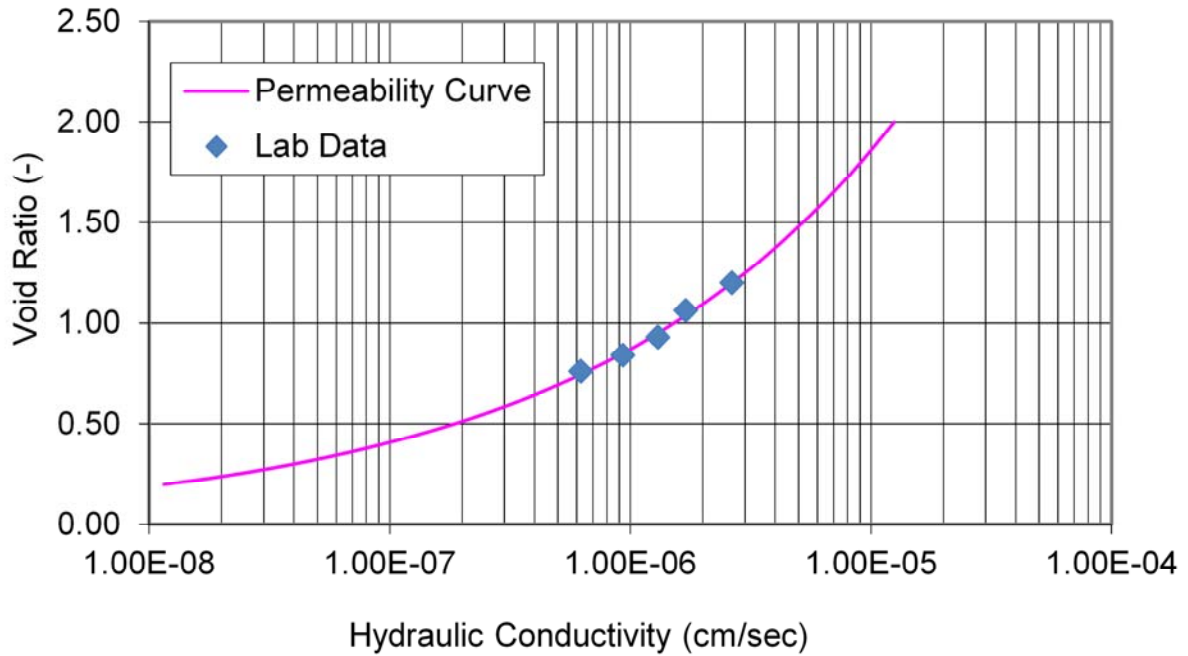
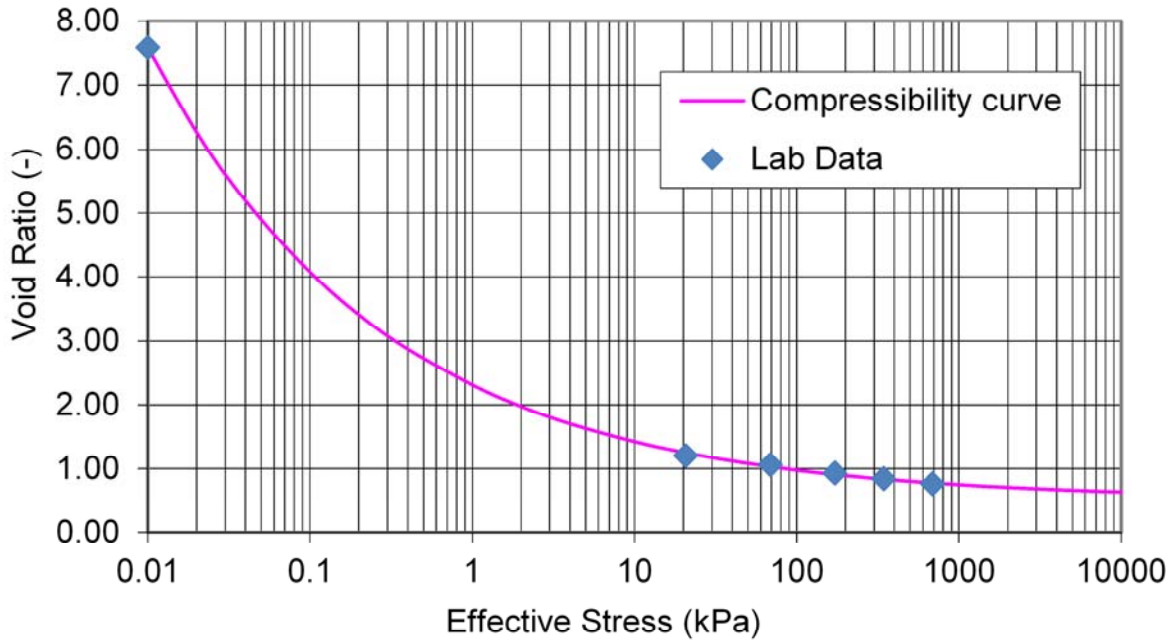
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## FIGURES

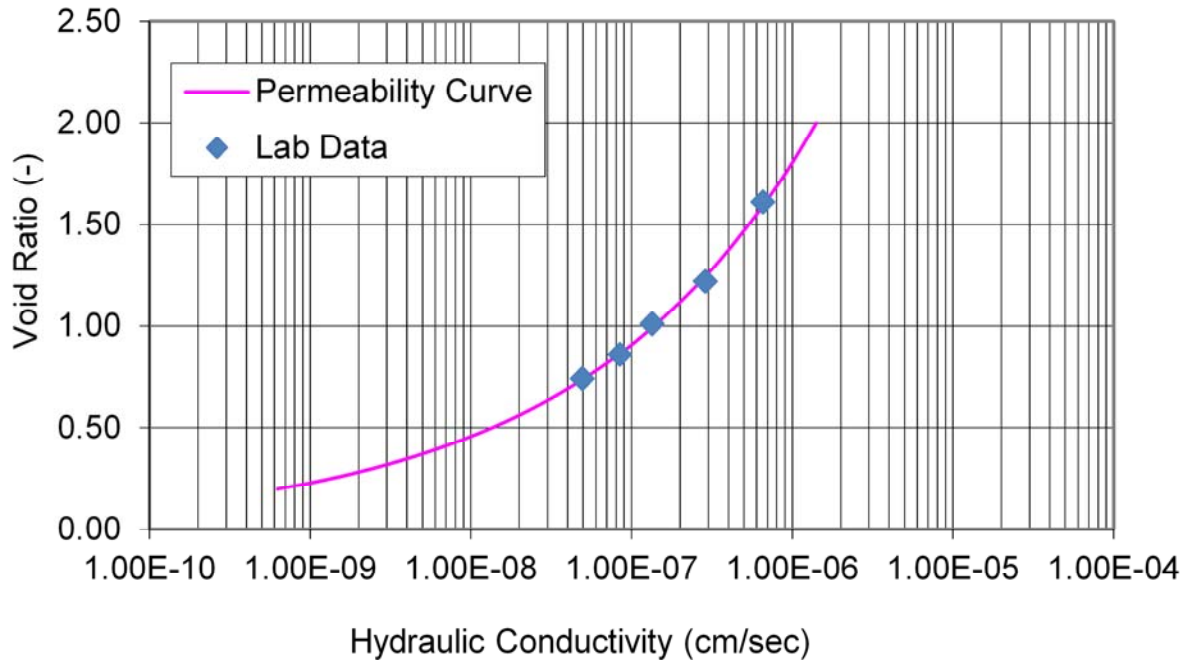
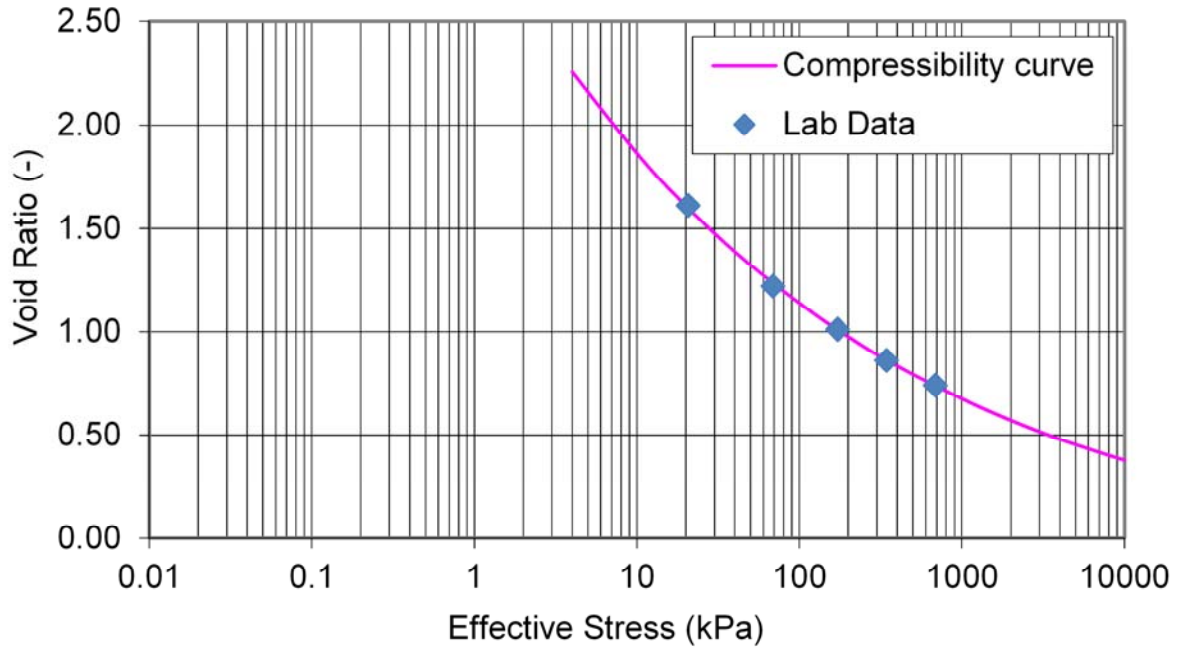
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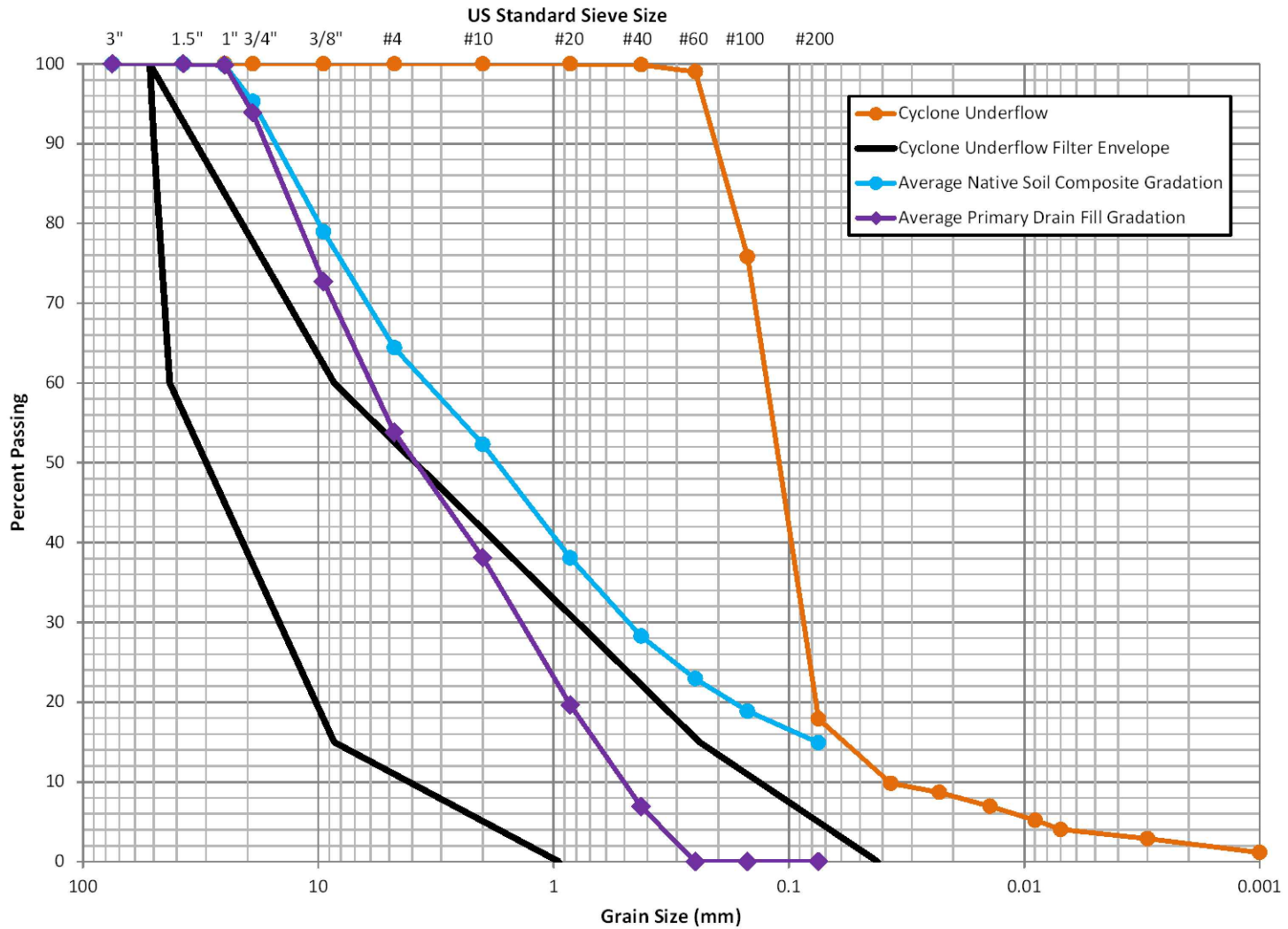
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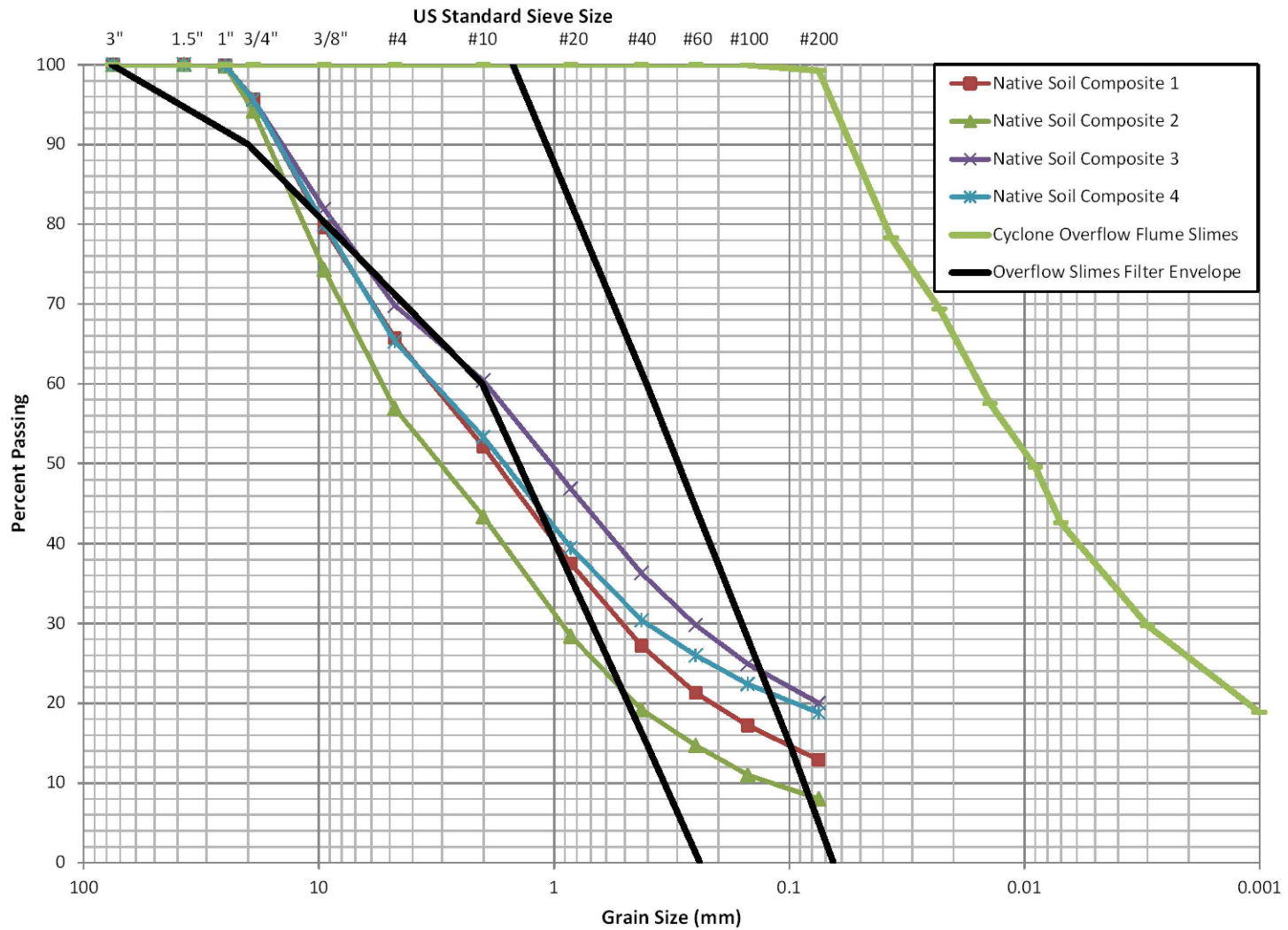
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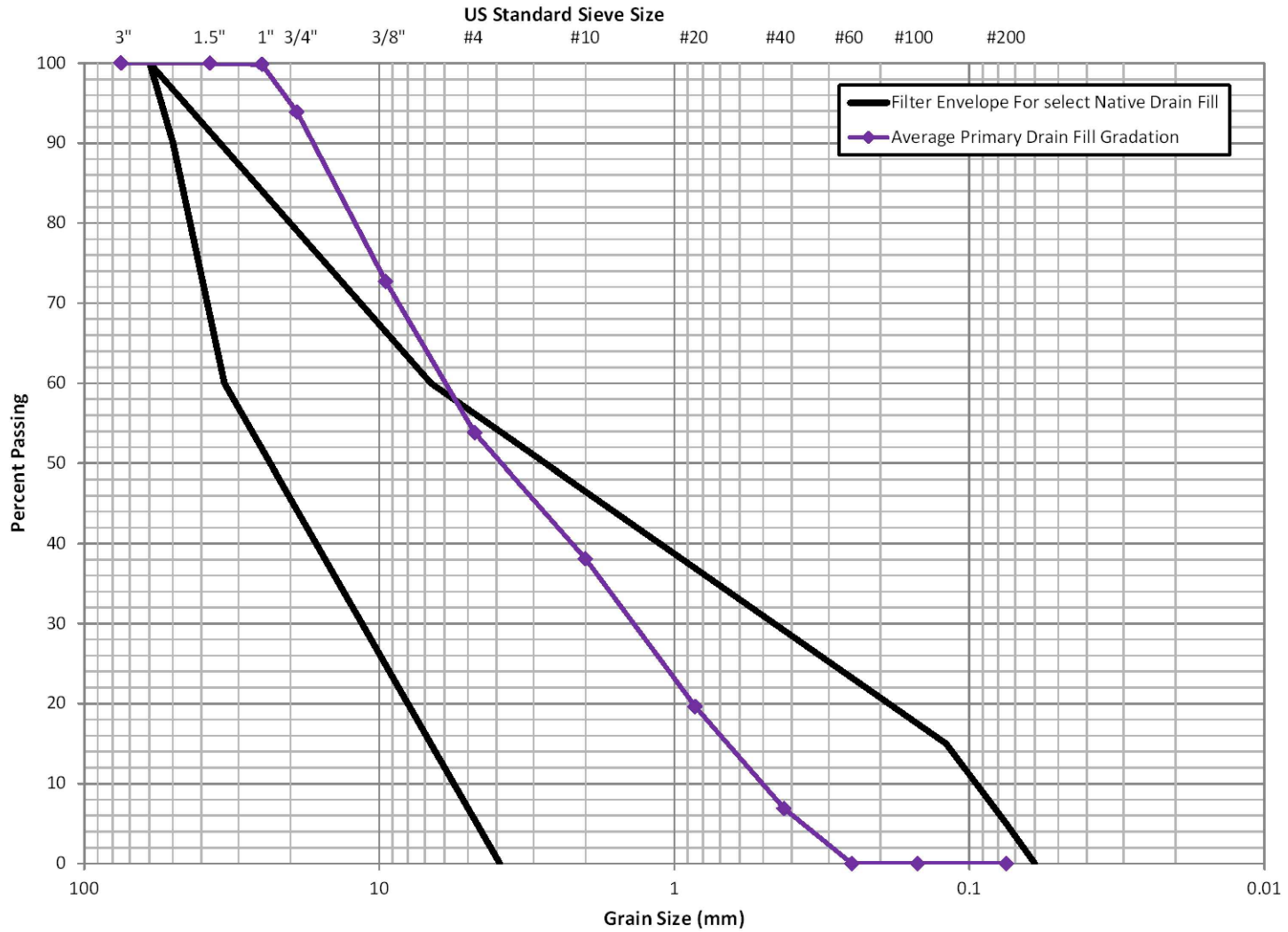
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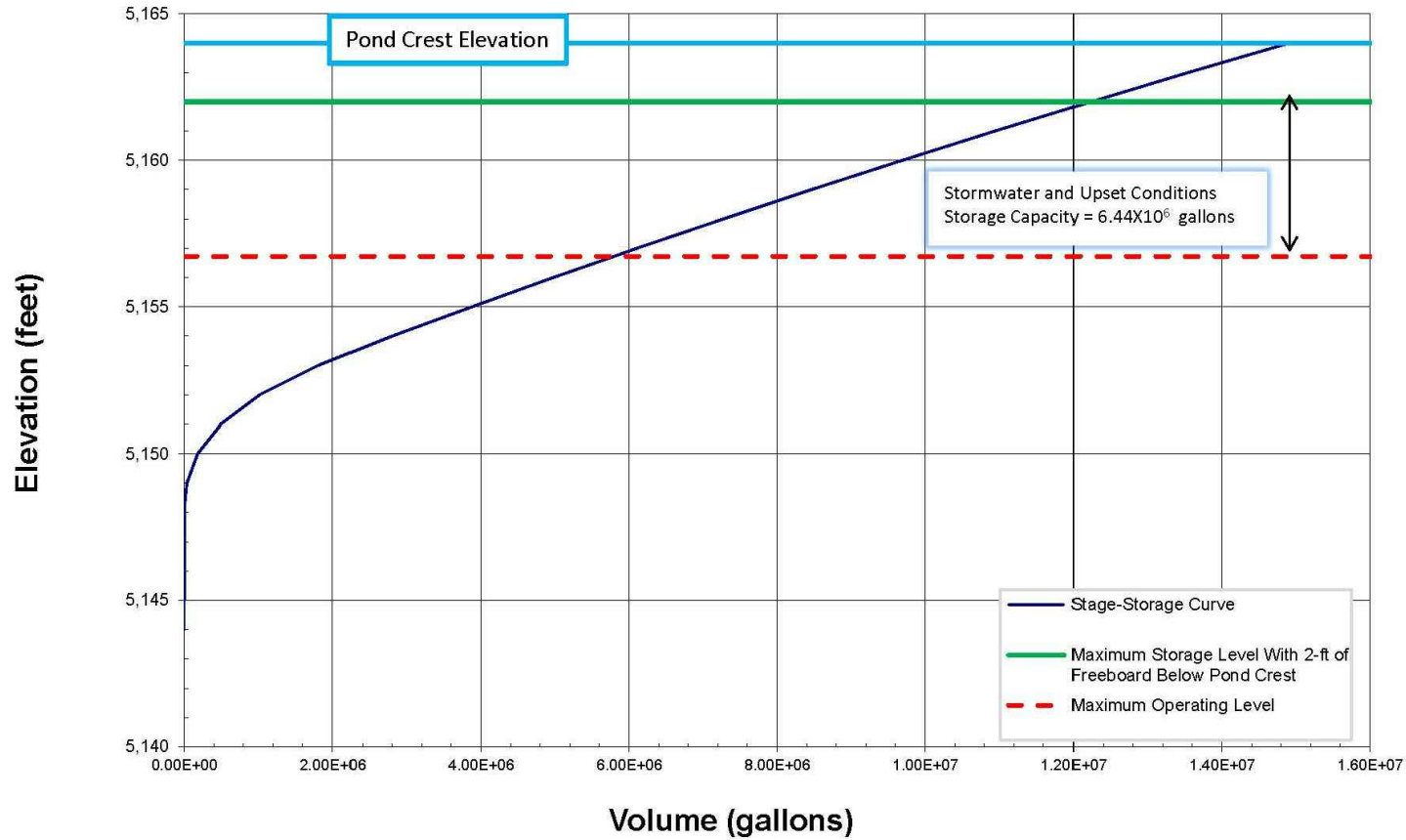





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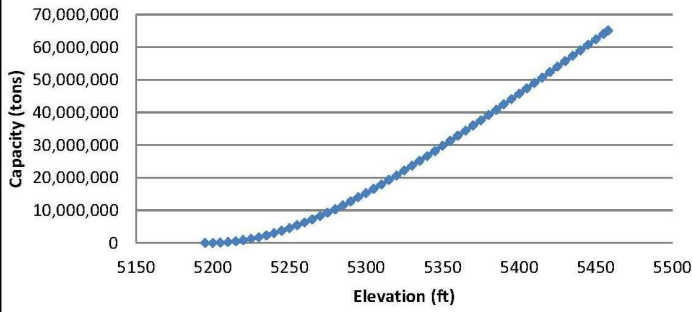




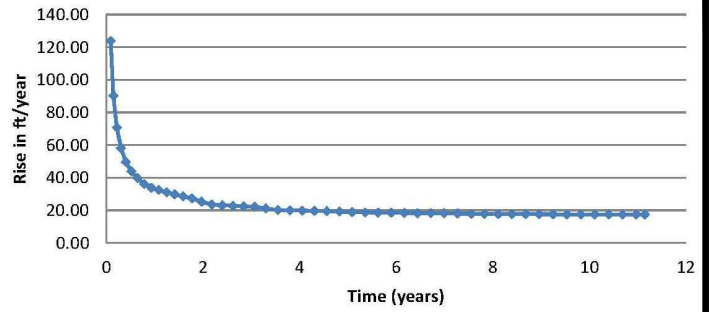
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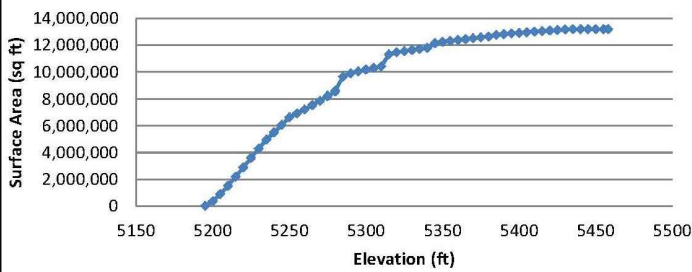
**Impoundment Capacity vs. Elevation**



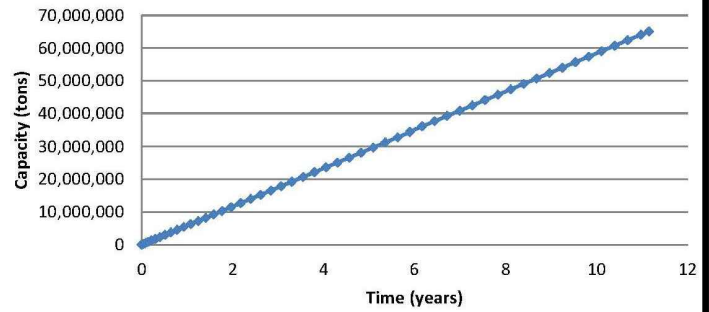
**Rate of Rise (ft/yr)**



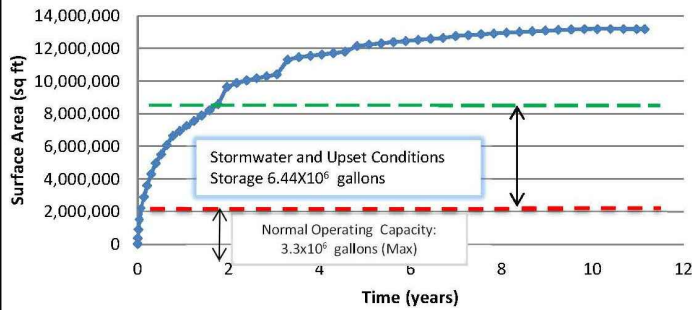
**Impoundment Surface Area vs. Elevation**



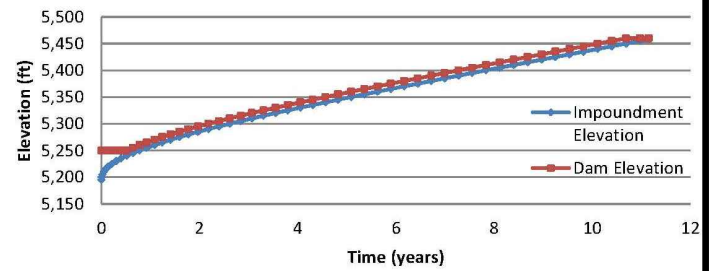
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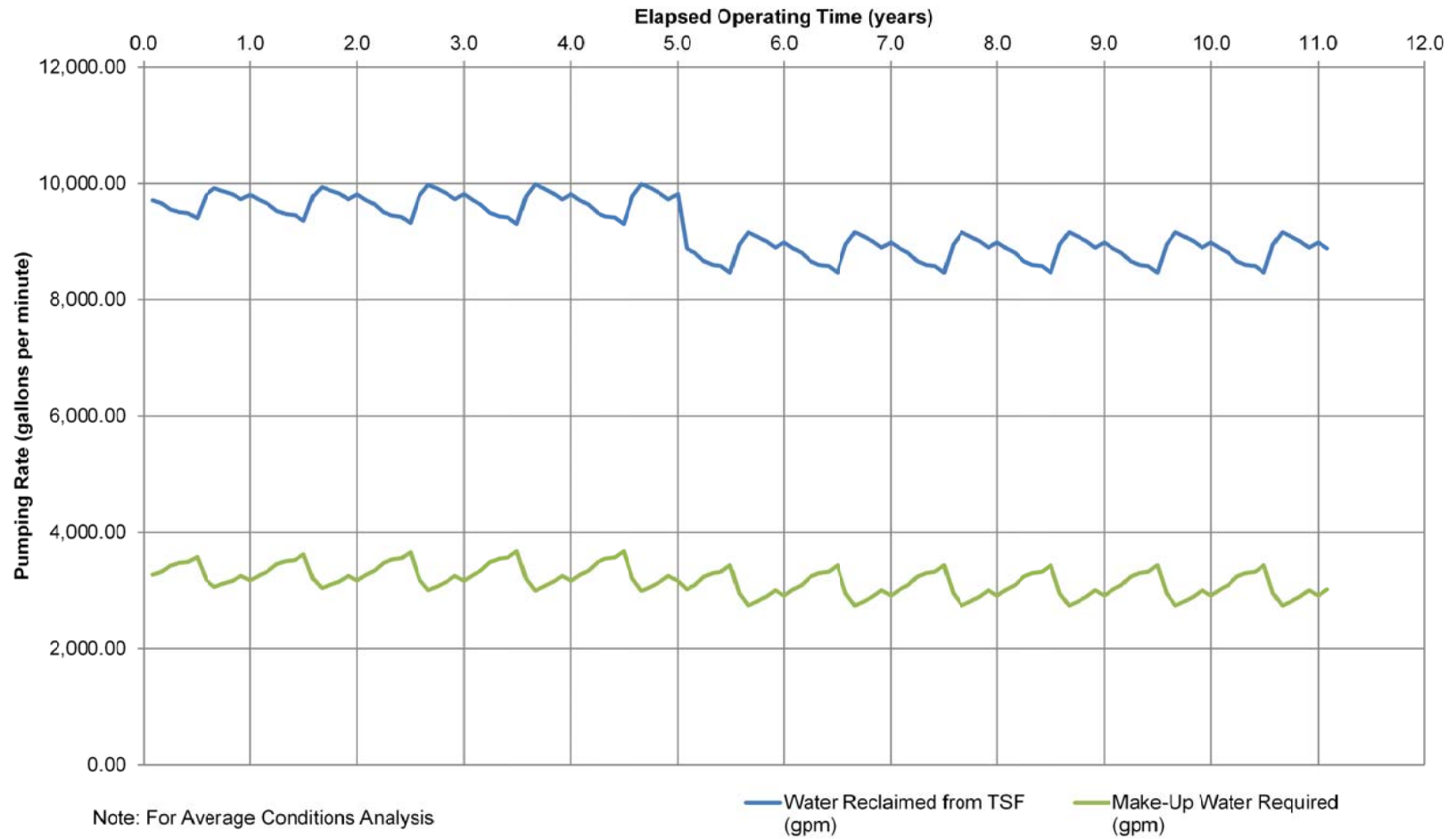
**Impoundment Surface Area vs. Time**






**Dam & Impoundment Elevation vs. Time**



<p>Environmentally Responsible. Community-Minded. Local Opportunities.</p>	<p>PROJECT: NEW MEXICO COPPER CORPORATION</p>		<p>DRAFT COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>		
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 <small>Environmentally Responsible. Community-Minded. Local Opportunities.</small>			<b>DRAFT COPPER FLAT PROJECT</b> <b>30K TPD TAILINGS STORAGE</b> <b>FACILITY FEASIBILITY DESIGN</b> <b>SIERRA COUNTY, NEW MEXICO</b>	
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**APPENDIX A  
SITE EXPLORATION**

**APPENDIX A.1  
TEST PIT LOGS**



# TEST PIT LOG: TP-1

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977739.46 E: 867983.49

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 2 ft.	GM	sandy SILTY GRAVEL, medium, sub-angular, 35% coarse sub-rounded sand, 20% medium plasticity fines, 30% sub-angular cobbles (3-10"); pinkish gray (7.5YR 6/2), weak CaCO <sub>3</sub> cementation; non-cohesive, dry, dense.
2 - 4 ft.	GP	GRAVEL, medium, and SAND fine to coarse, poorly graded, 10% non-plastic fines; 25% angular to sub-angular cobbles (3-10"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as cemented masses and disseminated; non-cohesive, dry, very dense.
4 - 8 ft.	SP	Friable/weathering rock (andesite); gravelly SAND, coarse, sub-rounded, 20% fine to medium sub-angular gravels; 10% non-plastic fines; 50% angular to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO <sub>3</sub> coating rock fragments and fractures.



**Samples:**  
None.

**Special Notes:**  
Archaeologist present during excavation.  
Pit located in road; top 2 ft. of original surface removed for road cut.  
Native hillslope has ~70% surface rocks; with weathered/friable boulders exposed along road cut.  
Test pit location immediately adjacent to existing waste rock disposal facility.



# TEST PIT LOG: TP-2

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11976945.00 E: 869820.78

**Date:** 12/21/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 15% fine to coarse sub-rounded to sub-angular gravel, 40% low plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 4/3), blocky; cohesive, slightly moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded to sub-angular gravel, 35% low plasticity fines; 5% sub-rounded cobbles (3-6"), pinkish white (7.5YR 8/2), moderate CaCO <sub>3</sub> cementation, blocky to platy; non-cohesive, dry, dense.
2 - 6 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded to sub-angular and SAND, fine to coarse, sub-rounded, 15% low to no plasticity fines; 20% sub-angular to sub-rounded cobbles and boulders (3-15"), pinkish gray (7.5YR 7/2), strong RXN with HCl, dry, dense.
6 - 7 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plasticity fines; 25% sub-angular cobbles and boulders (3-15"), light brown (7.5YR 6/3), strong RXN with HCl; non-cohesive, dry, very dense.
7 - 9 ft.	GW	Friable/weathering rock (andesite); GRAVEL, fine to coarse, angular, 10% coarse sand, 5% non-plastic fines; 50% angular cobbles and boulders (3- 15"), weak RXN with HCl.



### Samples:

2-6 ft., bag  
6-7 ft., bag

### Special Notes:

7 to 9 feet moderately strong rock, slightly weathered. Can slowly excavate with excavator (hard digging).



# TEST PIT LOG: TP-3

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977450.61 E: 869805.07

**Date:** 12/21/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM/ML	gravelly sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded to sub-angular gravels, 20% fine to coarse sub-rounded sand; 20% sub-angular cobbles and boulders (3-15");\, brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 20% low to non-plastic fines: 15% sub-angular cobbles, pinkish white (7.5YR 8/2), moderate CaCO <sub>3</sub> cementation; non-cohesive, dry, dense.
2 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 25% sub-angular cobbles and boulders (3-20"), pink (7.5YR 7/3), strong RXN with HCl; non-cohesive, dry, dense.
7 - 9 ft.	GC	CLAYEY GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 15% medium plasticity fines; 30% sub-angular cobbles and boulders (3-20"); brown (7.5YR 5/3), CaCO <sub>3</sub> disseminated and as coatings on rocks; non-cohesive, dry, dense.
9 - 11 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 10% low to non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/4), CaCO <sub>3</sub> as cemented masses, disseminated and coatings on rocks; non-cohesive, dry, very dense.



**Samples:**  
BMI samples all layers (bag samples)

**Special Notes:**  
Refusal at 11 ft., hit andesite. Fracturing andesite above.





# TEST PIT LOG: TP-5

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11977574.94 E: 868955.59

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	gravelly SILTY SAND, fine to coarse sub-rounded, 20% fine to coarse sub-angular gravel, 25% low plasticity fines; 25% sub-angular cobbles (3-8"), yellowish brown (10YR 5/4), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 20% low plasticity fines; 5% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses, weakly cemented in places, blocky; non-cohesive, dry, compact.
3 - 7 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 10% non-plastic fines; 5% sub-angular cobbles, light brown (7.5YR 6/3), moderate CaCO <sub>3</sub> cementation in places (large plates excavated), clay fingering at 3 to 4 ft.; non-cohesive, dry, very dense.



### Samples:

1-3 ft. bag, bucket  
3-7 ft. bag, bucket  
BMI samples (0-1ft, 1-3ft, 3-7ft)

### Special Notes:

Refusal at 7 ft., hit bedrock (andesite).



# TEST PIT LOG: TP-6

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11978206.72 E: 871778.25

**Date:** 1/4/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 25% fine to coarse poorly graded gravel, 20% non-plastic fines; 35% sub-rounded to sub-angular cobbles, brown (10YR 4/3), friable, strong RXN with HCl; non-cohesive, dry, loose.
1 - 3 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 25% fine to coarse sub-rounded sand, 35% low plasticity fines; 15% sub-rounded to sub-angular cobbles (3-5"), very pale brown (10YR 7/3), blocky, CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry, compact.
3 - 5 ft.	CL/GC	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded gravel, 25% fine to coarse sub-rounded sand; 5% sub-rounded to sub-angular cobbles (3-5"), strong brown (7.5YR 5/6), blocky, clay fingering, CaCO <sub>3</sub> as masses and moderately cementation in places, large blocky plates excavated; non-cohesive, dry, stiff.
5 - 7 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-rounded, poorly graded, 20% fine to coarse sub-rounded sand, 15% medium plasticity fines; 15% sub-angular cobbles (3-7"), light brown (7.5YR 6/4); non-cohesive, dry, dense.
7 - 13 ft.	SM	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 25% med. plasticity fines; 45% sub-angular to angular cobbles and boulders (3-12"); light brown (7.5YR 6/4), CaCO <sub>3</sub> coatings on rocks; non-cohesive, dry, very dense.



### Samples:

- 1-3 ft. bag
- 3-5 ft. bag
- 5-7 ft. bag, bucket
- 7-13 ft. bag, bucket

### Special Notes:

At 7 feet excavator broke through large boulder.



# TEST PIT LOG: TP-7

Checked GM 1/29/2013

**Client:** THEMAC **Date:** 12/17/2012  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974576.20 E: 872338.77

**Lithology:**

Depth	USCS	Description
0 - 1.5 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% medium to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1.5 - 4 ft.	CH	CLAY and SAND, high plasticity, 45% medium to coarse poorly graded subrounded sand, 5% fine to coarse poorly graded gravels; reddish brown (5YR 4/4), blocky, some CaCO <sub>3</sub> masses and disseminated; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 20% low plasticity fines; trace sub-angular cobbles (3-5"), pinkish gray (7.5YR 7/2), moderate CaCO <sub>3</sub> cementation; non-cohesive, dry dense.
6 - 8 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, 10% non-plastic fines; 5% sub-angular cobbles (3-5"), pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> masses and disseminated; non-cohesive, dry, very dense.
8 - 10 ft.	GP	GRAVEL, fine, poorly graded, and SAND, poorly graded, fine to coarse, 5% non-plastic fines; trace cobbles (3-4"), brown (7.5YR 5/3), CaCO <sub>3</sub> coating on rocks; non-cohesive, dry, very dense.
10 - 12 ft.	GP-GM	SILTY GRAVEL, fine to coarse, poorly graded, and SAND fine to coarse, sub-rounded, 10% low plasticity fines; 10% sub-angular cobbles (4-5"), brown (7.5YR 5/3), CaCO <sub>3</sub> coating on rock fragments; non-cohesive, dry, dense.



**Samples:**  
 0-1.5 ft. bag } 0-4ft. Bucket  
 1.5-4 ft. bag }  
 4-6 ft. bag  
 6-10 ft. bag, bucket  
 10-12 ft. bag, bucket  
 BMI bag samples all layers

**Special Notes:**



# TEST PIT LOG: TP-8

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974106.85 E: 872357.94

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy SILTY CLAY, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace cobbles (3-6"), brown (7.5YR 4/2), disseminated CaCO <sub>3</sub> , platy; cohesive, moist, firm.
2 - 5 ft.	CI	sandy gravelly SILTY CLAY, high plasticity, 30% fine to coarse sand, 20% fine to coarse poorly graded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), CaCO <sub>3</sub> masses and weakly cemented in places, platy; cohesive, dry, stiff.
5 - 7 ft.	SC/CI	CLAYEY SAND, fine to coarse, sub-rounded, 45% medium plasticity fines, 5% fine to coarse poorly graded gravels; dark brown (7.5YR 3/4), CaCO <sub>3</sub> masses, platy; cohesive, dry, stiff.
7 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse poorly graded gravels; trace cobbles (3-4"), reddish brown (5YR 5/4), moderate CaCO <sub>3</sub> cementation, large plates excavated; cohesive, dry, hard.
13 - 16 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 5% sub-rounded cobbles (3-4"), light brown (7.5YR 6/3); CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, compact to dense.



### Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-7 ft. bag
- 7-13 ft. bag
- 13-16 ft. bag

### Special Notes:



# TEST PIT LOG: TP-9

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974362.19 E: 873288.54

**Date:** 12/17/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 45% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, firm.
2 - 6 ft.	SP	TAILINGS. poorly graded SAND, medium, sub-rounded, 5% non-plastic fines; pale yellow (2.5Y 7/3), no RXN with HCl; non-cohesive, dry, loose. Tailing thickness in pit is tapered east to west: lower depth 6 ft. (east) and 4 ft. (west).
6 - 8 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine poorly graded gravel, 15% medium plasticity fines; 5% sub-angular cobbles (3-4"); very pale brown (10YR 7/3), CaCO <sub>3</sub> masses; non-cohesive, dry, dense.
8 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 5% angular cobbles (3-6"), pale brown (10YR 7/3), CaCO <sub>3</sub> coatings on coarse fragments and disseminated; non-cohesive, dry, dense.
10 - 11 ft.	GM	sandy SILTY GRAVEL, poorly graded, fine, sub-rounded, 35% fine to coarse sand, 15% medium plasticity fines; 10% angular cobbles (3-4"), pinkish gray (7.5YR 6/2), moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GP-GM	sandy SILTY GRAVEL, poorly graded, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 10% medium plasticity fines; pinkish gray (7.5YR 6/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.



**Samples:**  
 6-8 ft. bag } 6-10 ft. bucket  
 8-10 ft. bag }  
 10-11 ft. bag } 10-14 ft. bucket  
 11-14 ft. bag }

BMI bag samples: 6-8 ft., 8-10ft, 10-11 ft.

**Special Notes:**  
 Reclaimed area on tailing dam.



# TEST PIT LOG: TP-10

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974364.43 E: 873777.16

**Date:** 12/17/2012

### Lithology:

Depth	USCS	Description
0 - 0.5 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% medium sub-angular gravel; trace sub-angular cobbles (3-6"), dark yellowish brown (10YR 4/4), friable, strong RXN with HCl; cohesive, slightly moist, soft.
0.5 - 3 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 40% high plasticity fines, 15% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-6"), dark brown (10YR 3/3), blocky, disseminated CaCO <sub>3</sub> ; cohesive, dry, firm.
3 - 6 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 7/4), platy, no RXN with HCl; non-cohesive, dry, compact.
6 - 12 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; pale yellow (2.5Y 8/4), no RXN with HCl; non-cohesive, dry, loose.
12 - 13 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 25% medium plasticity fines, 25% fine to coarse sub-rounded gravels; 15% sub-rounded cobbles (3-10"), pale brown (10YR 6/3), CaCO <sub>3</sub> masses and coatings on rock fragments, mixing with tailing at horizon contact; non-cohesive, dry, dense.



### Samples:

- 0.5-3 ft. bag
- 3-6 ft. bag, bucket
- 6-12 ft. bag, bucket
- 12-13 ft. bag, bucket

### Special Notes:

Reclaimed area on tailing dam.  
Stop at 13 feet due to limit of backhoe, but appear to be on top of a layer of more gravels.



# TEST PIT LOG: TP-11

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974375.04 E: 874235.34

**Date:** 12/17/2012

### Lithology:

Depth	USCS	Description
0 - 0.83 ft.	ML	FILL. CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded gravel; 5% sub-angular cobbles (3-10"), brown (10YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
0.83 - 5 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; light gray (2.5Y 7/2), platy, weak RXN with HCl; non-cohesive, dry, compact.
5 - 11 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; brownish yellow (10YR 6/8), no RXN with HCl; non-cohesive, dry, loose to compact.
11 - 13 ft.	SP	TAILING. poorly graded SAND, medium, rounded, 5% non-plastic fines; grayish brown (2.5Y 5/2), no RXN with HCl; non-cohesive, dry, loose.



### Samples:

- 0-1 ft. bag
- 1-5 ft. bag, bucket
- 5-11 ft. bag, bucket
- 11-13 ft. bag, bucket

### Special Notes:

Reclaimed area on tailing dam.  
Stop at 13 feet due to limit of backhoe.



# TEST PIT LOG: TP-12

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974885.17 E: 875173.88

**Date:** 1/2/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	ML	SILT and SAND, low plasticity, 45% poorly graded fine sand, 5% medium gravel; trace cobbles (4-6"), strong brown (7.5YR 4/6), blocky; cohesive, dry, firm.
1 - 3 ft.	CL	sandy SILTY CLAY, medium plasticity, 35% fine to coarse sub-rounded sand, 5% medium gravel; trace sub-rounded to sub angular cobbles (4-6"), pink (7.5 YR 7/3), clay fingering, CaCO <sub>3</sub> masses and cemented in places, blocky; cohesive, dry, stiff.
3 - 7 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 40% sub-rounded to sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, dense.
7 - 8 ft.	SM/GM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded; 20% low to medium plasticity fines; 10% cobbles (3-10"), pink (7.5YR 7/4), blocky, strong CaCO <sub>3</sub> cementation; non-cohesive, dry, very dense.
8 - 11 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% non-plastic fines; 5% cobbles (3-10"), pale brown (10YR 6/3), weak to moderate SiO <sub>2</sub> cementation, CaCO <sub>3</sub> trace masses and disseminated; non-cohesive, dry, dense.
11 - 13 ft.	SM	SILTY SAND, fine to coarse, sub-rounded and GRAVEL, fine to coarse, sub-rounded; 20% low plasticity fines; trace cobbles (3-4"), yellowish brown (10YR 5/4), platy, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, CaCO <sub>3</sub> visible in pores and disseminated; non-cohesive, dry,
13 - 15 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 10% low to non-plastic fines; 15% cobbles (3-10"), yellowish brown (10YR 5/4), CaCO <sub>3</sub> coatings on rock fragments; non-cohesive, dry, dense.



**Samples:**

BMI bag samples: 0-1 ft., 1-3 ft., 3-7 ft., 8-11 ft., 11-13 ft.

**Special Notes:**

Surface reworked by wind, small dunes around shrubs (*Flourensia cernua*, tarbush and *Prosopis glandulosa*, honey mesquite).





# TEST PIT LOG: TP-13

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974438.98 E: 875447.89

**Date:** 1/2/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	ML	SILT, non-plastic, 10% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), brown (7.5YR 4/4), friable; cohesive, dry, soft.
1 - 3 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light yellowish brown (10YR 6/4), blocky, clay fingering, CaCO <sub>3</sub> as masses and weakly cemented in places; cohesive, dry, soft.
3 - 5 ft.	CL	sandy SILTY CLAY, low to medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; light brown (7.5YR 6/4), blocky, CaCO <sub>3</sub> as masses and disseminated; cohesive, dry, firm.
5 - 8 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse sub-rounded gravel; trace cobbles (3-6"), light brown (7.5YR 6/4), strong angular blocky, CaCO <sub>3</sub> along pores, weak RXN with HCl; cohesive, dry, stiff.
8 - 10 ft.	SM	SILTY SAND, fine to coarse, subrounded, and GRAVEL, fine to coarse, sub-rounded, 15% low plasticity fines; 15% sub-rounded to sub-angular cobbles and boulders (3-15"), brown (7.5YR 5/4), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
10 - 18 ft.	GW	sandy GRAVEL, fine to coarse, sub-rounded, 25% fine to coarse sub-rounded sand, 10% non-plastic fines; 30% sub-rounded to sub-angular cobbles and boulders (3-20"), dark yellowish brown (10YR 4/4), weak RXN with HCl; non-cohesive, dry, very, dense.



### Samples:

- 5-8 ft. bag, bucket
- 5-10 ft. bag, bucket
- 10-18 ft. bag, bucket

### Special Notes:



# TEST PIT LOG: TP-14

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974366.80 E: 874917.98

**Date:** 1/2/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	CI	gravelly sandy SILTY CLAY, high plasticity, 20% fine to coarse sub-angular gravel, 15% fine to coarse sub-rounded sand; 20% sub-angular cobbles (3-12"); dark reddish brown (5YR 3/4), blocky, no RXN with HCl; cohesive, dry, firm.
1 - 4 ft.	CL	gravelly sandy SILTY CLAY, moderate plasticity, 25% fine to coarse sub-angular gravel, 25% fine to coarse sub-rounded sand; 15% sub-angular cobbles (3-12"); pink (7.5YR 7/3), blocky, CaCO <sub>3</sub> masses and weak cementation in places, strong RXN with HCl clay fingering; cohesive, dry, firm.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 15% fine to coarse poorly graded gravels; trace cobbles (3"), pinkish white (7.5YR 8/2), strong CaCO <sub>3</sub> cementation (large plates excavated); non-cohesive, dry, dense.
7 - 12 ft.	ML	sandy SILT, non-plastic, 20% fine to medium poorly graded sand, trace fine poorly graded gravels; brown (7.5YR 5/4), platy, CaCO <sub>3</sub> lining pores and some masses, weak RXN with HCl; cohesive, dry, soft.
12 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 15% non-plastic fines; 15% sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16.5 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sand, 15% non-plastic fines; trace sub-angular cobbles (3-6"), pinkish white (7.5YR 8/2), weak SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rocks, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**

- 0-1 ft. bag
- 1-4 ft. bag
- 4-7 ft. bag
- 7-12 ft. bag
- 12-14 ft. bag
- 14-16.5 ft. bag

**Special Notes:**

Offset pit location approximately 25 feet to the west to keep disturbance on tracked road.



# TEST PIT LOG: TP-15

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973832.93 E: 874871.54

**Date:** 12/20/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse sub-angular gravels; 5% sub-angular cobbles (3-10"), pink (7.5 YR 7/4), blocky, strong RXN with HCl; cohesive, dry, firm.
4 - 8 ft.	CI	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to medium poorly graded gravel; light reddish brown (5YR 6/4), angular blocky (breaking to fine aggregates), CaCO <sub>3</sub> nodules and masses; cohesive, dry, firm.
8 - 10 ft.	CH	sandy CLAY, high plasticity, 15% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/4), angular blocky (breaking to gravel sized aggregates), weak RXN with HCl, clay pressure faces; cohesive, moist, firm.
10 - 20 ft.	CH	CLAY, high plasticity, 5% fine poorly graded sand, trace gravels; dark reddish brown (2.5YR 3/3), angular blocky (rock structure), weak RXN with HCl, clay pressure faces; cohesive, moist, stiff.



**Samples:**

8-10 ft. bag, bucket  
10-20 ft. bag, bucket

**Special Notes:**

Excavator leaves slick sidewalls at 8+ feet. Clays formed in place (not illuvial) from weathering primary minerals.



# TEST PIT LOG: TP-16

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973732.56 E: 875481.72

**Date:** 12/20/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 30% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; 10% sub-angular cobbles (3-6"), brown (7.5YR 4/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 4 ft.	ML	sandy CLAYEY SILT, low plasticity, 20% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravels; trace sub-angular cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, stiff.
4 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% non-plastic fines, 20% fine poorly graded sub-angular gravels; brown (7.5YR 5/3), friable, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
7 - 10 ft.	ML	sandy SILT, non-plastic, 30% fine to coarse sub-angular sand, 5% fine poorly graded sub-angular gravels; light brown (7.5YR 6/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl, thin layer (<1ft) of moderate cementation; non-cohesive, dry, hard.
10 - 17 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 20% sub-rounded cobbles (3-12"); brown (7.5YR 5/3), weak to strong CaCO <sub>3</sub> cementation (stratified), weak to strong RXN with HCl; non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-10 ft. bag, bucket
- BMI bag samples same as above

### Special Notes:

10 to 17 foot interval has varying degrees of CaCO<sub>3</sub> cementation (none to strong), but grouped together due to particle size similarities.



# TEST PIT LOG: TP-17

Checked GM 1/29/2013

**Client:** THEMAC **Date:** 12/18/2012  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973205.01 E: 873937.67

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	ML	sandy SILT, low plasticity, 40% fine to coarse sub-rounded sand, trace fine poorly graded gravels; trace sub-angular cobbles (3-6"), yellowish brown (10YR 5/4), platy, weak RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	CH	sandy CLAY, high plasticity, 35% fine to coarse poorly graded sand, 5% fine to coarse poorly graded gravels; trace cobbles (3"), reddish brown (5YR 4/4), blocky, disseminated CaCO <sub>3</sub> and masses, strong RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 5% fine to coarse poorly graded gravel; trace cobbles (3-4"), pink (7.5YR 7/3), blocky, moderate CaCO <sub>3</sub> cementation and masses, strong RXN with HCl; cohesive, dry, very stiff.
4 - 6 ft.	SM	gravelly SILTY SAND, fine to coarse, poorly graded, 25% low plasticity fines, 25% fine poorly graded gravels; trace cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> masses and coating rock fragments, strong RXN with HCl; cohesive, dry, firm.
6 - 14 ft.	GW	sandy GRAVEL, fine to coarse, poorly graded, 40% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), stratified, thickly bedded, some weak CaCO <sub>3</sub> cementation at 8 feet moderate cementation at 14 feet, CaCO <sub>3</sub> masses, strong RXN with HCl; non-cohesive, dry, dense



**Samples:**

0-2 ft. bag  
 2-4 ft. bag  
 4-6 ft. bag  
 6-14 ft. bag  
 BMI bag samples: 0-2 ft., 2-4 ft., 4-6 ft., 6-10 ft.

**Special Notes:**

Disturbed surface, pit located in depression on tailing dam (cow lay-down area). Salt cedar (*Tamarix chinensis*) and seep willow (*Baccharis salicina*) stand.



# TEST PIT LOG: TP-18

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973182.90 E: 874892.73

**Date:** 12/20/2012

**Lithology:**

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 2 ft.	GM/SM	sandy SILTY GRAVEL, poorly graded, fine to medium, sub-angular, 25% fine to medium poorly graded sub-rounded sand, 45% low plasticity fines; trace sub-angular cobbles (3-4"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; trace sub-angular cobbles (3-4"), light brown (7.5YR 6/3), blocky, moderate CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse poorly graded gravel, 30% low plasticity fines; pink (7.5YR 7/3), blocky, moderate to weak CaCO <sub>3</sub> cementation in places, disseminated, and coatings on rocks; non-cohesive, dry, dense.
5 - 7 ft.	SM/GM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded gravel, 25% low plasticity fines; reddish brown (5YR 5/3), large plates excavated, moderate SiO <sub>2</sub> cementation, strong reaction with HCl; non-cohesive, dry, dense.
7 - 9 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded gravel, 30% low plasticity fines; reddish brown (5YR 5/4), large plates excavated, strong SiO <sub>2</sub> cementation, weak reaction with HCl; non-cohesive, dry, very dense.
9 - 15 ft.	SP	SAND, poorly graded, medium, sub-rounded, 5% fine poorly graded gravel, 5% non-plastic fines; reddish brown (5YR 5/4), large plates excavated, moderate SiO <sub>2</sub> cementation, weak reaction with HCl; non-cohesive, dry, very dense.



**Samples:**

7-9 ft. bag, bucket  
 9-15 ft. bag, bucket

**Special Notes:**



# TEST PIT LOG: TP-19

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973045.18 E: 875451.64

**Date:** 12/19/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	GC	Disturbed/FILL. sandy CLAYEY GRAVEL, fine to coarse, sub-rounded, 45% high plasticity fines, 20% fine to coarse sub-rounded sand; 15% sub-rounded cobbles (3-6"), reddish brown (5YR 4/4), blocky, no RXN with HCl; cohesive, moist, firm.
2 - 3 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, poorly graded, sub-rounded, 40% high plasticity fines, 30% fine to coarse sub-rounded sand; 5% sub-rounded cobbles (3-6"), yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, dry, hard.
3 - 5 ft.	SW	SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% non-plastic fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), strong CaCO <sub>3</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, subangular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO <sub>3</sub> as masses, disseminated and weak cementation in places; non-cohesive, dry, dense.
10 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% medium poorly graded gravel, 25% low to medium plasticity fines; trace cobbles (3-10"), white (7.5YR 8/1), blocky, CaCO <sub>3</sub> as masses, disseminated and moderate cementation in places; non-cohesive, dry, dense.
11 - 14 ft.	GM-GC	sandy GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand; 25% low to medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 5/3), CaCO <sub>3</sub> as masses, disseminated and strong cementation in places; non-cohesive, dry, dense.



### Samples:

None.

### Special Notes:

stratified gravels and sands, thickly bedded, from 5 to 14 feet.



# TEST PIT LOG: TP-20

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972549.21 E: 875734.91

**Date:** 12/19/2012

**Lithology:**

Depth	USCS	Description
0 - 0.5 ft.	ML	sandy SILT, non-plastic, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; brown (7.5YR 4/2), friable, no RXN with HCl; cohesive, dry, soft.
0.5 - 2 ft.	CH	CLAY and SAND, high plasticity, 40% fine to coarse sub-rounded sand, 5% fine to medium poorly graded gravels; yellowish red (5YR 4/6), blocky, no RXN with HCl; cohesive, moist, stiff.
2 - 4 ft.	SC-SP	gravelly CLAYEY SAND, fine to coarse, poorly graded, 30% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 10% sub-rounded cobbles (3-7"), brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated; strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% medium plasticity fines, 15% fine to coarse poorly graded sub-rounded gravels; trace sub-rounded cobbles (3"), white (7.5YR 8/1), blocky, weakly cemented (CaCO <sub>3</sub> ); non-cohesive, dry, very dense.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-rounded gravels, 15% medium plasticity fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry, dense.
7 - 11 ft.	GP	GRAVEL, fine to coarse, poorly graded, and SAND, fine to coarse, sub-round SAND, 5% non-plastic fines; 5% sub-rounded cobbles (3-6"), brown (7.5YR 5/3), weakly cemented (CaCO <sub>3</sub> ); non-cohesive, dry, dense.
11 - 18.5 ft.	SW-SM	SILTY SAND fine to coarse, sub-rounded, and GRAVEL, fine to coarse, sub-rounded, 15% non-plastic fines; 10% sub-rounded cobbles (3-6"), grayish brown (10YR 5/2), blocky, CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments; non-cohesive, dry, dense to compact.



**Samples:**  
 0-2 ft. bag } 0-4 ft. bucket  
 2-4 ft. bag }  
 4-5 ft. bag } 4-7 ft. bucket  
 5-7 ft. bag }  
 7-11 ft. bag, bucket  
 11-18.5 ft. bag, bucket

**Special Notes:**  
 Surface disturbed.





# TEST PIT LOG: TP-21

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972274.72 E: 875755.12

**Date:** 12/19/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 45% fine to coarse sub-rounded sand, 5% coarse poorly graded sub-angular, gravels; 5% sub-angular cobbles (3-4"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC/SM	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% medium plasticity fines, 30% medium poorly graded sub-angular gravels; trace cobbles (3"), white (7.5YR 8/1), blocky, weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 25% fine to medium poorly graded sub-angular gravels; trace cobbles (3"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 7 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to medium poorly graded sub-angular gravels, 25% medium plasticity fines; 5% sub-rounded cobbles (3-5"), brown (7.5YR 4/3), disseminated CaCO <sub>3</sub> , masses and coatings on rock fragments, strong RXN with HCl; cohesive, dry, stiff.
7 - 11 ft.	GC	sandy CLAYEY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 20% medium plasticity fines; 10% sub-angular cobbles (3-6"), brown (7.5YR 5/4), weak RXN with HCl, moderate SiO <sub>2</sub> cementation; non-cohesive, dry, very dense.
11 - 14 ft.	GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, sub-angular, 40% fine to coarse sub-rounded sand, 15% low plasticity fines; 5% sub-rounded cobbles (3-6"); brown (7.5YR 5/4), weak RXN with HCl, weak SiO <sub>2</sub> cementation; non-cohesive, dry, dense.
14 - 18 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 20% low plasticity fines; trace cobbles (3-4"); light brown (7.5YR 6/4), large plates excavated, weak RXN with HCl, weak SiO <sub>2</sub> cementation; non-cohesive, dry, dense.



**Samples:**

- 7-11 ft. bag, bucket
- 11-14 ft. bag, bucket
- 14-18 ft. bag, bucket
- BMI bag samples: 7-11 ft., 11-14 ft., 14-18 ft.

**Special Notes:**

CaCO<sub>3</sub> masses and cementation confined to upper layers (2-7 ft.). Disseminated CaCO<sub>3</sub> and coatings on coarse fragments at 7+ feet. SiO<sub>2</sub> cementation/conglomerate at 7-18 feet.



# TEST PIT LOG: TP-22

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972597.58 E: 875051.00

**Date:** 12/20/2012

**Lithology:**

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse sub-angular gravel; 5% cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% low to medium plasticity fines, 25% fine to coarse sub-angular gravel; 5% cobbles (3-5"), pinkish white (7.5YR 8/2), weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SW	gravelly SAND, fine to coarse, sub-rounded, 35% fine to coarse sub-angular gravel, 10% non-plastic fines; 10% cobbles (3-12"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
5 - 8 ft.	GW	sandy GRAVEL, fine to coarse, sub-angular, 40% fine to coarse sub-rounded sand, 10% non-plastic fines; 20% cobbles and boulders (3-15"), brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, compact.
8 - 11 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 35% fine to coarse sub-rounded sand, 25% medium plasticity fines; 20% cobbles and boulders (3-15"), reddish brown (5YR 4/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
11 - 13 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravels, 25% medium plasticity fines; trace cobbles (3-4"), reddish brown (5YR 5/4), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, 10% fine to medium poorly graded gravels, 10% low plasticity fines; yellowish red (5YR 5/6), weak RXN to HCl, moderate cementation (SiO <sub>2</sub> ), large plates excavated; non-cohesive, dry, very dense.



**Samples:**

None.

**Special Notes:**

Stratified gravels at 5 to 11 feet, thickly bedded.  
 Disturbed surface; A horizon has been removed.



# TEST PIT LOG: TP-23

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972621.41 E: 874709.99

**Date:** 12/20/2012

**Lithology:**

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 2 ft.	ML	sandy CLAYEY SILT, low plasticity, 35% fine to medium poorly graded sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; trace sub-angular cobbles (3-5"), brown (7.5YR 4/3), blocky, strong RXN with HCl; cohesive, dry, soft.
2 - 3 ft.	ML	CLAYEY SILT and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/3), blocky, weak CaCO <sub>3</sub> cementation, strong RXN with HCl; cohesive, dry, very stiff.
3 - 5 ft.	ML-SM	CLAYEY SILT and SAND, low to medium plasticity, 45% fine to coarse sub-rounded sand, 5% fine to medium poorly graded sub-angular gravel; light brown (7.5YR 6/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, stiff.
5 - 8 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low to non-plastic fines, 5% fine to medium poorly sub-angular graded gravel; pink (7.5YR 7/4), moderate CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
8 - 11 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 25% low plasticity fines, 25% fine to medium poorly graded sub-angular gravel; pink (7.5YR 7/4), strong CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.
11 - 12 ft.	GM	sandy SILTY GRAVEL, fine to coarse, sub-angular, 30% low plasticity fines, 30% fine to coarse sub-rounded sand; 5% cobbles (3-5"), brown (7.5YR 5/2), strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, very dense.



**Samples:**

None.

**Special Notes:**

very hard digging at 10 feet. Refusal at 12 feet.



# TEST PIT LOG: TP-24

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972670.75 E: 873699.23

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 3 ft.	CL	SILTY CLAY and SAND, medium plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse poorly graded sub-rounded gravel; 20% cobbles (4-10"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, firm.
3 - 5 ft.	SC	CLAYEY SAND, fine to coarse, sub-rounded sand, 30% medium plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; 5% sub-angular cobbles (3-5"); brown (7.5YR 5/4), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
5 - 10 ft.	SM/SC	SILTY SAND, fine to coarse, well graded, sub-rounded, 25% medium plasticity fines, 10% fine to medium poorly graded sub-angular gravel; trace cobbles (3-5"); light brown (7.5YR 6/3), large blocky plates excavated, CaCO <sub>3</sub> as masses and weakly cemented (SiO <sub>2</sub> /CaCO <sub>3</sub> ) in places, strong RXN with HCl; non-cohesive, dry, dense.
10 - 14 ft.	SW-SM	SILTY SAND, fine to coarse, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; 15% sub-angular cobbles; brown (7.5YR 5/3), CaCO <sub>3</sub> coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.
14 - 16 ft.	GP-GM	sandy SILTY GRAVEL, fine to coarse, poorly graded, 30% fine to coarse well graded sand, 10% low plasticity fines; 20% sub-angular cobbles (3-10"), brown (7.5YR 5/3), CaCO <sub>3</sub> coatings on rock fragments and masses, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**  
0-3 ft. bag  
3-5 ft. bag, bucket  
5-10 ft. bag, bucket  
10-14 ft. bag, bucket  
14-16 ft. bag, bucket  
BMI samples all layers (bag samples)

**Special Notes:**  
Disturbed surface.



# TEST PIT LOG: TP-25

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972408.78 E: 872794.31

**Date:** 12/13/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	SC	FILL. gravelly CLAYEY SAND, fine to coarse, sub-rounded, 35% fine to medium poorly graded sub-rounded gravel, 20% medium plasticity fines; 5% sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky; cohesive, dry, firm.
2 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine to coarse poorly graded gravel, 10% low plasticity fines; 20% cobbles and boulders (3-15"), light brown (7.5YR 5/4), CaCO <sub>3</sub> as masses and disseminated; non-cohesive, dry,
5 - 6 ft.	GP	sandy GRAVEL, fine to coarse, poorly graded, sub-angular, 25% fine to coarse sub-rounded graded sand, 5% non-plastic fines; 15% cobbles and boulders (3-12"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.
6 - 7 ft.	GP	sandy GRAVEL, fine to coarse, poorly grades, sub-angular, 30% fine to coarse sub-rounded sand, 5% non-plastic fines; 15% cobbles (3-10"), light brown (7.5YR 6/3), large blocky plates excavated, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag
- 2-5 ft. bag
- 5-6 ft. bag
- 6-7 ft. bag

BMI sample: 2-5 ft.

### Special Notes:

Pit located in old borrow area. Hard to dig with backhoe at 5+ feet.



# TEST PIT LOG: TP-26

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973262.64 E: 872782.60

**Date:** 12/13/2012

### Lithology:

Depth	USCS	Description
0 - 1 ft.	SM	FILL. gravelly SILTY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse sub-rounded gravel; 5% sub-angular cobbles (3-4"), brown (10YR 5/3), friable; cohesive, dry, soft.
1 - 3 ft.	CL/SM	gravelly sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 20% fine to coarse sub-rounded gravel; trace cobbles (3-4"), brown (7.5YR 4/3), blocky, CaCO <sub>3</sub> as masses and disseminated; cohesive, dry, stiff.
3 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-rounded graded gravel, 15% low plasticity fines; 10% sub-angular cobbles (3-6"), light brown (7.5YR 6/4), large blocky plates excavated, moderate SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.
4 - 5 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 20% fine to coarse sub-angular gravel, 15% low plasticity fines; 5% sub-angular cobbles (3-5"), light brown (7.5YR 6/4), large blocky plates excavated, strong SiO <sub>2</sub> /CaCO <sub>3</sub> cementation (conglomerate); non-cohesive, dry, very dense.



### Samples:

- 0-3 ft. bag
- 3-4 ft. bag
- 4-5 ft. bag

### Special Notes:

Pit located in old borrow area. Hard to dig with backhoe; refusal at 5 feet.



# TEST PIT LOG: TP-27

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11972823.18 E: 871169.18

**Date:** 12/19/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	SM	SILTY SAND, fine to coarse, sub-rounded, 20% low plasticity fines, 10% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-5"), dark brown (10YR 3/3), friable, strong RXN with HCl; cohesive, moist, soft.
2 - 3 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded sand, 40% medium plasticity fines, 20% fine to coarse poorly graded sub-angular gravel; 10% sub-angular cobbles and boulders (3-20"); brown (7.5YR 4/3), blocky, CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, firm.
3 - 7 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-angular, 45% fine to coarse sub-rounded sand, 5% non-plastic fines; 5% sub-angular cobbles (3-10"), pinkish gray (7.5YR 7/2), CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl, moderate SiO <sub>2</sub> cementation at 5 feet; non-cohesive, dry, dense.
7 - 13 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), pinkish gray (7.5YR 6/2), moderate SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.
13 - 14 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 10% sub-angular cobbles (3-10"), pinkish gray (7.5YR 6/2), strong SiO <sub>2</sub> cementation (conglomerate), CaCO <sub>3</sub> coatings on rock fragments, weak RXN with HCl; non-cohesive, dry, very dense.



### Samples:

- 0-2 ft. bag, bucket
- 2-3 ft. bag, bucket
- 3-7 ft. bag, bucket
- 7-13 ft. bag, bucket
- 13-14 ft. bag
- BMI samples all layers (bag samples)

### Special Notes:

Hard digging due to oversize at 7 feet and cemented conglomerate at 13 feet.



# TEST PIT LOG: TP-28

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11973129.48 E: 871528.89

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 2 ft.	ML	CLAYEY SILT and SAND, low plasticity, 40% fine to coarse sub-rounded sand, 10% fine to coarse sub-rounded gravel; 10% sub-angular cobbles (3-8"), brown (7.5YR 4/2), blocky, weak RXN with HCl; cohesive, dry, soft.
2 - 4 ft.	SM/CL	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% medium plasticity fines, 30% fine to coarse sub-rounded gravel; trace sub-angular cobbles (3-8"), pinkish gray (7.5YR 6/2), blocky, weak CaCO <sub>3</sub> cementation in places and masses strong RXN with HCl; cohesive, dry, firm.
4 - 6 ft.	GP	GRAVEL, fine to coarse, poorly graded, sub-rounded and SAND, fine to coarse, poorly graded, sub-rounded, 10% low plasticity fines; trace cobbles (3-8"), brown (7.5YR 5/2), CaCO <sub>3</sub> coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.
6 - 9 ft.	GW	GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 15% cobbles and boulders (3-20"), light brown (7.5YR 6/4), moderate SiO <sub>2</sub> cementation, very weak RXN with HCl; non-cohesive, dry, very
9 - 14.5 ft.	GM	SILTY GRAVEL, fine to coarse, sub-rounded, and SAND, fine to coarse, sub-rounded, 15% low plasticity fines; 20% sub-angular cobbles (3-8"), light brown (7.5YR 6/4), strong SiO <sub>2</sub> cementation (conglomerate), very weak RXN with HCl; non-cohesive, dry, very dense.



**Samples:**  
None.

**Special Notes:**  
Difficult to excavate at 9 feet. Archaeologist present during excavation.





# TEST PIT LOG: TP-29

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974098.42 E: 871178.07

**Date:** 12/18/2012

**Lithology:**

<i>Depth</i>	<i>USCS</i>	<i>Description</i>
0 - 1 ft.	CL	sandy SILTY CLAY, medium plasticity, 30% fine to coarse sub-rounded sand, 5% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-12"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
1 - 2 ft.	CI	gravelly sandy SILTY CLAY, medium to high plasticity, 25% fine to coarse sub-angular gravel; 20% medium to coarse poorly graded sub-rounded sand; 5% cobbles (3-12"), reddish brown (5YR 4/4), blocky, disseminated CaCO <sub>3</sub> , strong RXN with HCl; cohesive, dry, stiff.
2 - 4 ft.	SC	gravelly CLAYEY SAND, fine to coarse, sub-rounded, 30% medium plasticity fines, 15% fine to coarse sub-angular gravels; 5% cobbles (3-6"), pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> as masses and disseminated, strong RXN with HCl; cohesive, dry, very stiff.
4 - 7 ft.	SP	gravelly SAND, fine to coarse, poorly graded, sub-rounded, 25% fine to coarse poorly graded gravels, 5% non-plastic fines; trace sub-angular cobbles (3-6"), light brown (7.5YR 6/3), CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, compact.
7 - 12 ft.	SW-GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 10% non-plastic fines; 10% sub-angular cobbles (3-8"), light brown (7.5YR 6/3), strong SiO <sub>2</sub> cementation (conglomerate) at 11 feet, strong RXN with HCl, CaCO <sub>3</sub> as masses, disseminated and coatings on rock fragments; non-cohesive, dry, very dense.



**Samples:**

- 0-2 ft. bag, bucket
- 2-4 ft. bag
- 4-7 ft. bag, bucket
- 7-12 ft. bag, bucket

**Special Notes:**

Refusal at 12 feet due to cementation and oversized.



# TEST PIT LOG: TP-30

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11974680.06 E: 871571.56

**Date:** 12/18/2012

### Lithology:

Depth	USCS	Description
0 - 2 ft.	CL	sandy gravelly SILTY CLAY, medium plasticity, 25% fine to coarse sub-rounded sand, 20% fine to coarse poorly graded sub-angular gravel; trace cobbles (3-10"), dark brown (7.5YR 3/3), friable, strong RXN with HCl; cohesive, dry, firm.
2 - 4 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 40% fine to coarse poorly graded sub-angular gravel, 15% low plasticity fines; 10% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO <sub>3</sub> and masses, strong RXN with HCl; non-cohesive, dry, compact.
4 - 5 ft.	SW	gravelly SILTY SAND, fine to coarse, sub-rounded, 35% fine poorly graded sub-angular gravels, 5% non-plastic fines; pinkish gray (7.5YR 6/2), CaCO <sub>3</sub> as masses and coatings on gravels, moderate SiO <sub>2</sub> cementation, strong RXN with HCl; non-cohesive, dry, dense.
5 - 12 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, sub-rounded, 5% non-plastic fines; 30% sub-angular cobbles and boulders (3-20"), light brown (7.5YR 6/3), disseminated CaCO <sub>3</sub> and coatings on rock fragments, strong RXN with HCl; non-cohesive, dry, dense.



**Samples:**  
None.

**Special Notes:**



# TEST PIT LOG: TP-31

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11975597.72 E: 872172.62

**Date:** 1/3/2013

**Lithology:**

Depth	USCS	Description
0 - 1 ft.	CL	gravelly sandy CLAYEY SILT, medium to high plasticity, 30% fine to coarse sub-rounded sand, 25% fine to coarse sub-angular gravel; 10% sub-angular to sub-rounded cobbles (3-6"), brown (7.5YR 5/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 2 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 30% medium plasticity fines; 15% cobbles (3-9"), brown (7.5YR 5/3), blocky, CaCO <sub>3</sub> masses, disseminated and weak cementation in places, clay fingering, strong RXN with HCl; cohesive, dry, stiff.
2 - 5 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, weak SiO <sub>2</sub> cementation, weak RXN with HCl, CaCO <sub>3</sub> coatings on rocks; non-cohesive, dry, dense.
5 - 8 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 5% non-plastic fines; trace cobbles (3-5"), light brown (7.5YR 6/3), platy, mod. SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, very dense.
8 - 13 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), brown (7.5YR 5/4), platy, weak SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, dense.
13 - 16 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, poorly graded, sub-angular, 10% low plasticity fines; 5% cobbles (3-6"), light brown (7.5YR 6/3), platy, strong SiO <sub>2</sub> cementation, no RXN with HCl; non-cohesive, dry, very dense.



**Samples:**  
 1-2 ft. bag  
 2-5 ft. bag  
 5-8 ft. bag  
 8-13 ft. bag  
 13-16 ft. bag  
 BMI bag samples all layers

**Special Notes:**  
 Surface disturbed- placer mining location. Pit located in drainage.



# TEST PIT LOG: TP-32

Checked GM 1/29/2013

**Client:** THEMAC  
**Project:** Copper Flat  
**Project No.:** 103-92557  
**Location:** Sierra County, NM  
**NAD 83:** N: 11975136.61 E: 872876.80

**Date:** 1/3/2013

### Lithology:

Depth	USCS	Description
0 - 1 ft.	CL-ML	sandy CLAYEY SILT, medium plasticity, 30% fine to coarse sub-rounded sand, 10% fine to coarse sub-angular gravel; trace sub-angular cobbles (3-6"), brown (7.5YR 4/4), blocky, strong RXN with HCl; cohesive, dry, soft.
1 - 3 ft.	SM	gravelly SILTY SAND, fine to coarse, sub-rounded, 30% fine to coarse sub-angular gravel, 25% medium plasticity fines; 20% cobbles (3-10"), pinkish gray (7.5YR 7/2), blocky, CaCO <sub>3</sub> masses, coatings on rock fragments and weak cementation in places, stron RXN with HCl; non-cohesive, dry, compact.
3 - 5 ft.	SM	SILTY SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 15% low plasticity fines; 5% cobbles (3-4"), light brown (7.5YR 6/3), weak SiO <sub>2</sub> /CaCO <sub>3</sub> cementation, strong RXN to HCl; non-cohesive, dry, dense.
5 - 10 ft.	GW	GRAVEL, fine to coarse, sub-angular, and SAND, fine to coarse, poorly graded, sub-rounded sand, 10% low plasticity fines; 20% cobbles and boulders (3-12"), pinkish gray (7.5YR 6/2), weak SiO <sub>2</sub> cementation, CaCO <sub>3</sub> coatings on rock fragments, weak RXN to HCl; non-cohesive, dry, very dense.
10 - 14 ft.	SP	SAND, fine to coarse, poorly graded, sub-rounded, and GRAVEL, fine to coarse, sub-angular, 5% low plasticity fines; 15% cobbles (3-8"), light brown (7.5YR 6/3), large platy blocks excavated, strong SiO <sub>2</sub> cementation, CaCO <sub>3</sub> masses, weak RXN to HCl; non-cohesive, dry, very dense.



**Samples:**  
3-5 ft. bag, bucket  
5-10 ft. bag, bucket  
10-14 ft. bag, bucket

**Special Notes:**  
Hard

**APPENDIX A.2  
DRILL HOLE LOGS**



# REPORT OF BOREHOLE: SOIL KEY

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Copper Flat  
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N , E  
 CHECKED: DP DATE: 2/21/13 ELEVATION: ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer, Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
5										<p><u>Blows Per Six Inches</u>            Number of sample hammer blows required to drive the sampler six inches, or recorded number of blows to drive the sampler the specified distance (e.g. 50/4" = 50 hammer blows to drive the sampler four inches).</p> <p><u>Blows Per Foot</u>            Number of sample hammer blows required to drive the sampler twelve inches. Resolved using the final twelve inches of the sample or the amount of penetration upon sample refusal (50 blow counts).</p>				
15			SS1		4	11	17 / 18			<p><u>Sample Types</u>            Standard Penetration Test - Full penetration with 17 of 18 inches recovered.</p>				
20			SS2		7	R	7 / 10			<p>Standard Penetration Test - Refusal at 10 inches, 7 of 10 inches recovered.</p>				
20			AUGER3							<p>Auger Bag/Bulk Sample - Bulk grab from auger cuttings to become a collective bag sample over an interval.</p>				
25														
30														
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT.GPJ TEMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:7/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations

## UNIFIED SOIL CLASSIFICATION (ASTM D 2487-00)

MATERIAL TYPES	CRITERIA FOR ASSIGNING SOIL GROUP NAMES AND GROUP SYMBOLS USING LABORATORY TESTS			GROUP SYMBOL	SOIL GROUP NAMES & LEGEND	
COARSE-GRAINED SOILS >50% OF COARSE FRACTION RETAINED ON NO. 200 SIEVE	GRAVELS  >50% OF COARSE FRACTION RETAINED ON NO. 4. SIEVE	CLEAN GRAVELS <5% FINES	$C_u > 4$ AND $1 < C_c < 3$	GW	WELL-GRADED GRAVEL	If soil contains >15% sand, add "with sand"
			$C_u > 4$ AND/OR $1 > C_c > 3$	GP	POORLY-GRADED GRAVEL	
		GRAVELS WITH FINES >12% FINES	FINES CLASSIFY AS ML OR CL	GM	SILTY GRAVEL	
			FINES CLASSIFY AS CL OR CH	GC	CLAYEY GRAVEL	
	SANDS  >50% OF COARSE FRACTION PASSES ON NO. 4. SIEVE	CLEAN SANDS <5% FINES	$C_u > 6$ AND $1 < C_c < 3$	SW	WELL-GRADED SAND	If soil contains >15% gravel, add "with gravel"
			$C_u > 6$ AND/OR $1 > C_c > 3$	SP	POORLY-GRADED SAND	
SANDS AND FINES >12% FINES		FINES CLASSIFY AS ML OR MH	SM	SILTY SAND		
		FINES CLASSIFY AS CL OR CH	SC	CLAYEY SAND		
FINE-GRAINED SOILS >50% PASSES NO. 200 SIEVE	SILTS AND CLAYS  LIQUID LIMIT <50		CL	LEAN CLAY	If soil contains coarse-grained soil from 15% to 29%, add "with sand" or "with gravel" for whichever type is prominent, or for >30%, add "sandy" or "gravelly"	
			ML	SILT		
			OL	ORGANIC CLAY OR SILT		
	SILTS AND CLAYS  LIQUID LIMIT >50		CH	FAT CLAY		
			MH	ELASTIC SILT		
			OH	ORGANIC CLAY OR SILT		
HIGHLY ORGANIC SOILS	PRIMARILY ORGANIC MATTER, DARK IN COLOR, AND ORGANIC ODOR	PT	PEAT			

$$C_u = \frac{D_{60}}{D_{10}} \quad C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$$

Gravels or sands with 5% to 12% fines require dual symbols (GW-GM, GW-GC, GP-GM, GP-GC, SW-SM, SW-SC, SP-SM, SP-SC) and add "with clay" or "with silt" to group name. If fines classify as CL-ML for GM or SM, use dual symbol GC-GM or SC-SM.

### DESCRIPTIVE TERMINOLOGY FOR PERCENTAGES (ASTM D 2488-00)

DESCRIPTIVE TERMS	RANGE OF PROPORTION
TRACE	0 - 5%
FEW	5 - 10%
LITTLE	15 - 25%
SOME	30 - 45%
MOSTLY	50 - 100%

### LABORATORY TEST ABBREVIATIONS

<b>AL</b> Atterberg Limits	<b>HY</b> Hydrometer	<b>SG</b> Specific Gravity
<b>CI</b> Chloride Content	<b>PT</b> Proctor	<b>SP</b> Swell Potential
<b>CO</b> Consolidation	<b>pH</b> Soil pH	<b>UC</b> Unconfined Compression
<b>CP</b> Collapse Potential	<b>RS</b> Restivity	<b>UU</b> Triaxial Unconsolidated, Undrained
<b>CU</b> Triaxial Consolidated Undrained	<b>RV</b> R-Value	
<b>DD</b> Dry Density	<b>SA</b> Sieve Analysis	
<b>DS</b> Direct Shear	<b>SC</b> Soluble Sulfate Content	

### CRITERIA FOR DESCRIBING MOISTURE CONDITION (ASTM D 2488-00)

DRY	Absence of moisture, dusty, dry to the touch
MOIST	Damp but no visible water
WET	Visible free water, usually soil is below water table

### RELATIVE DENSITY / CONSISTENCY ESTIMATE USING STANDARD PENETRATION TEST (SPT) VALUES

COHESIONLESS SOILS (GRAVEL, SAND, NONPLASTIC SILT)			COHESIVE SOILS (PLASTIC SILT, CLAY)		
DENSITY	N <sub>1</sub> (BLOWS /FOOT)*	RELATIVE DENSITY (%)	CONSISTENCY	N <sub>1</sub> (BLOWS /FOOT)*	COMPRESSIVE STRENGTH (TSF)
VERY LOOSE	0 - 4	0 - 15	VERY SOFT	0 - 2	0 - 0.25
LOOSE	4 - 10	15 - 35	SOFT	2 - 4	0.25 - 0.50
COMPACT	10 - 30	35 - 65	FIRM	4 - 8	0.50 - 1.0
DENSE	30 - 50	65 - 85	STIFF	8 - 15	1.0 - 2.0
VERY DENSE	OVER 50	> 85	VERY STIFF	15 - 30	2.0 - 4.0
			HARD	OVER 30	OVER 4.0

\*Refer to ASTM D 1586-99 for a definition of N. Values shown are based on N values corrected for overburden pressures (N<sub>1</sub>). N values may be affected by a number of factors including material size, depth, drilling method, and borehole disturbance. N values are only an approximate guide for consistency of cohesive soil.

### COMPONENT DEFINITIONS BY GRADATION

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. to 12 in.
GRAVEL	3 in. to No. 4 (4.76 mm)
COARSE GRAVEL	3 in. to 3/4 in.
FINE GRAVEL	3/4 in. to No. 4 (4.76 mm)
SAND	No. 4 (4.76 mm) to No. 200 (0.074 mm)
COARSE SAND	No. 4 (4.76 mm) to No. 10 (2.0 mm)
MEDIUM SAND	No. 10 (2.0 mm) to No. 40 (0.42 mm)
FINE SAND	No. 40 (0.42 mm) to No. 200 (0.074 mm)
SILT AND CLAY	Smaller than No. 200 (0.074 mm)
SILT	0.074 mm to 0.005 mm
CLAY	Less than 0.005 mm

### GENERAL NOTES

Report of Borehole logs present material classifications, test data, and observations from subsurface explorations at the subject site as reported by the field geologist, engineer, or scientist. In some cases, the classifications may be made based on laboratory test data when available. It should be noted that the investigation methods only recover a small part of the subsurface materials at the exploration location. Therefore, actual conditions between borings and sampled intervals may differ from those presented on the Report of Borehole logs.

This key and Report of Borehole logs must be read together with the attached report. The information presented on the logs and in this key provide only a basis for an evaluation of the subsurface conditions. Any evaluation of the conditions reported on the Report of Borehole logs must be performed by Professional Engineers or Geologists.



# REPORT OF BOREHOLE: BH-01

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/17/12  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-1 43.5 feet west  
 XY COORDINATES: N 11,973,679, E 871,432  
 ELEVATION: 5,298.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Compact, light gray, SILTY GRAVEL (GM) with sand, dry, some cobbles.				
5			SS1		18 17 32	49	18 / 18			Becomes dense, gray.				
				Bulk										
10			SS2		4 40 45	85	18 / 18		GM	Becomes very dense.				
				Bulk										
15			SS3		13 32 50	82	18 / 18							
				Bag										
	5280.5 17.5			Bulk						Hard, brown, GRAVELLY SILT (ML), dry, some cobbles.				
20			SS4		26 50/5"	R	11 / 11							
				Bulk										
25			SS5		45 30/2"	R	8 / 8		ML					
				Bag										
30			SS6		40 40/3"	R	3 / 6							
				Bag										
	5267.0 31.0			SS7	10 10/0"	R	0 / 0			Refusal at 31'. Backfilled with cuttings. No groundwater encountered in boring.				
35														
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations





# REPORT OF BOREHOLE: BH-02

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 12/18/12

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-2 2 feet east

XY COORDINATES: N 11,973,762, E 870,444

ELEVATION: 5,361.8 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: Hollow Stem Auger

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				B/Bag					GC	Compact, light gray, CLAYEY GRAVEL WITH SAND (GC), dry, some cobbles.				
5358.3 3.5			SS1		17 16 23	39	18 / 18		SM	Dense, light gray, SILTY SAND (SM), little gravel, dry.  Becomes light brown, some gravel, occasional cobbles.				
5352.3 5359.8 10.0			SS2		16 29 50	79	6 / 18		GW	6" lens of very dense, gray, GRAVEL (GW), dry.				
			Bulk						ML	Hard, brown, SANDY SILT and gravel (ML), dry, occasional cobble.				
5343.3 18.5			SS3		17 50/6"	R	12 / 12		SM	Very dense, brown and gray, SILTY SAND (SM), some gravel, dry.				
5340.3 21.5			SS4		26 47 50/3"	R	15 / 15		SM	Refusal at 21.5'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-03

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-3 5 feet south  
 LOGGED: CMT DATE: 1/25/13 XY COORDINATES: N 11,977,597, E 868,520  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,468.6 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, gray, WELL-GRADED SAND WITH GRAVEL (SW), trace fines, dry.				
5									SW					
8.0	5460.6			SS1 Bulk	50/0"	R	0 / 0			Slightly weathered, dark gray, strong rock.	Becomes rock.			
15.0	5453.6			Core						Refusal at 8'. Bottom of borehole at 15'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-04

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/5/13 XY COORDINATES: N 11,977,281, E 870,076  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,355.3 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					SW-SM	Compact, gray/light gray, SILTY SAND (SM), some gravel, cobbles, dry, cementation.				
5351.3 4.0			SS1		14 23 19	42	18 / 18			Dense, gray, SANDY SILT (SM), dry, some cementation.				
				Bag					ML					
5346.3 9.0			SS2 Core		25/0"	R	0 / 0			Weathered, dark gray, fragmented ROCK, trace cementation, trace silty sand.	Becomes rock. Switch to Diamond Coring.			
				Core						Becomes moderate cementation				
				Core						Become strong cementation. [CONGLOMERATE]				
5337.3 18.0				Core						Refusal of Hollow Stem Auger at 9'. Core from 9' to 18'. Bottom of borehole at 18'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-05

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/7/13 XY COORDINATES: N 11,977,243, E 869,166  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,385.6 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Compact, gray/ light gray, SILTY SAND and gravel (SW-SM), dry, trace cementation, cobbles.				
5	5379.6 6.0			Bulk	12 24 41	65	18 / 18		SW-SM	Becomes very dense.				
10				Core						Weathered, dark gray, fragmented ROCK, some sandy silt, strongly cemented.	Become rock. Switch to Diamond coring.			
15	5372.6 13.0									Refusal of Hollow Stem Auger at 6'. Diamond coring from 6-13'. Bottom of borehole at 13'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-06

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/25/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,597, E 870,667  
 ELEVATION: 5,308.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag							Very dense, brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk						SW-SM	Becomes gray.				
5300.1 8.0			SS1 Bulk		50/0"	R	0 / 0			Slightly weathered, dark gray, strong ROCK.	Becomes rock.			
			Core								Switch to Diamond Coring.			
5283.1 25.0										Diamond coring from 18-25'. Bottom of borehole at 25'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-07

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,181, E 871,532  
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0	5371.5 0.5		Bag Bag						SM GP	Loose, brown, SILTY SAND (SM), little gravel, dry. Compact, light gray, SANDY GRAVEL (GP), dry.				
5	5367.0 5.0		SS1 Bag		14 28 33	61	18 / 18			Lens of brown/white, SILTY SAND, trace gravel, dry. Becomes very dense. Becomes well-graded gravel. Very dense, gray, SILTY SAND and gravel (SW-SM), dry.				
10			SS2 Bag		12 39 50/5"	R	17 / 17		SW-SM					
15	5357.0 15.0		SS3 Bag Bag		11 37 50/2"	R	14 / 14			Very dense, gray, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
20			SS4 Bag		25 30/2"	R	10 / 10		GW-GM					
25	5347.5 24.5		SS5 Bag		23 32/3"	R	9 / 9			Very dense, brown, SILTY SAND and gravel (SW-SM), dry, few cobbles.				
30	5342.0 30.0		SS6 Core		36/5"	R	5 / 5		SW-SM					
35			SS7 Core		50/0"	R	0 / 0			Weathered, dark gray, fractured ROCK, strong cementation.	Becomes Rock. Switch to Diamond Coring.			
40			SS8 Core		20 50/0"	R	6 / 6							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-07

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,976,181, E 871,532  
 ELEVATION: 5,372.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Diamond Coring  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
										consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.				
40										Weathered, dark gray, fractured ROCK, strong cementation. (continued)				
45														
50	5324.0 48.0									Refusal of Hollow Stem Auger at 30'. Diamond coring from 30-48'. Bottom of borehole at 48'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-08

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,703, E 873,489  
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag						Dense, brown, SILTY SAND (SW-SM), some gravel, slightly cohesive, trace cementation, trace clayey silt, dry.				
5														
10			SS1	18 19 23	42	18 / 18			SW-SM					
			Bulk							Becomes very dense.				
15			SS2	50 53 30	83	18 / 18								
			Bulk											
20	5200.0 18.0		Bulk							Hard/very dense, brown, SANDY SILT and gravel (ML), some clayey silt (low plasticity), dry, trace cementation.				
25			SS3	50/5"	R	5 / 5			ML					
			Bulk											
30	5190.0 28.0		Bag						SW-SM	Very dense, gray/brown, SILTY SAND and gravel (SW-SM), some clayey silt, dry.				
35	5185.0 33.0		SS4	50/5"	R	5 / 5			SW	Very dense, gray, SAND and gravel (SW), dry.				
			Bulk											
40														

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations





# REPORT OF BOREHOLE: BH-08

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,703, E 873,489  
 ELEVATION: 5,218.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5175.0 43.0								SW	Very dense, gray, SAND and gravel (SW), dry. <i>(continued)</i>				
45									GW	Very dense, gray, GRAVEL and sand (GW), trace clayey silt, dry.				
50	5168.0 50.0		SS5		50/0"	R	0/0				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-09

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 12/21/13

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-9 2 feet east

XY COORDINATES: N 11,972,261, E 875,052

ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: HSA, Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, brown, SANDY SILT (ML), trace gravel, dry.				
			SS1		20 34 40	74	18 / 18		ML					
5	5170.7 6.0			Bulk										
			SS2	Bag(2)	20/0"	R	0 / 1			Dense to very dense, light gray/brown, SAND and gravel (SW), dry.				
			SS3		20 25 24	49	18 / 18		SW					
10	5165.7 11.0			Bag(2)										
				Bag						Hard, light brown, SILT (ML), trace gravel, dry, slightly cohesive, trace cementation.				
			SS4		16 25 27	52	18 / 18			Becomes light reddish brown, little gravel.				
15				Bulk					ML					
			SS5		4 14 24	38	18 / 18							
20	5156.7 20.0			Bulk						Hard, reddish brown, CLAYEY SILT (MH), dry, cohesive.				
									MH					
			SS6		22 14 19	33	18 / 18							
25	5150.7 26.0			Bulk										
				B/Bag						Hard, red SILTY CLAY (CL-ML), dry, cohesive, moderate plasticity.				
			SS7		16 15 19	34	18 / 18		CL-ML					
30														
			SS8		6 4 13	17	18 / 18							
35	5141.7 35.0			Bulk						Hard, red, CLAY (CL), dry.				
									CL					
			SS9		14 21 31	52	18 / 18							
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-09

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION: Moved BH-9 2 feet east  
 LOGGED: CMT DATE: 12/21/13 XY COORDINATES: N 11,972,261, E 875,052  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,176.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk					CL	Hard, red, CLAY (CL), dry. <i>(continued)</i>				
	5133.2 43.5		SS10		13 20 32	52	18 / 18		CL-ML	Hard, red, SILTY CLAY (CL-ML), dry.				
45	5131.7 45.0			Bulk					CH	Hard, red, CLAY, dry.				
50	5126.7 50.0		SS11		9 15 24	39	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-10

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/12/13 XY COORDINATES: N 11,972,513, E 874,813  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				B/Bag						Compact, light gray, CLAYEY SAND WITH GRAVEL (SC), dry, few cobbles.				
5			SS1	B/Bag	17 6 5	11	18 / 18		SC					
10			SS2	B/Bag	9 12 18	30	18 / 18		SC					
15	5167.5 14.5		SS3	Bag	20 16 14	30	18 / 18		MH	Very stiff, red/light brown, CLAYEY SILT (MH), dry, little gravel.				
20	5163.0 19.0		SS4	B/Bag	8 15 20	35	18 / 18		CL	Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity.				
25			SS5	B/Bag	3 10 17	27	18 / 18		CL					
30			SS6	B/Bag	10 19 41	60	18 / 18		CL					
35			SS7	B/Bag	8 11 17	28	18 / 18		CL	Becomes moderate plasticity				
40			SS8	B/Bag	4 6 19	25	18 / 18		CL	Becomes slightly moist, moderate plasticity.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-10

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/12/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,513, E 874,813  
 ELEVATION: 5,182.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			B/Bag											
	5138.5 43.5		SS9		4 7 14	21	18 / 18		CL	Very stiff-hard, red, LEAN CLAY (CL), some sand, dry, trace gravel, cohesive, low plasticity. <i>(continued)</i>				
45									CH	Very stiff, red, CLAY (CH), slightly moist, high plasticity.				
50	5132.0 50.0		SS10		7 10 16	26	18 / 18							
											Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-11

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/3/13 XY COORDINATES: N 11,972,894, E 874,891  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff-very stiff, brown, SILT (ML), trace sand and gravel, dry.				
5178.0 2.5			Bulk											
			SS1		9 13 27	40	18 / 18			Dense, white/light gray, GRAVELLY SAND (SW), dry.				
			Bulk						SW					
5172.0 8.5			SS2		12 27 23	50	18 / 18		SW-SM	Very dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			SS3		5 11 16	27	18 / 18		MH	Very stiff, brown, CLAYEY SILT (MH), dry, low plasticity.				
5167.0 13.5			Bulk											
			SS4		7 12 23	35	18 / 18			Hard, reddish brown, SILTY CLAY (CL-ML), white and black inclusions, dry.				
5162.0 18.5			B/Bag							Becomes red, trace gravel, moderate plasticity, slightly moist.				
			SS5		24 16 23	39	18 / 18							
			B/Bag						CL-ML					
			SS6		7 15 25	40	18 / 18							
			B/Bag											
5147.0 33.5			SS7		11 13 18	31	18 / 18			Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity.				
			Bulk						CH					
			SS8		12 18 30	48	18 / 18							

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-11

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/3/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,894, E 874,891  
 ELEVATION: 5,180.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9	X	11 13 18	31	18 / 18		CH	Hard, red/light brown, CLAY (CH), slightly moist, moderate plasticity. (continued)				
50	5130.5 50.0		SS10	X	11 12 21	33	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-12

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/4/13 XY COORDINATES: N 11,972,981, E 875,148  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						ML	Stiff, brown, SANDY SILT (ML) and gravel, dry.				
5175.5	4.0		SS1		5	27	18 / 18		GW	Compact, light gray, GRAVEL AND SAND (GW), dry.				
5174.5	5.0		Bulk		12					Compact, gray/light brown, GRAVEL and silty sand (GW-GM), dry, few cobbles.				
					15									
			SS2		39	93	17 / 17		GW-GM	Becomes very dense, trace cobbles.				
					43									
					50									
			SS3		13	83	16 / 16		ML	Hard, brown, SANDY SILT and gravel (ML), dry.				
5164.5	15.0		Bulk Bag		33									
					50									
			SS4		7	42	18 / 18		ML	Hard, red, SILTY CLAY (CL-ML), trace gravel, dry, low plasticity.				
5160.0	19.5		B/Bag		18									
					24									
			SS5		7	30	18 / 18		CL-ML					
					10									
					20									
			B/Bag											
			SS6		12	41	18 / 18							
					15									
					26									
			B/Bag											
			SS7		12	38	18 / 18		CH	Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity.				
5146.0	33.5		Bulk		14									
					24									
			SS8		12	42	18 / 18							
					16									
					26									

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-12

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,981, E 875,148  
 ELEVATION: 5,179.5 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Hard, red, CLAY (CH), slightly moist, trace gravel, high plasticity. <i>(continued)</i>				
45			SS9		10 11 18	29	18 / 18		CH	Becomes very stiff-hard.				
50	5129.5 50.0		SS10		10 12 20	32	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-13

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/17/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,776, E 875,471  
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, light gray, SANDY SILT and gravel (ML), dry, slightly cohesive.				
5			SS1	12 11 13	24	18 / 18								
				Bulk										
10			SS2	28 28 33	61	18 / 18			ML	Becomes hard, light brown, trace cementation.				
				Bag										
15			SS3	13 29 35	64	18 / 18								
				Bag										
20			SS4	21 30 31	61	18 / 18								
				Bulk										
25	5146.3 23.5		SS5	28 50/3"	R					Very dense, brown, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines.				
				Bulk										
30			SS6	21 50/4"	R					Lens of reddish brown clayey silt encountered at 34'.				
				Bulk										
35			SS7	27 49 50/2"	R									
				Bulk										
40	5131.3 38.5		SS8	13 24 33	57	18 / 18			CL-ML	Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMP:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-13

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/17/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,776, E 875,471  
 ELEVATION: 5,169.8 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			Bag							Hard, reddish brown, SILTY CLAY (CL-ML), slightly moist, cohesive, trace gravel. <i>(continued)</i>				
45			SS9		9 14 14	28	18 / 18		CL-ML	Becomes very stiff, red, dry.				
50	5119.3 50.5		Bulk											
			SS10		25 32 28	60	18 / 18							
										Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-14

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,766, E 875,868  
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, brown, SILT (ML), some gravel, dry.				
5			SS1	23 44 30	74	18 / 18		ML		Becomes hard, brown, some gravel, cementation at 4-5 ft.				
10	5149.7 8.5		SS2	15 19 23	42	18 / 18		GW-GM		Dense, light brown/gray, GRAVEL and sandy silt (GW-GM), dry, slightly cohesive fines, few cobbles.				
15			SS3	22 20 25	45	18 / 18		GW						
20			SS4	17 50/3"	R	9 / 9		GW		Becomes very dense, light brown/gray, GRAVEL and silty sand.				
25	5134.7 23.5		SS5	18 35 50/4"	R	16 / 16		ML		Hard, light gray, SANDY SILT and gravel (ML), dry.				
30	5129.7 28.5		SS6	25 50/6"	R	12 / 18		SW-SM		Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry.				
35			SS7	33 32 50/5"	R	17 / 17								
40			SS8	16 50/5"	R	11 / 11								

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-14

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,972,766, E 875,868  
 ELEVATION: 5,158.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45			SS9 Bulk		50/1"	R	1 / 1		SW-SM	Very dense, brown/gray, SILTY SAND and gravel (SW-SM), dry. (continued)				
50	5108.2 50.0		SS10		31 29 44	73	18 / 18				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-15

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/4/13 XY COORDINATES: N 11,973,609, E 875,724  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Hard, light gray/light brown, SILT (ML), little gravel, dry.				
5				SS1	9 15 26	41			ML					
8.0	5168.4			Bulk										
10				SS2	38 50/6"	R	12 / 12		GW	Very dense, brown, SANDY GRAVEL (GW), dry.				
10.0	5166.4			Bag					SW	Very dense, gray, SAND and gravel (SW), dry.				
12.0	5164.4			SS3	17 23 50	73			GW	Very dense, gray, SANDY GRAVEL (GW), dry.				
15				Bulk										
17.5	5158.9			SS4	30 50/3"	R	6 / 9		SW	Very dense, light gray, SAND and gravel (SW), dry.				
20				Bulk										
21.5	5154.9			SS5	28 50/4"	R	10 / 10		ML	Hard, light gray, SANDY SILT and gravel (ML), dry.				
25				Bag(2)										
27.5	5148.9			SS6	22 50/6"	R	11 / 12		SW	Very dense, gray, SAND and gravel (SW), dry.				
30				Bag(2)										
31.5	5144.9			SS7	200"	R			SW-SM	Very dense, light gray, SILTY SAND (SW-SM), some gravel, dry.				
35				Bag(2)										
36.5	5139.9			Bag(2)					SW	Very dense, light gray, SAND and gravel (SW), dry.				
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-15

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/4/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,609, E 875,724  
 ELEVATION: 5,176.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5134.9 41.5		SS8 Bag(2)		50/2"	R			SW	Very dense, light gray, SAND and gravel (SW), dry. <i>(continued)</i>				
45									ML	Very dense, gray/brown, SANDY SILT (ML), and gravel, dry, slightly cohesive.				
50	5126.4 50.0								MH	Hard, reddish brown, CLAYEY SILT (MH), some gravel, slightly moist, slightly cohesive.				
55	5122.1 54.3		SS9		24 54 38/3"	R				Bottom of borehole at 54.25'. Backfilled with cuttings. No groundwater encountered in boring.				
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-16

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north  
 XY COORDINATES: N 11,973,973, E 875,187  
 ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown, CLAYEY GRAVEL with sand (GC), dry.				
5			SS1	SS1	9 32 30	62	18 / 18		GC					
10	5183.2 8.5		SS2	SS2	26 18 19	37	18 / 18		GP-GM	Very dense, gray, GRAVEL and sandy silt (GP-GM), dry.				
15	5178.2 13.5		SS3 B/Bulk	SS3 B/Bulk	50/3"	R	3 / 3		GW	Very dense, light brown, GRAVEL and silty sand (GW), dry, trace CaCO3.				
20			SS4 B/Bulk	SS4 B/Bulk	26 50/4"	R	10 / 10		GW					
25			SS5 B/Bulk	SS5 B/Bulk	48 50/3"	R	9 / 9		GW					
30	5162.7 29.0		SS6	SS6	21 50/3"	R	9 / 9		SC	Very dense, gray, CLAYEY SAND with gravel, trace CaCO3, dry, some cementation.				
35	5157.7 34.0		SS7 Bulk	SS7 Bulk	13 8 12	20	18 / 18		MH	Very stiff, light reddish brown, CLAYEY SILT (MH), dry.				
40	5153.2 38.5		SS8	SS8	14 9 11	20	18 / 18		CL-ML	Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-16

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/22/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-16 4 feet north  
 XY COORDINATES: N 11,973,973, E 875,187  
 ELEVATION: 5,191.7 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40				Bulk						Very stiff, reddish brown, SILTY CLAY (CL-ML), slightly moist. <i>(continued)</i>				
45			SS9		17 11 13	24	18 / 18		CL-ML					
				Bulk										
50	5141.7 50.0		SS10		4 2 4	6	18 / 18			Becomes firm, caliche and little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-17

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/5/13 XY COORDINATES: N 11,974,131, E 875,734  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very stiff, gray/light brown, SANDY SILT and gravel (ML), dry.				
5			SS1	10 13 12	25	18 / 18		ML						
10	5177.5 8.5		SS2	4 45 50/4"	R	12 / 16		GW		Very dense, gray, SANDY GRAVEL (GW), dry. Becomes brown/gray.				
20			SS3	7 28 39	67	18 / 18		SW		Very dense, light brown, SAND and gravel (SW), dry. Becomes light gray/ light brown.				
25	5163.0 23.0		SS4	24 50 50/4"	R	16 / 16		SW-SM		Very dense, brown, SILTY SAND and gravel (SW-SM), dry, slightly cohesive fines.				
30	5158.0 28.0			Bulk						Very dense, white/gray CALICHE, dry.				
35	5154.0 32.0			Bulk										
40			SS5	25/0"	R	0 / 0								

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-17

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/5/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,131, E 875,734  
 ELEVATION: 5,186.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40										Very dense, white/gray CALICHE, dry. <i>(continued)</i>				
45										very dense, white/gray CALICHE, dry, little gravel, seam of light brown silt at 44-45'				
50	5136.0 50.0		SS6		25/0"	R	0 / 0			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-18

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-18 2 feet east  
 XY COORDINATES: N 11,974,701, E 874,701  
 ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					ML	Stiff, light brown, SANDY SILT (ML), some gravel, dry.				
5205.3 2.0				Bulk										
				SS1	9 23 20	43	18 / 18		GC	Dense, light brown, CLAYEY GRAVEL with sand (GC), dry.	Switch to Air Rotary drilling at 6'.			
5199.3 8.0				Bulk										
				SS2										
				Bulk										
5189.3 18.0				SS2	50/4"	R			GW	Very dense, light brown/gray, GRAVEL and sand (GW), dry, little CaCO3.				
				Bulk										
5184.3 23.0				SS3	24 31 31	62	18 / 18		ML	Hard, light reddish white, SANDY SILT (ML), some gravel, dry, some CaCO3.				
				Bulk										
5173.8 33.5				SS4	13 8 12	20	18 / 18		CL	Hard, light reddish brown, sandy LEAN CLAY (CL), trace gravel, dry.				
				Bulk										
5168.8 38.5				SS5	38 50/3"	R	9 / 9		CL-ML	Hard, reddish brown, CLAY (CL-ML), some silty clay, little gravel, moderate plasticity, dry.				
				Bulk										
				SS6	11 21 24	45	18 / 18		CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist.				
				Bulk										

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-18

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 1/23/13

CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-18 2 feet east

XY COORDINATES: N 11,974,701, E 874,701

ELEVATION: 5,207.3 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: HSA, Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5163.8 43.5			Bulk					CL	Hard, reddish brown, CLAY (CL), moderate plasticity, slightly moist. <i>(continued)</i>				
45			SS7	Bag	5 6 9	15	18 / 18		CH	Stiff-hard, reddish brown, CLAY (CH), high plasticity, slightly moist, blocky.				
50	5157.3 50.0		SS8		10 19 28	47	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-19

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/10/13 XY COORDINATES: N 11,974,564, E 875,328  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0										Soft, brown, SANDY SILT (ML), little gravel, dry.				
5193.4 3.0			SS1	11 21 22	43	18 / 18		ML		Dense, light brown, SILTY SAND and gravel (SW-SM), dry.				
			Bulk					SW-SM						
5187.4 9.0			SS2	19 38 50/5"	R	17 / 17				Very dense, brown/gray, SANDY GRAVEL (GW), dry.				
			Bulk											
			Bag							Becomes dark gray.				
			Bag(2)							Becomes brown/gray.				
			SS3	27 40 23	63	18 / 18		GW						
5174.4 22.0			B/Bag							Very dense, light gray/white, CALICHE, little gravel, dry.				
			SS4 B/Bag	50/0"	R	0 / 0								
			SS5 Bulk	25/0"	R	0 / 0				Seam of brown sandy silt at 41' - 42', dry.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-19

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/10/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,974,564, E 875,328  
 ELEVATION: 5,196.4 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5153.4 43.0			Bulk						Very dense, light gray/white, CALICHE, little gravel, dry. <i>(continued)</i>				
45				Bag					CL-ML	Very stiff, red, SILTY CLAY (CL-ML), slightly moist, cohesive				
50	5146.4 50.0			SS6	5 13 14	27	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-20

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to this location  
 XY COORDINATES: N 11,975,241, E 871,714  
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)							Compact, brown, SILTY SAND (SM), little gravel, dry.				
5			Bag						SM	Becomes compact to dense, light gray, slightly cohesive, trace clayey silt.				
8.0	5284.0		SS1		16	60	18 / 18			Very dense, gray, GRAVEL (GW), some silty sand, dry, trace clayey silt lenses.				
10			Bulk		29				GW					
18.0	5274.0		SS2		50/3"	R	3 / 3			Very dense, light brown/gray, SILTY SAND and gravel, (SW-SM) dry, slightly cohesive, some light reddish brown clayey silt.				
20			Bulk											
30			SS3		50/3"	R	3 / 3							
35			Bulk						SW-SM					
40	5252.0		SS4		15	R	9 / 9							
			Bulk		50/3"									

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13





# REPORT OF BOREHOLE: BH-20

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: BH-20 on top of waste rock pile, moved BH-20 to this location  
 XY COORDINATES: N 11,975,241, E 871,714  
 ELEVATION: 5,292.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0			Bulk										
45									SM	Very dense, light brown, SILTY SAND (SM), some gravel, slightly moist, slightly cohesive.				
50	5242.0 50.0		SS5	21 36 50/2"	R	14 / 14				Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-21

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/24/13 XY COORDINATES: N 11,975,236, E 874,685  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,214.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Very dense, brown/gray, WELL-GRADED SAND with silt and gravel (SW-SM), dry.				
21			SS1		21	83	18 / 18		SW-SM					
36				Bulk	36									
47					47									
5196.2	18.0		SS2		25	68	18 / 18		SW-SM	Very dense, light brown/reddish white, SILTY SAND and gravel (SW-SM), trace clayey silt, cementation, dry.				
30			SS3		50/0*	R	0 / 0							
5184.2	30.0		Bag(2)							Hard, light reddish brown, CLAYEY SILT and silty clay (MH), trace gravel, dry.				
5177.7	36.5		SS4		15	65	18 / 18		MH					
										Bottom of borehole at 36.5'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-22

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/21/13 XY COORDINATES: N 11,974,757, E 873,750  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag Bulk						Loose, dark brown, SANDY SILTY CLAY (CL-ML), some gravel, dry. Becomes very stiff, yellowish/orange, trace gravel, dry (old tailings).				
5			SS1	16 8 8	16	18 / 18			CL-ML	Becomes orange/brown, slightly moist, cohesive, (old tailings).				
10			SS2	5 4 6	10	18 / 18			CL-ML	Becomes stiff, greenish-gray, moist, cohesive, (old tailings).				
15	5217.5 14.5		SS3	9 10 50/3"	R	15 / 15			GW	Very dense, light gray-gray, GRAVEL and sand (GW), dry, CaCO3 inclusions.				
25	5210.0 22.0		SS4	23 50/3"	R	9 / 9			ML	Hard, light gray/yellowish, SILT (ML), some gravel, slightly moist, slightly cohesive.				
30	5202.0 30.0			7 11 13	24	18 / 18			CL-ML	Becomes brown, SANDY SILT, some clayey silt, trace gravel, moist, cohesive fines.		10.4		
35	5197.0 35.0								CL	Very stiff, reddish brown, SILTY CLAY (CL-ML), moist, cohesive.				
40										Hard, reddish brown, CLAY (CL), dry, moderate plasticity.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-22

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp. LOCATION:  
 LOGGED: CMT DATE: 1/21/13 XY COORDINATES: N 11,974,757, E 873,750  
 CHECKED: DP DATE: 2/21/13 ELEVATION: 5,232.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	5188.5 43.5		SS6		20 45 50/3"	R	15 / 15		CL	Hard, reddish brown, CLAY (CL), dry, moderate plasticity. <i>(continued)</i>				
45	5186.0 46.0		Bulk						SM	Hard, gray/brown, SILTY SAND (SM), slightly moist.				
50	5182.0 50.0		SS7		10 17 50/5"	R	17 / 17		CL-ML	Hard, brown, SILTY CLAY (SM), slightly moist, little gravel.				
										Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-23

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest  
 XY COORDINATES: N 11,974,757, E 873,750  
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0														
	5228.0 2.0			Bag					SP	Loose, brown, SAND and gravel (SP), dry.				
				SS						Very stiff, yellowish, SILT (ML), trace gravel, dry (old tailings).				
				Bulk						Becomes slightly cohesive.				
5									ML					
	5221.0 5220.6 9.5			SS1	13 38 36	74	18 / 18							
				Bag					GM	Very dense, gray/brown, GRAVEL and silt (GM), dry, slight cementation.				
										Hard, white, SANDY SILT (ML), little gravel, dry.				
									ML					
10														
	5216.0 14.0			SS2	13 16 14	30	18 / 30							
				Bulk						Dense, gray, SILTY SAND and gravel (SM), dry, CaCO3 inclusions, slight cementation.				
15														
				SS3	16 18 50/2"	R	14 / 14							
				Bag(2)						Becomes very dense.				
25														
				Bulk					SM	Becomes slightly moist.				
30														
				SS4	50/6"	R	6 / 4							
				Bulk						Becomes brown, some blocky and cohesive clayey silt (low plasticity), slightly moist.				
40														

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-23

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/20/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-23 20 feet southwest  
 XY COORDINATES: N 11,974,757, E 873,750  
 ELEVATION: 5,230.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45									SM	Dense, gray, SILTY SAND and gravel (SM), dry, CaCO <sub>3</sub> inclusions, slight cementation. <i>(continued)</i>				
48.0	5182.0													
49.5	5180.5		SS5	50/4"	R	4 / 5			ML	Very dense, brown, SANDY SILT (ML), wet, little gravel.				
50										Bottom of borehole at 49.5'. Backfilled with cuttings and bentonite. Groundwater encountered in boring at 45'.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-24

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility

PROJECT NO.: 103-92557

CLIENT: New Mexico Copper Corp.

LOGGED: CMT DATE: 1/19/13

CHECKED: DP DATE: 2/21/13

LOCATION:

XY COORDINATES: N 11,975,910, E 872,908

ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket

DRILL RIG: CME-1250

DRILLING METHOD: Air Hammer

HAMMER TYPE: Auto Hammer

HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0	5264.6		Bag(2)						SP	Dense, gray/brown, SAND and gravel (SP), dry, trace CaCO <sub>3</sub> .				
2.5			Bulk											
5														
			SS1 Bulk		50/4"	R	4 / 4							
10														
15									SW-SM					
20			SS2 Bulk		50/3"	R	3 / 3							
25														
28.0	5239.1		SS3 Bulk		41 50/3"	R	9 / 9				Hard/very dense, brown, SANDY SILT and gravel (ML), trace clayey silt, dry.			
30									ML					
33.0	5234.1		Bulk								Very dense, gray/brown, SILTY SAND (SM), some gravel, trace clayey silt, dry.			
35														
40	5227.1		SS4		44 50/3"	R	9 / 9		SM					

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-24

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/19/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,975,910, E 872,908  
 ELEVATION: 5,267.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40	40.0		Bulk											
45									ML					
50	5217.1 50.0		SS5		50/1"	R	1 / 1							
55														
60														
65														
70														
75														
80														

Hard, light reddish brown, SANDY SILT (ML), some gravel, trace clayey silt, dry, slightly cohesive.

Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.





# REPORT OF BOREHOLE: BH-25

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,726, E 874,570  
 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMPT (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bulk							Dense to very dense, gray/brown, CLAYEY GRAVEL with sand (GC), well graded, dry.				
5			SS1		22 28	57	18 / 18		GC					
10			Bulk											
	5199.7 12.5		SS2		39 26 16	42	18 / 18							
			Bulk											
15			SS3		18 21 20	41	18 / 18		ML	Hard, brown, SANDY SILT and gravel (ML), dry, slightly cohesive fines, trace CaCO3.				
			Bulk											
20			SS4		15 15 33	48	18 / 18							
	5190.2 22.0		Bulk							Very dense, light reddish brown, CLAYEY SAND (SC), dry, trace gravel, trace CaCO3.				
25			SS5		10 27 31	58	18 / 18		SC					
			Bulk											
30			SS6		5 32 35	57	18 / 18							
			Bulk											
35			SS7		6 9 16	25	18 / 18							
	5177.2 35.0		Bulk							Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive.				
40			SS8		8 8	25	18 / 18		CL-ML					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-25

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,971,726, E 874,570  
 ELEVATION: 5,212.2 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			Bulk		17					Very stiff, red, SILTY CLAY (CL-ML), dry, cohesive. <i>(continued)</i>				
45	5167.2 45.0		SS9 Bulk		11 10 25	35	18 / 18		CL-ML					
50	5161.7 50.5		SS10		10 18 26	44	18 / 18		CH	Hard, red, CLAY (CH), dry, sand seam at 44' to 44.5'.				
55														
60														
65														
70														
75														
80										Bottom of borehole at 50.5'. Backfilled with cuttings. No groundwater encountered in boring.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-26

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-26 2 feet south  
 XY COORDINATES: N 11,971,618, E 872,048  
 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0			Bag(2)						GW	Dense, brown, GRAVEL (GW), some sand, dry.				
5310.0 2.0			Bulk						SW	Very dense, gray/brown, SAND and gravel (SW), dry, sheen.				
5304.0 8.0			SS1 Bulk	27 50/3"	R	9 / 9			GW	Very dense, dark gray, GRAVEL (GW), some sand, dry.				
5297.0 15.0			Bag(2)						SW	Very dense, brown/dark gray, SAND and gravel (SW), dry.				
			SS2 Bulk	50/4"	R	4 / 4			SW	Becomes gray/light brown, trace CaCO3.				
			SS3 SS	25 50/2"	R	8 / 8			SW					
5279.0 33.0			Bag(2)						SW-SM	Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt.				
			SS4 Bulk	21 50/5"	R	11 / 4			SW-SM					

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-26

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/18/13  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-26 2 feet south  
 XY COORDINATES: N 11,971,618, E 872,048  
 ELEVATION: 5,312.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40														
45														
50	5262.0 50.0		SS5	50/2"	R	2 / 5			SW-SM	Very dense, gray/light brown, SILTY SAND (SW-SM), some gravel, dry, slightly cohesive, trace clayey silt. <i>(continued)</i>				
55											Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.			
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-27

SHEET: 1 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,983, E 874,918  
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bulk						Dense, brown, SILTY SAND and gravel (50%) (SM), dry, trace CaCO <sub>3</sub> , some cementation.				
5									SM					
10			SS1	17 16 15	31	18 / 18								
12.0	5186.1			Bulk										
15										Hard, light brown/white-gray, SANDY SILT (ML), some gravel, dry, some CaCO <sub>3</sub> , cementation.				
20			SS2	30 58/6'	R	12 / 12			ML					
23.0	5175.1			Bulk										
25										Very dense, brown, SAND and gravel (SW), dry.				
26.0	5172.1		SS3	20 24 33	57	18 / 18			CH	Hard, reddish brown, CLAY (CH), dry.				
27.5	5170.6			Bulk						Very stiff, light reddish brown, CLAYEY SILT (MH), some clay, dry, trace gravel.				
30														
35			SS4	10 7 13	20	18 / 18			MH					
38.5	5159.6			Bulk										
40			SS5	12 24 30	54	18 / 18			CL-ML	Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO <sub>3</sub> inclusions.				

Report of borehole must be read in conjunction with accompanying notes and abbreviations

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13



# REPORT OF BOREHOLE: BH-27

SHEET: 2 OF 2

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 1/23/13  
 CHECKED: DP DATE: 2/21/13

LOCATION:  
 XY COORDINATES: N 11,973,983, E 874,918  
 ELEVATION: 5,198.1 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: HSA, Air Hammer  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
40			Bulk							Very stiff-hard, reddish brown, SILTY CLAY (CL-ML), dry, low plasticity, trace CaCO3 inclusions. <i>(continued)</i>				
45			SS6		10 12 17	29	18 / 18	CL-ML						
			Bulk											
50	5148.1 50.0		SS7		14 20 27	47	18 / 18			Bottom of borehole at 50'. Backfilled with cuttings. No groundwater encountered in boring.				
55														
60														
65														
70														
75														
80														

Report of borehole must be read in conjunction with accompanying notes and abbreviations



# REPORT OF BOREHOLE: BH-28

SHEET: 1 OF 1

PROJECT: Geotech Investigation, Tailings Storage Facility  
 PROJECT NO.: 103-92557  
 CLIENT: New Mexico Copper Corp.  
 LOGGED: CMT DATE: 12/18/12  
 CHECKED: DP DATE: 2/21/13

LOCATION: Moved BH-28 2 feet east  
 XY COORDINATES: N 11,975,241, E 870,785  
 ELEVATION: 5,388.0 ft.

DRILLING CONTRACTOR: Yellow Jacket  
 DRILL RIG: CME-1250  
 DRILLING METHOD: Hollow Stem Auger  
 HAMMER TYPE: Auto Hammer  
 HOLE DIAMETER: 8.25

DEPTH feet	LAYER ELEVATION	WATER	SAMPLE NUMBER	SAMPLE TYPE	BLOWS PER SIX INCHES	BLOWS PER FOOT (N)	RECOVERY / ATTEMP (IN.)	GRAPHIC LOG	USCS	Sample Description consistency or density, color, grain size, MAJOR COMPONENT, minor components, moisture.	Comments	MOISTURE (%)	DRY DENSITY (pcf)	ADDITIONAL LAB TESTING
0				Bag					GW	Dense, light gray, SANDY GRAVEL (GW), dry, few cobbles.				
5	5383.0 5.0		SS1	9 13 18	31				GP	Dense, light gray, SANDY GRAVEL (GP), dry.				
10	5378.0 10.0 5377.0 11.0		SS2	22 32 50	82				GW	Becomes very dense, white inclusions, sheen, dry.				
15	5374.5 13.5		SS3	30 30/3"	R				SM	Very dense, light gray, SANDY GRAVEL (GW), dry, few cobbles. Very dense, light brown and gray, gravelly SILTY SAND (SM), dry.				
20	5369.0 19.0		SS4	50/4"	R				SW	Very dense, light brown and gray, gravelly SAND (SW), sheen, dry.				
21	5367.0 21.0 5366.0 22.0		SS5	50/0"	R				GW	Seam of very dense, light gray, SANDY GRAVEL (GW), dry. Becomes light brown.				
22									SW	Very dense, light brown and gray, gravelly SAND, (SW) sheen, dry.				
22										Refusal at 22'. Backfilled with cuttings. No groundwater encountered in boring.				

RPT:TUC GEOTECH SOIL PROJ:103-92557 COPPER FLAT - COPY.GPJ TMPL:GLDR\_TUC2.GDT LIB:GLDR\_TUC\_V1.GLB DATE:12/16/13

Report of borehole must be read in conjunction with accompanying notes and abbreviations

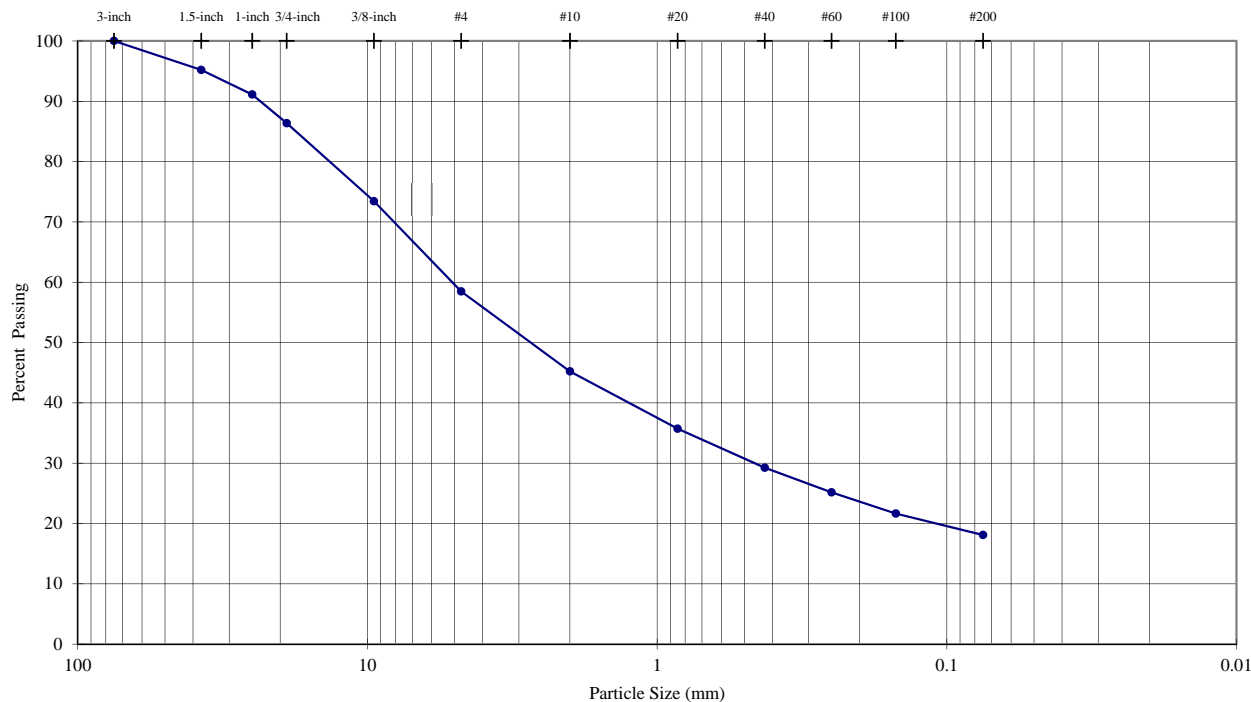
**APPENDIX A.3  
GEOTECHNICAL TEST RESULTS**



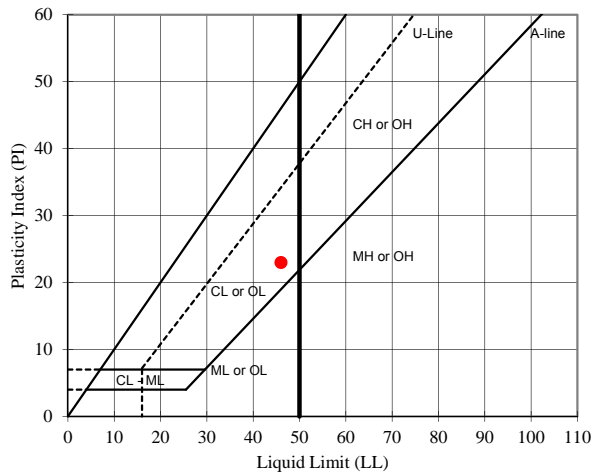
**APPENDIX A.3.1**  
**GRADATION MOISTURE/DENSITY TEST REPORTS**

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-2** DEPTH (ft): **0-3.5**  
 TYPE: **Pail/Bag**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>13.63</b>
1.5-inch	37.5	<b>95.2</b>		
1-inch	25.0	<b>91.1</b>		
3/4-inch	19.0	<b>86.4</b>	Fine Gravel	<b>27.89</b>
3/8-inch	9.5	<b>73.4</b>		
#4	4.8	<b>58.5</b>	Coarse Sand	<b>13.26</b>
#10	2.00	<b>45.2</b>		
#20	0.85	<b>35.7</b>	Medium Sand	<b>15.98</b>
#40	0.43	<b>29.2</b>		
#60	0.25	<b>25.2</b>	Fine Sand	<b>11.15</b>
#100	0.15	<b>21.7</b>		
#200	0.075	<b>18.1</b>		
			Silt or Clay Fines	<b>18.10</b>



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, olive brown, dry

LL	PL	PI
<b>46</b>	<b>23</b>	<b>23</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**GC**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

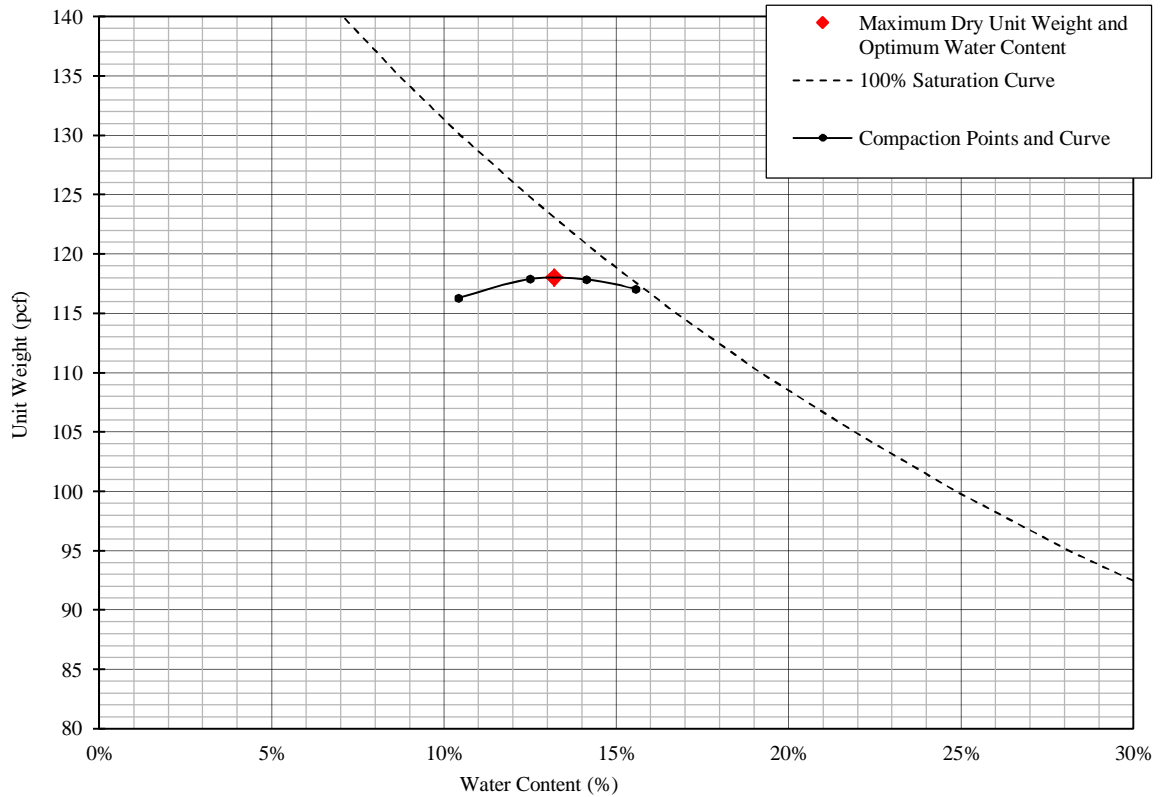
TECH MGC  
 DATE 2/22/2013  
 REVIEW MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method C

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-2**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-3.5**



% Test Fraction Passing 3/4-inch Sieve	<b>87%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.67</b>

Maximum Dry Unit Weight (pcf)	<b>118.0</b>
Optimum Water Content (%)	<b>13.2</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>122.4</b>
Corrected Optimum Water Content (%)	<b>11.5</b>

USCS Description (ASTM D 2487): Clayey gravel with sand, olive brown, dry

USCS GC

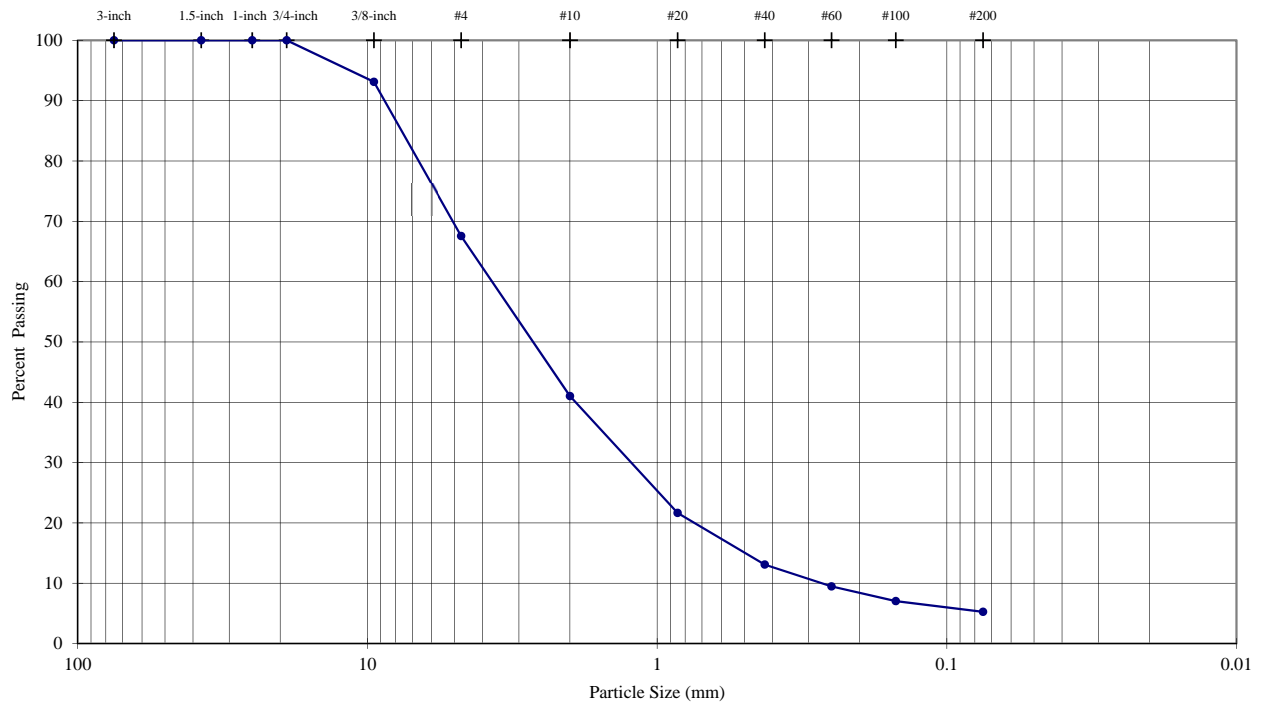
TECH	MGC
DATE	2-26-13
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

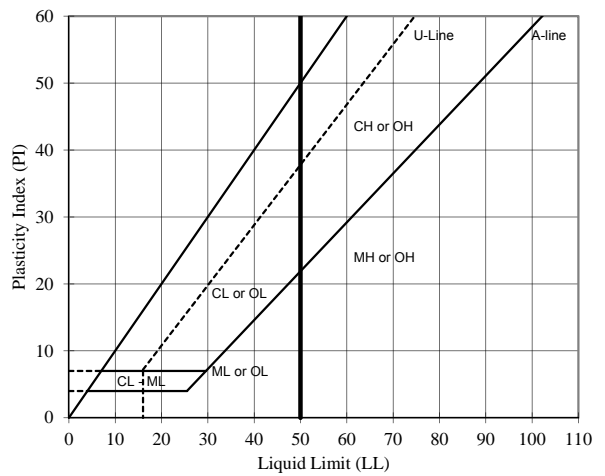
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-3**  
 TYPE: **Pail**

DEPTH (ft): **0-8**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	32.44
3/8-inch	9.5	93.1		
#4	4.8	67.6	Coarse Sand	26.53
#10	2.00	41.0		
#20	0.85	21.6		
#40	0.43	13.1	Medium Sand	27.93
#60	0.25	9.5		
#100	0.15	7.0	Fine Sand	7.83
#200	0.075	5.3		
			Silt or Clay Fines	5.26



Visual Description (Golder Procedure):  
 Gravelly SAND, some fines, greenish gray, dry

LL	PL	PI
--	--	--

As-Received Moisture Content (%)  
 --

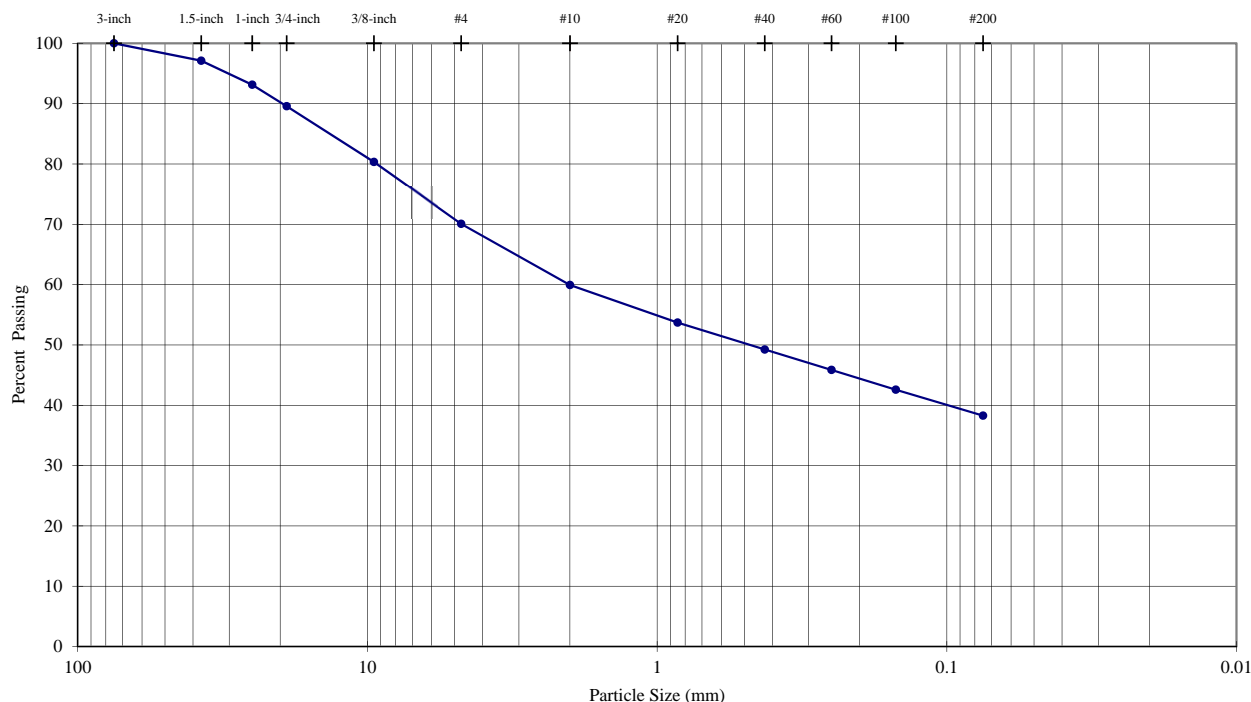
USCS Group Symbol  
 --

Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

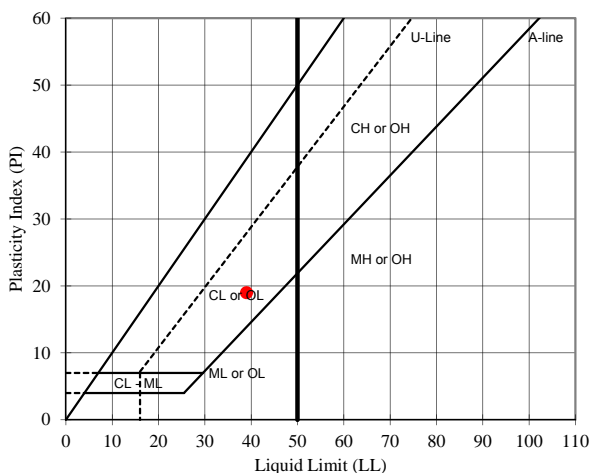
TECH MGC  
 DATE 2/22/2013  
 REVIEW MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10** DEPTH (ft): **0-14.5**  
 TYPE: **Pail/Bag**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	10.43
1.5-inch	37.5	97.1		
1-inch	25.0	93.1		
3/4-inch	19.0	89.6	Fine Gravel	19.50
3/8-inch	9.5	80.3		
#4	4.8	70.1	Coarse Sand	10.14
#10	2.00	59.9		
#20	0.85	53.7		
#40	0.43	49.2	Medium Sand	10.69
#60	0.25	45.9		
#100	0.15	42.6	Fine Sand	10.96
#200	0.075	38.3		
			Silt or Clay Fines	38.28



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, light yellowish brown, dry

LL	PL	PI
39	20	19

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SC

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

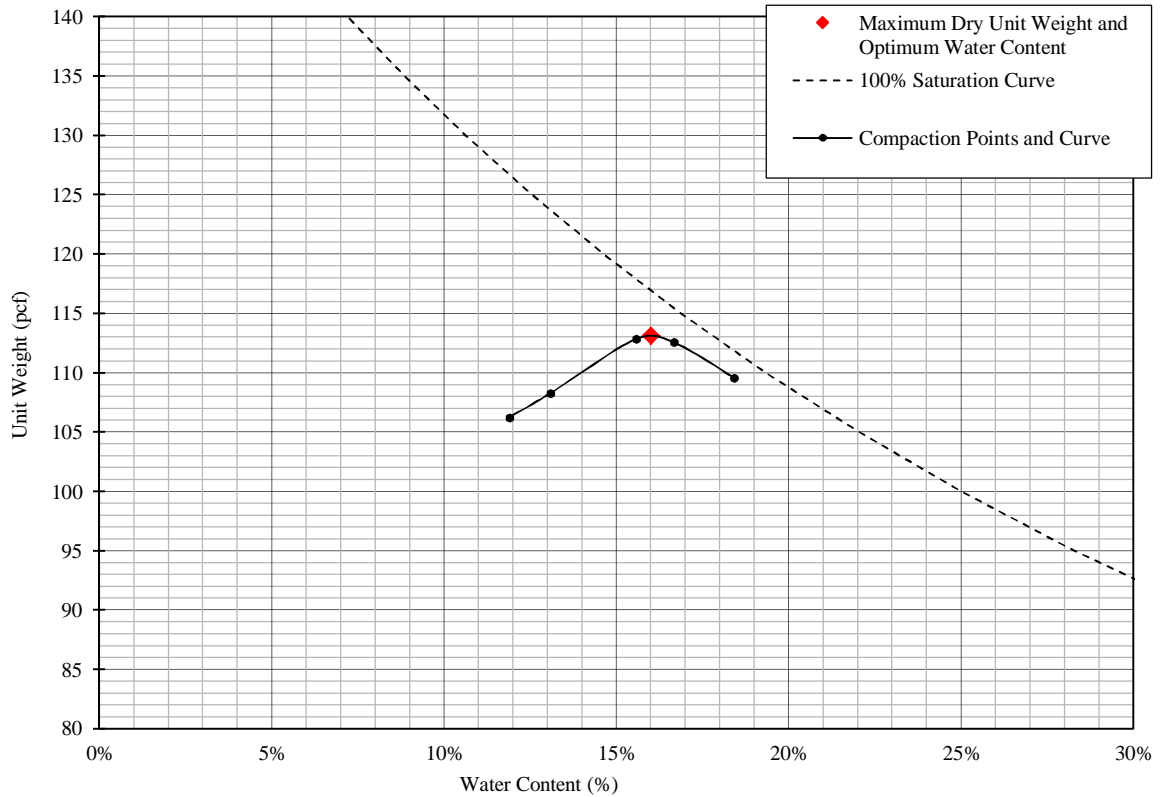
TECH **AMS/MGC**  
 DATE **2/25/2013**  
 REVIEW **MB**

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer    Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-14.5**



% Test Fraction Passing 3/8-inch Sieve	<b>81%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.68</b>

Maximum Dry Unit Weight (pcf)	<b>113.1</b>
Optimum Water Content (%)	<b>16.0</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>120.0</b>
Corrected Optimum Water Content (%)	<b>13.2</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, light yellowish brown, dry

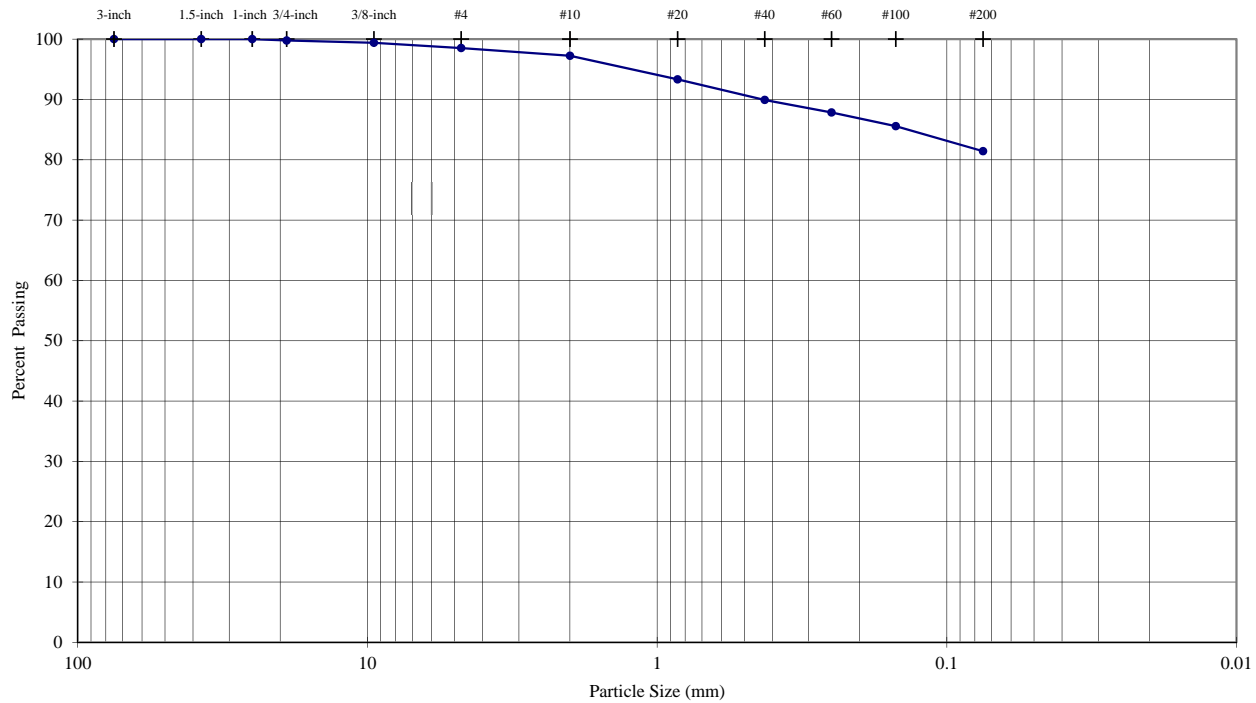
USCS SC

TECH	AMS
DATE	2-27-13
REVIEW	MB

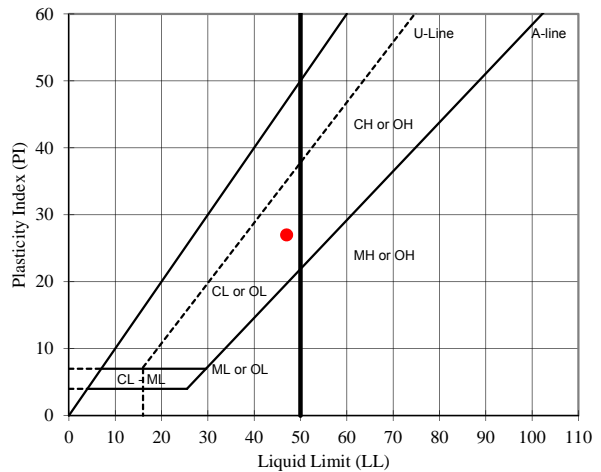
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-10**  
 TYPE: **Pail**

DEPTH (ft): **19-33**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>0.23</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>100.0</b>		
3/4-inch	19.0	<b>99.8</b>	Fine Gravel	<b>1.25</b>
3/8-inch	9.5	<b>99.4</b>		
#4	4.8	<b>98.5</b>	Coarse Sand	<b>1.28</b>
#10	2.00	<b>97.2</b>		
#20	0.85	<b>93.3</b>	Medium Sand	<b>7.32</b>
#40	0.43	<b>89.9</b>		
#60	0.25	<b>87.8</b>	Fine Sand	<b>8.49</b>
#100	0.15	<b>85.6</b>		
#200	0.075	<b>81.4</b>		
Silt or Clay Fines			<b>81.42</b>	



USCS Description (ASTM D 2487):

Lean clay with sand, yellowish red, dry

LL	PL	PI
47	20	27

As-Received Moisture Content (%)

14.4

USCS Group Symbol

CL

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/25/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

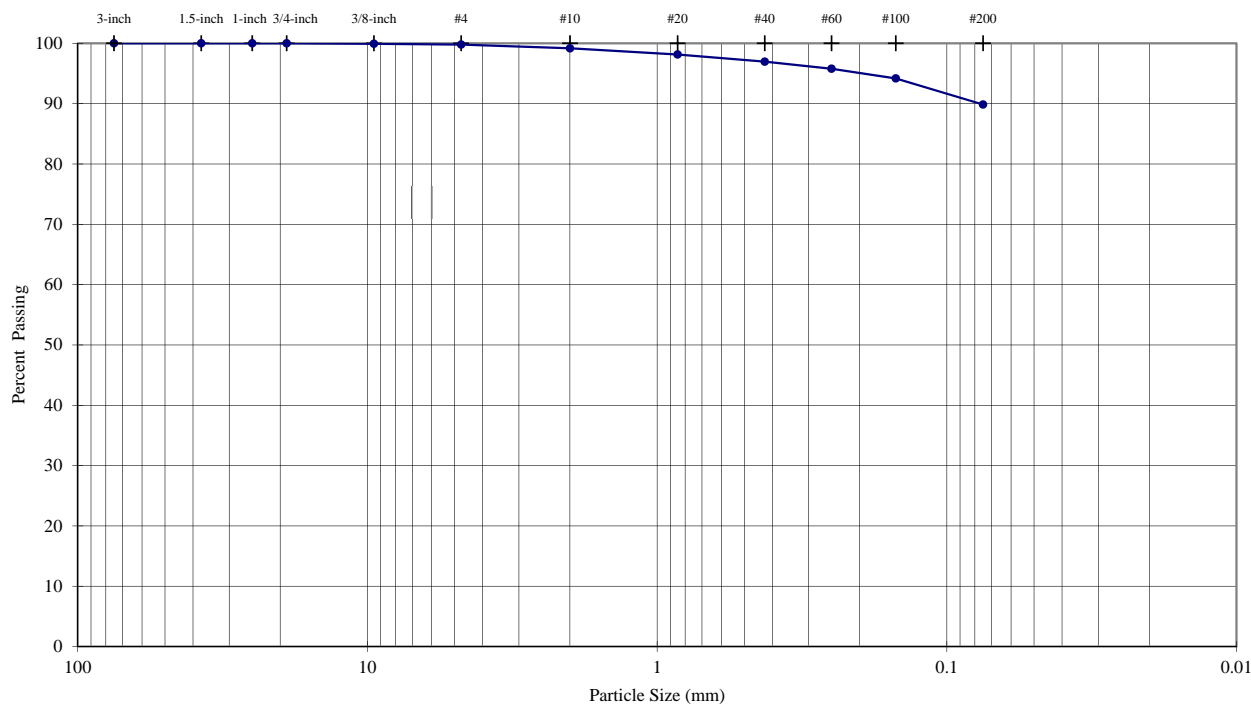
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**

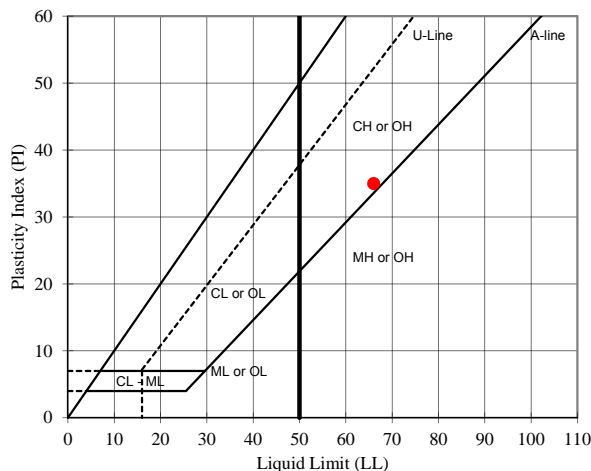
SAMPLE ID: **BH-12**

DEPTH (ft): **33.5-48.5**

TYPE: **Pail**



Sieve Analysis (Initial Separation on No. 4 Sieve)	Particle Size		Description	Percentage
	Sieve	(mm)		
	3-inch	75.0	100.0	Coarse Gravel
	1.5-inch	37.5	100.0	
	1-inch	25.0	100.0	
	3/4-inch	19.0	100.0	Fine Gravel
	3/8-inch	9.5	99.9	
	#4	4.8	99.8	Coarse Sand
	#10	2.00	99.2	
	#20	0.85	98.1	
	#40	0.43	97.0	Medium Sand
	#60	0.25	95.8	
	#100	0.15	94.2	Fine Sand
	#200	0.075	89.9	
				Silt or Clay Fines
				<b>89.86</b>



USCS Description (ASTM D 2487):  
 Fat clay, dark red, moist

LL	PL	PI
66	31	35

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 CH

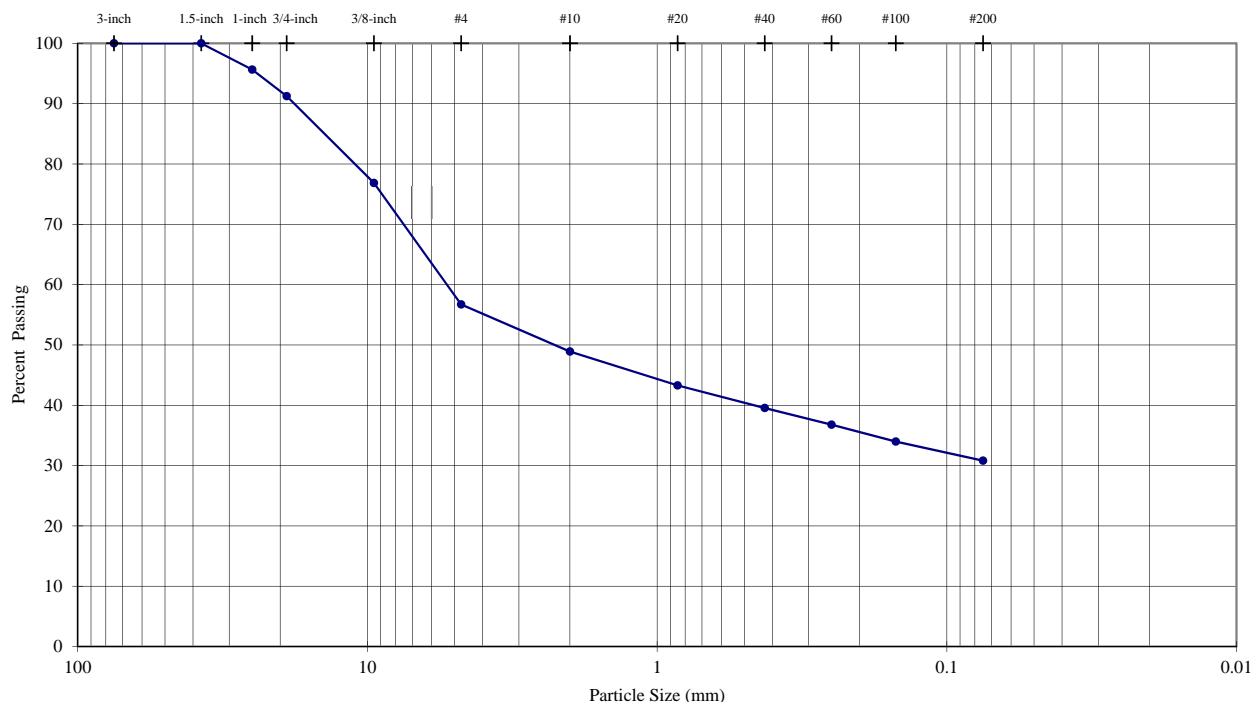
Notes: 0g of particles up to 19.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AM  
 DATE 2/26/2013  
 REVIEW MB

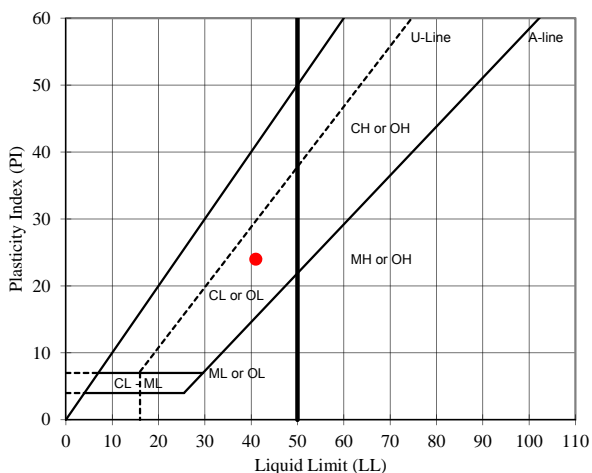


**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-16** DEPTH (ft): **0-8.5**  
 TYPE: **Pail**



Sieve Analysis (Initial Separation on No. 4 Sieve)	Particle Size		Description	Percentage
	Sieve	(mm)		
	3-inch	75.0	100.0	
	1.5-inch	37.5	100.0	Coarse Gravel <b>8.75</b>
	1-inch	25.0	95.7	
	3/4-inch	19.0	91.2	Fine Gravel <b>34.54</b>
	3/8-inch	9.5	76.9	
	#4	4.8	56.7	Coarse Sand <b>7.80</b>
	#10	2.00	48.9	
	#20	0.85	43.3	Medium Sand <b>9.36</b>
	#40	0.43	39.5	
	#60	0.25	36.8	Fine Sand <b>8.73</b>
	#100	0.15	34.0	
	#200	0.075	30.8	Silt or Clay Fines <b>30.81</b>



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, reddish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 GC

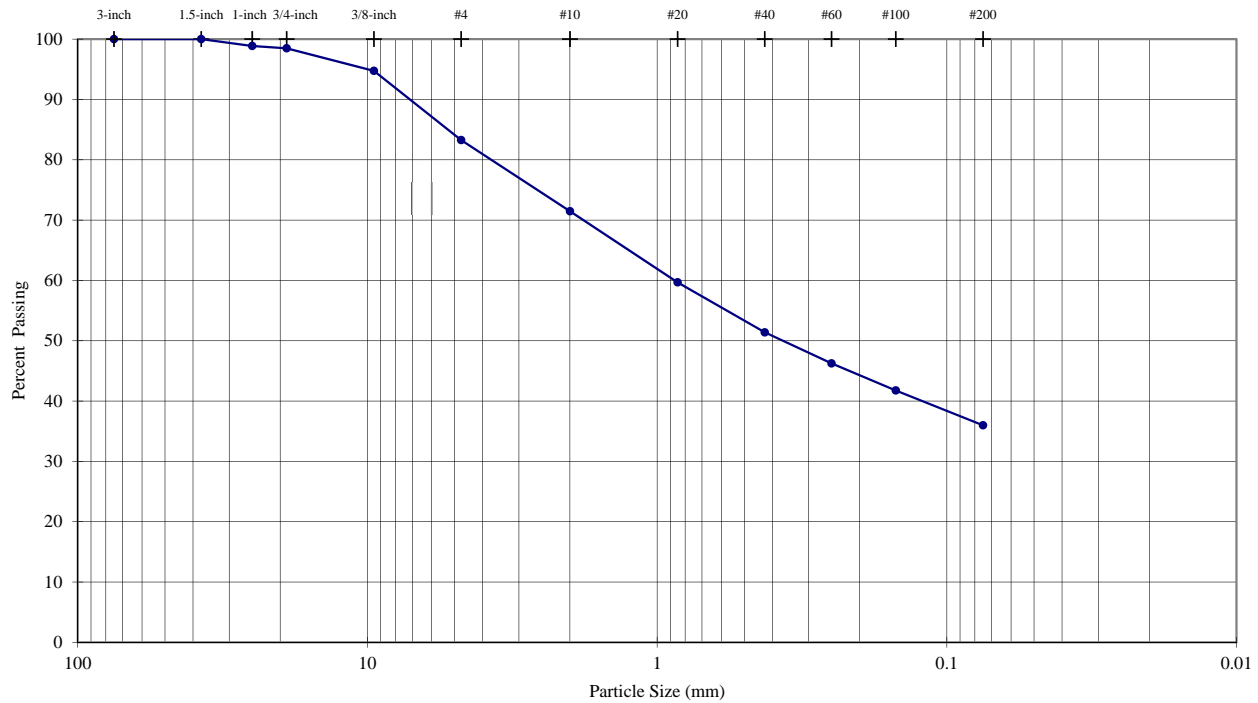
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/27/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

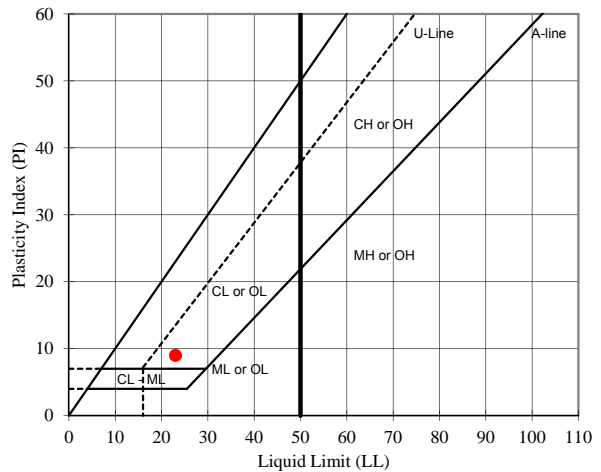
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-16**  
 TYPE: **Pail**

DEPTH (ft): **29-34**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>1.52</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>98.9</b>		
3/4-inch	19.0	<b>98.5</b>	Fine Gravel	<b>15.22</b>
3/8-inch	9.5	<b>94.7</b>		
#4	4.8	<b>83.3</b>	Coarse Sand	<b>11.79</b>
#10	2.00	<b>71.5</b>		
#20	0.85	<b>59.7</b>	Medium Sand	<b>20.08</b>
#40	0.43	<b>51.4</b>		
#60	0.25	<b>46.2</b>	Fine Sand	<b>15.41</b>
#100	0.15	<b>41.8</b>		
#200	0.075	<b>36.0</b>		
			Silt or Clay Fines	<b>35.98</b>

Sieve Analysis (Initial Separation on No. 4 Sieve)



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, yellowish brown, dry

LL	PL	PI
23	14	9

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SC

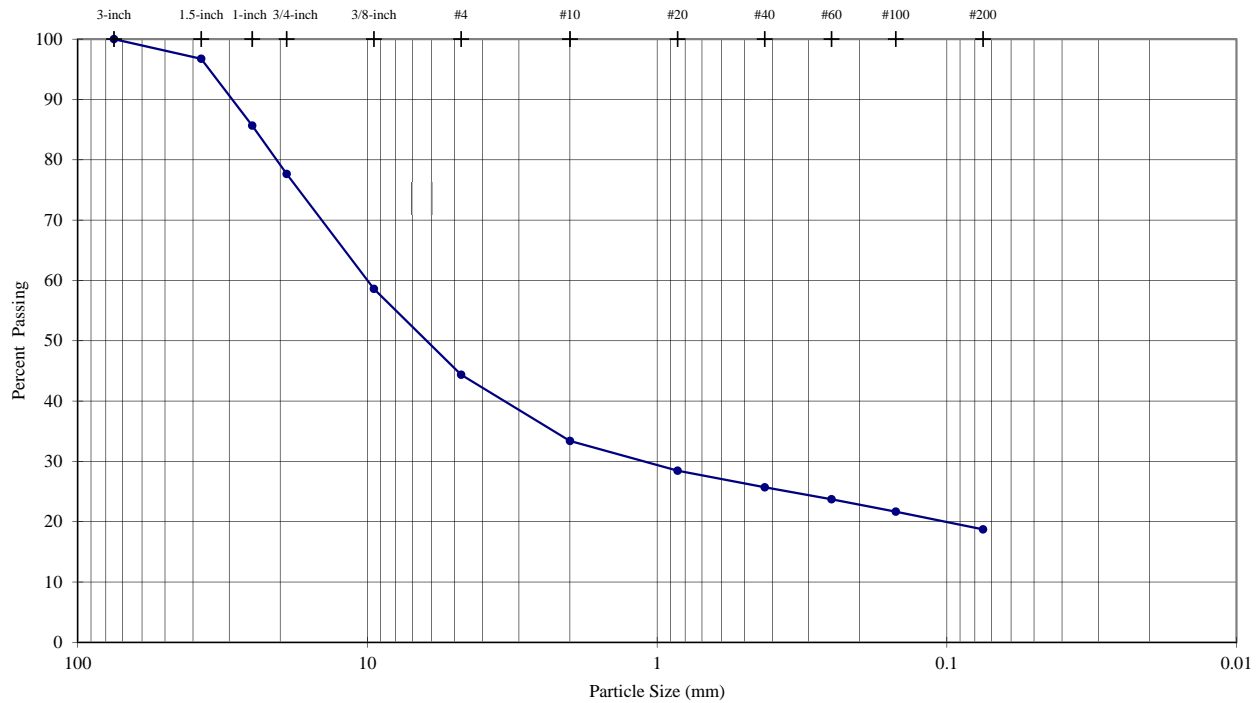
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	EH
DATE	2/26/2013
REVIEW	MB

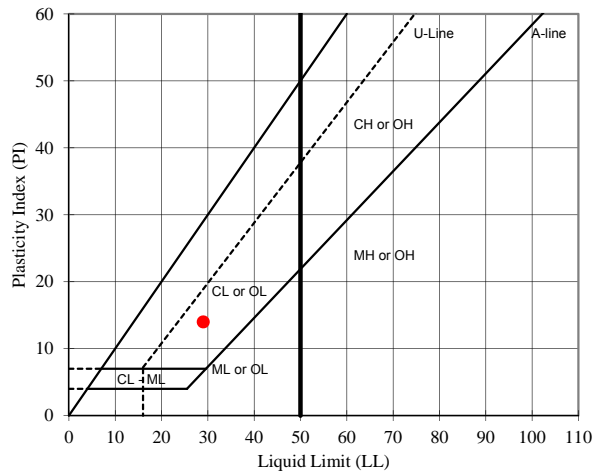
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18**  
 TYPE: **Pail**

DEPTH (ft): **2-8**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>22.35</b>
1.5-inch	37.5	<b>96.7</b>		
1-inch	25.0	<b>85.7</b>		
3/4-inch	19.0	<b>77.6</b>	Fine Gravel	<b>33.28</b>
3/8-inch	9.5	<b>58.6</b>		
#4	4.8	<b>44.4</b>	Coarse Sand	<b>10.98</b>
#10	2.00	<b>33.4</b>		
#20	0.85	<b>28.5</b>	Medium Sand	<b>7.68</b>
#40	0.43	<b>25.7</b>		
#60	0.25	<b>23.7</b>	Fine Sand	<b>6.98</b>
#100	0.15	<b>21.7</b>		
#200	0.075	<b>18.7</b>	Silt or Clay Fines	<b>18.73</b>



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, yellowish brown, dry

LL	PL	PI
<b>29</b>	<b>15</b>	<b>14</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**GC**

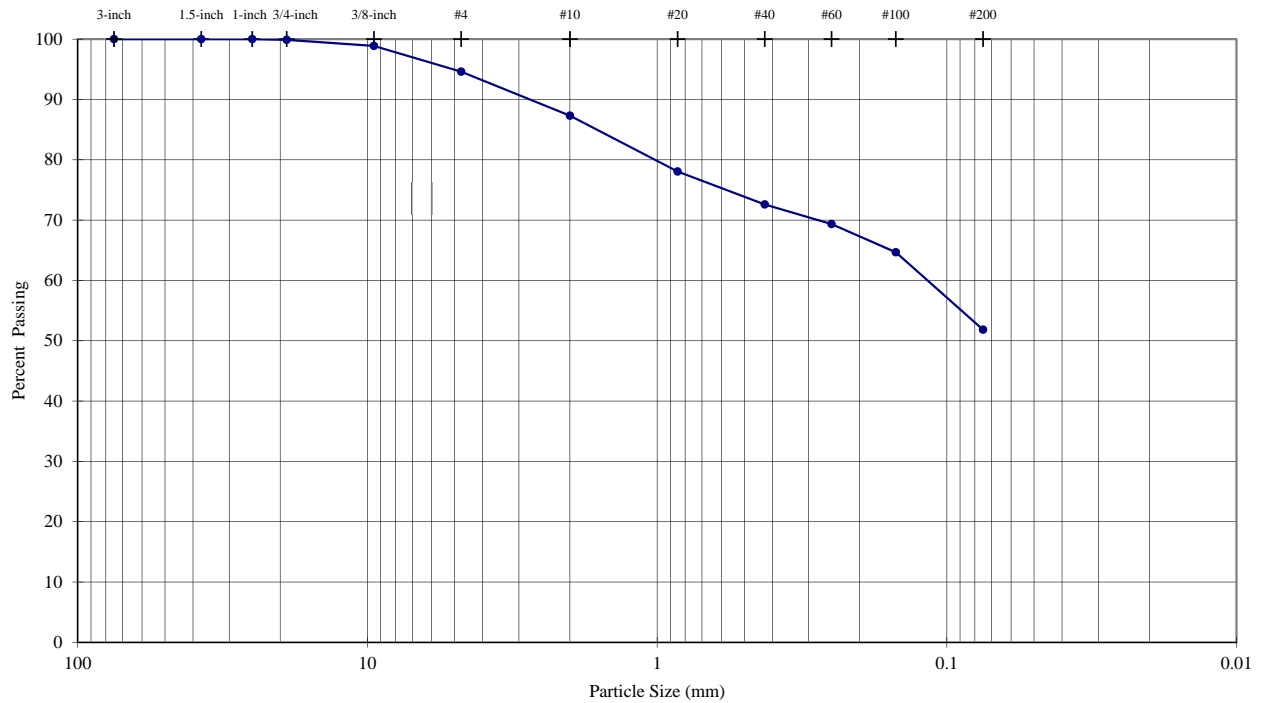
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH **AMS**  
 DATE **2/28/2013**  
 REVIEW **MB**

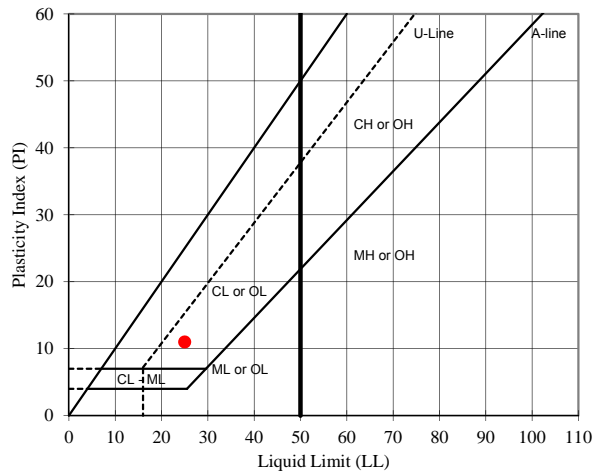
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18**  
 TYPE: **Pail**

DEPTH (ft): **23-33.5**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.12
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	5.27
3/8-inch	9.5	98.9		
#4	4.8	94.6	Coarse Sand	7.30
#10	2.00	87.3		
#20	0.85	78.1	Medium Sand	14.72
#40	0.43	72.6		
#60	0.25	69.3	Fine Sand	20.76
#100	0.15	64.7		
#200	0.075	51.8		
Sieve Analysis (Initial Separation on No. 4 Sieve)			Silt or Clay Fines	51.84



USCS Description (ASTM D 2487):

Sandy lean clay, reddish brown, moist

LL	PL	PI
25	14	11

As-Received Moisture Content (%)

#DIV/0!

USCS Group Symbol

CL

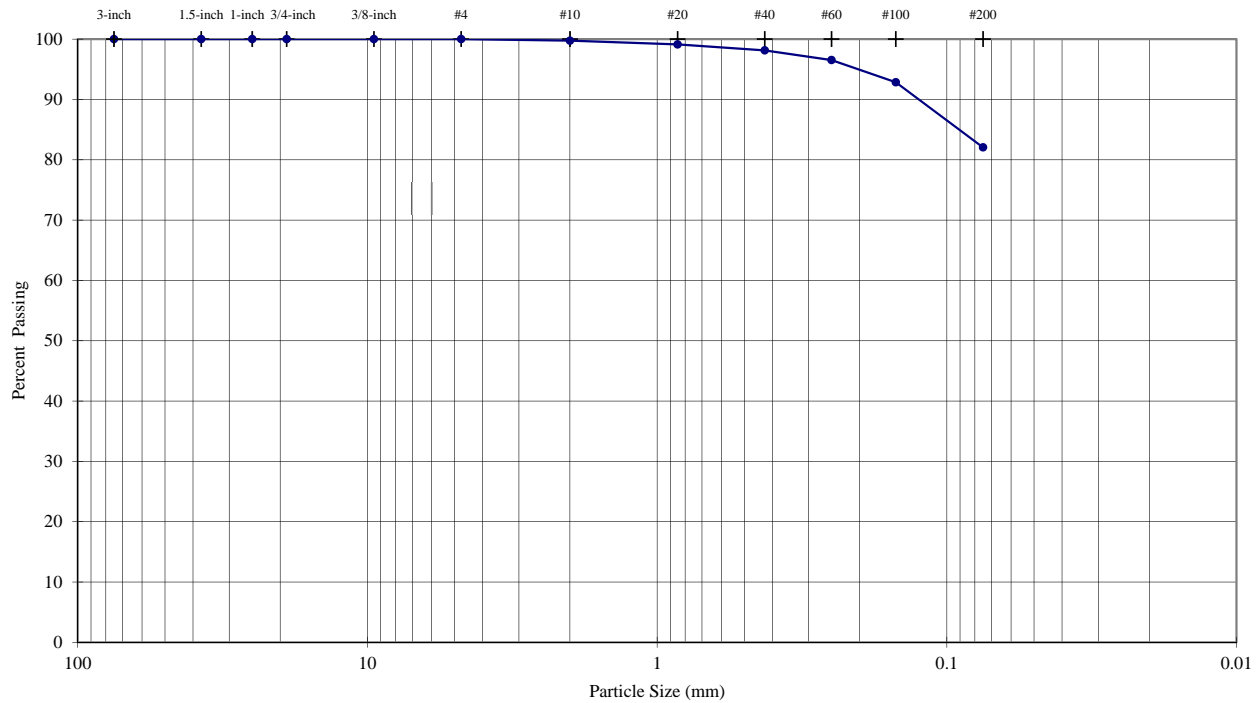
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/25/2013
REVIEW	MB

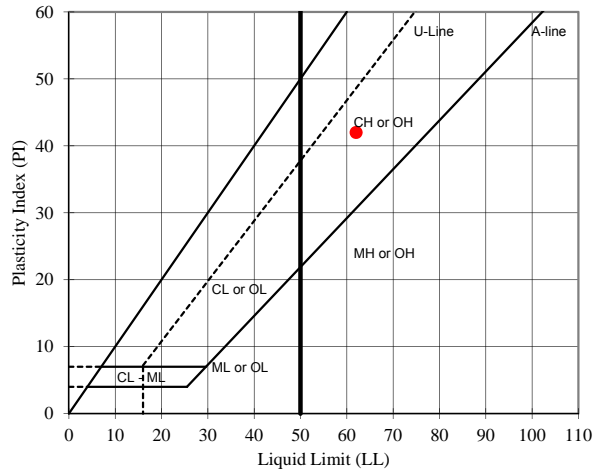
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-18**  
 TYPE: **Bag**

DEPTH (ft): **43.5-48.5**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	0.00
3/8-inch	9.5	100.0		
#4	4.8	100.0	Coarse Sand	0.25
#10	2.00	99.7		
#20	0.85	99.1		
#40	0.43	98.1	Medium Sand	1.61
#60	0.25	96.5		
#100	0.15	92.9	Fine Sand	16.07
#200	0.075	82.1		
Silt or Clay Fines				82.06



USCS Description (ASTM D 2487):  
 Fat clay with sand, dark red, wet

LL	PL	PI
62	20	42

As-Received Moisture Content (%) **29.6**      USCS Group Symbol **CH**

Notes: 0g of particles up to 4.8mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the wet method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

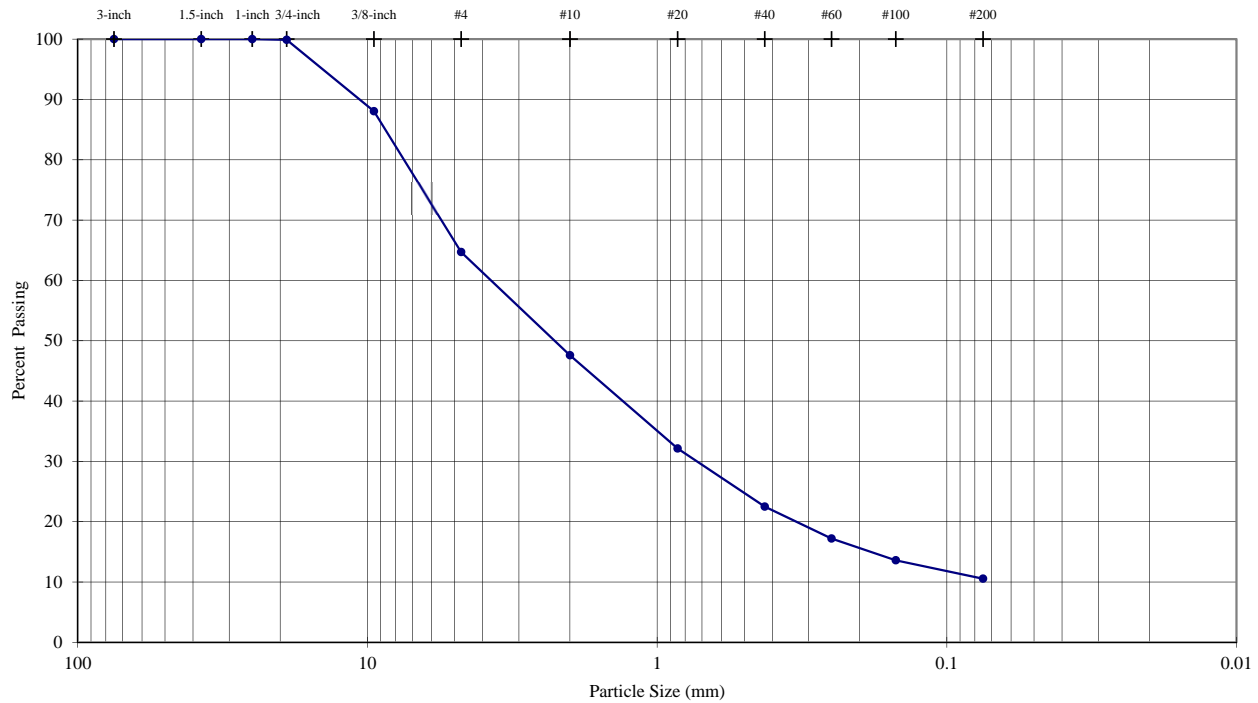
TECH **AMS**  
 DATE **2/25/2013**  
 REVIEW **MB**



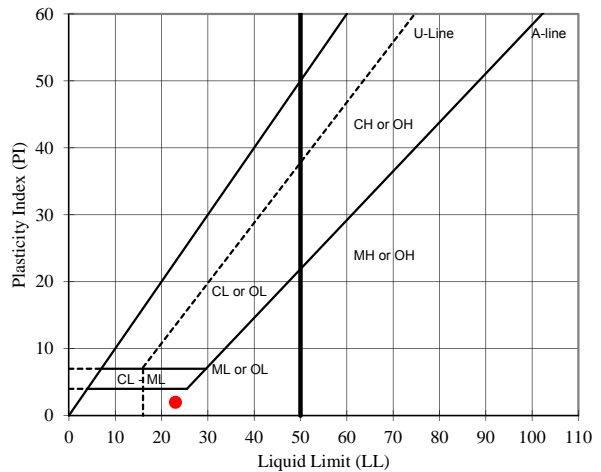
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-21**  
 TYPE: **Pail**

DEPTH (ft): **0-18**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.12
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	35.18
3/8-inch	9.5	88.1		
#4	4.8	64.7	Coarse Sand	17.11
#10	2.00	47.6		
#20	0.85	32.1	Medium Sand	25.09
#40	0.43	22.5		
#60	0.25	17.2	Fine Sand	11.95
#100	0.15	13.6		
#200	0.075	10.6	Silt or Clay Fines	10.55



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, yellowish red, dry

LL	PL	PI
23	21	2

As-Received Moisture Content (%) --

USCS Group Symbol SW-SM

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

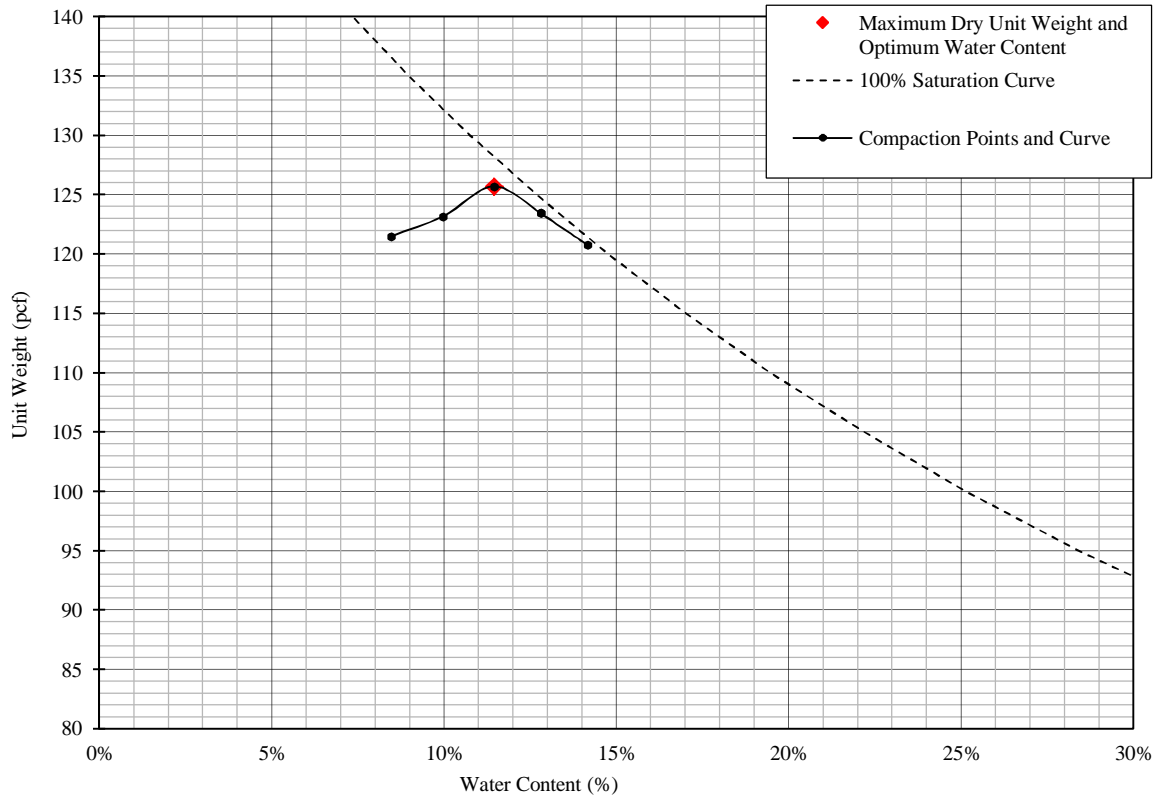
TECH AMS  
 DATE 2/26/2013  
 REVIEW MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-21**  
 TYPE: **Pail**

DEPTH (ft): **0-18**



% Test Fraction Passing 3/8-inch Sieve	<b>88%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.69</b>

Maximum Dry Unit Weight (pcf)	<b>125.7</b>
Optimum Water Content (%)	<b>11.4</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>129.2</b>
Corrected Optimum Water Content (%)	<b>10.1</b>

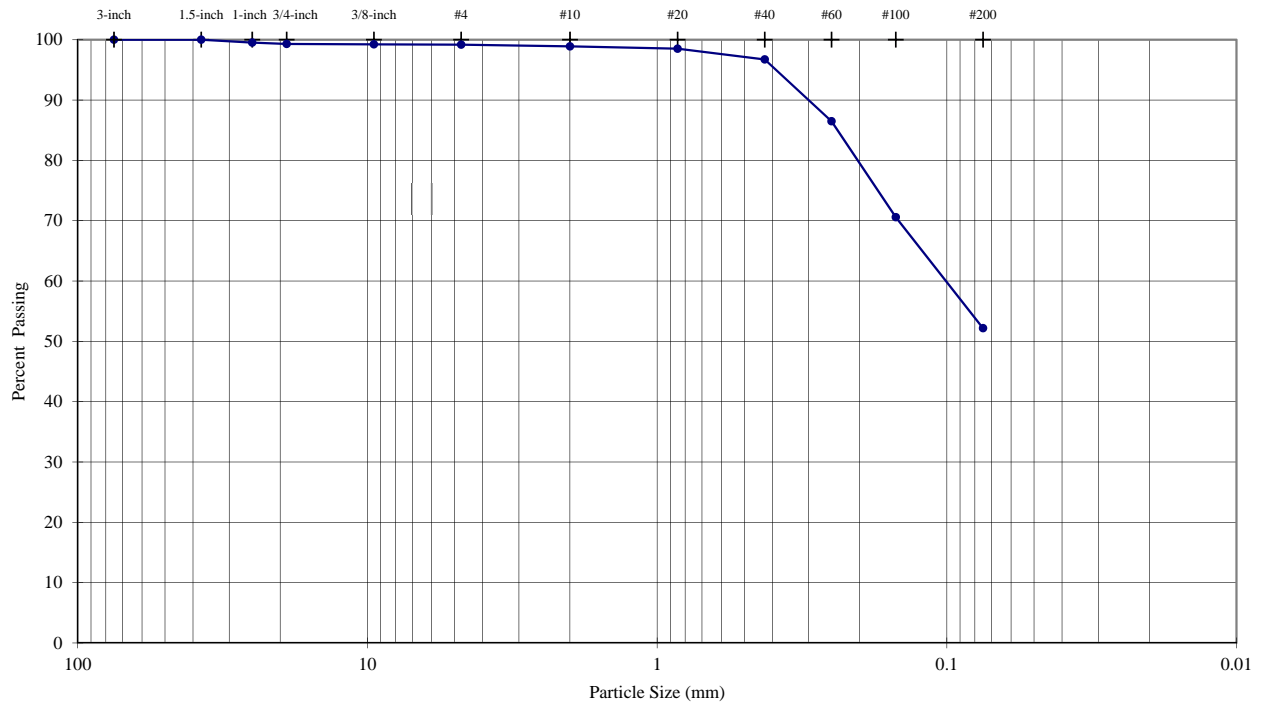
USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, yellowish red, dry

USCS SW-SM

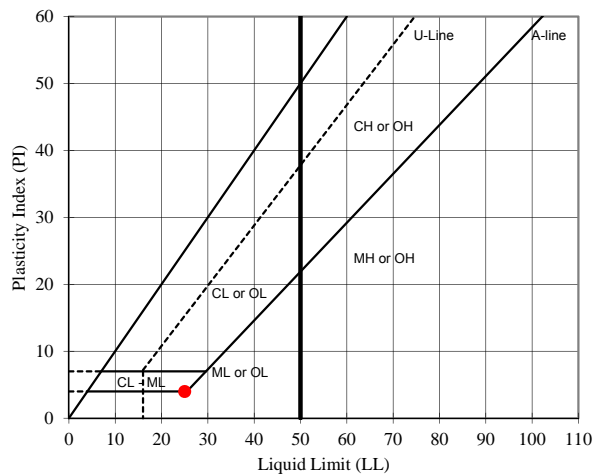
TECH	AMS
DATE	2-27-13
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-22** DEPTH (ft): **5-8.5**  
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.70
1.5-inch	37.5	100.0		
1-inch	25.0	99.5		
3/4-inch	19.0	99.3	Fine Gravel	0.12
3/8-inch	9.5	99.2		
#4	4.8	99.2	Coarse Sand	0.28
#10	2.00	98.9		
#20	0.85	98.5	Medium Sand	2.17
#40	0.43	96.7		
#60	0.25	86.5	Fine Sand	44.55
#100	0.15	70.6		
#200	0.075	52.2		
			Silt or Clay Fines	52.18



USCS Description (ASTM D 2487):  
 Sandy silty clay, brownish yellow, moist

LL	PL	PI
25	21	4

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 CL-ML

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

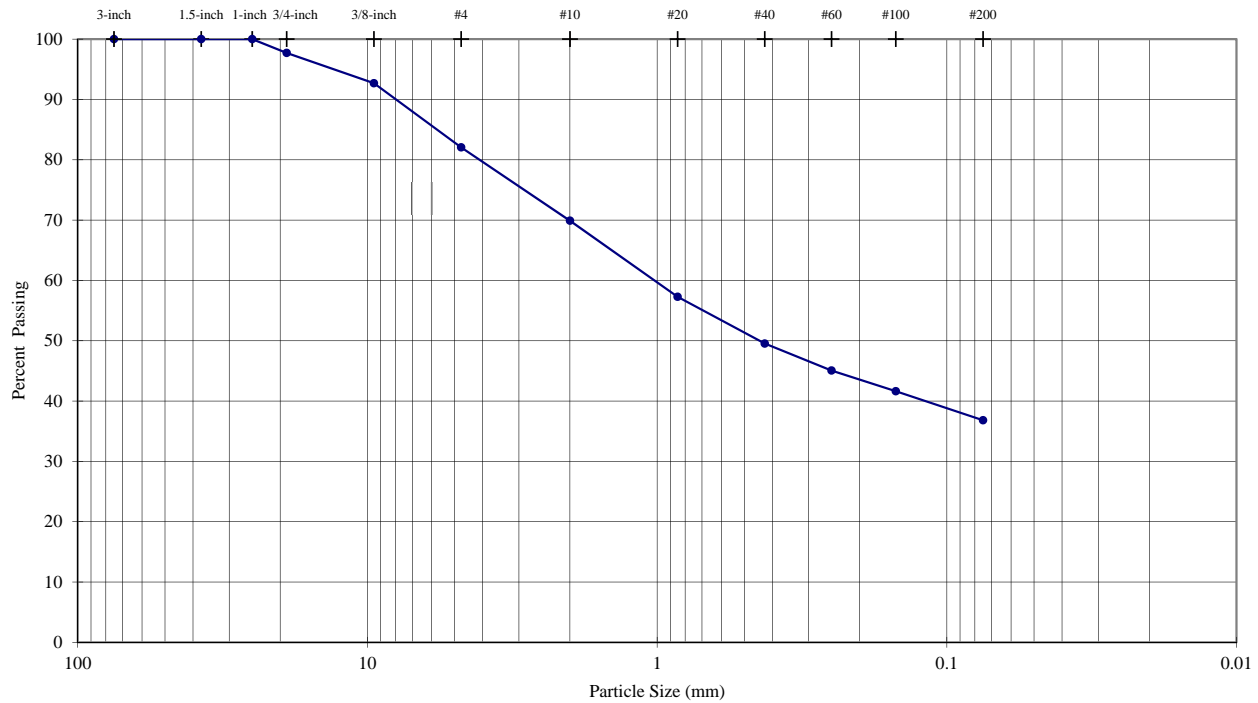
TECH	EH
DATE	2/26/2013
REVIEW	MB



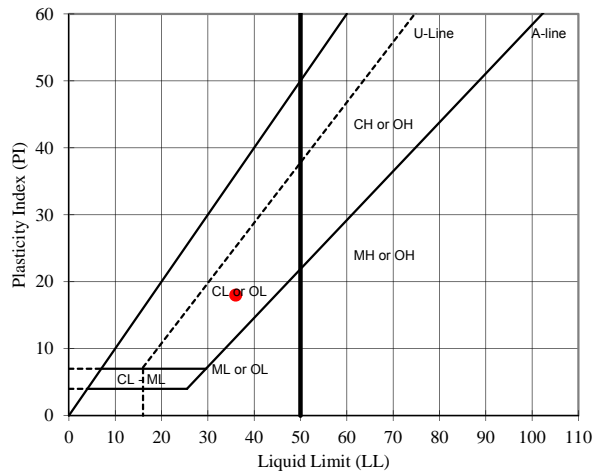
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-22**  
 TYPE: **Bag**

DEPTH (ft): **28-30**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	2.29
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	97.7		
3/8-inch	9.5	92.7	Fine Gravel	15.65
#4	4.8	82.1	Coarse Sand	12.16
#10	2.00	69.9		
#20	0.85	57.3		
#40	0.43	49.5	Medium Sand	20.36
#60	0.25	45.1		
#100	0.15	41.6	Fine Sand	12.71
#200	0.075	36.8		
			Silt or Clay Fines	36.82



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, reddish brown, moist

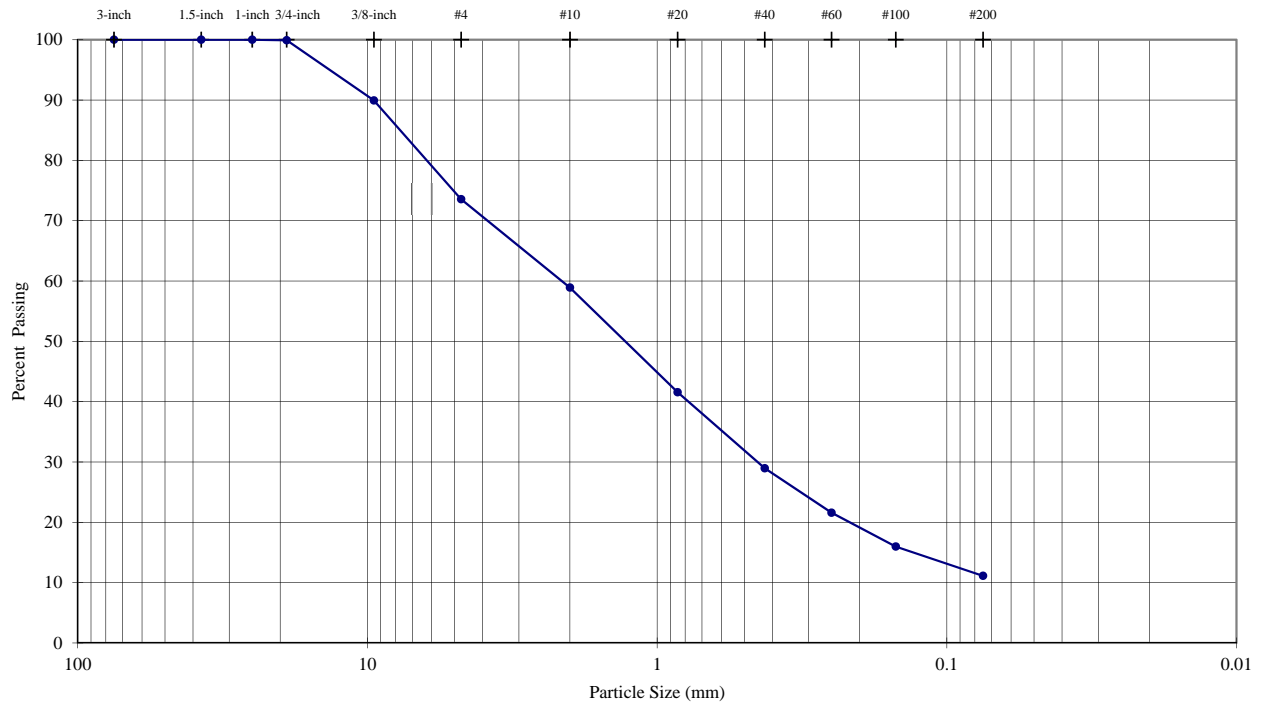
LL	PL	PI
36	18	18
As-Received Moisture Content (%)		
10.4		
USCS Group Symbol		
SC		

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

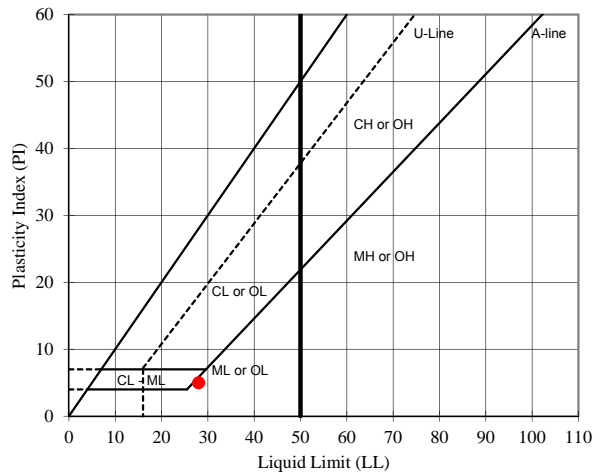
TECH: AMS  
 DATE: 2/25/2013  
 REVIEW: MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-24** DEPTH (ft): **2.5-18**  
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.09
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	26.35
3/8-inch	9.5	89.9		
#4	4.8	73.6	Coarse Sand	14.65
#10	2.0	58.9		
#20	0.85	41.6	Medium Sand	29.96
#40	0.43	29.0		
#60	0.25	21.6	Fine Sand	17.83
#100	0.15	16.0		
#200	0.075	11.1		
			Silt or Clay Fines	11.12



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, weak red, dry

LL	PL	PI
28	23	5

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 SW-SM

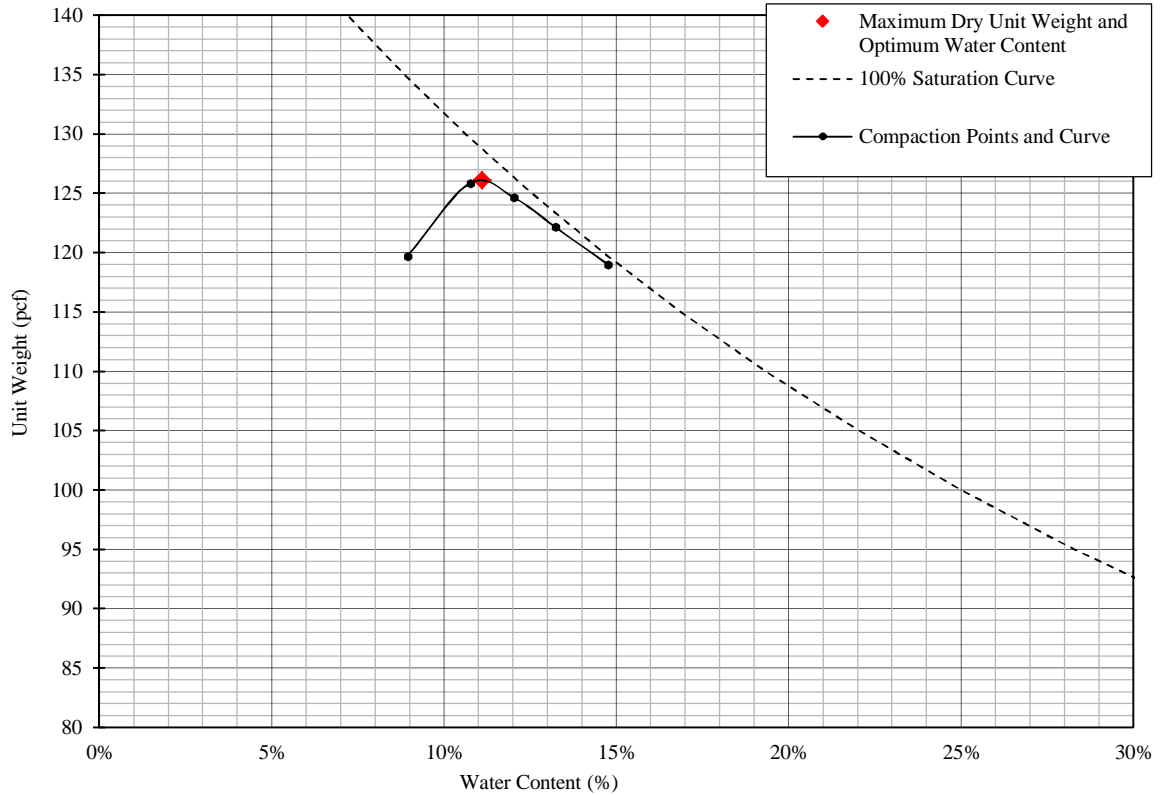
Notes: Og of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/27/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-24**      DEPTH (ft): **2.5-18**  
 TYPE: **Pail**



% Test Fraction Passing 3/8-inch Sieve	<b>90%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.68</b>

Maximum Dry Unit Weight (pcf)	<b>126.1</b>
Optimum Water Content (%)	<b>11.1</b>

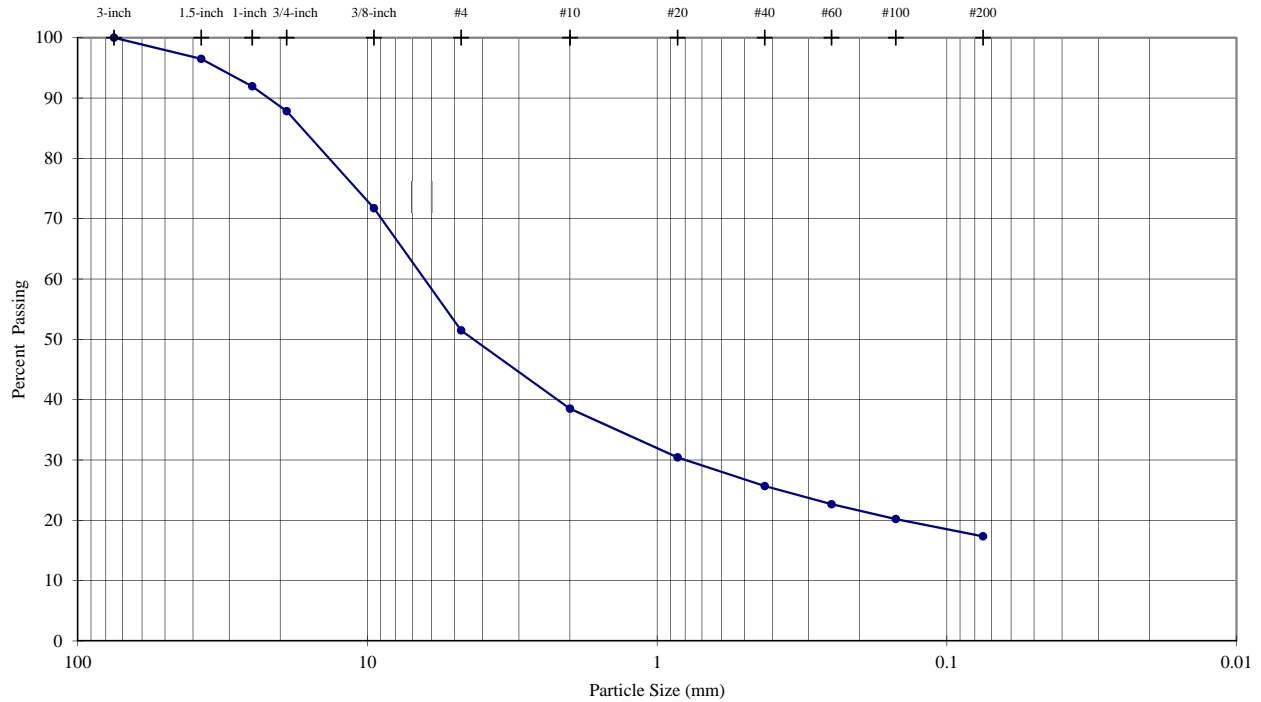
Corrected Maximum Dry Unit Weight (pcf)	<b>128.9</b>
Corrected Optimum Water Content (%)	<b>10.0</b>

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, weak red, dry  
 USCS: SW-SM

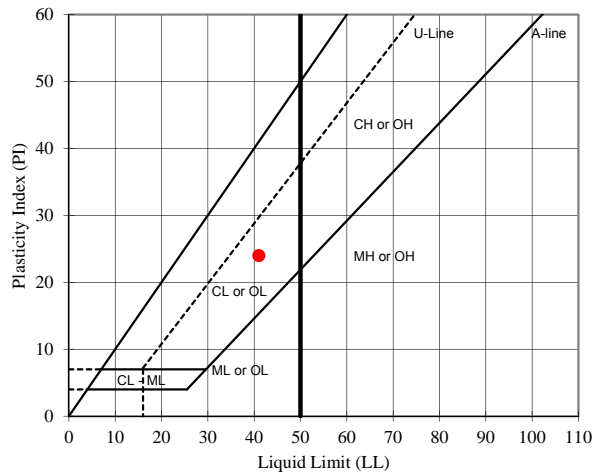
TECH	AMS
DATE	2-27-13
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-25** DEPTH (ft): **0-12.5**  
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	12.17
1.5-inch	37.5	96.5		
1-inch	25.0	91.9		
3/4-inch	19.0	87.8	Fine Gravel	36.36
3/8-inch	9.5	71.7		
#4	4.8	51.5	Coarse Sand	12.97
#10	2.00	38.5		
#20	0.85	30.4	Medium Sand	12.83
#40	0.43	25.7		
#60	0.25	22.7	Fine Sand	8.33
#100	0.15	20.2		
#200	0.075	17.3		
			Silt or Clay Fines	17.34



USCS Description (ASTM D 2487):  
 Clayey gravel with sand, dark yellowish brown, dry

LL	PL	PI
41	17	24

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 GC

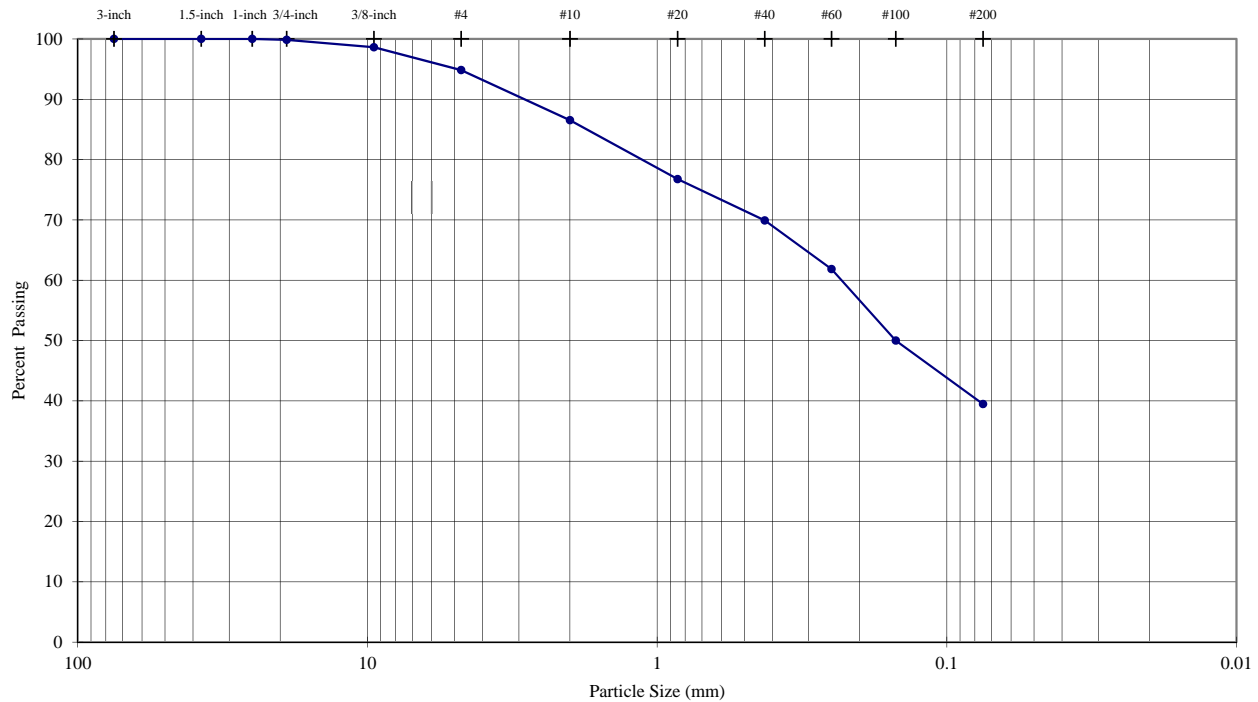
Notes: Og of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH AMS  
 DATE 2/27/2013  
 REVIEW MB

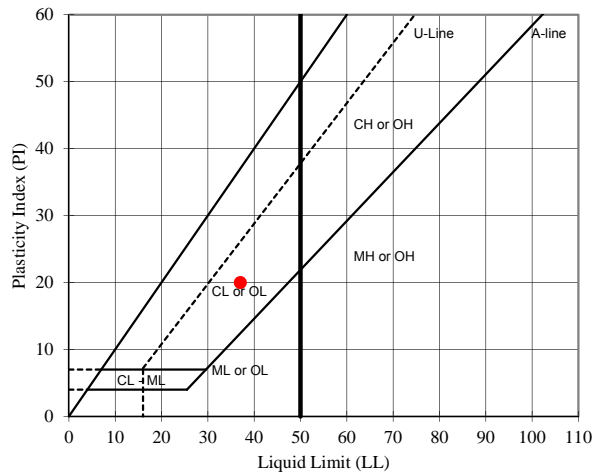
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **BH-25**  
 TYPE: **Pail**

DEPTH (ft): **22-34**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.16
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.8	Fine Gravel	5.01
3/8-inch	9.5	98.6		
#4	4.8	94.8	Coarse Sand	8.31
#10	2.00	86.5		
#20	0.85	76.8		
#40	0.43	69.9	Medium Sand	16.62
#60	0.25	61.8		
#100	0.15	50.0	Fine Sand	30.43
#200	0.075	39.5		
			Silt or Clay Fines	39.47



USCS Description (ASTM D 2487):  
 Clayey sand, reddish brown, moist

LL	PL	PI
37	17	20

As-Received Moisture Content (%)  
**#DIV/0!**

USCS Group Symbol  
**SC**

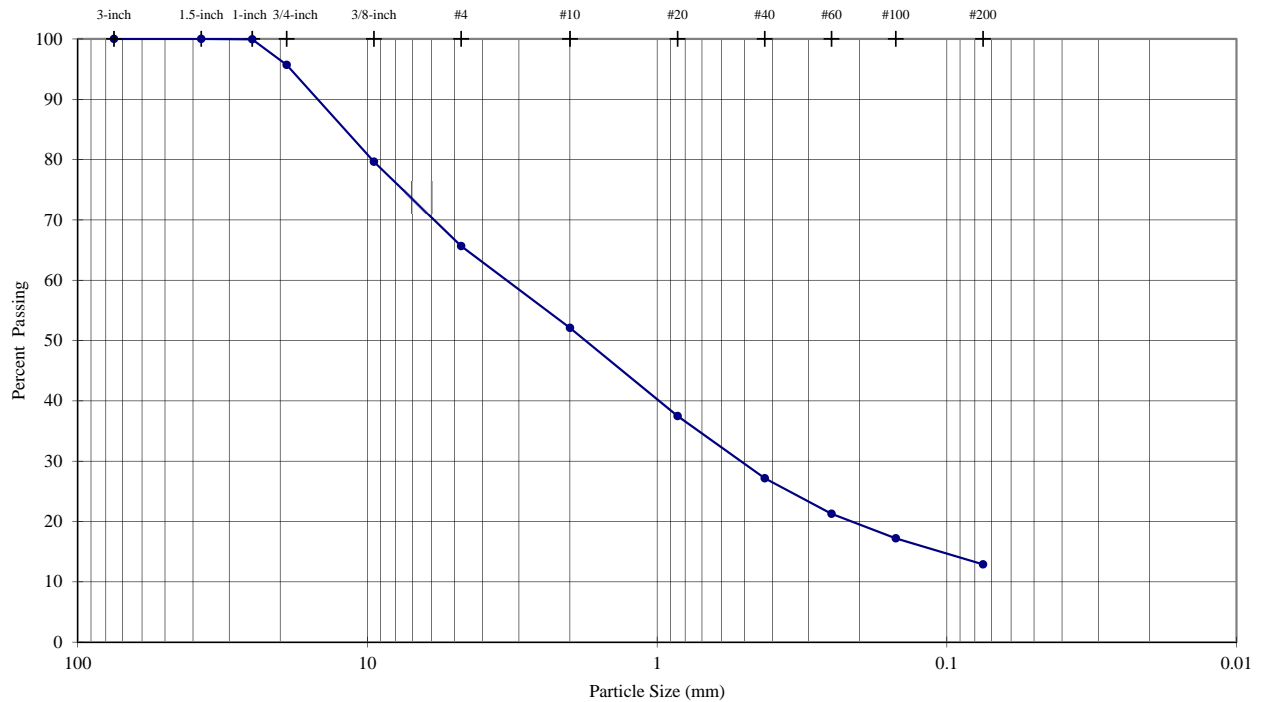
Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/26/2013
REVIEW	MB

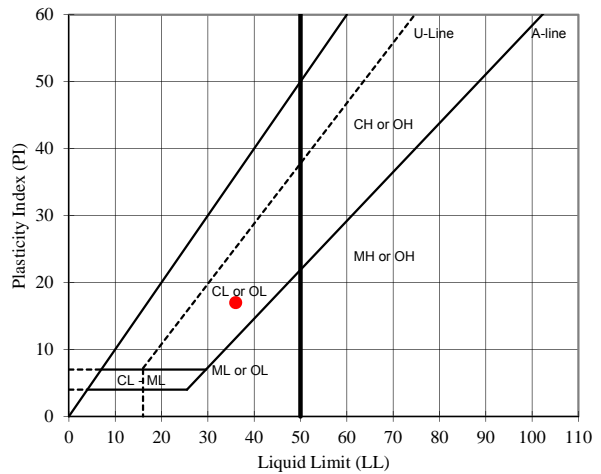
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #1**  
 TYPE: **Pails**

DEPTH (ft): **5-13**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>4.30</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>99.9</b>		
3/4-inch	19.0	<b>95.7</b>	Fine Gravel	<b>30.03</b>
3/8-inch	9.5	<b>79.6</b>		
#4	4.8	<b>65.7</b>	Coarse Sand	<b>13.57</b>
#10	2.00	<b>52.1</b>		
#20	0.85	<b>37.5</b>	Medium Sand	<b>24.93</b>
#40	0.43	<b>27.2</b>		
#60	0.25	<b>21.3</b>	Fine Sand	<b>14.27</b>
#100	0.15	<b>17.2</b>		
#200	0.075	<b>12.9</b>	Silt or Clay Fines	<b>12.90</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
<b>36</b>	<b>19</b>	<b>17</b>	<b>2.75</b>

As-Received Moisture Content (%) USCS Group Symbol

-- **SC**

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

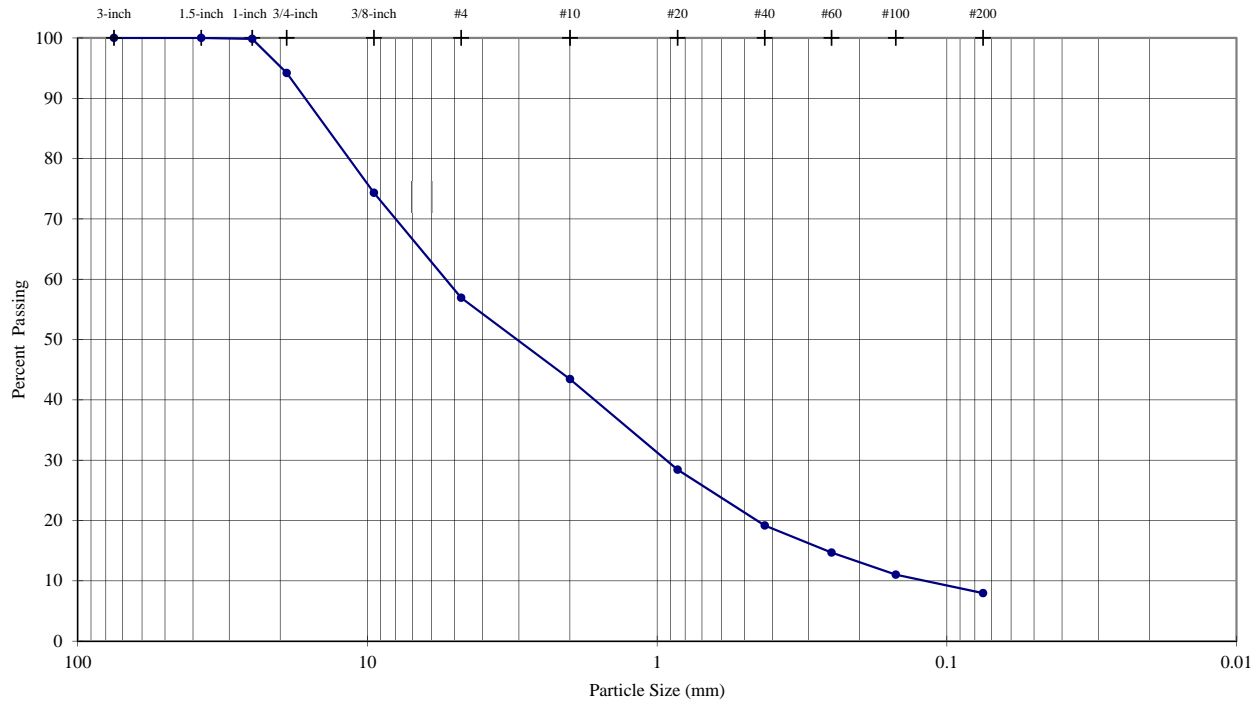
TECH	AMS
DATE	3/4/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

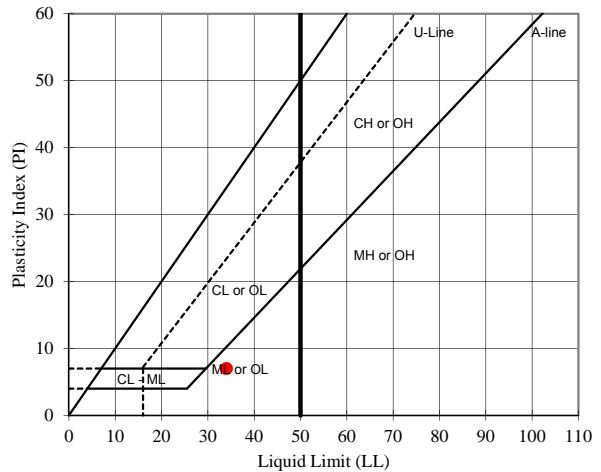
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #2**  
 TYPE: **Pails**

DEPTH (ft): **5-14**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	5.81
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	94.2	Fine Gravel	37.27
3/8-inch	9.5	74.3		
#4	4.8	56.9	Coarse Sand	13.48
#10	2.00	43.4		
#20	0.85	28.4	Medium Sand	24.28
#40	0.43	19.2		
#60	0.25	14.7	Fine Sand	11.20
#100	0.15	11.0		
#200	0.075	8.0		
			Silt or Clay Fines	7.96



USCS Description (ASTM D 2487):  
 Well-graded sand with silt and gravel, brown, dry

LL	PL	PI	SpG
34	27	7	2.74

As-Received Moisture Content (%)

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USCS Group Symbol

GW-GM

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

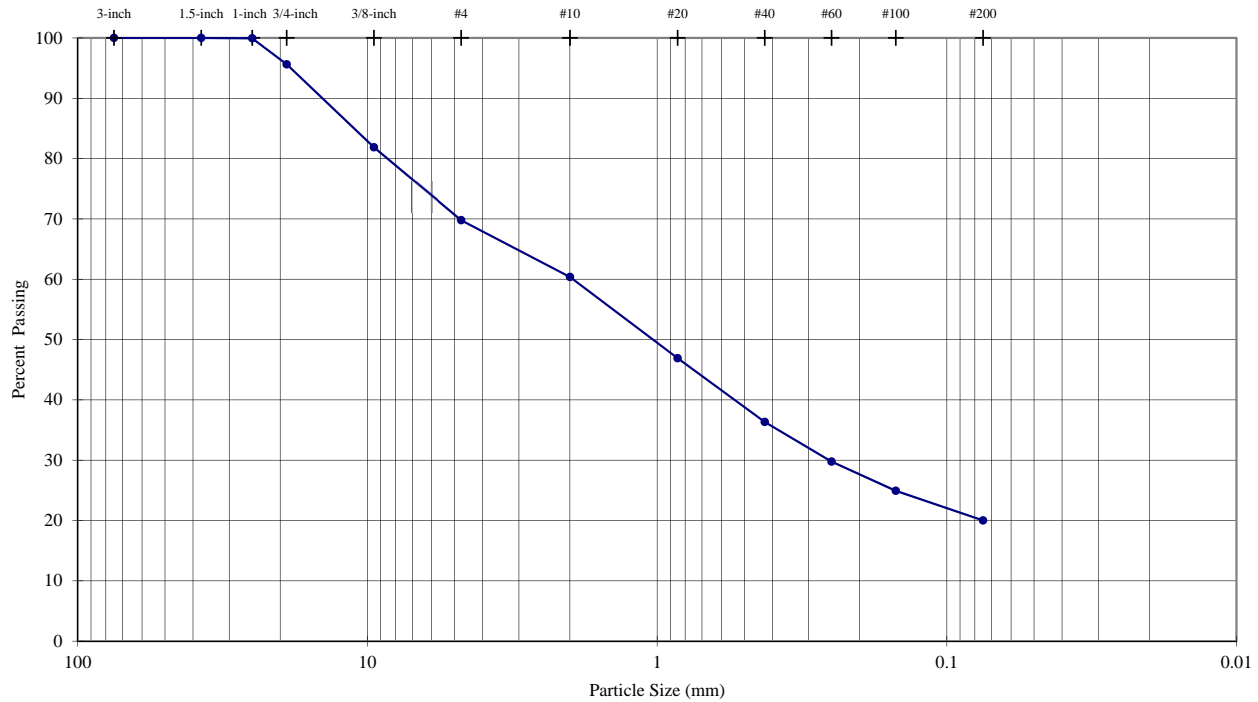
TECH	AMS
DATE	3/4/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**

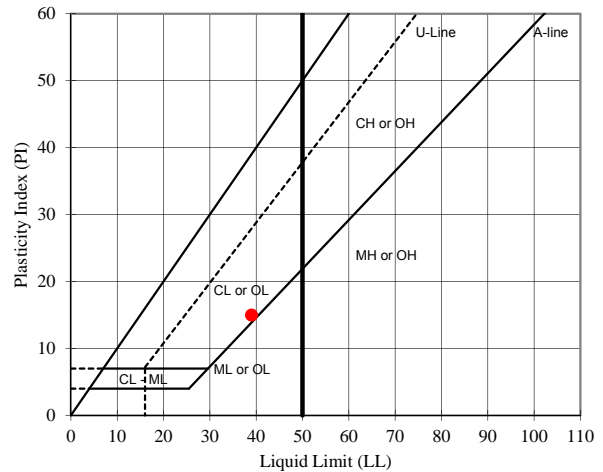
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #3**  
 TYPE: **Pails**

DEPTH (ft): **2-3**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	4.38
1.5-inch	37.5	100.0		
1-inch	25.0	99.9		
3/4-inch	19.0	95.6	Fine Gravel	25.84
3/8-inch	9.5	81.9		
#4	4.8	69.8	Coarse Sand	9.40
#10	2.00	60.4		
#20	0.85	46.9	Medium Sand	24.04
#40	0.43	36.3		
#60	0.25	29.8	Fine Sand	16.32
#100	0.15	24.9		
#200	0.075	20.0		
			Silt or Clay Fines	20.01



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
39	24	15	2.67

As-Received Moisture Content (%)

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USCS Group Symbol

SC

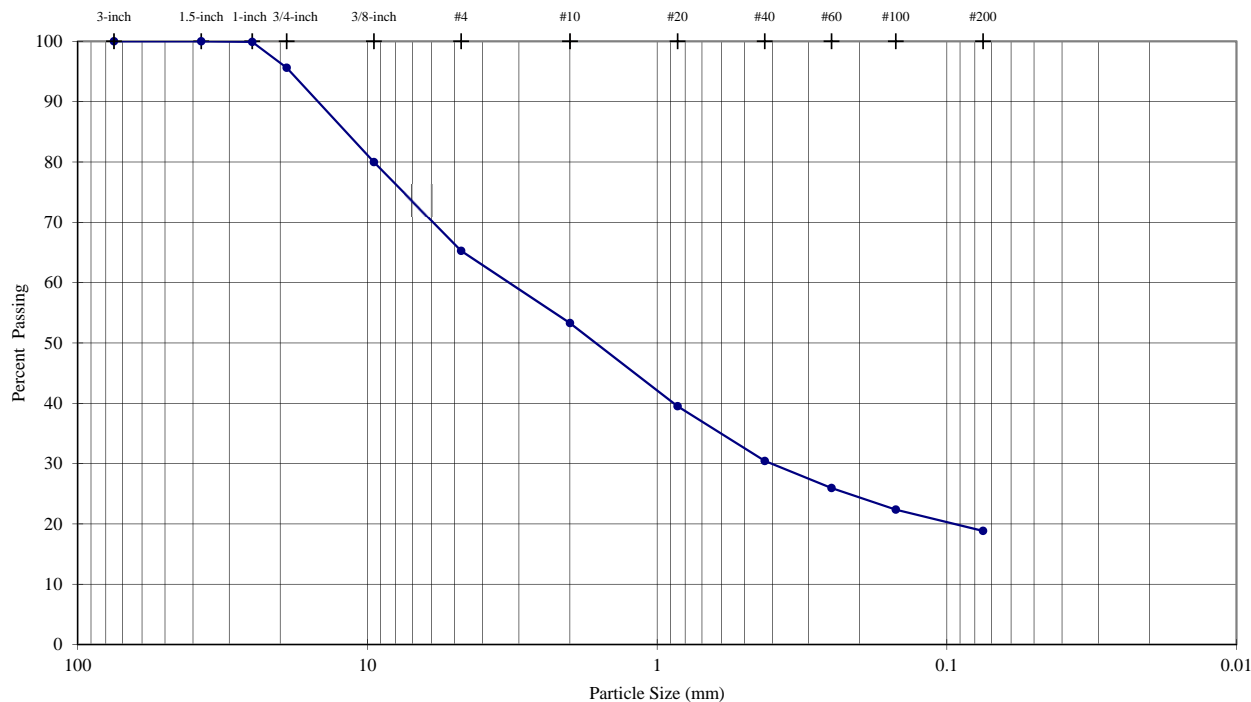
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

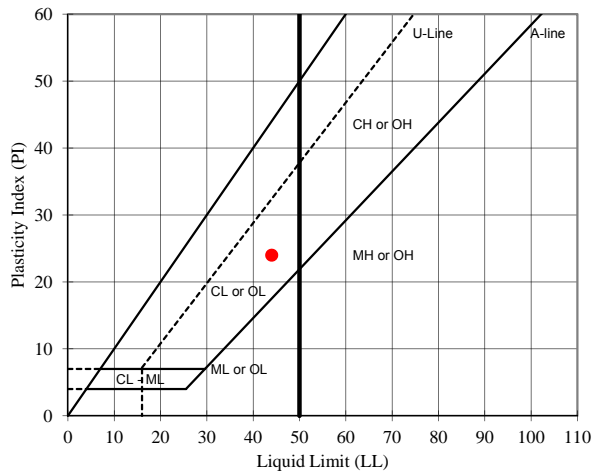


**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Composite #4**  
 TYPE: **Pails** DEPTH (ft): **0-10**



Sieve	Particle Size		Description	Percentage
	Sieve	(mm)		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>4.38</b>
1.5-inch	37.5	<b>100.0</b>		
1-inch	25.0	<b>99.9</b>		
3/4-inch	19.0	<b>95.6</b>	Fine Gravel	<b>30.35</b>
3/8-inch	9.5	<b>80.0</b>		
#4	4.8	<b>65.3</b>	Coarse Sand	<b>11.97</b>
#10	2.00	<b>53.3</b>		
#20	0.85	<b>39.5</b>		
#40	0.43	<b>30.4</b>	Medium Sand	<b>22.86</b>
#60	0.25	<b>26.0</b>		
#100	0.15	<b>22.4</b>	Fine Sand	<b>11.60</b>
#200	0.075	<b>18.8</b>		
			Silt or Clay Fines	<b>18.84</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, brown, dry

LL	PL	PI	SpG
<b>44</b>	<b>20</b>	<b>24</b>	<b>2.70</b>

As-Received Moisture Content (%) **--**      USCS Group Symbol **SC**

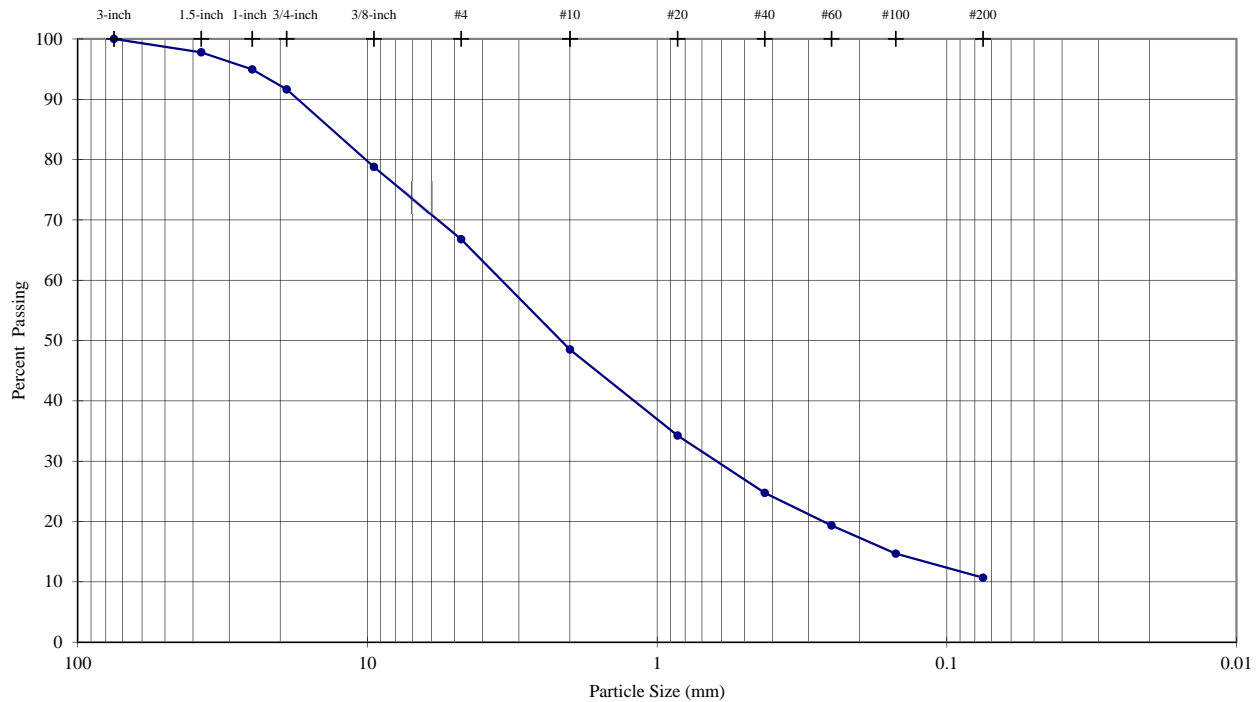
Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	3/4/2013
REVIEW	MB

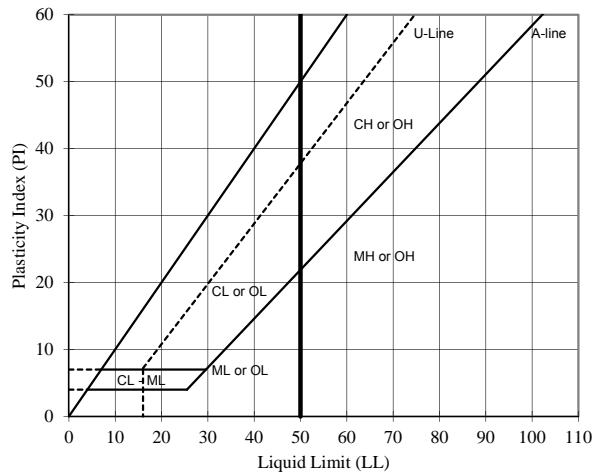
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-5**  
 TYPE: **Pail**

DEPTH (ft): **3-7**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>8.37</b>
1.5-inch	37.5	<b>97.8</b>		
1-inch	25.0	<b>94.9</b>		
3/4-inch	19.0	<b>91.6</b>	Fine Gravel	<b>24.84</b>
3/8-inch	9.5	<b>78.8</b>		
#4	4.8	<b>66.8</b>	Coarse Sand	<b>18.28</b>
#10	2.00	<b>48.5</b>		
#20	0.85	<b>34.2</b>		
#40	0.43	<b>24.7</b>	Medium Sand	<b>23.77</b>
#60	0.25	<b>19.3</b>		
#100	0.15	<b>14.7</b>	Fine Sand	<b>14.07</b>
#200	0.075	<b>10.7</b>		
			Silt or Clay Fines	<b>10.68</b>



Visual Description (Golder Procedure):  
 gravelly SAND, some non-plastic fines, yellowish brown, dry

LL	PL	PI
#VALUE!	#DIV/0!	#VALUE!

As-Received Moisture Content (%)  
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USCS Group Symbol  
 --

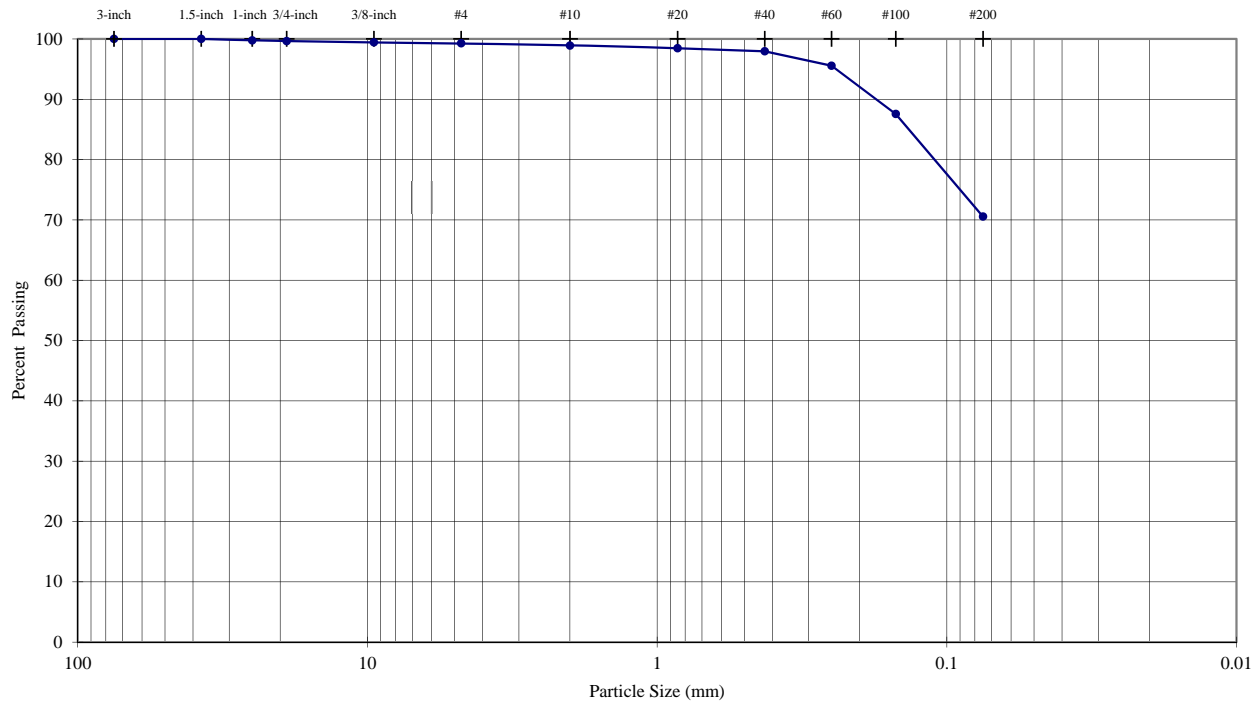
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

TECH	AMS
DATE	2/27/2013
REVIEW	MB

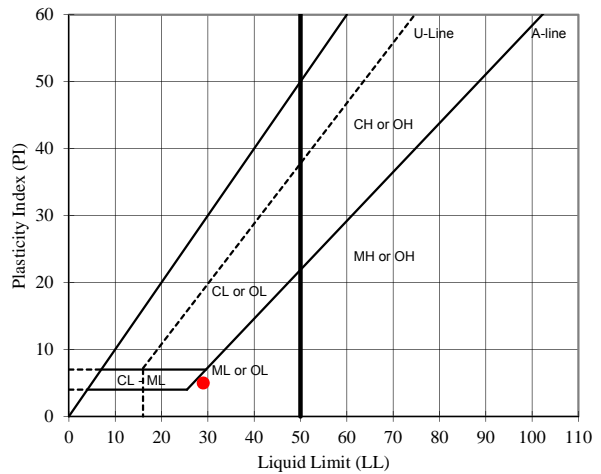
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-10**  
 TYPE: **Pail**

DEPTH (ft): **3-5(12)**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.37
1.5-inch	37.5	100.0		
1-inch	25.0	99.8		
3/4-inch	19.0	99.6	Fine Gravel	0.36
3/8-inch	9.5	99.4		
#4	4.8	99.3	Coarse Sand	0.38
#10	2.00	98.9		
#20	0.85	98.5	Medium Sand	0.95
#40	0.43	97.9		
#60	0.25	95.6	Fine Sand	27.39
#100	0.15	87.6		
#200	0.075	70.6		
			Silt or Clay Fines	70.55



USCS Description (ASTM D 2487):  
**Silt with sand, yellowish brown, dry**

LL	PL	PI
<b>29</b>	<b>24</b>	<b>5</b>

As-Received Moisture Content (%)

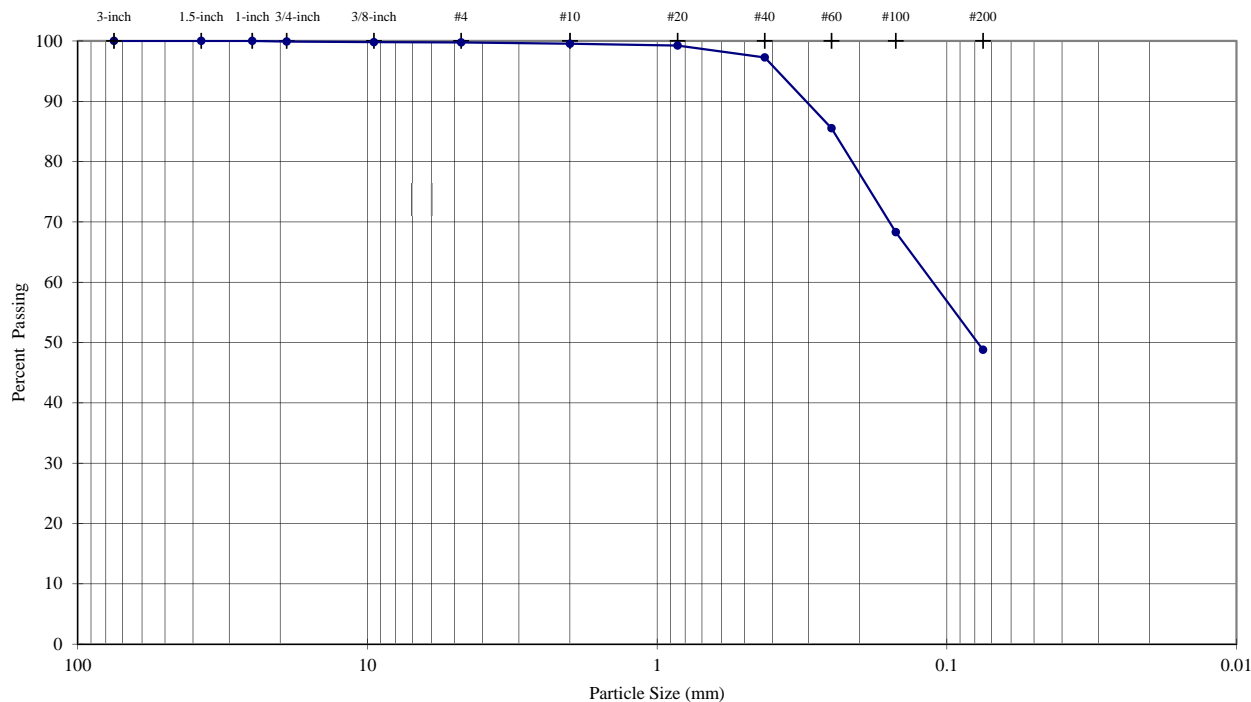
USCS Group Symbol

Notes: 0g of particles up to 37.5mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

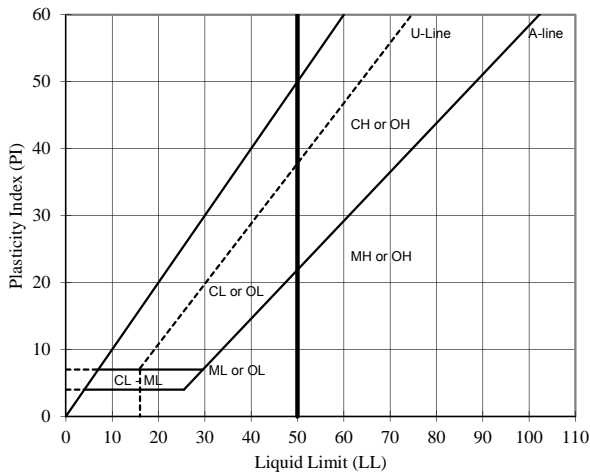
TECH	AMS
DATE	3/1/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-11** DEPTH (ft): **3-11**  
 TYPE: **Pail**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.11
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	99.9	Fine Gravel	0.14
3/8-inch	9.5	99.8		
#4	4.8	99.8	Coarse Sand	0.22
#10	2.00	99.5		
#20	0.85	99.2		
#40	0.43	97.3	Medium Sand	2.27
#60	0.25	85.5		
#100	0.15	68.3	Fine Sand	48.47
#200	0.075	48.8		
			Silt or Clay Fines	48.80



USCS Description (ASTM D 2487):  
**Silty sand, brownish yellow, moist**

LL	PL	PI	SpG
NP	NP	NP	2.74

As-Received Moisture Content (%)  
**--**

USCS Group Symbol  
**SM**

Notes: 0g of particles up to 25.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

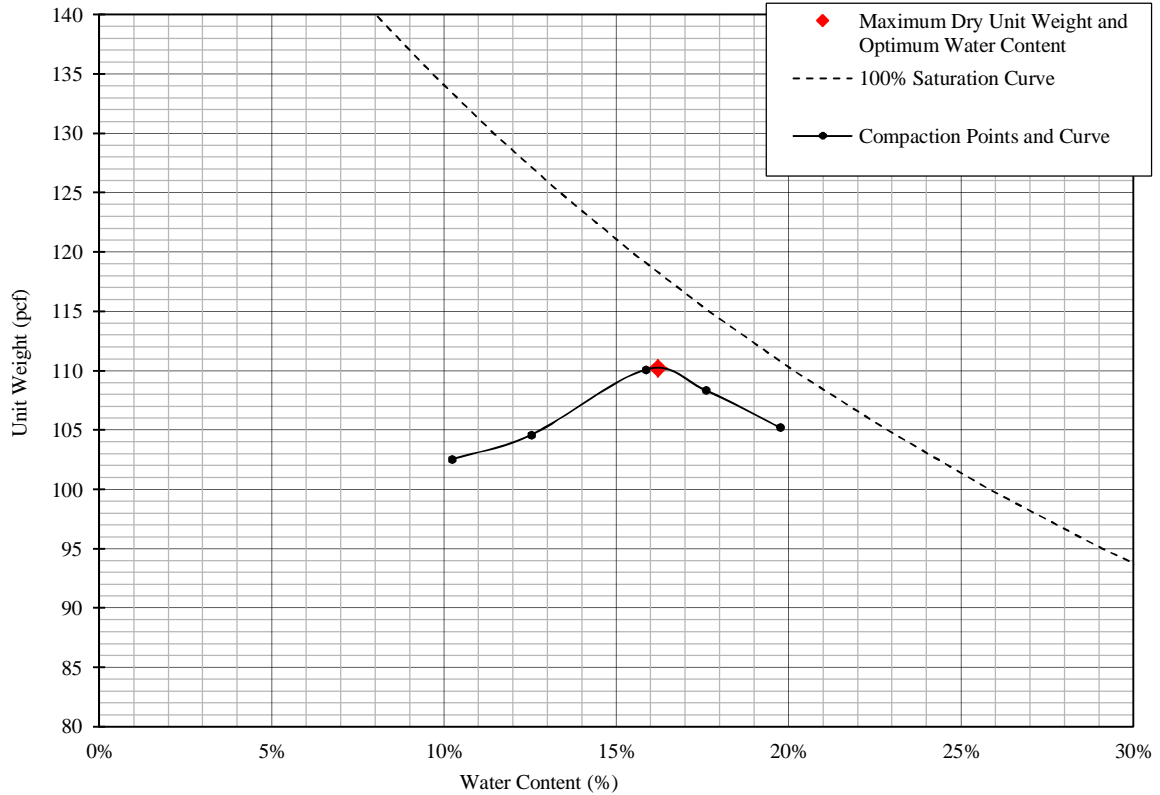
TECH **AM**  
 DATE **2/20/2013**  
 REVIEW **MB**

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-11**  
 TYPE: **Pail**

DEPTH (ft): **3-11**



% Test Fraction Passing #4 Sieve	<b>100%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM D854)	<b>2.74</b>

Maximum Dry Unit Weight (pcf)	<b>110.2</b>
Optimum Water Content (%)	<b>16.2</b>

USCS Description (ASTM D 2487): Silty sand, brownish yellow, moist

USCS SM

TECH	EH
DATE	2-22-2013
REVIEW	MB

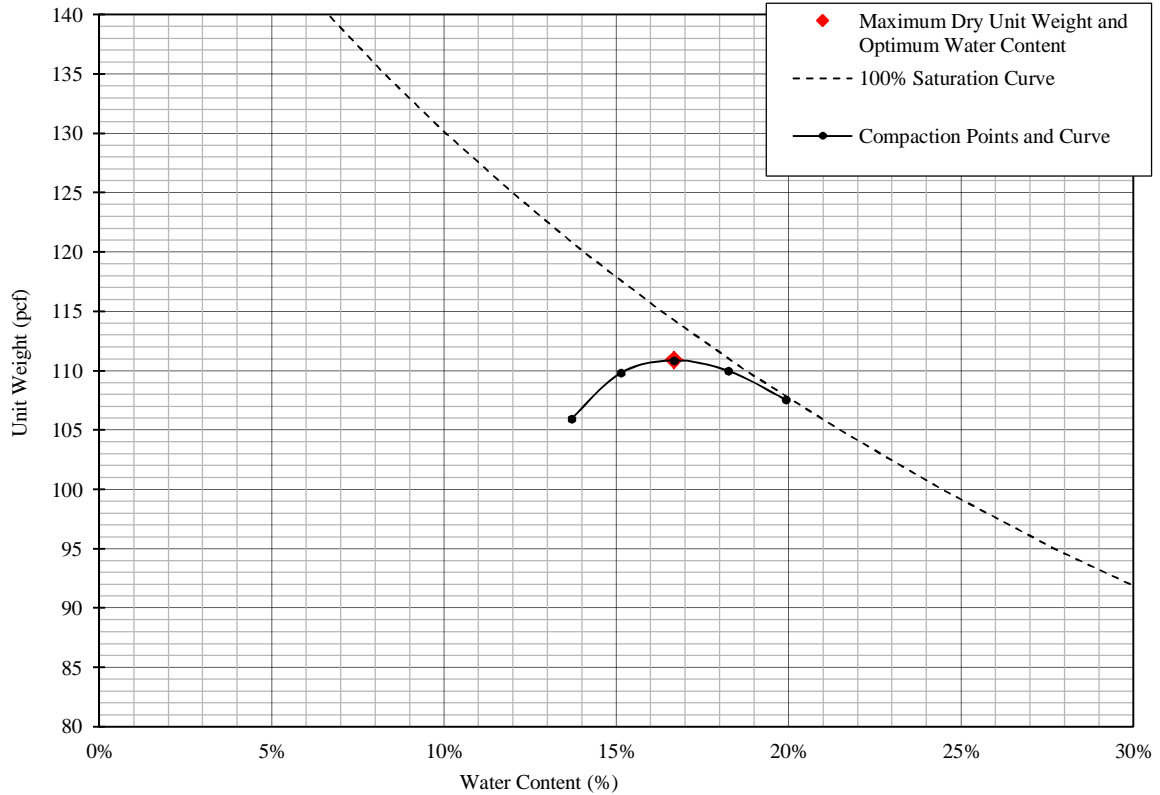


## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20**  
 TYPE: **Pail/Bag**

DEPTH (ft): **0-4**



% Test Fraction Passing #4 Sieve	<b>82%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.64</b>

Maximum Dry Unit Weight (pcf)	<b>110.9</b>
Optimum Water Content (%)	<b>16.7</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>116.9</b>
Corrected Optimum Water Content (%)	<b>13.9</b>

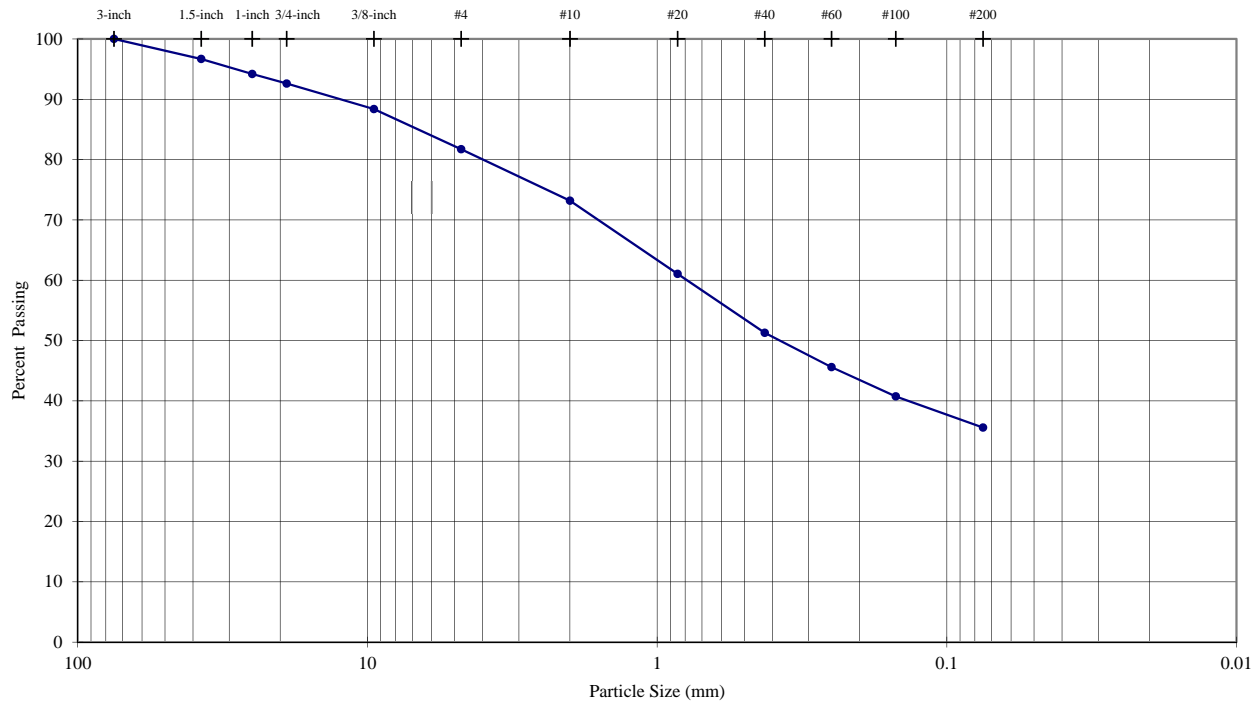
**USCS Description (ASTM D 2487):** Clayey sand with gravel, strong brown, wet

<b>USCS</b>	SC
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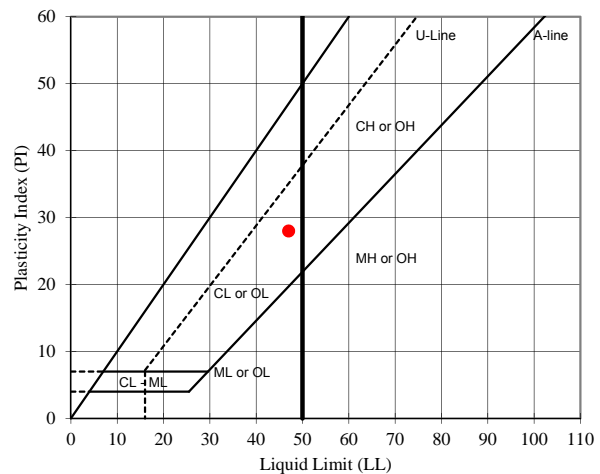
<b>TECH</b>	AM
<b>DATE</b>	2-27-13
<b>REVIEW</b>	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20** DEPTH (ft): **0-4**  
 TYPE: **Pail/Bag**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>7.40</b>
1.5-inch	37.5	<b>96.7</b>		
1-inch	25.0	<b>94.2</b>		
3/4-inch	19.0	<b>92.6</b>	Fine Gravel	<b>10.89</b>
3/8-inch	9.5	<b>88.4</b>		
#4	4.8	<b>81.7</b>	Coarse Sand	<b>8.53</b>
#10	2.00	<b>73.2</b>		
#20	0.85	<b>61.0</b>		
#40	0.43	<b>51.3</b>	Medium Sand	<b>21.90</b>
#60	0.25	<b>45.6</b>		
#100	0.15	<b>40.7</b>	Fine Sand	<b>15.70</b>
#200	0.075	<b>35.6</b>		
			Silt or Clay Fines	<b>35.57</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, strong brown, wet

LL	PL	PI
<b>47</b>	<b>19</b>	<b>28</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SC**

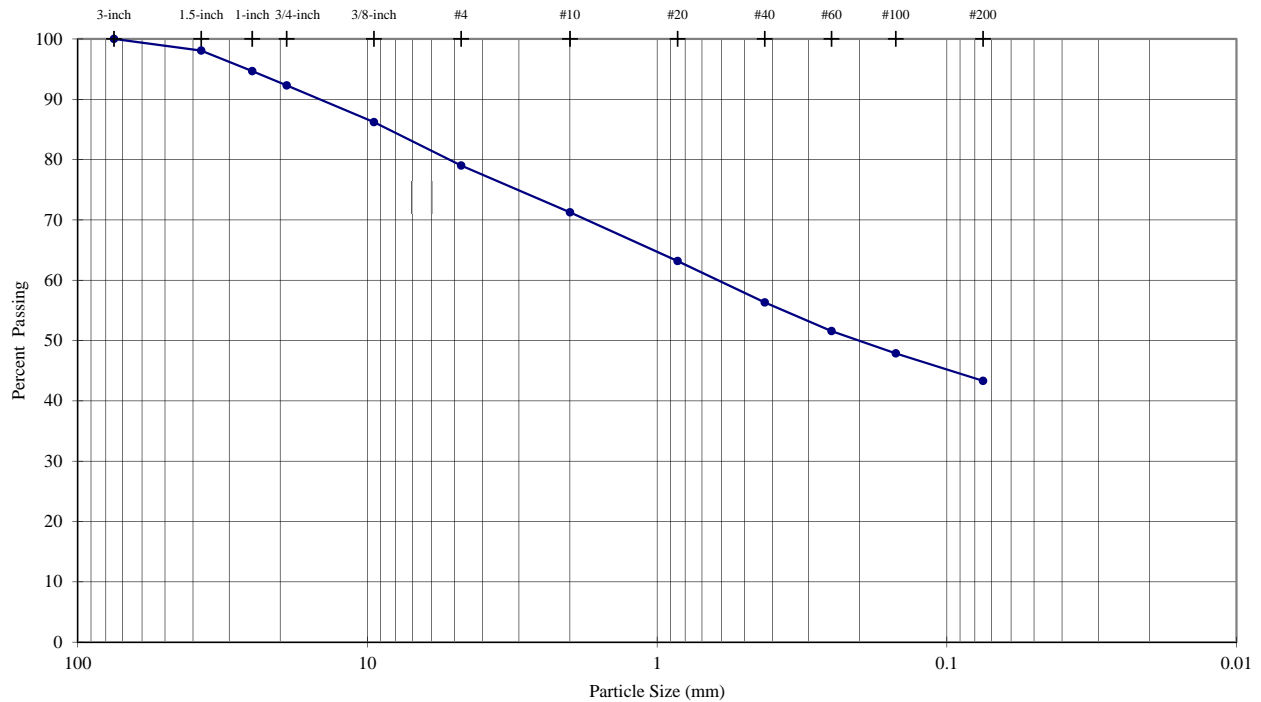
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/25/2013
REVIEW	MB

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

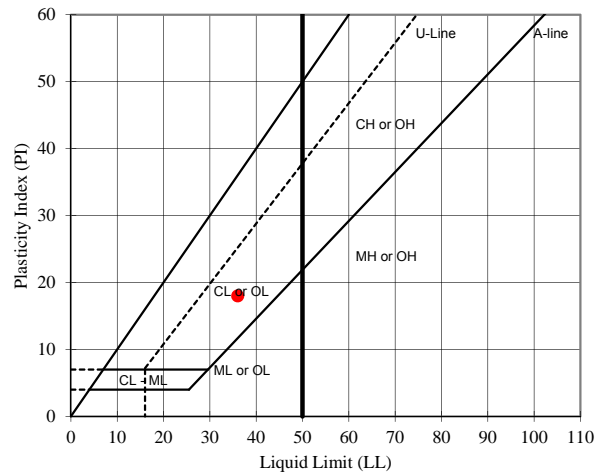
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20**  
 TYPE: **Pail/Bag**

DEPTH (ft): **4-7**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>7.71</b>
1.5-inch	37.5	<b>98.1</b>		
1-inch	25.0	<b>94.7</b>		
3/4-inch	19.0	<b>92.3</b>	Fine Gravel	<b>13.28</b>
3/8-inch	9.5	<b>86.2</b>		
#4	4.8	<b>79.0</b>	Coarse Sand	<b>7.75</b>
#10	2.00	<b>71.3</b>		
#20	0.85	<b>63.2</b>		
#40	0.43	<b>56.3</b>	Medium Sand	<b>14.95</b>
#60	0.25	<b>51.6</b>		
#100	0.15	<b>47.9</b>	Fine Sand	<b>12.99</b>
#200	0.075	<b>43.3</b>		
			Silt or Clay Fines	<b>43.32</b>

Sieve Analysis (Initial Separation on No. 4 Sieve)



USCS Description (ASTM D 2487):

Clayey sand with gravel, brown, dry

LL	PL	PI
<b>36</b>	<b>18</b>	<b>18</b>

As-Received Moisture Content (%)

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USCS Group Symbol

**SC**

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AMS
DATE	2/25/2013
REVIEW	MB

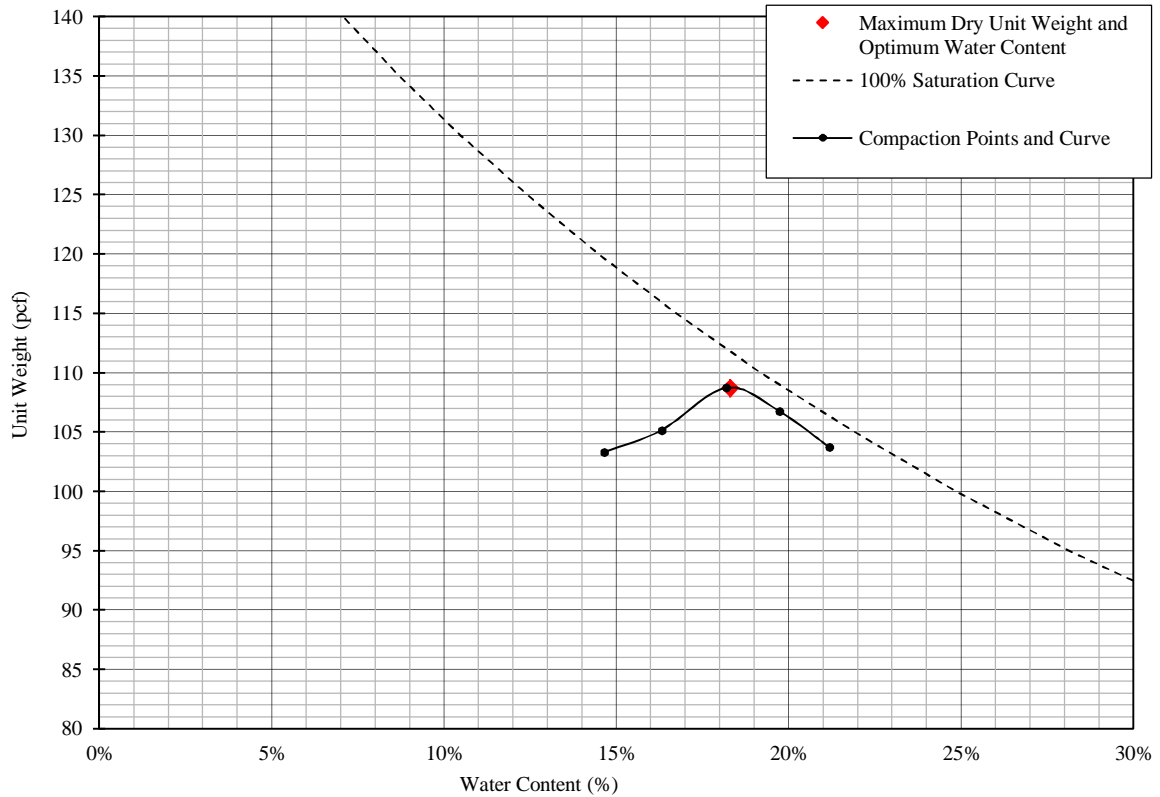


## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer   
 Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-20**  
 TYPE: **Pail/Bag**

DEPTH (ft): **4-7**



% Test Fraction Passing #4 Sieve	<b>79%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.67</b>

Maximum Dry Unit Weight (pcf)	<b>108.7</b>
Optimum Water Content (%)	<b>18.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>116.1</b>
Corrected Optimum Water Content (%)	<b>14.8</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, brown, dry

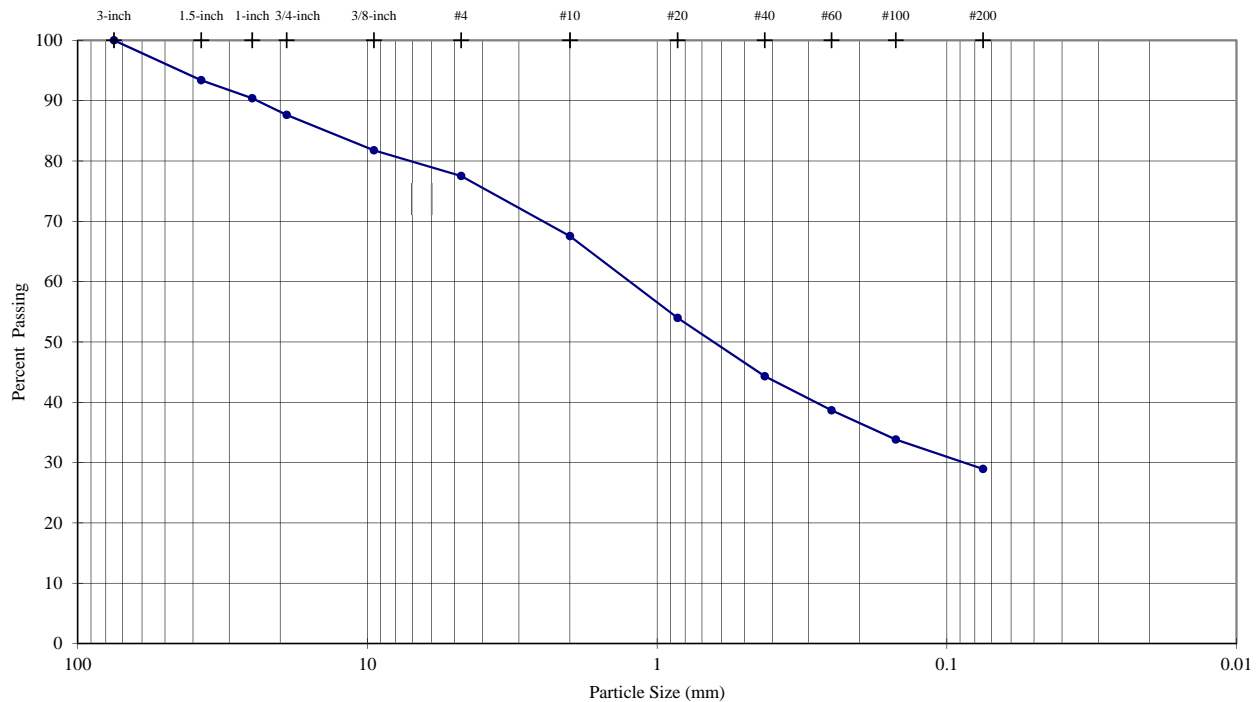
USCS SC

TECH	AMS
DATE	2-28-13
REVIEW	MB

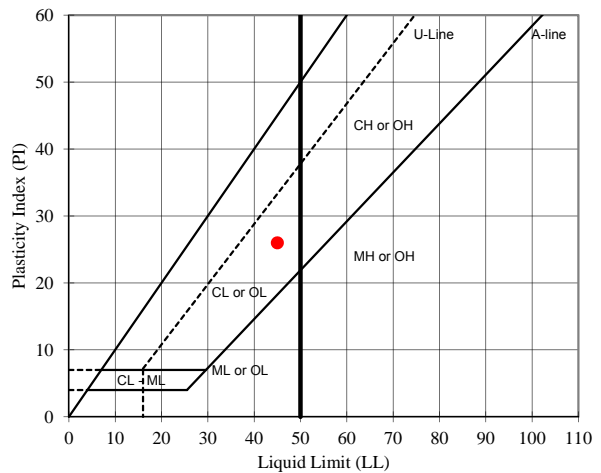
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-24**  
 TYPE: **Pail**

DEPTH (ft): **3-5**



Sieve	Particle Size	% Passing	Description	Percentage
	(mm)			
3-inch	75.0	100.0	Coarse Gravel	12.37
1.5-inch	37.5	93.4		
1-inch	25.0	90.4		
3/4-inch	19.0	87.6	Fine Gravel	10.12
3/8-inch	9.5	81.8		
#4	4.8	77.5	Coarse Sand	9.97
#10	2.00	67.5		
#20	0.85	54.0		
#40	0.43	44.3	Medium Sand	23.23
#60	0.25	38.7		
#100	0.15	33.8	Fine Sand	15.37
#200	0.075	28.9		
			Silt or Clay Fines	28.94



USCS Description (ASTM D 2487):

Clayey sand with gravel, yellowish brown, dry

LL	PL	PI
45	19	26

As-Received Moisture Content (%)

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USCS Group Symbol

SC

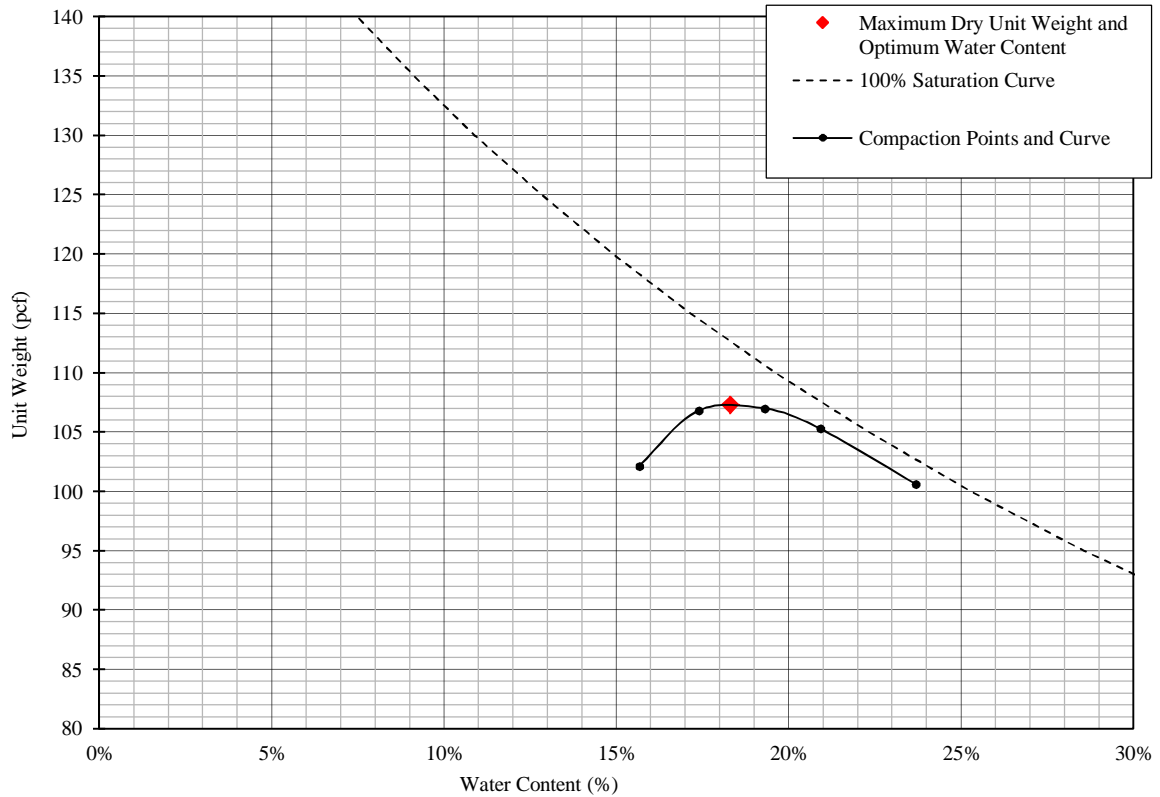
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM
DATE	2/20/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-24**      DEPTH (ft): **3-5**  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>79%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>107.3</b>
Optimum Water Content (%)	<b>18.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>114.7</b>
Corrected Optimum Water Content (%)	<b>14.4</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, yellowish brown, dry

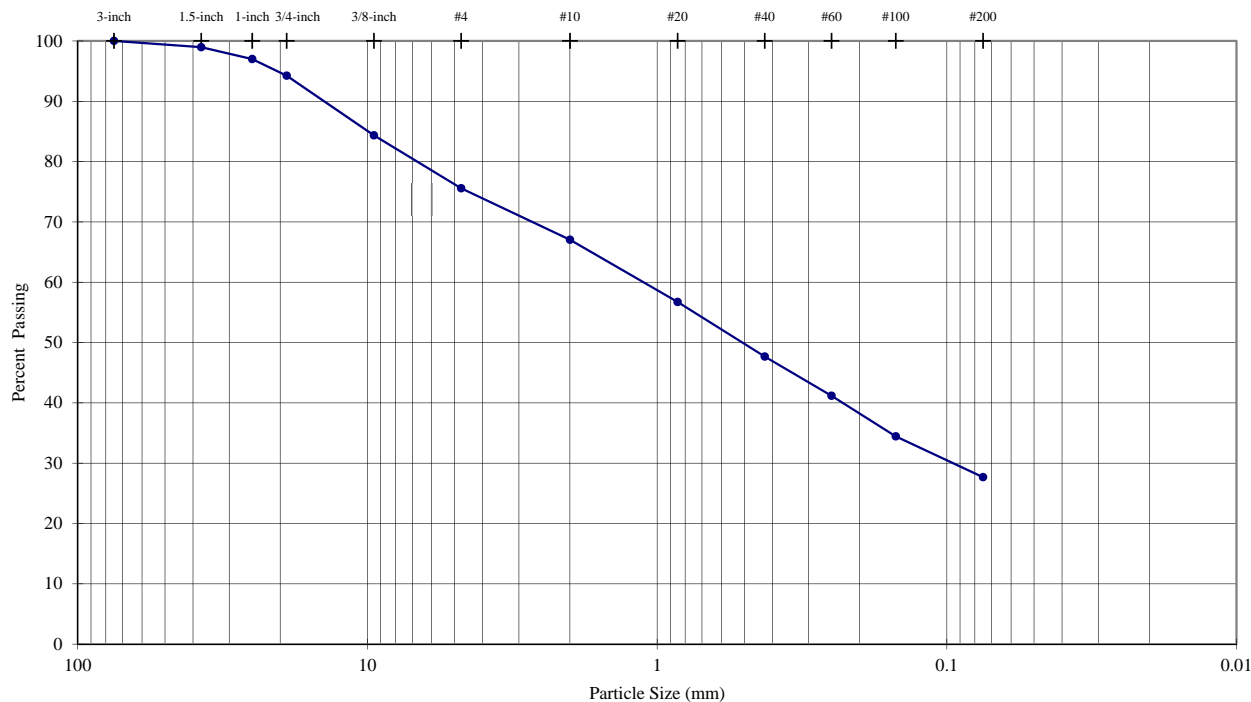
USCS SC

TECH	EH
DATE	2-22-2013
REVIEW	MB

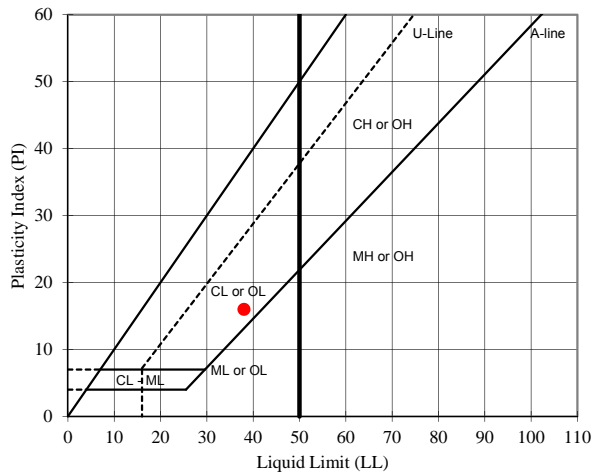


**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27** DEPTH (ft): **0-2**  
 TYPE: **Pail**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>5.76</b>
1.5-inch	37.5	<b>99.0</b>		
1-inch	25.0	<b>97.0</b>		
3/4-inch	19.0	<b>94.2</b>	Fine Gravel	<b>18.67</b>
3/8-inch	9.5	<b>84.4</b>		
#4	4.8	<b>75.6</b>	Coarse Sand	<b>8.53</b>
#10	2.00	<b>67.0</b>		
#20	0.85	<b>56.7</b>	Medium Sand	<b>19.37</b>
#40	0.43	<b>47.7</b>		
#60	0.25	<b>41.2</b>	Fine Sand	<b>19.99</b>
#100	0.15	<b>34.4</b>		
#200	0.075	<b>27.7</b>	Silt or Clay Fines	<b>27.69</b>



USCS Description (ASTM D 2487):  
 Clayey sand with gravel, dark yellowish brown, dry

LL	PL	PI
<b>38</b>	<b>22</b>	<b>16</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SC**

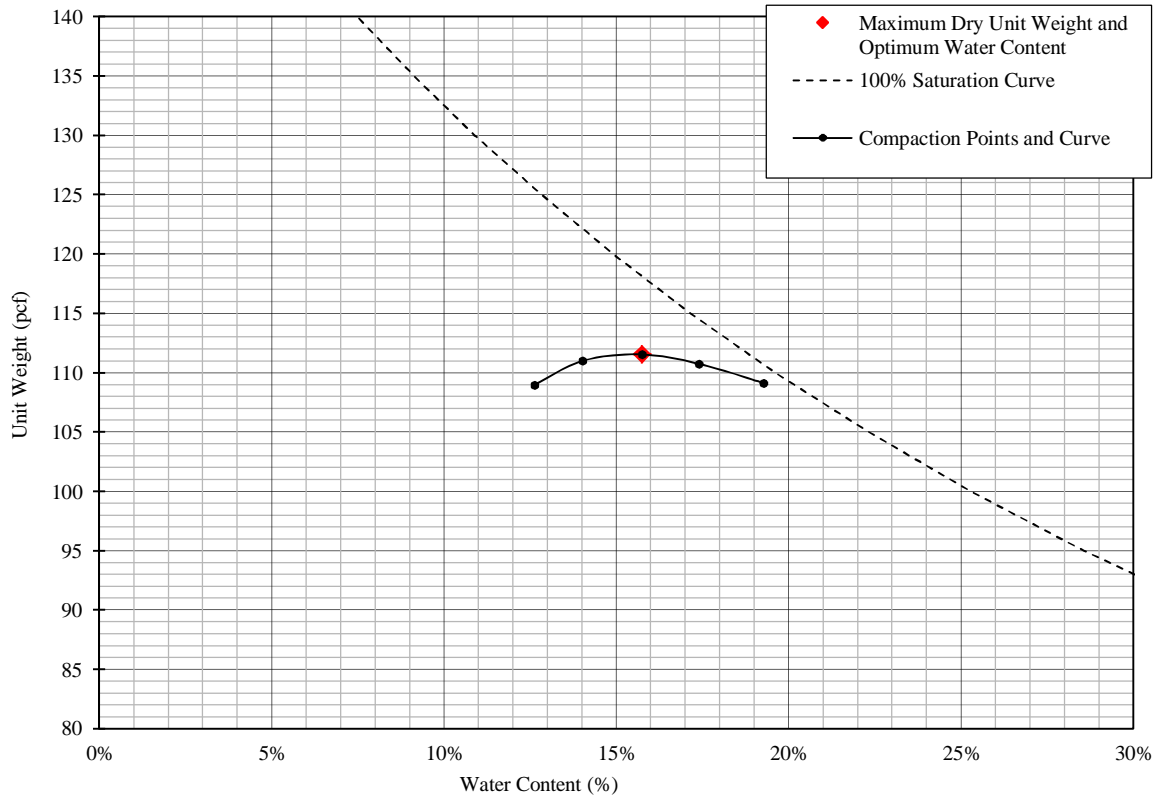
Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH MGC  
 DATE 2/22/2013  
 REVIEW MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**      DEPTH (ft): **0-2**  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>76%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>111.5</b>
Optimum Water Content (%)	<b>15.7</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>119.9</b>
Corrected Optimum Water Content (%)	<b>12.2</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, dark yellowish brown, dry

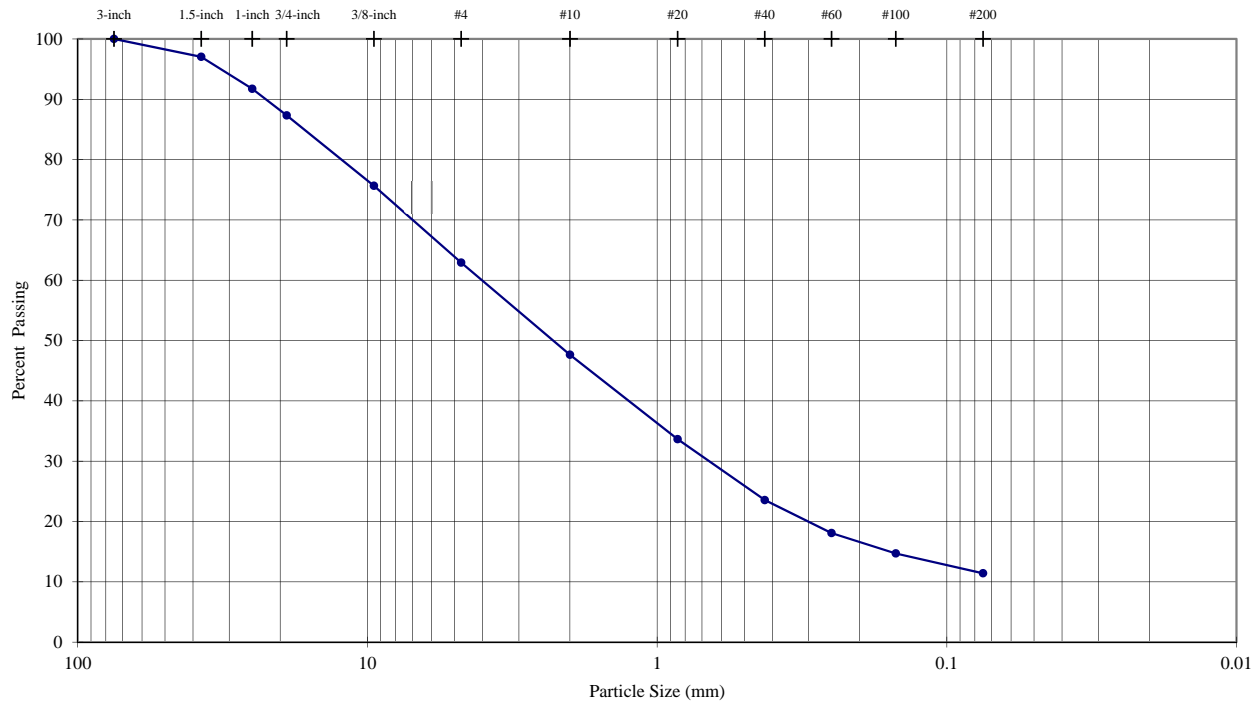
USCS      SC

TECH	MGC/AMS
DATE	2-26-2013
REVIEW	MB

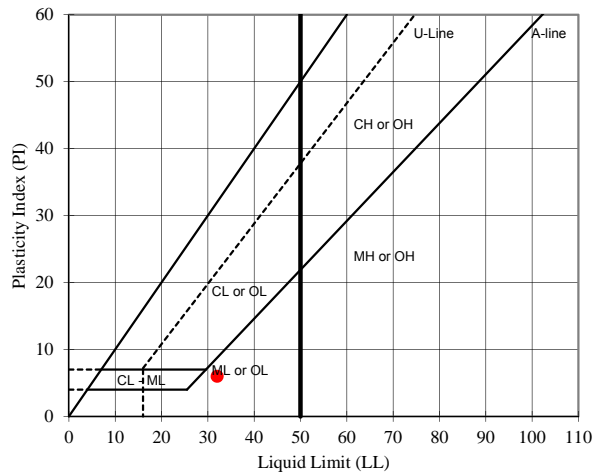
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Bag/Pail**

DEPTH (ft): **3-7**



Sieve	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>12.68</b>
1.5-inch	37.5	<b>97.0</b>		
1-inch	25.0	<b>91.7</b>		
3/4-inch	19.0	<b>87.3</b>	Fine Gravel	<b>24.40</b>
3/8-inch	9.5	<b>75.6</b>		
#4	4.8	<b>62.9</b>	Coarse Sand	<b>15.27</b>
#10	2.00	<b>47.6</b>		
#20	0.85	<b>33.7</b>	Medium Sand	<b>24.09</b>
#40	0.43	<b>23.6</b>		
#60	0.25	<b>18.1</b>	Fine Sand	<b>12.14</b>
#100	0.15	<b>14.7</b>		
#200	0.075	<b>11.4</b>	Silt or Clay Fines	<b>11.42</b>



USCS Description (ASTM D 2487):

Well-graded sand with silt and gravel, dark yellowish brown, dry

LL	PL	PI
<b>32</b>	<b>26</b>	<b>6</b>

As-Received Moisture Content (%)

--

USCS Group Symbol

SW-SM

Notes: 0g of particles up to 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

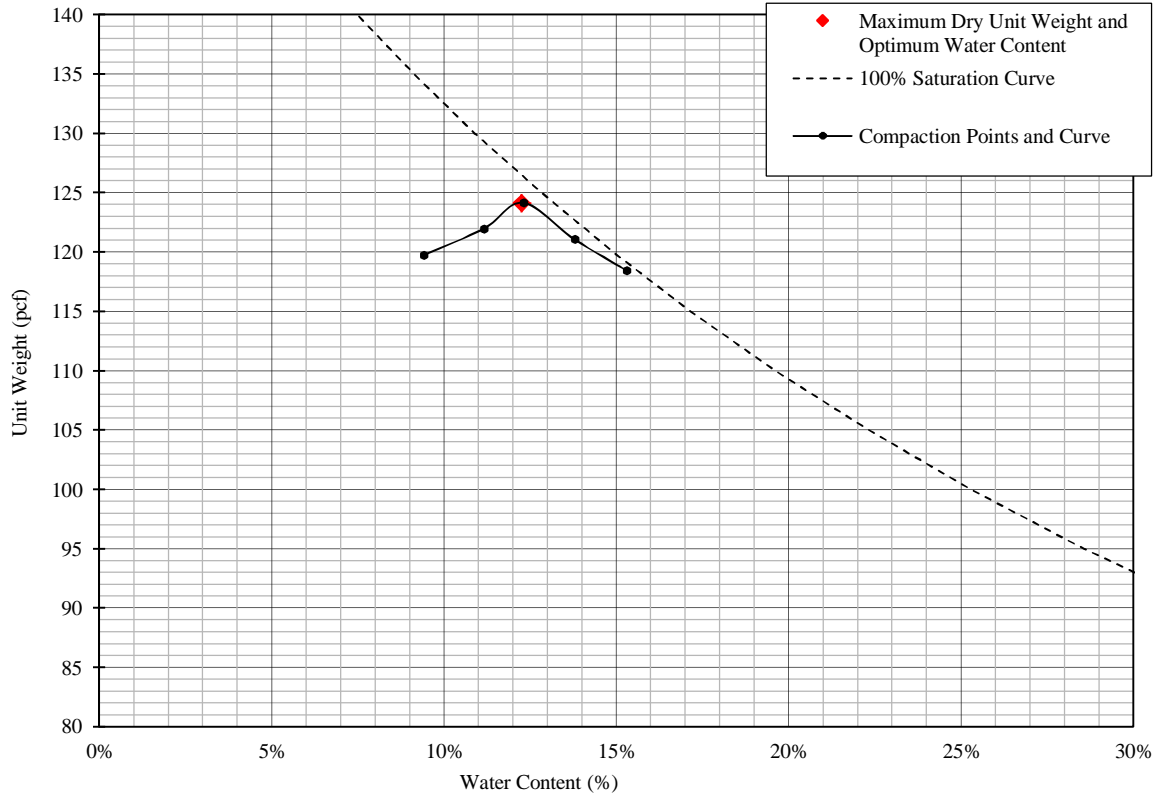
TECH	EH
DATE	2/20/2013
REVIEW	MB

## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method B

Manual Rammer      Dry Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-27**  
 TYPE: **Bag/Pail**

DEPTH (ft): **3-7**



% Test Fraction Passing 3/8-inch Sieve	<b>76%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (estimated)	<b>2.70</b>

Maximum Dry Unit Weight (pcf)	<b>124.1</b>
Optimum Water Content (%)	<b>12.3</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>130.5</b>
Corrected Optimum Water Content (%)	<b>9.6</b>

USCS Description (ASTM D 2487): Well-graded sand with silt and gravel, dark yellowish brown, dry

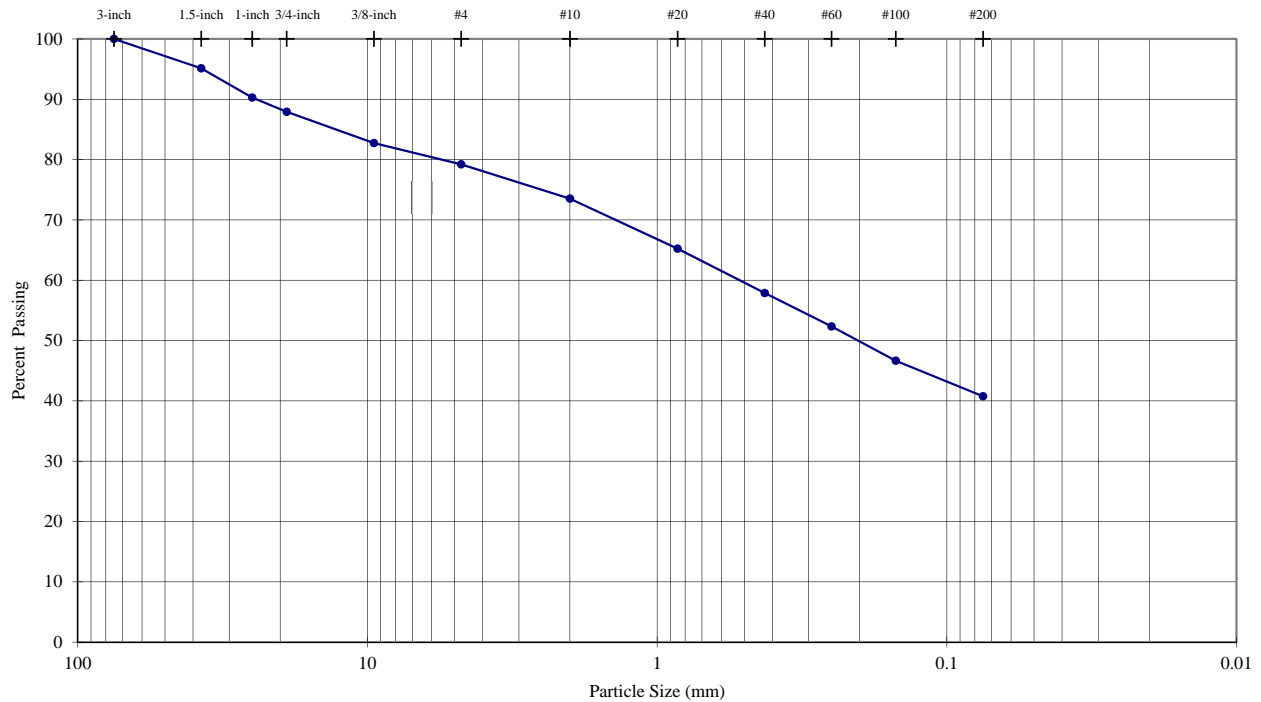
USCS SW-SM

TECH	EH
DATE	3-8-2013
REVIEW	MB

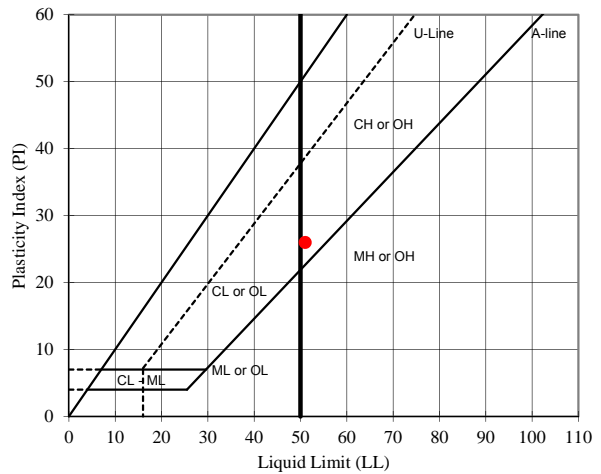
**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-29**  
 TYPE: **Pail**

DEPTH (ft): **0-2**



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	<b>100.0</b>	Coarse Gravel	<b>12.08</b>
1.5-inch	37.5	<b>95.1</b>		
1-inch	25.0	<b>90.3</b>		
3/4-inch	19.0	<b>87.9</b>	Fine Gravel	<b>8.71</b>
3/8-inch	9.5	<b>82.7</b>		
#4	4.8	<b>79.2</b>	Coarse Sand	<b>5.68</b>
#10	2.00	<b>73.5</b>		
#20	0.85	<b>65.2</b>		
#40	0.43	<b>57.9</b>	Medium Sand	<b>15.66</b>
#60	0.25	<b>52.3</b>		
#100	0.15	<b>46.7</b>	Fine Sand	<b>17.10</b>
#200	0.075	<b>40.8</b>		
			Silt or Clay Fines	<b>40.76</b>



USCS Description (ASTM D 2487):  
**Clayey sand with gravel, strong brown, dry**

LL	PL	PI
<b>51</b>	<b>25</b>	<b>26</b>

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
**SC**

Notes: 0g of particles up to plus 75.0mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

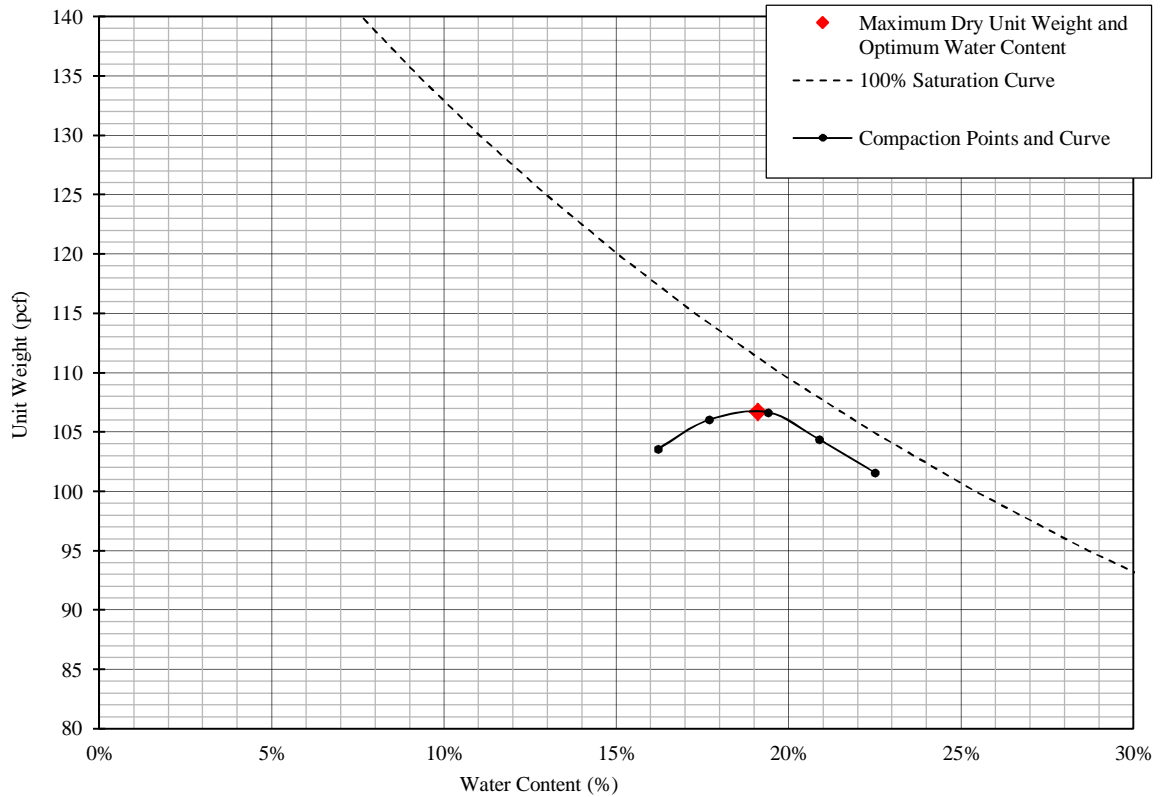
TECH	AM
DATE	2/21/2013
REVIEW	MB



## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer      Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **TP-29**      DEPTH (ft): **0-2**  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	<b>80%</b>
As-Received Moisture Content	<b>NA</b>
Specific Gravity (ASTM C127)	<b>2.71</b>

Maximum Dry Unit Weight (pcf)	<b>106.7</b>
Optimum Water Content (%)	<b>19.1</b>

Corrected Maximum Dry Unit Weight (pcf)	<b>114.4</b>
Corrected Optimum Water Content (%)	<b>15.5</b>

USCS Description (ASTM D 2487): Clayey sand with gravel, strong brown, dry

USCS      SC

TECH	AM
DATE	2-25-2013
REVIEW	MB

**APPENDIX A.3.2  
TRIAXIAL TEST REPORTS**

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: Comp 1-4  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.517 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.355 lb  
 Moisture Content = 15.8%  
 Wet Unit Weight = 126.6 pcf  
 Dry Unit Weight = 109.3 pcf  
 Void Ratio = 0.51  
 Percent Saturation = 83%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.506 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.332 lb  
 Moisture Content = 16.0%  
 Wet Unit Weight = 126.4 pcf  
 Dry Unit Weight = 108.9 pcf  
 Void Ratio = 0.51  
 Percent Saturation = 83%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.506 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 7.377 lb  
 Moisture Content = 15.3%  
 Wet Unit Weight = 126.4 pcf  
 Dry Unit Weight = 109.6 pcf  
 Void Ratio = 0.50  
 Percent Saturation = 81%

**After Consolidation**  
 Length = 9.173 in  
 Diameter = 3.814 in  
 Area = 11.424 in<sup>2</sup> (Method B)  
 Volume = 104.789 in<sup>3</sup>  
 Moisture Content = 13.5%  
 Wet Unit Weight = 137.7 pcf  
 Dry Unit Weight = 121.3 pcf  
 Void Ratio = 0.36  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.094 in  
 Diameter = 3.821 in  
 Area = 11.465 in<sup>2</sup> (Method B)  
 Volume = 104.264 in<sup>3</sup>  
 Moisture Content = 13.4%  
 Wet Unit Weight = 137.8 pcf  
 Dry Unit Weight = 121.5 pcf  
 Void Ratio = 0.35  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.057 in  
 Diameter = 3.799 in  
 Area = 11.333 in<sup>2</sup> (Method B)  
 Volume = 102.647 in<sup>3</sup>  
 Moisture Content = 12.3%  
 Wet Unit Weight = 139.5 pcf  
 Dry Unit Weight = 124.2 pcf  
 Void Ratio = 0.32  
 Percent Saturation = 100%

B Parameter = 0.97  
 Shear Rate = 0.071% /min.  
 t<sub>50</sub> = 5.6 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 1.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.027% /min.  
 t<sub>50</sub> = 14.5 min.  
 Strain at Failure = 5.0%

Cell Pressure = 75 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 25 psi

Cell Pressure = 100 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 50 psi

Cell Pressure = 150 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey sand with gravel, yellowish brown, moist  
 Atterberg limits: LL = -- PL = -- PI = -- (-- indicates test was not performed)  
 Percent finer: 3/4 in. = -- No. 4 = -- No. 200 = -- (-- indicates test was not performed)  
 Specimen type: 

	Intact	X
	Cuttings	
	Wet	
	(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	
	Corrected	

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

X
---

 Entire specimen  
 Saturation method: 

X
---

 Dry  
 Failure criterion: 

--

 (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub>

5
---

 % strain  
 Membrane effect: 

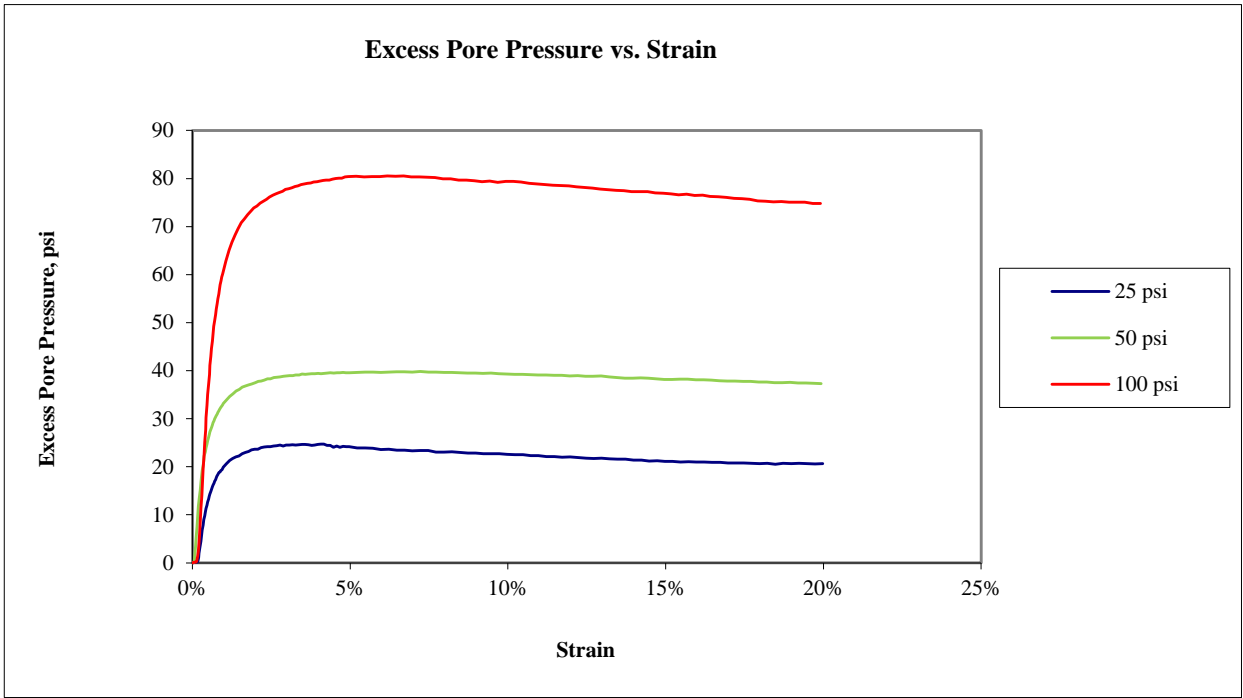
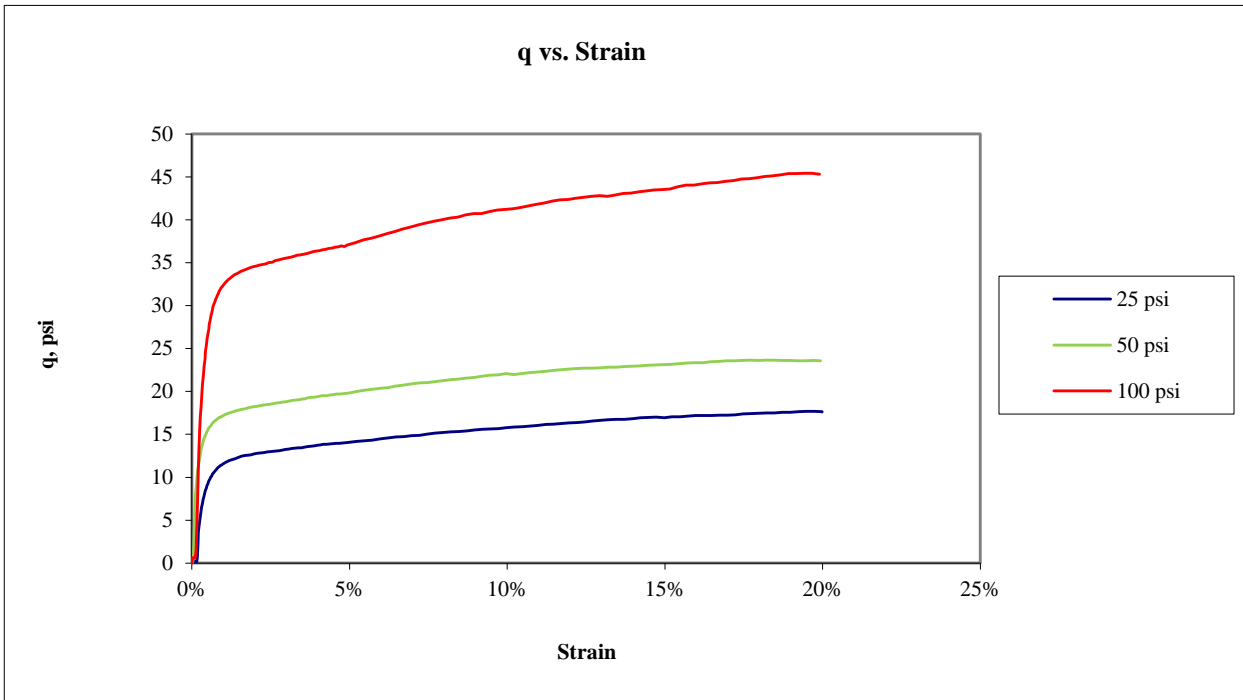
X
---

 Corrected 

--

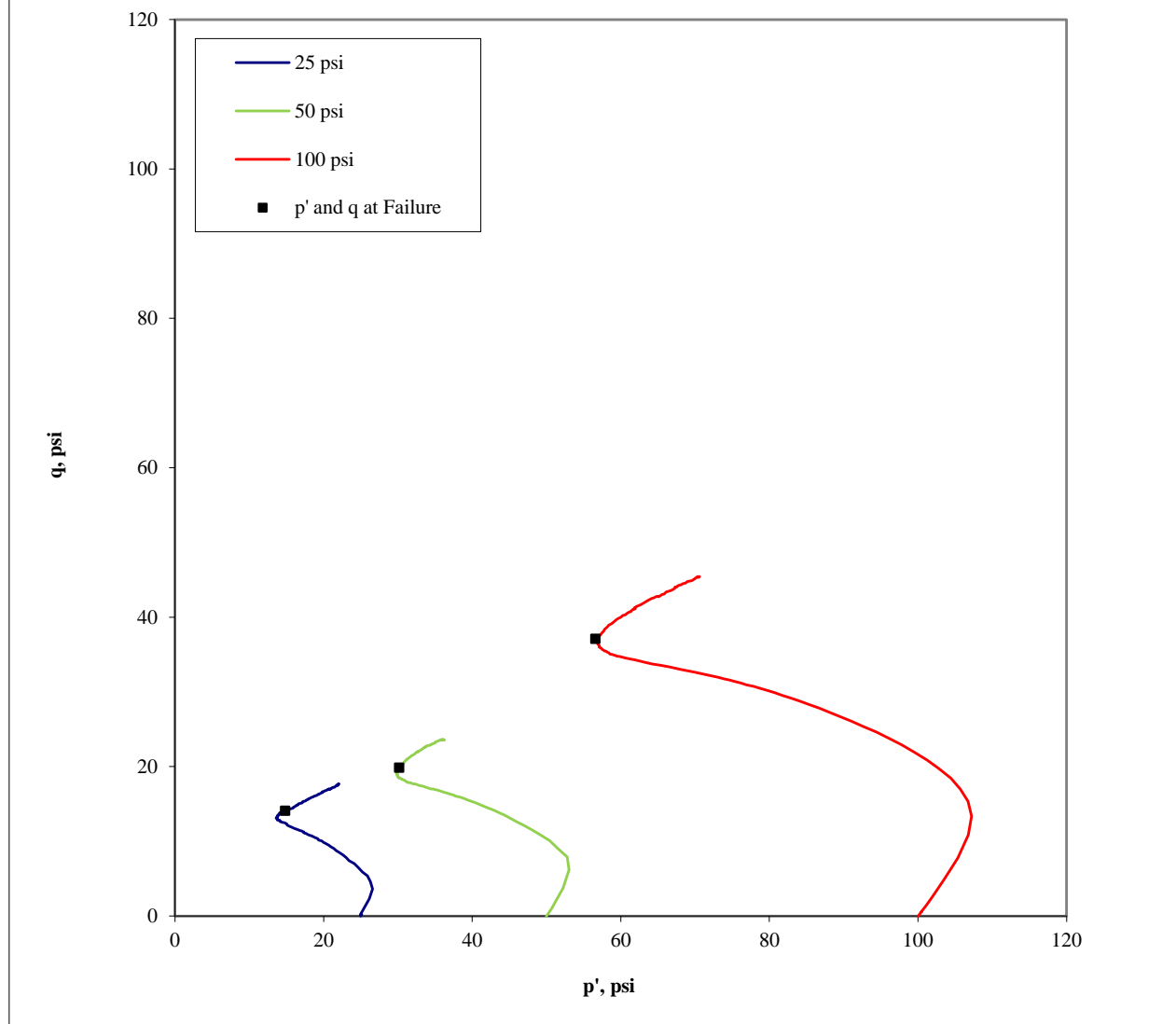
 Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> q AND EXCESS PORE PRESSURE PLOTS			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 2

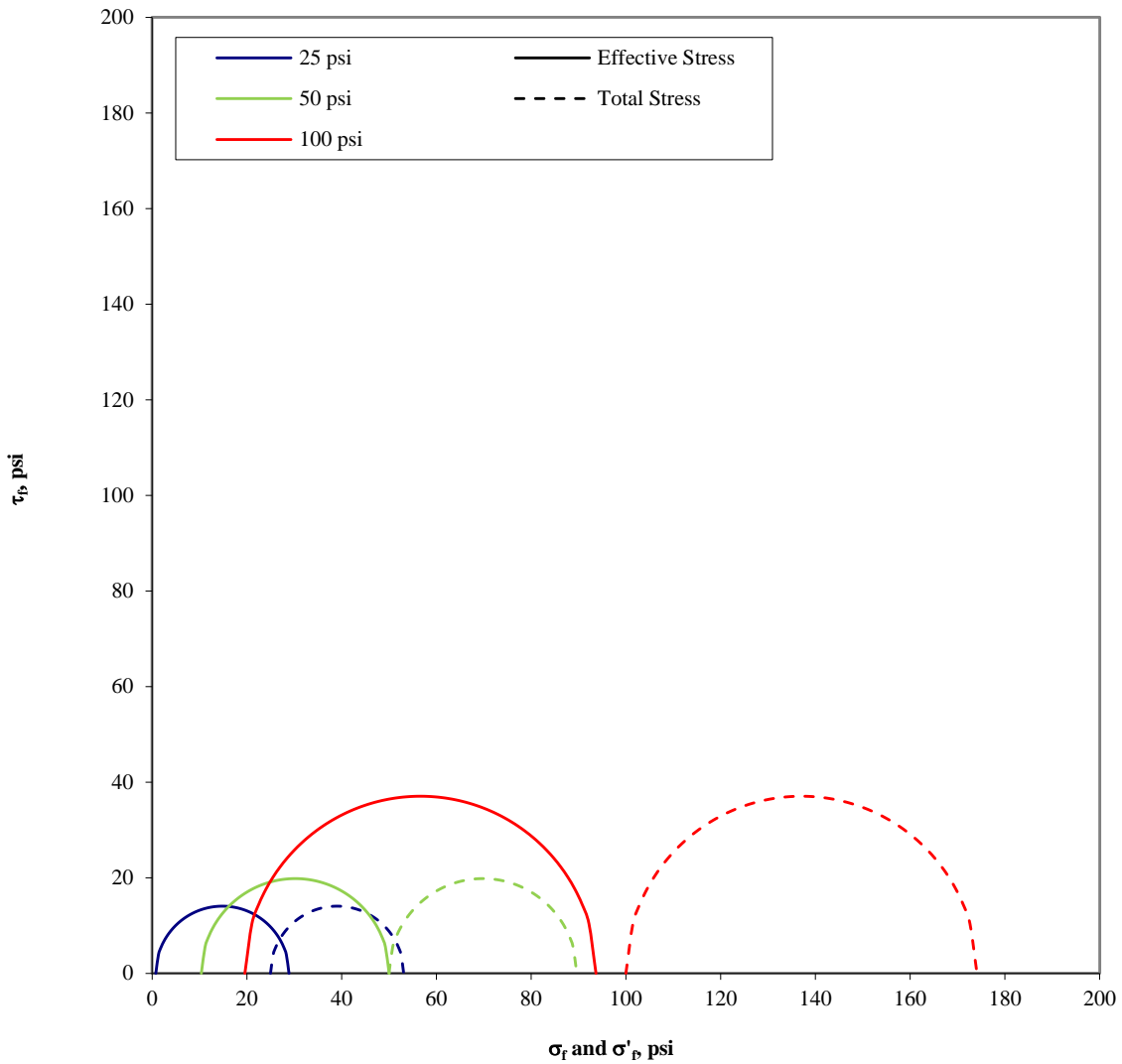
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	39.1	14.8	14.1
50	69.8	30.2	19.8
100	137.0	56.6	37.0

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>Composite 1-4</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>5/3/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	28.9	0.8	53.1	25.0
50	50.0	10.4	89.6	50.0
100	93.7	19.6	174.1	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Composite 1-4	Technician: RJM	Reviewed: CCS	Date: 5/3/2013	Job Number: 103-92557.006	Figure: 4



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Composite 1-4	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/3/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: BH-16  
 Depth: 0.0 - 8.5 ft  
 Point No.: 3

**Initial**  
 Length = 5.751 in  
 Diameter = 2.882 in  
 Wet Mass = 2.743 lb  
 Area = 6.523 in<sup>2</sup>  
 Volume = 37.516 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.371 lb  
 Moisture Content = 15.7%  
 Wet Unit Weight = 126.3 pcf  
 Dry Unit Weight = 109.2 pcf  
 Void Ratio = 0.56  
 Percent Saturation = 76%

**Initial**  
 Length = 5.785 in  
 Diameter = 2.812 in  
 Wet Mass = 2.734 lb  
 Area = 6.210 in<sup>2</sup>  
 Volume = 35.927 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.367 lb  
 Moisture Content = 15.5%  
 Wet Unit Weight = 131.5 pcf  
 Dry Unit Weight = 113.8 pcf  
 Void Ratio = 0.50  
 Percent Saturation = 85%

**Initial**  
 Length = 5.764 in  
 Diameter = 2.877 in  
 Wet Mass = 2.724 lb  
 Area = 6.501 in<sup>2</sup>  
 Volume = 37.471 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.348 lb  
 Moisture Content = 16.0%  
 Wet Unit Weight = 125.6 pcf  
 Dry Unit Weight = 108.3 pcf  
 Void Ratio = 0.58  
 Percent Saturation = 76%

**After Consolidation**  
 Length = 5.697 in  
 Diameter = 2.772 in  
 Area = 6.035 in<sup>2</sup> (Method B)  
 Volume = 34.380 in<sup>3</sup>  
 Moisture Content = 15.8%  
 Wet Unit Weight = 138.0 pcf  
 Dry Unit Weight = 119.2 pcf  
 Void Ratio = 0.43  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.692 in  
 Diameter = 2.760 in  
 Area = 5.984 in<sup>2</sup> (Method B)  
 Volume = 34.058 in<sup>3</sup>  
 Moisture Content = 15.4%  
 Wet Unit Weight = 138.6 pcf  
 Dry Unit Weight = 120.1 pcf  
 Void Ratio = 0.42  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.579 in  
 Diameter = 2.731 in  
 Area = 5.859 in<sup>2</sup> (Method B)  
 Volume = 32.690 in<sup>3</sup>  
 Moisture Content = 13.7%  
 Wet Unit Weight = 141.2 pcf  
 Dry Unit Weight = 124.1 pcf  
 Void Ratio = 0.38  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 2.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.8 min.  
 Strain at Failure = 5.0%

Cell Pressure = 75 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 25 psi

Cell Pressure = 110 psi  
 Back Pressure = 60 psi  
 Confining Pressure = 50 psi

Cell Pressure = 150 psi  
 Back Pressure = 50 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, reddish brown, moist  
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)  
 Percent finer: 3/4 in. = 91% No. 4 = 57% No. 200 = 31% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

Intact	X
Cuttings	
Wet	
(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	
Corrected	

 Reconstituted Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

Entire specimen
Dry

  
 Saturation method: 

X
---

  
 Failure criterion: 

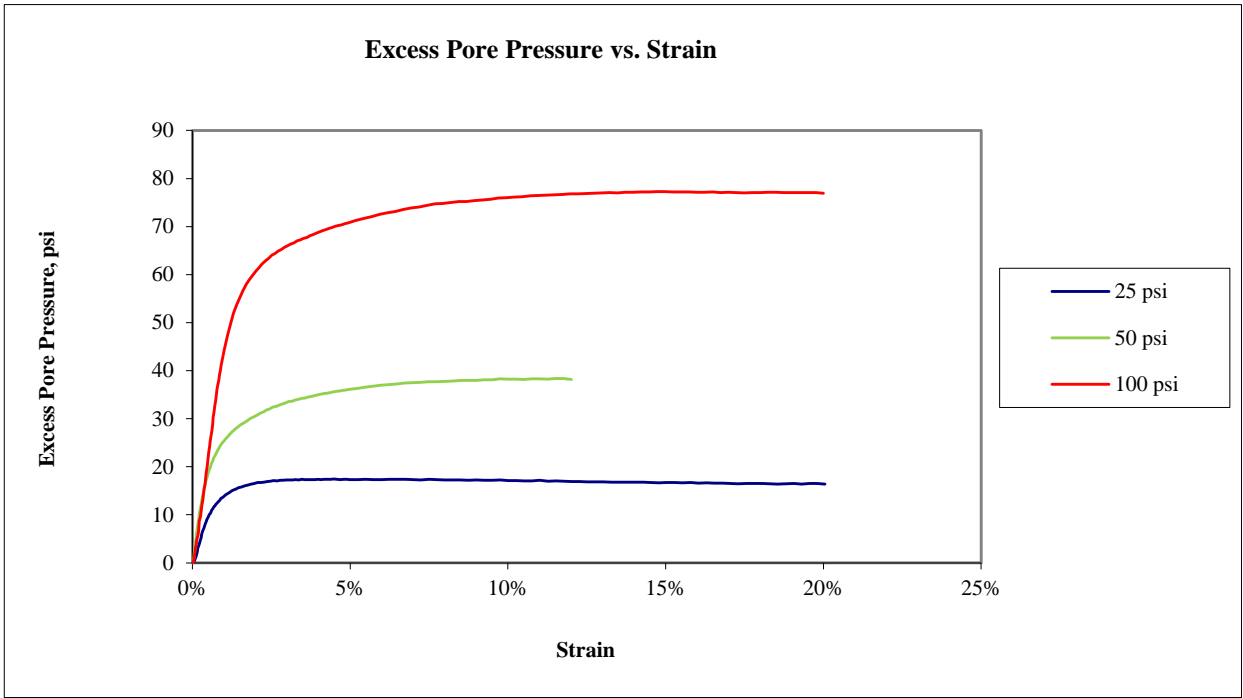
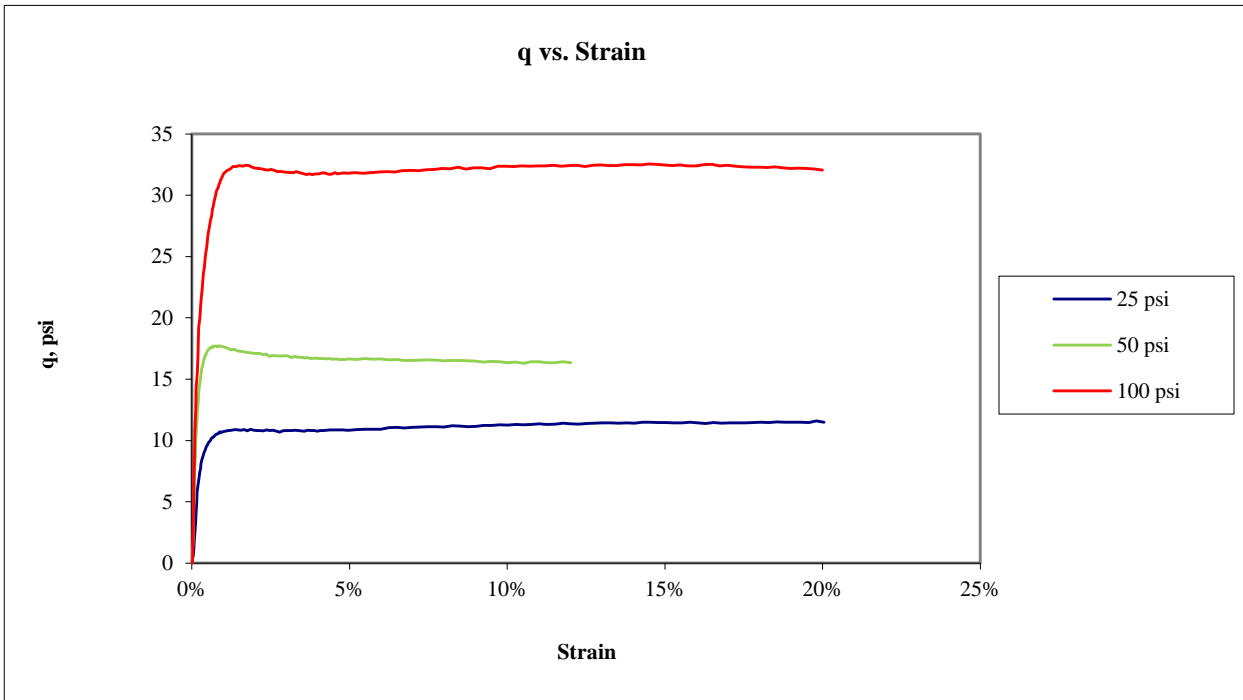
(σ <sub>1</sub> -σ <sub>3</sub> ) <sub>max</sub>	5
--	---

 % strain  
 Membrane effect: 

X	Corrected
---	-----------

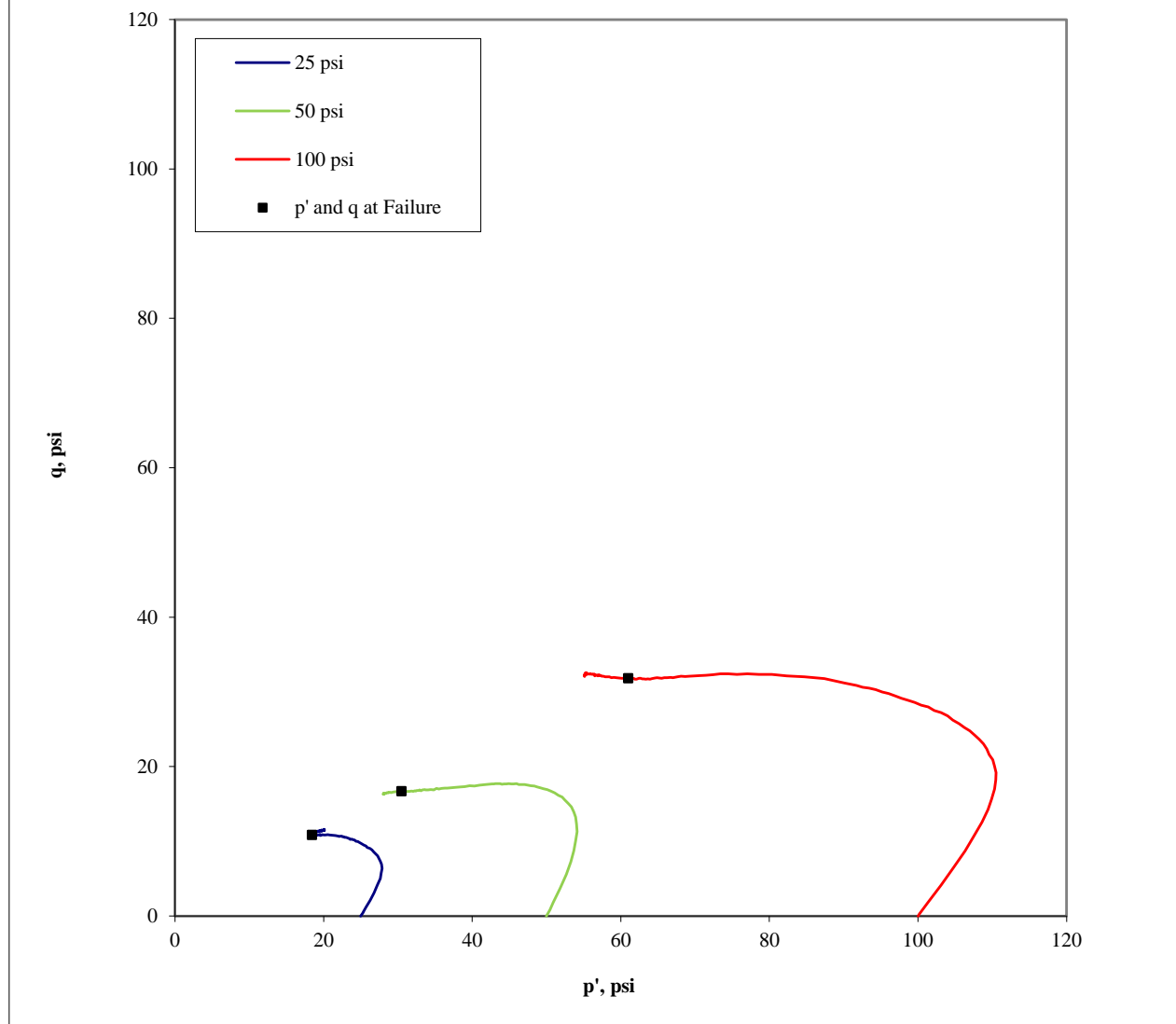
 Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

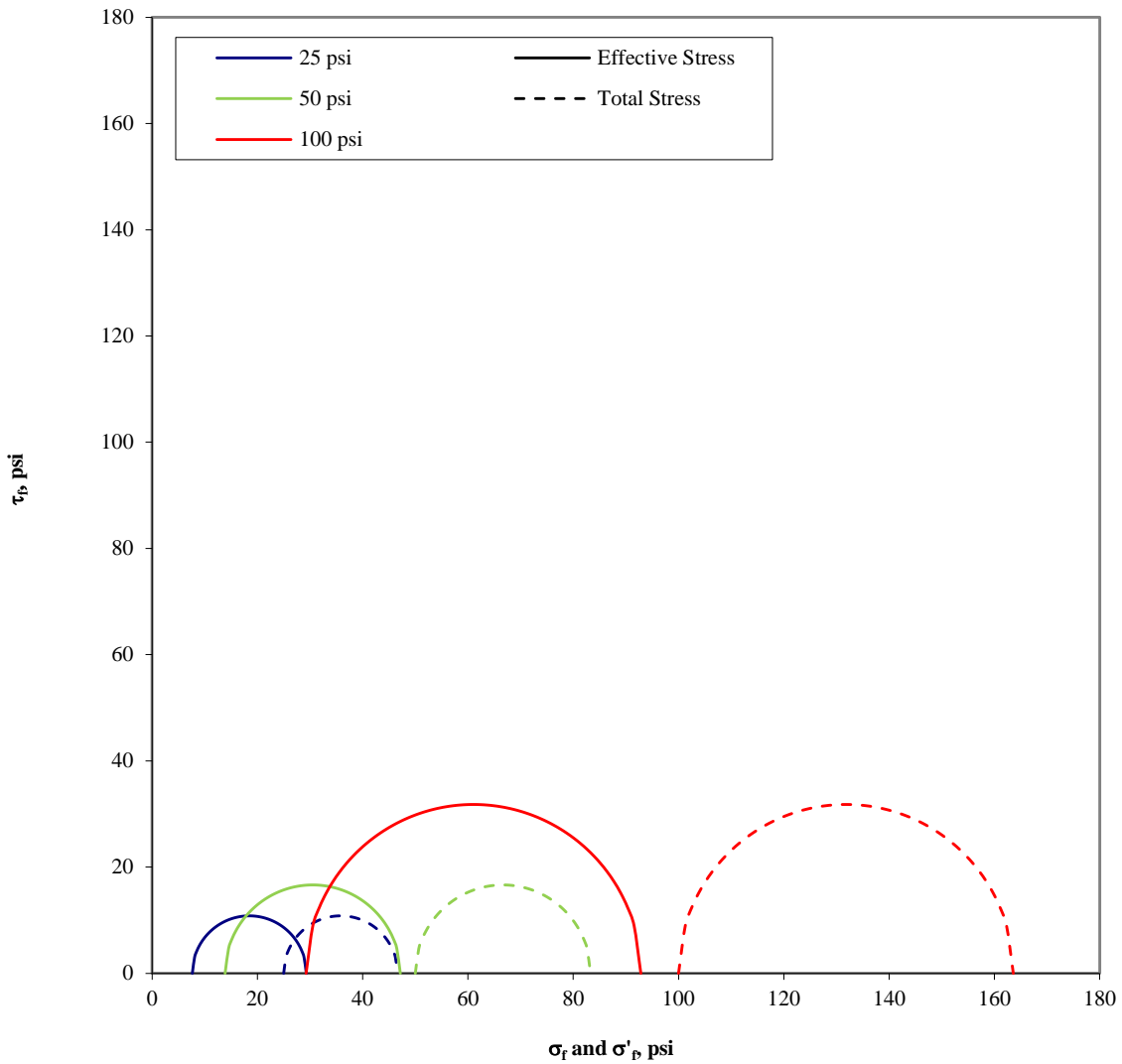
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	35.8	18.5	10.8
50	66.6	30.5	16.6
100	131.8	61.1	31.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>BH-16 @ 0.0 - 8.5 ft</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>3/26/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	29.3	7.6	46.7	25.0
50	47.2	13.9	83.3	50.0
100	92.8	29.3	163.6	100.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 0.0 - 8.5 ft	Technician: RJM	Reviewed: CCS	Date: 3/26/2013	Job Number: 103-92557.006	Figure: 4



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-16 @ 0.0 - 8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 3/26/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7



Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: BH-25  
 Depth: 0.0 - 12.5 ft  
 Point No.: 3

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.589 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.443 lb  
 Moisture Content = 15.4%  
 Wet Unit Weight = 127.6 pcf  
 Dry Unit Weight = 110.6 pcf  
 Void Ratio = 0.54  
 Percent Saturation = 78%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.601 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.421 lb  
 Moisture Content = 15.9%  
 Wet Unit Weight = 127.8 pcf  
 Dry Unit Weight = 110.3 pcf  
 Void Ratio = 0.55  
 Percent Saturation = 79%

**Initial**  
 Length = 9.250 in  
 Diameter = 4.001 in  
 Wet Mass = 8.587 lb  
 Area = 12.573 in<sup>2</sup>  
 Volume = 116.297 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 7.416 lb  
 Moisture Content = 15.8%  
 Wet Unit Weight = 127.6 pcf  
 Dry Unit Weight = 110.2 pcf  
 Void Ratio = 0.55  
 Percent Saturation = 79%

**After Consolidation**  
 Length = 9.195 in  
 Diameter = 3.788 in  
 Area = 11.267 in<sup>2</sup> (Method B)  
 Volume = 103.604 in<sup>3</sup>  
 Moisture Content = 13.7%  
 Wet Unit Weight = 141.2 pcf  
 Dry Unit Weight = 124.1 pcf  
 Void Ratio = 0.38  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.131 in  
 Diameter = 3.765 in  
 Area = 11.133 in<sup>2</sup> (Method B)  
 Volume = 101.651 in<sup>3</sup>  
 Moisture Content = 12.9%  
 Wet Unit Weight = 142.4 pcf  
 Dry Unit Weight = 126.2 pcf  
 Void Ratio = 0.35  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 9.054 in  
 Diameter = 3.749 in  
 Area = 11.037 in<sup>2</sup> (Method B)  
 Volume = 99.932 in<sup>3</sup>  
 Moisture Content = 12.1%  
 Wet Unit Weight = 143.7 pcf  
 Dry Unit Weight = 128.2 pcf  
 Void Ratio = 0.33  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.010% /min.  
 t<sub>50</sub> = 38.0 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.033% /min.  
 t<sub>50</sub> = 12.0 min.  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.047% /min.  
 t<sub>50</sub> = 8.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 65 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 25 psi

Cell Pressure = 90 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 50 psi

Cell Pressure = 145 psi  
 Back Pressure = 45 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Clayey gravel with sand, yellowish brown, moist  
 Atterberg limits: LL = 41 PL = 17 PI = 24 (ASTM D4318)  
 Percent finer: 3/4 in. = 88% No. 4 = 51% No. 200 = 17% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

	Intact	<input checked="" type="checkbox"/>	Reconstituted
	Cuttings	<input type="checkbox"/>	Entire specimen
	Wet	<input type="checkbox"/>	Dry
	(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	<input type="checkbox"/>	(σ <sub>1</sub> -σ <sub>3</sub> ) <sub>max</sub> <input type="text" value="5"/> % strain
	Corrected	<input type="checkbox"/>	Not Corrected

 Remold targets: 110.0 pcf (dry) at 16.0% moisture  
 Moisture from: 

<input checked="" type="checkbox"/>	
-------------------------------------	--

  
 Saturation method: 

<input checked="" type="checkbox"/>	
-------------------------------------	--

  
 Failure criterion: 

<input type="checkbox"/>	
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 Membrane effect: 

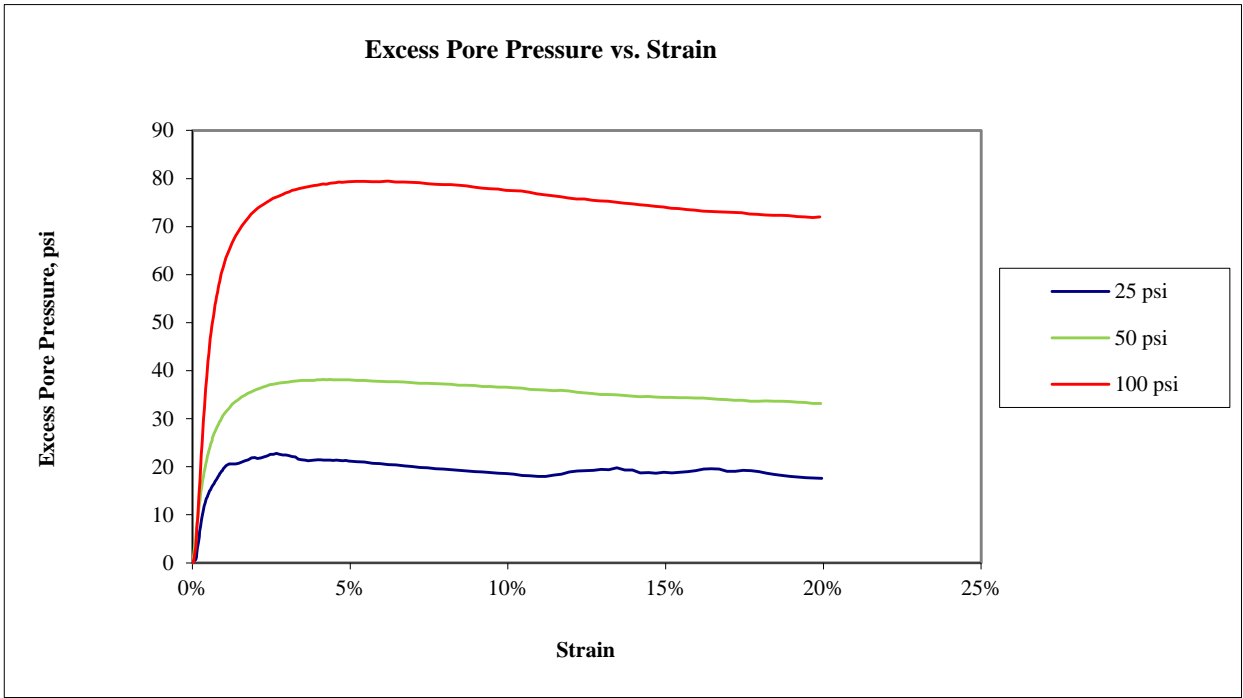
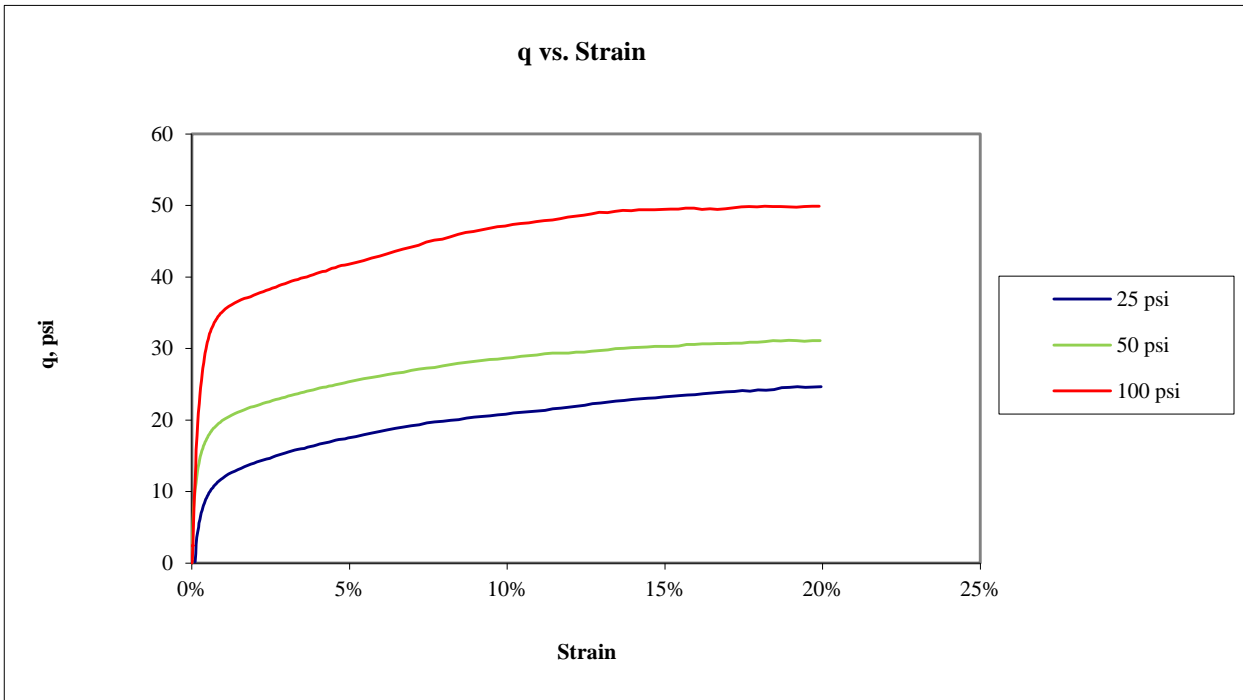
<input checked="" type="checkbox"/>	
-------------------------------------	--

**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
 ASTM D4767  
 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT  
 SAMPLE AND TEST DATA

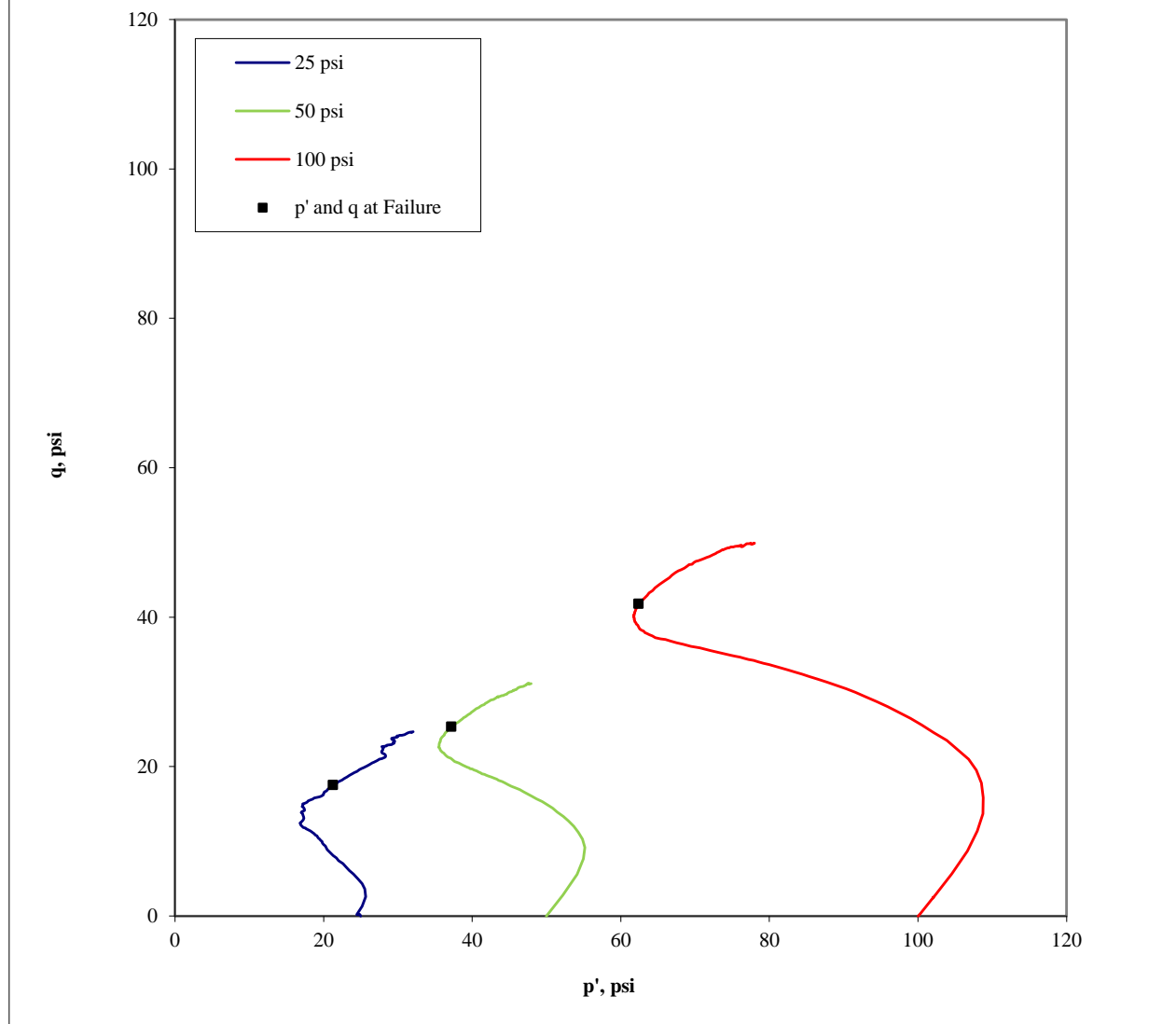
**Job Short Title:**  
 Copper Flat Tailings Design Study

<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1
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<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

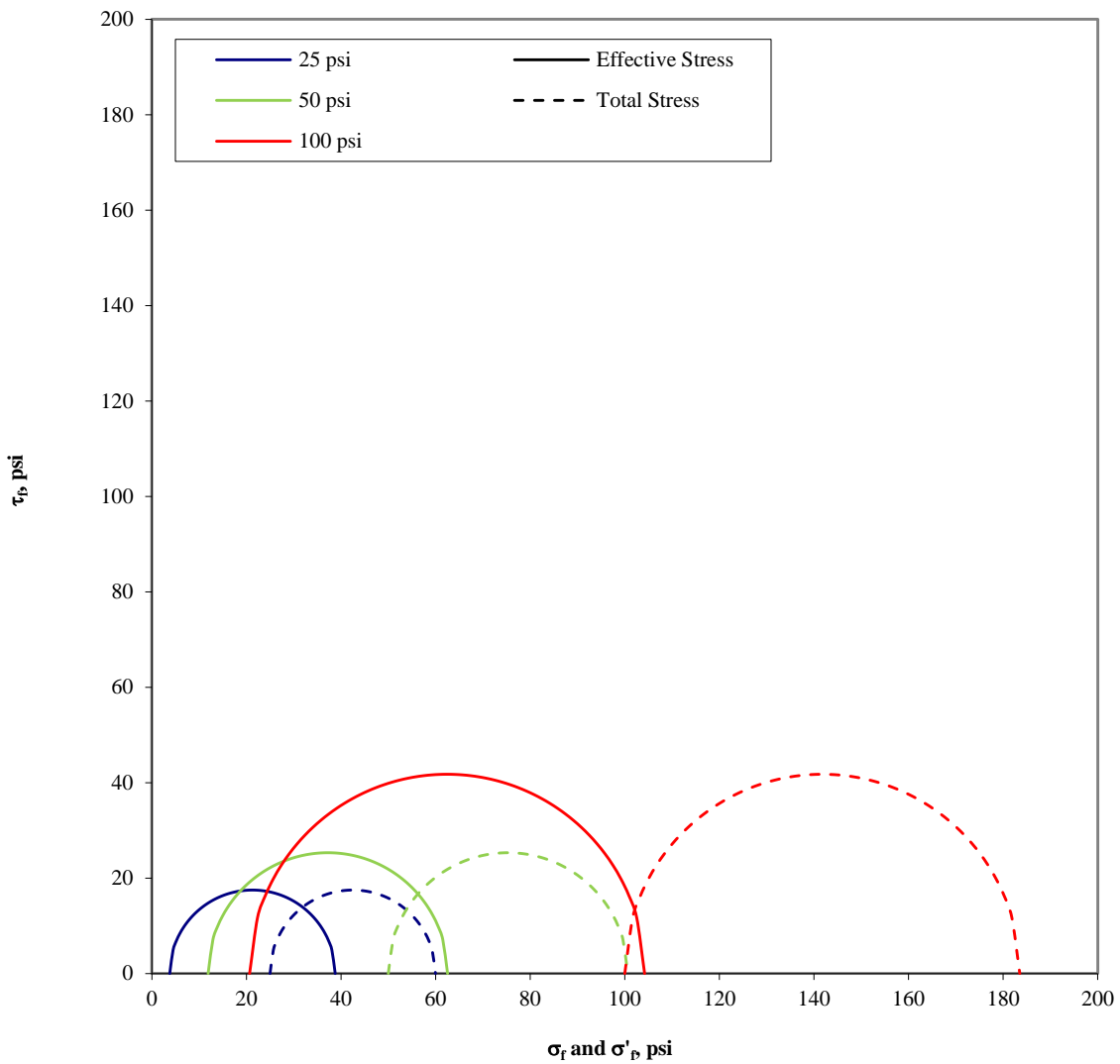
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	42.5	21.3	17.5
50	75.3	37.2	25.3
100	141.8	62.4	41.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>BH-25 @ 0.0 - 12.5 ft.</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>5/1/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	38.8	3.8	60.0	25.0
50	62.5	11.9	100.6	50.0
100	104.2	20.7	183.5	100.0

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-25 @ 0.0 - 12.5 ft.	Technician: RJM	Reviewed: CCS	Date: 5/1/2013	Job Number: 103-92557.006	Figure: 4



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 0.0 - 12.5 ft.	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 5/1/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: TP-10  
 Depth: 3.0 - 12.0 ft  
 Point No.: 3

**Initial**  
 Length = 5.786 in  
 Diameter = 2.886 in  
 Wet Mass = 2.610 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.850 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.256 lb  
 Moisture Content = 15.7%  
 Wet Unit Weight = 119.2 pcf  
 Dry Unit Weight = 103.0 pcf  
 Void Ratio = 0.66  
 Percent Saturation = 65%

**Initial**  
 Length = 5.784 in  
 Diameter = 2.886 in  
 Wet Mass = 2.611 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.836 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.247 lb  
 Moisture Content = 16.2%  
 Wet Unit Weight = 119.2 pcf  
 Dry Unit Weight = 102.6 pcf  
 Void Ratio = 0.66  
 Percent Saturation = 67%

**Initial**  
 Length = 5.796 in  
 Diameter = 2.886 in  
 Wet Mass = 2.607 lb  
 Area = 6.542 in<sup>2</sup>  
 Volume = 37.915 in<sup>3</sup>  
 Specific Gravity = 2.74 (Provided)  
 Dry Mass of Solids = 2.240 lb  
 Moisture Content = 16.4%  
 Wet Unit Weight = 118.8 pcf  
 Dry Unit Weight = 102.1 pcf  
 Void Ratio = 0.67  
 Percent Saturation = 67%

**After Consolidation**  
 Length = 5.747 in  
 Diameter = 2.842 in  
 Area = 6.345 in<sup>2</sup> (Method B)  
 Volume = 36.467 in<sup>3</sup>  
 Moisture Content = 21.8%  
 Wet Unit Weight = 130.2 pcf  
 Dry Unit Weight = 106.9 pcf  
 Void Ratio = 0.60  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.724 in  
 Diameter = 2.855 in  
 Area = 6.400 in<sup>2</sup> (Method B)  
 Volume = 36.631 in<sup>3</sup>  
 Moisture Content = 22.3%  
 Wet Unit Weight = 129.6 pcf  
 Dry Unit Weight = 106.0 pcf  
 Void Ratio = 0.61  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 5.667 in  
 Diameter = 2.884 in  
 Area = 6.532 in<sup>2</sup> (Method B)  
 Volume = 37.016 in<sup>3</sup>  
 Moisture Content = 23.1%  
 Wet Unit Weight = 128.7 pcf  
 Dry Unit Weight = 104.6 pcf  
 Void Ratio = 0.63  
 Percent Saturation = 100%

B Parameter = 0.96  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.5 min.  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.6 min.  
 Strain at Failure = 5.0%

B Parameter = 0.95  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 0.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 95 psi  
 Back Pressure = 70 psi  
 Confining Pressure = 25 psi

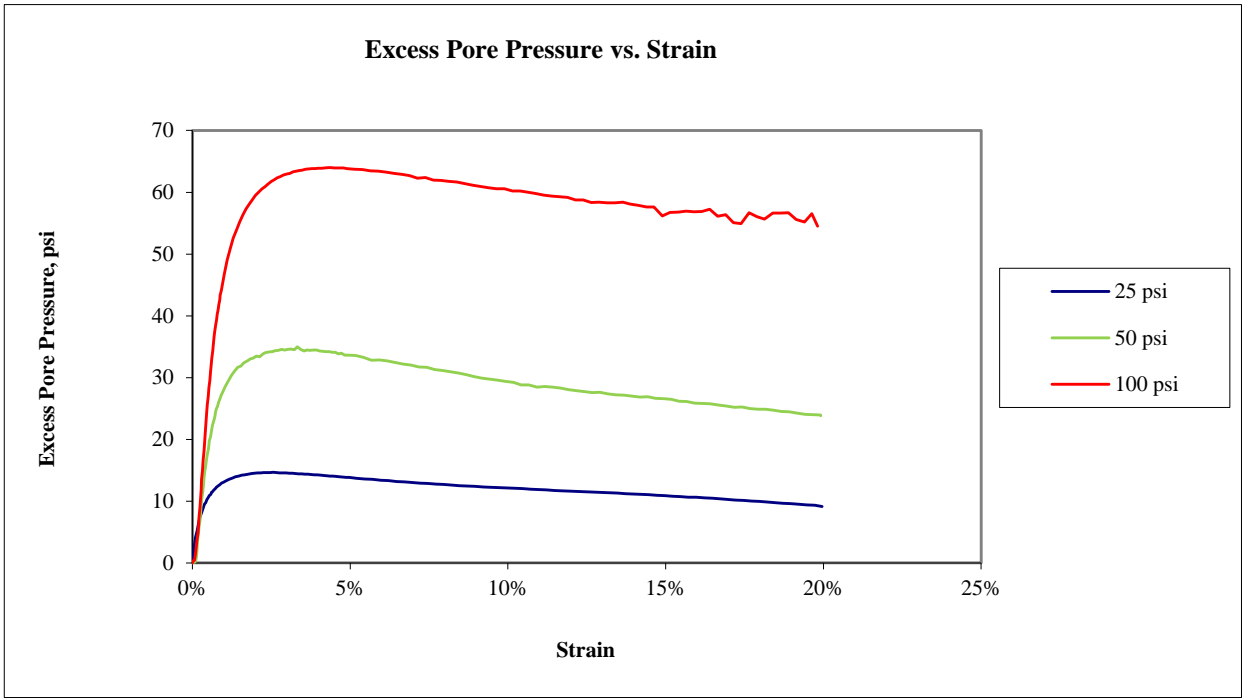
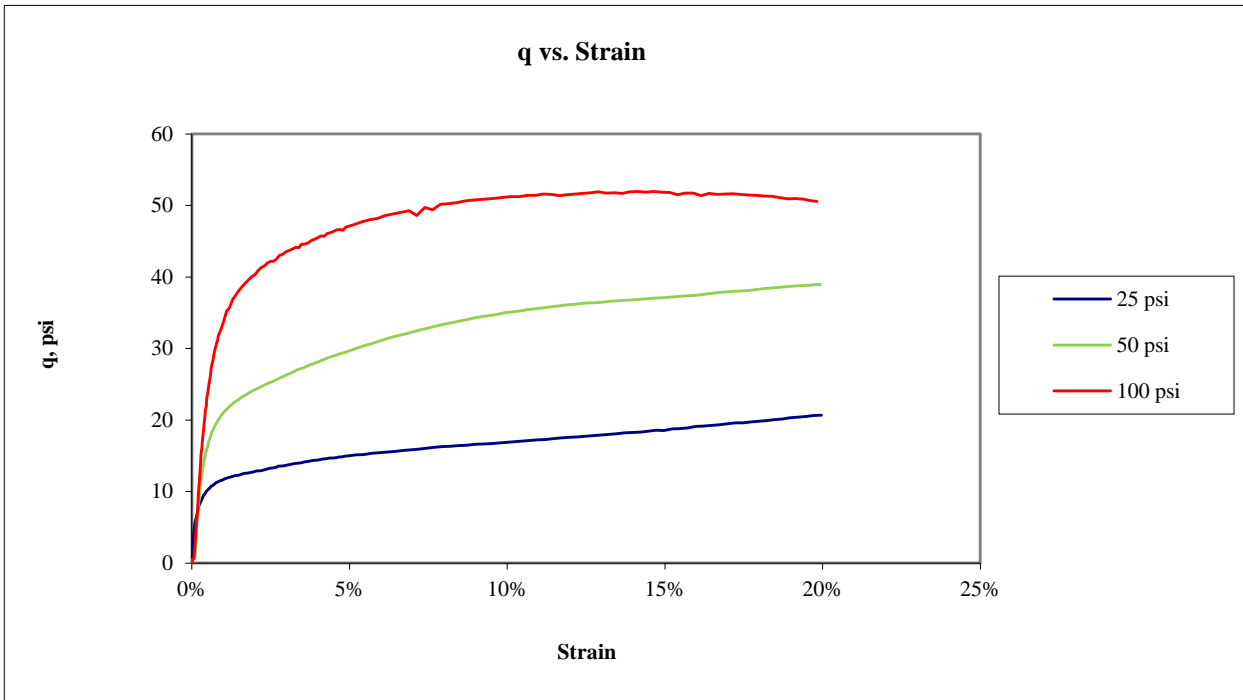
Cell Pressure = 130 psi  
 Back Pressure = 80 psi  
 Confining Pressure = 50 psi

Cell Pressure = 190 psi  
 Back Pressure = 90 psi  
 Confining Pressure = 100 psi

Notes: Sample description: Silty sand, brownish yellow, moist  
 Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 49% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type:  Intact  Reconstituted Remold targets: 104.7 pcf (dry) at 16.2% moisture  
 Moisture from:  Cuttings  Entire specimen  
 Saturation method:  Wet  Dry  
 Failure criterion:  (σ<sub>1</sub>/σ<sub>3</sub>)<sub>max</sub>  (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub>  % strain  
 Membrane effect:  Corrected  Not Corrected

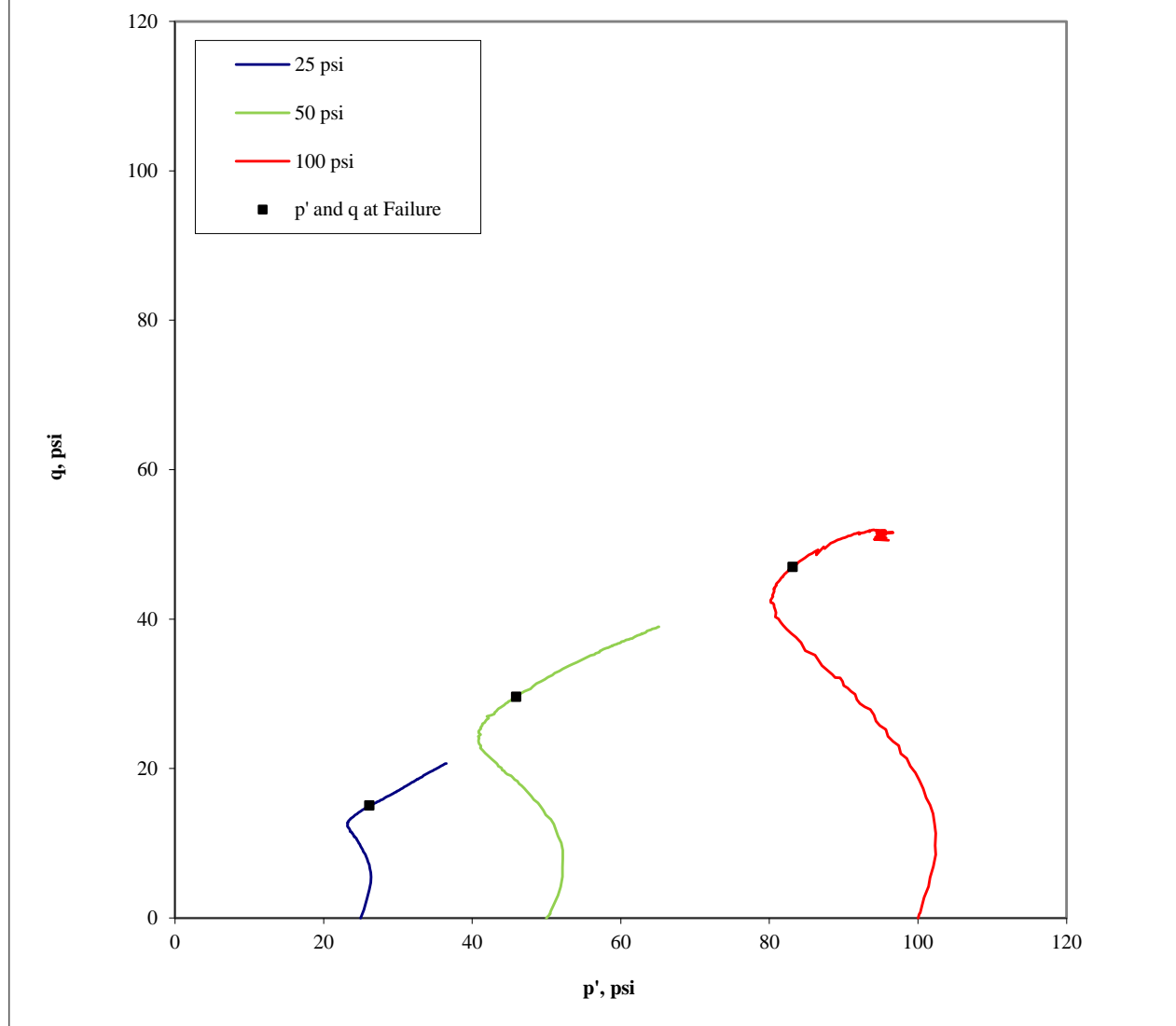
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1





<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

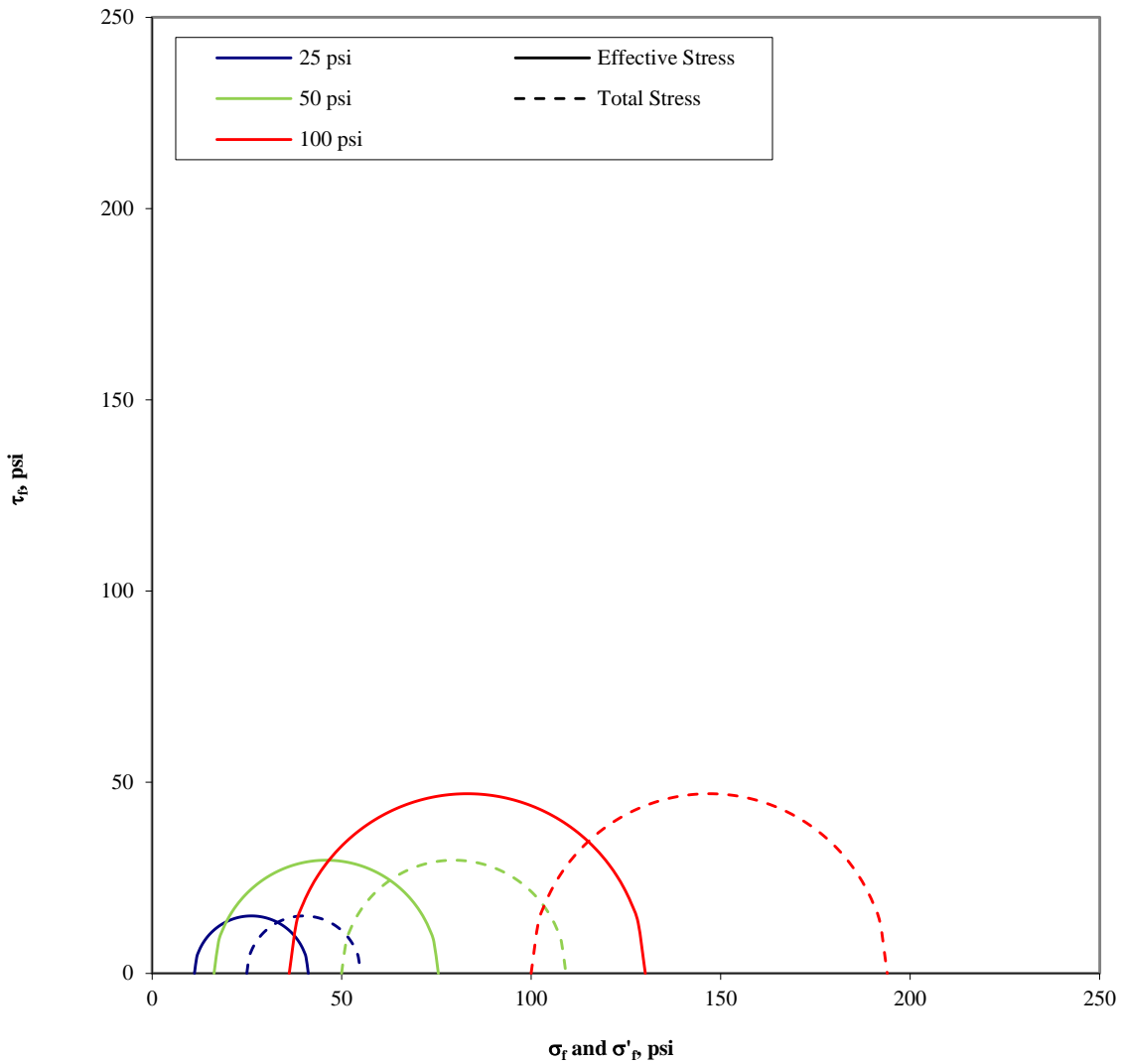
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
25	40.0	26.2	15.0
50	79.6	45.9	29.6
100	147.0	83.2	47.0

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>TP-10 @ 3 - 12 ft</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>4/8/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
25	41.2	11.2	55.0	25.0
50	75.5	16.4	109.2	50.0
100	130.1	36.2	193.9	100.0

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: <b>ASTM D4767</b> <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>MOHR'S CIRCLE DIAGRAM</b>			
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>TP-10 @ 3 - 12 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Date: <b>4/8/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>4</b>



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 25 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 6



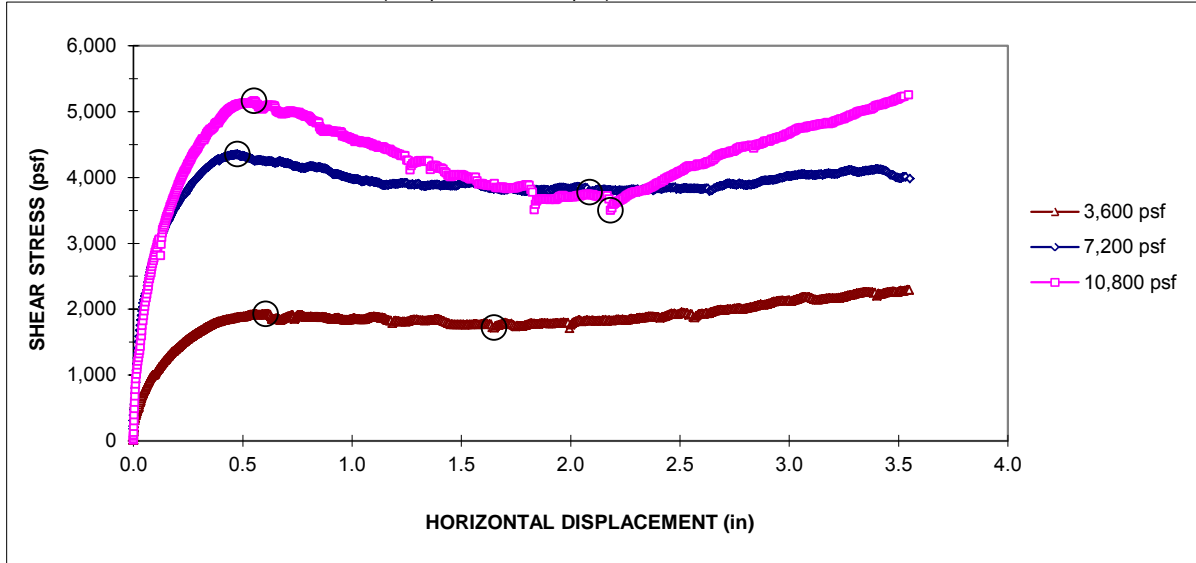
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 100 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> TP-10 @ 3 - 12 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Date:</b> 4/8/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 7

## DIRECT SHEAR TEST RESULTS

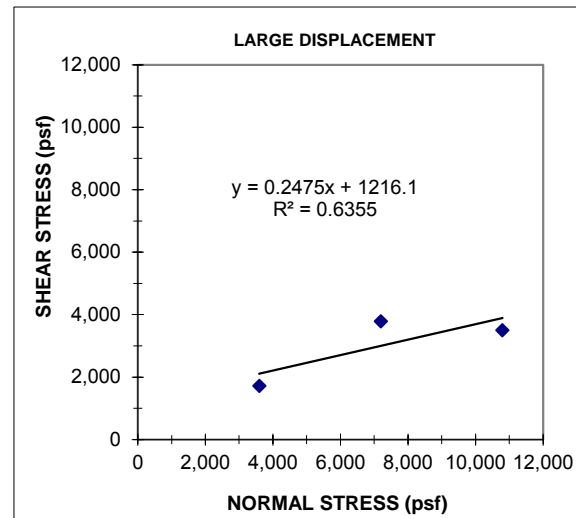
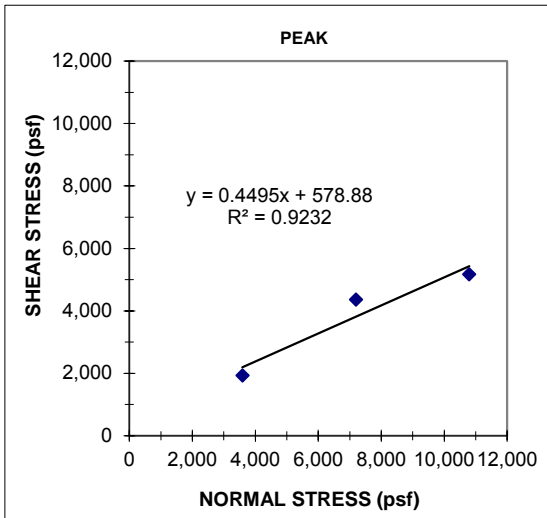
ASTM D5321

PROJECT NAME: Copper Flat Tailings Design Study  
 SAMPLE NUMBER: Tailings / 80-mil Textured Geomembrane / Drain Rock

**APPARTUS:** 12.0 inch by 12.0 inch shear box; air bladder used to apply normal loads.  
**INTERFACE TESTED:** Top: Tailings (remolded to 104.7 pcf @ 15.7% moisture content)  
 Bottom: GSE 80-mil HDPE double-sided textured geomembrane  
**TEST CONDITIONS:** Drain Rock inundated; consolidated overnight at normal load; Floating geomembrane  
**SHEAR RATE:** 0.04 in/min  
**SUBSTRATE:** Drain Rock (Composites 1-4, scalped)



Normal Stress (psf)	Shear Stress		Peak		Large Displacement	
	Peak <sup>1</sup> (psf)	Lg. Displ. (psf)	Friction Angle	Adhesion <sup>2</sup> (psf)	Friction Angle	Adhesion <sup>2</sup> (psf)
3,600	1,927	1,717	24.2	578.9	13.9	1216.1
7,200	4,354	3,778				
10,800	5,164	3,500				



**Observations After Test**

Peak and residual shear strengths were chosen based on testing observations. Shear stresses measured at greater horizontal displacements than those chosen for the large-displacement shear strength appeared to have been affected by gravel particles pushing against the end of the bottom shear box.

- 3,600 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface
- 7,200 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface
- 10,800 psf: Primary failure occurred at Geomembrane-Drain Rock interface; some displacement at Geomembrane-Tailings interface  
 Drain Gravel interfered with shear boxes at approximately 2.2 inches displacement and may have affected

Tech: PRH  
Review: CCS

(1) Peak shear stresses for 3,600 psf, 7,200 psf, and 10,800 psf normal stresses were chosen at 0.606, 0.476, and 0.552 inches horizontal displacements, respectively.

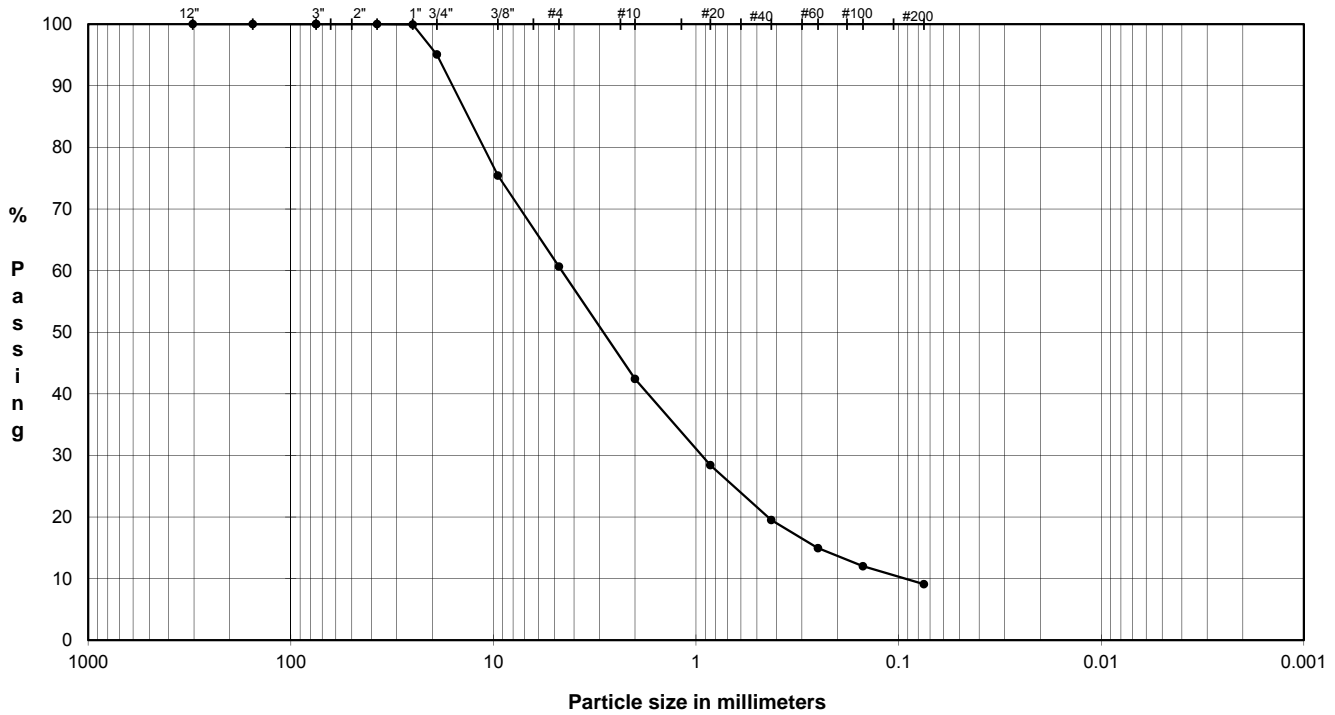
(2) Interface shear parameters are based on the "best-fit" line per ASTM D5321. Interpretation of the test data by a qualified professional for the specific application is required.

**APPENDIX A.3.3  
PERMEABILITY TEST REPORTS**



**PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4** Depth (ft) **--**  
 TYPE: **Pail**

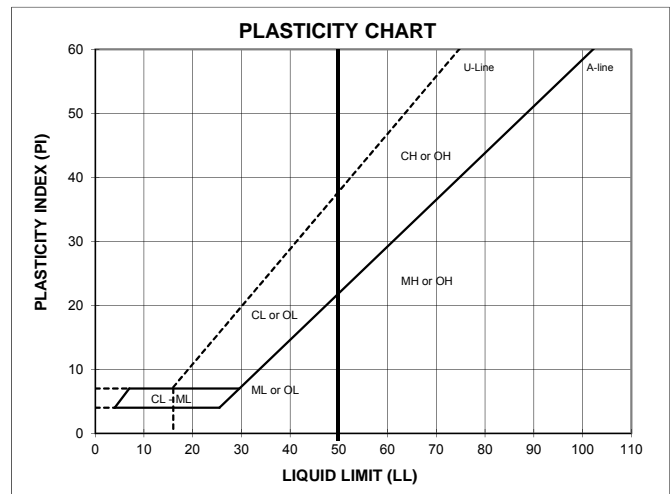


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**PRE-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2		<b>100.0</b>
3.0"	75.0		<b>100.0</b>
3.0"	75.0		<b>100.0</b>
1.5"	37.5		<b>100.0</b>
1.0"	25.0		<b>99.9</b>
0.75"	19.0	Coarse Gravel	<b>4.93</b>
0.375"	9.5		<b>75.4</b>
#4	4.8	Fine Gravel	<b>34.42</b>
#10	2.0	Coarse Sand	<b>18.25</b>
#20	0.9		<b>28.4</b>
#40	0.4	Medium Sand	<b>22.89</b>
#60	0.3		<b>14.9</b>
#100	0.2		<b>12.0</b>
#200	0.1	Fine Sand	<b>10.42</b>
		Fines	<b>9.09</b>



**ATTERBERG LIMITS**

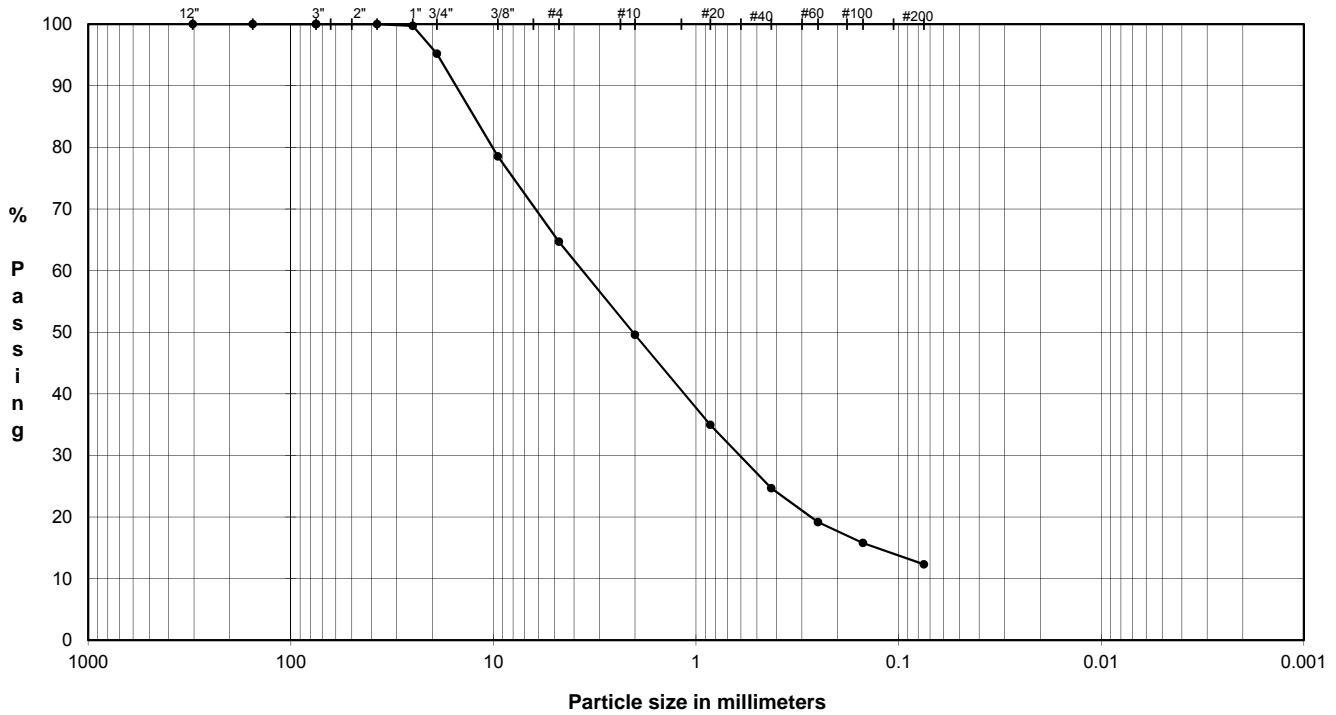
$M_c$	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown,  
 (Golder Procedure): dry  
 USCS: --

TECH: EH  
 DATE: 4/10/13  
 REVIEW: MB

**POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4** Depth (ft) **--**  
 TYPE: **Pail**

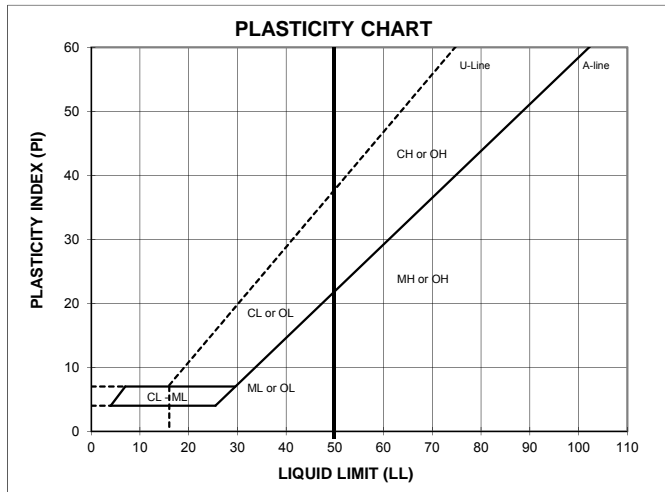


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**POST-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2	Cobbles	<b>0.00</b>
3.0"	75.0		
3.0"	75.0		
1.5"	37.5	Coarse Gravel	<b>4.79</b>
1.0"	25.0		
0.75"	19.0		
0.375"	9.5	Fine Gravel	<b>30.52</b>
#4	4.8		
#10	2.0	Coarse Sand	<b>15.12</b>
#20	0.9	Medium Sand	<b>24.90</b>
#40	0.4		
#60	0.3		
#100	0.2	Fine Sand	<b>12.36</b>
#200	0.1		
Fines			<b>12.31</b>



**ATTERBERG LIMITS**

$M_c$	LL	PL	PI	SG
--	--	--	--	--

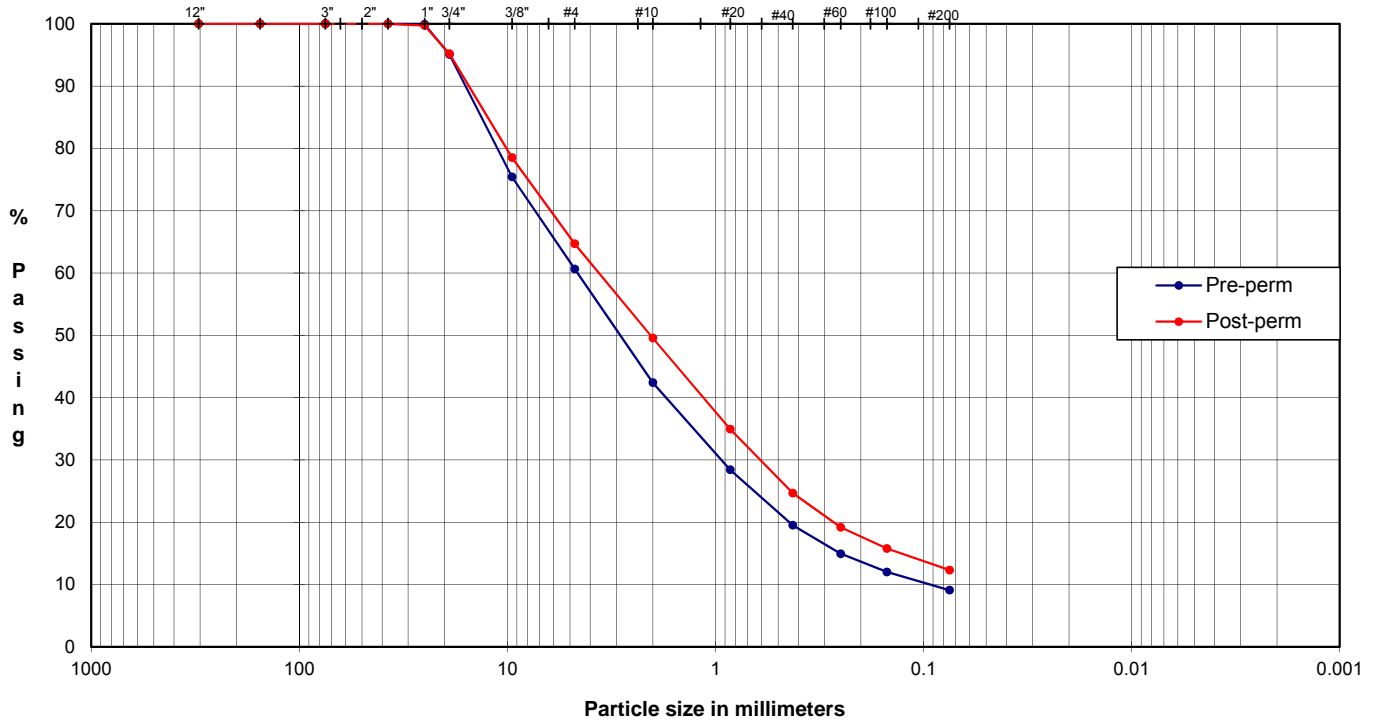
Visual Description: silty clayey SAND and GRAVEL, yellowish brown,  
 (Golder Procedure): dry  
 USCS: --

TECH: EH  
 DATE: 4/16/2013  
 REVIEW: MB

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Compostie 1-4**  
 TYPE: **Pail**

Depth (ft) **--**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			

#### PRE-PERM

#### POST-PERM

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles <b>0.00</b>
6.0"	154.2	100.0	
3.0"	75.0	100.0	
3.0"	75.0	100.0	
1.5"	37.5	100.0	
1.0"	25.0	99.9	
0.75"	19.0	95.1	Coarse Gravel <b>4.93</b>
0.375"	9.5	75.4	Fine Gravel <b>34.42</b>
#4	4.8	60.7	
#10	2.0	42.4	Coarse Sand <b>18.25</b>
#20	0.9	28.4	Medium Sand <b>22.89</b>
#40	0.4	19.5	
#60	0.3	14.9	
#100	0.2	12.0	Fine Sand <b>10.42</b>
#200	0.1	9.1	
Fines			<b>9.09</b>

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles <b>0.00</b>
6.0"	154.2	100.0	
3.0"	75.0	100.0	
3.0"	75.0	100.0	
1.5"	37.5	100.0	
1.0"	25.0	99.7	
0.75"	19.0	95.2	Coarse Gravel <b>4.79</b>
0.375"	9.5	78.5	Fine Gravel <b>30.52</b>
#4	4.8	64.7	
#10	2.0	49.6	Coarse Sand <b>15.12</b>
#20	0.9	34.9	Medium Sand <b>24.90</b>
#40	0.4	24.7	
#60	0.3	19.2	
#100	0.2	15.8	Fine Sand <b>12.36</b>
#200	0.1	12.3	
Fines			<b>12.31</b>

#### ATTERBERG LIMITS

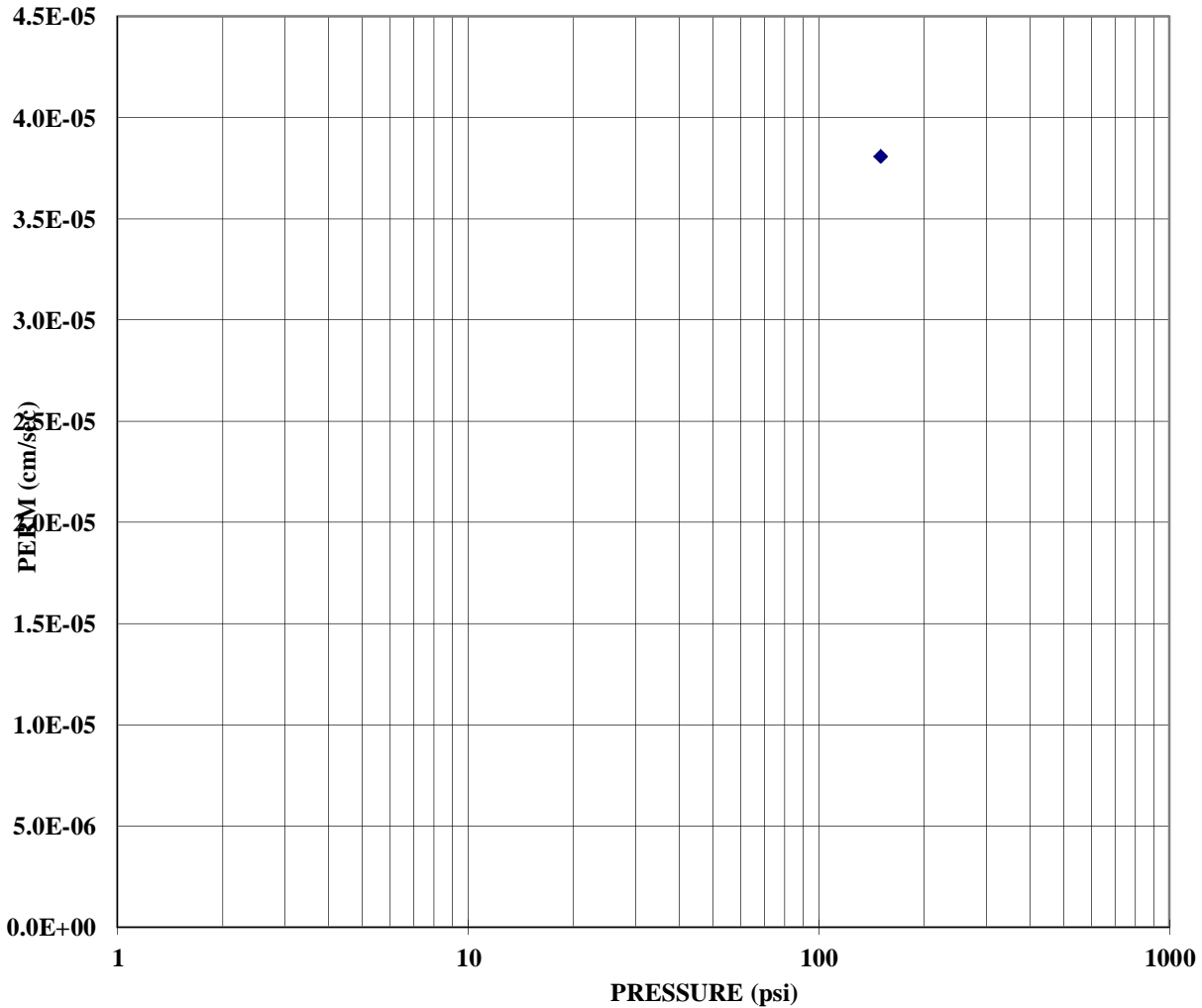
$M_c$	LL	PL	PI	SG
--	--	--	--	--

Visual Description: silty clayey SAND and GRAVEL, yellowish brown, (Golder Procedure): dry

USCS: --

TECH	EH
DATE	4/10/13
REVIEW	MB

ONE-DIMENSIONAL CONSOLIDATION X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Repor



SAMPLE #: Composite 1-4

Visual Description: silty clayey SAND and  
 (Golder Procedure): GRAVEL, yellowish brown, dry

DATE 4/11/2013

TECH MGC

REVIEW MB

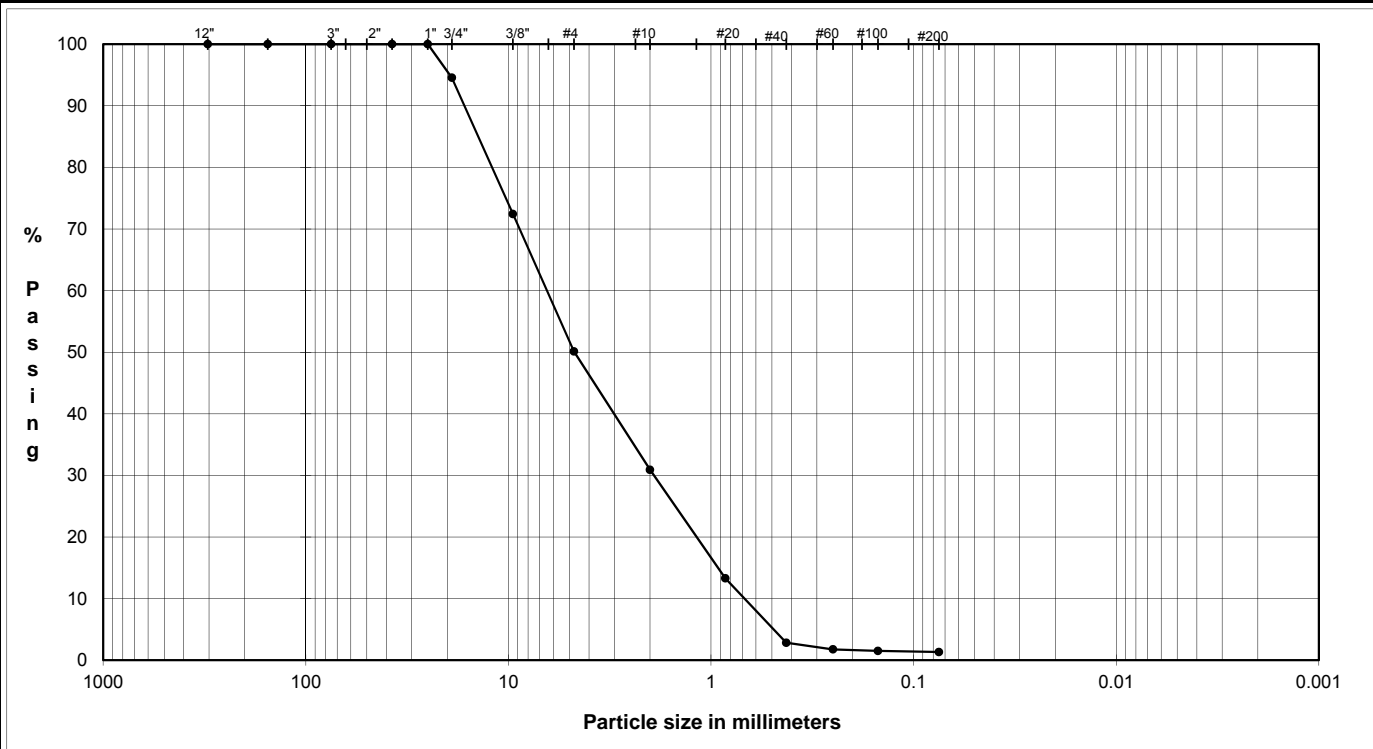
103-92557.006  
 Copper Flat Tailings Design Study

GOLDER ASSOCIATES INC.  
 LAKEWOOD, COLORADO



**PRE-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) **--**  
 TYPE: **Pail**

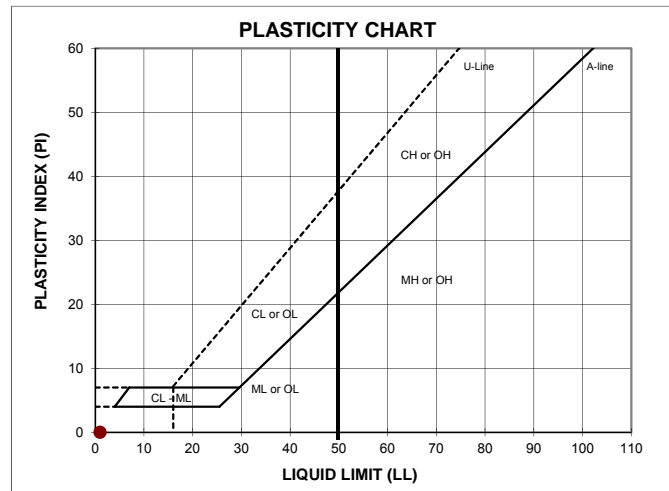


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**PRE-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		
6.0"	154.2		
3.0"	75.0	Cobbles	0.00
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	5.45
0.375"	9.5		
#4	4.8	Fine Gravel	44.42
#10	2.0	Coarse Sand	19.22
#20	0.9		
#40	0.4	Medium Sand	28.06
#60	0.3		
#100	0.2		
#200	0.1	Fine Sand	1.51
		Fines	1.33



**ATTERBERG LIMITS**

$M_c$	LL	PL	PI	SG
--	--	--	--	--

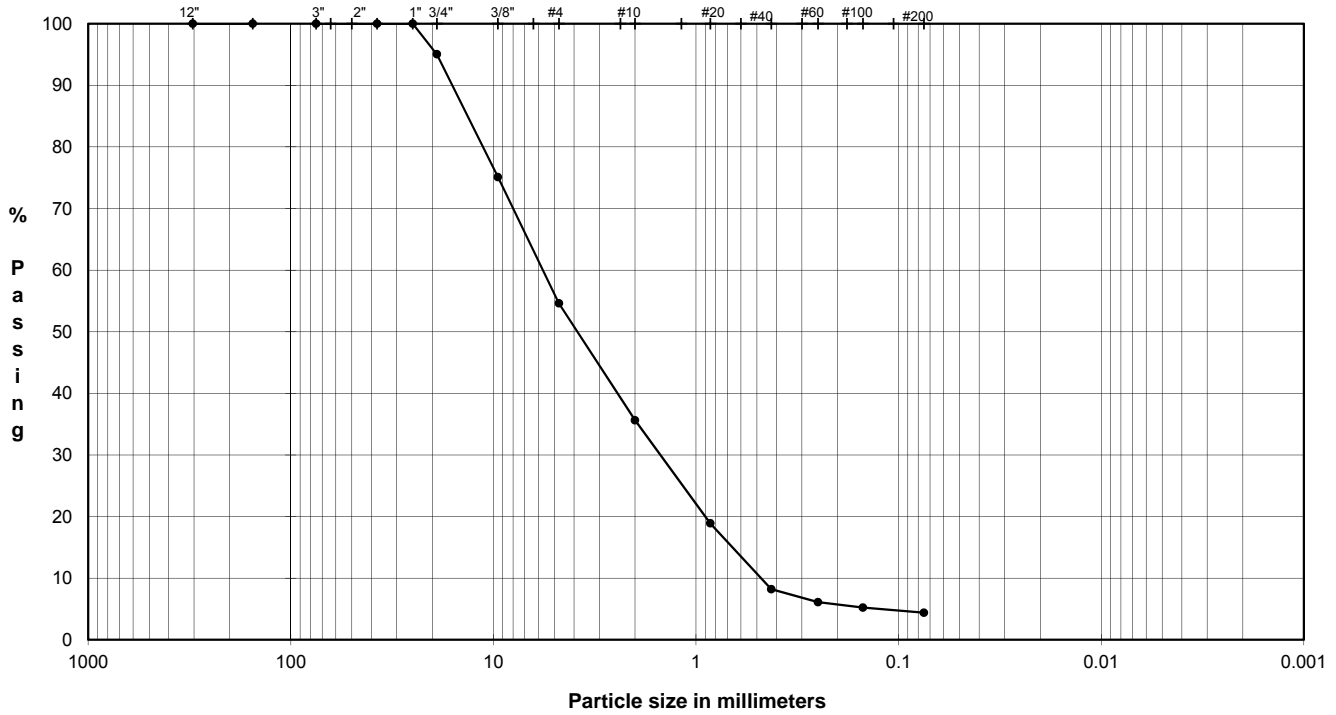
DESCRIPTION: SAND and GRAVEL, yellowish brown

USCS: --

TECH: AM  
 DATE: 4/22/13  
 REVIEW: PRH

**POST-PERM PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED** Depth (ft) **--**  
 TYPE: **Pail**

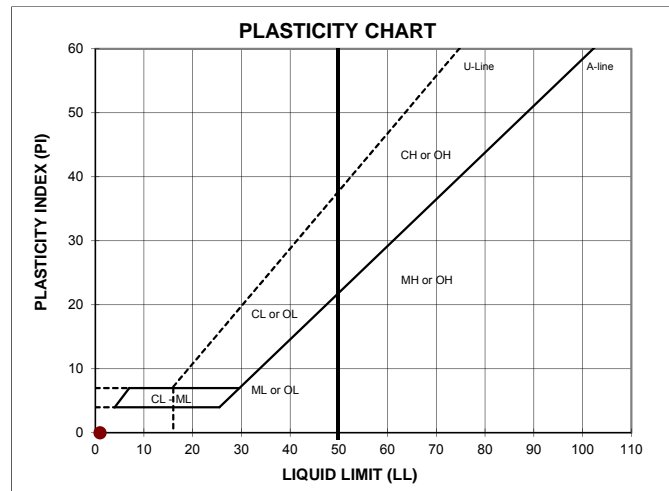


COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

**POST-PERM**

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8		<b>100.0</b>
6.0"	154.2		<b>100.0</b>
3.0"	75.0	Cobbles	<b>0.00</b>
1.5"	37.5		
1.0"	25.0		
0.75"	19.0	Coarse Gravel	<b>4.96</b>
0.375"	9.5		<b>75.1</b>
#4	4.8	Fine Gravel	<b>40.43</b>
#10	2.0	Coarse Sand	<b>18.97</b>
#20	0.9		<b>18.9</b>
#40	0.4	Medium Sand	<b>27.43</b>
#60	0.3		<b>6.1</b>
#100	0.2		<b>5.2</b>
#200	0.1	Fine Sand	<b>3.82</b>
		Fines	<b>4.39</b>



**ATTERBERG LIMITS**

<b>M<sub>c</sub></b>	<b>LL</b>	<b>PL</b>	<b>PI</b>	<b>SG</b>
--	--	--	--	--

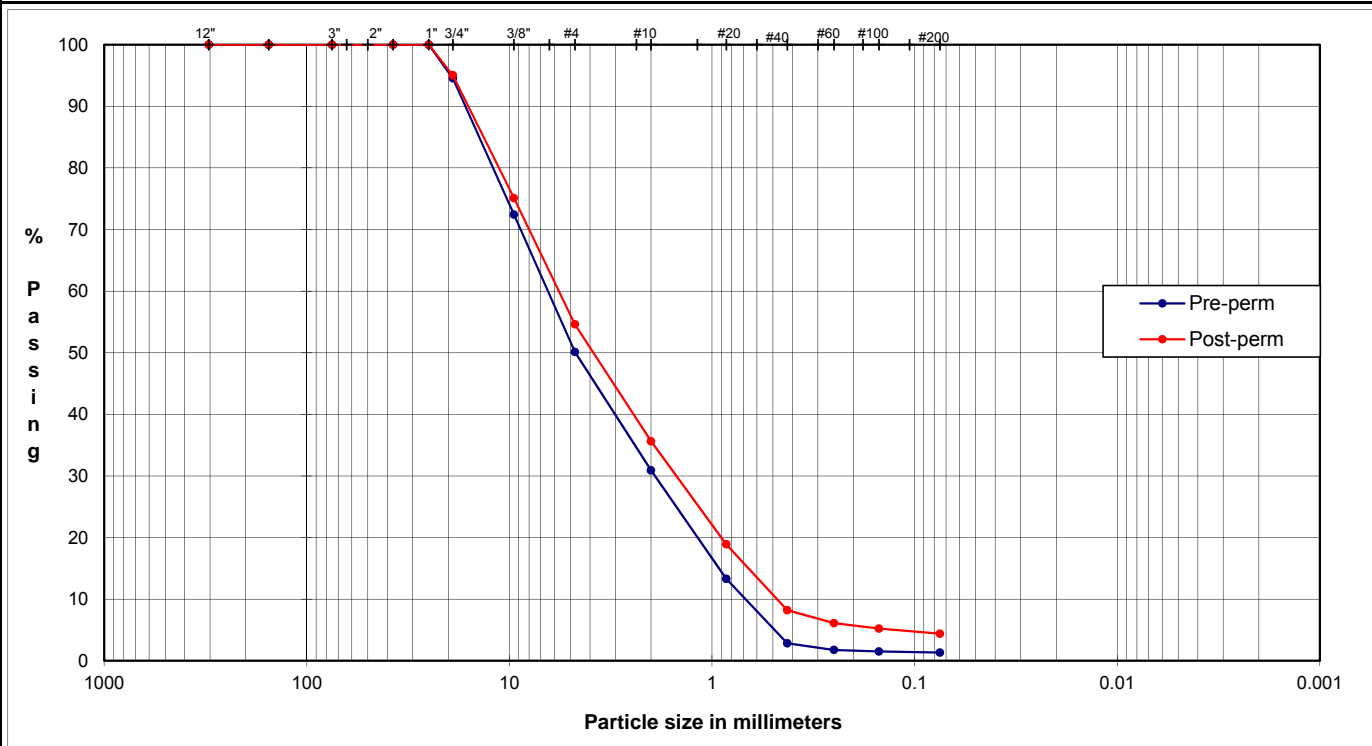
DESCRIPTION: SAND and GRAVEL, yellowish brown

USCS: --

TECH: EH  
 DATE: 5/1/2013  
 REVIEW: PRH

**PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS**  
**ASTM D421, D422, D4318**

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Comp 1-4 SCALPED**      Depth (ft)    --  
 TYPE: **Pail**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

		PRE-PERM			
U.S. Standard Sieves Sizes and Numbers	Particle Size	Particle Size	Classification	Percentage	
	(mm)	% Passing			
	12.0"	304.8	<b>100.0</b>		
	6.0"	154.2	<b>100.0</b>		
	3.0"	75.0	<b>100.0</b>	Cobbles	<b>0.00</b>
	1.5"	37.5	<b>100.0</b>		
	1.0"	25.0	<b>100.0</b>		
	0.75"	19.0	<b>94.5</b>	Coarse Gravel	<b>5.45</b>
	0.375"	9.5	<b>72.4</b>		
	#4	4.8	<b>50.1</b>	Fine Gravel	<b>44.42</b>
	#10	2.0	<b>30.9</b>	Coarse Sand	<b>19.22</b>
	#20	0.9	<b>13.3</b>		
	#40	0.4	<b>2.8</b>	Medium Sand	<b>28.06</b>
	#60	0.3	<b>1.8</b>		
	#100	0.2	<b>1.5</b>		
	#200	0.1	<b>1.3</b>	Fine Sand	<b>1.51</b>
				Fines	<b>1.33</b>

		POST-PERM			
U.S. Standard Sieves Sizes and Numbers	Particle Size	Particle Size	Classification	Percentage	
	(mm)	% Passing			
	12.0"	304.8	<b>100.0</b>		
	6.0"	154.2	<b>100.0</b>		
	3.0"	75.0	<b>100.0</b>	Cobbles	<b>0.00</b>
	1.5"	37.5	<b>100.0</b>		
	1.0"	25.0	<b>100.0</b>		
	0.75"	19.0	<b>95.0</b>	Coarse Gravel	<b>4.96</b>
	0.375"	9.5	<b>75.1</b>		
	#4	4.8	<b>54.6</b>	Fine Gravel	<b>40.43</b>
	#10	2.0	<b>35.6</b>	Coarse Sand	<b>18.97</b>
	#20	0.9	<b>18.9</b>		
	#40	0.4	<b>8.2</b>	Medium Sand	<b>27.43</b>
	#60	0.3	<b>6.1</b>		
	#100	0.2	<b>5.2</b>		
	#200	0.1	<b>4.4</b>	Fine Sand	<b>3.82</b>
				Fines	<b>4.39</b>

		ATTERBERG LIMITS				
		M <sub>c</sub>	LL	PL	PI	SG
DESCRIPTION: SAND and GRAVEL, yellowish brown		--	--	--	--	--
USCS: --						
TECH	AM					
DATE	4/22/13					
REVIEW	PRH					

**Copper Flat Tailing Design Study**  
**Table 1 - Rigid Wall Compression**  
**Falling Head Permeability - 10 inch diameter cell**

**Project Title:** Copper Flat Tailings Design Study  
**Project Number:** 103-92557.006  
**Dates Tested:** 4/26/2013 To: 4/29/2013  
**Boring:** --  
**Sample:** Comp 1-4 SCALPED  
**Depth (ft):** --

**Sample Setup**

Initial Sample Height, in	9.194
Mold Diameter, in	10.00
Sample Area, in <sup>2</sup>	78.54
Wet Sample Weight, g	19,998.9
Wet Sample Weight, lb	44.10
Dry Sample Weight, g	19,416.3
Dry Sample Weight, lb	42.81

**Initial Sample:**

**Moisture Determination**

Tare	PGC
Wet Weight and Tare, g	663.26
Dry Weight and Tare, g	647.65
Tare Weight, g	127.37
Moisture Content, %	3.0

**Initial Sample Density and Void Ratio**

Specific Gravity <sup>1</sup>	2.70
Initial Sample Volume, ft <sup>3</sup>	0.418
Initial Wet Density, lb/ft <sup>3</sup>	105.5
Initial Dry Density, lb/ft <sup>3</sup>	102.5
Initial Void Ratio	0.64

**Final Sample Density and Void Ratio**

Final Sample Height, in	8.330
Final Sample Volume, ft <sup>3</sup>	0.379
Final Dry Density, lb/ft <sup>3</sup>	113.1
Final Void Ratio	0.49

Load (psi)	Height (in)	Dry Density (pcf)	Void Ratio	Flow Rate (ml/sec)	Gradient	Permeability (cm/sec)	Porosity
150	8.330	113.1	0.49	10.62	0.23	9.1E-02	0.33

**NOTES:** <sup>1</sup>Specific Gravity = Assumed Value



### CONSTANT-HEAD PERMEABILITY (RIGID-WALL)

JOB NUMBER: 103-92557.006  
 JOB NAME: Copper Flat Tailings Design Study  
 DATE TESTED: 04/26/13

BORING NUMBER: --  
 SAMPLE NUMBER: Comp 1-4 SCALPED  
 SAMPLE DEPTH: --

**Initial Moisture Content**

Tare:	PGC
Wet Weight & Tare, g:	663.26
Dry Weight & Tare, g:	647.65
Tare Weight, g:	127.37
Moisture, %:	<b>3.0</b>

**Final Moisture Content**

Tare:	J11
Wet Weight & Tare, g:	662.43
Dry Weight & Tare, g:	607.20
Tare Weight, g:	82.33
Moisture, %:	<b>10.5</b>

**Initial Height Determination (Inches)**

(Height from mold rim to plate on sample)	
1.	3.447
2.	3.442
3.	3.404
4.	3.379
5.	3.404
6.	3.365
Average	3.406
Cell Height	12.600
Sample Height <sup>1</sup>	9.194

**Density**

Wet Weight:	19,998.90 g	
Dry Weight:	19,416.35 g	
Diameter:	10.000 in	25.40 cm
Area:	78.540 in <sup>2</sup>	506.71 cm <sup>2</sup>
Initial Height:	9.194 in	23.35 cm
Final Height <sup>2</sup> :	8.330 in	21.16 cm
Initial Volume:	0.418 ft <sup>3</sup>	11,832.48 cm <sup>3</sup>
Final Volume:	0.379 ft <sup>3</sup>	10,721.13 cm <sup>3</sup>
Initial Wet Density:	105.6 pcf	
Final Wet Density:	116.5 pcf	
Initial Dry Density:	102.5 pcf	
Final Dry Density:	113.1 pcf	

TRIAL	TIME	OUTFLOW	BURETTE	Q	i	k
	seconds	ml	(mm)	(cm <sup>3</sup> /sec)		cm/sec
01	240	1704.9	35	7.1	0.165	8.48E-02
02	240	1704.5	35	7.1	0.165	8.47E-02
03	240	1705.2	35	7.1	0.165	8.48E-02
04	240	2513.2	48	10.5	0.227	9.11E-02
05	240	2508.93	48	10.5	0.227	9.09E-02
06	240	2507.75	48	10.4	0.227	9.09E-02
07	240	3436.38	62	14.3	0.293	9.64E-02
08	240	3434.01	62	14.3	0.293	9.64E-02
09	240	3435.08	62	14.3	0.293	9.64E-02
10						
<b>Average</b>	240.0	2549.996	48.3	10.6	0.228	9.07E-02

**NOTES:** <sup>1</sup>Sample Height = Cell Height - Average distance to rim of mold

<sup>2</sup>Final Height = Initial Height - Displacement of Final Reading *Before* Running Perm Test

**CONSTANT-HEAD PERMEABILITY  
(RIGID-WALL)**

JOB NUMBER: 103-92557.006  
 JOB NAME: Copper Flat Tailings Design Study  
 DATE TESTED: 04/26/13

BORING NUMBER: --  
 SAMPLE NUMBER: Comp 1-4 SCALPED  
 SAMPLE DEPTH: --

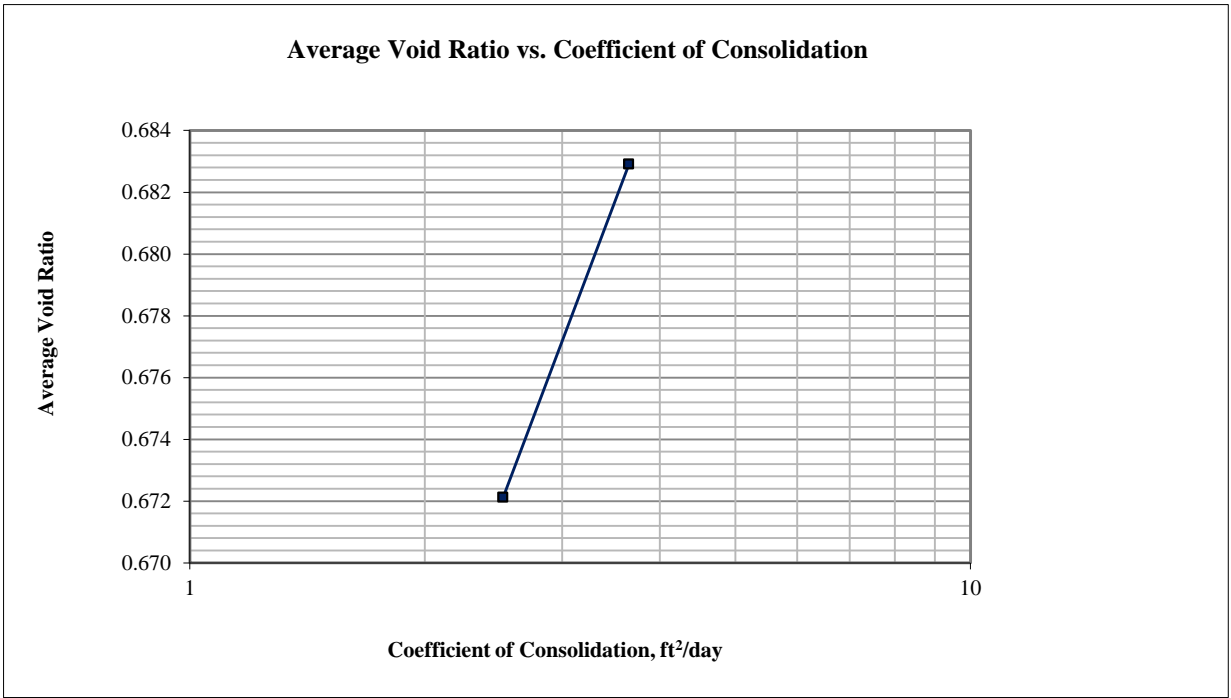
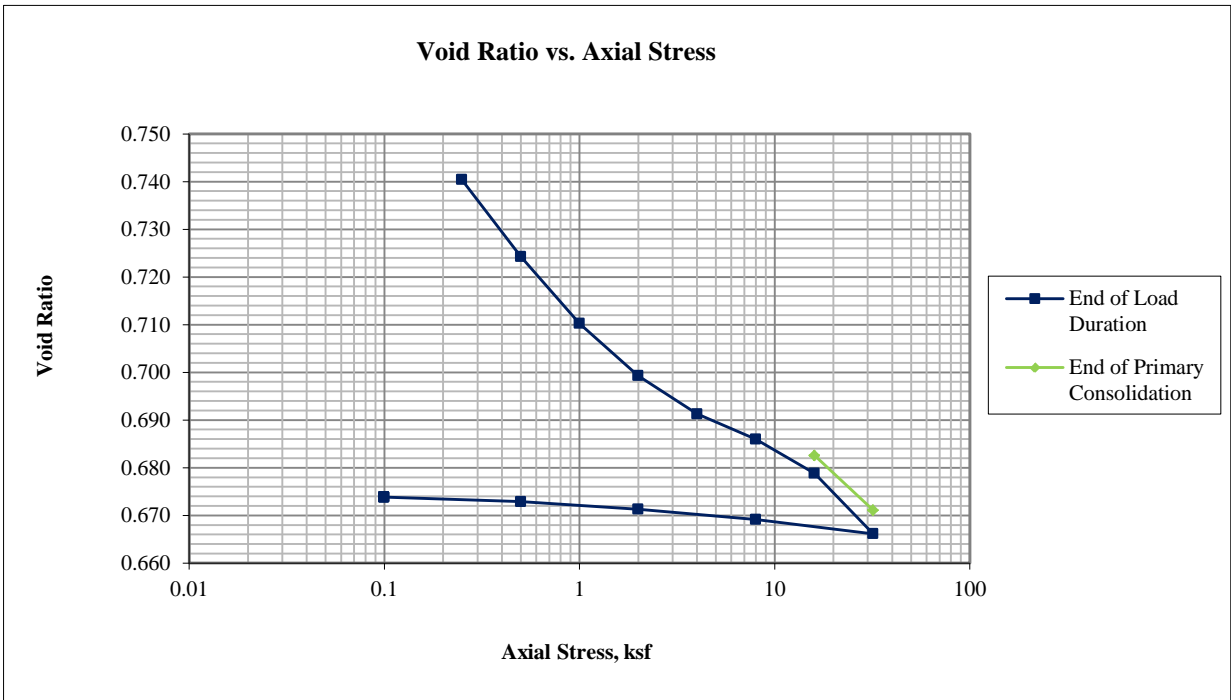
DATE	TIME	LOAD (psi)	DIAL (Left)	DIAL (Right)	AVERAGE	DISPLACEMENT
INITIAL	10:10 AM	0	0.005	0.004	0.005	--
SATURATE	10:25 AM	0	0.021	0.044	0.033	0.028
LOAD	12:40 PM	150	0.775	0.742	0.759	0.754
	8:55 AM	150	0.880	0.844	0.862	0.858
PERM	7:55 AM	150	0.886	0.850	0.868	0.864
END PERM	12:10 PM	150	0.887	0.851	0.869	0.865

**APPENDIX A.3.4  
CONSOLIDATION TEST REPORTS**

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.994 in		0.960 in	USCS description (ASTM D2487):	Lean clay with sand, yellowish red, moist
Diameter =	2.499 in		2.499 in	Atterberg Limits (ASTM D4318):	LL = 47 PL = 20 PI = 27
Area =	4.905 in <sup>2</sup>		4.905 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 99% No. 200 = 81%
Volume =	4.875 in <sup>3</sup>		4.709 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	14.4%		4.8%	Remold Targets:	95.0 pcf (dry) at 15.0% moisture
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.6%
Height of Solids =	0.5641 in		0.5641 in	Trimming Procedure:	Specimen remolded in ring
Void Ratio =	0.762		0.702	Inundation:	<input checked="" type="checkbox"/> Not inundated <input type="checkbox"/> Inundated
Degree of Saturation =	51.1%		18.5%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.308 lb		0.282 lb	Apparatus:	Frame No. 1 (Wykeham Farrance 24251)
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	109.3 pcf		103.6 pcf	Final Differential Height:	-0.0158 in
Dry Unit Weight =	95.5 pcf		98.9 pcf	Estimated Preconsolidation Stress:	Not Computed

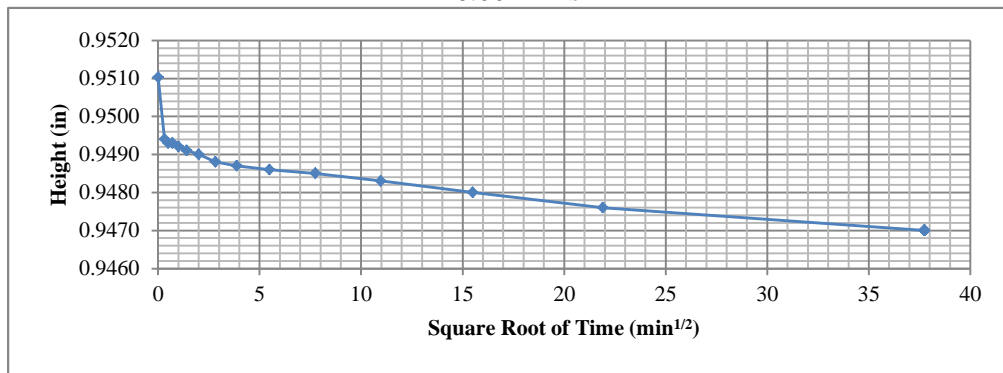
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-10 @ 19-33 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1

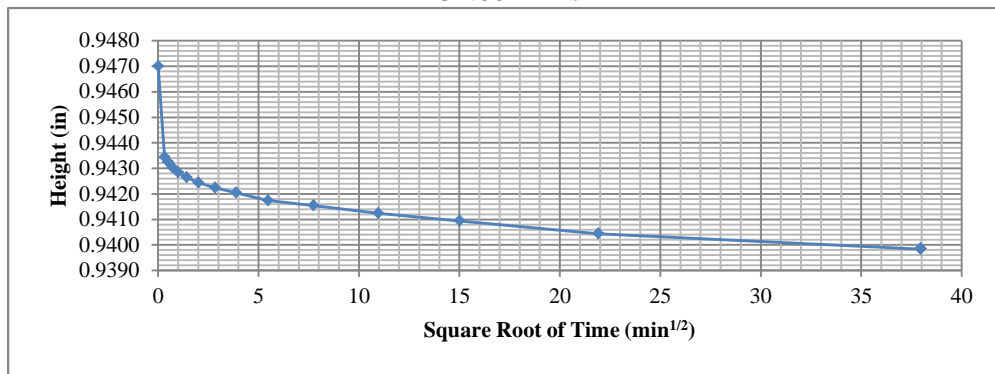


<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-10 @ 19-33 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

16.00 ksf



32.00 ksf



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 TIME-DEFORMATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-10 @ 19-33 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/11/2013

Job Number:

103-92557.006

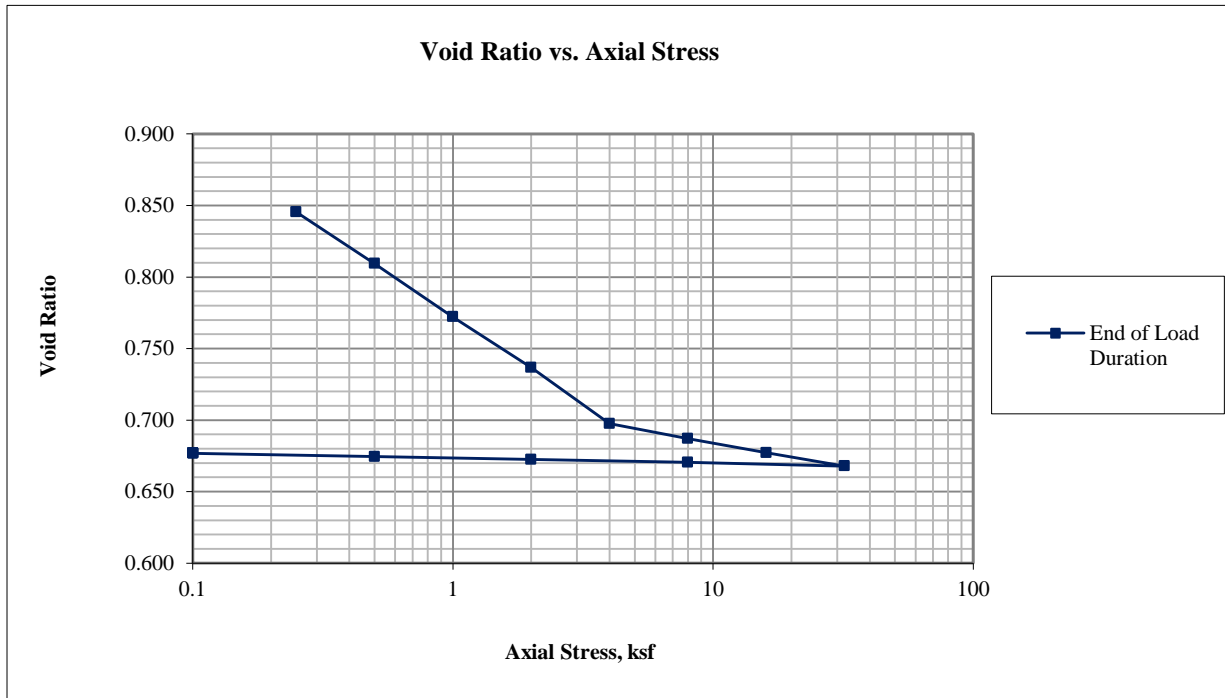
Figure:

3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in <sup>2</sup>		4.893 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in <sup>3</sup>		4.428 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-12 @ 33.5-48.5 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1



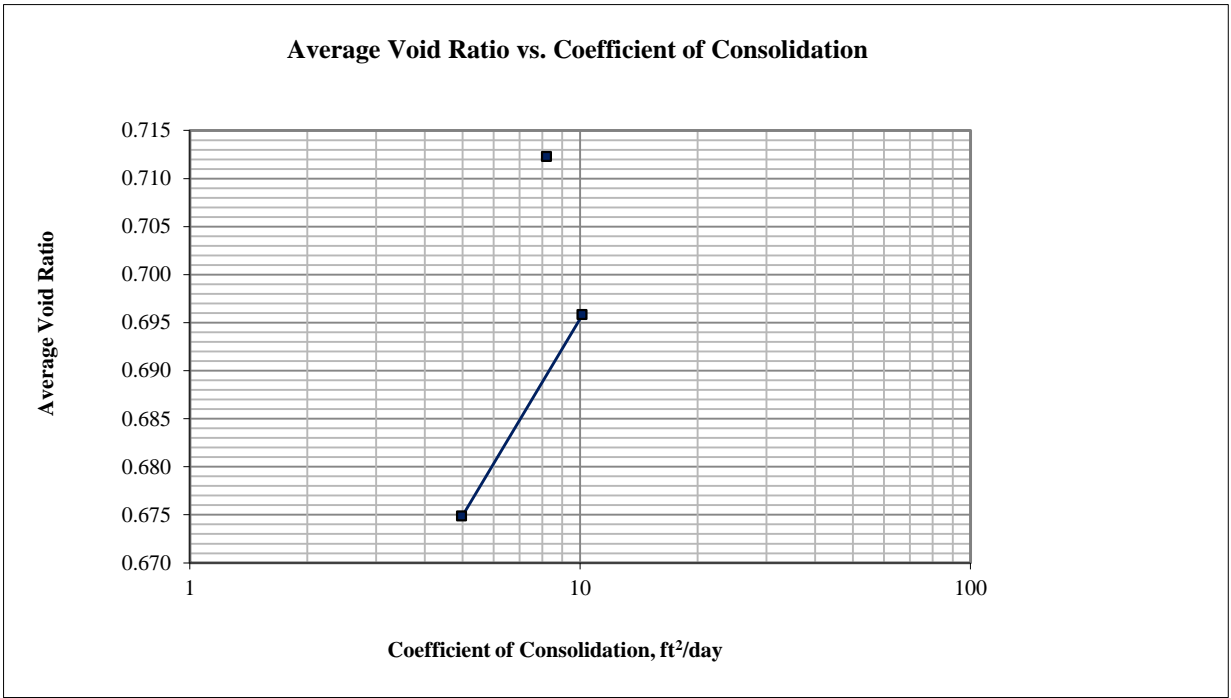
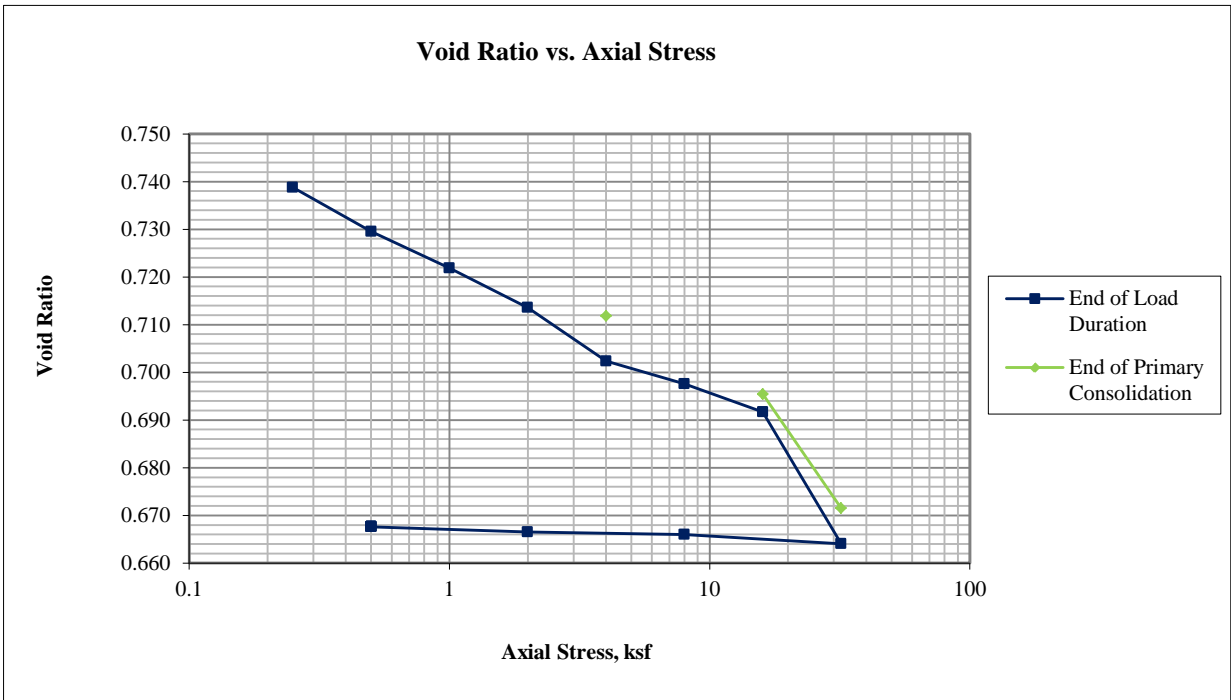
<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-12 @ 33.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2



	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.993 in		0.887 in	USCS description (ASTM D2487):	Clayey sand with gravel, yellowish brown, dry
Diameter =	2.500 in		2.500 in	Atterberg Limits (ASTM D4318):	LL = 23 PL = 14 PI = 9
Area =	4.909 in <sup>2</sup>		4.909 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98% No. 4 = 83% No. 200 = 36%
Volume =	4.874 in <sup>3</sup>		4.354 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	14.1%		1.7%	Remold Targets:	95.0 pcf (dry) at 15.0% moisture
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%
Height of Solids =	0.5636 in		0.5636 in	Trimming Procedure:	Specimen remolded in ring
Void Ratio =	0.762		0.574	Inundation:	<input checked="" type="checkbox"/> Not inundated <input type="checkbox"/> Inundated
Degree of Saturation =	49.8%		8.0%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.307 lb		0.274 lb	Apparatus:	Frame No. 1 (Wykeham Farrance 24251)
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	108.9 pcf		108.7 pcf	Final Differential Height:	0.0528 in
Dry Unit Weight =	95.5 pcf		106.9 pcf	Estimated Preconsolidation Stress:	16.4 ksf

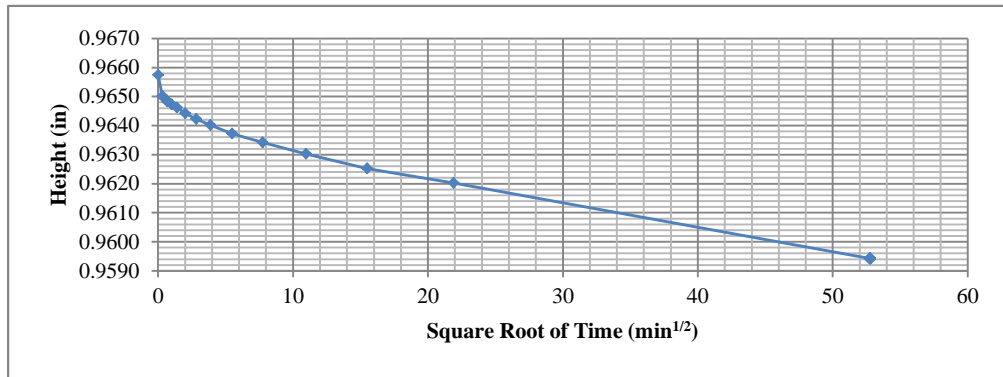
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-16 @ 29-34 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/25/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1

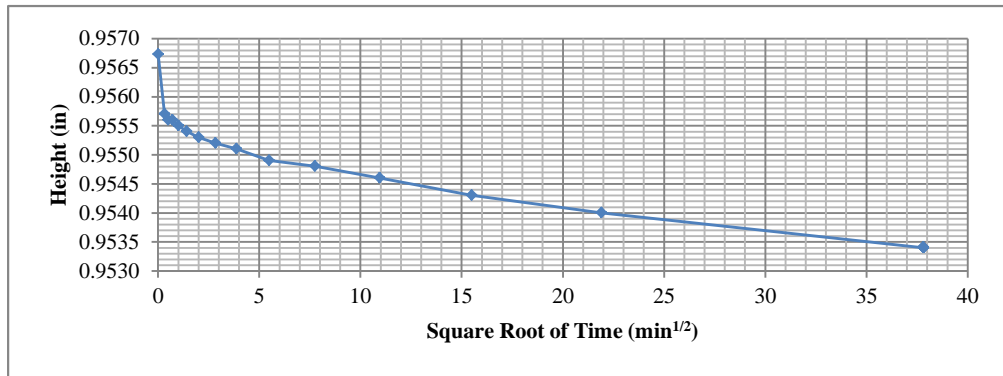


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-16 @ 29-34 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

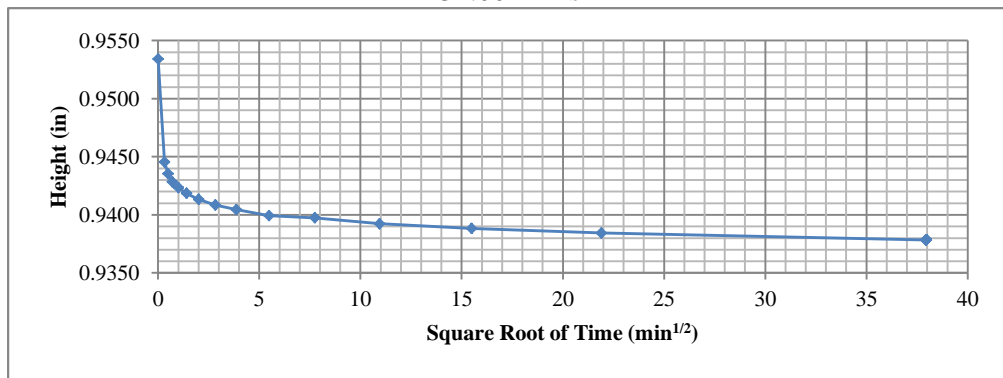
4.00 ksf



16.00 ksf



32.00 ksf



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 TIME-DEFORMATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-16 @ 29-34 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/25/2013

Job Number:

103-92557.006

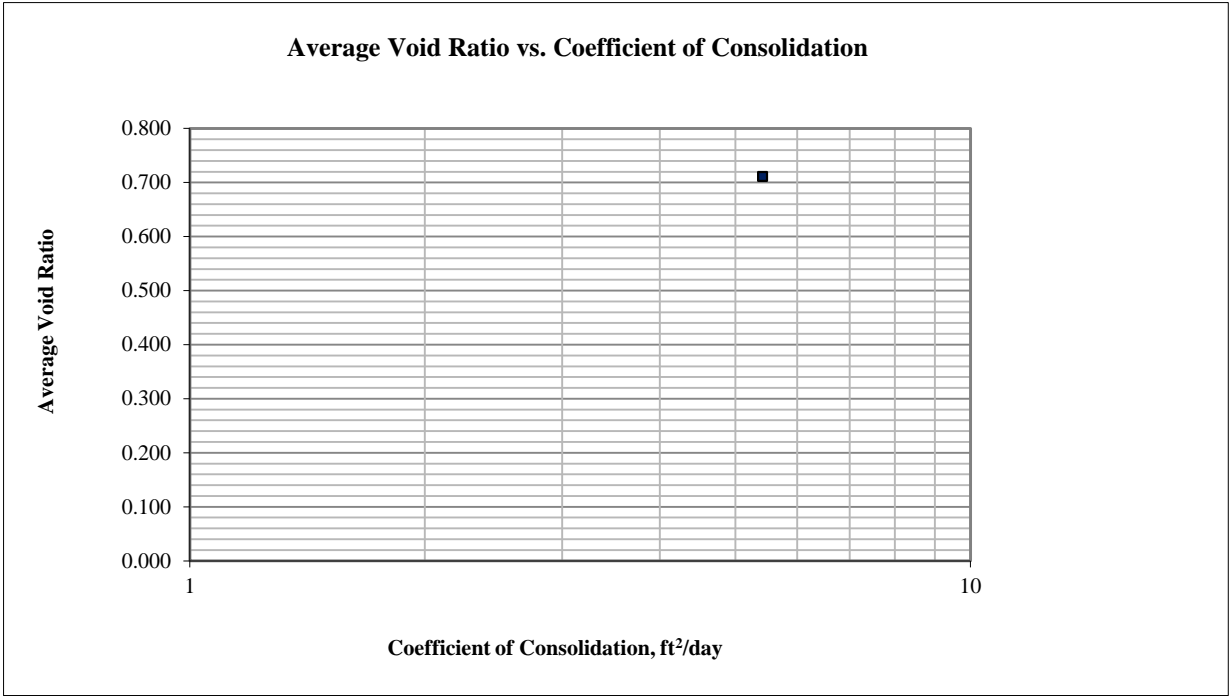
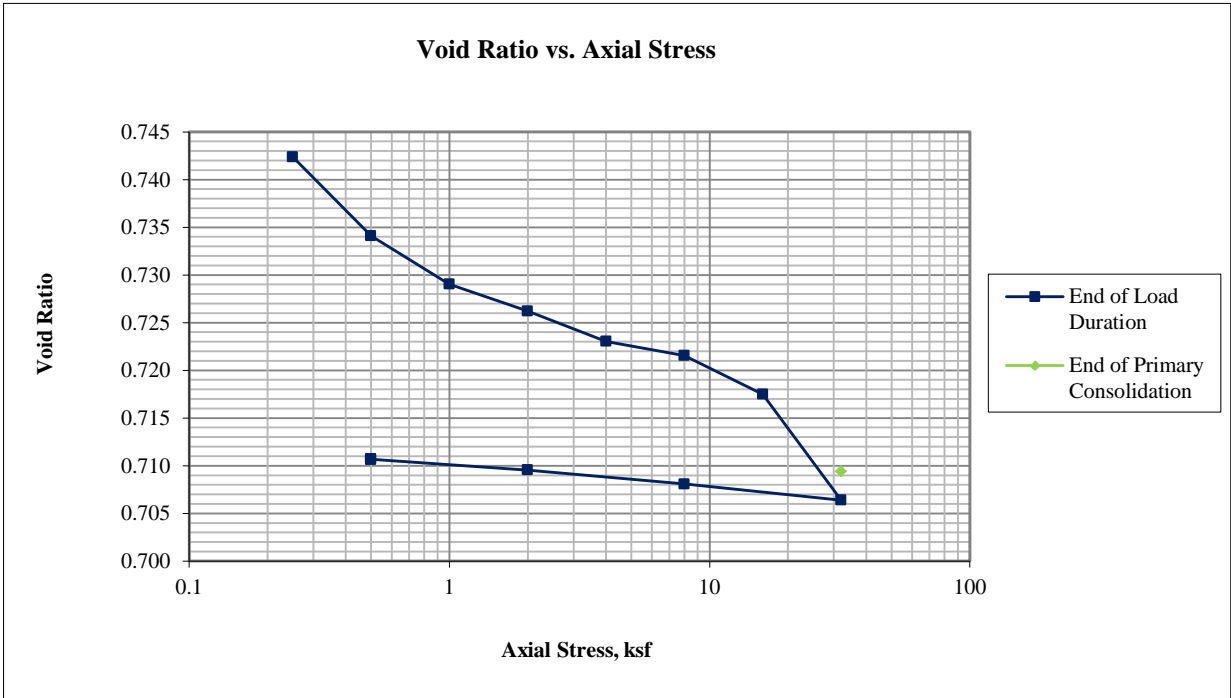
Figure:

3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997	in	0.960	in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist	
Diameter =	2.498	in	2.498	in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 14
Area =	4.901	in <sup>2</sup>	4.901	in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%
Volume =	4.886	in <sup>3</sup>	4.705	in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted
Water Content =	14.5%		2.7%		Remold Targets:	95.0 pcf (dry) at	15.0% moisture
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%	
Height of Solids =	0.5638	in	0.5638	in	Trimming Procedure:	Specimen remolded in ring	
Void Ratio =	0.768		0.703		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated
Degree of Saturation =	51.1%		10.4%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B
Wet Mass =	0.308	lb	0.276	lb	Apparatus:	Frame No. 4	(ELE C-320A)
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial
Wet Unit Weight =	109.0	pcf	101.5	pcf	Final Differential Height:	0.0045 in	
Dry Unit Weight =	95.1	pcf	98.8	pcf	Estimated Preconsolidation Stress:	14.7 ksf	

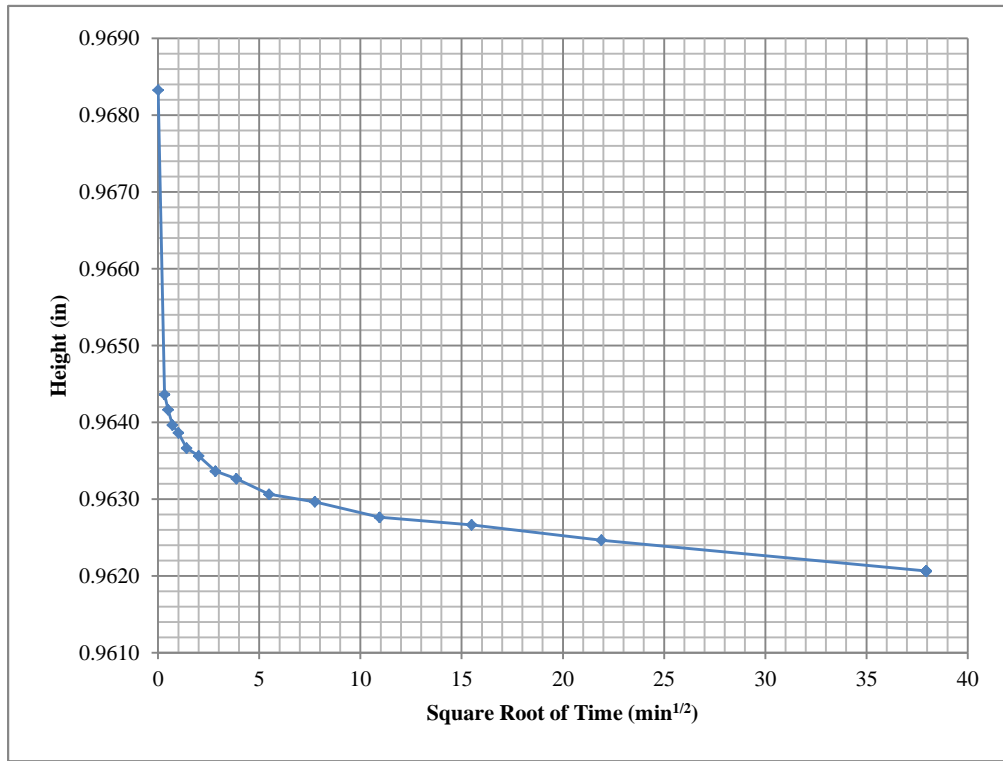
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

<b>Golder Associates Inc. Denver, Colorado</b>			<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
<b>Job Short Title:</b> Copper Flat Tailings Design Study							
<b>Sample:</b> BH-18 @ 23-33.5 ft			<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-18 @ 23-33.5 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>

32.00 ksf



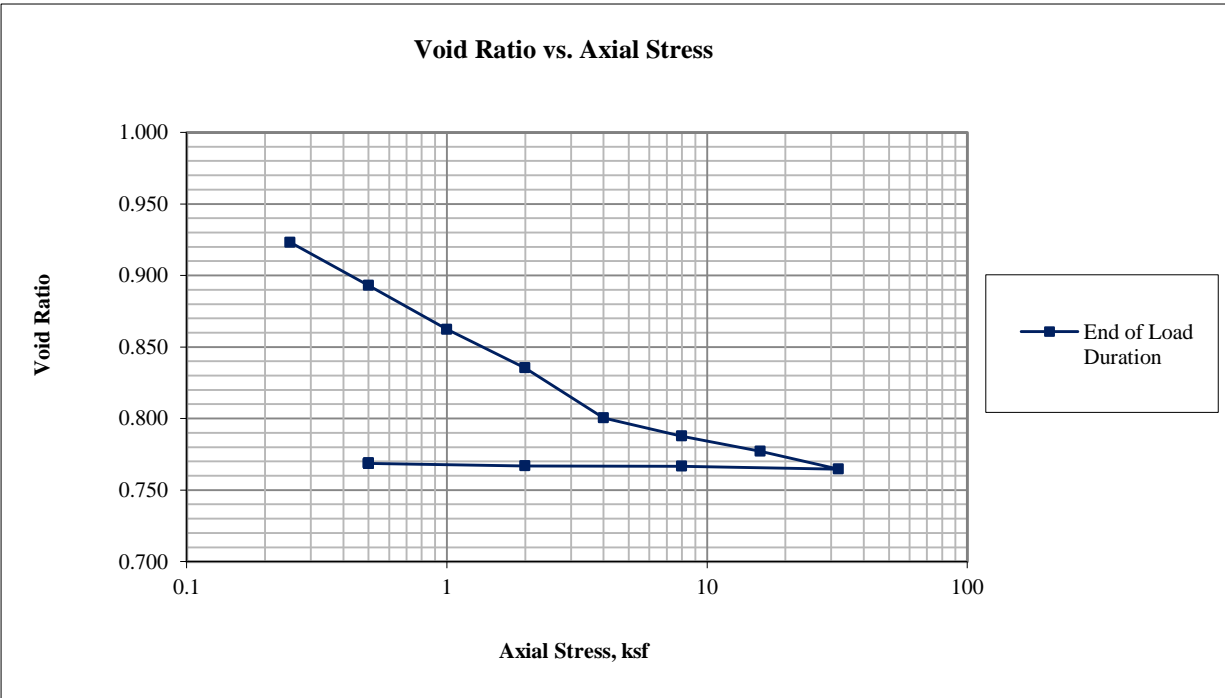
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.994 in		0.924 in	USCS description (ASTM D2487):	Fat clay with sand, dark red, wet		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 62	PL = 20	PI = 42
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 82%
Volume =	4.871 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	28.9%		9.0%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	29.1%		
Height of Solids =	0.5085 in		0.5085 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.955		0.817	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	81.7%		29.7%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.243 lb		0.243 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.9 pcf		100.9 pcf	Final Differential Height:	-0.0246 in		
Dry Unit Weight =	86.1 pcf		92.6 pcf	Estimated Preconsolidation Stress:	-- ksf		

-- indicates test was not performed

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-18 @ 43.5-48.5 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/25/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1



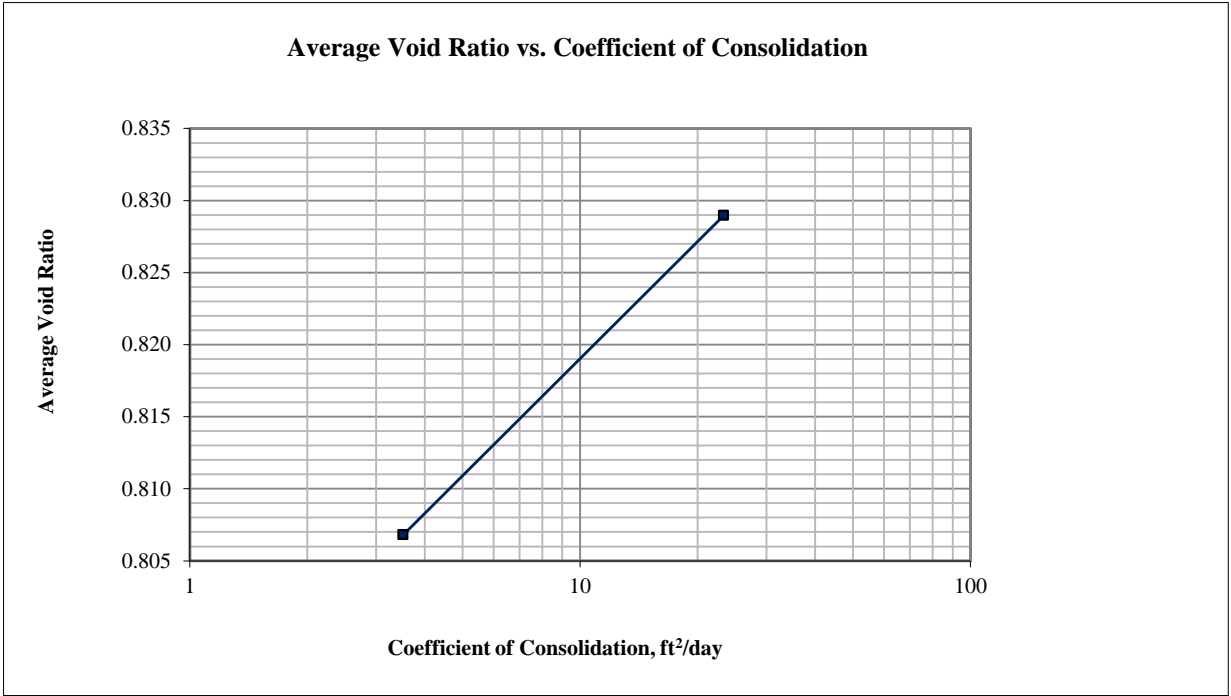
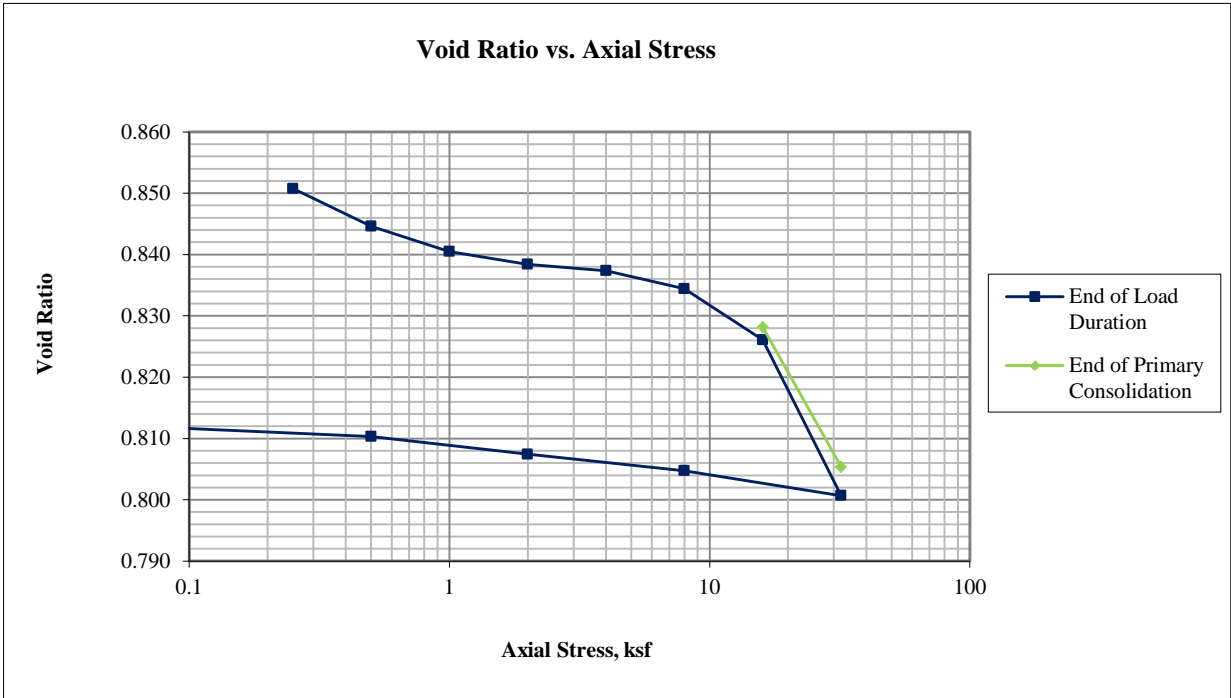
<b>Golder Associates Inc. Denver, Colorado</b>	<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 43.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2



	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	1.000 in		0.981 in	USCS description (ASTM D2487):	Sandy silty clay, brownish yellow, moist		
Diameter =	2.497 in		2.497 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 21	PI = 4
Area =	4.897 in <sup>2</sup>		4.897 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 99%	No. 4 = 99%	No. 200 = 52%
Volume =	4.897 in <sup>3</sup>		4.804 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	9.7%		1.1%	Remold Targets:	90.0 pcf (dry) at	10.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.4%		
Height of Solids =	0.5363 in		0.5363 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.865		0.829	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	30.3%		3.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.281 lb		0.259 lb	Apparatus:	Frame No. 6	(ELE C-320A)	
Dry Mass =	0.256 lb		0.256 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	99.0 pcf		93.0 pcf	Final Differential Height:	-0.0094 in		
Dry Unit Weight =	90.2 pcf		92.0 pcf	Estimated Preconsolidation Stress:	13.0 ksf		

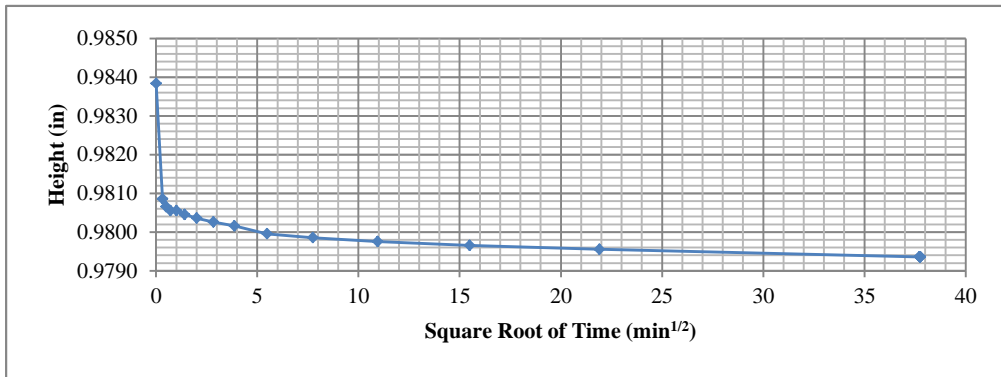
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-22 @ 0-8.5 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1

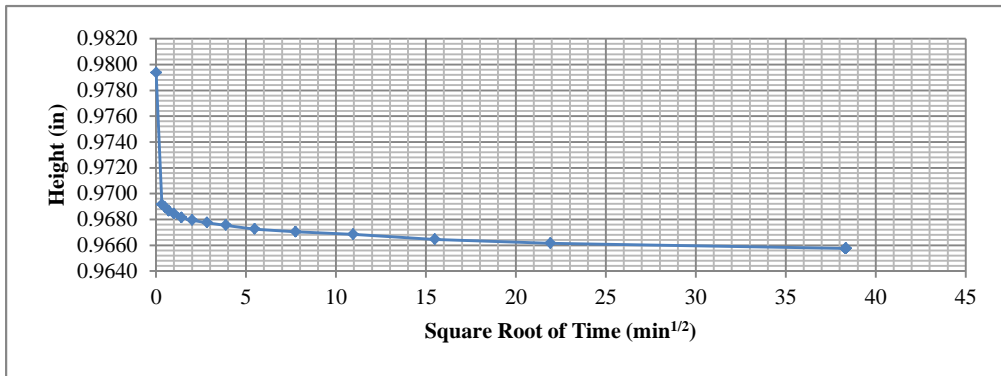


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>BH-22 @ 0-8.5 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>2</b>

16.00 ksf



32.00 ksf

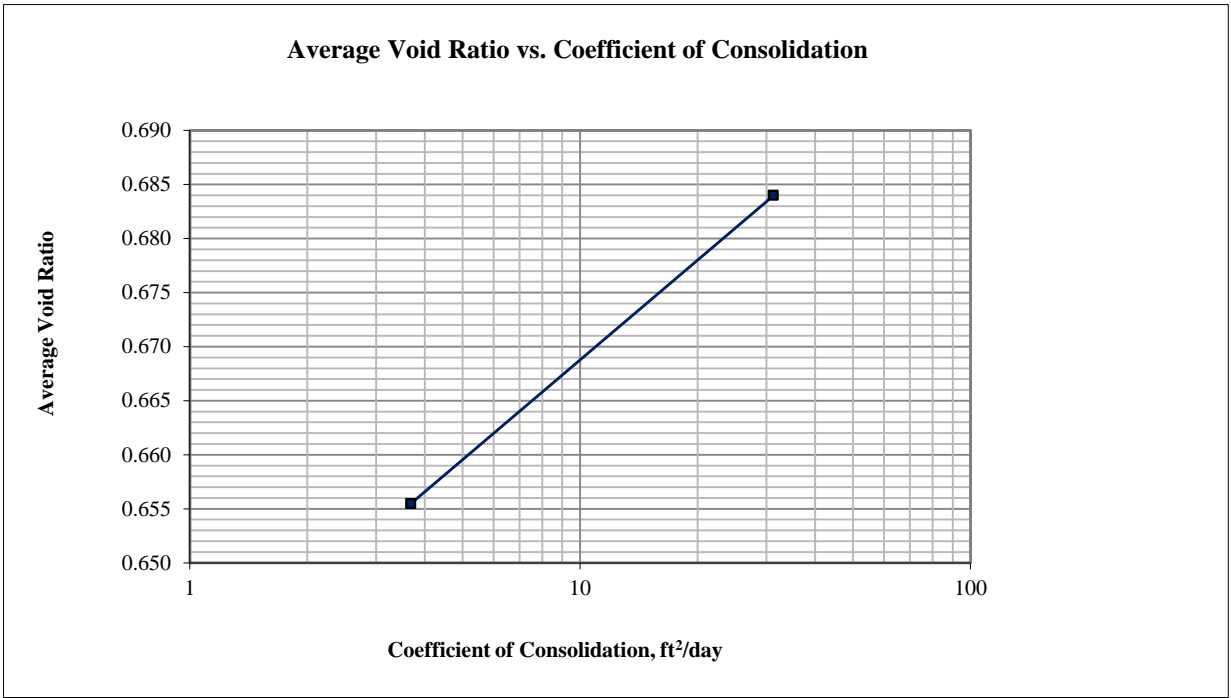
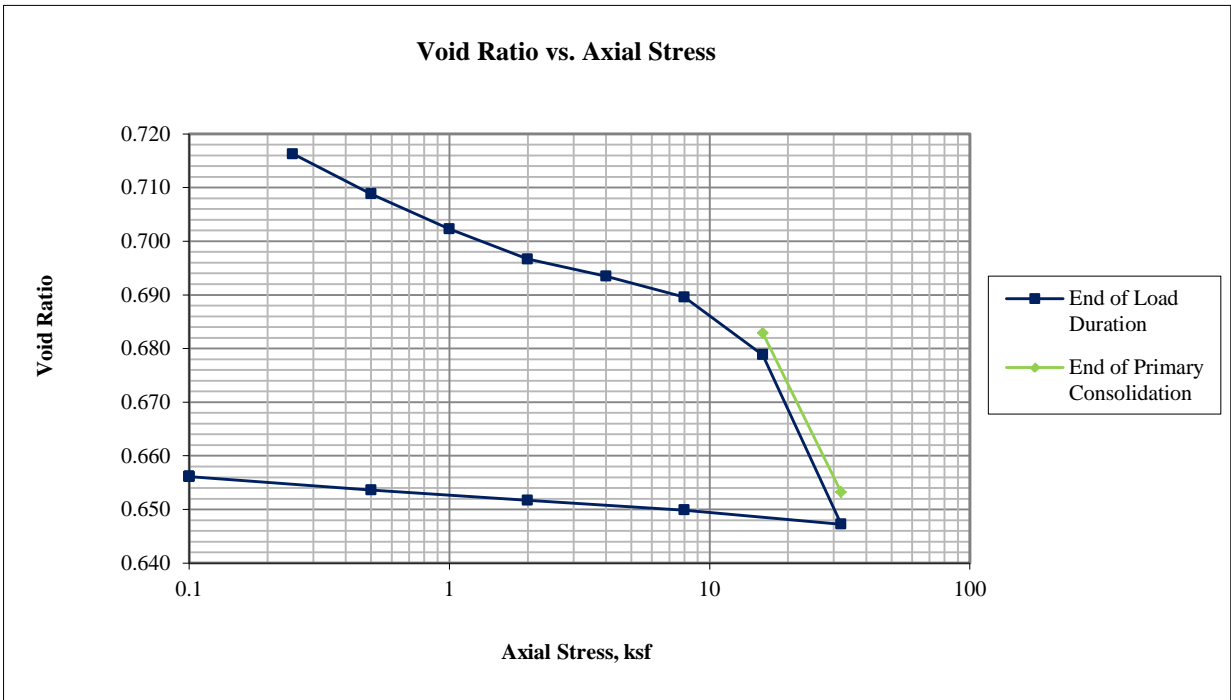


<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-22 @ 0-8.5 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>	

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36	PL = 18	PI = 18
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 82%	No. 200 = 37%
Volume =	4.886 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%		
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in		
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf		

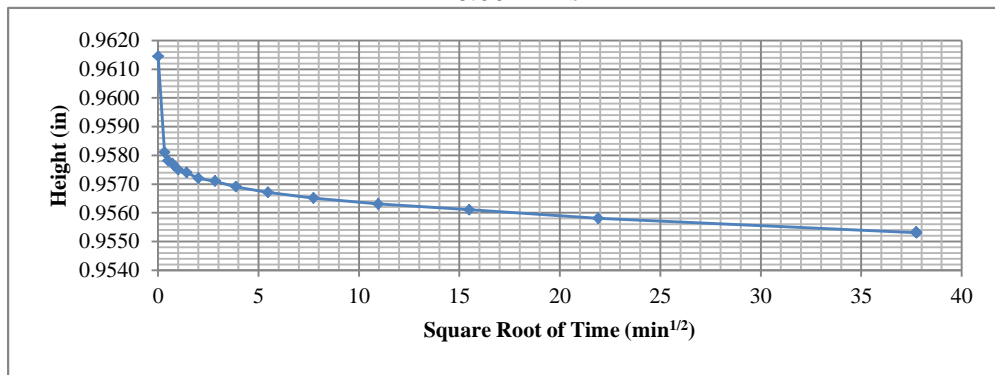
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  BH-22 @ 28-30 ft	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  3/11/2013	<b>Job Number:</b>  103-92557.006	<b>Figure:</b>  1

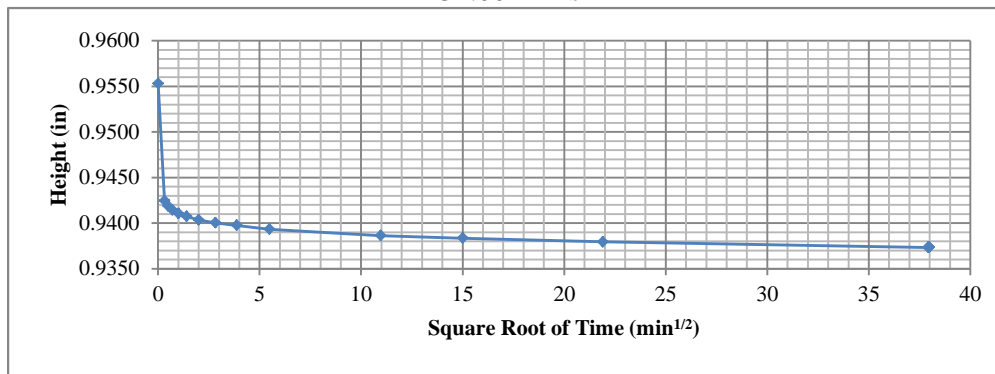


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>BH-22 @ 28-30 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>2</b>

16.00 ksf



32.00 ksf



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 TIME-DEFORMATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-22 @ 28-30 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

3/11/2013

Job Number:

103-92557.006

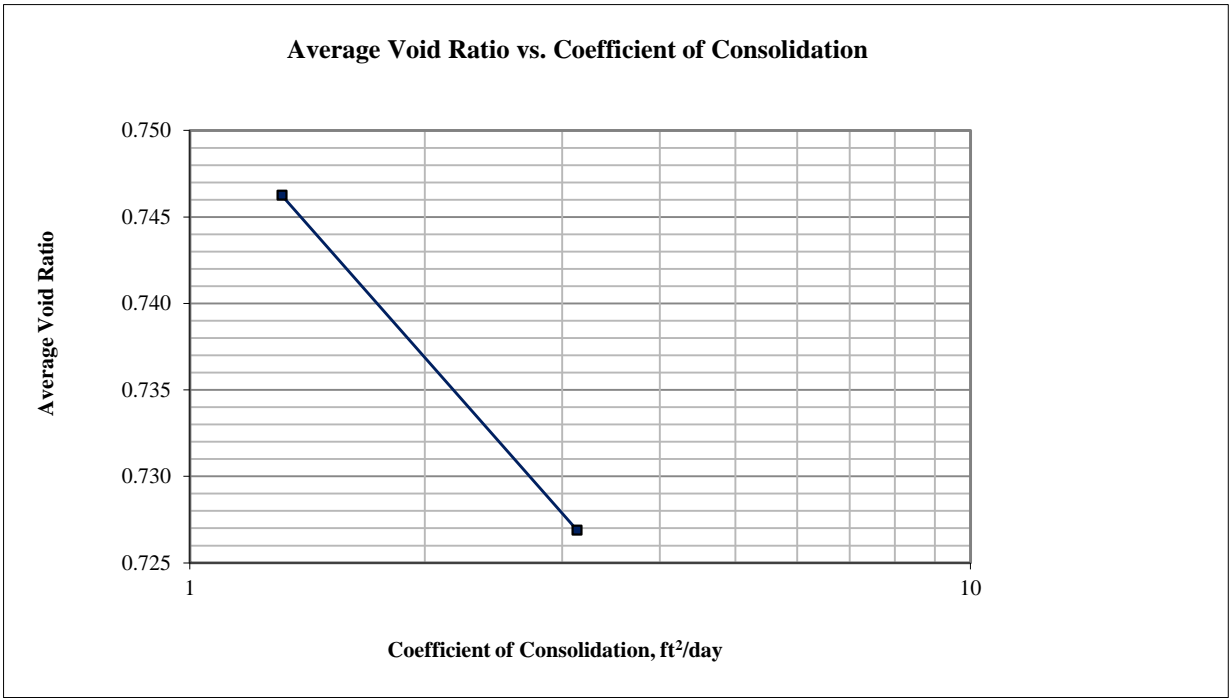
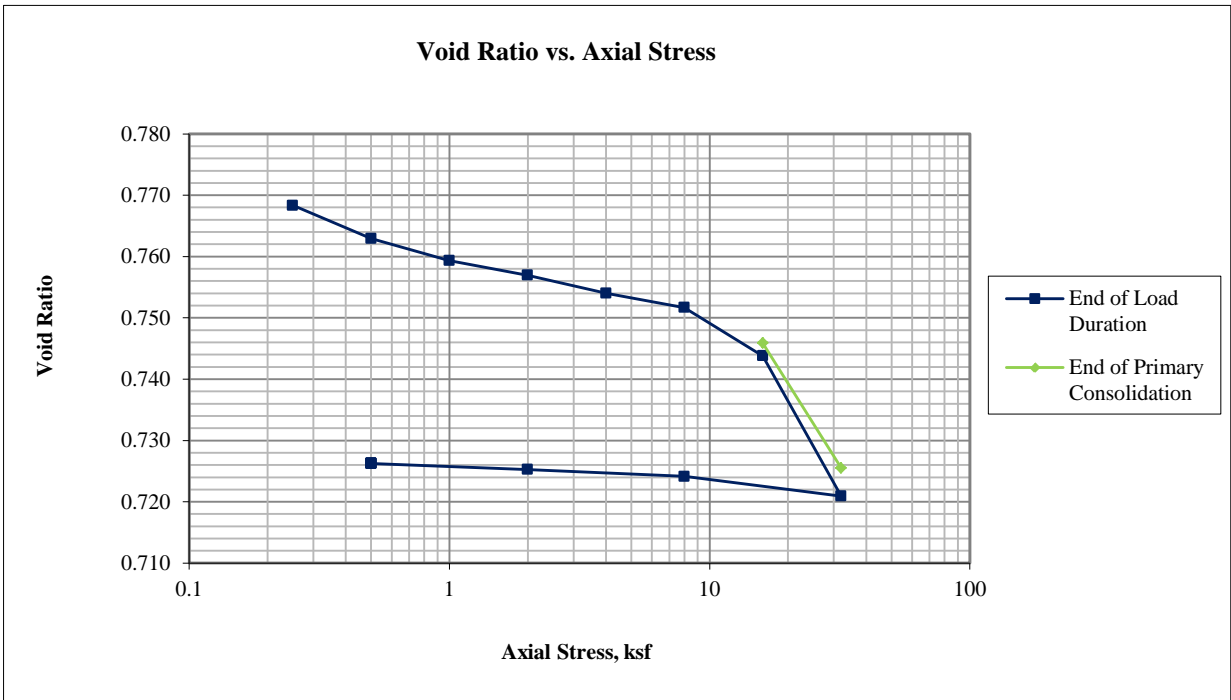
Figure:

3

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	1.000 in		0.973 in	USCS description (ASTM D2487):	Clayey sand, reddish brown, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 37	PL = 17	PI = 20
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 39%
Volume =	4.901 in <sup>3</sup>		4.769 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	9.8%		4.2%	Remold Targets:	93.0 pcf (dry) at	10.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	9.9%		
Height of Solids =	0.5558 in		0.5558 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.799		0.751	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	33.0%		15.1%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.291 lb		0.276 lb	Apparatus:	Frame No. 6	(ELE C-320A)	
Dry Mass =	0.265 lb		0.265 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	102.7 pcf		100.1 pcf	Final Differential Height:	-0.0136 in		
Dry Unit Weight =	93.5 pcf		96.1 pcf	Estimated Preconsolidation Stress:	13.8 ksf		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

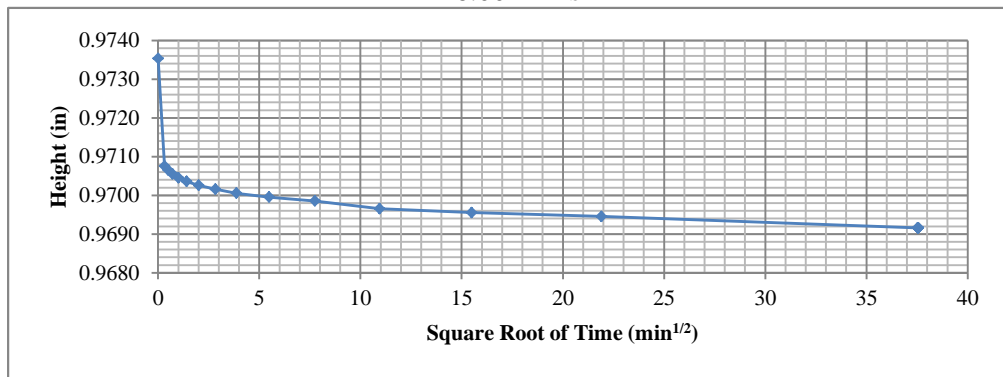
<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 22-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1



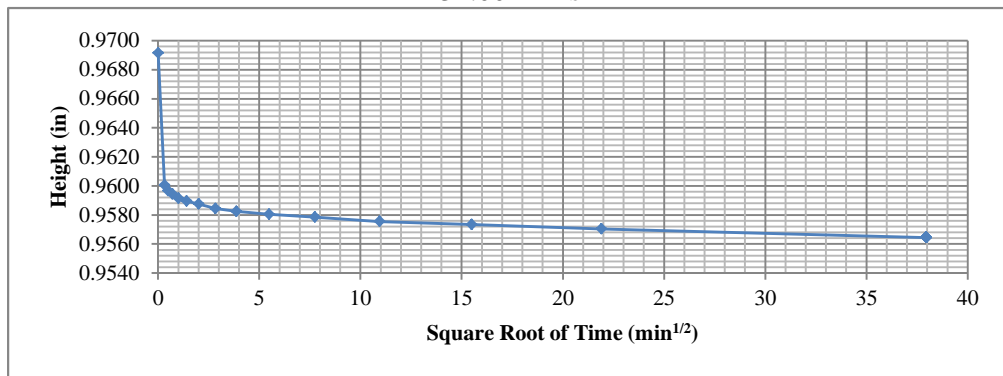
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample:</b> <b>BH-25 @ 22-34 ft</b>	<b>Technician:</b> <b>RJM</b>	<b>Reviewed:</b> <b>CCS</b>	<b>Start Date:</b> <b>3/25/2013</b>	<b>Job Number:</b> <b>103-92557.006</b>	<b>Figure:</b> <b>2</b>



16.00 ksf



32.00 ksf



<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>						
Sample: <b>BH-25 @ 22-34 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>	

**APPENDIX B  
TAILINGS TEST RESULTS**

**APPENDIX B.1  
CYCLONE TEST RESULTS,  
FULL SCALE CYCLONE PERFORMANCE SIMULATION**

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1222 STPH; 55.5% -200 mesh

U/F = 18.2% -200 mesh; 45.2% recovery

Number, Model Krebs Cyclones: 15 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 13-14 PSI

Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1222.00	669.86	552.15
STPH Liquids	2977.31	2740.68	236.63
STPH Slurry	4199.31	3410.53	788.78
Wt Solids	29.10	19.64	70.00
S.G. Slurry	1.221	1.139	1.773
Vol% Solids	13.41	8.44	46.82
GPM Slurry	13734.13	11956.71	1777.42
M3/Hr. Slurry	3119.34	2715.64	403.69

Ref: 72.3 4.5 38.9\*

Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	22.0	0.00	0.00	0.0	3.98	3.98	22.0	100.0
100	149.0	18.50	16.70	204.1	0.23	0.23	1.6	40.66	36.68	202.5	99.2
150	104.0	30.60	12.10	147.9	2.69	2.46	16.5	64.46	23.79	131.4	88.9
200	74.0	44.50	13.90	169.9	13.74	11.05	74.0	81.81	17.36	95.8	56.4
270	53.0	56.40	11.90	145.4	29.20	15.46	103.5	89.40	7.59	41.9	28.8
325	45.0	61.10	4.70	57.4	35.93	6.73	45.0	91.64	2.24	12.4	21.6
400	37.0	63.70	2.60	31.8	39.89	3.97	26.6	92.58	0.94	5.2	16.4
-400	-37.0	100.00	36.30	443.6	100.00	60.11	402.6	100.00	7.42	40.9	9.2
<b>TOTAL</b>				<b>1222.00</b>			<b>669.86</b>			<b>552.15</b>	<b>45.2</b>

Client: Golder Assoc. Copper Flat

Problem: Feed = 29.1% solids; 1333 STPH; 55.5% -200 mesh

U/F = 18.4% -200 mesh; 45.6% recovery

Number, Model Krebs Cyclones: 16 operating gMAX15U-20

Orifices: Inlet Area 18.00 sq. in. Vortex Finder 6.75 in. Apex TBD Pressure Drop 14 PSI  
 Specific Gravity: Solids: 2.650 Liquid: 1.000 Temperature: Amb. °F Viscosity: 1 Cps

	FEED	OVERFLOW	UNDERFLOW
STPH Solids	1333.00	724.53	608.47
STPH Liquids	3247.76	2986.98	260.77
STPH Slurry	4580.76	3711.51	869.24
Wt Solids	29.10	19.52	70.00
S.G. Slurry	1.221	1.138	1.773
Vol% Solids	13.41	8.39	46.82
GPM Slurry	14981.66	13022.93	1958.73
M3/Hr. Slurry	3402.68	2957.81	444.87

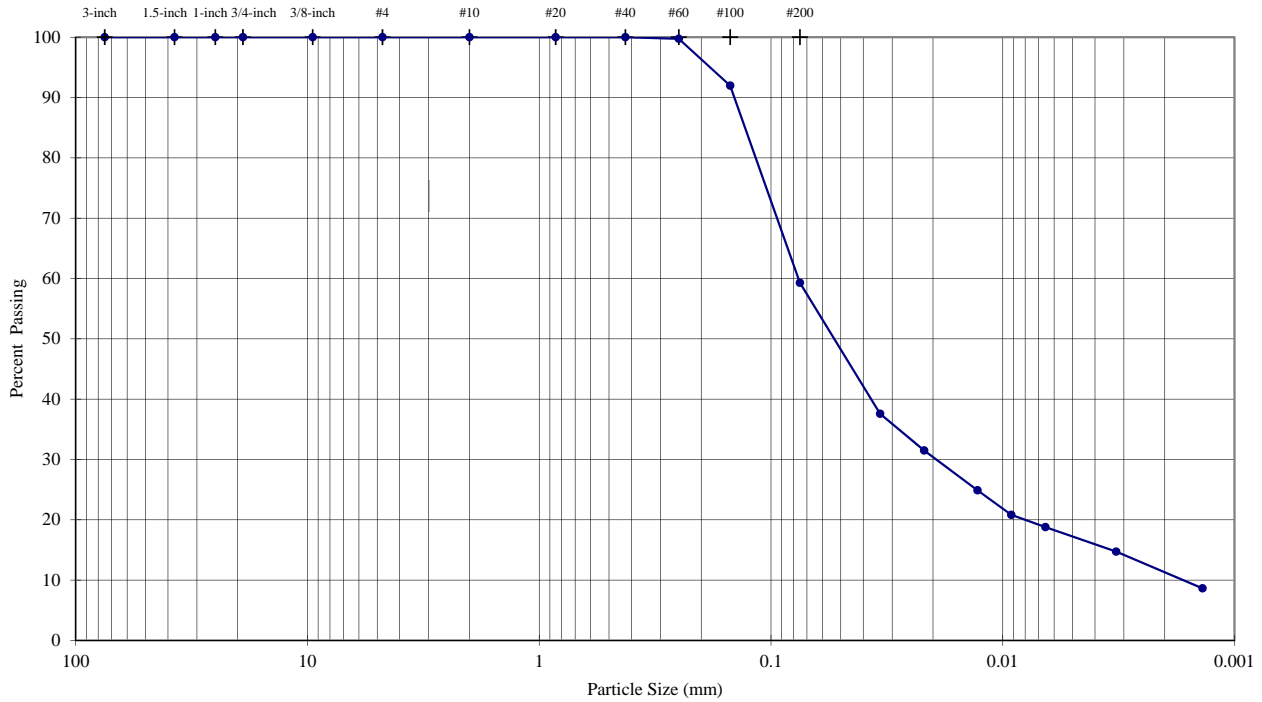
Ref: 71.4 4.5 38.9\*

Mesh	Micron	FEED			OVERFLOW			UNDERFLOW			ACT. REC.
		Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	Cum. % +	Ind. % +	STPH	
65	208.0	1.80	1.80	24.0	0.00	0.00	0.0	3.94	3.94	24.0	100.0
100	149.0	18.50	16.70	222.6	0.21	0.21	1.5	40.28	36.34	221.1	99.3
150	104.0	30.60	12.10	161.3	2.52	2.31	16.7	64.04	23.76	144.6	89.6
200	74.0	44.50	13.90	185.3	13.31	10.79	78.2	81.64	17.60	107.1	57.8
270	53.0	56.40	11.90	158.6	28.73	15.42	111.7	89.35	7.71	46.9	29.6
325	45.0	61.10	4.70	62.7	35.47	6.74	48.8	91.62	2.27	13.8	22.1
400	37.0	63.70	2.60	34.7	39.45	3.98	28.9	92.58	0.95	5.8	16.7
-400	-37.0	100.00	36.30	483.9	100.00	60.55	438.7	100.00	7.42	45.2	9.3
<b>TOTAL</b>				<b>1333.00</b>			<b>724.53</b>			<b>608.47</b>	<b>45.6</b>

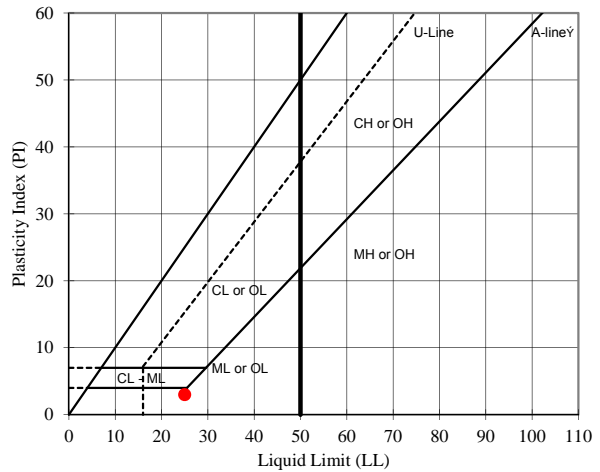
**APPENDIX B.2  
TAILINGS GRADATIONS**

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Drum** DEPTH (ft): **--**  
 TYPE: **Drum**



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	Fine Gravel
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Coarse Sand
		#10	2.0	100.0	
		#20	0.85	100.0	Medium Sand
		#40	0.425	100.0	
		#60	0.25	99.7	Fine Sand
	#100	0.15	92.0		
	#200	0.075	59.3		
	0.034	0.034	37.6		
Hydrometer Analysis		0.022	0.022	31.5	Silt or Clay Fines
		0.013	0.013	24.9	
		0.009	0.009	20.8	
		0.007	0.007	18.8	
		0.003	0.003	14.7	
	0.001	0.001	8.6		



USCS Description (ASTM D 2487):

Dry, yellow sandy silt

LL	PL	PI	SpG
25	22	3	2.64

As-Received Moisture Content (%)

--

USCS Group Symbol

ML

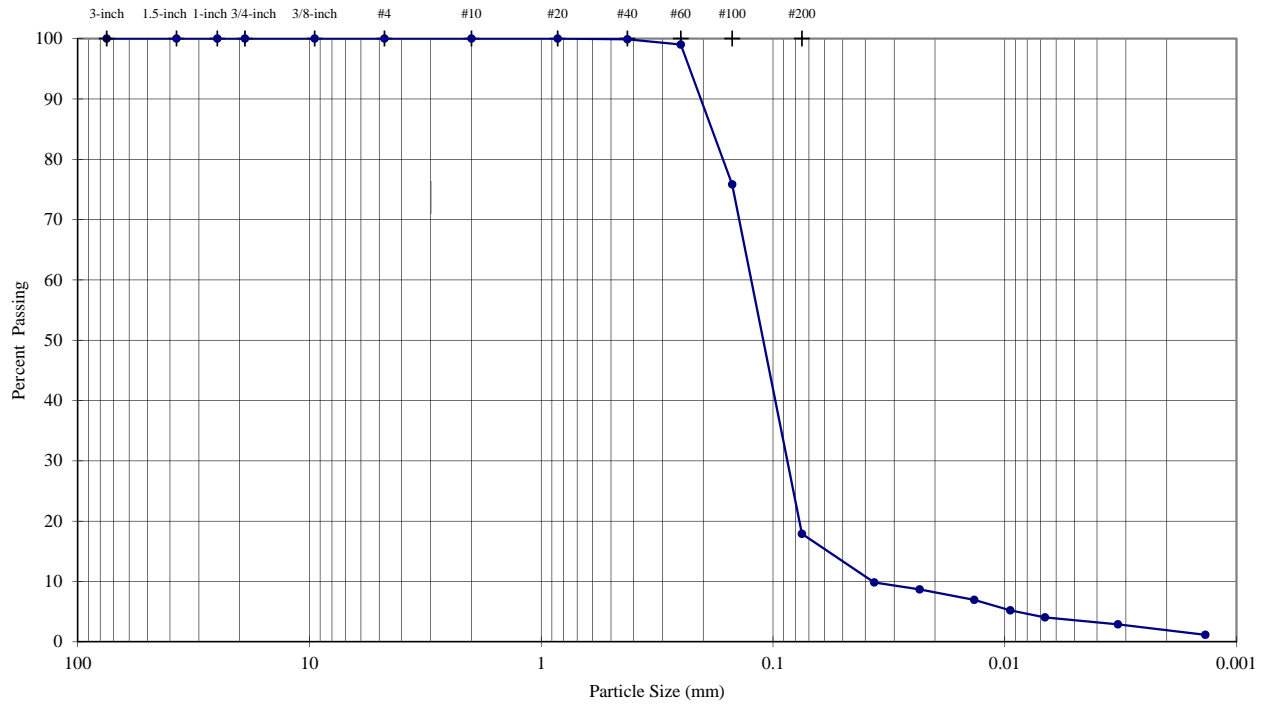
Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	11/13/2012
REVIEW	MB

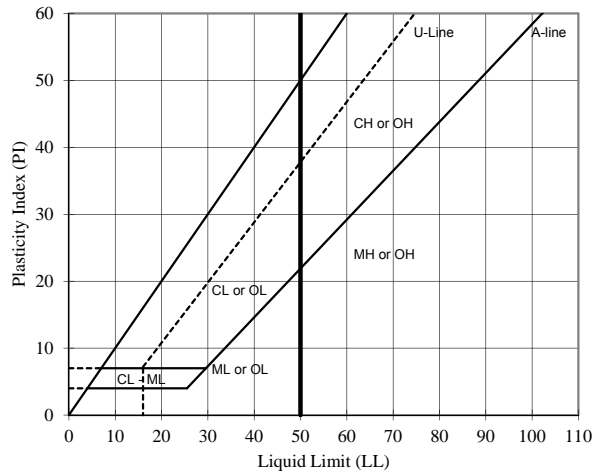
### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Underflow**  
 TYPE: **Pail**

DEPTH (ft): --



Sieve	Particle Size		Description	Percentage
	(mm)	% Passing		
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0		
3/8-inch	9.5	100.0	Fine Gravel	0.00
#4	4.75	100.0	Coarse Sand	0.00
#10	2.0	100.0		
#20	0.85	100.0		
#40	0.425	99.9	Medium Sand	0.12
#60	0.25	99.0	Fine Sand	81.98
#100	0.15	75.8		
#200	0.075	17.9	Silt or Clay Fines	17.91
Hydrometer	0.037	9.8		
Hydrometer	0.023	8.7		
Hydrometer	0.014	6.9		
Hydrometer	0.009	5.2		
Hydrometer	0.007	4.1		
Hydrometer	0.003	2.9		
Hydrometer	0.001	1.2		



USCS Description (ASTM D 2487):

Wet, light, yellowish brown silty sand

LL	PL	PI	Spg (assumed)
NP	NP	NP	2.7

As-Received Moisture Content (%)  
--

USCS Group Symbol  
SM

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

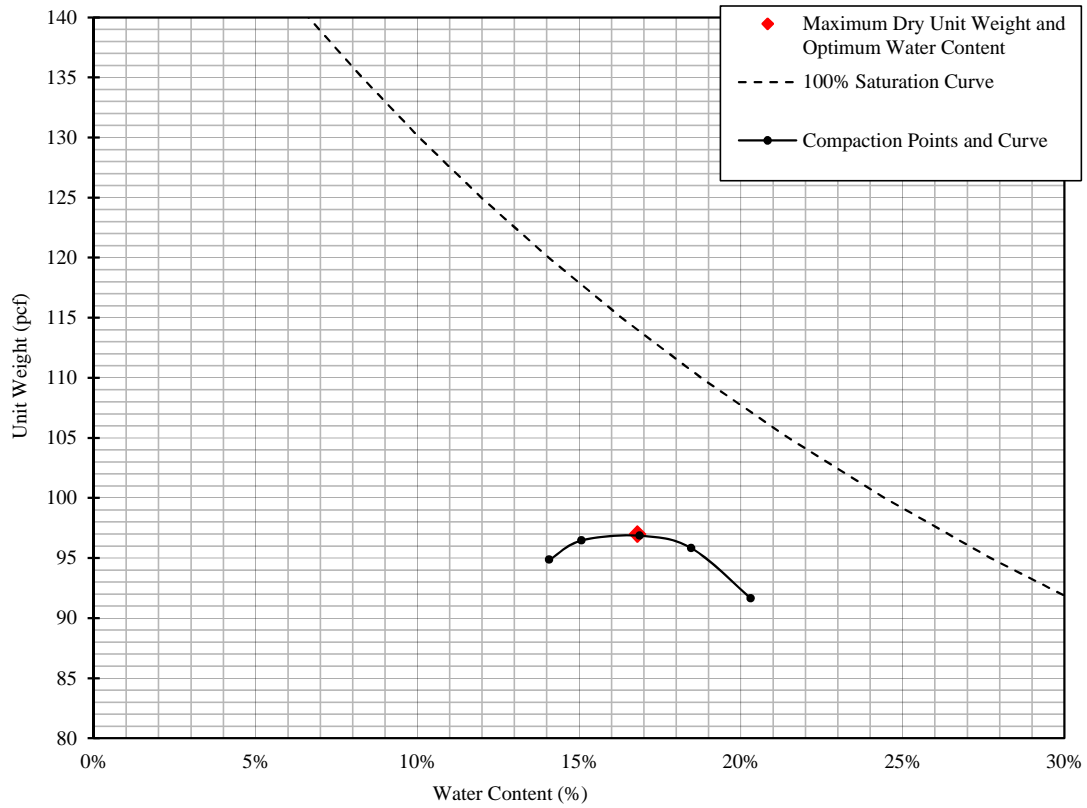
TECH	MC/SRS
DATE	10/24/2012
REVIEW	MB



## LABORATORY COMPACTION CHARACTERISTICS OF SOIL ASTM D698 - Method A

Manual Rammer    Moist Preparation

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Underflow**                      DEPTH (ft): --  
 TYPE: **Pail**



% Test Fraction Passing #4 Sieve	100%
As-Received Moisture Content	NA
Specific Gravity (ASTM D854)	2.64

Maximum Dry Unit Weight (pcf)	97.0
Optimum Water Content (%)	16.8

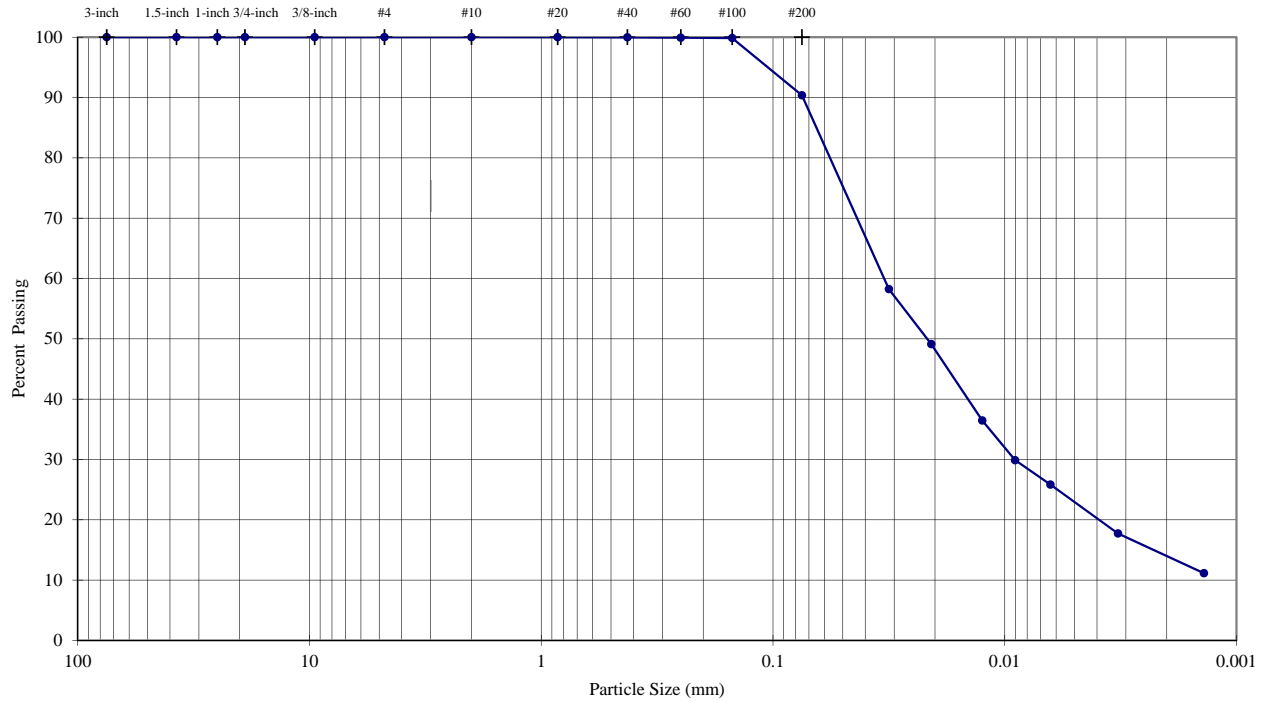
USCS Description (ASTM D 2487): Wet, light, yellowish brown silty sand

USCS: SM

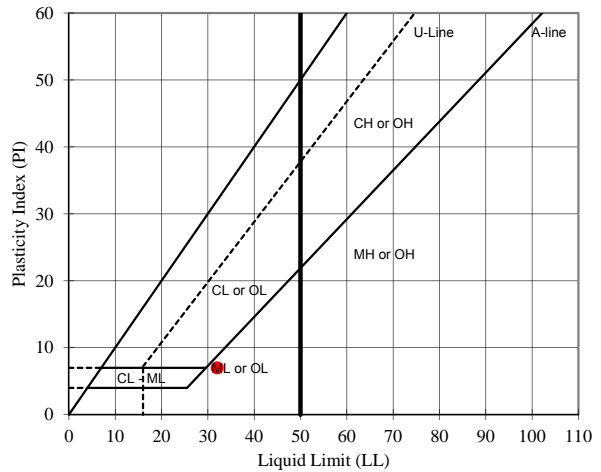
TECH	ACE/MC
DATE	10-25-12
REVIEW	MB

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **--**  
 TYPE: **Drum**



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Fine Gravel
		#10	2.0	100.0	
		#20	0.85	100.0	Medium Sand
		#40	0.425	100.0	
		#60	0.25	99.9	Fine Sand
		#100	0.15	99.9	
		#200	0.075	90.4	
			0.032	58.2	
Hydrometer Analysis			0.021	49.1	Silt or Clay Fines
			0.013	36.5	
			0.009	29.9	
			0.006	25.8	
			0.003	17.7	
			0.001	11.1	



USCS Description (ASTM D 2487):  
Dry, olive yellow SILT

LL	PL	PI	SpG (assumed)
32	25	7	2.64

As-Received Moisture Content (%)  
--

USCS Group Symbol  
ML

Notes: 0 g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH	AM/SRS
DATE	1/2/2013
REVIEW	MB

**APPENDIX B.3  
CYCLONE UNDERFLOW TEST RESULTS**

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: Tailings Underflow  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.526 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.311 lb  
 Moisture Content = 16.4%  
 Wet Unit Weight = 107.3 pcf  
 Dry Unit Weight = 92.2 pcf  
 Void Ratio = 0.79  
 Percent Saturation = 55%

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.523 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.312 lb  
 Moisture Content = 16.1%  
 Wet Unit Weight = 107.1 pcf  
 Dry Unit Weight = 92.3 pcf  
 Void Ratio = 0.78  
 Percent Saturation = 54%

**Initial**  
 Length = 5.006 in  
 Diameter = 2.500 in  
 Wet Mass = 1.517 lb  
 Area = 4.909 in<sup>2</sup>  
 Volume = 24.573 in<sup>3</sup>  
 Specific Gravity = 2.64 (Assumed)  
 Dry Mass of Solids = 1.306 lb  
 Moisture Content = 16.1%  
 Wet Unit Weight = 106.7 pcf  
 Dry Unit Weight = 91.9 pcf  
 Void Ratio = 0.79  
 Percent Saturation = 54%

**After Consolidation**  
 Length = 4.989 in  
 Diameter = 2.470 in  
 Area = 4.792 in<sup>2</sup> (Method B)  
 Volume = 23.906 in<sup>3</sup>  
 Moisture Content = 27.9%  
 Wet Unit Weight = 121.2 pcf  
 Dry Unit Weight = 94.7 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.977 in  
 Diameter = 2.469 in  
 Area = 4.787 in<sup>2</sup> (Method B)  
 Volume = 23.824 in<sup>3</sup>  
 Moisture Content = 27.6%  
 Wet Unit Weight = 121.4 pcf  
 Dry Unit Weight = 95.2 pcf  
 Void Ratio = 0.73  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.971 in  
 Diameter = 2.429 in  
 Area = 4.633 in<sup>2</sup> (Method B)  
 Volume = 23.033 in<sup>3</sup>  
 Moisture Content = 25.7%  
 Wet Unit Weight = 123.2 pcf  
 Dry Unit Weight = 98.0 pcf  
 Void Ratio = 0.68  
 Percent Saturation = 100%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.97  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

B Parameter = 0.96  
 Shear Rate = 0.082% /min.  
 t<sub>50</sub> = -- (not computed)  
 Strain at Failure = 5.0%

Cell Pressure = 120 psi  
 Back Pressure = 100 psi  
 Confining Pressure = 20 psi

Cell Pressure = 150 psi  
 Back Pressure = 100 psi  
 Confining Pressure = 50 psi

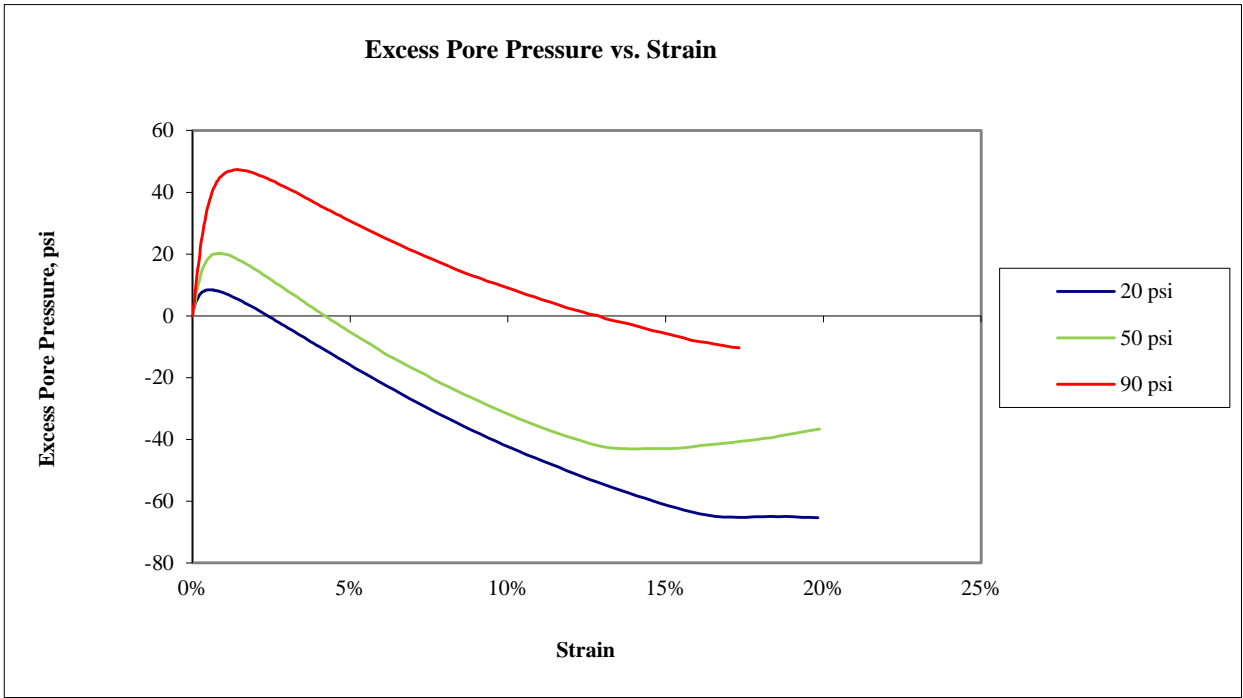
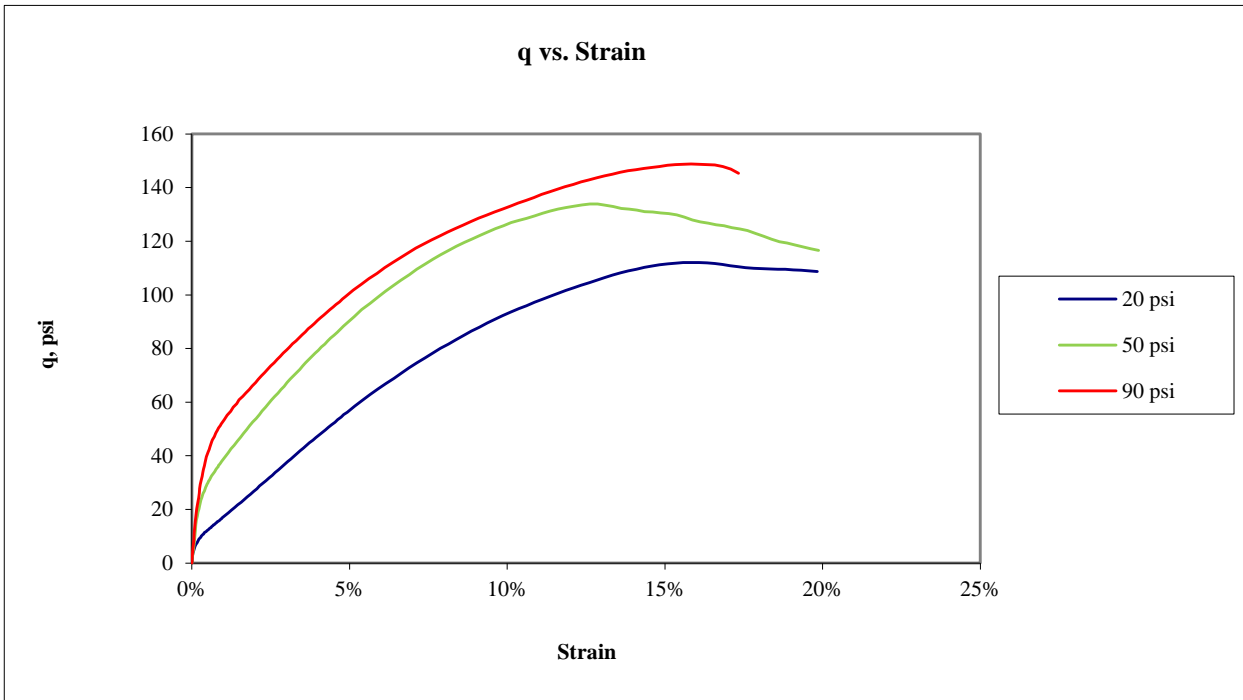
Cell Pressure = 170 psi  
 Back Pressure = 80 psi  
 Confining Pressure = 90 psi

Notes: Sample description: Light yellowish brown silty sand  
 Atterberg limits: LL = NP PL = NP PI = NP (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 18% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type: 

Intact	X
Cuttings	X
Wet	
(σ <sub>1</sub> /σ <sub>3</sub> ) <sub>max</sub>	
Corrected	

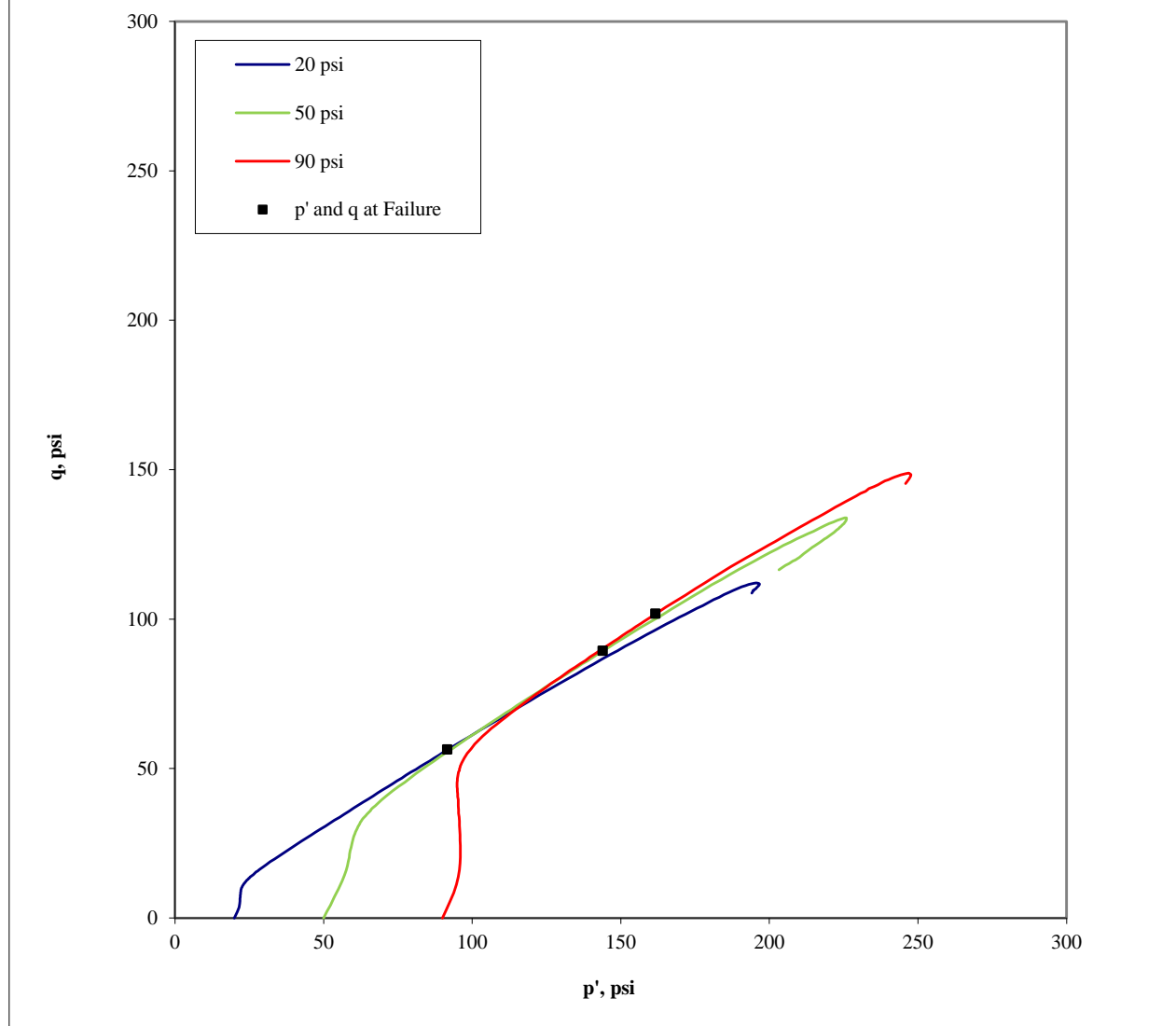
 Reconstituted Remold targets: 92.2 pcf (dry) at 16.8% moisture (+/- 2%)  
 Moisture from: Entire specimen  
 Saturation method: X Wet Dry  
 Failure criterion: (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub> 5 % strain  
 Membrane effect: X Corrected Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT q AND EXCESS PORE PRESSURE PLOTS			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 2

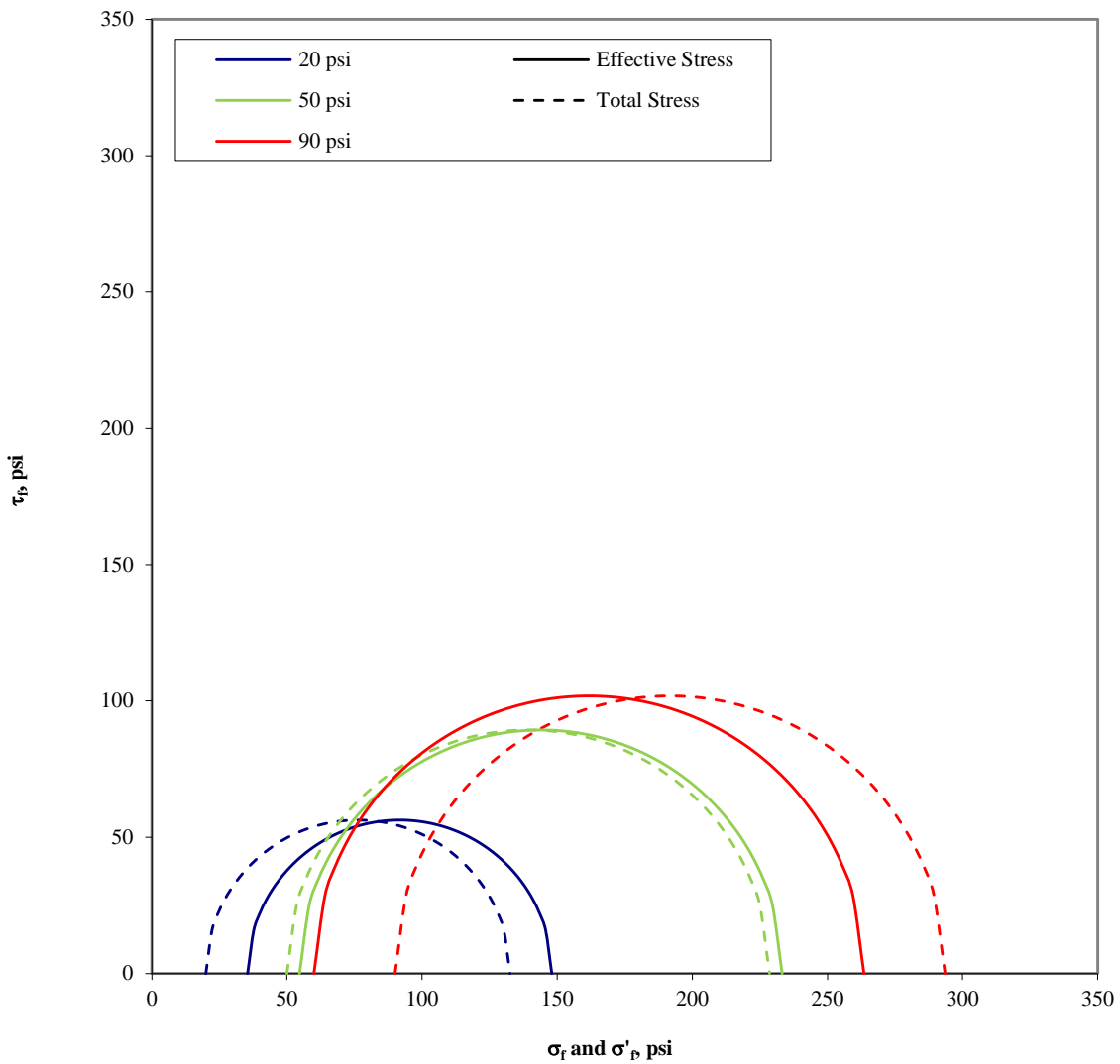
Stress Path (p'-q) Plot



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	76.3	91.7	56.3
50	139.3	143.9	89.3
90	191.8	161.7	101.8

<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title: <b>ASTM D4767</b>  <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b>  <b>STRESS PATH PLOT</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>Tailing Underflow</b></p>	<p>Technician:  <b>RJM/PRH</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Date:  <b>1/8/2013</b></p>	<p>Job Number:  <b>103-92557</b></p>	<p>Figure:  <b>3</b></p>

### Mohr's Circle Diagram



Confining Pressure (psi)	$\sigma'_1$ at failure (psi)	$\sigma'_3$ at failure (psi)	$\sigma_1$ at failure (psi)	$\sigma_3$ at failure (psi)
20	148.0	35.4	132.6	20.0
50	233.2	54.7	228.6	50.0
90	263.5	60.0	293.5	90.0

<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM</b>			
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>Tailing Underflow</b>	Technician: <b>RJM/PRH</b>	Reviewed: <b>CCS</b>	Date: <b>1/8/2013</b>	Job Number: <b>103-92557</b>	Figure: <b>4</b>



<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 5





<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013	<b>Job Number:</b> 103-92557	<b>Figure:</b> 6



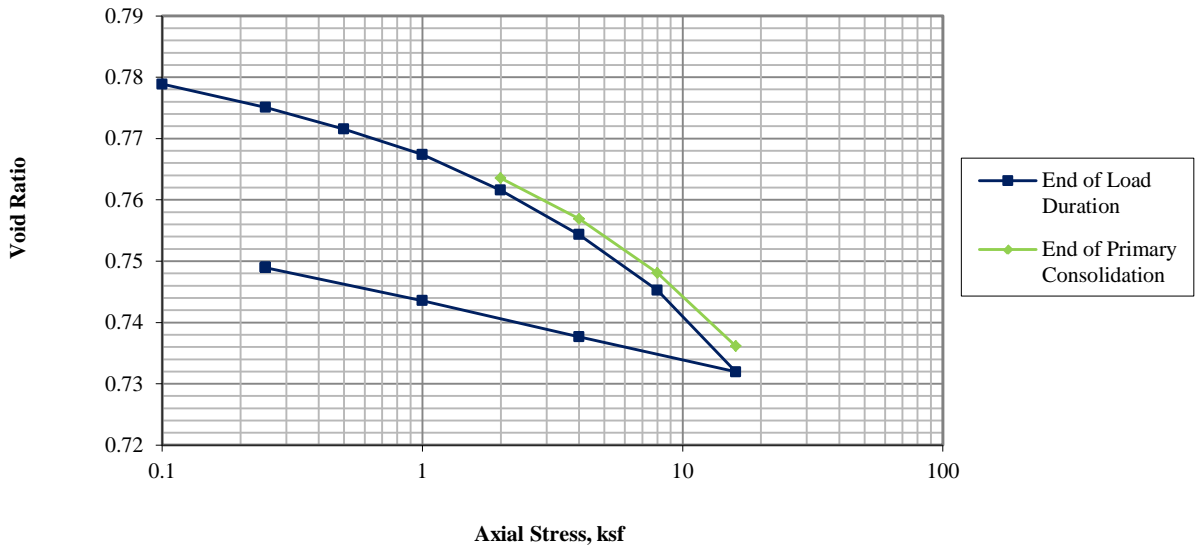
<b>Golder Associates Inc. Denver, Colorado</b>		<b>Title:</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study		ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
		<b>Sample:</b> Tailing Underflow	<b>Technician:</b> RJM/PRH	<b>Reviewed:</b> CCS	<b>Date:</b> 1/8/2013
					<b>Figure:</b> 7

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>	
Height =	0.994 in		0.973 in	Visual description (Golder procedure):	Damp, yellowish brown SILTY SAND
Diameter =	2.501 in		2.501 in	Atterberg Limits (ASTM D4318):	LL = NP PL = NP PI = NP
Area =	4.913 in <sup>2</sup>		4.913 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100% No. 4 = 100% No. 200 = 18%
Volume =	4.883 in <sup>3</sup>		4.780 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact <input checked="" type="checkbox"/> Reconstituted
Water Content =	16.5%		25.0%	Remold Targets:	92.2 pcf (dry) at 16.8% moisture (+/- 2.0%)
Specific Gravity =	2.64 (Assumed)		2.64 (Assumed)	Water Content of Trimmings (ASTM D2216):	16.6%
Height of Solids =	0.5584 in		0.5584 in	Trimming Procedure:	Specimen trimmed in ring
Void Ratio =	0.780		0.743	Inundation:	<input type="checkbox"/> Not inundated <input checked="" type="checkbox"/> Inundated at 0.1 ksf
Degree of Saturation =	55.8%		88.9%	Test Method:	<input type="checkbox"/> A <input checked="" type="checkbox"/> B
Wet Mass =	0.304 lb		0.326 lb	Apparatus:	Frame No. 2 (Wykeham Farrance 24251)
Dry Mass =	0.261 lb		0.261 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire <input type="checkbox"/> Partial
Wet Unit Weight =	107.6 pcf		118.0 pcf	Final Differential Height:	0.0036 in
Dry Unit Weight =	92.4 pcf		94.4 pcf	Estimated Preconsolidation Stress:	3.7 ksf

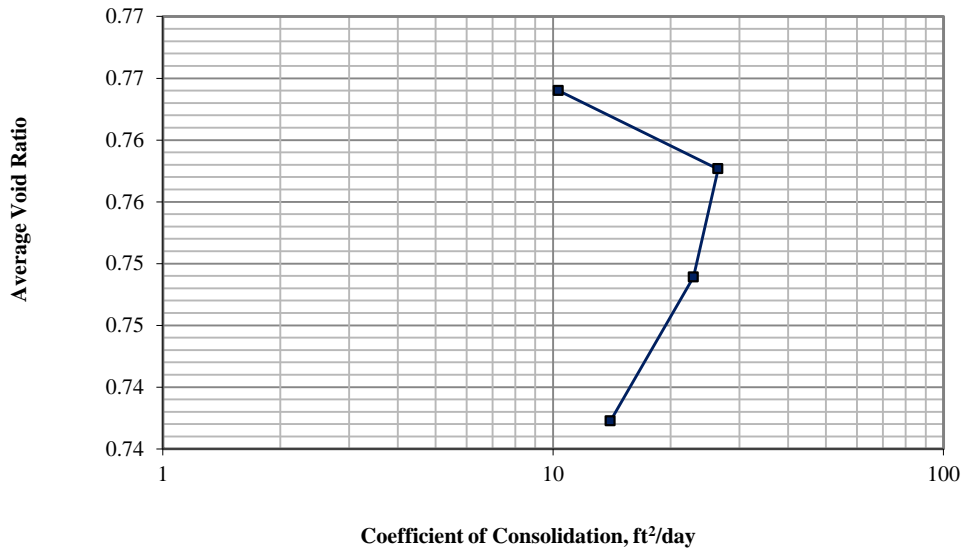
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	160					0.0000	0.9937	0.00	0.780				
1	0.10	1000					0.0004	0.9933	0.04	0.779				
2	0.25	1500					0.0025	0.9912	0.25	0.775				
3	0.50	1440					0.0045	0.9892	0.45	0.772				
4	1.00	1440					0.0068	0.9869	0.68	0.767				
5	2.00	1440	0.0090	0.9847	0.90	0.764	0.0101	0.9836	1.01	0.762	2 (Root time)	0.764	10.328	0.3
6	4.00	1440	0.0127	0.9810	1.27	0.757	0.0141	0.9796	1.42	0.754	2 (Root time)	0.758	26.471	0.3
7	8.00	1440	0.0176	0.9761	1.77	0.748	0.0192	0.9745	1.93	0.745	2 (Root time)	0.749	22.898	0.3
8	16.00	1440	0.0243	0.9694	2.44	0.736	0.0266	0.9671	2.68	0.732	2 (Root time)	0.737	14.013	0.4
9	4.00	150					0.0234	0.9703	2.35	0.738				
10	1.00	870					0.0201	0.9736	2.02	0.744				
11	0.25	1145					0.0171	0.9766	1.72	0.749				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>SPECIMEN AND SUMMARY DATA</b>			
<b>Job Short Title:</b>  Copper Flat Tailings Design Study					
<b>Sample:</b>  Tailings Underflow	<b>Technician:</b>  RJM	<b>Reviewed:</b>  CCS	<b>Start Date:</b>  11/12/2012	<b>Job Number:</b>  103-92557	<b>Figure:</b>  1

**Void Ratio vs. Axial Stress**



**Average Void Ratio vs. Coefficient of Consolidation**



**Golder Associates Inc.  
Denver, Colorado**

Title:

**ASTM D2435  
ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
CONSOLIDATION PLOTS**

Job Short Title:

**Copper Flat Tailings Design Study**

Sample:

**Tailings Underflow**

Technician:

**RJM**

Reviewed:

**CCS**

Start Date:

**11/12/2012**

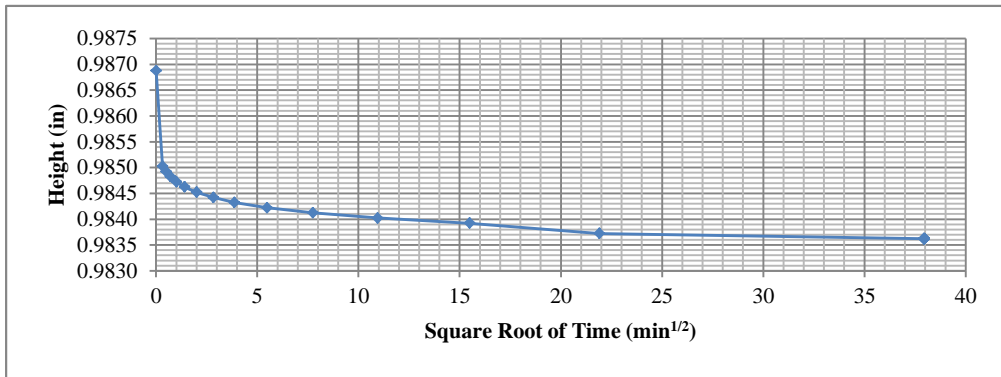
Job Number:

**103-92557**

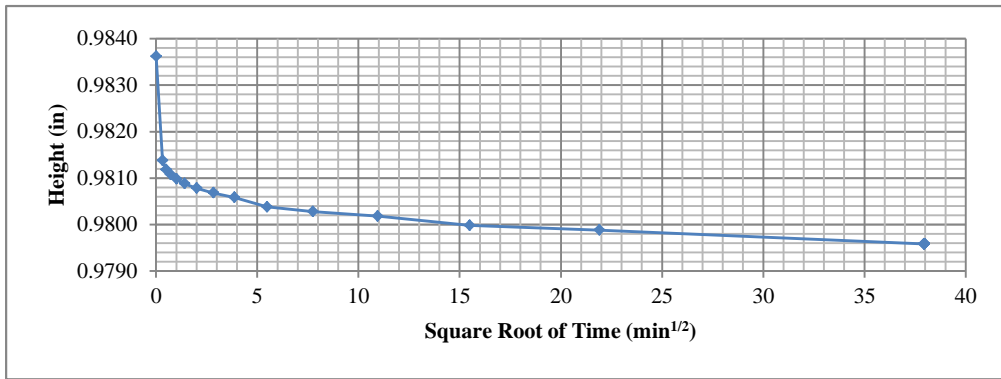
Figure:

**2**

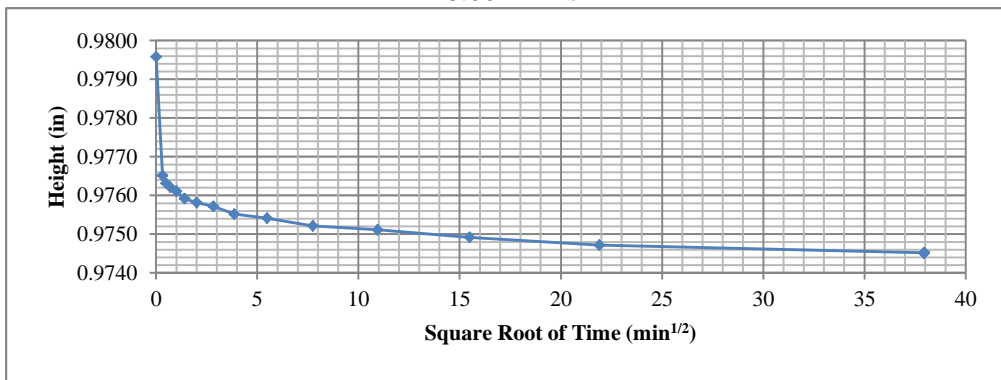
2.00 ksf



4.00 ksf



8.00 ksf



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 TIME-DEFORMATION PLOTS (1)

Job Short Title:

Copper Flat Tailings Design Study

Sample:

Tailings Underflow

Technician:

RJM

Reviewed:

CCS

Start Date:

11/12/2012

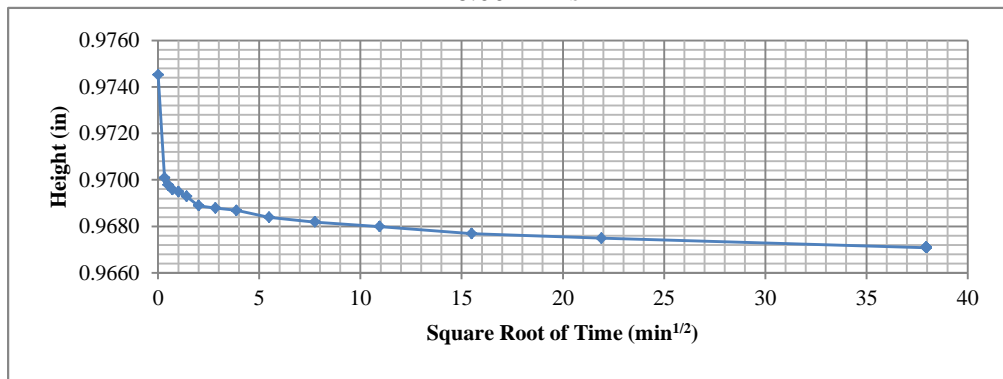
Job Number:

103-92557

Figure:

3

16.00 ksf



<b>Golder Associates Inc. Denver, Colorado</b>		Title: <b>ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS (2)</b>			
Job Short Title: Copper Flat Tailings Design Study					
Sample: Tailings Underflow	Technician: RJM	Reviewed: CCS	Start Date: 11/12/2012	Job Number: 103-92557	Figure: 4

**APPENDIX B.4  
CYCLONE OVERFLOW AND WHOLE TAILINGS  
TEST RESULTS**

**APPENDIX B.4.1  
CYCLONE OVERFLOW TEST REPORTS**



Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 1

Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 2

Boring or Test Pit: --  
 Sample: O/F Flume Beach  
 Depth: -- ft  
 Point No.: 3

**Initial**  
 Length = 4.183 in  
 Diameter = 1.924 in  
 Wet Mass = 0.851 lb  
 Area = 2.907 in<sup>2</sup>  
 Volume = 12.162 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.666 lb  
 Moisture Content = 27.7%  
 Wet Unit Weight = 120.9 pcf  
 Dry Unit Weight = 94.6 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 99%

**Initial**  
 Length = 4.127 in  
 Diameter = 1.930 in  
 Wet Mass = 0.832 lb  
 Area = 2.926 in<sup>2</sup>  
 Volume = 12.074 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.653 lb  
 Moisture Content = 27.4%  
 Wet Unit Weight = 119.1 pcf  
 Dry Unit Weight = 93.5 pcf  
 Void Ratio = 0.76  
 Percent Saturation = 95%

**Initial**  
 Length = 4.257 in  
 Diameter = 1.929 in  
 Wet Mass = 0.868 lb  
 Area = 2.922 in<sup>2</sup>  
 Volume = 12.441 in<sup>3</sup>  
 Specific Gravity = 2.64 (Provided)  
 Dry Mass of Solids = 0.682 lb  
 Moisture Content = 27.3%  
 Wet Unit Weight = 120.5 pcf  
 Dry Unit Weight = 94.7 pcf  
 Void Ratio = 0.74  
 Percent Saturation = 98%

**After Consolidation**  
 Length = 4.091 in  
 Diameter = 1.894 in  
 Area = 2.816 in<sup>2</sup> (Method B)  
 Volume = 11.522 in<sup>3</sup>  
 Moisture Content = 24.5%  
 Wet Unit Weight = 124.4 pcf  
 Dry Unit Weight = 99.9 pcf  
 Void Ratio = 0.65  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.080 in  
 Diameter = 1.866 in  
 Area = 2.734 in<sup>2</sup> (Method B)  
 Volume = 11.156 in<sup>3</sup>  
 Moisture Content = 23.7%  
 Wet Unit Weight = 125.2 pcf  
 Dry Unit Weight = 101.2 pcf  
 Void Ratio = 0.63  
 Percent Saturation = 100%

**After Consolidation**  
 Length = 4.224 in  
 Diameter = 1.849 in  
 Area = 2.685 in<sup>2</sup> (Method B)  
 Volume = 11.339 in<sup>3</sup>  
 Moisture Content = 22.1%  
 Wet Unit Weight = 126.9 pcf  
 Dry Unit Weight = 103.9 pcf  
 Void Ratio = 0.58  
 Percent Saturation = 100%

B Parameter = 0.95  
 Shear Rate = 0.051% /min.  
 t<sub>50</sub> = 7.8 min.  
 Strain at Failure = 5.0%

B Parameter = 0.99  
 Shear Rate = 0.084% /min.  
 t<sub>50</sub> = 3.7 min.  
 Strain at Failure = 5.0%

B Parameter = 0.98  
 Shear Rate = 0.083% /min.  
 t<sub>50</sub> = 4.4 min.  
 Strain at Failure = 5.0%

Cell Pressure = 60 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 20 psi

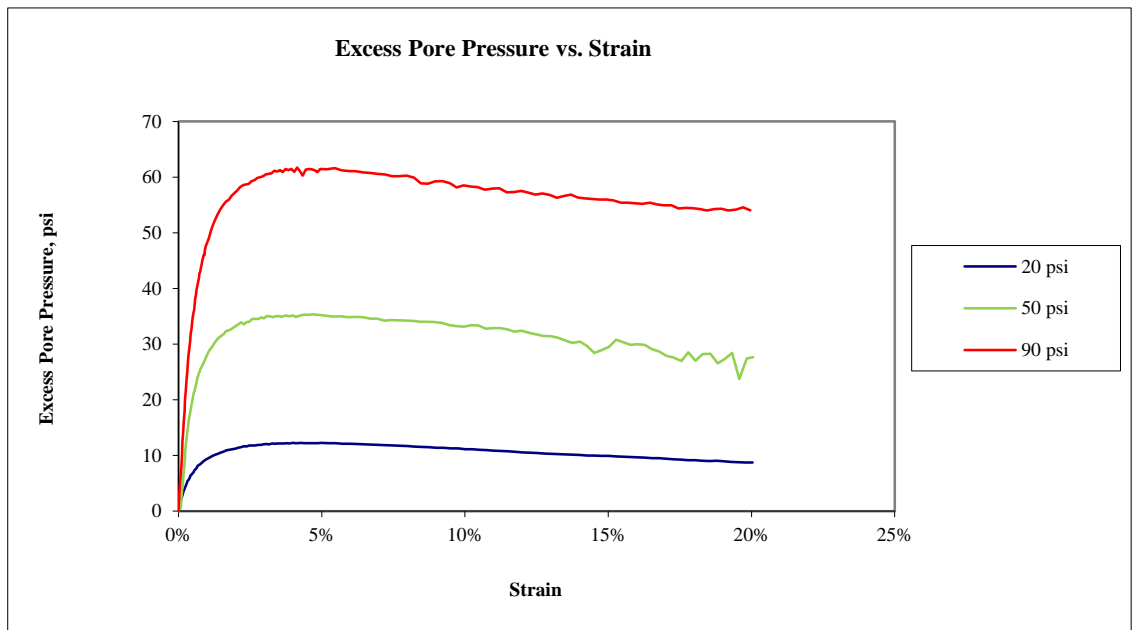
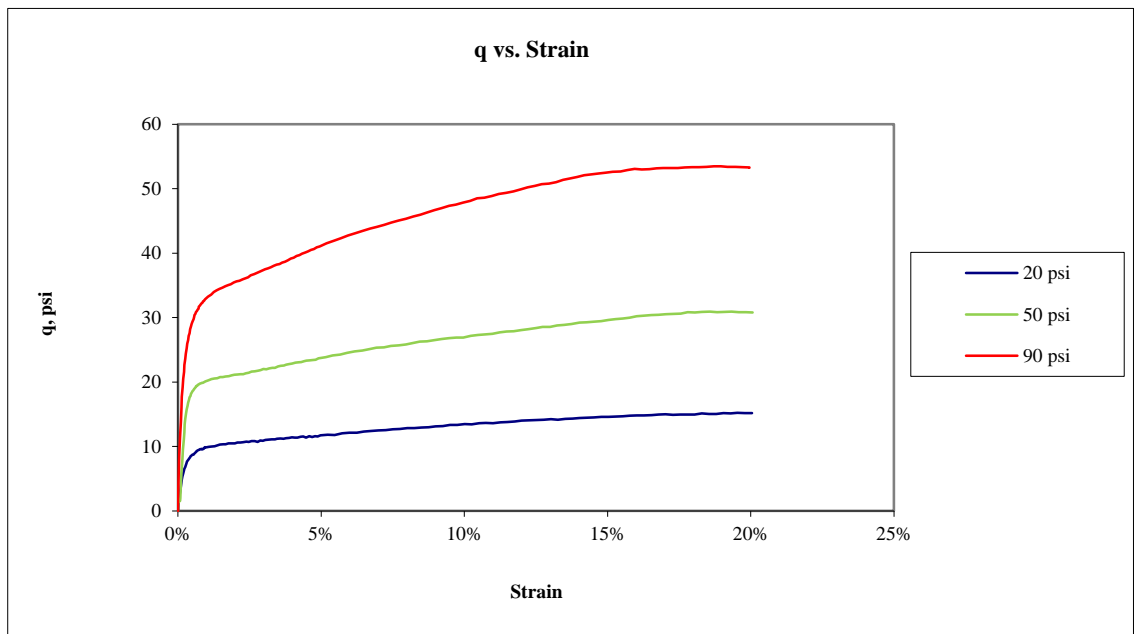
Cell Pressure = 90 psi  
 Back Pressure = 40 psi  
 Confining Pressure = 50 psi

Cell Pressure = 120 psi  
 Back Pressure = 30 psi  
 Confining Pressure = 90 psi

Notes: Sample description: Silt, pale yellow, moist

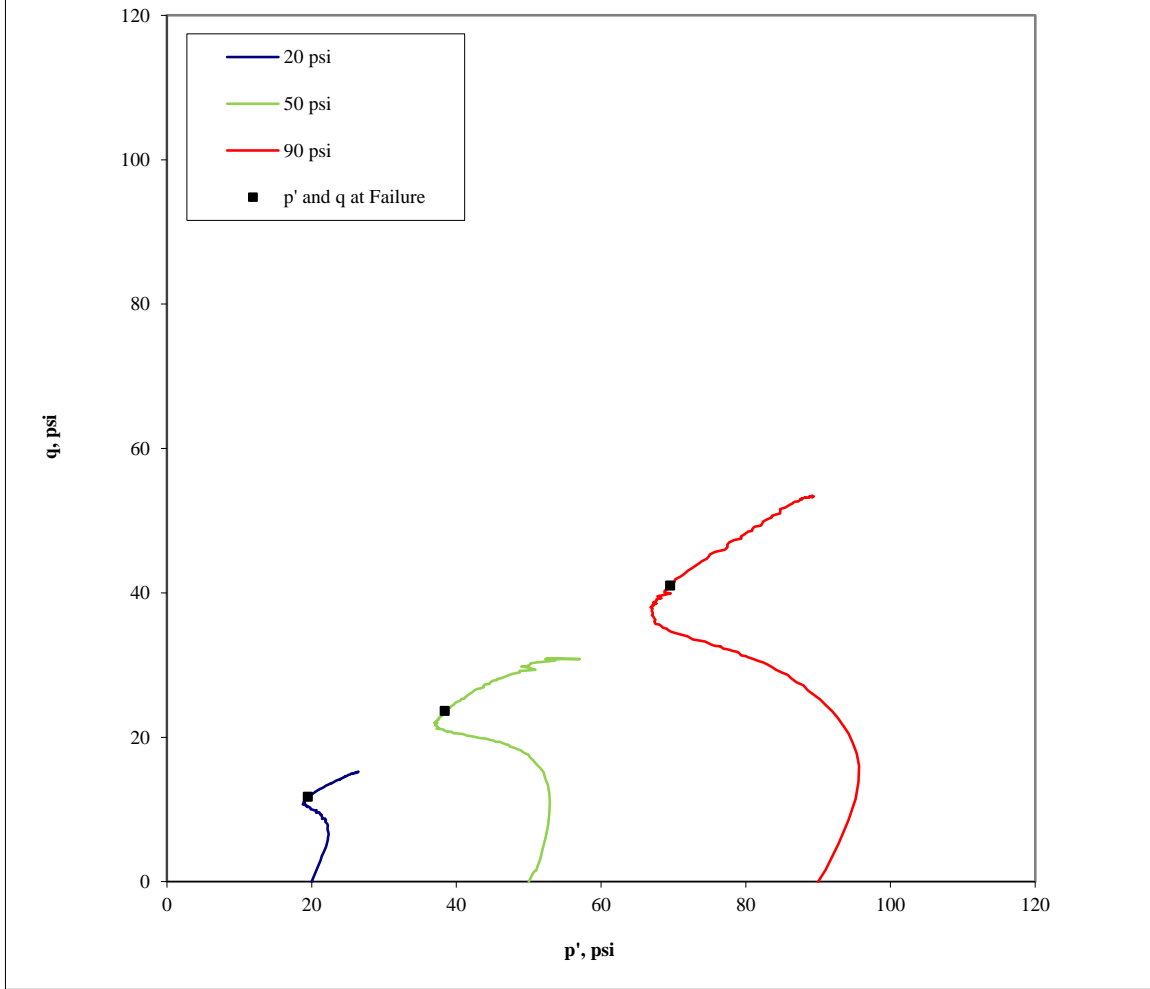
Atterberg limits: LL = 28 PL = 25 PI = 3 (ASTM D4318)  
 Percent finer: 3/4 in. = 100% No. 4 = 100% No. 200 = 84% (ASTM D422, refer to separate report for gradation curve)  
 Specimen type:  Intact  Reconstituted  Slurry consolidated in tube and extruded  
 Moisture from:  Cuttings  Entire specimen  
 Saturation method:  Wet  Dry  
 Failure criterion:  (σ<sub>1</sub>/σ<sub>3</sub>)<sub>max</sub>  (σ<sub>1</sub>-σ<sub>3</sub>)<sub>max</sub>  5 % strain  
 Membrane effect:  Corrected  Not Corrected

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>ASTM D4767</b> <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>SAMPLE AND TEST DATA</b>			
<b>Job Short Title:</b> <b>Copper Flat Tailngs Design Study</b>		<b>Technician:</b> <b>RJM</b>		<b>Reviewed:</b> <b>DAR</b>	<b>Date:</b> <b>3/12/2013</b>
<b>Sample:</b> <b>Overflow Flume Test Beach</b>		<b>Job Number:</b> <b>103-92557.010</b>		<b>Figure:</b> <b>1</b>	



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D4767 <b>CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT</b> <b>q AND EXCESS PORE PRESSURE PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 2

**Stress Path (p'-q) Plot**



Confining Pressure (psi)	p at failure (psi)	p' at failure (psi)	q at failure (psi)
20	31.7	19.5	11.7
50	73.7	38.4	23.7
90	131.0	69.6	41.0

**Golder Associates Inc.  
Denver, Colorado**

**Title:**

**ASTM D4767  
CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT  
STRESS PATH PLOT**

**Job Short Title:  
Copper Flat Tailings Design Study**

**Sample:  
Overflow Flume Test Beach**

**Technician:  
RJM**

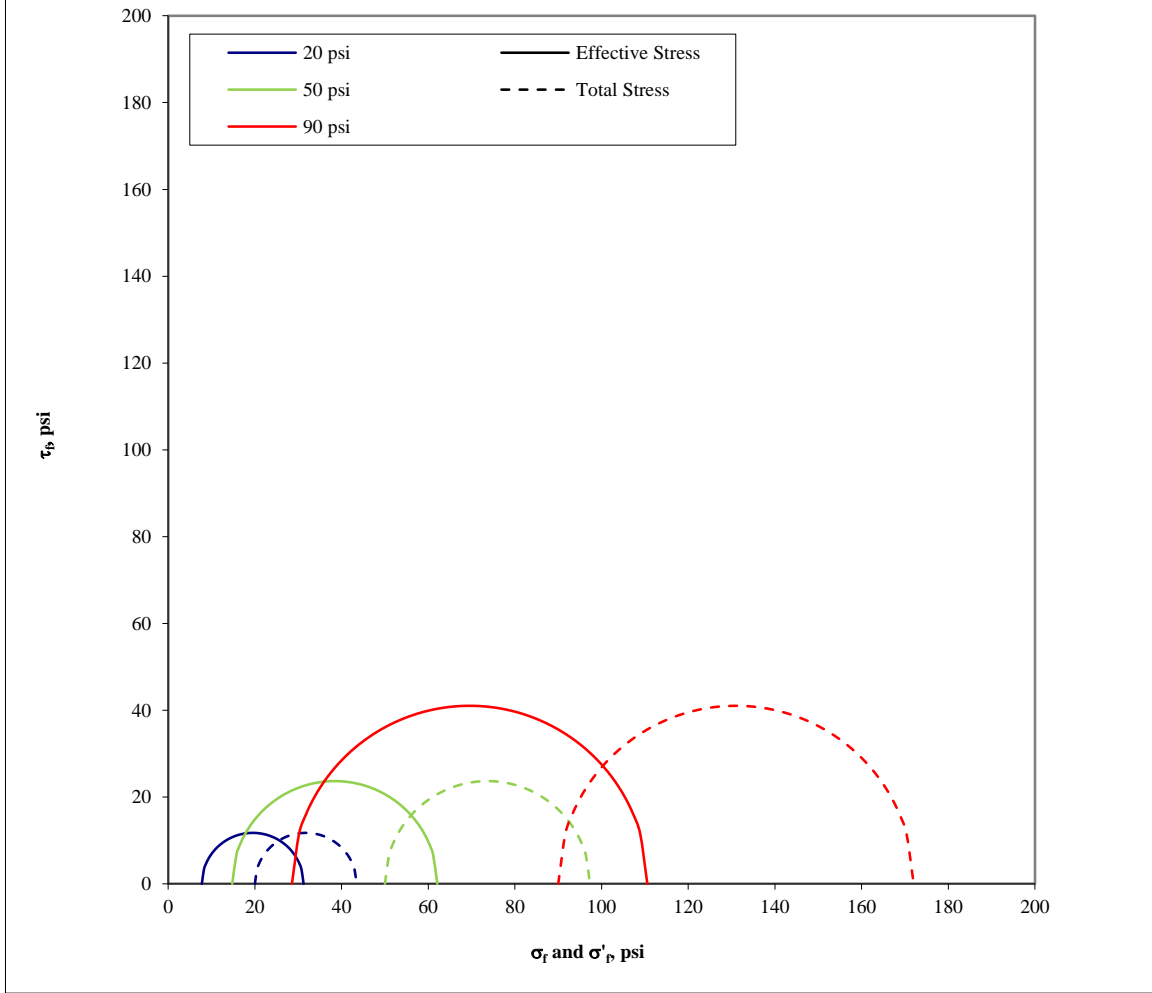
**Reviewed:  
DAR**

**Date:  
3/12/2013**

**Job Number:  
103-92557.010**

**Figure:  
3**

### Mohr's Circle Diagram



Confining Pressure (psi)	σ <sub>1</sub> at failure (psi)	σ <sub>3</sub> at failure (psi)	σ <sub>1</sub> at failure (psi)	σ <sub>3</sub> at failure (psi)
20	31.2	7.8	43.5	20.0
50	62.1	14.8	97.3	50.0
90	110.6	28.5	172.0	90.0

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT MOHR'S CIRCLE DIAGRAM				
		<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample:</b> Overflow Flume Test Beach		<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 4



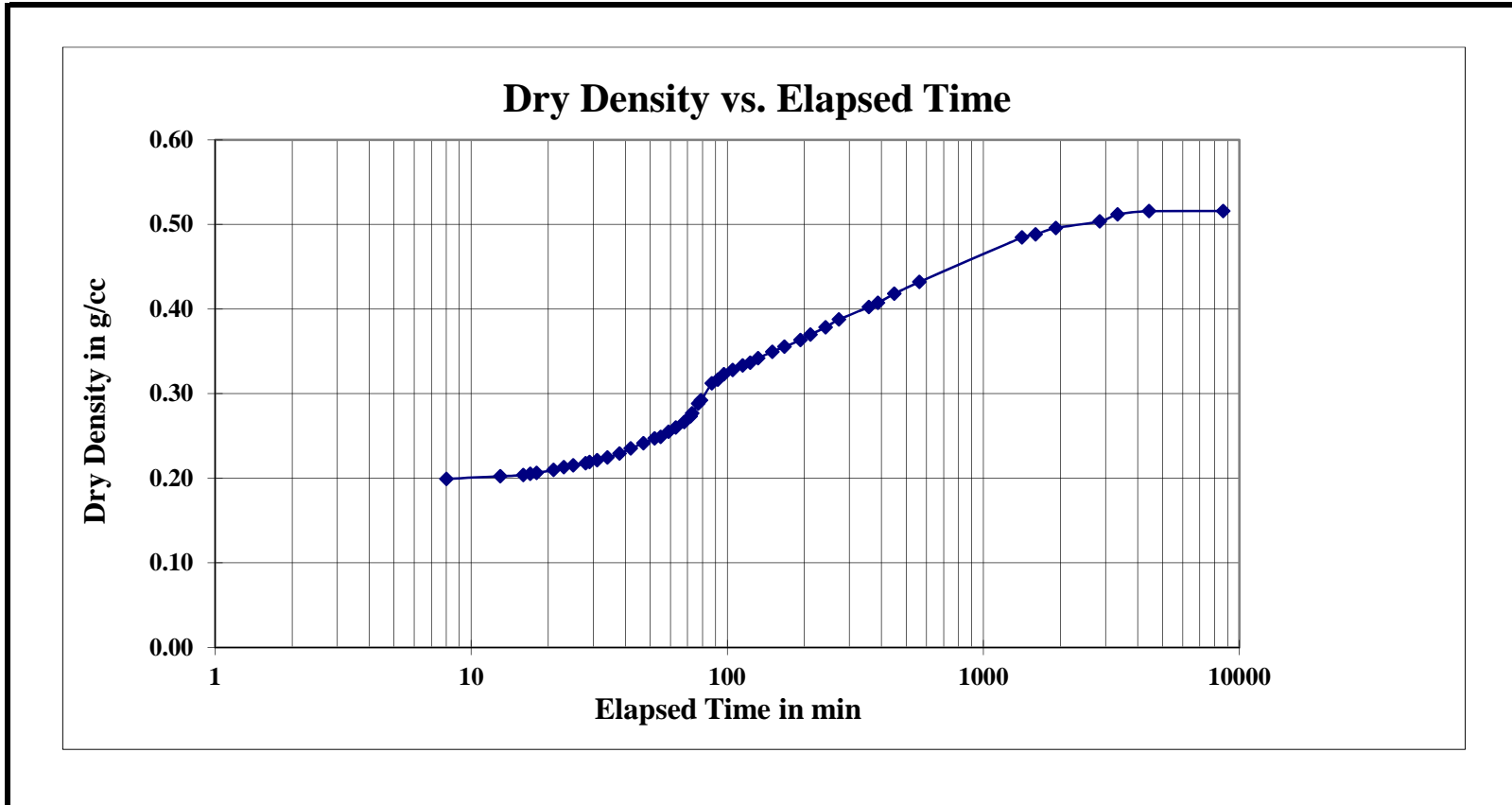
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 20 psi			
<b>Job Short Title:</b> Copper Flat Tailngs Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 5



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 50 psi			
<b>Job Short Title:</b> Copper Flat Tailngs Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 6



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D4767 CONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST REPORT SPECIMEN PHOTOGRAPH - 90 psi			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> Overflow Flume Test Beach	<b>Technician:</b> RJM	<b>Reviewed:</b> DAR	<b>Date:</b> 3/12/2013	<b>Job Number:</b> 103-92557.010	<b>Figure:</b> 7



<b>Golder Associates, Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SEDIMENTATION TESTING</b> <b>GRAPHICAL DATA</b>			
<b>Job Short Title:</b> <b>Copper Flat Tailings Design Study</b>					
<b>Sample No.</b> <b>Tailings Overflow Beach</b>	<b>System</b> <b>Single Drain</b>	<b>Reviewed:</b> MB	<b>Date:</b> 08-Nov-12	<b>Job Number:</b> 103-92557	<b>Figure:</b> 2

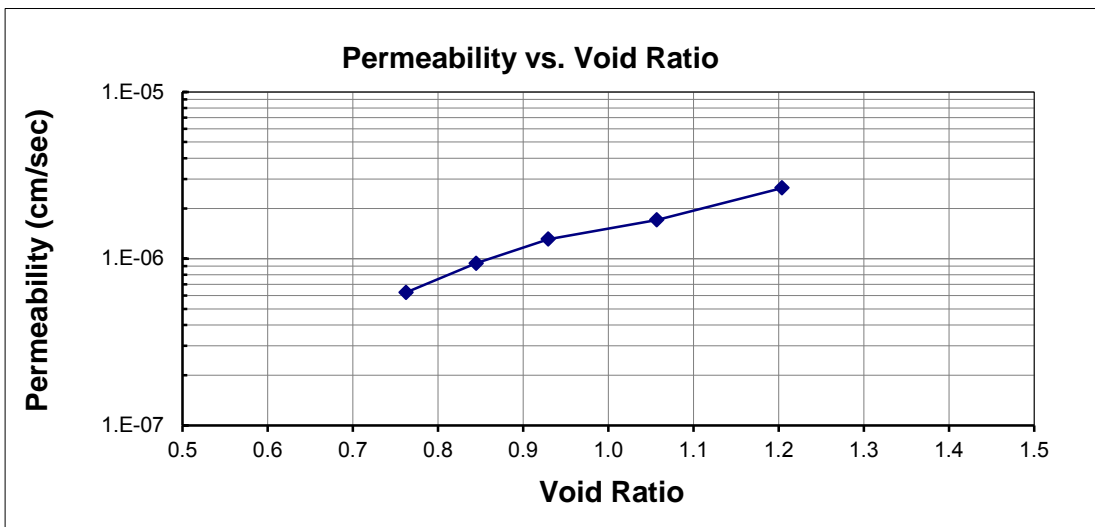
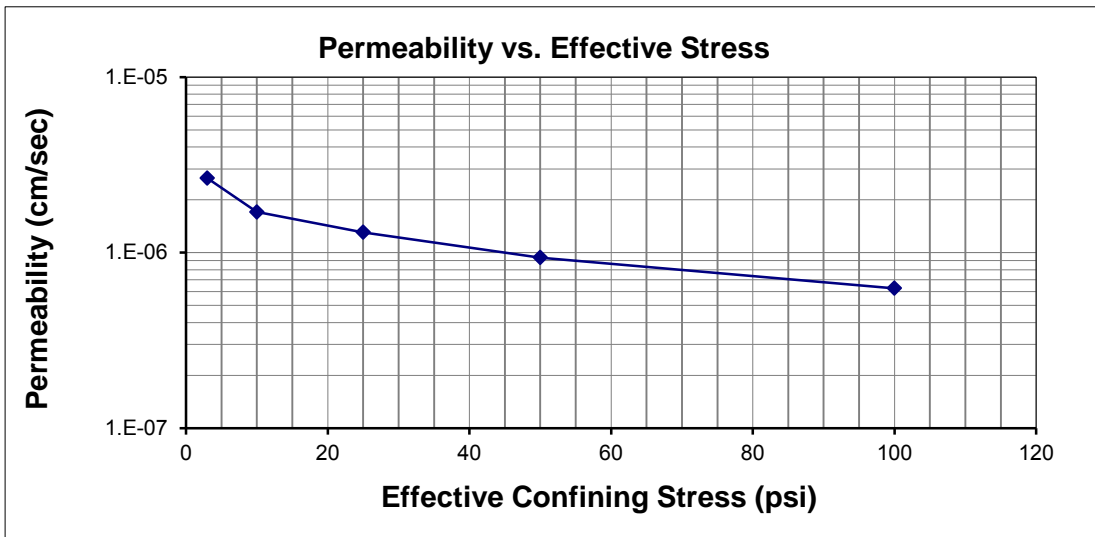




	Initial	Final	
Length =	7.826	1.25	cm
Diameter =	7.11	7.11	cm
Wet Mass =	358.37	98.17	g
Area =	39.70	39.70	cm <sup>2</sup>
Volume =	310.7	49.5	cm <sup>3</sup>
Moisture Content =	378.6%	32.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	74.88	74.09	g
Unit Weight =	1.15	1.98	g/cm <sup>3</sup>
Dry Unit Weight =	0.24	1.50	g/cm <sup>3</sup>
Unit Weight =	72.00	123.62	lb/ft <sup>3</sup>
Dry Unit Weight =	15.04	93.30	lb/ft <sup>3</sup>
Percent Solids =	20.9%	75.5%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
6.1	4.70E-06	1.24E-03	9.4	2.82E-06	2.1E-02	13.4	1.86E-06	3.2E-02	18.5	1.29E-06	5.1E-02	25.8	8.81E-07	7.0E-02
13.3	3.51E-06	9.28E-04	19.1	2.28E-06	1.7E-02	26.1	1.56E-06	2.7E-02	35.8	1.09E-06	4.4E-02	49.9	7.48E-07	5.9E-02
17.5	3.27E-06	8.65E-04	24.8	2.16E-06	1.6E-02	33.7	1.49E-06	2.5E-02	46.0	1.04E-06	4.2E-02	64.4	7.11E-07	5.6E-02
22.3	3.10E-06	8.19E-04	31.41	2.06E-06	1.5E-02	42.27	1.44E-06	2.4E-02	57.41	1.01E-06	4.0E-02	80.74	6.86E-07	5.4E-02
27.8	2.98E-06	7.88E-04	38.77	2.00E-06	1.5E-02	51.99	1.40E-06	2.4E-02	70.07	9.92E-07	4.0E-02	99.34	6.69E-07	5.3E-02
34.3	2.89E-06	7.63E-04	47.58	1.94E-06	1.4E-02	63.55	1.37E-06	2.3E-02	85.49	9.71E-07	3.9E-02	121.11	6.55E-07	5.2E-02
42.2	2.81E-06	7.43E-04	58.69	1.89E-06	1.4E-02	77.12	1.35E-06	2.3E-02	103.67	9.57E-07	3.8E-02	147.26	6.44E-07	5.1E-02
52.5	2.72E-06	7.19E-04	72.92	1.83E-06	1.4E-02	94.4	1.33E-06	2.3E-02	126.62	9.46E-07	3.8E-02	179.88	6.36E-07	5.0E-02
66.3	2.66E-06	7.03E-04	93.21	1.76E-06	1.3E-02	117.21	1.32E-06	2.2E-02	157	9.40E-07	3.8E-02	205.76	6.28E-07	5.0E-02
88.2	2.58E-06	6.82E-04	126.08	1.69E-06	1.2E-02	152.46	1.31E-06	2.2E-02	183.27	9.31E-07	3.7E-02	224.62	6.28E-07	5.0E-02
			145.49	1.67E-06	1.2E-02	175.79	1.30E-06	2.2E-02	203.01	9.40E-07	3.8E-02	248.31	6.24E-07	4.9E-02
Average (of final 3 values)	2.66E-06	7.01E-04	Average (of final 3 values)	1.71E-06	1.26E-02	Average (of final 3 values)	1.31E-06	2.23E-02	Average (of final 3 values)	9.37E-07	3.74E-02	Average (of final 3 values)	6.26E-07	4.95E-02

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Tailings Overflow Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 12/3/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**

Job Short Title:  
 Copper Flat Tailings Design Study

**PERMEABILITY DATA**

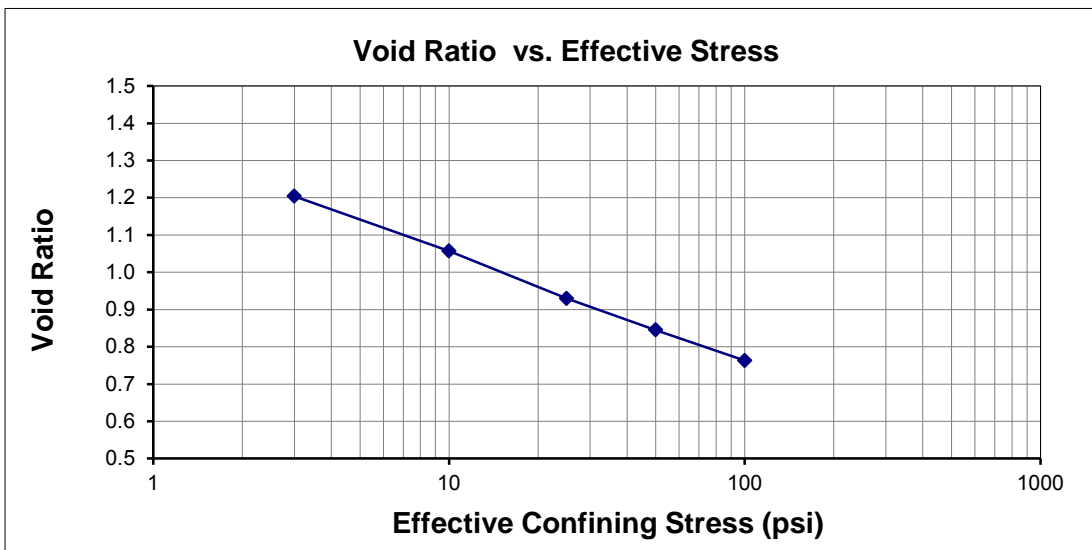
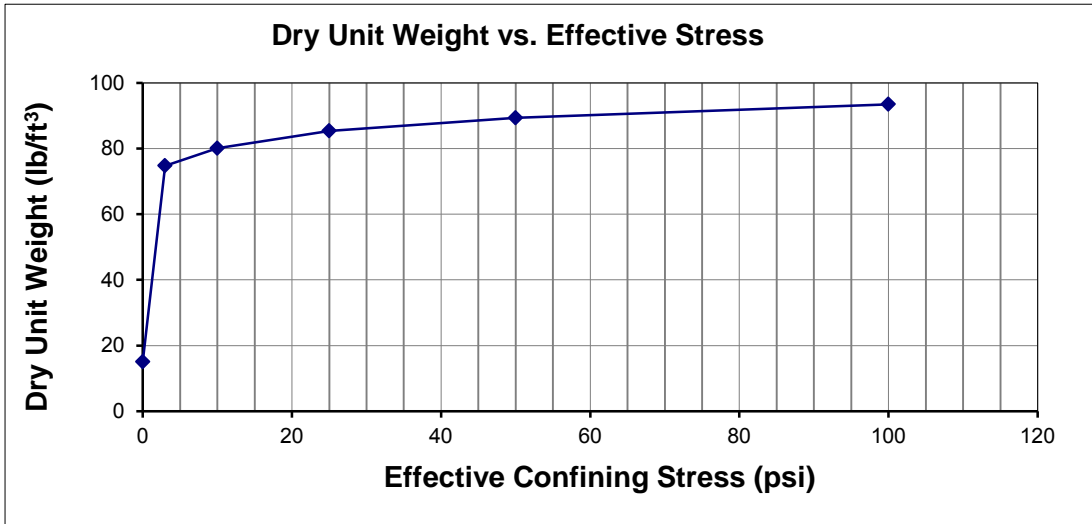
Sample No.  
 Tailings Overflow Flume Test Beach

Reviewed:  
 CCS

Date:  
 12/3/2012

Job Number:  
 103-92557

Figure:  
 2



**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
**SLURRY CONSOLIDATION TEST RESULTS**  
**DENSITY DATA**

**Job Short Title:**  
 Copper Flat Tailings Design Study

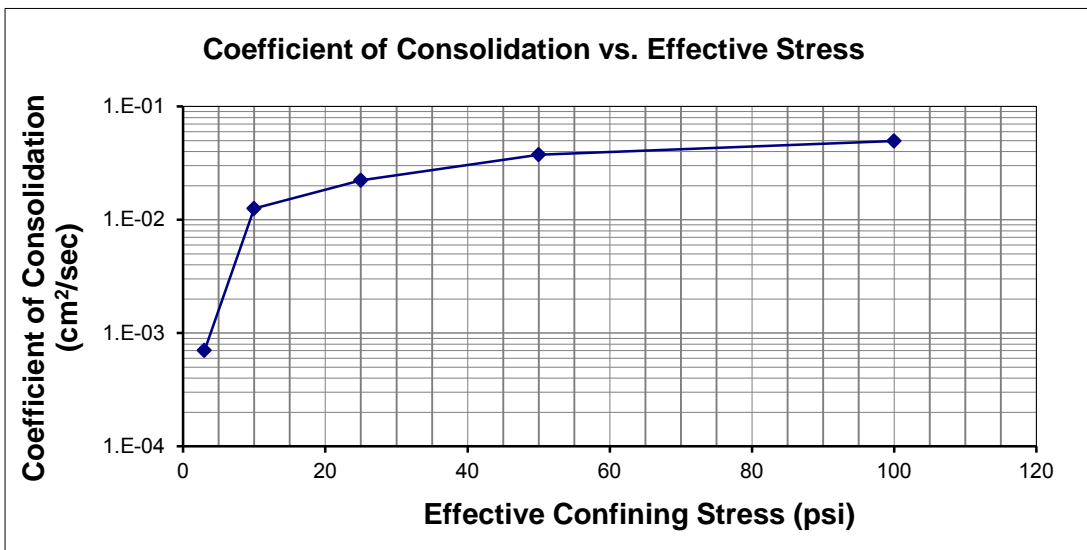
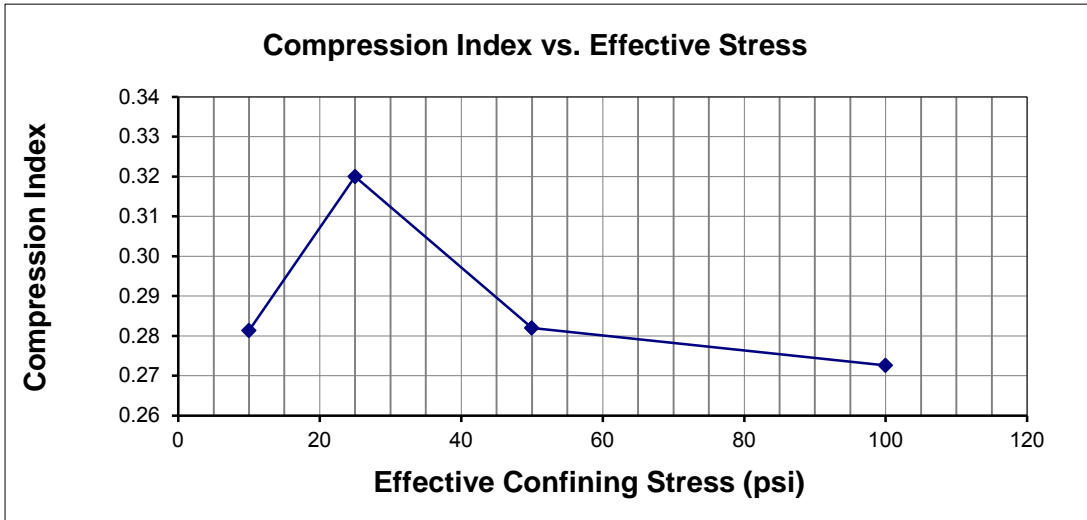
**Sample No.**  
 Tailings Overflow Flume Test Beach

**Reviewed:**  
 CCS

**Date:**  
 12/3/2012

**Job Number:**  
 103-92557

**Figure:**  
 3



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample No.  
 Tailings Overflow Flume Test Beach

Reviewed:  
 CCS

Date:  
 12/3/2012

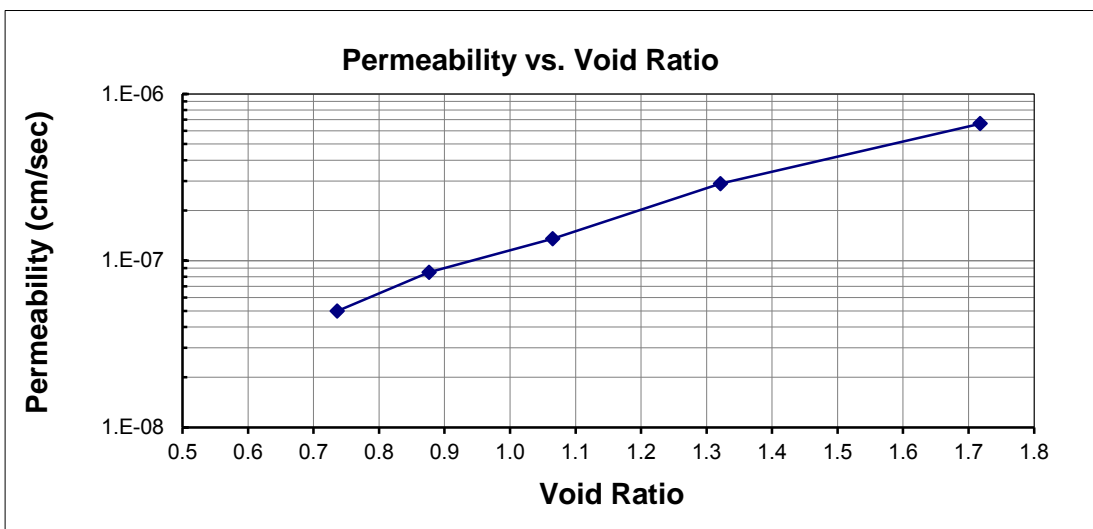
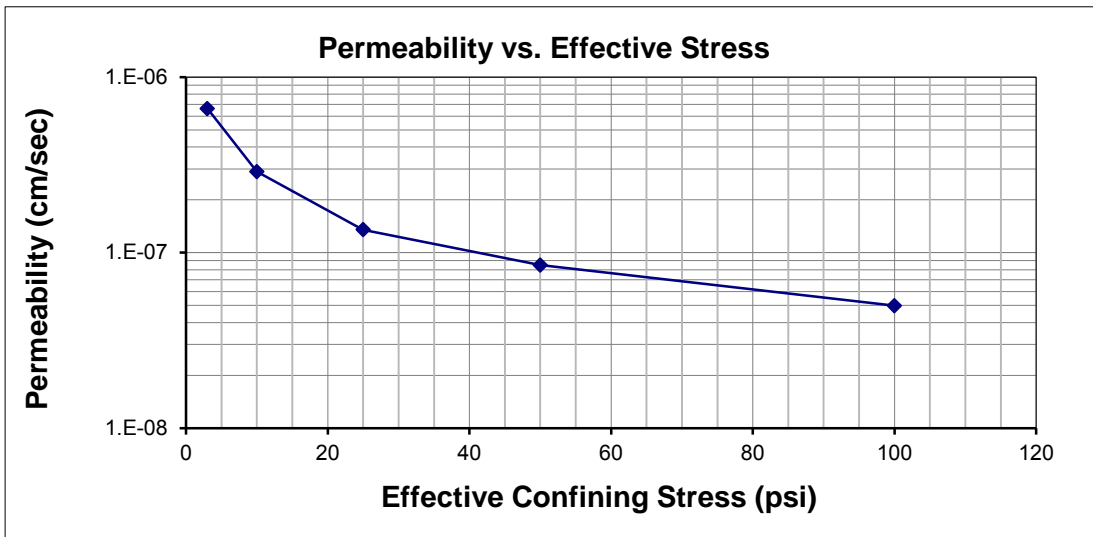
Job Number:  
 103-92557

Figure:  
 4

	Initial	Final	
Length =	8.172	0.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	351.12	53.21	g
Area =	39.64	39.64	cm <sup>2</sup>
Volume =	323.9	25.5	cm <sup>3</sup>
Moisture Content =	786.2%	37.1%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	39.62	38.81	g
Unit Weight =	1.08	2.08	g/cm <sup>3</sup>
Dry Unit Weight =	0.12	1.52	g/cm <sup>3</sup>
Unit Weight =	67.67	130.13	lb/ft <sup>3</sup>
Dry Unit Weight =	7.64	94.92	lb/ft <sup>3</sup>
Percent Solids =	11.3%	72.9%	

Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
22.46	8.20E-07	1.98E-04	50.77	3.10E-07	1.0E-03	98.24	1.42E-07	1.4E-03	144.92	8.78E-08	1.7E-03	228.9	5.14E-08	2.4E-03
41.52	7.28E-07	1.76E-04	86.68	2.98E-07	1.0E-03	166.02	1.38E-07	1.3E-03	243.05	8.59E-08	1.7E-03	384.0	5.03E-08	2.4E-03
52.62	7.06E-07	1.70E-04	107.84	2.94E-07	9.9E-04	205.40	1.37E-07	1.3E-03	301.11	8.52E-08	1.6E-03	474.8	5.00E-08	2.4E-03
65.16	6.89E-07	1.66E-04	131.71	2.91E-07	9.8E-04	250.52	1.36E-07	1.3E-03	364.17	8.52E-08	1.6E-03	576.64	4.98E-08	2.3E-03
79.43	6.78E-07	1.64E-04	158.8	2.90E-07	9.8E-04	299.74	1.36E-07	1.3E-03	436.95	8.51E-08	1.6E-03	690.24	4.98E-08	2.3E-03
96.23	6.68E-07	1.61E-04	190.02	2.89E-07	9.7E-04	359.30	1.36E-07	1.3E-03	524.36	8.46E-08	1.6E-03	824.08	4.98E-08	2.3E-03
115.84	6.63E-07	1.60E-04	226.55	2.90E-07	9.8E-04	430.18	1.36E-07	1.3E-03	625.14	8.49E-08	1.6E-03			
140.26	6.61E-07	1.60E-04				521.68	1.35E-07	1.3E-03	751.92	8.51E-08	1.6E-03			
172.37	6.63E-07	1.60E-04				590.52	1.35E-07	1.3E-03	849.86	8.51E-08	1.6E-03			
						638.40	1.36E-07	1.3E-03						
						707.55	1.35E-07	1.3E-03						
Average (of final 3 values)	6.62E-07	1.60E-04	Average (of final 3 values)	2.89E-07	9.77E-04	Average (of final 3 values)	1.35E-07	1.29E-03	Average (of final 3 values)	8.50E-08	1.64E-03	Average (of final 3 values)	4.98E-08	2.34E-03

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Tailings Overflow Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 12/17/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**

Job Short Title:  
 Copper Flat Tailings Design Study

**PERMEABILITY DATA**

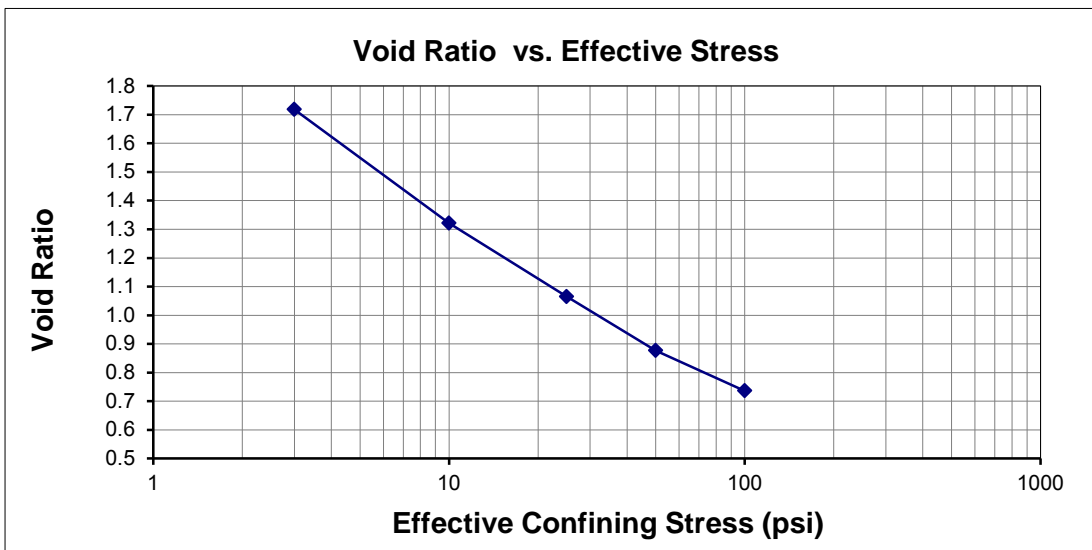
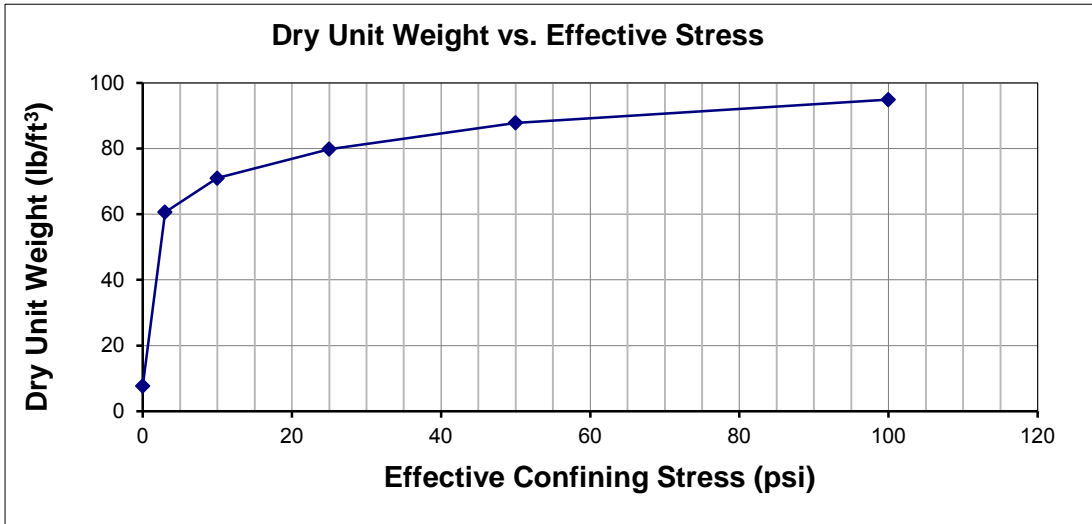
Sample No.  
 Tailings Overflow Flume Test Slime

Reviewed:  
 CCS

Date:  
 12/17/2012

Job Number:  
 103-92557

Figure:  
 2



**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
**SLURRY CONSOLIDATION TEST RESULTS**  
**DENSITY DATA**

**Job Short Title:**  
 Copper Flat Tailings Design Study

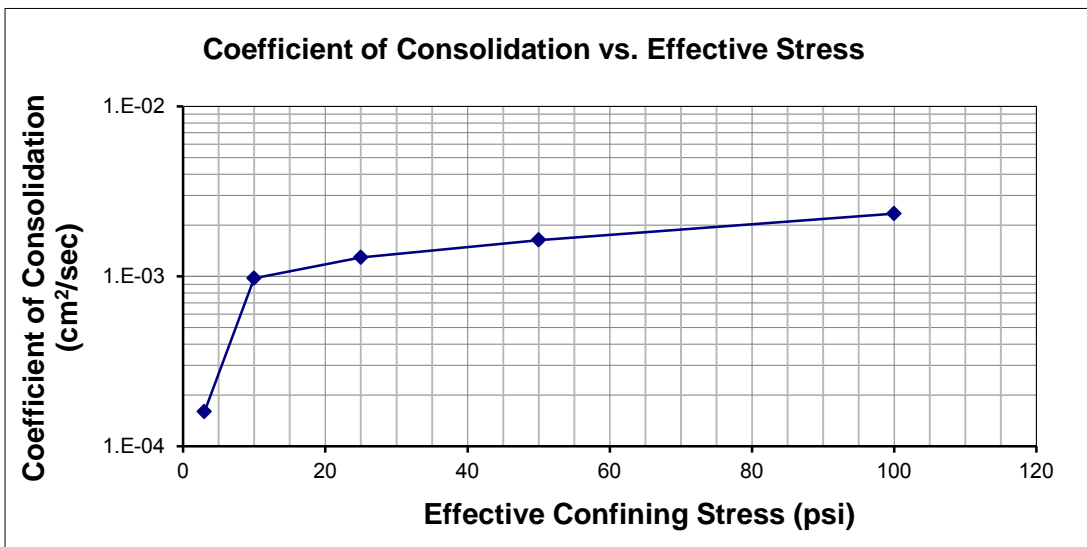
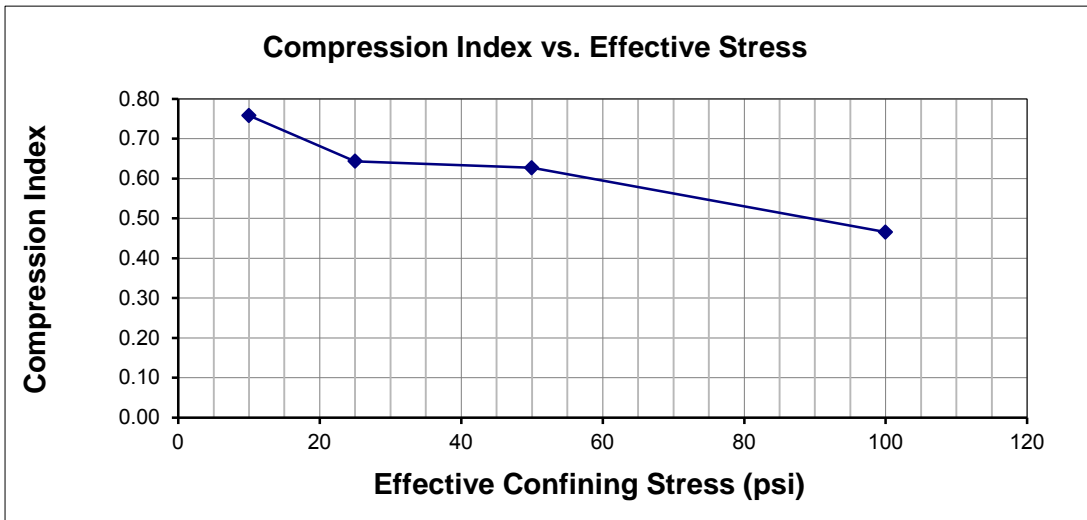
**Sample No.**  
 Tailings Overflow Flume Test Slime

**Reviewed:**  
 CCS

**Date:**  
 12/17/2012

**Job Number:**  
 103-92557

**Figure:**  
 3

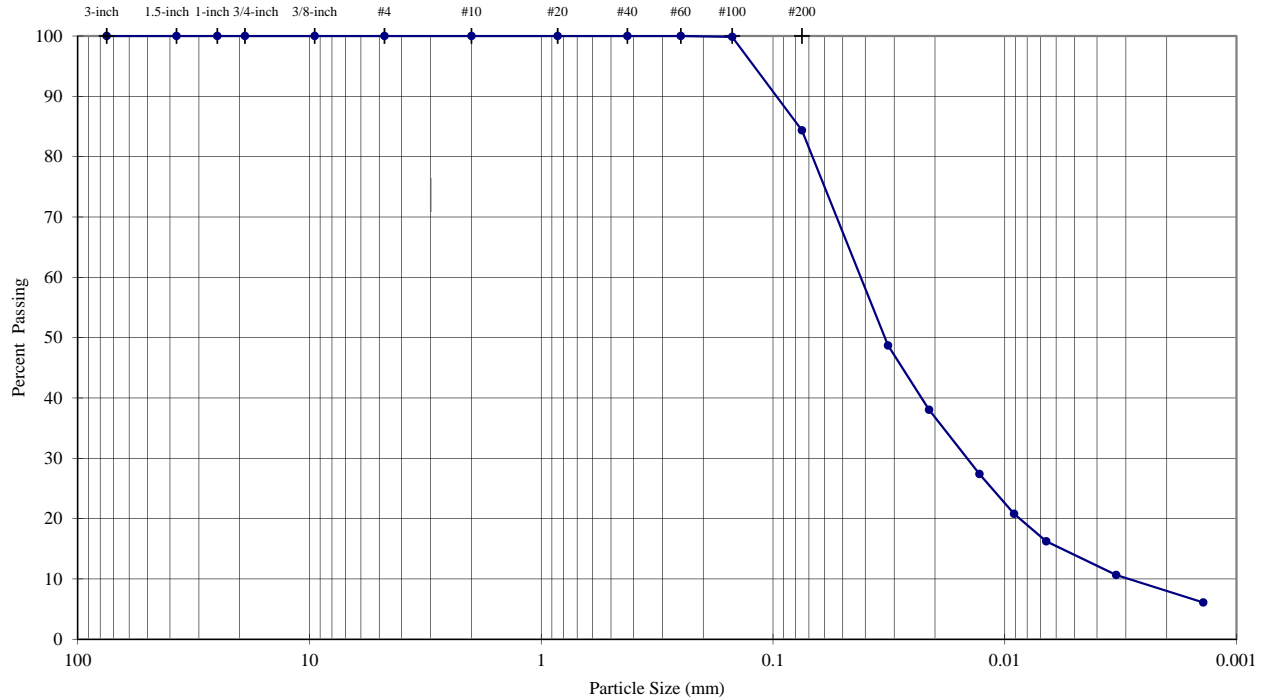


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>COMPRESSION DATA</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.</b> Tailings Overflow Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 12/17/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 4

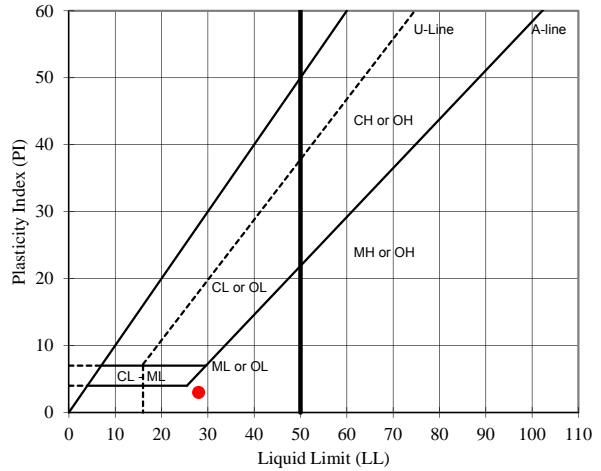


## PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Beach**  
 TYPE: **Flume Test**



		Particle Size			
		Sieve	(mm)	% Passing	
Sieve Analysis (Initial Separation on No. 4 Sieve)		3-inch	75.0	100.0	Coarse Gravel
		1.5-inch	37.5	100.0	
		1-inch	25.0	100.0	
		3/4-inch	19.0	100.0	Fine Gravel
		3/8-inch	9.5	100.0	
		#4	4.75	100.0	Coarse Sand
		#10	2.0	100.0	
		#20	0.85	100.0	Medium Sand
		#40	0.425	100.0	
		#60	0.25	100.0	Fine Sand
	#100	0.15	99.9		
	#200	0.075	84.4		
Hydrometer Analysis			0.032	48.7	Silt or Clay Fines
			0.021	38.0	
			0.013	27.4	
			0.009	20.8	
			0.007	16.2	
			0.003	10.7	
		0.001	6.1		
				<b>84.36</b>	



USCS Description (ASTM D 2487):

Dry, pale yellow silt

LL	PL	PI	SpG (assumed)
28	25	3	2.64

As-Received Moisture Content (%)

#DIV/0!

USCS Group Symbol

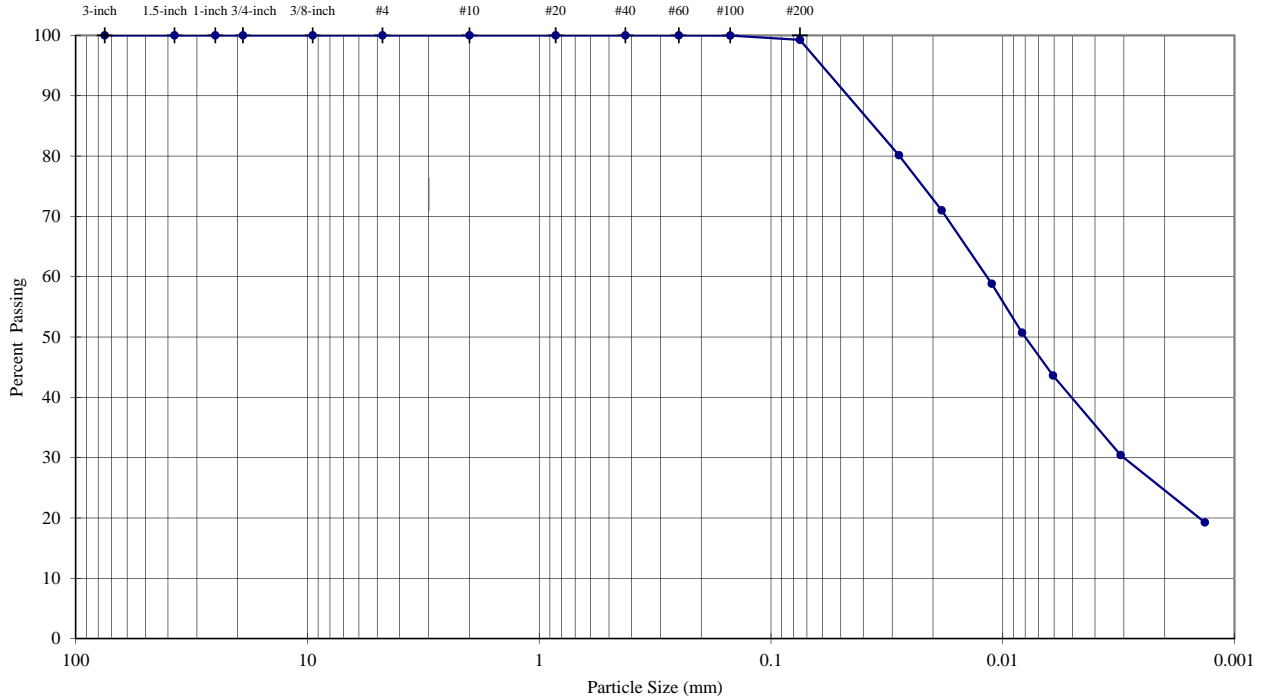
ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute  
 Sample prepared for Atterberg Limits testing by the dry method  
 Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving  
 Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

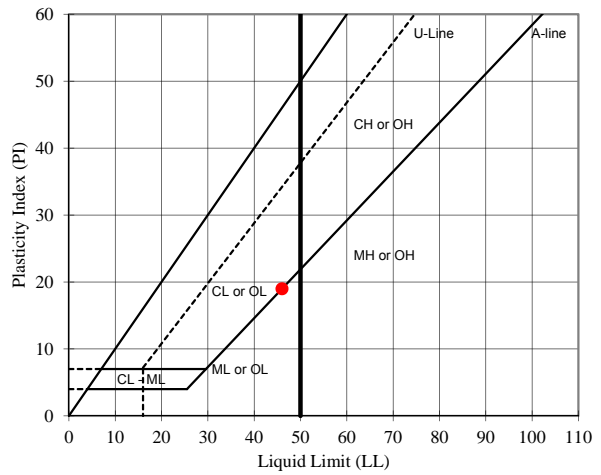
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

## PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Tailings Overflow** DEPTH (ft): **Slime**  
 TYPE: **Flume Test**



	Particle Size		Description	Percentage	
	Sieve	(mm)			% Passing
Sieve Analysis (Initial Separation on No. 4 Sieve)	3-inch	75.0	100.0	Coarse Gravel	0.00
	1.5-inch	37.5	100.0		
	1-inch	25.0	100.0		
	3/4-inch	19.0	100.0	Fine Gravel	0.00
	3/8-inch	9.5	100.0		
	#4	4.75	100.0	Coarse Sand	0.00
	#10	2.0	100.0		
	#20	0.85	100.0	Medium Sand	0.00
	#40	0.425	100.0		
	Hydrometer Analysis	#60	0.25	100.0	Fine Sand
#100		0.15	100.0		
#200		0.075	99.3	Silt or Clay Fines	99.26
		0.028	80.1		
		0.018	71.0		
		0.011	58.8		
		0.008	50.7		
	0.006	43.6			
	0.003	30.4			
	0.001	19.3			



Visual Description (Golder Procedure):  
 Dry, pale yellow silty clay

LL	PL	PI	SpG (assumed)
46	27	19	2.64

As-Received Moisture Content (%) USCS Group Symbol  
 -- CL-ML

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

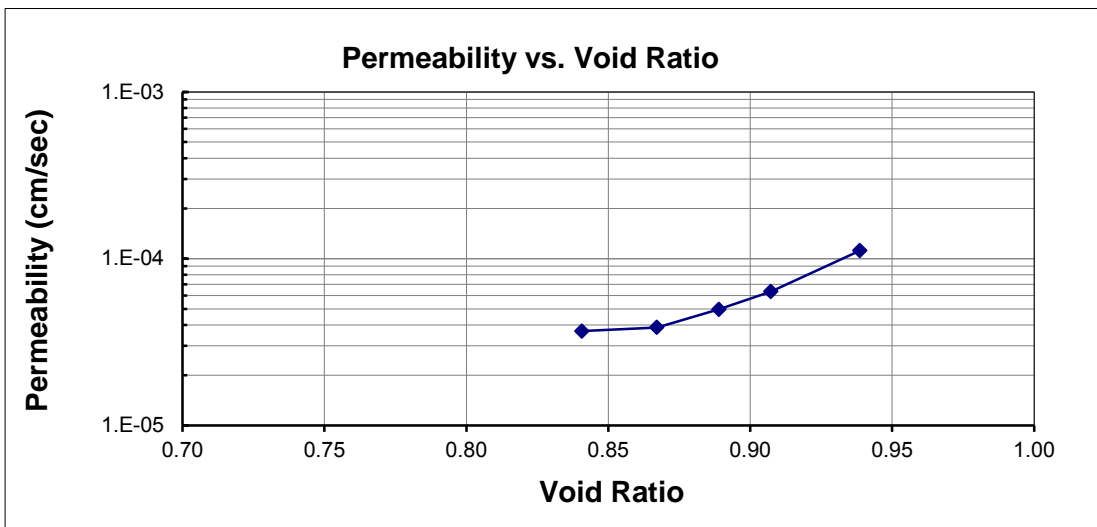
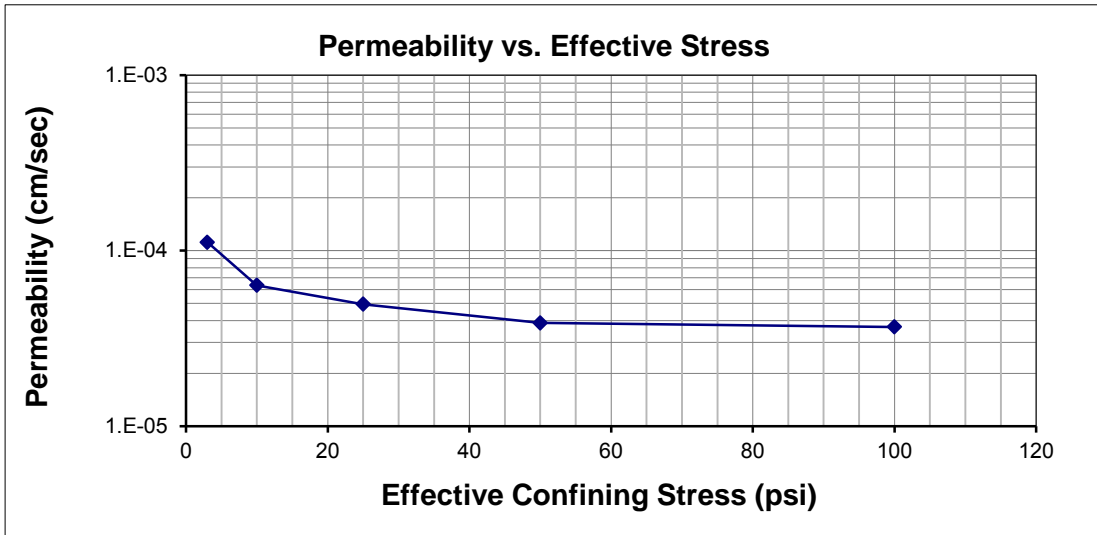
TECH	PRH/SRS
DATE	12/16/2012
REVIEW	MB

**APPENDIX B.4.2  
WHOLE TAILINGS TEST REPORTS**

	Initial	Final	
Length =	7.852	5.64	cm
Diameter =	7.104	7.104	cm
Wet Mass =	515.99	424.51	g
Area =	39.64	39.64	cm <sup>2</sup>
Volume =	311.2	223.6	cm <sup>3</sup>
Moisture Content =	60.1%	32.4%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	322.29	320.63	g
Unit Weight =	1.66	1.90	g/cm <sup>3</sup>
Dry Unit Weight =	1.04	1.43	g/cm <sup>3</sup>
Unit Weight =	103.50	118.30	lb/ft <sup>3</sup>
Dry Unit Weight =	64.65	89.35	lb/ft <sup>3</sup>
Percent Solids =	62.5%	75.5%	

Piston Pressure: 8 psi			Piston Pressure: 15 psi			Piston Pressure: 30 psi			Piston Pressure: 55 psi			Piston Pressure: 105 psi		
Sample Pressure:	5 psi	562.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	1,054.6 g/cm <sup>2</sup>	Sample Pressure:	5 psi	2,109.2 g/cm <sup>2</sup>	Sample Pressure:	5 psi	3,866.9 g/cm <sup>2</sup>	Sample Pressure:	5 psi	7,382.3 g/cm <sup>2</sup>
Consolidation pressure:	3 psi	210.9 g/cm <sup>2</sup>	Consolidation Pressure:	10 psi	703.1 g/cm <sup>2</sup>	Consolidation Pressure:	25 psi	1,757.7 g/cm <sup>2</sup>	Consolidation Pressure:	50 psi	3,515.4 g/cm <sup>2</sup>	Consolidation Pressure:	100 psi	7,030.8 g/cm <sup>2</sup>
<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>		
Initial Sample Height:	7.85	cm	Initial Sample Height:	5.94	cm	Initial Sample Height:	5.84	cm	Initial Sample Height:	5.79	cm	Initial Sample Height:	5.72	cm
Initial Dry Unit Weight:	1.04	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.36	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.38	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.40	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.41	g/cm <sup>3</sup>
Initial Void Ratio:	1.55		Initial Void Ratio:	0.94		Initial Void Ratio:	0.91		Initial Void Ratio:	0.89		Initial Void Ratio:	0.87	
<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>		
Final Sample Height:	5.94	cm	Final Sample Height:	5.84	cm	Final Sample Height:	5.79	cm	Final Sample Height:	5.72	cm	Final Sample Height:	5.64	cm
Final Dry Unit Weight:	1.36	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.38	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.40	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.41	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.43	g/cm <sup>3</sup>
Final Void Ratio:	0.94		Final Void Ratio:	0.91		Final Void Ratio:	0.89		Final Void Ratio:	0.87		Final Void Ratio:	0.84	
<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>		
Coefficient of Compressibility, a <sub>v</sub>	2.90E-03	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	6.37E-05	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	1.73E-05	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	1.24E-05	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	7.52E-06	cm <sup>2</sup> /g
Coefficient of Volume Compressibility, m <sub>v</sub>	1.14E-03	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	3.28E-05	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	9.09E-06	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	6.59E-06	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	4.03E-06	cm <sup>2</sup> /g
Compression Index, C <sub>c</sub>	-		Compression Index, C <sub>c</sub>	0.06		Compression Index, C <sub>c</sub>	0.05		Compression Index, C <sub>c</sub>	0.07		Compression Index, C <sub>c</sub>	0.09	
Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)	Δ Time (sec)	Permeability k (cm/sec)	Coefficient of Consolidation, c <sub>v</sub> (cm <sup>2</sup> /sec)
3.85	1.12E-04	9.86E-02	6.70	6.33E-05	1.93	10.15	4.96E-05	5.46	10.77	3.86E-05	5.86	8.18	3.68E-05	9.14
4.60	1.12E-04	9.88E-02	8.03	6.33E-05	1.93	12.44	4.92E-05	5.41	12.80	3.89E-05	5.90	9.75	3.69E-05	9.16
5.68	1.11E-04	9.73E-02	9.69	6.38E-05	1.94	15.42	4.99E-05	5.50	15.65	3.86E-05	5.87	11.77	3.65E-05	9.07
Average (of final 3 values)	1.12E-04	9.83E-02	Average (of final 3 values)	6.35E-05	1.93E+00	Average (of final 3 values)	4.96E-05	5.46E+00	Average (of final 3 values)	3.87E-05	5.88E+00	Average (of final 3 values)	3.67E-05	9.12E+00

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST</b> <b>SAMPLE DATA AND CALCULATIONS</b>			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample No.</b> Whole Tailings Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 1	



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**PERMEABILITY DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

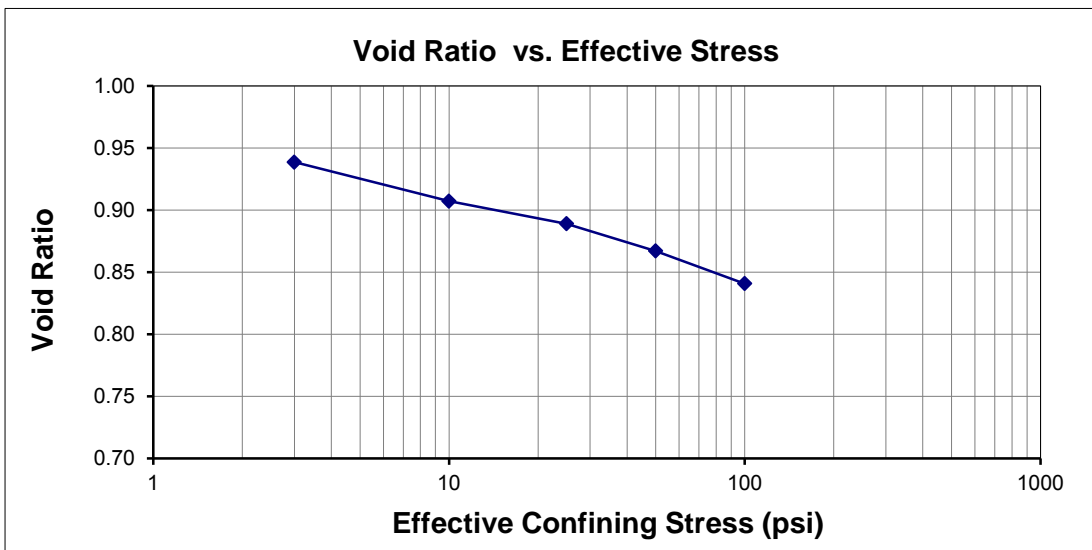
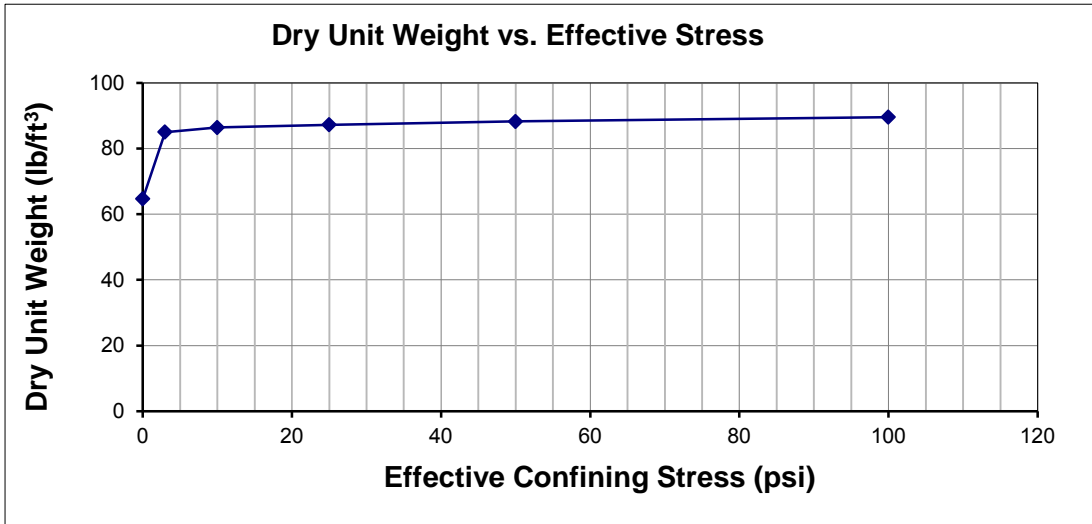
Sample No.  
 Whole Tailings Flume Test Beach

Reviewed:  
 CCS

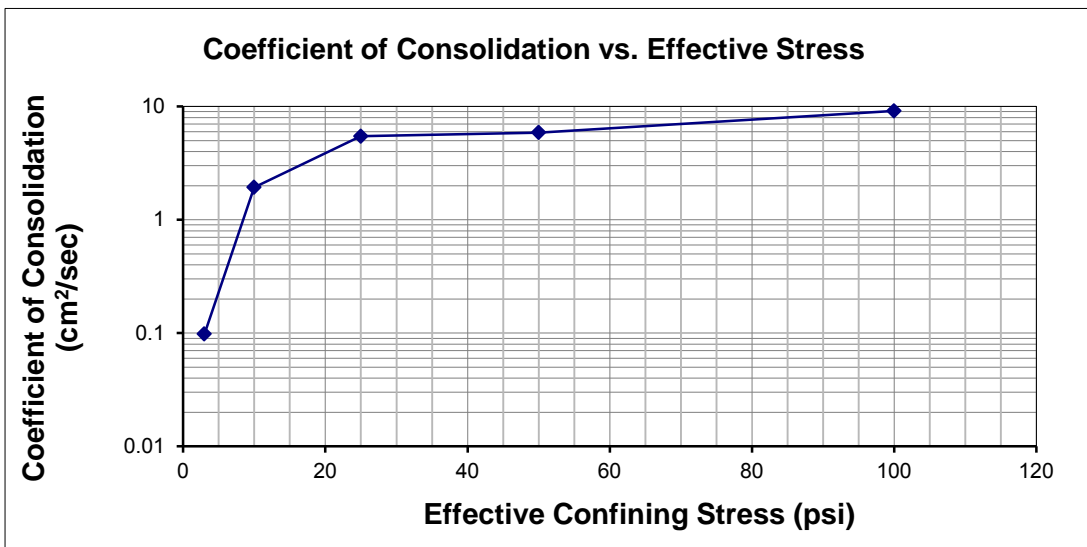
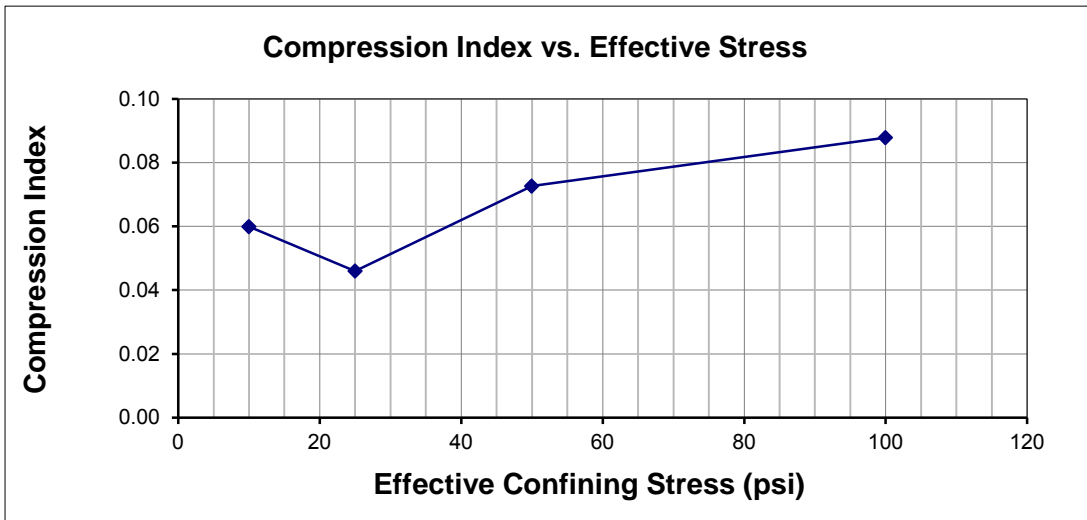
Date:  
 11/12/2012

Job Number:  
 103-92557

Figure:  
 2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> <b>SLURRY CONSOLIDATION TEST RESULTS</b> <b>DENSITY DATA</b>		
<b>Job Short Title:</b> Copper Flat Tailings Design Study				
<b>Sample No.</b> Whole Tailings Flume Test Beach	<b>Reviewed:</b> CCS	<b>Date:</b> 11/12/2012	<b>Job Number:</b> 103-92557	<b>Figure:</b> 3



**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

**Job Short Title:**  
 Copper Flat Tailings Design Study

**Sample No.**  
 Whole Tailings Flume Test Beach

**Reviewed:**  
 CCS

**Date:**  
 11/12/2012

**Job Number:**  
 103-92557

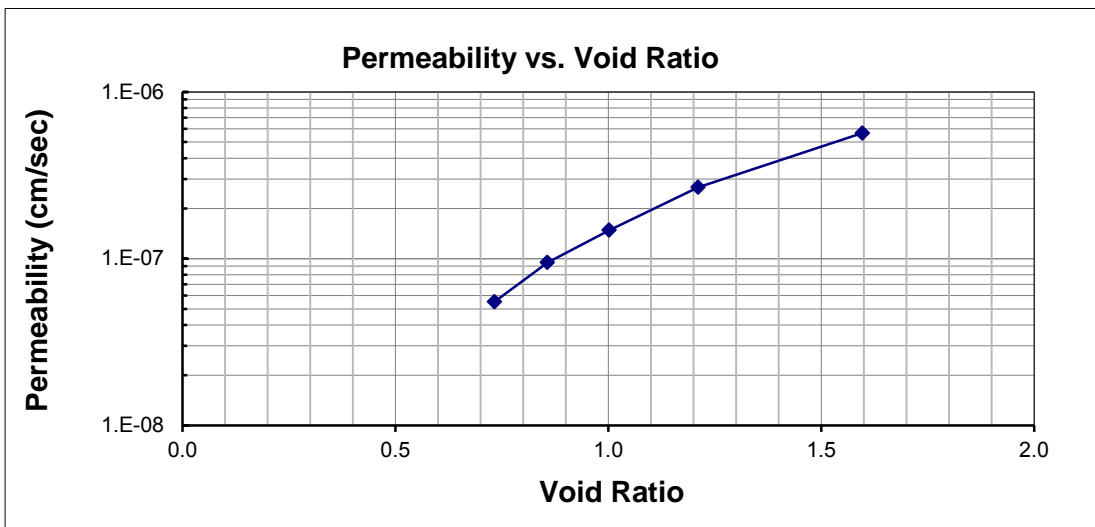
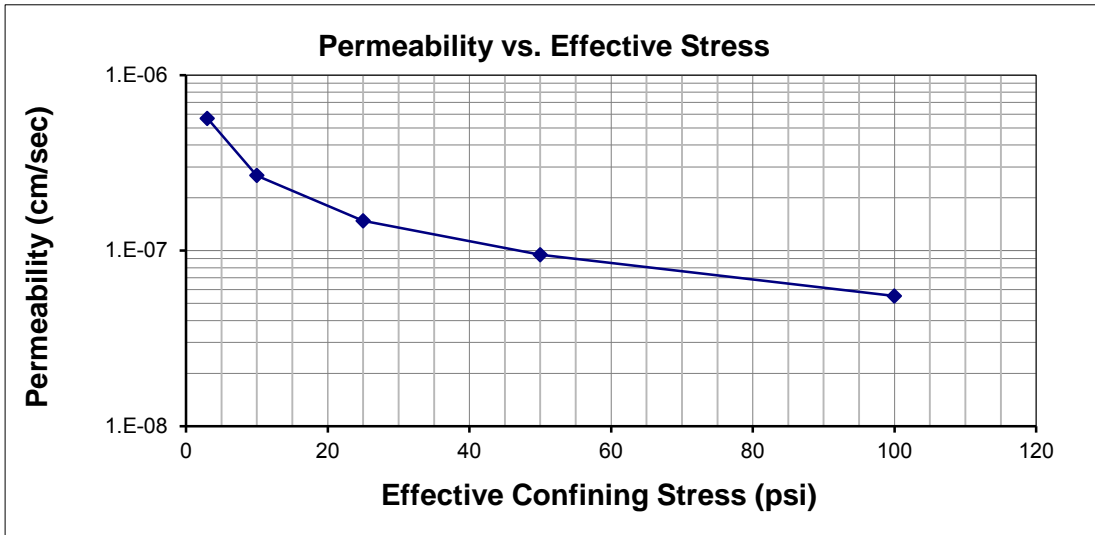
**Figure:**  
 4

	Initial	Final	
Length =	7.285	0.740	cm
Diameter =	7.099	7.099	cm
Wet Mass =	338.65	60.46	g
Area =	39.58	39.58	cm <sup>2</sup>
Volume =	288.3	29.29	cm <sup>3</sup>
Moisture Content =	628.5%	35.5%	
Specific Gravity =	2.64	2.64	
Dry Mass of Solids =	46.49	44.62	g
Unit Weight =	1.174	2.064	g/cm <sup>3</sup>
Dry Unit Weight =	0.1612	1.523	g/cm <sup>3</sup>
Unit Weight =	73.32	128.6	lb/ft <sup>3</sup>
Dry Unit Weight =	10.06	94.90	lb/ft <sup>3</sup>
Percent Solids =	13.7%	73.8%	

Piston Pressure:	8 psi	562.5 g/cm <sup>2</sup>	Piston Pressure:	15 psi	1,054.6 g/cm <sup>2</sup>	Piston Pressure:	30 psi	2,109.2 g/cm <sup>2</sup>	Piston Pressure:	55 psi	3,866.9 g/cm <sup>2</sup>	Piston Pressure:	105 psi	7,382.3 g/cm <sup>2</sup>
Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>	Sample Pressure:	5 psi	351.5 g/cm <sup>2</sup>
Consolidation pressure:	3 psi	210.9 g/cm <sup>2</sup>	Consolidation Pressure:	10 psi	703.1 g/cm <sup>2</sup>	Consolidation Pressure:	25 psi	1,757.7 g/cm <sup>2</sup>	Consolidation Pressure:	50 psi	3,515.4 g/cm <sup>2</sup>	Consolidation Pressure:	100 psi	7,030.8 g/cm <sup>2</sup>
<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>			<b>Before Consolidation</b>		
Initial Sample Height:	7.29	cm	Initial Sample Height:	1.11	cm	Initial Sample Height:	0.94	cm	Initial Sample Height:	0.86	cm	Initial Sample Height:	0.79	cm
Initial Dry Unit Weight:	0.16	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.02	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.19	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.32	g/cm <sup>3</sup>	Initial Dry Unit Weight:	1.42	g/cm <sup>3</sup>
Initial Void Ratio:	15.38		Initial Void Ratio:	1.60		Initial Void Ratio:	1.21		Initial Void Ratio:	1.00		Initial Void Ratio:	0.86	
<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>			<b>After Consolidation</b>		
Final Sample Height:	1.11	cm	Final Sample Height:	0.94	cm	Final Sample Height:	0.86	cm	Final Sample Height:	0.79	cm	Final Sample Height:	0.74	cm
Final Dry Unit Weight:	1.02	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.19	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.32	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.42	g/cm <sup>3</sup>	Final Dry Unit Weight:	1.52	g/cm <sup>3</sup>
Final Void Ratio:	1.60		Final Void Ratio:	1.21		Final Void Ratio:	1.00		Final Void Ratio:	0.86		Final Void Ratio:	0.73	
<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>			<b>Calculations</b>		
Coefficient of Compressibility, a <sub>v</sub>	6.53E-02	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	7.85E-04	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	1.98E-04	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	8.26E-05	cm <sup>2</sup> /g	Coefficient of Compressibility, a <sub>v</sub>	3.53E-05	cm <sup>2</sup> /g
Coefficient of Volume Compressibility, m <sub>v</sub>	3.99E-03	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	3.02E-04	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	8.94E-05	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	4.13E-05	cm <sup>2</sup> /g	Coefficient of Volume Compressibility, m <sub>v</sub>	1.90E-05	cm <sup>2</sup> /g
Compression Index, C <sub>c</sub>	-		Compression Index, C <sub>c</sub>	0.74		Compression Index, C <sub>c</sub>	0.52		Compression Index, C <sub>c</sub>	0.48		Compression Index, C <sub>c</sub>	0.41	
<b>Δ Time (sec)</b>	<b>Permeability k (cm/sec)</b>	<b>Coefficient of Consolidation, c<sub>v</sub> (cm<sup>2</sup>/sec)</b>	<b>Δ Time (sec)</b>	<b>Permeability k (cm/sec)</b>	<b>Coefficient of Consolidation, c<sub>v</sub> (cm<sup>2</sup>/sec)</b>	<b>Δ Time (sec)</b>	<b>Permeability k (cm/sec)</b>	<b>Coefficient of Consolidation, c<sub>v</sub> (cm<sup>2</sup>/sec)</b>	<b>Δ Time (sec)</b>	<b>Permeability k (cm/sec)</b>	<b>Coefficient of Consolidation, c<sub>v</sub> (cm<sup>2</sup>/sec)</b>	<b>Δ Time (sec)</b>	<b>Permeability k (cm/sec)</b>	<b>Coefficient of Consolidation, c<sub>v</sub> (cm<sup>2</sup>/sec)</b>
204	5.66E-07	1.42E-04	367	2.67E-07	8.8E-04	604	1.47E-07	1.6E-03	533	9.48E-08	2.3E-03	1024	5.51E-08	2.9E-03
221	5.69E-07	1.43E-04	397	2.69E-07	8.9E-04	652	1.49E-07	1.7E-03	639	9.46E-08	2.3E-03	1233	5.52E-08	2.9E-03
245	5.65E-07	1.42E-04	440	2.67E-07	8.8E-04	718	1.48E-07	1.7E-03	770	9.47E-08	2.3E-03	1396	5.51E-08	2.9E-03
Average (of final 3 values)	5.66E-07	1.42E-04	Average (of final 3 values)	2.68E-07	8.86E-04	Average (of final 3 values)	1.48E-07	1.66E-03	Average (of final 3 values)	9.47E-08	2.30E-03	Average (of final 3 values)	5.51E-08	2.90E-03

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>	
<b>Job Short Title:</b> Copper Flat Tailings Design Study		<b>SLURRY CONSOLIDATION TEST SAMPLE DATA AND CALCULATIONS</b>	
<b>Sample No.</b> Whole Tailings Flume Test Slime	<b>Reviewed:</b> CCS	<b>Date:</b> 10/30/2012	<b>Job Number:</b> 103-92557
			<b>Figure:</b> 1





**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**PERMEABILITY DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

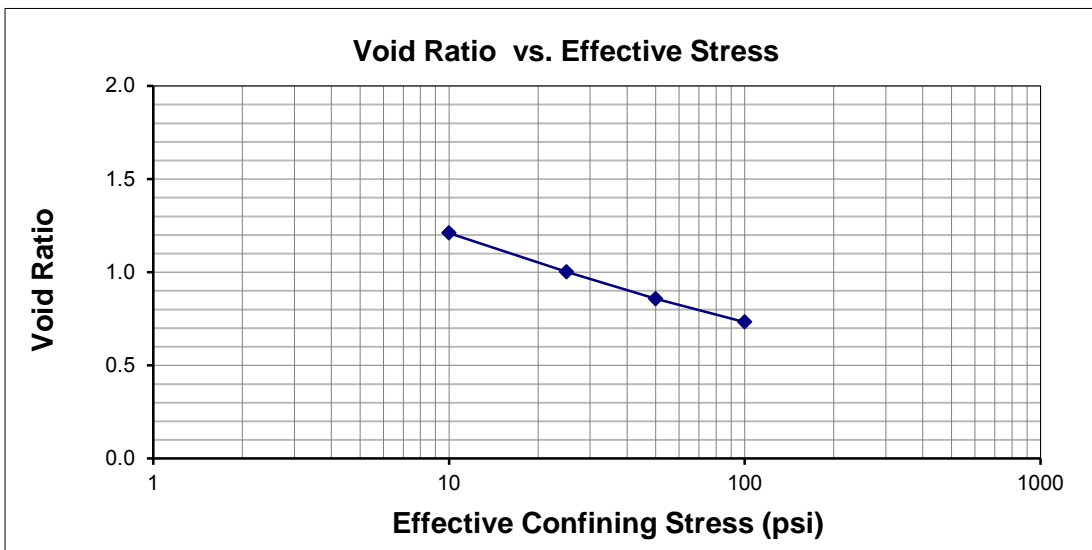
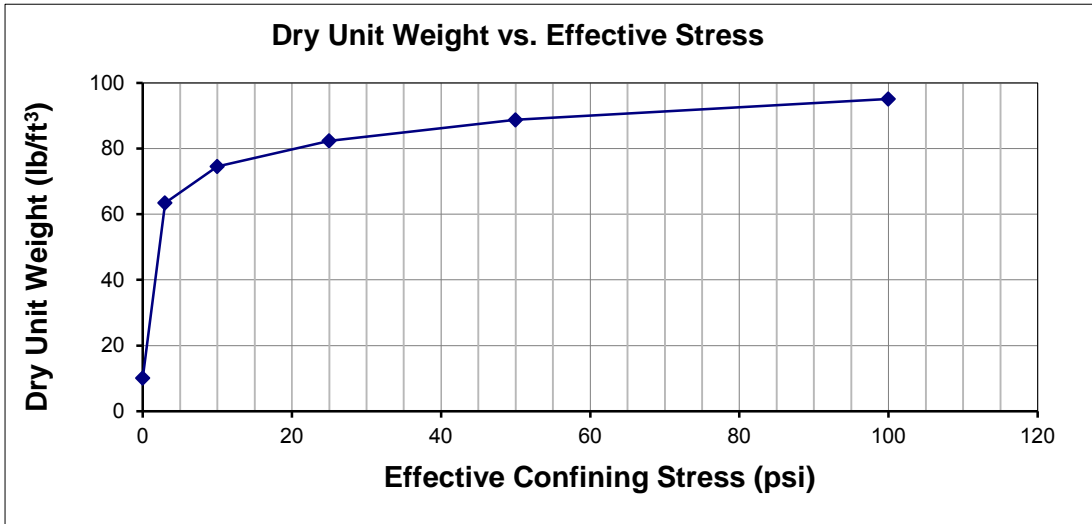
Sample No.  
 Whole Tailings Flume Test Slime

Reviewed:  
 CCS

Date:  
 10/30/2012

Job Number:  
 103-92557

Figure:  
 2



**Golder Associates Inc.**  
**Denver, Colorado**

**Title:**  
**SLURRY CONSOLIDATION TEST RESULTS**  
**DENSITY DATA**

**Job Short Title:**  
 Copper Flat Tailings Design Study

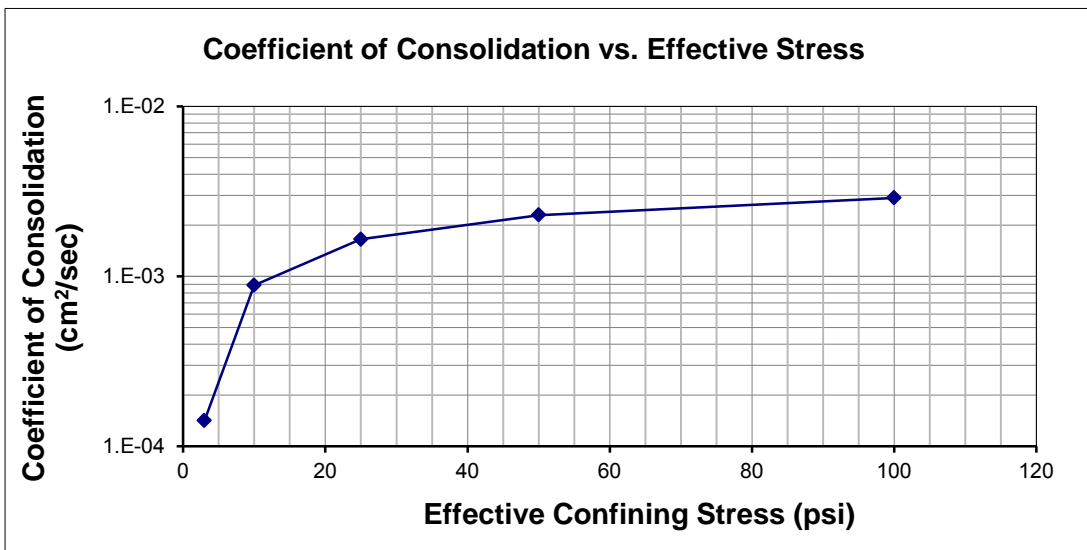
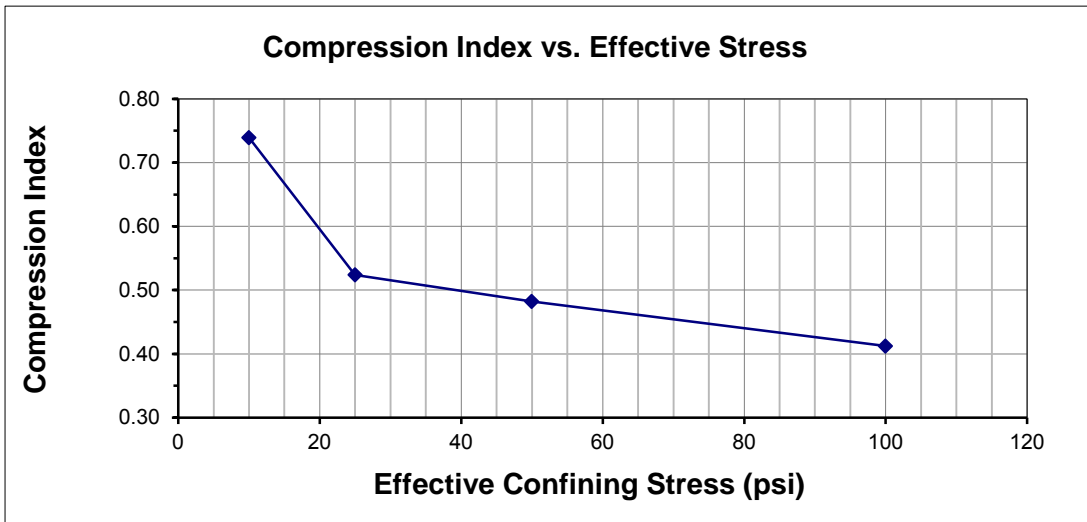
**Sample No.**  
 Whole Tailings Flume Test Slime

**Reviewed:**  
 CCS

**Date:**  
 10/30/2012

**Job Number:**  
 103-92557

**Figure:**  
 3



**Golder Associates Inc.**  
**Denver, Colorado**

Title:  
**SLURRY CONSOLIDATION TEST RESULTS**  
**COMPRESSION DATA**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample No.  
 Whole Tailings Flume Test Slime

Reviewed:  
 CCS

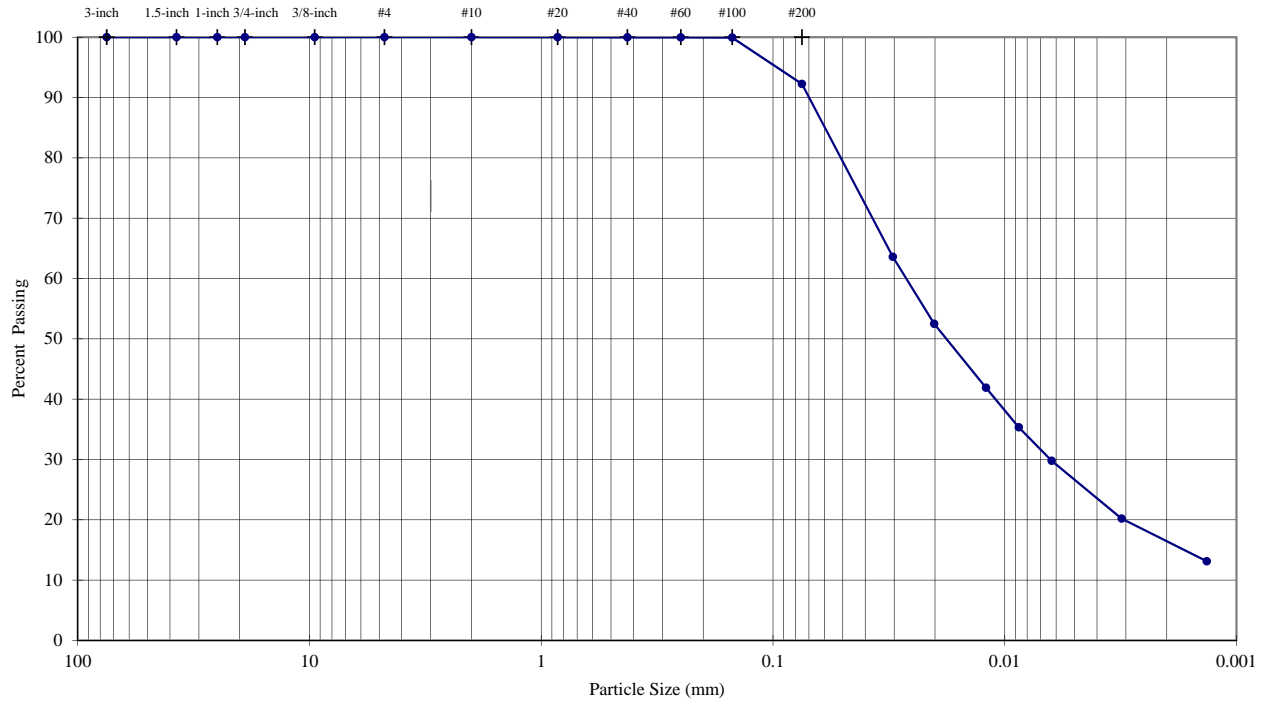
Date:  
 10/30/2012

Job Number:  
 103-92557

Figure:  
 4

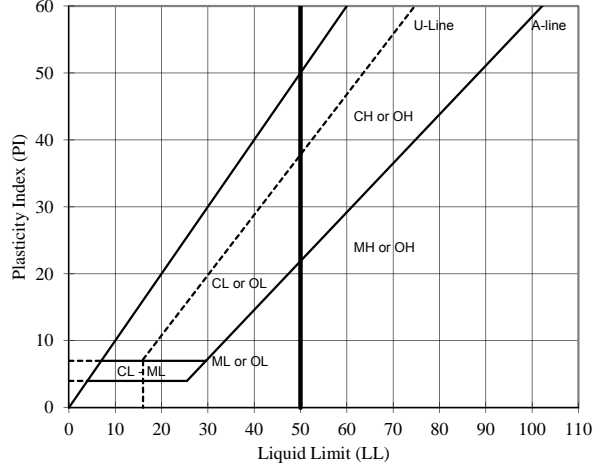
## PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Flume** DEPTH (ft): **Beach**  
 TYPE: **Single Drain**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.02
#40	0.425		
#60	0.25	Fine Sand	7.72
#100	0.15		
#200	0.075		
	0.030	Silt or Clay Fines	92.26
	0.020		
	0.012		
	0.009		
	0.006		
	0.003		
	0.001		



Visual Description (Golder Procedure):  
**Wet, light yellowish brown SITLY SAND**

LL	PL	PI	Spg (assumed)
--	--	--	2.64

As-Received Moisture Content (%)  
 --

USCS Group Symbol  
 --

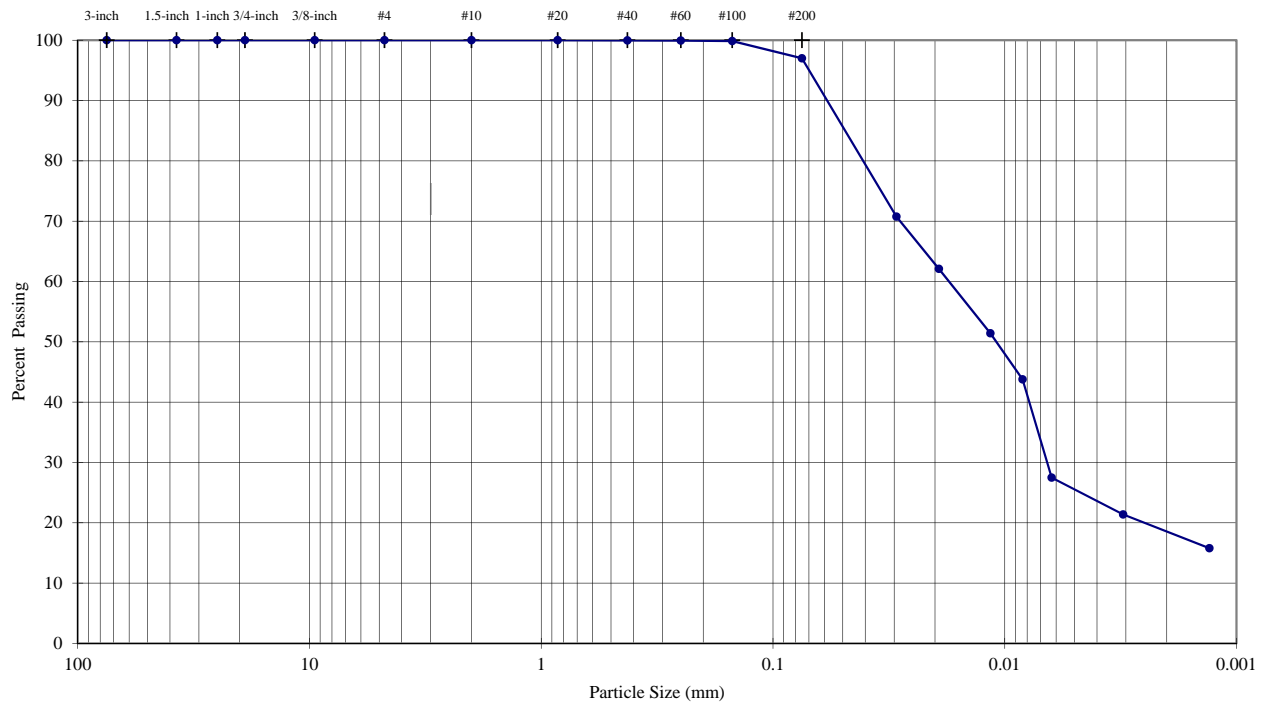
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH: **RJM/SRS**  
 DATE: **11/8/2012**  
 REVIEW: **MB**

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

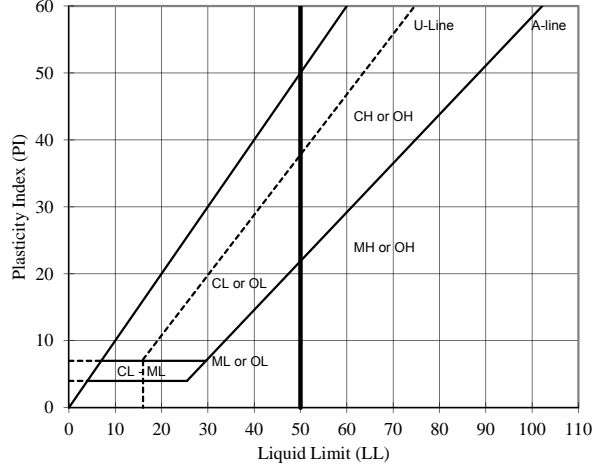
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings**  
 TYPE: **Flume**

DEPTH (ft): **Slime**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	% Passing	Description	Percentage
3-inch	75.0		
1.5-inch	37.5		
1-inch	25.0	Coarse Gravel	0.00
3/4-inch	19.0		
3/8-inch	9.5	Fine Gravel	0.00
#4	4.75		
#10	2.0	Coarse Sand	0.00
#20	0.85	Medium Sand	0.04
#40	0.425		
#60	0.25	Fine Sand	2.96
#100	0.15		
#200	0.075		
0.029	70.8		
0.019	62.1	Silt or Clay Fines	97.00
0.012	51.4		
0.008	43.8		
0.006	27.5		
0.003	21.4		
0.001	15.8		



Visual Description (Golder Procedure):  
**Wet, light yellowish brown silty sand**

LL	PL	PI	SpG (assumed)
--	--	--	2.64

As-Received Moisture Content (%) USCS Group Symbol  
 -- --

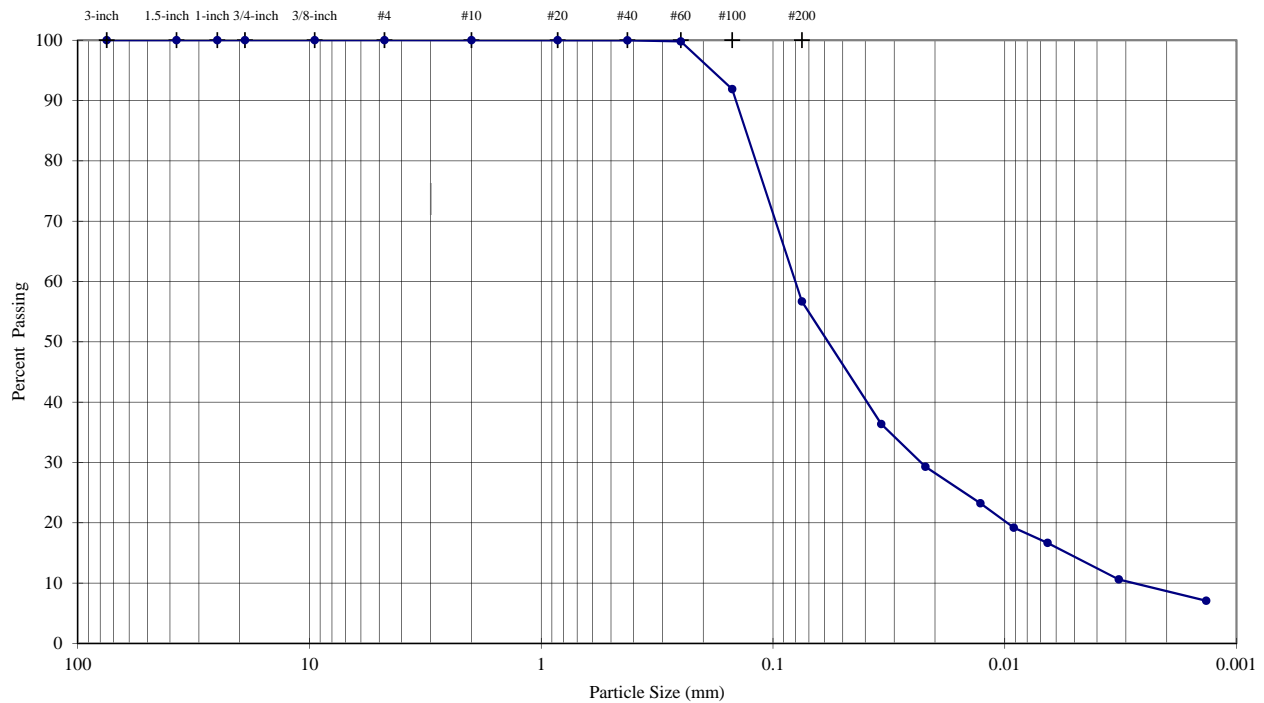
Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH AM/SRS  
 DATE 11/8/2012  
 REVIEW MB

### PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

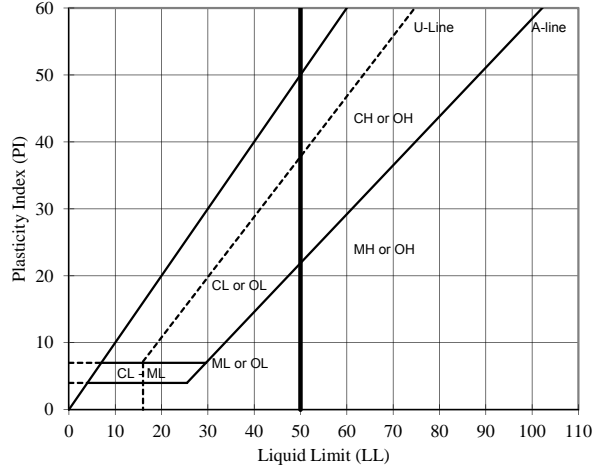
PROJECT NAME: **Copper Flat Tailings Design Study**  
 SAMPLE ID: **Whole Tailings Flume**  
 TYPE: **Single Drain**

DEPTH (ft): **Slime**



X:\Tucson\Projects\13proj\103-92557\Copper Flat TSF\30,000 TPD Report\Appendix B.4 Flume Sample Test Results\B.4.2

Sieve (mm)	Particle Size (mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Coarse Gravel	0.00
1.5-inch	37.5	100.0		
1-inch	25.0	100.0		
3/4-inch	19.0	100.0	Fine Gravel	0.00
3/8-inch	9.5	100.0		
#4	4.75	100.0	Coarse Sand	0.00
#10	2.0	100.0		
#20	0.85	100.0	Medium Sand	0.02
#40	0.425	100.0		
#60	0.25	99.8	Fine Sand	43.29
#100	0.15	91.9		
#200	0.075	56.7		
0.034	0.034	36.4		
0.022	0.022	29.3		
0.013	0.013	23.2	Silt or Clay Fines	56.69
0.009	0.009	19.2		
0.007	0.007	16.7		
0.003	0.003	10.6		
0.001	0.001	7.1		



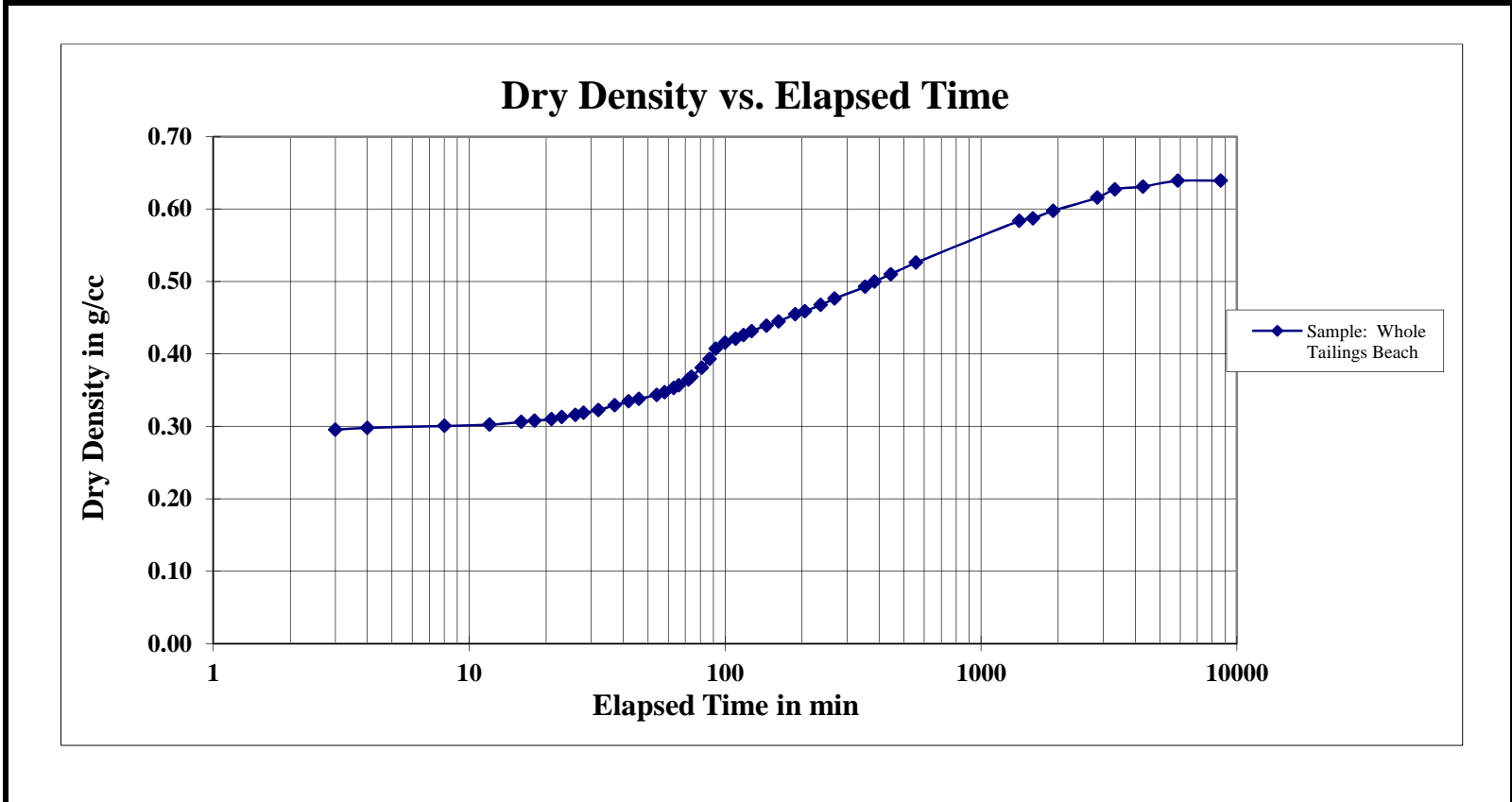
Visual Description (Golder Procedure):  
**Wet, light yellowish brown SITLY SAND**

LL	PL	PI	SpG (assumed)
--	--	--	2.64

As-Received Moisture Content (%) USCS Group Symbol  
 -- --

Notes: 0g of particles up to 4.75mm maximum size were removed from particle size analysis sample prior to testing  
 Particle size analysis sample mechanically dispersed using Stirring Apparatus A for about 1 minute

TECH **RJM/SRS**  
 DATE **11/8/2012**  
 REVIEW **MB**



<b>Golder Associates, Inc.</b> Denver, Colorado			Title: <b>SEDIMENTATION TESTING GRAPHICAL DATA</b>		
Job Short Title: Copper Flat Tailings Design Study					
Sample No. <b>Whole Tailings Beach</b>	System <b>Single Drain</b>	Reviewed: MB	Date: 23-Oct-12	Job Number: 103-92557	Figure: 2



**APPENDIX C**  
**FSCONSOL MODEL OUTPUT**



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<b>Date:</b>	November 11, 2013	<b>Made by:</b>	CDJ and DMW
<b>Project No.:</b>	133-92505	<b>Checked by:</b>	GM
<b>Subject:</b>	Cyclone Overflow Beach and Slimes Tailings Consolidation Analyses	<b>Reviewed by:</b>	GG
<b>Project Short Title:</b>	<b>COPPER FLAT FEASIBILITY DESIGN</b>		

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### 1.0 OBJECTIVE

Estimate the Tailing Storage Facility (TSF) impoundment capacity by computing the average dry density of tailings at the end-of-filling using a one-dimensional (1D) large strain consolidation model to obtain the void ratio profile with depth. Use calculated 1D percent errors to compute an adjusted average dry density, which is then weighted based on the tailings cyclone overflow split between tailings “beach” versus tailings “slimes” within the TSF impoundment.

### 2.0 ASSUMPTIONS

- Total tailings production rate (overflow and underflow): 27,618 tons per day
- Total tailings capacity (overflow and underflow): 112 million tons
- Cyclone Availability: 100 percent (i.e. no whole tailings deposited in the TSF impoundment based on the recommendation to produce as much cyclone underflow tailings sand as possible)
- Percentage split of cyclone overflow tailings: 54.75 percent of total
- Tailings production rate (overflow): 15,121 tons per day (calculated based on total tailings production rate and percentage of cyclone overflow)
- Percentage split of slimes: 40 percent by mass of storage in TSF impoundment
- Initial Tailings Overflow Solids Content: 25.9 percent
- Tailings Overflow filling rate
  - Years 0-1:13.3 million kilograms per day
  - Years 1-5:15.4 million kilograms per day
  - Years 5-11:14.2 million kilograms per day
  - Years 11-11.12:13.2 million kilograms per day
- Impermeable Bottom Boundary condition



### 3.0 INPUTS

#### 3.1 Geometry

Model geometry was based on the existing ground topography (THEMAC Resources, 2011) and Golder's optimized TSF design. Stage storage curve data used for TSF modeling is displayed in Table 1.

- Bottom Elevation: 5,196 feet above mean sea level (ft-msl)
- Top Dam Crest Elevation: 5,450 ft-msl
- Maximum depth of Impoundment: 254 feet
- Maximum Overflow Impoundment Area: 13.66 million square feet
- Impoundment volume used in the FSConsol analyses: 96.88 million cubic yards (MCY)

**Table 1: Elevation–Area-Volume Relationship**

Elevation (ft-msl)	Height (ft)	Area (acre)	Cumulative Volume (yd <sup>3</sup> )
5,196	0	0.0	0
5,210	14	16.3	367,859
5,225	29	58.6	1,786,931
5,235	39	101.7	3,427,369
5,250	54	141.8	6,859,887
5,265	69	180.3	11,223,775
5,280	84	203.4	16,145,709
5,295	99	240.6	21,968,132
5,310	114	255.1	28,141,570
5,325	129	280.1	34,919,345
5,340	144	288.9	41,911,903
5,355	159	300.0	49,171,933
5,370	174	305.0	56,552,360
5,385	189	308.6	64,019,422
5,400	204	312.6	71,583,431
5,425	229	313.6	84,231,795
5,435	239	313.6	89,291,466
5,450	254	313.6	96,880,975

#### 3.2 Material Properties

Material properties and numerical model inputs are based on laboratory tests performed by Golder Associates Inc. in Denver, CO. Slurry consolidation tests were performed to determine void ratio and permeability versus applied effective stresses. The specific gravity of the tailings material is 2.65 based on laboratory tests.

## 4.0 METHOD

Calculations were performed using the computer program FSConsol (GWP Software, 1999), which performs a one-dimensional, large-strain consolidation analysis using finite strain consolidation theory as presented in Gibson (1967).

### 4.1 Numerical Model

#### 4.1.1 Equations

For modeling purposes, it is convenient to express constitutive relationships (permeability and compressibility, respectively) in a closed form. Non-linear relationships are proposed by Abu-Hejleh and Znidarcic (1994 and 1996), defined by Equations 4.1 and 4.2, which are used in consolidation and desiccation numerical models.

$$k = C e^D \quad \text{Equation 4.1}$$

$$e = A(\sigma' + Z)^B \quad \text{Equation 4.2}$$

When using FSConsol model, Equation 4.1 remains the same; however, Equation 4.2 is rewritten by the modified power law form to represent compressibility, shown in Equation 4.3.

$$e = A(\sigma')^B + M \quad \text{Equation 4.3}$$

In the above relationships,  $e$  is the void ratio,  $\sigma'$  is the effective stress, and  $k$  is hydraulic conductivity functionally dependent on void ratio.

#### 4.1.2 Parameters

The five material parameters,  $A$ ,  $B$ ,  $C$ ,  $D$ , and  $M$  (or  $Z$ ), were determined by fitting constitutive relationships to laboratory data, as shown in Figures 1 through 4, which are on the following next two pages (pages 4 and 5). The fitted parameters calculated for those constitutive relationships are shown in Tables 2 and 3 for different systems of units.

**Table 2: Permeability Input Parameters for Cyclone Overflow and Whole Tailings**

Sample	C (centimeters per second)	D (dimensionless) <sup>1</sup>
Slimes	$1.380 \times 10^{-7}$	3.353
Beach	$1.523 \times 10^{-6}$	3.035

**Table 3: Compressibility Input Parameters for Cyclone Overflow and Whole Tailings**

Sample	A (1/kilopascals) <sup>B</sup>	B (dimensionless) <sup>1</sup>	M (dimensionless) <sup>1</sup>
Slimes	3.144	-0.1952	-0.1424
Beach	1.787	-0.2983	0.5224

Note:

<sup>1</sup> B, D, and M are dimensionless and valid for English, International, and centimeter-gram-second (cgs) units

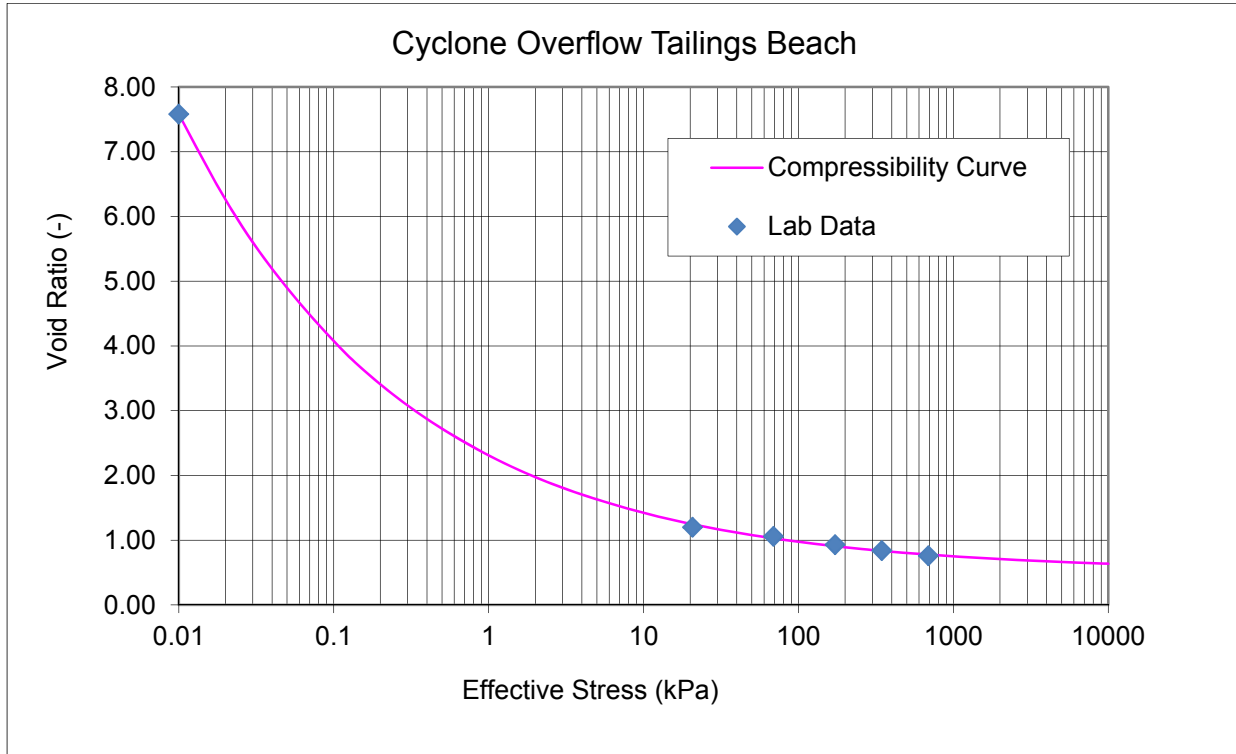


Figure 1: Cyclone Overflow Tailings Beach Compressibility Constitutive Relationship versus Lab Data

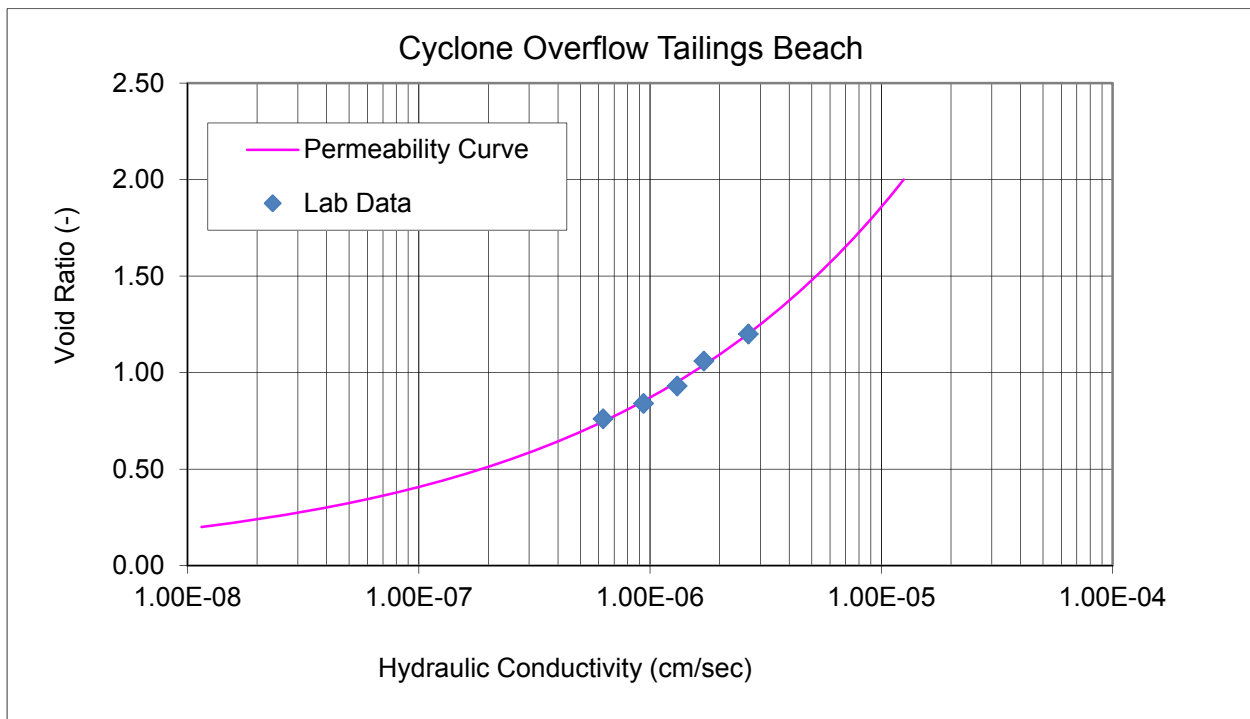


Figure 2: Cyclone Overflow Tailings Beach Permeability Constitutive Relationship versus Lab Data

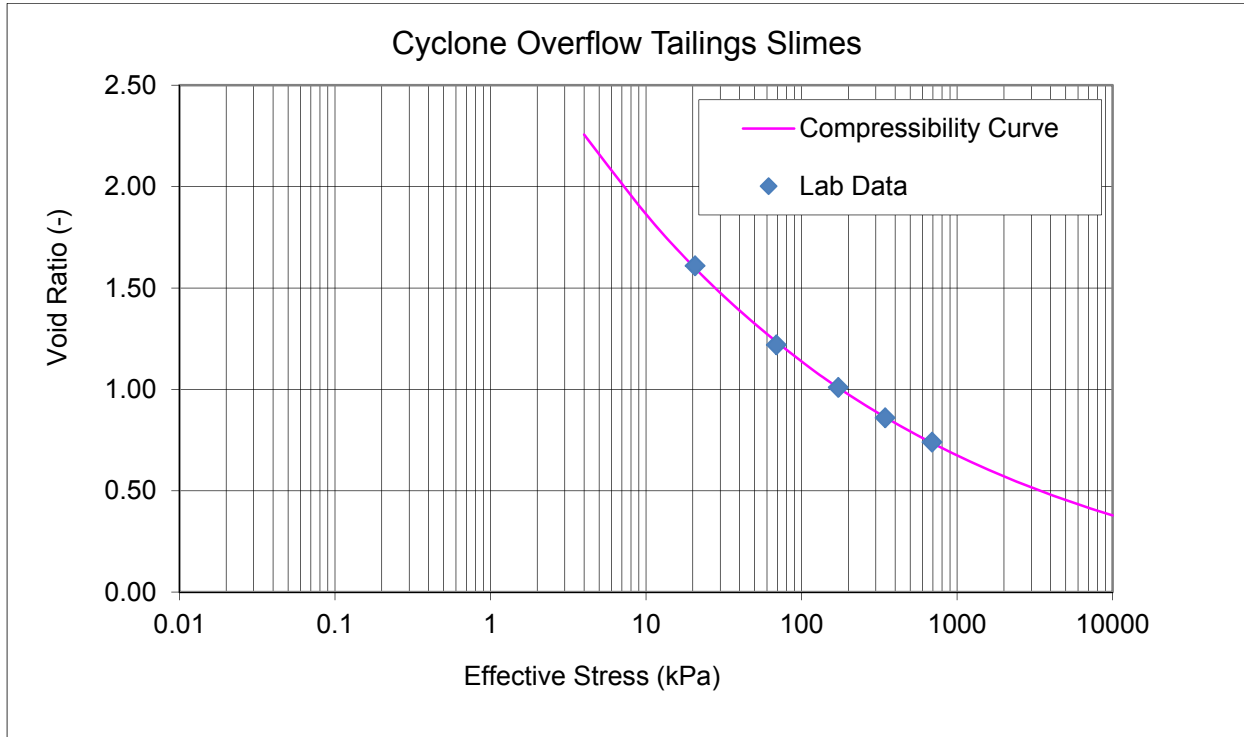


Figure 3: Cyclone Overflow Tailings Slimes Compressibility Constitutive Relationship versus Lab Data

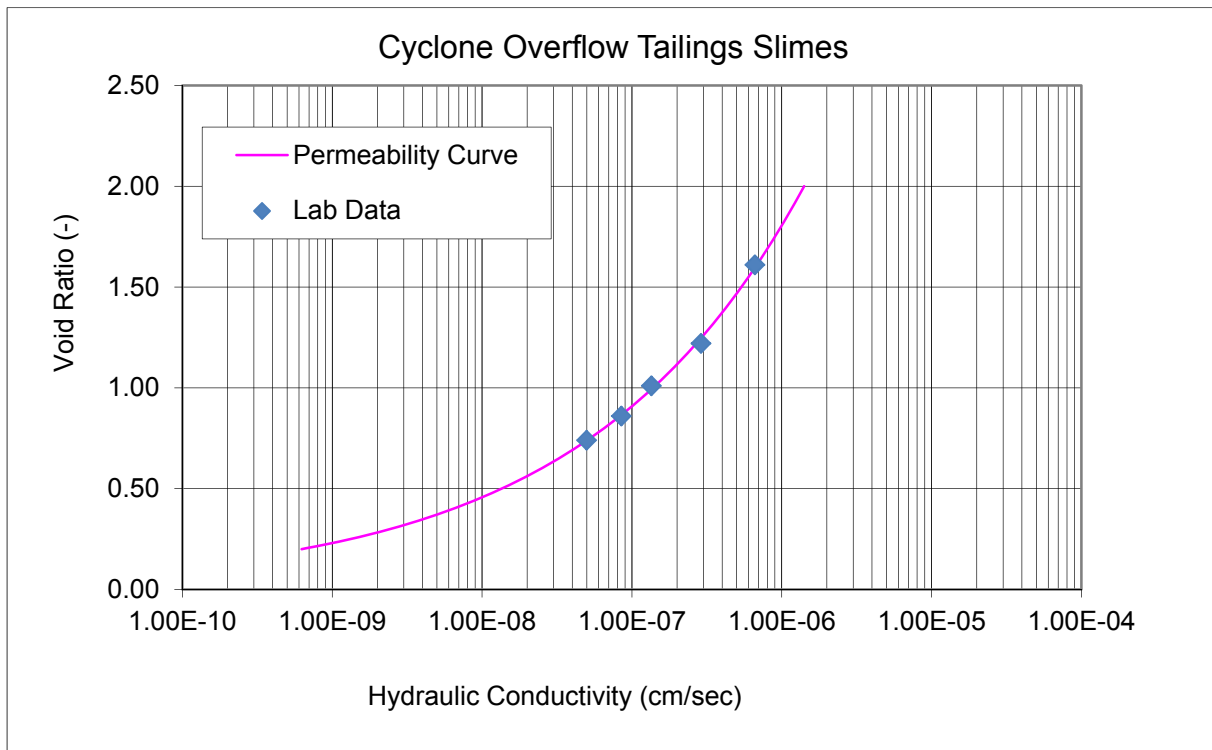


Figure 4: Cyclone Overflow Tailings Slimes Permeability Constitutive Relationship versus Lab Data

## 4.2 Procedure

The tailings cyclone overflow portion of the TSF was modeled by running a consolidation analysis on a single tailings column located in the deepest part of the impoundment. Using the parameters described above in Section 4.1.2 to define the compressibility and permeability relationships, separate analyses were performed for each material representing either the tailings beach or tailings slimes components of the cyclone overflow.

In each respective model run, the model analyzed each material component assuming it to represent 100 percent of the inflow into the model storage volume (i.e. the impoundment). In the model, the incremental area was calculated based on elevation and volume differences with Golder's TSF design volumes determined using AutoCAD Civil 3D (2013). The storage volume requirement for the TSF impoundment is estimated by applying FSConsol (1999) calculated densities from the final void ratio profile of each tailings component to obtain an overall density that is weighted based on laboratory grain-size distribution results, which is detailed further in Section 4.2.1. Furthermore, the calculated densities based on the FSConsol analyses are adjusted for 1D percent errors as discussed in Section 4.2.2.

### 4.2.1 Overflow Beach/Slimes Split

Grain-size distribution tests were performed by Golder in the Denver, CO laboratory on tailings beach and slime samples from laboratory flume tests and on total overflow tailings before the flume tests. From the grain-size distributions, the percent by weight of sand versus fines (silt and/or clay) can be used to approximate the weight or mass percentage of the tailings beach and tailings slime that will report to the respective areas within the TSF impoundment. To determine the split, the percentage summation of sand and fines from the tailings beach and tailings slimes samples should equate to the percentage for the original cyclone overflow head sample. Table 4 depicts the results of the laboratory test and the split calculation, which results in an approximate ratio of 60:40 (beach versus slimes).

**Table 4: Laboratory and Split Results**

Sample	Grain Size Distribution		Split Calculation		
	Sand (%)	Fines (%)	% Solids by Weight	% Sand of Total Overflow	% Fines of Total Overflow
Beach	15.64	84.36	59.7	9.33	50.34
Slimes	0.74	99.26	40.3	0.3	40.03
Cyclone Overflow Head	9.63	90.37	100	9.63	90.37

### 4.2.2 1D Percent Error

The consolidation model constructed in FSConsol is 1D, and does not accurately take into account the overall geometry of the TSF (i.e. "bowl" shaped), and that mass and volume quantities are larger towards the top than at the bottom of the TSF due to an increase in surface area. The approximated average dry density of each tailings component is corrected based on the calculated average percent error between 1D versus three-dimensional (3D) by checking: height of solids, volume of solids, and mass of solids.

Based on Golder's experience, 1D analysis resulting in less than an average 15 percent error reasonably depicts expected consolidation in the field, and 3D impoundment geometry can be accounted for by reducing the 1D results of estimated average dry density values by the average percent error.

## 5.0 RESULTS

The average dry density generally increases during filling until the ultimate height of tailings is reached. At closure, the tailings density continues to increase as the consolidation continues. If 100 percent of the inflow into the TSF impoundment is represented by the cyclone overflow tailings slimes samples from the tail section of the flume (i.e., the finest fraction of the tailings), FSConsol predicts that the final average density of the slimes fraction over the modeled profile will be approximately 33.55 pcf. With 100 percent of the inflow representative of cyclone overflow tailings beach materials, FSConsol predicts a final average tailings density of approximately 76.65 pcf. Attachment 1 contains FSConsol outputs for each respective material properties used in the consolidation analysis.

Because a 1D model was used, percent error calculations were performed to account for 3D effects. The error is expected to be greater for cone-shaped impoundments than for flat-based (i.e. bowl shaped) impoundments, and greater with higher rates of rise than lower rates of rise. Based on calculated material properties, the TSF geometry, and the anticipated rate of rise, Golder calculated the 1D modeling error to range between 1.1 to 13.7 percent with an average error ranging between 2.7 to 5.4 percent when considering calculated impoundment capacities. Attachment 2 contains the error calculations for each respective cyclone overflow tailings components.

After accounting for 3D characteristics of the impoundment geometry, the recommended dry density of each tailings component is 31.7 and 74.6 pcf, respectively. In addition, the FSConsol results were submitted to error checking to determine that solids and water inputs calculated by FSConsol were consistent with the delivery rates of the various components.

## 6.0 CONCLUSIONS

Based on model results and error checking, 31.7 and 74.6 pcf have been assumed, respectively, for the average dry density of the cyclone overflow tailings slimes and cyclone overflow tailings beach materials at the end-of-filling. Capacity has been estimated with 40 percent of the total represented by tailings slime materials, which is based on the calculated beach to slime ratio split discussed previously in Section 4.2.1. Therefore without consideration of 3D and managed deposition effects, the calculated weighted density of tailings slimes and tailings beach based on each component's volume within the TSF impoundment is 48.4 pcf. At this density, the TSF will require approximately 93.8 MCY of storage, which can be achieved by building to an ultimate tailings surface elevation of 5450 ft-msl and an ultimate cyclone underflow tailings sand dam crest elevation of 5460 ft-msl.

A summary for the cyclone overflow tailings beach and slimes components is located in Attachment 3, which contains the percent errors, adjusted densities, corresponding volumes, and the weighted density based on the percentage of the overall volume occupied by that tailings materials.

3D and managed deposition effects could potentially increase the post deposition density to reduce storage volume requirements. On average, a density increase in 5 pcf results in approximately 10 MCY of additional TSF impoundment storage capacity and a decrease in ultimate crest elevation by approximately 20 feet.

Actual flows into the impoundment will be mixed, with cyclone overflow tailings beach inter-fingered with slimes layers as the decant pond migrates upstream away from the embankment and points of cyclone overflow discharge are relocated. The denser and more permeable beach layers will increase the rate of consolidation in the underlying slimes material, resulting in a higher rate of consolidation than that calculated in the model. In addition, the consolidation modeling does not account for managed deposition of tailings. Thin-lift, sub-aerial deposition techniques will result in increases of dry density and a decrease in required storage volume by enhancing evaporation of exposed tailings. Managed deposition will increase consolidation of both the cyclone overflow tailings beach and slimes. However due to the height of the proposed tailings embankment and the design out-slopes, an embankment raise of several feet will also be feasible near the end of the mining operation if additional capacity is needed.

## 7.0 REFERENCES

- Abu-Hejleh, A.N., and D. Znidarcic, 1994. *Estimation of the Consolidation Constitutive Relations*, Computer Methods and Advances in Geomechanics, Siriwardane and Zaman (eds), Balkema, Rotterdam, pp. 499-504.
- Abu-Hejleh, A.N., and D. Znidarcic, 1996. *Consolidation Characteristics of Phosphatic Clays*, Journal of Geotechnical Engineering", ASCE, New-York, Vol. 122, No. 4, pp. 295-301.
- AutoCAD Civil 3D Package, Version 2013.
- Gibson, R.E., England, G. L., and Hussey, M. J. L. (1967). The Theory of One-Dimensional Consolidation of Saturated Clays. *Geotechnique*, 17: 261-273.
- GWP Software, 1999. *FSConsol User's Manual*, GWP Software Inc.
- THEMAC Resources, 2011. *2-Foot Topography*. Developed. and provided by Cooper Aerial Survey Co., June 18, 2011.

Attachments: Attachment 1- FSConsol Model Output  
Attachment 2 - FSConsol Error Checking  
Attachment 3 - FSConsol Model Summary



**ATTACHMENT 1  
FSCONSOL MODEL OUTPUT**

FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.  
Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 12:58:34  
input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-slimes.fs1

Overflow - slimes (Non-thickened)  
Revised flow rates and pond areas 11/11/2013  
CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond  
Analysis Time = 16.115 years  
Maximum Pond Height = 77.42 meters  
Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 3.144; B = -0.1952 ; Mconst = -0.1424

Permeability Parameters (cm/second)

C = 1.38e-07; D = 3.353

#### BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

#### NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

#### OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

## Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	8.952323	27.665	99.338	0.662	4.84E+08	32.2474
73	11.974489	28.534	99.338	0.662	9.69E+08	23.1181
109.5	13.962894	29.275	99.29	0.71	1.45E+09	23.1181
146	15.951077	29.826	99.268	0.732	1.94E+09	23.1181
182.5	17.505217	30.356	99.233	0.767	2.42E+09	18.1828
219	18.909844	30.834	99.199	0.801	2.91E+09	18.1828
255.5	20.314418	31.243	99.176	0.824	3.39E+09	18.1828
292	21.599814	31.627	99.153	0.847	3.87E+09	16.1217
328.5	22.7607	31.998	99.128	0.872	4.36E+09	16.1217
365	23.92199	32.331	99.109	0.891	4.84E+09	18.7619
401.5	25.394898	32.553	99.113	0.887	5.41E+09	18.7619
438	26.573655	32.822	99.103	0.897	5.97E+09	15.86
474.5	27.703532	33.079	99.093	0.907	6.53E+09	15.86
511	28.833398	33.316	99.085	0.915	7.10E+09	15.86
547.5	29.963253	33.533	99.08	0.92	7.66E+09	15.86
584	31.006515	33.755	99.073	0.927	8.23E+09	14.9583
620.5	32.029764	33.966	99.066	0.934	8.79E+09	14.9583
657	33.053007	34.164	99.06	0.94	9.35E+09	14.9583
693.5	34.076244	34.349	99.056	0.944	9.92E+09	14.9583
730	35.045301	34.535	99.051	0.949	1.05E+10	13.6253
766.5	35.910971	34.735	99.042	0.958	1.10E+10	13.6253
803	36.776635	34.924	99.035	0.965	1.16E+10	13.6253
839.5	37.642296	35.105	99.028	0.972	1.22E+10	13.6253
876	38.507953	35.276	99.023	0.977	1.27E+10	13.6253
912.5	39.37049	35.441	99.018	0.982	1.33E+10	13.207
949	40.186694	35.608	99.012	0.988	1.39E+10	13.207
985.5	41.002895	35.768	99.007	0.993	1.44E+10	13.207
1022	41.819093	35.922	99.002	0.998	1.50E+10	13.207
1058.5	42.635291	36.069	98.998	1.002	1.56E+10	13.207
1095	43.4515	36.211	98.995	1.005	1.61E+10	13.207
1131.5	44.241255	36.353	98.991	1.009	1.67E+10	12.7196
1168	45.000136	36.496	98.987	1.013	1.72E+10	12.7196
1204.5	45.759528	36.633	98.982	1.018	1.78E+10	12.7196
1241	46.519832	36.766	98.979	1.021	1.84E+10	12.7196
1277.5	47.281436	36.893	98.975	1.025	1.89E+10	12.7196
1314	48.04463	37.015	98.972	1.028	1.95E+10	12.7196
1350.5	48.798638	37.134	98.969	1.031	2.01E+10	12.5125
1387	49.541361	37.25	98.966	1.034	2.06E+10	12.5125
1423.5	50.286082	37.362	98.963	1.037	2.12E+10	12.5125
1460	51.032752	37.47	98.961	1.039	2.18E+10	12.5125
1496.5	51.7813	37.573	98.959	1.041	2.23E+10	12.5125

1533	52.53164	37.672	98.957	1.043	2.29E+10	12.5125
1569.5	53.278167	37.768	98.956	1.044	2.34E+10	12.3672
1606	54.015293	37.863	98.954	1.046	2.40E+10	12.3672
1642.5	54.754054	37.954	98.953	1.047	2.46E+10	12.3672
1679	55.494353	38.041	98.952	1.048	2.51E+10	12.3672
1715.5	56.236103	38.126	98.951	1.049	2.57E+10	12.3672
1752	56.979224	38.207	98.951	1.049	2.63E+10	12.3672
1788.5	57.720815	38.286	98.951	1.049	2.68E+10	12.2088
1825	58.448575	38.365	98.95	1.05	2.74E+10	11.2472
1861.5	59.070262	38.461	98.946	1.054	2.79E+10	11.2472
1898	59.694243	38.553	98.943	1.057	2.84E+10	11.2472
1934.5	60.319981	38.642	98.94	1.06	2.89E+10	11.2472
1971	60.947381	38.728	98.937	1.063	2.95E+10	11.2472
2007.5	61.576356	38.812	98.934	1.066	3.00E+10	11.2472
2044	62.206642	38.893	98.932	1.068	3.05E+10	11.21
2080.5	62.834453	38.972	98.929	1.071	3.10E+10	11.21
2117	63.463654	39.049	98.927	1.073	3.15E+10	11.21
2153.5	64.094161	39.124	98.926	1.074	3.21E+10	11.21
2190	64.725911	39.197	98.924	1.076	3.26E+10	11.21
2226.5	65.358846	39.268	98.923	1.077	3.31E+10	11.21
2263	65.992908	39.337	98.921	1.079	3.36E+10	11.21
2299.5	66.628047	39.404	98.92	1.08	3.41E+10	11.21
2336	67.264213	39.469	98.919	1.081	3.47E+10	11.21
2372.5	67.90136	39.532	98.919	1.081	3.52E+10	11.21
2409	68.539445	39.594	98.918	1.082	3.57E+10	11.21
2445.5	69.178426	39.654	98.918	1.082	3.62E+10	11.21
2482	69.818263	39.713	98.917	1.083	3.67E+10	11.2094
2518.5	70.458862	39.77	98.917	1.083	3.73E+10	11.2094
2555	71.100248	39.826	98.917	1.083	3.78E+10	11.2094
2591.5	71.742389	39.88	98.917	1.083	3.83E+10	11.2094
2628	72.385254	39.933	98.917	1.083	3.88E+10	11.2094
2664.5	73.028814	39.985	98.917	1.083	3.93E+10	11.2094
2701	73.67304	40.036	98.918	1.082	3.99E+10	11.2094
2737.5	74.317906	40.085	98.918	1.082	4.04E+10	11.2094
2774	74.963389	40.134	98.919	1.081	4.09E+10	11.2094
2810.5	75.609464	40.181	98.919	1.081	4.14E+10	11.2094
2847	76.256109	40.227	98.92	1.08	4.19E+10	11.2094
2883.5	76.903302	40.272	98.921	1.079	4.24E+10	11.2094





Table with 49 columns and 5000 rows. The first 49 columns contain numerical data representing various parameters, and the last column is a column of zeros. The rows are indexed from 1058.5 to 1752.

SOLIDS CONTENT (%)

Table with 49 columns and 5000 rows. The first 49 columns contain numerical data representing various parameters, and the last column is a column of zeros. The rows are indexed from 0 to 1752.









FSConsol (c) 1994-2011 - version 3.45 GWP Geo Software Inc.  
Date and Time of Analysis (mm/dd/yy) 11/11/2013 at 14:40:56  
input filename: P:\2013 Projects\133-92505 Cu Flat TSF\Consolidation\Nov 2013\OF-beach.fs1

Overflow - beach (Non-thickened)  
Revised flow rates and pond areas 11/11/2013  
CDJ

ANALYSIS CONDITIONS

Analysis Type = Pond  
Analysis Time = 16.115 years  
Maximum Pond Height = 77.42 meters  
Number of Filling Rates = 5

Rate kg/day	Period yrs
1.33E+07	1
1.54E+07	4
1.42E+07	6
1.32E+07	0.12
0.00E+00	5

Number of Pond Areas = 17

Pond Height m	Pond Area sq. km
4.27	0.07
4.57	0.24
3.05	0.41
4.57	0.57
4.57	0.73
4.57	0.82
4.57	0.97
4.57	1.03
4.57	1.13
4.57	1.17
4.57	1.21
4.57	1.23
4.57	1.25
4.57	1.26
7.62	1.27
3.05	1.27
4.57	1.27

Number of Surcharge Episodes = 0

INITIAL CONDITIONS

Initial Height = 0.00000 meters

MATERIAL PROPERTIES

Number of Soil Types = 1

Soil Type #1 duration = 16.11 years

Initial Solids Content = 25.90 %

void ratio @ initial S.C. = 7.582; effective stress @ initial S.C. = 10.00 Pascals

Specific Gravity = 2.65

Compressibility Parameters (stress in kPa)

A = 1.787; B = -0.2983 ; Mconst = 0.5224

Permeability Parameters (cm/second)

C = 1.523e-06; D = 3.035

#### BOUNDARY CONDITIONS

Bottom Boundary: Impermeable

Top Boundary: Constant Water Cap of: 0.000 m thickness

#### NUMERICAL TWEAKS

Time Step = 0.050 days

Maximum Stress Difference = 1e+06 kPa

#### OUTPUT PREFERENCES

Time Step Multiplier = 730

Time Step Units from: Time Step (days)

## Time Step Output

Elapsed Time days	Height metres	Ave.Sol.Cont. %	Ave. Normalized Ue %	Ave. Dissipation %	Total Solids (kg)	Solids Flux (kg/d/m2)
0	0	25.9	100	0	0	201.363
36.5	6.098115	39.528	96.536	3.464	4.84E+08	55.9199
73	8.737703	43.656	95.973	4.027	9.69E+08	55.9199
109.5	9.507187	47.682	94.382	5.618	1.45E+09	32.2474
146	10.476951	50.081	93.328	6.672	1.94E+09	32.2474
182.5	11.558634	51.655	92.671	7.329	2.42E+09	32.2474
219	12.236617	53.232	91.655	8.345	2.91E+09	23.1181
255.5	12.810342	54.565	90.672	9.328	3.39E+09	23.1181
292	13.436283	55.614	89.868	10.132	3.87E+09	23.1181
328.5	14.096505	56.462	89.212	10.788	4.36E+09	23.1181
365	14.780841	57.164	88.681	11.319	4.84E+09	26.904
401.5	15.714982	57.566	88.64	11.36	5.41E+09	26.904
438	16.566093	58.003	88.43	11.57	5.97E+09	21.1605
474.5	17.159336	58.556	87.926	12.074	6.53E+09	21.1605
511	17.773119	59.023	87.525	12.475	7.10E+09	21.1605
547.5	18.397756	59.433	87.186	12.814	7.66E+09	21.1605
584	19.030285	59.798	86.897	13.103	8.23E+09	21.1605
620.5	19.668975	60.125	86.649	13.351	8.79E+09	21.1605
657	20.312613	60.421	86.438	13.562	9.35E+09	21.1605
693.5	20.960293	60.689	86.258	13.742	9.92E+09	21.1605
730	21.488683	61.011	85.902	14.098	1.05E+10	18.7619
766.5	22.017166	61.298	85.604	14.396	1.10E+10	18.7619
803	22.551957	61.559	85.34	14.66	1.16E+10	18.7619
839.5	23.091308	61.8	85.105	14.895	1.22E+10	18.7619
876	23.634322	62.023	84.893	15.107	1.27E+10	18.7619
912.5	24.180393	62.229	84.703	15.297	1.33E+10	18.7619
949	24.729068	62.422	84.532	15.468	1.39E+10	18.7619
985.5	25.279985	62.603	84.378	15.622	1.44E+10	18.7619
1022	25.762592	62.813	84.108	15.892	1.50E+10	15.86
1058.5	26.168928	63.039	83.774	16.226	1.56E+10	15.86
1095	26.583726	63.244	83.478	16.522	1.61E+10	15.86
1131.5	27.003419	63.436	83.204	16.796	1.67E+10	15.86
1168	27.426795	63.616	82.95	17.05	1.72E+10	15.86
1204.5	27.853193	63.785	82.711	17.289	1.78E+10	15.86
1241	28.282173	63.946	82.487	17.513	1.84E+10	15.86
1277.5	28.713408	64.098	82.277	17.723	1.89E+10	15.86
1314	29.146638	64.242	82.08	17.92	1.95E+10	15.86
1350.5	29.581652	64.38	81.894	18.106	2.01E+10	15.86
1387	30.018268	64.511	81.72	18.28	2.06E+10	15.86
1423.5	30.42497	64.651	81.498	18.502	2.12E+10	14.9583
1460	30.82099	64.787	81.28	18.72	2.18E+10	14.9583
1496.5	31.21996	64.915	81.079	18.921	2.23E+10	14.9583

1533	31.620997	65.037	80.889	19.111	2.29E+10	14.9583
1569.5	32.023724	65.154	80.709	19.291	2.34E+10	14.9583
1606	32.42791	65.266	80.539	19.461	2.40E+10	14.9583
1642.5	32.833389	65.373	80.378	19.622	2.46E+10	14.9583
1679	33.24003	65.477	80.225	19.775	2.51E+10	14.9583
1715.5	33.647726	65.576	80.079	19.921	2.57E+10	14.9583
1752	34.056384	65.671	79.941	20.059	2.63E+10	14.9583
1788.5	34.465926	65.764	79.81	20.19	2.68E+10	14.9583
1825	34.852455	65.864	79.636	20.364	2.74E+10	12.5521
1861.5	35.144936	65.994	79.325	20.675	2.79E+10	12.5521
1898	35.447468	66.11	79.059	20.941	2.84E+10	12.5521
1934.5	35.754018	66.22	78.81	21.19	2.89E+10	12.5521
1971	36.063325	66.325	78.572	21.428	2.95E+10	12.5521
2007.5	36.374798	66.426	78.345	21.655	3.00E+10	12.5521
2044	36.688086	66.523	78.126	21.874	3.05E+10	12.5521
2080.5	37.002952	66.616	77.914	22.086	3.10E+10	12.5521
2117	37.319222	66.706	77.71	22.29	3.15E+10	12.5521
2153.5	37.636758	66.793	77.513	22.487	3.21E+10	12.5521
2190	37.955451	66.877	77.322	22.678	3.26E+10	12.5521
2226.5	38.275209	66.958	77.137	22.863	3.31E+10	12.5521
2263	38.59595	67.038	76.958	23.042	3.36E+10	12.5521
2299.5	38.917606	67.114	76.785	23.215	3.41E+10	12.5521
2336	39.240116	67.189	76.617	23.383	3.47E+10	12.5521
2372.5	39.548675	67.267	76.425	23.575	3.52E+10	12.1668
2409	39.855497	67.342	76.241	23.759	3.57E+10	12.1668
2445.5	40.163729	67.415	76.064	23.936	3.62E+10	12.1668
2482	40.473023	67.485	75.893	24.107	3.67E+10	12.1668
2518.5	40.783223	67.553	75.728	24.272	3.73E+10	12.1668
2555	41.094232	67.62	75.567	24.433	3.78E+10	12.1668
2591.5	41.405978	67.685	75.411	24.589	3.83E+10	12.1668
2628	41.718406	67.748	75.26	24.74	3.88E+10	12.1668
2664.5	42.031468	67.809	75.113	24.887	3.93E+10	12.1668
2701	42.345125	67.869	74.971	25.029	3.99E+10	12.1668
2737.5	42.65934	67.927	74.832	25.168	4.04E+10	12.1668
2774	42.974082	67.984	74.698	25.302	4.09E+10	12.1668
2810.5	43.28932	68.04	74.567	25.433	4.14E+10	12.1668
2847	43.605029	68.094	74.44	25.56	4.19E+10	12.1668
2883.5	43.918571	68.149	74.309	25.691	4.24E+10	11.7178
2920	44.213906	68.207	74.151	25.849	4.30E+10	11.7178
2956.5	44.511467	68.263	74.005	25.995	4.35E+10	11.7178
2993	44.810075	68.317	73.864	26.136	4.40E+10	11.7178
3029.5	45.109458	68.369	73.728	26.272	4.45E+10	11.7178
3066	45.409486	68.42	73.596	26.404	4.50E+10	11.7178
3102.5	45.710079	68.47	73.468	26.532	4.56E+10	11.7178
3139	46.01118	68.519	73.344	26.656	4.61E+10	11.7178
3175.5	46.312747	68.567	73.222	26.778	4.66E+10	11.7178
3212	46.614745	68.614	73.105	26.895	4.71E+10	11.7178

3248.5	46.917146	68.659	72.99	27.01	4.76E+10	11.7178
3285	47.219924	68.704	72.878	27.122	4.82E+10	11.7178
3321.5	47.523058	68.748	72.769	27.231	4.87E+10	11.7178
3358	47.826528	68.791	72.663	27.337	4.92E+10	11.7178
3394.5	48.130317	68.833	72.559	27.441	4.97E+10	11.7178
3431	48.434408	68.875	72.458	27.542	5.02E+10	11.7178
3467.5	48.730334	68.918	72.344	27.656	5.08E+10	11.527
3504	49.026746	68.96	72.236	27.764	5.13E+10	11.527
3540.5	49.323742	69.001	72.131	27.869	5.18E+10	11.527
3577	49.621177	69.04	72.029	27.971	5.23E+10	11.527
3613.5	49.918983	69.08	71.93	28.07	5.28E+10	11.527
3650	50.21712	69.118	71.834	28.166	5.34E+10	11.527
3686.5	50.515559	69.156	71.74	28.26	5.39E+10	11.527
3723	50.814276	69.193	71.649	28.351	5.44E+10	11.527
3759.5	51.113254	69.229	71.56	28.44	5.49E+10	11.527
3796	51.412476	69.264	71.473	28.527	5.54E+10	11.527
3832.5	51.71193	69.3	71.388	28.612	5.60E+10	11.527
3869	52.011603	69.334	71.305	28.695	5.65E+10	11.527
3905.5	52.311483	69.368	71.224	28.776	5.70E+10	11.527
3942	52.611561	69.401	71.146	28.854	5.75E+10	11.527
3978.5	52.911828	69.434	71.069	28.931	5.80E+10	11.527
4015	53.20818	69.467	70.986	29.014	5.85E+10	10.5821
4051.5	53.463995	69.511	70.833	29.167	5.90E+10	10.5821
4088	53.301506	69.666	69.884	30.116	5.91E+10	0
4124.5	53.129642	69.794	69.049	30.951	5.91E+10	0
4161	52.978348	69.906	68.274	31.726	5.91E+10	0
4197.5	52.839417	70.01	67.527	32.473	5.91E+10	0
4234	52.709426	70.107	66.798	33.202	5.91E+10	0
4270.5	52.586503	70.199	66.079	33.921	5.91E+10	0
4307	52.469474	70.287	65.368	34.632	5.91E+10	0
4343.5	52.357536	70.372	64.661	35.339	5.91E+10	0
4380	52.250103	70.453	63.958	36.042	5.91E+10	0
4416.5	52.14673	70.531	63.257	36.743	5.91E+10	0
4453	52.047066	70.607	62.558	37.442	5.91E+10	0
4489.5	51.950825	70.68	61.86	38.14	5.91E+10	0
4526	51.857772	70.751	61.164	38.836	5.91E+10	0
4562.5	51.767705	70.82	60.468	39.532	5.91E+10	0
4599	51.680452	70.887	59.773	40.227	5.91E+10	0
4635.5	51.59586	70.952	59.079	40.921	5.91E+10	0
4672	51.513796	71.015	58.385	41.615	5.91E+10	0
4708.5	51.43414	71.076	57.693	42.307	5.91E+10	0
4745	51.356784	71.136	57.001	42.999	5.91E+10	0
4781.5	51.281628	71.194	56.31	43.69	5.91E+10	0
4818	51.208581	71.25	55.62	44.38	5.91E+10	0
4854.5	51.137561	71.305	54.932	45.068	5.91E+10	0
4891	51.068488	71.359	54.244	45.756	5.91E+10	0
4927.5	51.001291	71.411	53.559	46.441	5.91E+10	0



4964	50.9359	71.462	52.875	47.125	5.91E+10	0
5000.5	50.872254	71.512	52.193	47.807	5.91E+10	0
5037	50.810289	71.56	51.513	48.487	5.91E+10	0
5073.5	50.749951	71.607	50.836	49.164	5.91E+10	0
5110	50.691184	71.653	50.16	49.84	5.91E+10	0
5146.5	50.633936	71.698	49.488	50.512	5.91E+10	0
5183	50.578159	71.742	48.818	51.182	5.91E+10	0
5219.5	50.523806	71.784	48.151	51.849	5.91E+10	0
5256	50.470832	71.826	47.488	52.512	5.91E+10	0
5292.5	50.419194	71.867	46.827	53.173	5.91E+10	0
5329	50.368851	71.906	46.17	53.83	5.91E+10	0
5365.5	50.319763	71.945	45.517	54.483	5.91E+10	0
5402	50.271894	71.983	44.867	55.133	5.91E+10	0
5438.5	50.225206	72.02	44.221	55.779	5.91E+10	0
5475	50.179665	72.056	43.58	56.42	5.91E+10	0
5511.5	50.135237	72.091	42.942	57.058	5.91E+10	0
5548	50.09189	72.125	42.309	57.691	5.91E+10	0
5584.5	50.049592	72.159	41.68	58.32	5.91E+10	0
5621	50.008314	72.192	41.056	58.944	5.91E+10	0
5657.5	49.968027	72.224	40.437	59.563	5.91E+10	0
5694	49.928703	72.255	39.822	60.178	5.91E+10	0
5730.5	49.890315	72.286	39.213	60.787	5.91E+10	0
5767	49.852837	72.315	38.608	61.392	5.91E+10	0
5803.5	49.816243	72.345	38.008	61.992	5.91E+10	0
5840	49.780511	72.373	37.414	62.586	5.91E+10	0
5876.5	49.745615	72.401	36.825	63.175	5.91E+10	0

Observation Point Data

Number of Observation Points: 50
Number of Observation Point Readings: 162
Observation Point Time Step: 36.50 days

EXCESS PORE PRESSURE (kPa)

Table with columns: Time, Height from Bottom (metres), and 50 observation points (days 0 to 49). Each cell contains a numerical value representing excess pore pressure at a specific time and height.

Table with 100 columns and 500 rows of numerical data.

NORMALIZED EXCESS PORE PRESSURE (%)

Table with 100 columns and 30 rows of numerical data.

Table with 45 columns and 927 rows. The first column contains numerical IDs, and the remaining columns contain various numerical values. The data is organized in a regular grid format.

Table with 48 columns and 50 rows of numerical data. The first column contains a range of values from 5146.5 to 5876.5. The remaining 47 columns contain various numerical values, some with multiple decimal places.

PORE PRESSURE (kPa)

Table with 48 columns and 50 rows. The first column is labeled 'Time' and the second column is 'Height from Bottom (metres)'. The remaining 46 columns are numerical values representing pore pressure at different heights and times.



















**ATTACHMENT 2  
FSCONSOL ERROR CHECKING**

### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

Parameters used for modeling						
H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

Check 1D Calc - Volume	
Compare volume based on filling time, production rate, & average density w/ stage-curve volume	
Time	2,883.50 days
Production	15,121.0 ton/day
Total mass	43,601,404 ton
Void Ratio	3.9303
Dry Density	33.540 pcf
Volume	2,599,975,881 ft^3
Cum Vol	2,615,786,325 ft^3
Error	-0.60%

Note: from FSConsol Time Step Data

Note: volume=Qm\*/rho\_dry

EL 5,448.31 Note: from Rate of Rise Ht vs. Cap Table

Material Properties	
Gs	2.65
A	5.6895 (1/psf)^B
B	-0.1952
M	-0.1424
e0	7.581
w	286.1%
s	25.9%
C	3.91E-04 ft/day
D	3.35

Production	
	15,121 t/day
	182,886 ft^3/day of dry solids
	1,569,366 ft^3/day of tailing
	2,883.50 days
	7.90 years

Check 1D Calc - height of solids				
Compare height of solids based on model inputs and based on the calculated void ratio profile				
Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	17.40	2.2121	4.49
2	29	36.04	0.6144	1.33
3	39	71.95	0.3543	1.48
4	54	157.93	0.2540	2.55
5	69	275.86	0.1998	2.75
6	84	407.95	0.1771	2.73
7	99	554.92	0.1497	2.56
8	114	718.78	0.1412	2.70
9	129	910.33	0.1286	2.87
10	144	1,115.33	0.1247	2.98
11	159	1,334.27	0.1201	3.06
12	174	1,557.63	0.1181	3.07
13	189	1,782.92	0.1168	3.07
14	204	2,042.42	0.1153	3.49
15	229	2,480.92	0.1149	5.87
16	239	2,654.21	0.1149	2.32
17	254	2,883.50	0.1149	3.07
Total				50.38
Hs_FSConsol				51.35
Error				1.93%

Note: based on e profile

### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

**Check 1D calc - Mass**

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = 2,883.50

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	1.434	
2	5.080	1.533	2.044
3	10.160	1.645	1.962
4	14.000	1.740	1.426
5	15.240	1.771	0.450
6	20.320	1.911	1.788
7	25.400	2.064	1.700
8	29.000	2.180	1.153
9	30.480	2.227	0.462
10	35.560	2.397	1.534
11	39.000	2.513	0.996
12	40.639	2.569	0.463
13	45.719	2.740	1.390
14	50.799	2.908	1.328
15	54.000	3.010	0.809
16	55.879	3.069	0.465
17	60.959	3.225	1.225
18	66.039	3.373	1.182
19	69.000	3.455	0.671
20	71.119	3.514	0.472
21	76.199	3.649	1.109
22	81.279	3.778	1.078
23	84.000	3.844	0.566
24	86.359	3.901	0.484
25	91.439	4.020	1.024
26	96.519	4.134	1.001
27	99.000	4.187	0.481
28	101.599	4.243	0.498
29	106.678	4.349	0.959
30	111.758	4.452	0.941
31	114.000	4.496	0.409
32	116.838	4.552	0.514
33	121.918	4.648	0.907
34	126.998	4.742	0.892
35	129.000	4.778	0.348
36	132.078	4.834	0.530
37	137.158	4.923	0.864
38	142.238	5.010	0.851
39	144.000	5.039	0.292
40	147.318	5.095	0.547
41	152.398	5.178	0.828
42	157.478	5.260	0.817
43	159.000	5.284	0.243
44	162.558	5.340	0.564
45	167.637	5.418	0.796
46	172.717	5.495	0.787
47	174.000	5.514	0.197
48	177.797	5.570	0.580
49	182.877	5.644	0.769
50	187.957	5.717	0.760
51	189.000	5.732	0.155
52	193.037	5.789	0.597
53	198.117	5.859	0.744
54	203.197	5.929	0.737
55	204.000	5.939	0.116
56	208.277	5.997	0.614
57	213.357	6.064	0.723
58	218.437	6.131	0.716
59	223.517	6.196	0.709
60	228.597	6.261	0.703
61	229.000	6.266	0.056
62	233.677	6.324	0.641
63	238.757	6.387	0.691
64	239.000	6.390	0.033
65	243.836	6.449	0.652
66	248.916	6.510	0.679
67	254.000	7.581	0.632

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
Δ Height (ft)	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
Height (ft)	14	29	39	54	69	84	99	114	129
Hs (ft)	5.43	5.09	2.99	3.99	3.54	3.22	2.99	2.81	2.66
e_avg	1.58	1.95	2.34	2.76	3.23	3.65	4.02	4.34	4.64
rho_dry (pcf)	64.17	56.12	49.47	43.99	39.06	35.55	32.96	30.95	29.33
Vol (ft^3)	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
Mass (ton)	318,662	1,075,213	1,095,477	2,038,244	2,300,832	2,362,077	2,590,583	2,579,371	2,683,614

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
Δ Height (ft)	15.0	15.0	15.0	15.0	15.0	25.0	10.0	15.0
Height (ft)	144.0	159.0	174.0	189.0	204.0	229.0	239.0	254.0
Hs (ft)	2.54	2.43	2.34	2.26	2.19	3.52	1.36	1.96
e_avg	4.91	5.16	5.40	5.62	5.84	6.10	6.33	6.64
rho_dry (pcf)	27.98	26.83	25.84	24.97	24.19	23.28	22.57	21.64
Vol (ft^3)	188,799,066	196,020,810	199,271,529	201,610,674	204,228,243	341,505,828	136,611,117	204,916,743
Mass (ton)	2,641,449	2,630,075	2,574,663	2,516,815	2,470,154	3,975,111	1,541,374	2,217,043

Time 2,883.5 days  
 Production 15,121 ton/day  
 Total mass 43,601,404 ton  
 TDF - total mass 37,610,756 ton  
 Error (%) 13.74%

Note: based on the time of filling and production rate  
 Note: based on the integration of the void ratio profile



### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

#### Parameters used for modeling

H0 (ft)	H1 (ft)	dh (ft)	Volume (ft^3)	Cum Vol (ft^3)	Area (ft^2)	q (ft/day)
5,196	5,210	14	9,932,193	9,932,193	709,442	2.2121
5,210	5,225	15	38,314,944	48,247,137	2,554,330	0.6144
5,225	5,235	10	44,291,826	92,538,963	4,429,183	0.3543
5,235	5,250	15	92,677,986	185,216,949	6,178,532	0.2540
5,250	5,265	15	117,824,976	303,041,925	7,854,998	0.1998
5,265	5,280	15	132,892,218	435,934,143	8,859,481	0.1771
5,280	5,295	15	157,205,421	593,139,564	10,480,361	0.1497
5,295	5,310	15	166,682,826	759,822,390	11,112,188	0.1412
5,310	5,325	15	182,999,925	942,822,315	12,199,995	0.1286
5,325	5,340	15	188,799,066	1,131,621,381	12,586,604	0.1247
5,340	5,355	15	196,020,810	1,327,642,191	13,068,054	0.1201
5,355	5,370	15	199,271,529	1,526,913,720	13,284,769	0.1181
5,370	5,385	15	201,610,674	1,728,524,394	13,440,712	0.1168
5,385	5,400	15	204,228,243	1,932,752,637	13,615,216	0.1153
5,400	5,425	25	341,505,828	2,274,258,465	13,660,233	0.1149
5,425	5,435	10	136,611,117	2,410,869,582	13,661,112	0.1149
5,435	5,450	15	204,916,743	2,615,786,325	13,661,116	0.1149

#### Check 1D calc - Volume

Compare volume based on filling time, production rate, & average density w/ stage-curve volume

Time	4,057.00	days
Production	15,121.0	ton/day
Total mass	61,345,897	ton
Void Ratio	1.1581	
Dry Density	76.623	pcf
Volume	1,601,244,549	ft^3
Cum Vol	1,547,514,922	ft^3
Error	3.47%	

Note: from FSConsol Time Step Data

Note: volume=Qm\*t/rho\_dry

Note: from Rate of Rise Ht vs. Cap Table

EL 5,371.53

#### Material Properties

Gs	2.65
A	4.4241 (1/psf)^B
B	-0.2983
M	0.5224
e0	7.581
w	286.1%
s	25.9%
C	4.32E-03 ft/day
D	3.04

#### Production

15,121	t/day
182,886	ft^3/day of dry solids
1,569,327	ft^3/day of tailing
4,057.00	days
11.12	years

#### Check 1D Calc - height of solids

Compare height of solids based on model inputs and based on the calculated void ratio profile

Stage	Height (ft)	Filling time (day)	q (ft/day)	Hs_production
1	14	25.54	2.2121	6.58
2	29	77.82	0.6144	3.74
3	39	200.19	0.3543	5.05
4	54	433.42	0.2540	6.90
5	69	698.40	0.1998	6.17
6	84	1,009.95	0.1771	6.43
7	99	1,401.09	0.1497	6.83
8	114	1,815.07	0.1412	6.81
9	129	2,345.37	0.1286	7.95
10	144	2,880.33	0.1247	7.77
11	159	3,434.57	0.1201	7.76
12	174	3,993.71	0.1181	7.70
13	189	4,057.00	0.1168	0.86
14	204	4,057.00	0.1153	0.00
15	229	4,057.00	0.1149	0.00
16	239	4,057.00	0.1149	0.00
17	254	4,057.00	0.1149	0.00
Total				80.56
Hs_FSConsol				81.47
Error				1.13%

Note: based on e profile

### 1-DIMENSIONAL VERSUS 3-DIMENSIONAL PERCENT ERROR CALCULATION SUMMARY

**Check 1D calc - Mass**

Calculate total mass based on the void ratio profile and TDF stage storage and compare to mass determined from production rate and total filling time

TIME = 4,057.00

Node	Elev (ft)	Void Ratio	dHs(ft)
1	0.003	0.885	
2	5.080	0.892	2.688
3	10.160	0.899	2.680
4	14.000	0.905	2.019
5	15.240	0.907	0.650
6	20.320	0.916	2.658
7	25.400	0.924	2.646
8	29.000	0.931	1.868
9	30.480	0.934	0.766
10	35.560	0.944	2.620
11	39.000	0.951	1.767
12	40.639	0.955	0.839
13	45.719	0.966	2.591
14	50.799	0.978	2.576
15	54.000	0.986	1.615
16	55.879	0.991	0.945
17	60.959	1.005	2.542
18	66.039	1.020	2.524
19	69.000	1.030	1.462
20	71.119	1.036	1.042
21	76.199	1.054	2.484
22	81.279	1.072	2.463
23	84.000	1.083	1.310
24	86.359	1.092	1.130
25	91.439	1.114	2.416
26	96.519	1.136	2.391
27	99.000	1.149	1.158
28	101.599	1.161	1.206
29	106.678	1.188	2.336
30	111.758	1.216	2.307
31	114.000	1.229	1.009
32	116.838	1.246	1.268
33	121.918	1.278	2.246
34	126.998	1.313	2.213
35	129.000	1.328	0.863
36	132.078	1.350	1.316
37	137.158	1.390	2.144
38	142.238	1.433	2.106
39	144.000	1.450	0.722
40	147.318	1.481	1.346
41	152.398	1.537	2.025
42	157.478	1.603	1.977
43	159.000	1.630	0.582
44	162.558	1.691	1.337
45	167.637	1.830	1.840
46	172.717	2.190	1.688
47	174.000	2.340	0.393
48	177.797	2.787	1.066
49	175.533	7.581	-0.366

Calculate total mass of the impoundment based on 1D void ratio profile

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5	Layer 6	Layer 7	Layer 8	Layer 9
Δ Height (ft)	14	15.0	10.0	15.0	15.0	15.0	15.0	15.0	15.0
Height (ft)	14	29	39	54	69	84	99	114	129
Hs (ft)	7.39	7.82	5.15	7.62	7.47	7.30	7.09	6.86	6.59
e_avg	0.90	0.92	0.94	0.97	1.01	1.06	1.11	1.19	1.28
rho_dry (pcf)	87.25	86.22	85.20	84.02	82.38	80.46	78.21	75.60	72.64
Vol (ft^3)	9,932,193	38,314,944	44,291,826	92,677,986	117,824,976	132,892,218	157,205,421	166,682,826	182,999,925
Mass (ton)	433,286	1,651,849	1,886,806	3,893,369	4,853,484	5,346,370	6,147,490	6,300,939	6,646,956

	Layer 10	Layer 11	Layer 12	Layer 13	Layer 14	Layer 15	Layer 16	Layer 17
Δ Height (ft)	15.0	15.0	15.0	1.5				
Height (ft)	144.0	159.0	174.0	175.5				
Hs (ft)	6.29	5.93	5.26	0.70				
e_avg	1.39	1.53	1.85	1.19				
rho_dry (pcf)	69.32	65.36	57.96	75.44				
Vol (ft^3)	188,799,066	196,020,810	199,271,529	201,610,674				
Mass (ton)	6,543,555	6,406,050	5,775,192	7,605,173				

Time	4,057	days
Production	15,121	ton/day
Total mass	61,345,897	ton
TDF - total mass	63,490,519	ton
Error (%)	-3.50%	

Note: based on the time of filling and production rate  
 Note: based on the integration of the void ratio profile

**ATTACHMENT 3  
FSCONSOL MODEL SUMMARY**

## Input Parameters

Total Mine Tailings	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Cyclone Availability	100.00%		Assumed Plant Operations
Sand Recovery	45.25%		Average Recovery from Cyclone Plant
Slimes Percentage	40.00%		Slimes vs. Beach (Overflow + Whole)
Overflow Percentage	54.75%		Tailings (Overflow + Whole)
Whole Percentage	0.00%		Tailings (Whole versus Total)
Production	27,618	t/day	Total Tailings (Underflow + Overflow)
	15,121	t/day	Tailings Overflow + Whole
	0	t/day	Tailings Whole (Beach + Slimes)
	15,121	t/day	Tailings Overflow (Beach + Slimes)

## Overflow

Production	6,048	t/day	Tailings Overflow Slimes
Production	9,073	t/day	Tailings Overflow Beach
Production	0	t/day	Tailings Whole Slimes
Production	0	t/day	Tailings Whole Beach
Capacity	6.13E+07	tons	Tailings Overflow (Beach + Slimes)
Time	4,057	days	Tailings Overflow (Beach + Slimes)
	11.115	years	

## FSConsol 1D Percent Errors

	Height	Volume	Mass	Average
OF-slimes	1.93%	0.60%	13.74%	5.42%
OF-beach	1.13%	3.47%	3.50%	2.70%
Whole-slimes	0.00%	0.00%	0.00%	0.00%
Whole-beach	0.00%	0.00%	0.00%	0.00%

## FSConsol average densities (for deepest column in TSF)

	Unadjusted	Adjusted
OF-slimes	33.55 pcf	31.73 pcf
OF-beach	76.65 pcf	74.58 pcf
Whole-slimes	0.00 pcf	0.00 pcf
Whole-beach	0.00 pcf	0.00 pcf

## Required Capacity

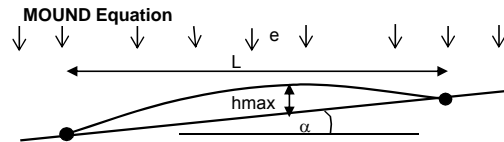
	5.07E+07	tons	Tailings Underflow Cyclone Sand
	2.45E+07	tons	Tailings Overflow Slimes
	3.68E+07	tons	Tailings Overflow Beach
	0.00E+00	tons	Tailings Whole Slimes
	0.00E+00	tons	Tailings Whole Beach
	1.12E+08	tons	Total Tailings (Underflow + Overflow)
Check	0.0000	tons	OK

## Required Volume and Weighted Density

	Unadjusted	Adjusted	Volume	Density
OF-slimes	1.46E+09 ft <sup>3</sup>	1.55E+09 ft <sup>3</sup>	61.04%	19.37 pcf
OF-beach	9.60E+08 ft <sup>3</sup>	9.87E+08 ft <sup>3</sup>	38.96%	29.05 pcf
Whole-slimes	0.00E+00 ft <sup>3</sup>	0.00E+00 ft <sup>3</sup>	0.00%	0.00 pcf
Whole-beach	0.00E+00 ft <sup>3</sup>	0.00E+00 ft <sup>3</sup>	0.00%	0.00 pcf
<b>Total</b>	<b>8.97E+07</b> yd <sup>3</sup>	<b>9.38E+07</b> yd <sup>3</sup>	<b>100.00%</b>	<b>48.42</b> pcf

**APPENDIX D  
UNDERDRAIN DESIGN CALCULATIONS**

**APPENDIX D.1  
DRAIN PIPE SPACING CALCULATIONS**



$$h_{max} = \frac{L\sqrt{C}}{2} \left[ \frac{\tan^2 \alpha}{C} + \left( 1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing  
C=Inflow Ratio (e/K)  
e=Constant Recharge Rate  
k=Drain Media Hydraulic Conductivity  
tan α=Slope Between Pipes

$h_{max}$ (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.41	45	5.07E-04	1.44E+00	1.00E-01	2.83E+02	5.07E-03	0.01

Assume sand applied over a 600 foot by 100 foot area.  
Estimate an equivalent constant recharge rate (e) for the drainage through the sand in the active sand application area  
Sand K will be higher but flow will be limited by water in underflow

**Estimate Equivalent Constant Recharge Rate (e)**

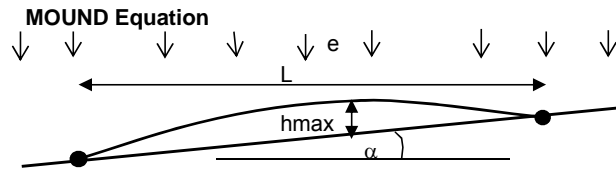
Tons milled	30,000 tpd
Cyclone recovery	46 percent
Q sand	13,800 tpd
Solids content	70 wt percent
Total underflow	19,714 tpd
Water in UF	5,914 tpd
Water in UF	1,042 gpm
Water retained	42.00%
Water lost to evap	15.00%
Seepage	448 gpm

Application area	600 L (feet)	Estimate
Application area	100 w (feet)	Estimate
Application area	60,000 ft <sup>2</sup>	

Application rate 1.66392E-05 feet/sec  
**Application rate (e) 5.07E-04 cm/sec**  
Equivalent flow rate for active sand application area

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,<sup>1</sup> Yunmin Chen,<sup>2</sup> and Chuangbing Huang<sup>3</sup>  
Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers  
J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)



$$h_{max} = \frac{L\sqrt{C}}{2} \left[ \frac{\tan^2 \alpha}{C} + \left( 1 - \frac{\tan \alpha}{C} (\tan^2 \alpha + C)^{1/2} \right) \right]$$

Where: L=Drain Spacing  
 C=Inflow Ratio (e/K)  
 e=Constant Recharge Rate  
 k=Drain Media Hydraulic Conductivity  
 tan α=Slope Between Pipes

$h_{max}$ (ft)	L (ft)	e (cm/sec)	e (ft/day)	k (cm/sec)	k (ft/day)	C	tan α
1.33	35	2.75E-07	7.80E-04	3.80E-05	1.08E-01	7.24E-03	0.01

**K drain test result 3.8e-05 cm/sec, select (native)drain fill**  
**Average of beach and slimes hydraulic conductivity applied (2.75x10<sup>7</sup>cm/sec).**

Masada, T., 1998, "Leachate Flow Mound Equations for Steady-State Flow Over a Landfill Geosynthetic Bottom Liner", Geosynthetics International, Vol. 5, No. 4, pp. 383-397.

Han Ke,<sup>1</sup> Yunmin Chen,<sup>2</sup> and Chuangbing Huang<sup>3</sup>  
 Estimation of Maximum Liquid Depth in Layered Drainage Blankets over Landfill Barriers  
 J. Envir. Engrg. Volume 134, Issue 1, pp. 67-76 (January 2008)



**APPENDIX D.2  
DRAIN PIPE DEFLECTION CALCULATIONS**

**Date:** April 23, 2013  
**Project No.:** 103-92557  
**Subject:** Underdrain Pipe Deflection Calculations

**Made by:** CDJ  
**Checked by:** GM  
**Reviewed by:** MJG

**Project  
Short Title:** COPPER FLAT FEASIBILITY DESIGN

### 1.0 OBJECTIVE

Determine the amount of deflection that will be experienced by the piping system within the tailings storage facility (TSF) underdrains caused by the surcharge load of the ultimate tailings height.

### 2.0 ASSUMPTIONS

- Underdrain Fill has a cohesion of 0 pounds per square inch (psi) and a friction angle of 39 degrees based on Golder laboratory tests for tailings underflow cyclone sand material.
- Underdrain Fill has a stress versus strain relationship similar to Silty Sand at a relative density of 80 percent per Standard AASHTO compaction specifications.
- Linear relationship between an applied vertical load of 52.1 to 69.4 psi to extrapolate the appropriate strain for an applied vertical load of 200 psi, which is the total vertical stress component imposed by the ultimate tailings height.
- The maximum burial depth is 240 feet.
- All pipes are perforated corrugated polyethylene (PCPE) N-12 pipes with a flexural modulus of 22,000 psi for a 50-year life span.
- The optimum dry density of the tailings underflow cyclone sand in the TSF is 97 pounds per cubic foot (pcf) per laboratory soil density/moisture content compaction proctor tests. As-delivered moisture content is 30 percent for a wet density of 126 pcf. After initial draindown and evaporation, moisture content of approximately 17 percent (optimum water content) for a wet density of 113.5 pcf. Therefore, use the worse-case scenario of 126 pcf for the loading conditions applied to the underdrain.
- Underdrain pipes will be subject to surface loads associated with saturated tailings overflow. The average dry density of the tailings overflow thickened slimes is estimated to be 73 pcf with a wet density of 108 pcf. The loading condition imposed by the cyclone sand dam fill represents the worst-case scenario for pipe deflection analyses.
- Maximum deflection allowed is 15 percent.

### 3.0 CALCULATION OF PIPE DEFLECTIONS

Golder analyzed the pipe stresses and deformations based on the work of Burns and Richards (Burns and Richards, 1964) and Hoeg (1968) with modifications to the closed-form, plane strain solutions by Lupo (2001). The closed form equations were modified to allow an incremental stress approach and non-linear material compression. Calculations of deflections were completed using an Excel™ spreadsheet. Calculations are included in Attachment 1.

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix D.2 Drain Pipe Deflection Calculations

**Golder Associates Inc.**  
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Tucson, AZ 85705 USA  
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**Golder Associates: Operations in Africa, Asia, Australasia, Europe, North America and South America**

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## 4.0 RESULTS

A summary of the maximum deflections of the underdrain collection system pipes within the Underdrain Fill for the TSF is shown below in Table 1.

**Table 1: Summary of Maximum Pipe Deflections**

Pipe Information	No Interface Slippage Vertical Deflection (%)	Full Slippage Vertical Deflection (%)
4-inch PCPE N-12	12.6	13.5
10-inch PCPE N-12	12.9	13.8
12-inch PCPE N-12	10.9	11.5

## 5.0 CONCLUSIONS

The calculated deflections of the 4-inch, 10-inch and 12-inch diameter PCPE N-12 pipes proposed for use in the impoundment and embankment underdrains at the Copper Flat TSF is estimated to be within the allowable limits under the anticipated worst-case loading and placement conditions. The maximum deflection is estimated to be 11 to 11.5 percent for 12-inch diameter pipe and 12.5 to 14 percent for 4-inch to 10-inch diameter pipes. The maximum allowable deflection of these pipes is 15 percent.

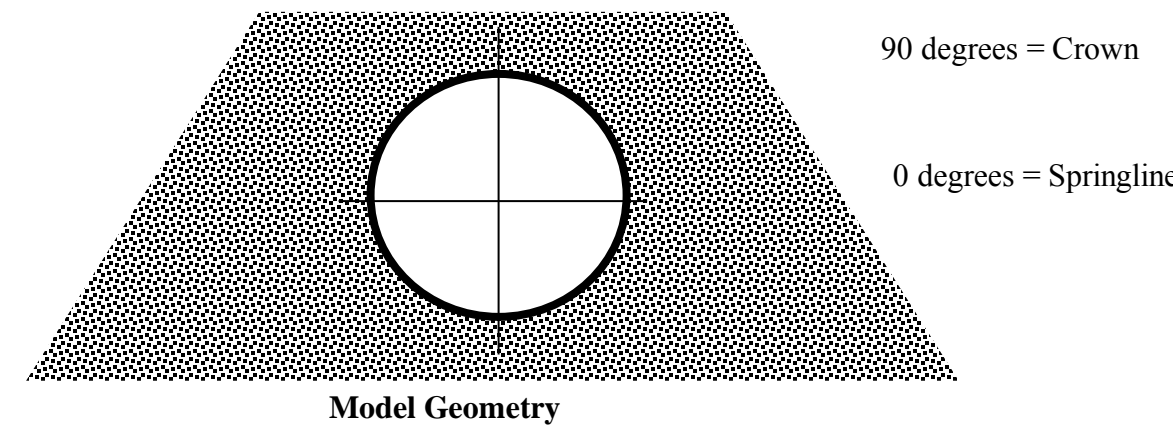
## 6.0 REFERENCES

- Burns, J.Q. and R.M. Richard, 1964. Attenuation of Stresses for Buried Conduits. Proceedings of the Symposium Soil-Structure Interaction, University of Arizona.
- Hoeg, K., 1968. Stresses Against Underground Structural Cylinders. J. Soil Mech. and Foundation Div. ASCE, Vol. 94, No. SM4.
- Lupo, J.F., 2001. Stability of HDPE Pipes Under High Heap Loads. SME Annual Meeting, Denver, Colorado. February 26-28, 2001.

**ATTACHMENT 1  
PIPE DEFLECTION CALCULATION**

**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
4 IN N-12 PIPE

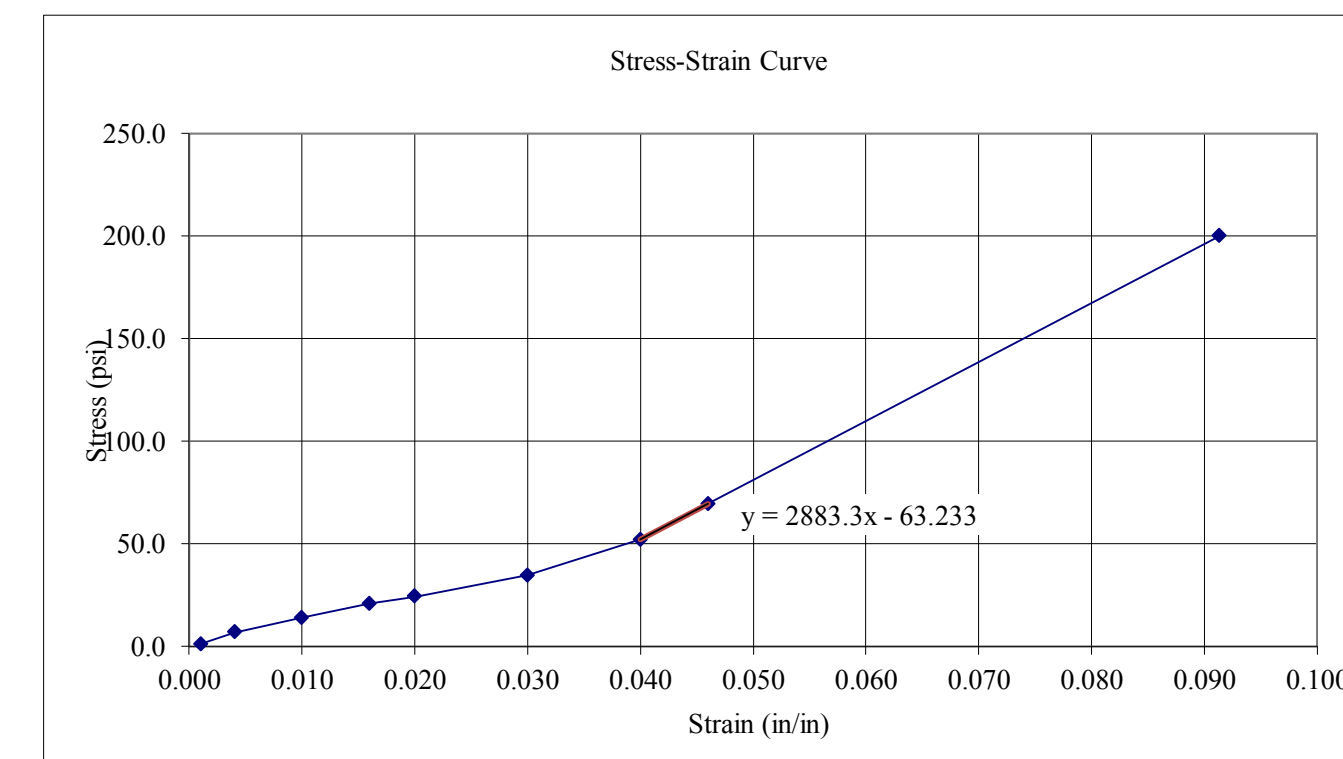
By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

**SOIL and PIPE Input Data**

Lateral Pressure Parameters						
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30
Pipe Diameter OD (in):	4.78					
Pipe ID (in):	4.10					
Weight of Pipe (lb/ft):	0.41					
Pipe Corrugated (y/n):	y		64.50			
Prescribed Constrained Modulus (y/n):	n		0.23			
Prescribed Constrained Modulus (psi):	na					
Pipe Wall Thickness (in):	0.02					
Pipe Area (in <sup>2</sup> /in):	0.07					
Flexural Modulus (psi), E <sub>r</sub> =	22,000					
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000					
C value (in)	0.29					
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	11.8					
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0014	selected I:	0.0014			
Stiffness Coefficients						
Flexural Stiffness	13.7					
Ring Compression Stiffness	647.1					
Ring Stiffness Factor:		2.3	Pipe Stiffness Less Than Soil			

Shell-Medium Parameters		
UF	4.72	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	97.0	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	2.38	
Depth of Burial (ft):	240.0	
Applied Surface Stress (psf):	0.0	
Soil Density (pcf):	120.0	
Total Vertical Stress Component (psf):	28800.0	Free Field Stress Values
Total Vertical Stress Component (psi):	200.0	



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

Critical Stress For Buckling Failure (psf): 194.7  
Ovality: 0.3  
Corrected Stress (psf): 58.4

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	119.6	308.1	0.0	-0.074	0.00E+00	280.7	7.7	4009.8	0.1823	0.0757	1593	1483	5602	5493
10	117.2	301.5	29.6	-0.054	5.92E-02	274.6	7.3	3923.3	0.1783	0.0741	1516	1412	5440	5335
20	110.3	282.5	55.6	0.005	1.11E-01	257.2	6.3	3674.5	0.1670	0.0694	1297	1208	4972	4882
30	99.7	253.4	75.0	0.095	1.50E-01	230.5	4.6	3293.2	0.1497	0.0622	961	895	4254	4188
40	86.8	217.7	85.2	0.205	1.70E-01	197.8	2.7	2825.5	0.1284	0.0533	549	511	3375	3337
50	73.0	179.7	85.2	0.322	1.70E-01	162.9	0.5	2327.8	0.1058	0.0440	110	103	2438	2431
60	60.0	144.0	75.0	0.433	1.50E-01	130.2	-1.5	1860.1	0.0846	0.0351	-302	-281	1558	1579
70	49.5	114.9	55.6	0.522	1.11E-01	103.5	-3.1	1478.9	0.0672	0.0279	-638	-594	841	885
80	42.6	95.9	29.6	0.581	5.92E-02	86.1	-4.1	1230.0	0.0559	0.0232	-857	-798	373	432
90	40.2	89.3	0.0	0.602	2.12E-17	80.1	-4.5	1143.6	0.0520	0.0216	-933	-869	210	275

Vertical Deflection (%):	12.58		149.93	
Horizontal Deflection (%):	-3.11		258.17	
Radial Soil Pressure at Crown (psi):	40.2	5788		Max. Compressive Stress (psi): 5602
Circumferential Shortening (in):	1.95			Max. Tensile Stress (psi): No Tensile Stress
Arc length of each sector (in) =	0.42			

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.9	81.3	0.0	-0.116	0.00E+00	183.4	8.4	2620.0	0.1191	0.0495	1748	1628	4368	4247
10	76.1	88.4	121.8	-0.093	1.31E-01	183.2	8.0	2617.4	0.1190	0.0494	1663	1548	4280	4165
20	76.8	108.8	229.0	-0.027	2.46E-01	182.7	6.8	2609.8	0.1186	0.0493	1416	1319	4026	3928
30	77.9	140.0	308.5	0.074	3.31E-01	181.9	5.0	2598.3	0.1181	0.0491	1039	967	3637	3566
40	79.2	178.3	350.8	0.198	3.77E-01	180.9	2.8	2584.2	0.1175	0.0488	576	536	3160	3121
50	80.6	219.1	350.8	0.330	3.77E-01	179.8	0.4	2569.2	0.1168	0.0485	83	78	2653	2647
60	81.9	257.4	308.5	0.453	3.31E-01	178.9	-1.8	2555.0	0.1161	0.0482	-379	-353	2176	2202
70	83.0	288.6	229.0	0.554	2.46E-01	178.0	-3.7	2543.5	0.1156	0.0480	-757	-705	1787	1839
80	83.7	309.0	121.8	0.620	1.31E-01	177.5	-4.8	2536.0	0.1153	0.0479	-1003	-934	1533	1602
90	83.9	316.1	0.0	0.643	4.69E-17	177.3	-5.3	2533.4	0.1152	0.0478	-1089	-1013	1445	1520

Vertical Deflection (%):	13.46
Horizontal Deflection (%):	-4.87
Radial Soil Pressure at Crown (psi):	83.9
Circumferential Shortening (in):	1.95
Arc length of each sector (in) =	0.42

12084

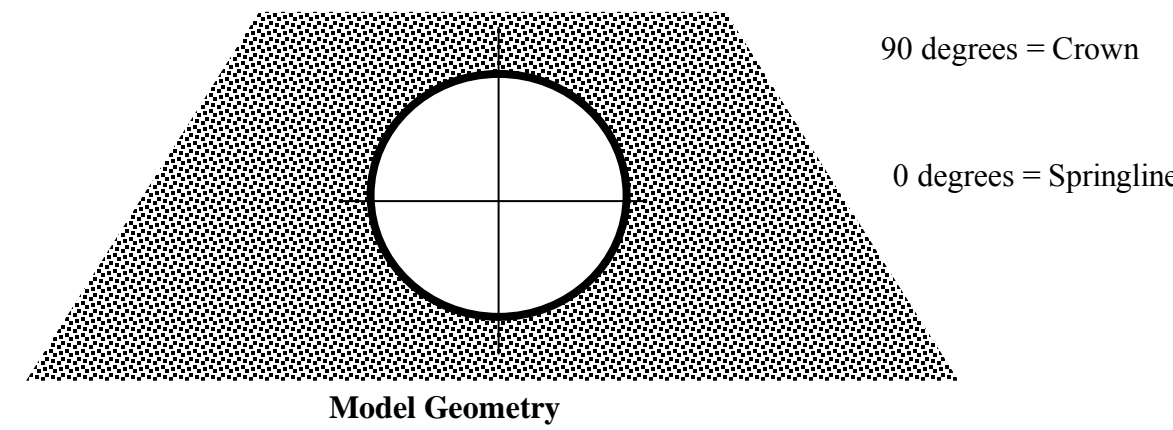
Max. Compressive Stress (psi): 4368  
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 19404 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
2.38	177.3	80.1	316.1	89.3	183.4	280.7	81.3	308.1
2.88	216.6	97.8	198.1	96.5	224.0	342.9	159.9	261.5
3.38	255.9	115.5	147.9	95.9	264.6	405.1	187.5	239.6
3.88	295.2	133.2	123.0	93.6	305.2	467.2	198.2	227.6
4.38	334.4	150.9	109.2	91.4	345.9	529.4	202.6	220.4
4.88	373.7	168.6	100.9	89.4	386.5	591.6	204.3	215.7
5.38	413.0	186.3	95.5	87.8	427.1	653.8	204.9	212.5
5.88	452.3	204.0	91.8	86.5	467.7	716.0	204.9	210.2
6.38	491.5	221.7	89.3	85.4	508.3	778.2	204.7	208.5
6.88	530.8	239.4	87.4	84.6	548.9	840.3	204.4	207.2
7.38	570.1	257.1	85.9	83.8	589.6	902.5	204.0	206.1
7.88	609.4	274.8	84.8	83.2	630.2	964.7	203.7	205.3
8.38	648.6	292.5	84.0	82.7	670.8	1026.9	203.4	204.7
8.88	687.9	310.2	83.3	82.3	711.4	1089.1	203.1	204.1
9.38	727.2	328.0	82.7	81.9	752.0	1151.3	202.9	203.7
9.88	766.5	345.7	82.2	81.6	792.6	1213.4	202.6	203.3
10.38	805.7	363.4	81.9	81.3	833.3	1275.6	202.4	203.0
10.88	845.0	381.1	81.5	81.1	873.9	1337.8	202.2	202.7
11.38	884.3	398.8	81.2	80.9	914.5	1400.0	202.1	202.4
11.88	923.6	416.5	81.0	80.7	955.1	1462.2	201.9	202.2
12.38	962.8	434.2	80.8	80.5	995.7	1524.4	201.8	202.1
12.88	1002.1	451.9	80.6	80.4	1036.3	1586.5	201.7	201.9
13.38	1041.4	469.6	80.4	80.3	1077.0	1648.7	201.6	201.7
13.88	1080.7	487.3	80.3	80.1	1117.6	1710.9	201.5	201.6
14.38	1119.9	505.0	80.2	80.0	1158.2	1773.1	201.4	201.5
Check Values:	648.6	292.5	155.8	92.0	670.8	1026.9	176.9	240.8
Soil Arching:	Positive Arch	Positive Arch	Negative Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
10 IN N-12 PIPE

By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

**SOIL and PIPE Input Data**

Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30

Pipe Diameter OD (in):	11.36
Pipe ID (in):	9.90
Weight of Pipe (lb/ft):	2.26
Pipe Corrugated (y/n):	y
Prescribed Constrained Modulus (psi):	na
Pipe Wall Thickness (in):	0.03
Pipe Area (in <sup>2</sup> /in):	0.15
Flexural Modulus (psi), E <sub>f</sub> =	22,000
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000
C value (in)	0.34
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	346.0
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0110
<b>Stiffness Coefficients</b>	
Flexural Stiffness	8.0
Ring Compression Stiffness	562.9

Lateral Pressure Parameters	
64.50	0.23
selected I:	0.0110
Ring Stiffness Factor:	1.3
Pipe Stiffness Less Than Soil	

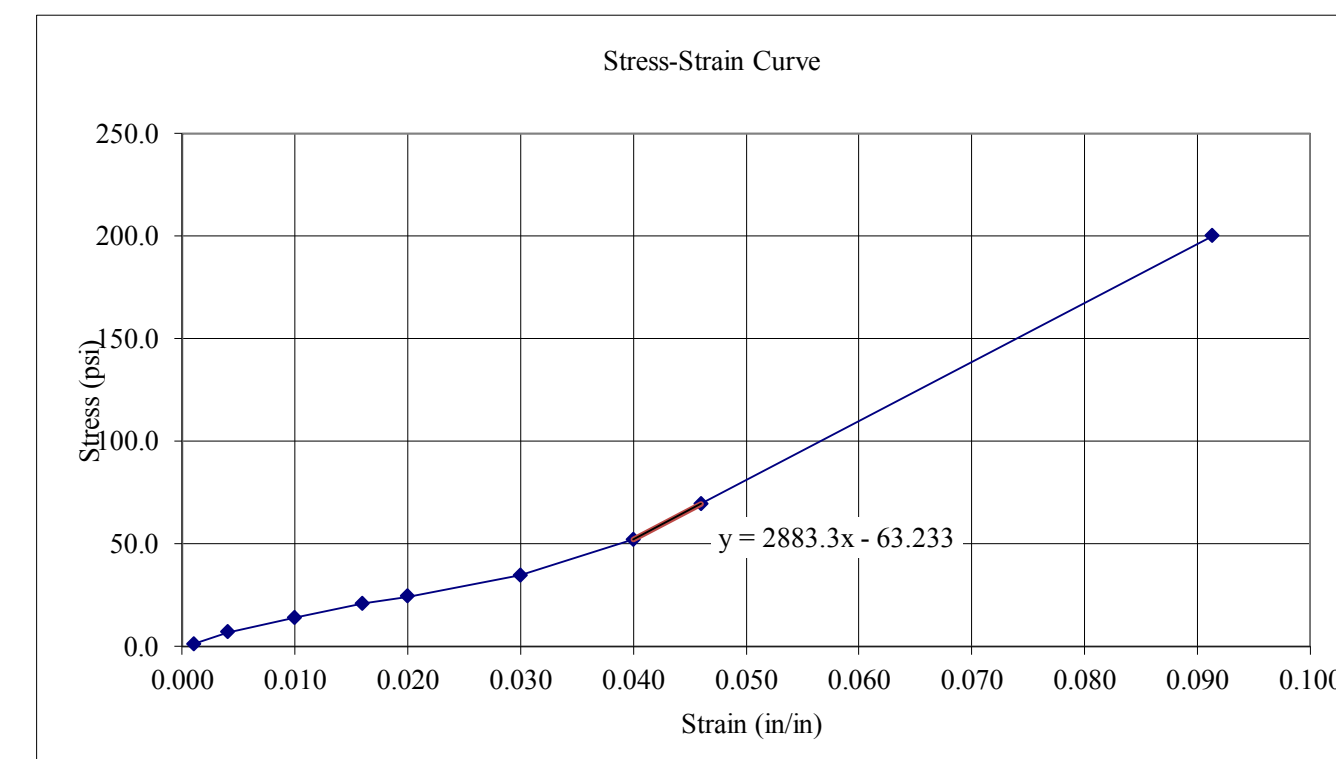
Shell-Medium Parameters	
UF	5.42
VF	166.7

Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.  
Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and tangential loading.  
If both UF and VF are zero then a perfectly rigid embedded pipe.

Pipe Mean Radius (in):	5.67
Depth of Burial (ft):	240.0
Applied Surface Stress (psf):	0.0
Soil Density (pcf):	120.0
Total Vertical Stress Component (psf):	28800.0
Total Vertical Stress Component (psi):	200.0

Free Field Stress Values

Critical Stress For Buckling Failure (psf): 642.5  
Ovality: 0.3  
Corrected Stress (psf): 192.8



Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	118.2	313.4	0.0	-0.172	0.00E+00	652.7	25.6	4501.3	0.2046	0.2024	793	734	5294	5236
10	115.8	306.6	29.3	-0.123	1.41E-01	638.5	24.4	4403.7	0.2002	0.1980	755	699	5159	5103
20	108.7	287.1	55.1	0.019	2.65E-01	597.8	20.9	4122.8	0.1874	0.1854	646	598	4769	4721
30	97.9	257.2	74.2	0.237	3.57E-01	535.4	15.5	3692.3	0.1678	0.1660	479	444	4171	4136
40	84.6	220.6	84.3	0.504	4.06E-01	458.8	8.9	3164.3	0.1438	0.1423	274	254	3439	3418
50	70.4	181.6	84.3	0.788	4.06E-01	377.3	1.8	2602.3	0.1183	0.1170	57	52	2659	2655
60	57.1	144.9	74.2	1.055	3.57E-01	300.8	-4.8	2074.3	0.0943	0.0933	-148	-137	1926	1937
70	46.3	115.1	55.1	1.273	2.65E-01	238.4	-10.2	1643.8	0.0747	0.0739	-315	-292	1329	1352
80	39.2	95.6	29.3	1.415	1.41E-01	197.6	-13.7	1362.9	0.0619	0.0613	-424	-393	939	970
90	36.8	88.8	0.0	1.464	5.05E-17	183.5	-14.9	1265.3	0.0575	0.0569	-462	-428	804	837

Vertical Deflection (%):	12.89
Horizontal Deflection (%):	-3.03
Radial Soil Pressure at Crown (psi):	36.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

5295      149.93      258.17  
Max. Compressive Stress (psi): 5294  
Max. Tensile Stress (psi): No Tensile Stress

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	75.2	82.0	0.0	-0.271	0.00E+00	422.3	28.1	2912.3	0.1324	0.1309	869	805	3781	3717
10	75.3	89.2	123.0	-0.216	3.15E-01	422.0	26.7	2910.6	0.1323	0.1309	826	766	3737	3676
20	75.7	109.9	231.1	-0.057	5.93E-01	421.3	22.8	2905.5	0.1321	0.1306	704	652	3610	3558
30	76.3	141.5	311.4	0.187	7.98E-01	420.2	16.7	2897.8	0.1317	0.1303	517	479	3415	3377
40	77.1	180.4	354.1	0.487	9.08E-01	418.8	9.3	2888.3	0.1313	0.1299	288	266	3176	3155
50	77.9	221.8	354.1	0.805	9.08E-01	417.3	1.4	2878.3	0.1308	0.1294	43	40	2922	2918
60	78.7	260.6	311.4	1.105	7.98E-01	416.0	-6.0	2868.8	0.1304	0.1290	-186	-173	2683	2696
70	79.3	292.3	231.1	1.349	5.93E-01	414.9	-12.1	2861.0	0.1300	0.1286	-373	-346	2488	2515
80	79.7	313.0	123.0	1.508	3.15E-01	414.1	-16.0	2856.0	0.1298	0.1284	-495	-459	2361	2397
90	79.8	320.2	0.0	1.564	1.13E-16	413.9	-17.4	2854.3	0.1297	0.1283	-538	-498	2316	2356

Vertical Deflection (%):	13.76
Horizontal Deflection (%):	-4.79
Radial Soil Pressure at Crown (psi):	79.8
Circumferential Shortening (in):	5.19
Arc length of each sector (in) =	0.99

11497

Max. Compressive Stress (psi): 3781  
Max. Tensile Stress (psi): No Tensile Stress

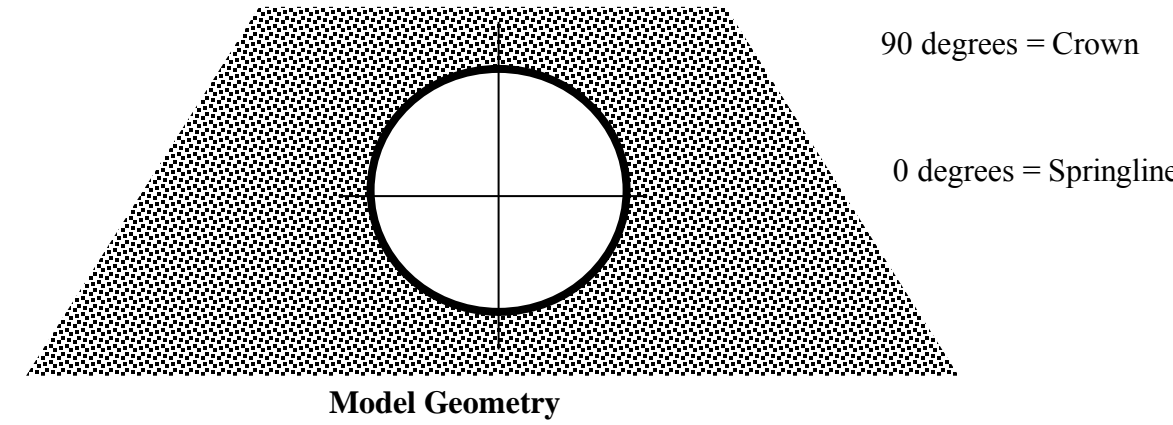
Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 33347 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
5.67	413.9	183.5	320.2	88.8	422.3	652.7	82.0	313.4
6.17	452.2	200.5	256.3	94.2	461.4	713.2	125.7	287.8
6.67	490.6	217.5	213.4	96.5	500.6	773.7	153.0	270.0
7.17	529.0	234.4	183.6	97.1	539.7	834.2	170.5	257.0
7.67	567.3	251.4	162.2	96.9	578.9	894.7	182.0	247.3
8.17	605.7	268.4	146.5	96.2	618.0	955.2	189.7	240.0
8.67	644.0	285.4	134.6	95.3	657.1	1015.8	194.9	234.2
9.17	682.4	302.4	125.5	94.3	696.3	1076.3	198.4	229.6
9.67	720.8	319.4	118.4	93.3	735.4	1136.8	200.8	225.9
10.17	759.1	336.4	112.7	92.4	774.6	1197.3	202.5	222.8
10.67	797.5	353.4	108.2	91.5	813.7	1257.8	203.6	220.3
11.17	835.9	370.4	104.4	90.6	852.9	1318.3	204.3	218.2
11.67	874.2	387.4	101.4	89.8	892.0	1378.8	204.8	216.4
12.17	912.6	404.4	98.8	89.1	931.1	1439.3	205.1	214.8
12.67	950.9	421.4	96.7	88.4	970.3	1499.8	205.2	213.5
13.17	989.3	438.4	94.8	87.8	1009.4	1560.4	205.3	212.3
13.67	1027.7	455.4	93.3	87.2	1048.6	1620.9	205.3	211.3
14.17	1066.0	472.4	91.9	86.7	1087.7	1681.4	205.2	210.4
14.67	1104.4	489.3	90.8	86.2	1126.8	1741.9	205.1	209.6
15.17	1142.7	506.3	89.7	85.8	1166.0	1802.4	205.0	208.9
15.67	1181.1	523.3	88.8	85.4	1205.1	1862.9	204.8	208.3
16.17	1219.5	540.3	88.0	85.0	1244.3	1923.4	204.7	207.8
16.67	1257.8	557.3	87.3	84.6	1283.4	1983.9	204.6	207.3
17.17	1296.2	574.3	86.7	84.3	1322.6	2044.4	204.4	206.8
17.67	1334.6	591.3	86.2	84.0	1361.7	2105.0	204.3	206.4
Check Values:	874.2	387.4	202.4	95.0	892.0	1378.8	156.8	264.2
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch



**BURIED PLASTIC PIPE LOADING WORKSHEET V2.0**

[With Incremental Stress Analysis (non-linear)]



Project: Copper Flat  
12 IN N-12 PIPE

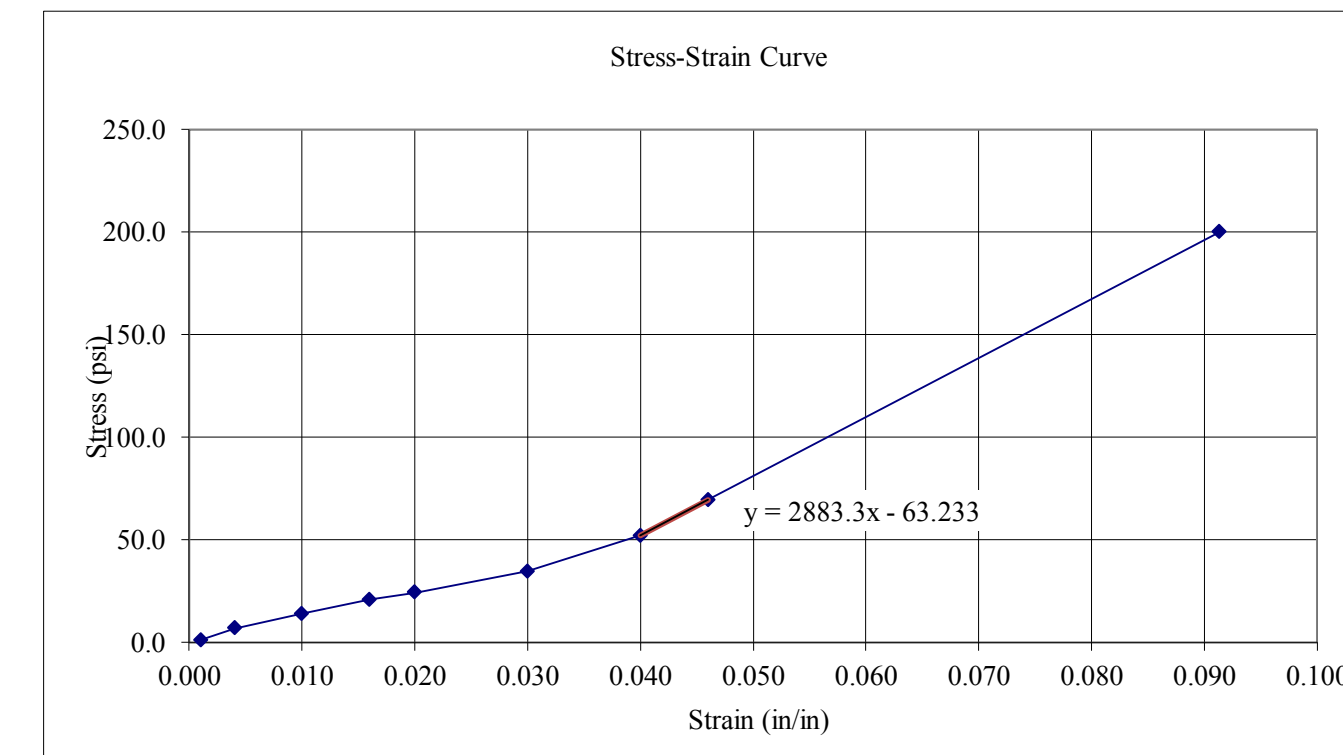
By: CDJ  
Date: 04/23/2013  
Note: Compression is positive, tension is negative

**SOIL and PIPE Input Data**

		Lateral Pressure Parameters				
Material	Cohesion	Friction Angle	Constrained Modulus (psi)	Lateral Stress Ratio	B	C
Pipe	n/a	n/a				
Soil	0	39	2191	0.39	0.696	0.30
Pipe Diameter OD (in):	14.45					
Pipe ID (in):	12.15					
Weight of Pipe (lb/ft):	3.19					
Pipe Corrugated (y/n):	y		64.50			
Prescribed Constrained Modulus (y/n):	n		0.23			
Prescribed Constrained Modulus (psi):	na					
Pipe Wall Thickness (in):	0.04					
Pipe Area (in <sup>2</sup> /in):	0.19					
Flexural Modulus (psi), E <sub>f</sub> =	22,000					
Ring Compression Modulus (psi), E <sub>rc</sub> =	22,000					
C value (in)	0.53					
Moment of Inertia (in <sup>4</sup> /in) non-corrugated:	1070.4					
Moment of Inertia (in <sup>4</sup> /in) corrugated (input from manufacturer data):	0.0410	selected I:	0.0410			
<b>Stiffness Coefficients</b>						
Flexural Stiffness	14.5					
Ring Compression Stiffness	573.8					
		Ring Stiffness Factor:	2.4	Pipe Stiffness Less Than Soil		

Shell-Medium Parameters		
UF	5.32	Extensional Flexibility ratio = Compressibility ratio = relative flexibility of pipe and soil under uniform loading.
VF	92.0	Bending Flexibility ratio = Flexibility ratio = relative flexibility of pipe and soil under varying radial and
If both UF and VF are zero then a perfectly rigid embedded pipe.		

Pipe Mean Radius (in):	7.21	
Depth of Burial (ft):	240.0	
Applied Surface Stress (psf):	0.0	
Soil Density (pcf):	120.0	
Total Vertical Stress Component (psf):	28800.0	Free Field Stress Values
Total Vertical Stress Component (psi):	200.0	



Soil Compression Data		
Applied Vertical Load (psi)	Vertical Strain (in/in)	Constrained Modulus (psi)
1.0	0.001	1000.00
6.9	0.004	1725.00
13.9	0.010	1390.00
20.8	0.016	1300.00
24.3	0.020	1215.00
34.7	0.030	1156.67
52.1	0.040	1302.50
69.4	0.046	1508.70
200.0	0.091	2190.68

Critical Stress For Buckling Failure (psf): 1883.1  
Ovality: 0.3  
Corrected Stress (psf): 564.9

**NO INTERFACE SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	97.9	343.0	0.0	-0.122	0.00E+00	693.6	63.5	3689.5	0.1677	0.2110	820	766	4510	4456
10	95.8	335.2	25.4	-0.071	1.40E-01	678.0	60.5	3606.5	0.1639	0.2062	782	730	4389	4337
20	90.0	312.9	47.7	0.075	2.64E-01	633.1	51.9	3367.6	0.1531	0.1926	671	627	4039	3994
30	80.9	278.8	64.2	0.301	3.56E-01	564.3	38.8	3001.4	0.1364	0.1716	502	468	3503	3470
40	69.9	236.9	73.0	0.577	4.04E-01	479.8	22.7	2552.3	0.1160	0.1459	293	274	2846	2826
50	58.1	192.3	73.0	0.871	4.04E-01	390.0	5.6	2074.3	0.0943	0.1186	72	67	2146	2141
60	47.1	150.4	64.2	1.147	3.56E-01	305.5	-10.5	1625.2	0.0739	0.0929	-136	-127	1489	1498
70	38.1	116.2	47.7	1.372	2.64E-01	236.7	-23.7	1259.0	0.0572	0.0720	-306	-286	953	973
80	32.2	93.9	25.4	1.519	1.40E-01	191.8	-32.2	1020.1	0.0464	0.0583	-417	-389	603	631
90	30.1	86.2	0.0	1.570	5.03E-17	176.2	-35.2	937.1	0.0426	0.0536	-455	-425	482	512

Vertical Deflection (%):	10.86			
Horizontal Deflection (%):	-1.70			
Radial Soil Pressure at Crown (psi):	30.1	4340	149.93	Max. Compressive Stress (psi): 4510
Circumferential Shortening (in):	5.29		258.17	Max. Tensile Stress (psi): No Tensile Stress
Arc length of each sector (in) =	1.26			

**FULL SLIPPAGE**

Angle	Soil Stresses (psi)			Pipe Displacements (in)		Circumferential Thrust	Moment Thrust	Ring Compression Stress (psi)	Ring Compression Strain (in/in)	Ring Shortening (in)	Inner Bending Stress (psi)	Outer Bending Stress (psi)	Total Inner Stress (psi)	Total Outer Stress (psi)
	Radial	Hoop	Shear	Radial	Hoop									
0	60.5	96.7	0.0	-0.214	0.00E+00	442.9	68.7	2356.0	0.1071	0.1347	889	830	3245	3186
10	60.7	103.8	122.2	-0.157	3.23E-01	442.4	65.4	2353.4	0.1070	0.1346	846	790	3199	3144
20	61.3	124.3	229.6	0.006	6.07E-01	441.1	56.0	2346.0	0.1066	0.1341	723	676	3069	3022
30	62.2	155.6	309.3	0.255	8.18E-01	438.9	41.4	2334.7	0.1061	0.1335	536	500	2870	2835
40	63.4	194.1	351.7	0.561	9.30E-01	436.3	23.6	2320.7	0.1055	0.1327	305	285	2626	2606
50	64.6	235.0	351.7	0.887	9.30E-01	433.5	4.6	2305.9	0.1048	0.1318	60	56	2366	2362
60	65.8	273.5	309.3	1.192	8.18E-01	430.9	-13.2	2291.9	0.1042	0.1311	-170	-159	2122	2133
70	66.7	304.9	229.6	1.442	6.07E-01	428.8	-27.7	2280.6	0.1037	0.1304	-358	-335	1922	1946
80	67.3	325.3	122.2	1.605	3.23E-01	427.4	-37.2	2273.2	0.1033	0.1300	-481	-449	1792	1824
90	67.6	332.4	0.0	1.661	1.16E-16	426.9	-40.5	2270.6	0.1032	0.1298	-523	-489	1747	1782

Vertical Deflection (%):	11.50
Horizontal Deflection (%):	-2.96
Radial Soil Pressure at Crown (psi):	67.6
Circumferential Shortening (in):	5.29
Arc length of each sector (in) =	1.26


9728

Max. Compressive Stress (psi): 3245  
Max. Tensile Stress (psi): No Tensile Stress

Free Field Stress: 200.0 psi  
Free Field Stress Times Pipe Radius: 18401 psi

Radius (in)	CROWN				SPRINGLINE			
	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)	Circumferential Thrust (full slip)	Circumferential Thrust (no slip)	Hoop Stress, psi (full slip)	Hoop Stress, psi (no slip)
7.21	426.9	176.2	332.4	86.2	442.9	693.6	96.7	343.0
7.71	458.3	189.1	278.0	93.0	475.5	744.7	131.0	316.0
8.21	489.7	202.0	238.5	96.8	508.1	795.8	154.3	295.9
8.71	521.1	215.0	209.0	98.7	540.7	846.9	170.3	280.6
9.21	552.5	227.9	186.6	99.4	573.3	897.9	181.4	268.6
9.71	583.9	240.8	169.3	99.5	605.9	949.0	189.3	259.1
10.21	615.3	253.8	155.7	99.1	638.5	1000.1	194.9	251.5
10.71	646.8	266.7	144.8	98.5	671.1	1051.2	198.9	245.2
11.21	678.2	279.6	136.0	97.7	703.7	1102.2	201.8	240.1
11.71	709.6	292.6	128.8	96.9	736.3	1153.3	203.8	235.8
12.21	741.0	305.5	122.9	96.0	768.9	1204.4	205.3	232.1
12.71	772.4	318.4	117.9	95.2	801.5	1255.5	206.3	229.0
13.21	803.8	331.4	113.7	94.3	834.1	1306.5	207.0	226.3
13.71	835.2	344.3	110.1	93.5	866.7	1357.6	207.4	224.0
14.21	866.6	357.2	107.1	92.8	899.3	1408.7	207.7	222.0
14.71	898.1	370.2	104.5	92.0	931.9	1459.7	207.8	220.2
15.21	929.5	383.1	102.2	91.4	964.4	1510.8	207.9	218.7
15.71	960.9	396.0	100.2	90.7	997.0	1561.9	207.8	217.3
16.21	992.3	409.0	98.4	90.1	1029.6	1613.0	207.7	216.1
16.71	1023.7	421.9	96.9	89.5	1062.2	1664.0	207.6	215.0
17.21	1055.1	434.8	95.5	89.0	1094.8	1715.1	207.5	214.0
17.71	1086.5	447.8	94.3	88.5	1127.4	1766.2	207.3	213.1
18.21	1118.0	460.7	93.2	88.0	1160.0	1817.3	207.1	212.3
18.71	1149.4	473.6	92.2	87.6	1192.6	1868.3	206.9	211.5
19.21	1180.8	486.6	91.3	87.2	1225.2	1919.4	206.7	210.9
Check Values:	803.8	331.4	224.2	96.1	834.1	1306.5	159.7	287.8
Soil Arching:	Positive Arch	Positive Arch	Positive Arch	Negative Arch	Positive Arch	Positive Arch	Positive Arch	Positive Arch

**APPENDIX E  
TSF UNDERDRAIN COLLECTION POND  
INFLOW ESTIMATION**

		<b>Tailings Storage Facility - Underdrain Collection Pond Inflow Estimate</b>			
<b>Client</b>	<b>New Mexico Copper Corp</b>	<b>By</b>	<b>GM/DW</b>	<b>Date</b>	<b>19-Apr-13</b>
		<b>Checked</b>	<b>GM</b>	<b>Date</b>	<b>15-Jul-13</b>
<b>Project No.</b>	<b>103-92557 Phase 011</b>	<b>Reviewed</b>	<b>MJG</b>	<b>Date</b>	

Requirements	Criteria	
1 Collect downstream embankment face and toe area runoff	100-yr 24 hour storm	3.73 inches
2 Collect Underflow drainage	Upset period	24 hours
3 Collect impoundment seepage	Upset period	24 hours
4 Provide for contact of free water pond with exposed underdrain	Upset period	24 hours

Hydraulic Conductivity-fully consolidated (from slurry consol tests)		
Product	k (cm/sec)	K (ft/sec)
Cyclone overflow beach (cob)	5.00E-07	1.64E-08
Cyclone overflow slimes (cos)	5.00E-08	1.64E-09

The above assume fully consolidated material against drain

**1) Downstream Dam Face Runoff**

Worst case prior to Final Build-out	Runoff Volume	12.1 acre-feet
	See HEC model Output	3,942,528 Gallons

**2) Underflow Drainage**

Mining Rate		30000 tpd
Underflow percentage	(Cyclone Simulation)	45.5 percent
Underflow solids content	(Cyclone Simulation)	70 percent by weight
Slurry Volume	(Cyclone Simulation)	568 tons per hour max
Water volume	(Cyclone Simulation)	260 tons per hour max

Delivered water (wt water/wt solids)	1042 gpm
Available for Drainage and Evaporation	58.00% estimate
Entrainment	42.00% estimate
Evaporative loss	15.00% estimate
Draindown	43.00%
Draindown (gpm)	448 gpm

**3) Impoundment Interior Seepage**

Assume Areal Split	Beach	60%	Slimes	40%
--------------------	-------	-----	--------	-----

	Total Area (ft <sup>3</sup> )	Beach Area	Slimes Area
Final Build-out	14,012,401	8,407,441	5,604,960

Assume unit hydraulic gradient Hydraulic conductivity based on results of slurry consolidation testing						
Beach Seepage	K floor (COB)			Slimes Seepage		
	ft <sup>3</sup> /sec	gpm	Upset (gal)	ft <sup>3</sup> /sec	gpm	upset (gal)
	0.1379	62	89,132	0.0092	4.13	5,942

**4) Seepage Through Exposed Drain**

Area exposed	20 Acre	871,200 ft <sup>3</sup>	Assumption/estimate
K drain surface layer	3.8000E-05 cm/sec	1.2467E-06 ft/sec	K drain based on permeability test result
Assume max depth 5 feet, average depth 2.5 feet	i=	2.5	
	cfs	gpm	Upset (gal)
Q	2.7154	1,219	1,754,857

<b>Summary</b>	Pond Capacity Requirements	Duration	Storage (gallons)
Dam exterior Runoff		Event	3,942,528 (HEC runoff estimate)
Beach Seepage		Upset	89,132
Slimes Seepage		Upset	5,942
Exposed impoundment Underdrain Seepage		Upset	1,754,857
Dam underdrainage		Upset	645,206
Total			6,437,666
			gpm
Normal flows		Dam drain	448 gpm
		Beach/slimes	66 gpm
		Free Water Pond seepage	1,219 gpm
		<b>Total</b>	<b>1,733 gpm</b>

**APPENDIX F**  
**HYDROLOGIC CALCULATIONS**

**APPENDIX F.1  
IMPOUNDMENT DIVERSION DITCH CALCULATIONS**

**Copper Flat Diversions, Copper Flat New Mexico**  
**Hydrologic and Hydraulic Calculations / Calculation of Time Concentration (t<sub>c</sub>)**  
**AMC II Moisture Conditions**

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile			S	L	H1	H2	Y	Lag	t <sub>c</sub>	t <sub>c</sub>	Lag	Area
	Area		Area		Area	Wt.											
	(ac)	CN	(ac)	CN	(ac)	CN	CN										
<b>Northeast- PH1</b>	66.41	85	0.00	92	0.00	50	85.0	1.76	5,318	5,537	5,317	4.13	0.50	0.84	50.5	30	0.10376
<b>Southwest- PH1</b>	47.92	85	0.00	92	0.00	50	85.0	1.76	5,835	5,452	5,307	2.48	0.70	1.17	70.2	42	0.07488
<b>SW-Periphery- PH3</b>	26.09	85	0.00	92	0.00	50	85.0	1.76	4,052	5,452	5,346	2.60	0.51	0.85	51.1	31	0.04076

**Curve Number Estimation:**

*Undisturbed Native Ground*  
 Arid and semiarid rangelands  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*Compacted Cover*  
 Arid and semiarid rangelands  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*Fill*  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*TSF*  
 OutSlope  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

*Sand*  
 Sand  
 Cover type =  
 Antecedent condition =  
 Hydrologic condition =  
 Hydrologic soil group =  
 Curve number =

**Notes:**

ac = acres  
 CN = curve number  
 Wt. CN = weighted curve number  
 S = soil and cover parameter  
 L = length of longest flow path (feet)  
 H1 = elevation at top of longest flow path (feet)  
 H2 = elevation at bottom of longest flow path (feet)  
 Y =  $H1-H2/L \times 100$  = slope (%)  
 Lag = Lag time (hours)  
 t<sub>c</sub> = time of concentraton (hr or min)

Hydrologic Condition:  
 Poor: <30% ground cover  
 Fair: 30 to 70% ground cover  
 Good: >70% ground cover

Copper Flat Diversions - Hydrologic and Hydraulic Results - PMP Storm Evaluation

**Global Summary Results for Run "PMP Evaluation"**

Project: 103-92557 Phase 1 Diversions      Simulation Run: PMP Evaluation  
 Start of Run: 23Apr2013, 00:00      Basin Model: Copper Flat  
 End of Run: 28Apr2013, 00:01      Meteorologic Model: PMP  
 Compute Time: 26Apr2013, 13:07:57      Control Specifications: 5-Day

Show Elements: All Elements      Volume Units:  IN  AC-FT      Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Northeast-1	0.10376	524.666	25Apr2013, 00:35	143.715
D-1	0.10376	524.666	25Apr2013, 00:35	143.715
Southwest-1	0.07488	339.047	25Apr2013, 00:47	103.714
D-2	0.07488	339.047	25Apr2013, 00:47	103.714
Southwest Periphery	0.04076	204.294	25Apr2013, 00:36	56.456
D-3	0.04076	204.294	25Apr2013, 00:36	56.456

**Subbasin Area [Copper Flat]**

Subbasin	Area (MI2)
Northeast-1	0.10376
Southwest-1	0.07488
Southwest Periphery	0.04076

**Curve Number Loss [Copper Flat]**

Subbasin	Initial Abstraction (IN)	Curve Number
Northeast-1		85
Southwest-1		85
Southwest Periphery		85

**SCS Transform [Copper Flat]**

Subbasin	Lag Time (MIN)
Northeast-1	30
Southwest-1	42
Southwest Periphery	31

**Precipitation**

**Met Name: PMP**  
 Probability: 0.2 Percent  
 Input Type: Partial Duration  
 Output Type: Annual Duration  
 Intensity Duration: 15 Minutes  
 Storm Duration: 4 Days  
 Intensity Position: 50 Percent  
 Storm Area (MI2): 0.7903

5 Minutes (IN):  
 \*15 Minutes (IN): 1.9500  
 \*1 Hour (IN): 8.0000  
 \*2 Hours (IN): 10.750  
 \*3 Hours (IN): 12.500  
 \*6 Hours (IN): 15.000  
 \*12 Hours (IN): 18.500  
 \*1 day (IN): 22.000  
 \*2 Days (IN): 24.500  
 \*4 Days (IN): 28.000  
 7 Days (IN):  
 10 Days (IN):

WARNING 20045: Control specifications time intervals less than duration or maximum intensity. Precipitation data will be interpolated.  
 NOTE 40040: The basin model contains 3 outlets: D-1, D-2, D-3  
 NOTE 40049: Found no parameter problems in basin model "Copper Flat".  
 NOTE 10185: Finished computing simulation run "PMP Evaluation" at time 26Apr2013, 13:07:57.



# 103-92557 Copper Flat Stormwater Diversions (PMP) Report

Label	Solve For	Friction Method	Roughness Coefficient	Channel Slope (ft/ft)
D-1 North PMP	Normal Depth	Manning Formula	0.040	0.00500
D-2 West PMP	Normal Depth	Manning Formula	0.045	0.00500
D-3 SW Periphery PMP	Normal Depth	Manning Formula	0.045	0.00500

Normal Depth (ft)	Left Side Slope (ft/ft (H:V))	Right Side Slope (ft/ft (H:V))	Bottom Width (ft)	Discharge (ft <sup>3</sup> /s)
4.52	3.50	2.00	10.00	525.00
4.15	2.00	2.00	10.00	340.00
3.20	2.00	2.00	10.00	205.00

Flow Area (ft <sup>2</sup> )	Wetted Perimeter (ft)	Hydraulic Radius (ft)	Top Width (ft)	Critical Depth (ft)
101.29	36.54	2.77	34.85	3.27
75.89	28.55	2.66	26.59	2.73
52.54	24.32	2.16	22.81	2.04

Critical Slope (ft/ft)	Velocity (ft/s)	Velocity Head (ft)	Specific Energy (ft)	Froude Number
0.01893	5.18	0.42	4.93	0.54
0.02529	4.48	0.31	4.46	0.47
0.02713	3.90	0.24	3.44	0.45

Flow Type	Notes	Messages
-----------	-------	----------

Subcritical

Subcritical

Subcritical

**APPENDIX F.2**  
**DAM AND TSF UNDERDRAIN COLLECTION POND**  
**AREA HYDROLOGIC CALCULATIONS**

**Copper Flat Dam, Copper Flat, New Mexico  
Seepage and Runoff / Calculation of Time Runoff (t<sub>c</sub>)**

**Hydrologic and Hydraulic Calculations  
AMC II Moisture Conditions**

Sub-Basin ID	Undisturbed		Compacted Cover		Stockpile												Rational method Inputs				
	Area		Area		Area	Wt.		L	H1	H2	Y	Lag	t <sub>c</sub>	t <sub>c</sub>	Lag	Area	Area	C	Intensity	Q	
	(ac)	CN	(ac)	CN	(ac)	CN	CN	S	(ft)	(ft)	(ft)	(%)	(hr)	(hr)	(min)	(min)	(mi <sup>2</sup> )	(acre)		(in/hr)	(cfs)
Phase 4	71.09	85	0.00	92	91.82	50	65.3	5.32	8,640	5,385	5,170	2.49	1.71	2.85	171.0	103	0.25454	162.91	0.38	0.907	71.1

**Curve Number Estimation:**

*Undisturbed Native Ground*

Arid and semiarid rangelands

Cover type = Desert Shrub / Pinyon-Juniper

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = C

Curve number = **85**

C = **0.43**

*Compacted Cover*

Arid and semiarid rangelands

Cover type = Fill

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = D

Curve number = **92**

C = **0.46**

*TSF*

OutSlope

Cover type = Sand

Antecedent condition = II

Hydrologic condition = Poor

Hydrologic soil group = A

Curve number = **50**

C = **0.35**

**Notes:**

ac = acres

CN = curve number

Wt. CN = weighted curve number

S = soil and cover parameter

L = length of longest flow path (feet)

H1 = elevation at top of longest flow path (feet)

H2 = elevation at bottom of longest flow path (feet)

Y = H1-H2/L x 100 = slope (%)

Lag = Lag time (hours)

t<sub>c</sub> = time of concentration (hr or min)

C = Rational Method Coefficient

C<sub>f</sub> = Correction factor for 100-year storm event (1.25)

I = rainfall intensity (inches/hour)

A = Area (acres)

Hydrologic Condition:

Poor: <30% ground cover

Fair: 30 to 70% ground cover

Good: >70% ground cover

Rational Method

Q=CiA

Q=C<sub>f</sub>CiA

i= 0.907

Includes correction factor for 100-yr storm duration.

100-year, 3 hour storm intensity

**Copper Flat Dam Toe / Seepage and Runoff  
Hydrologic and Hydraulic Results / 100-yr, 24-h Storm Volume**

**Global Summary Results for Run "100-yr, 24-hr"**

Project: 103-92557 Copper Flat    Simulation Run: 100-yr, 24-hr  
 Start of Run: 15Mar2013, 00:00    Basin Model: Basin 1  
 End of Run: 20Mar2013, 12:01    Meteorologic Model: 100 yr-24hr  
 Compute Time: 26Apr2013, 16:33:13    Control Specifications: 5 day

Show Elements: All Elements    Volume Units:  IN  AC-FT    Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
Copper Flat Outslope...	0.25454	31.790	15Mar2013, 13:53	12.101

**Subbasin Area [Basin 1]**

Show Elements: All Elements    Sorting: Hydrologic

Subbasin	Area (MI2)
Copper Flat Outslo...	0.25454

**Curve Number Loss [Basin 1]**

Show Elements: All Elements    Sorting: Hydrologic

Subbasin	Initial Abstraction (IN)	Curve Number	Impervious (%)
Copper Flat Outslo...		65.3	0.0

**SCS Transform[Basin 1]**

Show Elements: All Elements    Sorting: Hydrologic

Subbasin	Lag Time (MIN)
Copper Flat Outslo...	103

**Precipitation Summary**

Met Name: 100 yr-24hr  
 Method: Type 2  
 \*Depth (IN) 3.73

NOTE 10180: Opened meteorologic model "100 yr-24hr" at time 26Apr2013, 16:33:10.  
 NOTE 10184: Began computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.  
 NOTE 20364: Found no parameter problems in meteorologic model "100 yr-24hr".  
 NOTE 40049: Found no parameter problems in basin model "Basin 1".  
 NOTE 10185: Finished computing simulation run "100-yr, 24-hr" at time 26Apr2013, 16:33:13.

## Worksheet for Dam Toe Seepage & Runoff Collection Trench

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.012	
Channel Slope	0.02500	ft/ft
Left Side Slope	2.00	ft/ft (H:V)
Right Side Slope	2.00	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	71.10	ft <sup>3</sup> /s

### Results

Normal Depth	0.54	ft
Flow Area	5.94	ft <sup>2</sup>
Wetted Perimeter	12.40	ft
Hydraulic Radius	0.48	ft
Top Width	12.14	ft
Critical Depth	1.08	ft
Critical Slope	0.00226	ft/ft
Velocity	11.98	ft/s
Velocity Head	2.23	ft
Specific Energy	2.77	ft
Froude Number	3.02	
Flow Type	Supercritical	

### GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

### GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.54	ft
Critical Depth	1.08	ft
Channel Slope	0.02500	ft/ft

---

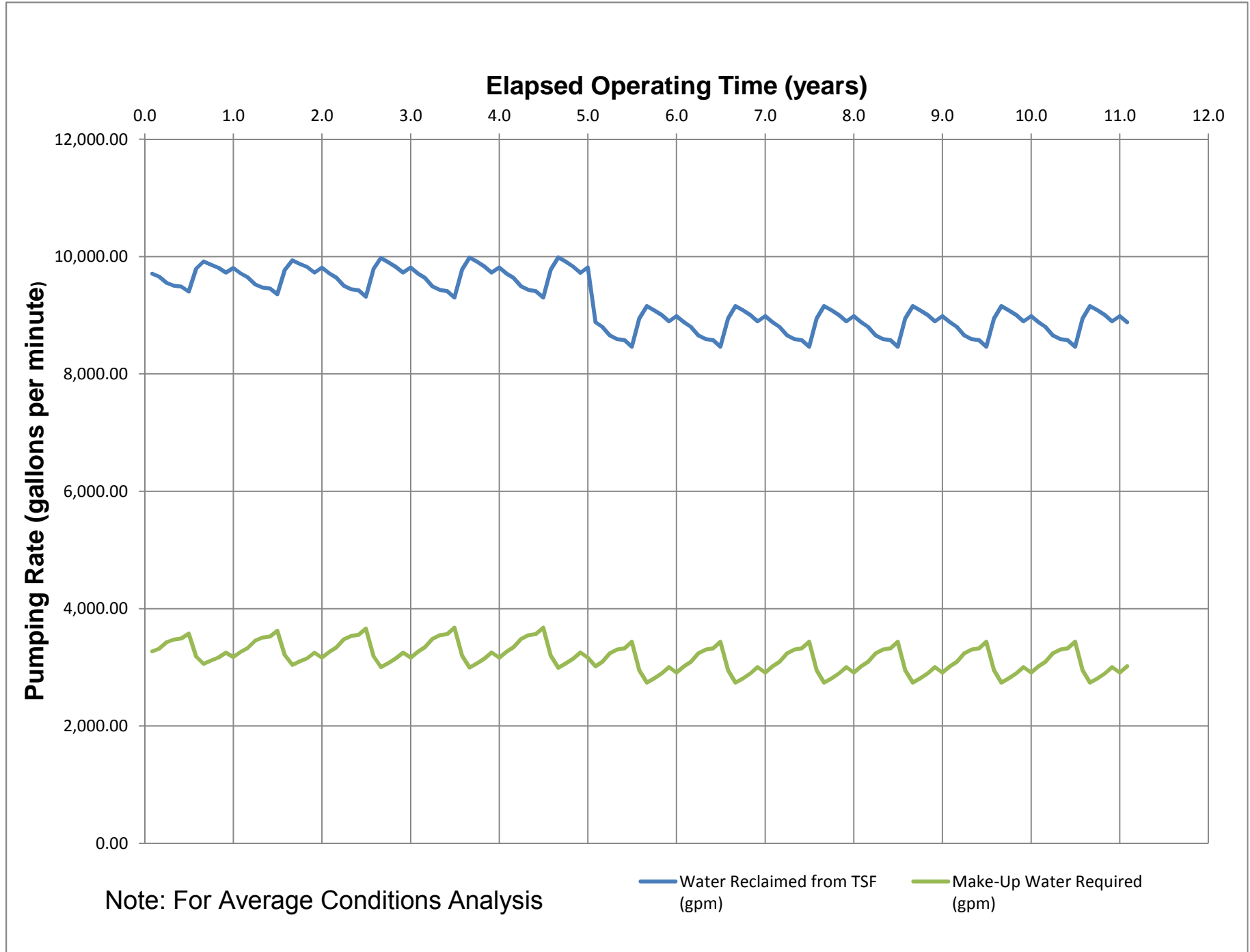
## Worksheet for Dam Toe Seepage & Runoff Collection Trench

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### GVF Output Data

Critical Slope 0.00226 ft/ft

**APPENDIX G**  
**WATER BALANCE CALCULATIONS**





**Table 1: Water Balance Calculations**

<b>Date:</b>	12/16/2013	<b>Made by:</b>	4/29/2013 RL/GM
<b>Project No.:</b>	103-92557	<b>Checked by:</b>	7/15/2013 GM
<b>Subject:</b>	Copper Flat Climate Summary	<b>Reviewed</b>	7/15/2013 MG
<b>Project Short Title:</b>	<b>Copper Flat Feasibility Study, New Mexico</b>	<b>Page:</b>	1 of 3

Elevation at the site is approximately 5180-5270 feet.

**Average Precipitation (mm)**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,270.00	4.00
Caballo Dam (1936-2010)	0.42	0.38	0.27	0.24	0.37	0.70	1.80	2.03	1.55	0.89	0.44	0.56	9.65	4,190.00	12.30
Copper Flat Site Estimate	0.58	0.56	0.38	0.33	0.53	0.72	2.33	2.46	2.11	1.18	0.55	0.81	12.54	5,200.00	

**Average Days with Rain**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
Hillsboro (1893 - 2010)	3	3	2	2	3	3	9	9	6	4	2	3	49	5,270.00	4.00
Caballo Dam (1936-2010)	3	2	2	1	2	3	8	9	6	4	2	3	45	4,190.00	12.30
Copper Flat Site Estimate	3	3	2	2	3	3	9	9	6	4	2	3	49	5,200.00	

**Local Normal Evaporation (mm)**

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Altitude (ft)	Approx Distance to Site (mi)
														4,190.00	
Copper Flat Site Estimate	4.22	5.57	8.55	9.52	11.15	14.25	10.34	5.94	6.18	3.96	3.68	2.74	86.10	5,385.00	

Table 2: Water Balance Calculations Year 1-5 30,000 tpd mine plan

<b>Date:</b> 12/16/2013 <b>Project No.:</b> 103-92557 <b>Subject:</b> Tailings Properties and Flow rates <b>Project Short Title:</b> Copper Flat Feasibility Study, New Mexico		<b>Made by:</b> 4/29/2013 RL <b>Checked by:</b> 11/15/2013 GM <b>Reviewed:</b> 11/15/2013 BNS <b>Page:</b> 2 of 3	
<b>Year 1-5</b> Design Discharge Rate 1,333 tph Cyclone Recovery 608.47 tph Krebs		Net (post concentrate recovery) 1316.0 tph Factor for average TSF inputs 99%	
<b>Tailings Discharge</b> Overflow 725 tph Total 1333 tph		Net (post concentrate recovery) 1316.0 tph Factor for average TSF inputs 99%	
Cyclone feed solids 29.10% Year 1-5 delivery 31,992 tpd SG 2.64		<b>Cyclone Discharge</b> UF Fraction (wt%) 45.60% UF Solids content (wt%) 70.00% COF Solids content (wt%) 19.52% Beach to Slimes Split 60% Krebs =input values (Krebs) Estimate as beach	
<b>Cyclone Feed</b> Total (wt) 109,938 tpd Water (wt) 77,946 tpd Solids (wt) 31,992 tpd		<b>Underflow Tailings</b> Total (wt) 20,862 tpd Water (wt) 6,258.55 tpd Solids (wt) 14,603 tpd	
14,996 gpm 12,978 gpm 2,018 gpm		1,963 gpm 1,042 gpm 921 gpm	
<b>Cyclone Overflow (Slimes)</b> Total (wt) 35,633 tpd Water (wt) 28,677 tpd Solids (wt) 6,955 tpd		5,213 gpm 4,775 gpm 439 gpm	
<b>Cyclone Overflow (Beach)</b> Total (wt) 53,449 tpd Water (wt) 43,016 tpd Solids (wt) 10,433 tpd		7,601 gpm 7,162 gpm 439 gpm	
<b>Post Deposition</b> Dry Unit Weight 95.00 pcf Vsolids 0.58 ft^3 Vvoids 0.42 ft^3 Void ratio 0.73 Saturated Moisture (wt%) 28% Assumed Moisture (wt%) 18% Vwater 0.27 ft^3 Total weight 112.10 lbs Entrained Water 84,250 ft^3/day Drainage and Evaporation 604.42 gpm Evaporation 156.31 gpm Drainage 448.11 gpm		<b>Post Deposition</b> Dry Unit Weight 31.70 pcf (FSCONSOL) Vsolids 0.19 ft^3 Vvoids 0.81 ft^3 Void ratio 4.20 Saturated Moisture (wt%) 159% Assumed Moisture (wt%) 159% Vwater 0.81 ft^3 Total weight 82.09 lbs Entrained Water 354,388 ft^3/day Free Water 2,934 gpm	
<b>Post Deposition</b> Dry Unit Weight 74.60 pcf (FSCONSOL) Vsolids 0.45 ft^3 Vvoids 0.55 ft^3 Void ratio 1.21 Saturated Moisture (wt%) 46% Assumed Moisture (wt%) 46% Vwater 0.55 ft^3 Total weight 108.74 lbs Entrained Water 153,045 ft^3/day Free Water 6,367 gpm			
<b>Year 6-11 delivery</b> 29,328 tpd SG 2.64		Krebs =input values (Krebs) Estimate as beach	
<b>Tailings Discharge</b> Design Discharge Rate 1,222 tph Cyclone Recovery 552.15 tph Krebs		Net (post concentrate recovery) 1206.0 tph Factor for average TSF inputs 99%	
Overflow 670 tph Total 1222 tph		Net (post concentrate recovery) 1206.0 tph Factor for average TSF inputs 99%	
Cyclone feed solids 29.10% Year 6-11 delivery 29,328 tpd SG 2.64		<b>Cyclone Discharge</b> UF Fraction (wt%) 45.20% UF Solids content (wt%) 70.00% COF Solids content (wt%) 19.64% Beach to Slimes Split 60% Krebs =input values (Krebs) Estimate as beach	
<b>Cyclone Feed</b> Total (wt) 100,784 tpd Water (wt) 71,456 tpd Solids (wt) 29,328 tpd		<b>Underflow Tailings</b> Total (wt) 18,931 tpd Water (wt) 5,679.26 tpd Solids (wt) 13,252 tpd	
13,747 gpm 11,897 gpm 1,850 gpm		1,781 gpm 946 gpm 836 gpm	
<b>Cyclone Overflow (Slimes)</b> Total (wt) 32,742 tpd Water (wt) 26,312 tpd Solids (wt) 6,431 tpd		4,786 gpm 4,381 gpm 406 gpm	
<b>Cyclone Overflow (Beach)</b> Total (wt) 49,113 tpd Water (wt) 39,467 tpd Solids (wt) 9,646 tpd		6,977 gpm 6,571 gpm 406 gpm	
Water in 182,027 ft^3/day		Water in 843,320 ft^3/day	
<b>Post Deposition</b> Dry Unit Weight 95.00 pcf Vsolids 0.58 ft^3 Vvoids 0.42 ft^3 Void ratio 0.73 Saturated Moisture (wt%) 28% Assumed Moisture (wt%) 18% Vwater 0.27 ft^3 Total weight 112.10 lbs Entrained Water 76,452 ft^3/day Drainage and Evaporation 548 gpm Evaporation 142 gpm Drainage 406.63 gpm		<b>Post Deposition</b> Dry Unit Weight 31.70 pcf (FSCONSOL) Vsolids 0.19 ft^3 Vvoids 0.81 ft^3 Void ratio 4.20 Saturated Moisture (wt%) 159% Assumed Moisture (wt%) 159% Vwater 0.81 ft^3 Total weight 82.09 lbs Entrained Water 327,642 ft^3/day Free Water 2,679 gpm	
<b>Post Deposition</b> Dry Unit Weight 74.60 pcf (FSCONSOL) Vsolids 0.45 ft^3 Vvoids 0.55 ft^3 Void ratio 1.21 Saturated Moisture (wt%) 46% Assumed Moisture (wt%) 46% Vwater 0.55 ft^3 Total weight 108.74 lbs Entrained Water 141,495 ft^3/day Free Water 5,836 gpm			

Table 3: Water Balance Calculations Rev 1.

Header information table including Date (12/16/2013), Project No. (103-9257), Subject (Average Year Rainfall Conditions), Project Short Title (Copper Flat Feasibility Study, New Mexico), Made by (4/29/2013 RL), Checked (11/15/2013 GM), Reviewed (11/15/2013 BNS), and Page (3 of 3).

Precipitation and Evaporation Data table with columns for Month, Days, Precip. (in), Pan evap. (in), Storms/Month, Precip/Storm, S, Runoff/storm (in), and Monthly runoff (in). Includes Average Make-up (gpm) calculation.

Variables used in Water Balance table listing parameters like Water in Delivered Tailings Year 1-5 (12,978 gpm), Beach Loss evap factor (50% of Pan Evap), and Dam Evap (underflow water) for Year 1-5 and Year 6-11.

Notes: Tailings direct precip. reports 100% to water balance. Impoundment refers to the interior, cyclone overflow and whole tailings storage area. This model does not consider whole tailings discharge which accounts for approximately 7.5% of inflow, except as noted below.

Main Water Balance Calculations table with columns for Year, Month, Elapsed Years, Total Impoundment Area (ft²), Pond Area (ft²), Beach Area (ft²), Water Inflows (Tailings Water, Direct Precip., Runon from undiverted Area), Water Losses (Total Entrained Water, Free Water Pond Evap, Impoundment Beach Evap, Embankment Evap), Monthly Balance at TSF, Water Reclaimed from TSF, End of Month Water Storage, Water Reclaimed from TSF (gpm), and Make-Up Water Required (gpm).

Table 3: Water Balance Calculations Rev 1.

Table with metadata including Date (12/16/2013), Project No. (103-92557), Subject (Average Year Rainfall Conditions), Project Short Title (Copper Flat Feasibility Study, New Mexico), Made by (4/29/2013 RL), Checked (11/15/2013 GM), Reviewed (11/15/2013 BNS), and Page (3 of 3).

Precipitation and Evaporation Data

Table with 10 columns: Month, Days, Precip. (in), Pan evap. (in), Storms/Month, Precip/Storm, S, Runoff/storm (in), Monthly runoff (in). Rows include months from January to December.

Average Make-up (gpm)

Summary table for Average Make-up (gpm) with values: Result (3,169), Entrainment (2924), Precip/runon (2098), Evap (4931), Water in (12438).

Notes:

Tailings direct precip. reports 100% to water balance. Impoundment refers to the interior, cyclone overflow and whole tailings storage area. This model does not consider whole tailings discharge which accounts for approximately 7.5% of inflow, except as noted below. Average post deposition densities reflect periodic whole tailings discharge.

Variables used in Water Balance

Table listing variables for water balance such as Water in Delivered Tailings Year 1-5 (12,978 gpm), Beach Loss evap factor (50% of Pan Evap), Net discharge rate factor (99.0%), and various evaporation and storage parameters.

Table with columns: elev, Phase, Time, Total Imp. Area, Lined Area (ft²), Undiverted Area (ft²). Rows show elevation points from 5250 to 5435.

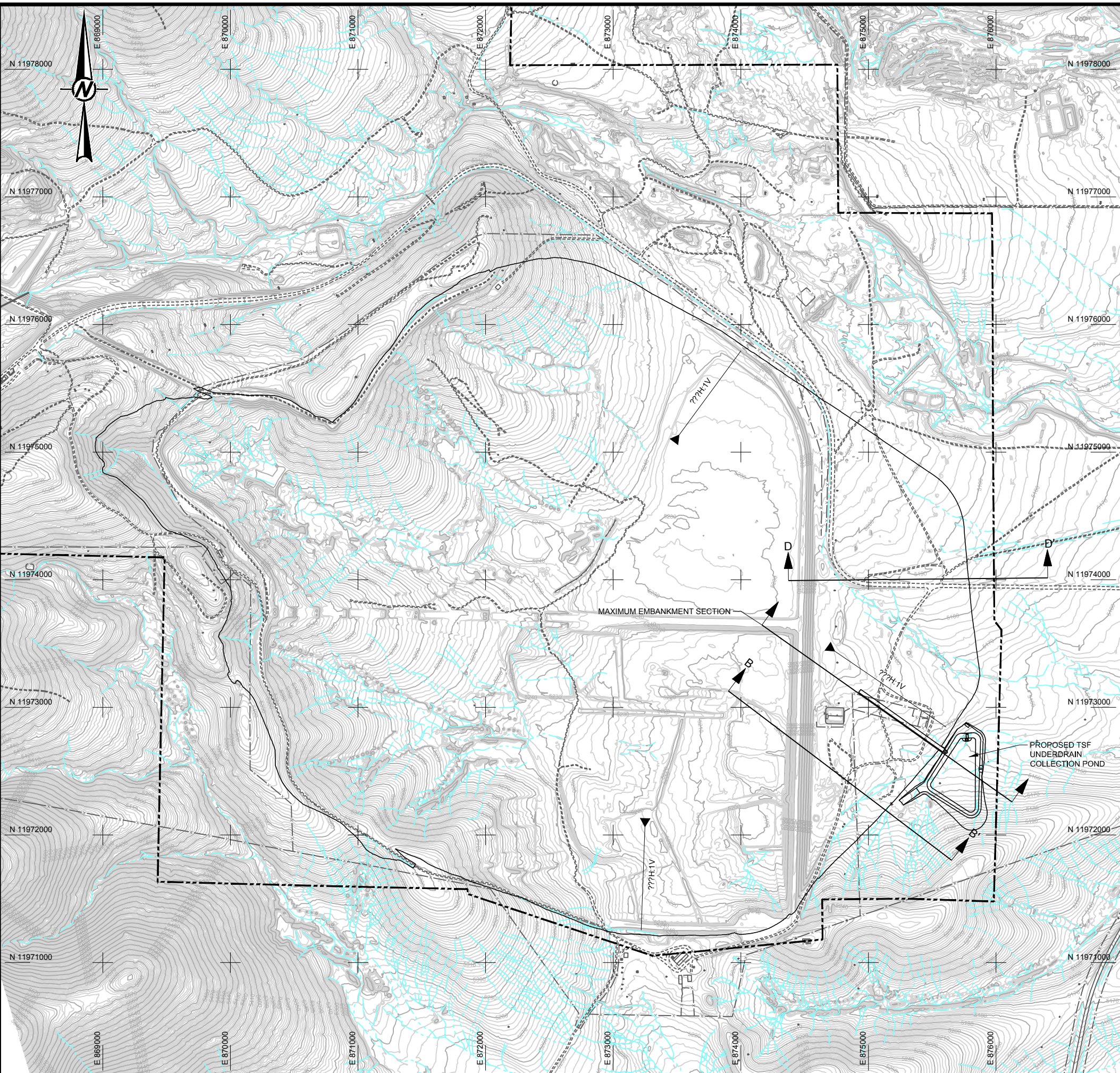
Results: Average Reclaim (9,215 gpm), Average Make-up (3,169 gpm). Total input values = 9,215 + 3,169 = 12,384 gpm.

Main Water Balance Table with columns: Year, Month, Elapsed Years, Total Impoundment Area (ft²), Pond Area (ft²), Beach Area (ft²), Water Inflows (Tailings Water, Direct Precip., Runon from undiverted Area), Water Losses (Total Entrained Water, Free Water Pond Evap, Impoundment Beach Evap., Embankment Evap.), Monthly Balance at TSF, Water Reclaimed from TSF, End of Month Water Storage, Water Reclaimed from TSF (gpm), Make-Up Water Required (gpm). Rows are grouped by year (10, 11, 12).

Average (gpm): 12,368 (Tailings Water), 186 (Direct Precip.), 3 (Runon), 2,909 (Total Entrained Water), 133 (Free Water Pond Evap), 165 (Impoundment Beach Evap.), 147 (Embankment Evap.), 9,215 (Monthly Balance at TSF), 3,169 (Water Reclaimed from TSF), 3,676 (End of Month Water Storage).

**APPENDIX H  
STABILITY ANALYSIS SUPPORTING DATA  
AND COMPUTER OUTPUT**

P:\ABO Projects\2015 Projects\1531453 THE MAC DP Permit Support\Supporting Documentation\Vol 1\Facility Design\10392557\202\_10222015.dwg | Layout: H.1 STABILITY SECTION LOCATION PLAN | Modified: C:\MONTVOYA\_10222015\_845.AVI | Plotted: C:\MONTVOYA\_10222015

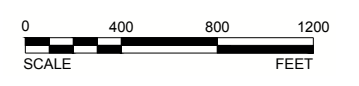


**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE STABILITY SECTION LOCATION

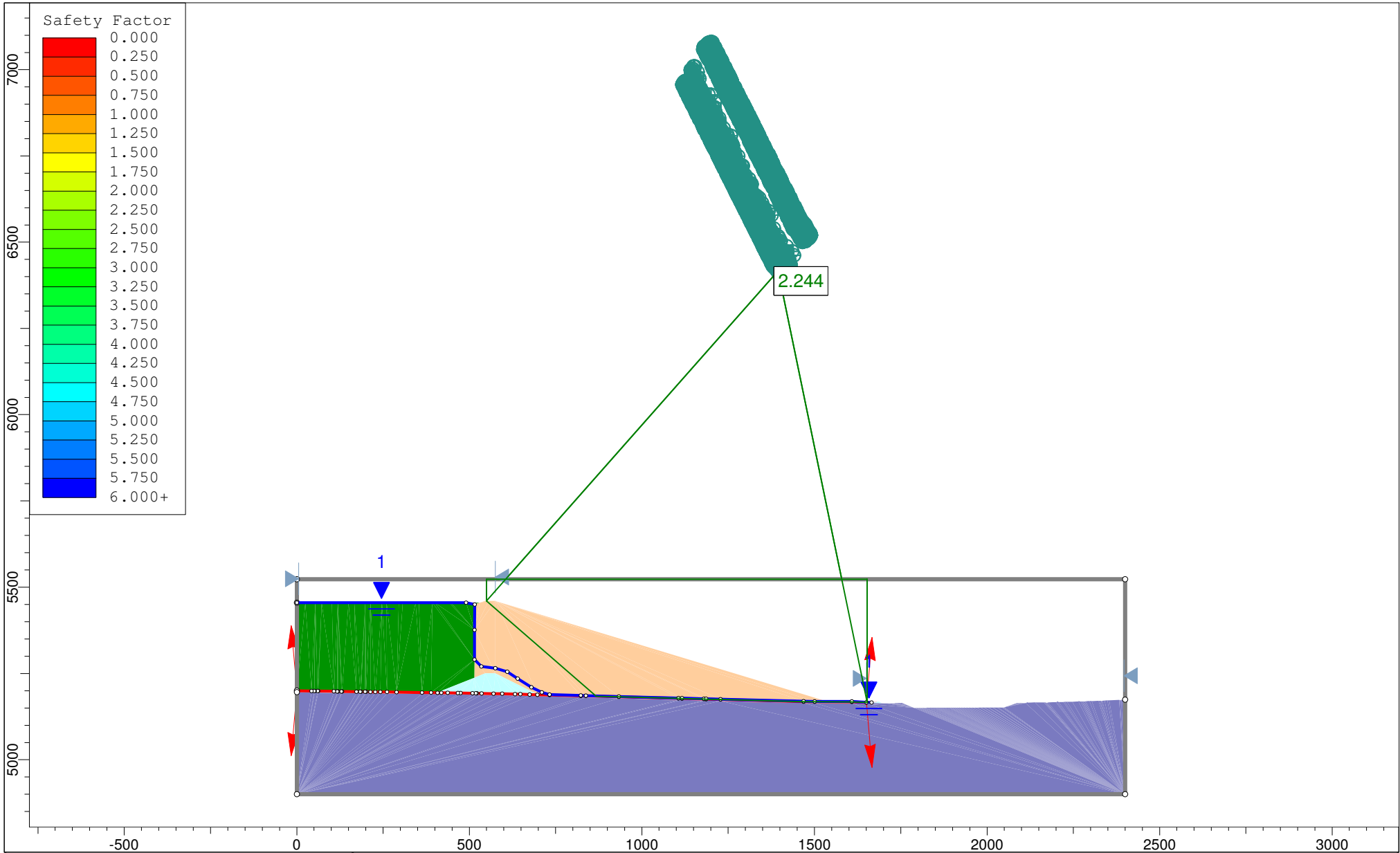
**NOTES**

1. TAILINGS STORAGE FACILITY SHOWN AT FINAL BUILDOUT.



<p>PROJECT <b>THE MAC</b> RESOURCES <small>Environmentally Responsible. Community-Minded. Local Opportunities.</small></p>	<p>NEW MEXICO COPPER CORPORATION</p>	<p>DRAFT COPPER FLAT PROJECT 30 TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO</p>																			
<b>STABILITY SECTION LOCATION PLAN</b>																					
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>PROJECT No.</td> <td>103-92557</td> <td>FILE No.</td> <td>10392557K202_10222015</td> </tr> <tr> <td>DESIGN</td> <td>DMW</td> <td>2013-04-30</td> <td>SCALE AS SHOWN</td> </tr> <tr> <td>CADD</td> <td>JHR</td> <td>2013-04-30</td> <td>DRAWING</td> </tr> <tr> <td>CHECK</td> <td>GM</td> <td>2013-05-07</td> <td rowspan="2" style="text-align: center; vertical-align: middle;"><b>H.1</b></td> </tr> <tr> <td>REVIEW</td> <td>DAK</td> <td>2013-05-07</td> </tr> </table>	PROJECT No.	103-92557	FILE No.	10392557K202_10222015	DESIGN	DMW	2013-04-30	SCALE AS SHOWN	CADD	JHR	2013-04-30	DRAWING	CHECK	GM	2013-05-07	<b>H.1</b>	REVIEW	DAK	2013-05-07	
PROJECT No.	103-92557	FILE No.	10392557K202_10222015																		
DESIGN	DMW	2013-04-30	SCALE AS SHOWN																		
CADD	JHR	2013-04-30	DRAWING																		
CHECK	GM	2013-05-07	<b>H.1</b>																		
REVIEW	DAK	2013-05-07																			

**APPENDIX H.1  
MAXIMUM EMBANKMENT SECTION**



SLIDEINTERPRET 6.008

Project		Copper Flat	
Analysis Description		Section A Stability: Downstream, Static, Block Failure, Global	
Drawn By	GS	Scale	1:4620
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	1a - SectionA 5460R DS_S_B_G.slim



## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 1a - SectionA 5460R DS\_S\_B\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.244240  
 Axis Location: 1394.865, 6416.541  
 Left Slip Surface Endpoint: 549.279, 5460.000  
 Right Slip Surface Endpoint: 1652.746, 5166.147  
 Left Slope Intercept: 549.279 5522.990  
 Right Slope Intercept: 1652.746 5522.990  
 Resisting Moment=8.08765e+009 lb-ft

Driving Moment=3.60374e+009 lb-ft  
 Resisting Horizontal Force=5.71009e+006 lb  
 Driving Horizontal Force=2.54433e+006 lb

## Global Minimum Coordinates

### Method: spencer

X	Y
549.279	5460
868.036	5183.72
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1608.27	5167.61
1623.49	5166.77
1652.75	5166.15
1652.75	5522.99

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 757  
 Number of Invalid Surfaces: 4064

#### Error Codes:

- Error Code -108 reported for 7 surfaces
- Error Code -111 reported for 2282 surfaces
- Error Code -112 reported for 19 surfaces
- Error Code -116 reported for 1756 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	52.9277	124924	Cyclone Underflow	0	39	591.131	1326.64	1638.28	0	1638.28
2	52.9277	315093	Cyclone Underflow	0	39	1658.67	3722.45	4596.85	0	4596.85
3	52.9277	493540	Cyclone Underflow	0	39	2613.17	5864.58	7242.16	0	7242.16
4	52.9277	671986	Cyclone Underflow	0	39	3567.68	8006.72	9887.48	0	9887.48
5	52.9277	850433	Cyclone Underflow	0	39	4522.2	10148.9	12532.8	0	12532.8
6	52.9277	1.02888e+006	Cyclone Underflow	0	39	5476.69	12291	15178.1	0	15178.1
7	1.19075	25204	Liner Interface Zone	0	26.5	4101.89	9205.63	18494.8	31.1774	18463.6
8	65.4393	1.32103e+006	Liner Interface Zone	0	26.5	4296.98	9643.45	19404.1	62.3547	19341.7
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	3873.77	8693.66	17499.2	62.3583	17436.9
10	57.4662	946268	Liner Interface Zone	0	26.5	3476.66	7802.45	15711.7	62.3583	15649.3
11	57.4662	842833	Liner Interface Zone	0	26.5	3079.55	6911.24	13924.2	62.3583	13861.8
12	10.1889	138642	Liner Interface Zone	0	26.5	2845.74	6386.52	12871.8	62.3576	12809.4
13	63.358	789325	Liner Interface Zone	0	26.5	2592.07	5817.23	11729.9	62.3559	11667.6
14	6.66227	75711.8	Liner Interface Zone	0	26.5	2350.93	5276.05	10644.5	62.3581	10582.1
15	41.4898	440289	Liner Interface Zone	0	26.5	2184.77	4903.15	9896.55	62.3559	9834.2
16	48.1756	443630	Liner Interface Zone	0	26.5	1875.42	4208.9	8504.11	62.3594	8441.75
17	48.1756	370848	Liner Interface Zone	0	26.5	1542.11	3460.86	7003.76	62.3594	6941.4
18	48.1756	298067	Liner Interface Zone	0	26.5	1208.79	2712.81	5503.4	62.3594	5441.04
19	48.1756	225285	Liner Interface Zone	0	26.5	875.468	1964.76	4003.07	62.3594	3940.71
20	48.1756	152503	Liner Interface Zone	0	26.5	542.148	1216.71	2502.71	62.3594	2440.36
21	31.9454	59717.2	Liner Interface Zone	0	26.5	256.986	576.739	1219.16	62.3993	1156.76

22	53.9361	28069.4	Liner Interface Zone	0	26.5	6.26742	14.0656	90.6102	62.3991	28.2111
23	53.9361	6472.33	Liner Interface Zone	0	26.5	10.742	24.1077	110.752	62.3991	48.3526
24	15.217	1826.04	Liner Interface Zone	0	26.5	0	0	0.432806	62.2097	-61.7769
25	29.2604	1755.63	Liner Interface Zone	0	26.5	2857.95	6413.92	12895.4	31.1049	12864.3

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.24424

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	549.279	5460	1.98387e-022	0	0
2	602.206	5414.12	-113840	11116	-5.57701
3	655.134	5368.25	-69899.2	6825.37	-5.57702
4	708.062	5322.37	28843.8	-2816.47	-5.577
5	760.99	5276.5	182389	-17809.5	-5.57701
6	813.917	5230.62	390735	-38153.7	-5.57702
7	866.845	5184.75	653884	-63849.1	-5.57702
8	868.036	5183.72	664679	-64903.2	-5.57702
9	933.475	5181.95	217883	-21275.3	-5.577
10	990.941	5180.47	-174451	17034.4	-5.57701
11	1048.41	5178.98	-565551	55223.8	-5.57702
12	1105.87	5177.5	-955418	93292.6	-5.57701
13	1116.06	5177.23	-1.02438e+006	100027	-5.57705
14	1179.42	5175.55	-1.45202e+006	141784	-5.57703
15	1186.08	5175.37	-1.49696e+006	146172	-5.57702
16	1227.57	5174.27	-1.77617e+006	173435	-5.57699
17	1275.75	5173.04	-2.10008e+006	205064	-5.57701
18	1323.92	5171.81	-2.42308e+006	236604	-5.57702
19	1372.1	5170.58	-2.74518e+006	268055	-5.57701
20	1420.28	5169.35	-3.06636e+006	299418	-5.57703
21	1468.45	5168.13	-3.38664e+006	330691	-5.57701
22	1500.4	5168.02	-3.59915e+006	351442	-5.57701
23	1554.33	5167.81	-3.83666e+006	374634	-5.57702
24	1608.27	5167.61	-3.84174e+006	375130	-5.57702
25	1623.49	5166.77	-3.86037e+006	376950	-5.57703
26	1652.75	5166.15	6.36684e-021	0	0

### List Of Coordinates

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63

632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5

0	5195.5
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**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23



1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54

1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6

2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**



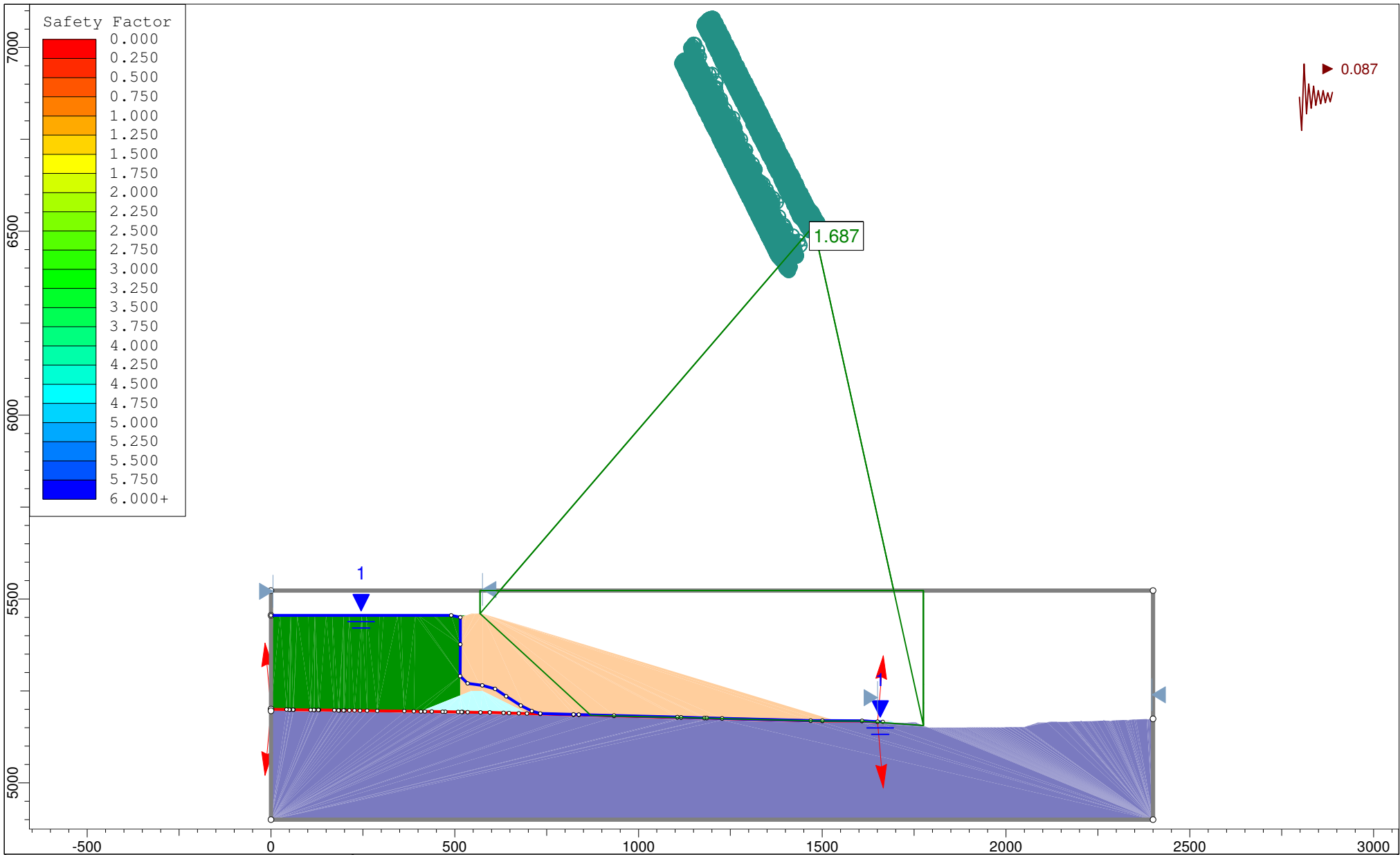
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83


**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



	Project			Copper Flat		
	Analysis Description					Section A Stability: Downstream, Pseudo Static, Block Failure, Global
	Drawn By	GS	Scale	1:4338	Company	Golder Associates Inc.
	Date	11/4/2013, 3:02:21 PM		File Name	2a - SectionA 5460R DS_PS_B_G.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 2a - SectionA 5460R DS\_PS\_B\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None



## Random Numbers

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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

---

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined







## Loading

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Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

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Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

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Method: spencer

FS: 1.686520

Axis Location: 1476.350, 6514.282  
 Left Slip Surface Endpoint: 568.826, 5460.000  
 Right Slip Surface Endpoint: 1775.261, 5155.693  
 Left Slope Intercept: 568.826 5522.990  
 Right Slope Intercept: 1775.261 5522.990  
 Resisting Moment=8.24296e+009 lb-ft  
 Driving Moment=4.88754e+009 lb-ft  
 Resisting Horizontal Force=5.38918e+006 lb  
 Driving Horizontal Force=3.19543e+006 lb

## Global Minimum Coordinates

---

### Method: spencer

X	Y
568.826	5460
870.106	5183.66
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1468.45	5168.13
1500.4	5168.02
1601.92	5167.63
1775.26	5155.69
1775.26	5522.99

## Valid / Invalid Surfaces

---

### Method: spencer

Number of Valid Surfaces: 518  
 Number of Invalid Surfaces: 4303

#### Error Codes:

Error Code -108 reported for 22 surfaces  
 Error Code -111 reported for 2509 surfaces  
 Error Code -112 reported for 16 surfaces  
 Error Code -116 reported for 1756 surfaces

#### Error Codes

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

- 111 = safety factor equation did not converge
- 112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

## Slice Data

### Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	50.0261	97432.4	Cyclone Underflow	0	39	554.036	934.392	1153.88	0	1153.88
2	50.0261	271871	Cyclone Underflow	0	39	1829.51	3085.51	3810.28	0	3810.28
3	50.0261	445560	Cyclone Underflow	0	39	3022.48	5097.47	6294.86	0	6294.86
4	50.0261	619248	Cyclone Underflow	0	39	4215.44	7109.43	8779.42	0	8779.42
5	50.0261	792937	Cyclone Underflow	0	39	5408.41	9121.4	11264	0	11264
6	50.0261	966626	Cyclone Underflow	0	39	6601.4	11133.4	13748.6	0	13748.6
7	1.12325	23701.6	Liner Interface Zone	0	26.5	5119.91	8634.83	17350	31.1774	17318.8
8	63.369	1.27719e+006	Liner Interface Zone	0	26.5	5707.87	9626.44	19369.9	62.3547	19307.6
9	57.4662	1.0497e+006	Liner Interface Zone	0	26.5	5154.91	8693.86	17499.6	62.3583	17437.2
10	57.4662	946268	Liner Interface Zone	0	26.5	4626.75	7803.1	15713	62.3583	15650.6
11	57.4662	842833	Liner Interface Zone	0	26.5	4098.59	6912.35	13926.4	62.3583	13864
12	10.1889	138642	Liner Interface Zone	0	26.5	3787.56	6387.79	12874.2	62.3576	12811.9
13	63.358	789325	Liner Interface Zone	0	26.5	3450.05	5818.58	11732.6	62.3559	11670.3
14	6.66227	75711.8	Liner Interface Zone	0	26.5	3129.49	5277.95	10648.3	62.3581	10585.9
15	41.4898	440289	Liner Interface Zone	0	26.5	2908.36	4905.01	9900.29	62.3559	9837.93
16	60.2195	543166	Liner Interface Zone	0	26.5	2441.73	4118.02	8321.82	62.3594	8259.46
17	60.2195	429444	Liner Interface Zone	0	26.5	1887.56	3183.41	6447.29	62.3594	6384.93
18	60.2195	315723	Liner Interface Zone	0	26.5	1333.4	2248.8	4572.76	62.3594	4510.4
			Liner Interface							

		Zone								
20	31.9454	59717.2	Liner Interface Zone	0	26.5	345.09	582.001	1229.71	62.3993	1167.31
21	50.7606	27688.4	Liner Interface Zone	0	26.5	9.96282	16.8025	96.0997	62.3991	33.7006
22	50.7606	6091.28	Liner Interface Zone	0	26.5	14.4972	24.4499	111.438	62.3991	49.0388
23	62.898	13110.9	Liner Interface Zone	0	26.5	0	0	102.782	109.089	-6.30725
24	55.2228	27974.9	Foundation Materials	150	29	213.592	360.228	379.262	0	379.262
25	55.2228	30469.2	Foundation Materials	150	29	2490.45	4200.2	7306.76	0	7306.76

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.68652

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	568.826	5460	1.98387e-022	0	0
2	618.852	5414.12	-147187	14297.2	-5.54809
3	668.878	5368.23	-119420	11600	-5.54809
4	718.904	5322.35	-36557.4	3551.05	-5.54809
5	768.93	5276.46	101402	-9849.77	-5.54807
6	818.957	5230.58	294457	-28602.4	-5.54808
7	868.983	5184.69	542608	-52706.9	-5.54809
8	870.106	5183.66	553563	-53771	-5.54809
9	933.475	5181.95	142080	-13801.1	-5.54808
10	990.941	5180.47	-232631	22596.9	-5.54809
11	1048.41	5178.98	-607567	59016.7	-5.54808
12	1105.87	5177.5	-982726	95458.3	-5.54809
13	1116.06	5177.23	-1.04924e+006	101919	-5.54808
14	1179.42	5175.55	-1.46262e+006	142073	-5.54807
15	1186.08	5175.37	-1.50616e+006	146303	-5.5481
16	1227.57	5174.27	-1.77711e+006	172622	-5.5481
17	1287.79	5172.73	-2.17125e+006	210907	-5.54809
18	1348.01	5171.2	-2.56556e+006	249209	-5.54809
19	1408.23	5169.66	-2.96006e+006	287529	-5.54809
20	1468.45	5168.13	-3.35475e+006	325867	-5.54808
21	1500.4	5168.02	-3.56488e+006	346279	-5.54809
22	1551.16	5167.82	-3.79988e+006	369105	-5.54807
23	1601.92	5167.63	-3.80432e+006	369537	-5.54809
24	1664.82	5163.3	-3.87274e+006	376183	-5.54808
25	1720.04	5159.5	-3.93678e+006	382404	-5.54809
26	1775.26	5155.69	6.74534e-021	0	0

## List Of Coordinates

---

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

### Block Search Polyline

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92

732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03
595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98

2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92

837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5



1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61

2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83

2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199

117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8

1675.27	5168.86
1680.63	5168.86

**Material Boundary**

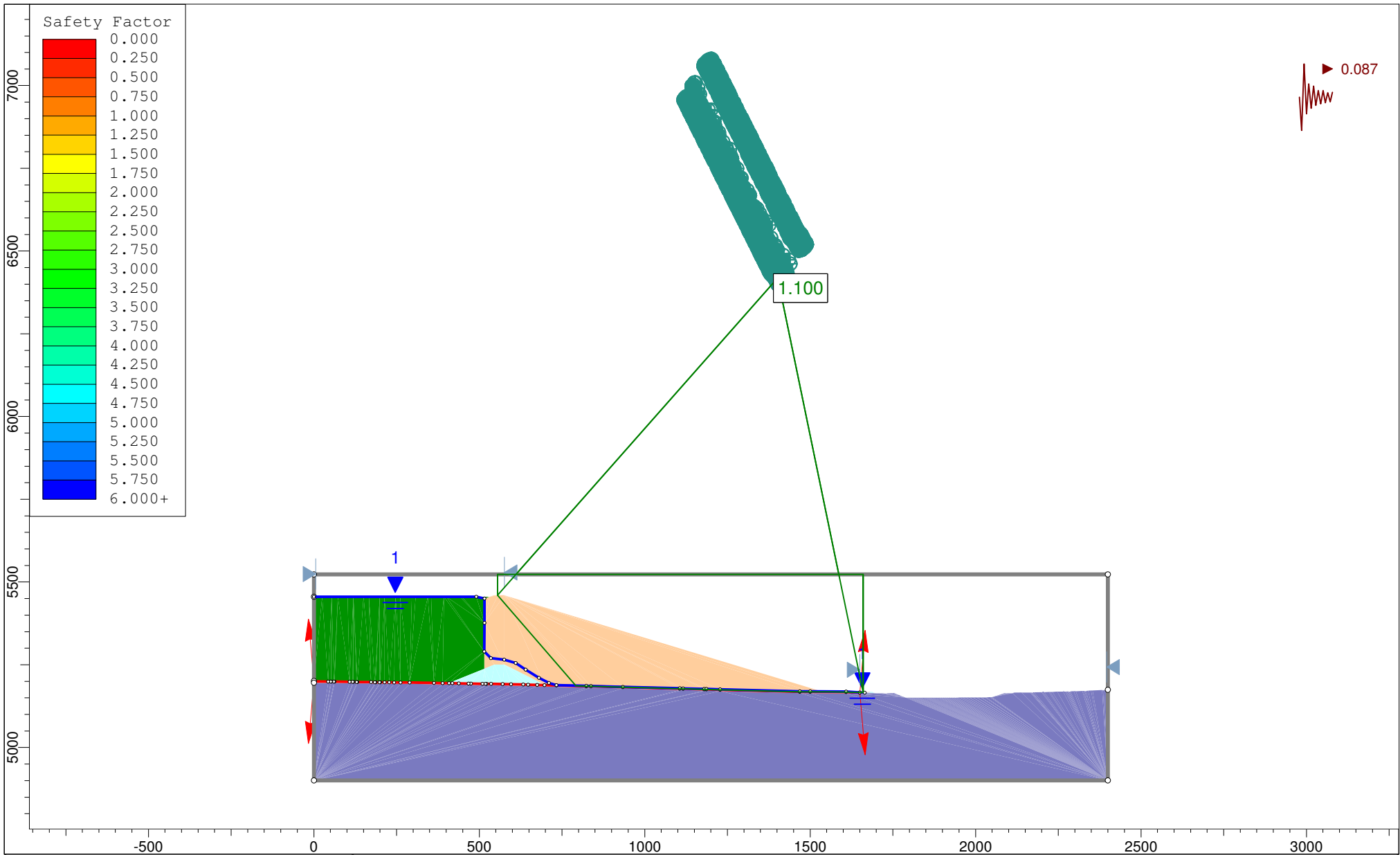
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83


**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



	Project			Copper Flat														
	Analysis Description						Section A Stability: Downstream, Pseudo Static, Block Failure, Global											
	Drawn By			GS			Scale			1:4820			Company			Golder Associates Inc.		
	Date			11/4/2013, 3:02:21 PM			File Name			3a - SectionA 5460R DS_PS_B_G_min_phi.slim								

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 3a - SectionA 5460R\_DS\_PS\_B\_G\_min\_phi.slim  
Last saved with Slide version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:

103-92557  
Material Property Edits 12/2013  
Liner interface min phi=13.6deg

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3







## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## List Of Coordinates

### Piezoline

X	Y



0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03

595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

### Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**



X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**





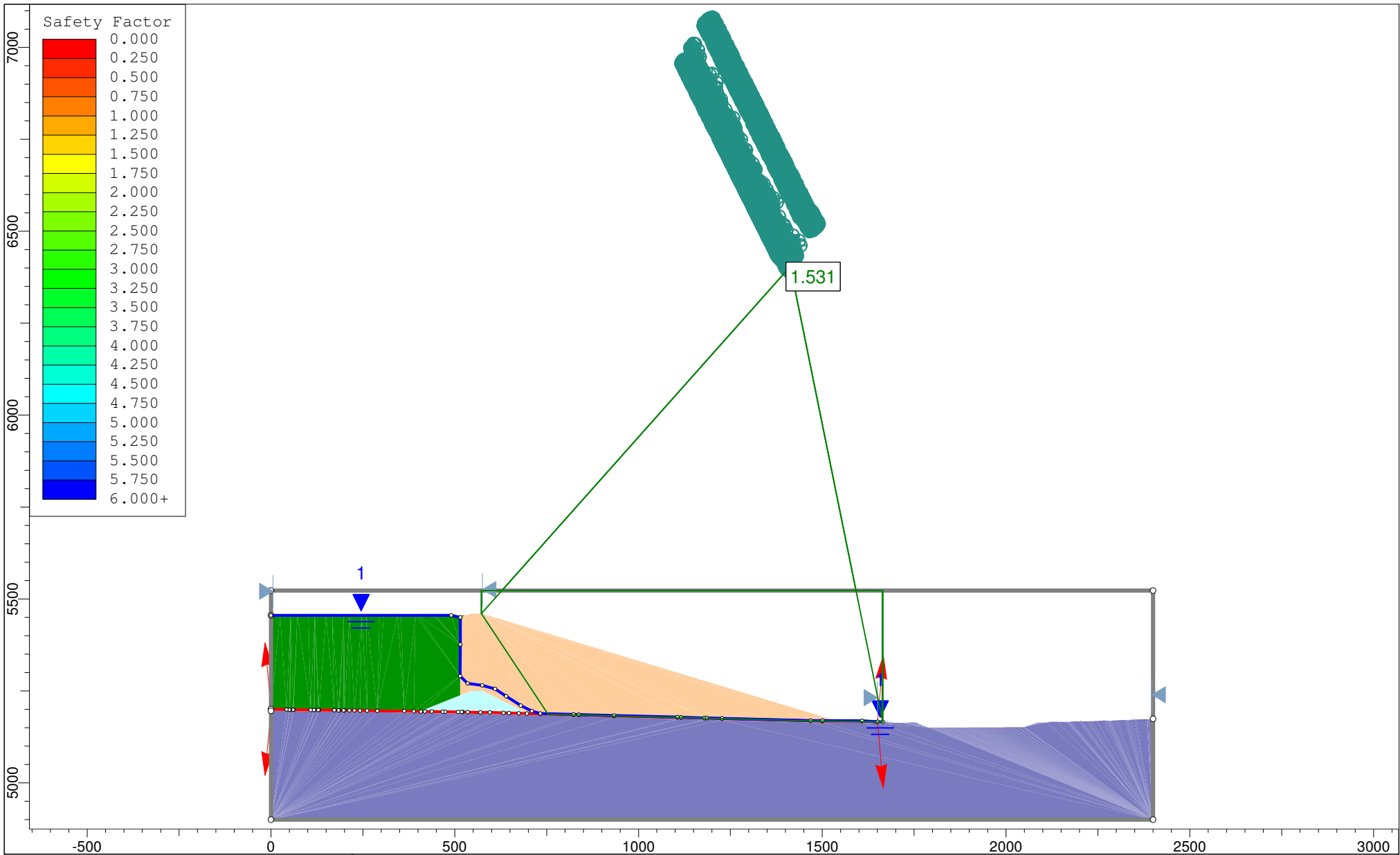
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83


**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



	Project			Copper Flat		
	Analysis Description			Section A Stability: Downstream, Static, Block Failure, Global		
	Drawn By	GS	Scale	1:4338	Company	Golder Associates Inc.
	Date	11/4/2013, 3:02:21 PM		File Name	3b - SectionA 5460R DS_S_B_G_min_phi.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 3b - SectionA 5460R DS\_S\_B\_G\_min\_phi.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:

103-92557  
Material Property Edits 12/2013  
Liner Interface phi =13.6deg

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	13.6	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 1.530610  
 Axis Location: 1412.961, 6404.397  
 Left Slip Surface Endpoint: 572.648, 5460.000  
 Right Slip Surface Endpoint: 1664.291, 5165.509  
 Left Slope Intercept: 572.648 5522.990  
 Right Slope Intercept: 1664.291 5522.990  
 Resisting Moment=5.18841e+009 lb-ft

Driving Moment=3.38976e+009 lb-ft  
 Resisting Horizontal Force=3.37882e+006 lb  
 Driving Horizontal Force=2.20749e+006 lb

## Global Minimum Coordinates

### Method: spencer

X	Y
572.648	5460
753.658	5186.79
823.107	5184.92
837.37	5184.54
933.475	5181.95
1105.87	5177.5
1116.06	5177.23
1179.42	5175.55
1186.08	5175.37
1227.57	5174.27
1456.16	5168.44
1664.29	5165.51
1664.29	5522.99

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 1279  
 Number of Invalid Surfaces: 3542

#### Error Codes:

Error Code -108 reported for 11 surfaces  
 Error Code -111 reported for 1763 surfaces  
 Error Code -112 reported for 12 surfaces  
 Error Code -116 reported for 1756 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha =  $\cos(\alpha)(1+\tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.
- 116 = Not enough slices to analyze the surface Increase the number of slices in the job control in the modeler.

**Slice Data**

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	45.0838	142718	Cyclone Underflow	0	39	872.365	1335.25	1648.9	0	1648.9
2	45.0838	419922	Cyclone Underflow	0	39	2833.01	4336.24	5354.81	0	5354.81
3	45.0838	696992	Cyclone Underflow	0	39	4722.56	7228.4	8926.34	0	8926.34
4	45.0838	974063	Cyclone Underflow	0	39	6612.14	10120.6	12497.8	0	12497.8
5	0.674563	16680.5	Liner Interface Zone	0	13.6	3447.02	5276.04	21839.8	31.1774	21808.6
6	69.4483	1.64551e+006	Liner Interface Zone	0	13.6	3646.43	5581.26	23132.6	62.3548	23070.2
7	14.2633	319322	Liner Interface Zone	0	13.6	3440.05	5265.38	21826.8	62.3568	21764.5
8	48.0526	1.02905e+006	Liner Interface Zone	0	13.6	3286.15	5029.81	20853.1	62.3547	20790.8
9	48.0526	957013	Liner Interface Zone	0	13.6	3049.14	4667.05	19353.6	62.3547	19291.3
10	57.4662	1.0497e+006	Liner Interface Zone	0	13.6	2788.59	4268.24	17705.2	62.3583	17642.8
11	57.4662	946268	Liner Interface Zone	0	13.6	2504.03	3832.69	15904.8	62.3583	15842.4
12	57.4662	842833	Liner Interface Zone	0	13.6	2219.46	3397.13	14104.4	62.3583	14042.1
13	10.1889	138642	Liner Interface Zone	0	13.6	2051.94	3140.72	13044.6	62.3576	12982.2
14	63.358	789325	Liner Interface Zone	0	13.6	1870.22	2862.57	11894.8	62.3559	11832.4
15	6.66227	75711.8	Liner Interface Zone	0	13.6	1697.36	2597.99	10801.2	62.3581	10738.8
16	41.4898	440289	Liner Interface Zone	0	13.6	1578.34	2415.83	10048.2	62.3559	9985.82
17	45.7181	422762	Liner Interface Zone	0	13.6	1362.68	2085.73	8683.74	62.3594	8621.38
18	45.7181	357216	Liner Interface Zone	0	13.6	1136.02	1738.8	7249.7	62.3594	7187.34
19	45.7181	291670	Liner Interface Zone	0	13.6	909.35	1391.86	5815.63	62.3594	5753.27
20	45.7181	226124	Liner Interface Zone	0	13.6	682.689	1044.93	4381.59	62.3594	4319.23
21	45.7181	160579	Liner Interface Zone	0	13.6	456.025	697.997	2947.52	62.3594	2885.17
			Liner Interface							

		Zone								
23	52.032	22869.3	Liner Interface Zone	0	13.6	5.72276	8.75932	132.838	96.6312	36.2067
24	52.032	12959.2	Liner Interface Zone	0	13.6	16.2967	24.9439	233.108	130.003	103.105
25	52.032	6697.29	Liner Interface Zone	0	13.6	1011.39	1548.04	6465.57	66.7275	6398.84

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.53061

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	572.648	5460	1.98387e-022	0	0
2	617.732	5391.95	-106388	9073.38	-4.87472
3	662.816	5323.9	59899.5	-5108.59	-4.87473
4	707.9	5255.86	372435	-31763.5	-4.87473
5	752.984	5187.81	831219	-70891.4	-4.87474
6	753.658	5186.79	849640	-72462.5	-4.87474
7	823.107	5184.92	472351	-40285	-4.87474
8	837.37	5184.54	393696	-33576.8	-4.87474
9	885.422	5183.25	126906	-10823.3	-4.87473
10	933.475	5181.95	-143674	12253.4	-4.87474
11	990.941	5180.47	-473336	40368.9	-4.87473
12	1048.41	5178.98	-808251	68932.5	-4.87473
13	1105.87	5177.5	-1.14842e+006	97944.2	-4.87473
14	1116.06	5177.23	-1.20925e+006	103132	-4.87473
15	1179.42	5175.55	-1.59087e+006	135679	-4.87473
16	1186.08	5175.37	-1.63143e+006	139138	-4.87473
17	1227.57	5174.27	-1.88531e+006	160790	-4.87472
18	1273.29	5173.1	-2.16872e+006	184961	-4.87472
19	1319.01	5171.94	-2.45542e+006	209413	-4.87473
20	1364.73	5170.77	-2.74542e+006	234146	-4.87473
21	1410.44	5169.6	-3.03871e+006	259160	-4.87474
22	1456.16	5168.44	-3.3353e+006	284454	-4.87472
23	1508.2	5167.71	-3.67744e+006	313635	-4.87474
24	1560.23	5166.97	-3.86453e+006	329590	-4.87473
25	1612.26	5166.24	-3.87412e+006	330409	-4.87474
26	1664.29	5165.51	6.38965e-021	0	0

### List Of Coordinates

#### Piezoline

X	Y

0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**Block Search Polyline**

X	Y
1650.32	5165.28
1608.27	5167.61
1500.4	5168.02
1468.45	5168.13
1227.57	5174.27
1186.08	5175.37
1179.42	5175.55
1116.06	5177.23
1105.87	5177.5
933.475	5181.95
837.37	5184.54
823.107	5184.92
732.639	5187.36
696.805	5188.32
674.437	5188.92
647.545	5189.63
632.517	5190.03



595.395	5191.04
570.193	5191.45
535.401	5192.01
521.537	5192.24
518.618	5192.28
509.317	5192.41
474.418	5192.97
466.941	5193.08
437.521	5193.55
418.604	5193.82
407.909	5193.99
388.869	5194.29
362.969	5194.7
289.277	5195.79
261.362	5196.12
242.453	5196.34
227.348	5196.51
213.325	5196.67
200.27	5196.81
198.026	5196.84
185.964	5196.98
183.907	5197.01
172.729	5197.13
131.064	5197.72
129.194	5197.75
118.942	5197.86
117.225	5197.89
107.708	5198
61.8783	5198.65
60.3563	5198.67
51.8447	5198.77
50.4444	5198.79
42.5304	5198.88
0	5199.5

**External Boundary**

X	Y
0	4900
2400	4900
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

### Material Boundary

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55

1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55

1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46

2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**



X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13

183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**



X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

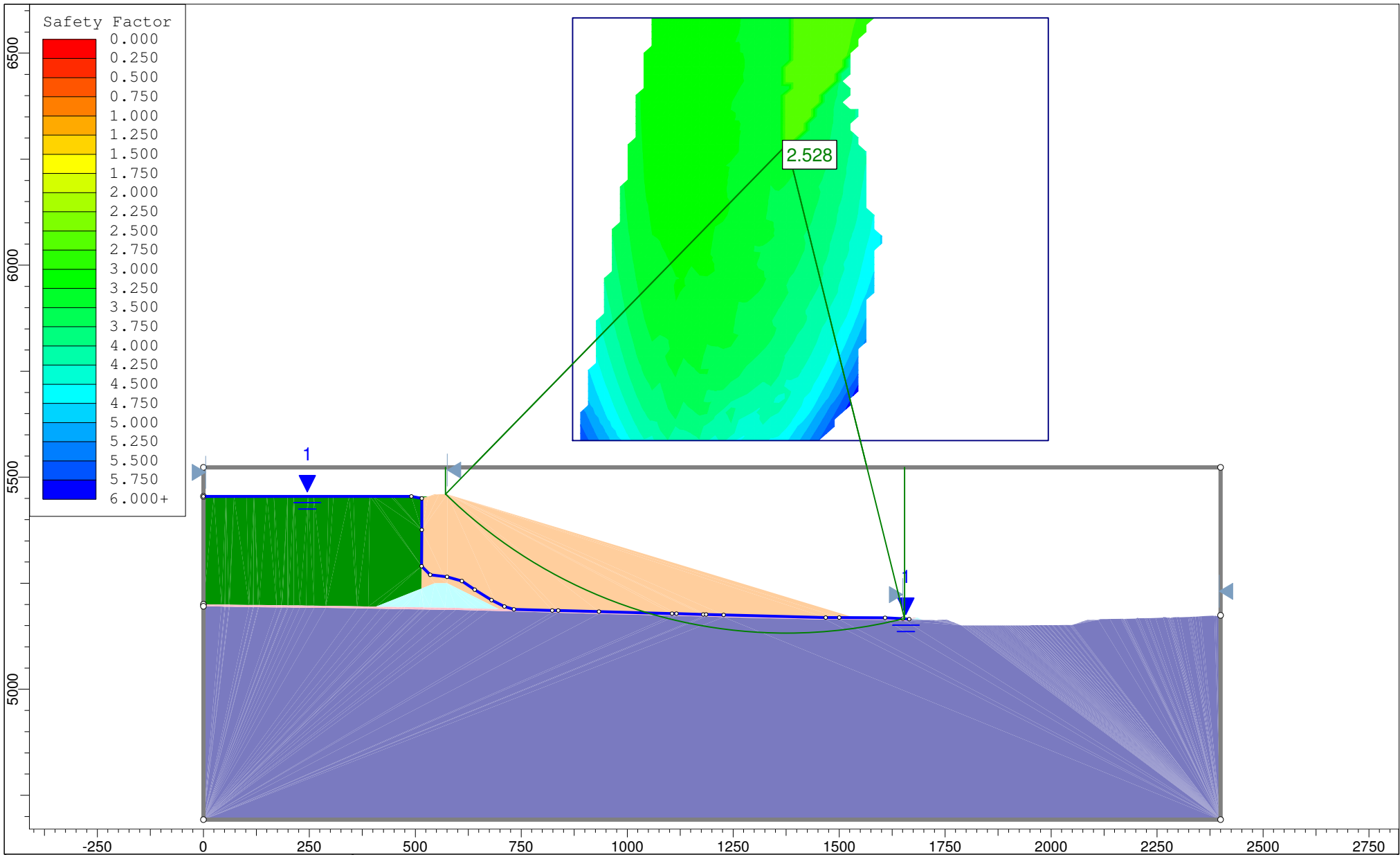
**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32





SLIDEINTERPRET 6.008

Project	Copper Flat		
Analysis Description	Section A Stability: Downstream, Static, Circular Failure, Global		
Drawn By	GS	Scale	1:3762
		Company	Golder Associates Inc.
Date	11/4/2013, 3:02:21 PM	File Name	4a - SectionA 5460R DS_S_C_G.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 4a - SectionA 5460R DS\_S\_C\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Circular Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.527760  
 Center: 1376.188, 6284.432  
 Radius: 1152.416  
 Left Slip Surface Endpoint: 570.971, 5460.000  
 Right Slip Surface Endpoint: 1654.244, 5166.064  
 Left Slope Intercept: 570.971 5522.990  
 Right Slope Intercept: 1654.244 5522.990  
 Resisting Moment=8.33108e+009 lb-ft  
 Driving Moment=3.29583e+009 lb-ft  
 Resisting Horizontal Force=6.7452e+006 lb

Driving Horizontal Force=2.66845e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 11730

Number of Invalid Surfaces: 29201

#### Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4317 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3813 surfaces
- Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.216	953.512	1177.49	0	1177.49
2	47.6079	233141	Cyclone Underflow	0	39	1249.89	3159.43	3901.57	0	3901.57
3	47.6079	356305	Cyclone Underflow	0	39	1954.83	4941.33	6102.03	0	6102.03
4	47.6079	458937	Cyclone Underflow	0	39	2561.02	6473.65	7994.29	0	7994.29
5	47.6079	543134	Cyclone Underflow	0	39	3074.2	7770.84	9596.18	0	9596.18

6	47.6079	610541	Cyclone Underflow	0	39	3499.06	8844.78	10922.4	0	10922.4
7	47.6079	662469	Cyclone Underflow	0	39	3839.45	9705.2	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.49	10360	12793.5	0	12793.5
9	47.6079	723916	Cyclone Underflow	0	39	4278.69	10815.5	13356	0	13356
10	47.6079	734997	Cyclone Underflow	0	39	4381.94	11076.5	13678.4	0	13678.4
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.41	7119.2	14434.9	155.948	14278.9
12	43.9418	677489	Foundation Materials	150	29	3185.32	8051.73	14255.1	0	14255.1
13	43.9418	666065	Foundation Materials	150	29	3142.02	7942.28	14057.7	0	14057.7
14	43.9418	645159	Foundation Materials	150	29	3052.52	7716.04	13649.5	0	13649.5
15	43.9418	614993	Foundation Materials	150	29	2917.32	7374.28	13032.9	0	13032.9
16	43.9418	575744	Foundation Materials	150	29	2736.7	6917.73	12209.3	0	12209.3
17	43.9418	527526	Foundation Materials	150	29	2510.7	6346.44	11178.7	0	11178.7
18	43.9418	470419	Foundation Materials	150	29	2239.12	5659.95	9940.21	0	9940.21
19	43.9418	404462	Foundation Materials	150	29	1921.59	4857.32	8492.24	0	8492.24
20	43.9418	329659	Foundation Materials	150	29	1557.52	3937.03	6831.97	0	6831.97
21	43.9418	246078	Foundation Materials	150	29	1146.59	2898.3	4958.07	0	4958.07
22	43.9418	156715	Foundation Materials	150	29	744.157	1881.05	3122.9	0	3122.9
23	43.9418	104701	Foundation Materials	150	29	581.701	1470.4	2382.07	0	2382.07
24	43.9418	56711	Foundation Materials	150	29	328.326	829.93	1226.63	0	1226.63
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4370.71	11048.1	22314.9	155.762	22159.1

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 2.52776

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146707	13731.9	-5.34735

3	666.187	5376.71	-127700	11952.8	-5.34734
4	713.795	5341.41	-93307	8733.58	-5.34733
5	761.402	5309.7	-62718.8	5870.51	-5.34733
6	809.01	5281.25	-50008.5	4680.82	-5.34733
7	856.618	5255.79	-65397.2	6121.21	-5.34733
8	904.226	5233.09	-116109	10867.9	-5.34735
9	951.834	5212.99	-206964	19371.9	-5.34732
10	999.442	5195.34	-340792	31898.3	-5.34733
11	1047.05	5180.02	-518731	48553.5	-5.34734
12	1066.08	5174.52	-568159	53179.9	-5.34733
13	1110.02	5163.18	-727759	68118.6	-5.34733
14	1153.96	5153.65	-924346	86519.3	-5.34734
15	1197.9	5145.89	-1.15617e+006	108218	-5.34733
16	1241.85	5139.87	-1.42056e+006	132965	-5.34733
17	1285.79	5135.57	-1.71391e+006	160423	-5.34734
18	1329.73	5132.95	-2.03175e+006	190173	-5.34734
19	1373.67	5132.02	-2.36865e+006	221707	-5.34734
20	1417.61	5132.76	-2.71825e+006	254430	-5.34734
21	1461.56	5135.18	-3.07316e+006	287649	-5.34733
22	1505.5	5139.29	-3.42492e+006	320574	-5.34733
23	1549.44	5145.11	-3.67939e+006	344392	-5.34732
24	1593.38	5152.67	-3.72663e+006	348815	-5.34734
25	1637.32	5161.99	-3.78937e+006	354686	-5.34732
26	1654.24	5166.06	6.3698e-021	0	0

## List Of Coordinates

### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5

1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27



1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455

530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

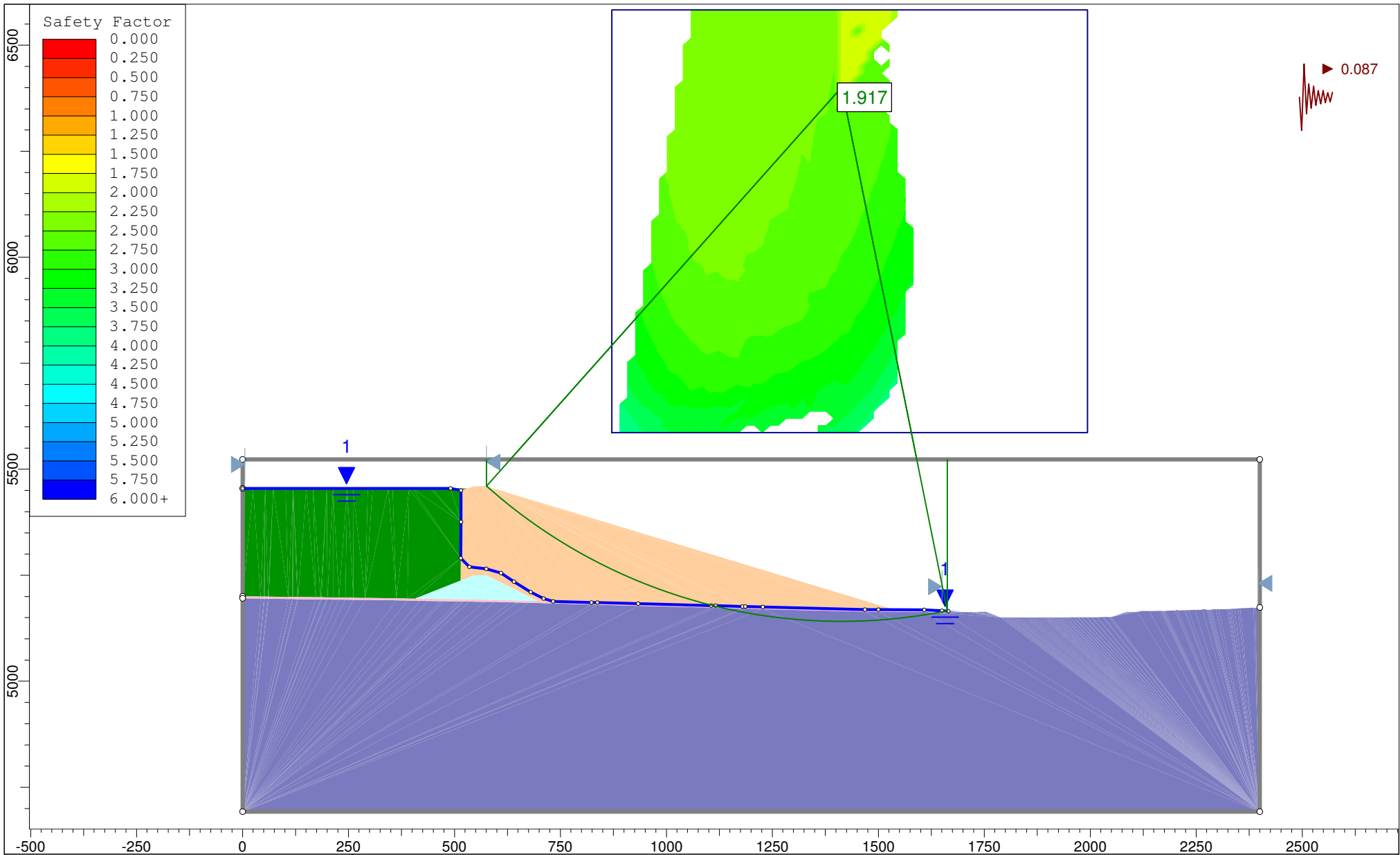
X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

<i>Project</i>		Copper Flat	
<i>Analysis Description</i>		Section A Stability: Downstream, Pseudo Static, Circular Failure, Global	
<i>Drawn By</i>	GS	<i>Scale</i>	1:3762
<i>Date</i>	11/4/2013, 3:02:21 PM	<i>Company</i>	Golder Associates Inc.
		<i>File Name</i>	5a - SectionA 5460R DS_PS_C_G.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 5a - SectionA 5460R\_DS\_PS\_C\_G.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Pseudo Static, Circular Failure, Global  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3







## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 1.916600  
 Center: 1413.608, 6400.750  
 Radius: 1259.972  
 Left Slip Surface Endpoint: 575.445, 5460.000



Right Slip Surface Endpoint: 1662.506, 5165.607  
 Left Slope Intercept: 575.445 5522.990  
 Right Slope Intercept: 1662.506 5522.990  
 Resisting Moment=7.97722e+009 lb-ft  
 Driving Moment=4.16218e+009 lb-ft  
 Resisting Horizontal Force=5.93993e+006 lb  
 Driving Horizontal Force=3.0992e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 10557  
 Number of Invalid Surfaces: 30374

#### Error Codes:

Error Code -101 reported for 408 surfaces  
 Error Code -103 reported for 2326 surfaces  
 Error Code -108 reported for 4489 surfaces  
 Error Code -110 reported for 440 surfaces  
 Error Code -111 reported for 4814 surfaces  
 Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.9166

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	44.3652	60862.7	Cyclone Underflow	0	39	321.259	615.725	760.358	0	760.358
2	44.3652	174209	Cyclone Underflow	0	39	1244.73	2385.64	2946.02	0	2946.02
			Cyclone							

			Underflow								
4	44.3652	354600	Cyclone Underflow	0	39	2655.19	5088.94	6284.31	0	6284.31	
5	44.3652	424335	Cyclone Underflow	0	39	3230.76	6192.08	7646.58	0	7646.58	
6	44.3652	481812	Cyclone Underflow	0	39	3723.19	7135.87	8812.07	0	8812.07	
7	44.3652	527840	Cyclone Underflow	0	39	4134.48	7924.15	9785.52	0	9785.52	
8	44.3652	563095	Cyclone Underflow	0	39	4466.28	8560.07	10570.8	0	10570.8	
9	44.3652	588143	Cyclone Underflow	0	39	4719.87	9046.11	11171	0	11171	
10	44.3652	603459	Cyclone Underflow	0	39	4896.24	9384.13	11588.4	0	11588.4	
11	44.3652	609444	Cyclone Underflow	0	39	4996.03	9575.4	11824.6	0	11824.6	
12	44.3652	606436	Cyclone Underflow	0	39	5019.62	9620.6	11880.4	0	11880.4	
13	23.4132	316539	Liner Interface Zone	0	26.5	3196.81	6127.01	12445	156.077	12288.9	
14	46.4639	615182	Foundation Materials	150	29	3573.33	6848.64	12084.7	0	12084.7	
15	46.4639	590270	Foundation Materials	150	29	3445.2	6603.07	11641.7	0	11641.7	
16	46.4639	555377	Foundation Materials	150	29	3255.27	6239.06	10985	0	10985	
17	46.4639	510675	Foundation Materials	150	29	3003.43	5756.37	10114.2	0	10114.2	
18	46.4639	456280	Foundation Materials	150	29	2689.22	5154.15	9027.73	0	9027.73	
19	46.4639	392275	Foundation Materials	150	29	2311.95	4431.08	7723.26	0	7723.26	
20	46.4639	318701	Foundation Materials	150	29	1870.65	3585.29	6197.43	0	6197.43	
21	46.4639	235654	Foundation Materials	150	29	1364.61	2615.41	4447.71	0	4447.71	
22	46.4639	146214	Foundation Materials	150	29	861.912	1651.94	2709.57	0	2709.57	
23	46.4639	98503.8	Foundation Materials	150	29	693.875	1329.88	2128.55	0	2128.55	
24	46.4639	56028.4	Foundation Materials	150	29	408.79	783.486	1142.84	0	1142.84	
25	20.1633	6058.23	Liner Interface Zone	0	26.5	4794.2	9188.57	18585.2	155.762	18429.4	

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 1.9166



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	575.445	5460	1.98387e-022	0	0
2	619.81	5422.28	-162525	15041.2	-5.28749
3	664.175	5387.89	-170970	15822.8	-5.2875
4	708.54	5356.52	-168647	15607.8	-5.2875
5	752.906	5327.9	-167853	15534.3	-5.28749
6	797.271	5301.82	-178146	16486.9	-5.2875
7	841.636	5278.09	-206894	19147.4	-5.28749
8	886.001	5256.57	-259659	24030.7	-5.2875
9	930.366	5237.13	-340500	31512.3	-5.2875
10	974.731	5219.68	-452179	41847.8	-5.28749
11	1019.1	5204.13	-596323	55187.9	-5.28749
12	1063.46	5190.41	-773548	71589.6	-5.2875
13	1107.83	5178.45	-983538	91023.5	-5.28749
14	1131.24	5172.83	-1.06166e+006	98253.7	-5.2875
15	1177.7	5163.06	-1.26537e+006	117106	-5.28748
16	1224.17	5155.1	-1.50305e+006	139103	-5.2875
17	1270.63	5148.92	-1.77205e+006	163998	-5.28749
18	1317.1	5144.48	-2.06868e+006	191450	-5.28749
19	1363.56	5141.77	-2.38827e+006	221027	-5.28749
20	1410.02	5140.78	-2.7251e+006	252200	-5.28749
21	1456.49	5141.51	-3.07236e+006	284338	-5.2875
22	1502.95	5143.95	-3.42212e+006	316707	-5.28749
23	1549.42	5148.12	-3.68084e+006	340651	-5.2875
24	1595.88	5154.03	-3.72095e+006	344363	-5.28749
25	1642.34	5161.71	-3.78664e+006	350442	-5.28749
26	1662.51	5165.61	6.38612e-021	0	0

### List Of Coordinates

#### Piezoline

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195

732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81

213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79
362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164

1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27
1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85

1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95
1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42

2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03
2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455



441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455
530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01

570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92
696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

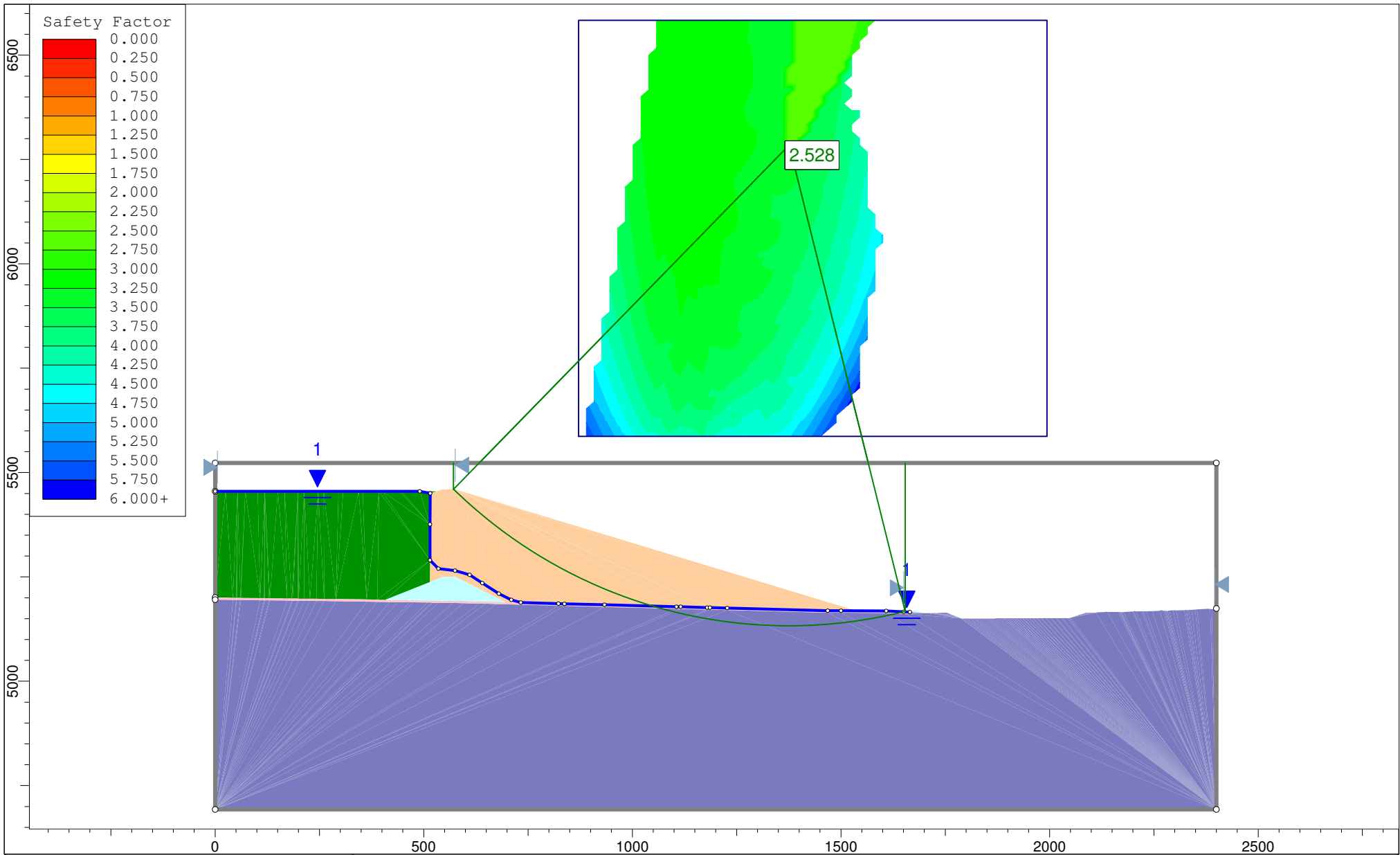
**Material Boundary**

X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455

**Material Boundary**

X	Y
514.508	5237.63

545.445	5250
575.445	5250
696.805	5189.32



SLIDEINTERPRET 6.008

<i>Project</i>		Copper Flat	
<i>Analysis Description</i>		Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength	
<i>Drawn By</i>	GS	<i>Scale</i>	1:3822
<i>Date</i>	11/4/2013, 3:02:21 PM	<i>Company</i>	Golder Associates Inc.
		<i>File Name</i>	6a - SectionA 5460R DS_S_C_G_postliq.slim

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 6a - SectionA 5460R DS\_S\_C\_G\_postliq.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section A Stability: Downstream, Static, Block Failure, Global, Post Liquefaction Strength  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 3:02:21 PM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

---

##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m_{\alpha} < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

---

Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None







## Random Numbers

Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Cyclone Overflow
Color						
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	108
Cohesion [psf]		0	0	150	0	
Friction Angle [deg]		39	29	29	26.5	
Tau/Sigma Ratio						0.05
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated	Automatically Calculated
Ru Value	0		0	0		

## Global Minimums

### Method: spencer

FS: 2.527620  
 Center: 1376.188, 6284.432  
 Radius: 1152.416  
 Left Slip Surface Endpoint: 570.971, 5460.000  
 Right Slip Surface Endpoint: 1654.244, 5166.064  
 Left Slope Intercept: 570.971 5522.990  
 Right Slope Intercept: 1654.244 5522.990  
 Resisting Moment=8.3306e+009 lb-ft  
 Driving Moment=3.29583e+009 lb-ft  
 Resisting Horizontal Force=6.74519e+006 lb

Driving Horizontal Force=2.6686e+006 lb

## Valid / Invalid Surfaces

### Method: spencer

Number of Valid Surfaces: 11802  
 Number of Invalid Surfaces: 29129

#### Error Codes:

- Error Code -101 reported for 408 surfaces
- Error Code -103 reported for 2326 surfaces
- Error Code -108 reported for 4262 surfaces
- Error Code -110 reported for 440 surfaces
- Error Code -111 reported for 3796 surfaces
- Error Code -1000 reported for 17897 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	47.6079	86369.8	Cyclone Underflow	0	39	377.344	953.782	1177.82	0	1177.82
2	47.6079	233141	Cyclone Underflow	0	39	1249.93	3159.35	3901.47	0	3901.47
3	47.6079	356305	Cyclone Underflow	0	39	1954.88	4941.2	6101.88	0	6101.88
4	47.6079	458937	Cyclone Underflow	0	39	2561.11	6473.51	7994.12	0	7994.12
5	47.6079	543134	Cyclone Underflow	0	39	3074.33	7770.73	9596.06	0	9596.06

6	47.6079	610541	Cyclone Underflow	0	39	3499.23	8844.72	10922.3	0	10922.3
7	47.6079	662469	Cyclone Underflow	0	39	3839.67	9705.22	11984.9	0	11984.9
8	47.6079	699974	Cyclone Underflow	0	39	4098.76	10360.1	12793.7	0	12793.7
9	47.6079	723916	Cyclone Underflow	0	39	4279.01	10815.7	13356.3	0	13356.3
10	47.6079	734997	Cyclone Underflow	0	39	4382.34	11076.9	13678.8	0	13678.8
11	19.0295	294789	Liner Interface Zone	0	26.5	2816.62	7119.35	14435.1	155.948	14279.2
12	43.9418	677489	Foundation Materials	150	29	3185.59	8051.96	14255.5	0	14255.5
13	43.9418	666065	Foundation Materials	150	29	3142.32	7942.59	14058.2	0	14058.2
14	43.9418	645159	Foundation Materials	150	29	3052.84	7716.42	13650.2	0	13650.2
15	43.9418	614993	Foundation Materials	150	29	2917.65	7374.71	13033.7	0	13033.7
16	43.9418	575744	Foundation Materials	150	29	2737.05	6918.22	12210.2	0	12210.2
17	43.9418	527526	Foundation Materials	150	29	2511.05	6346.98	11179.6	0	11179.6
18	43.9418	470419	Foundation Materials	150	29	2239.47	5660.53	9941.26	0	9941.26
19	43.9418	404462	Foundation Materials	150	29	1921.94	4857.93	8493.33	0	8493.33
20	43.9418	329659	Foundation Materials	150	29	1557.85	3937.65	6833.1	0	6833.1
21	43.9418	246078	Foundation Materials	150	29	1146.9	2898.93	4959.21	0	4959.21
22	43.9418	156715	Foundation Materials	150	29	744.38	1881.51	3123.73	0	3123.73
23	43.9418	104701	Foundation Materials	150	29	581.769	1470.49	2382.23	0	2382.23
24	43.9418	56711	Foundation Materials	150	29	328.39	830.044	1226.83	0	1226.83
25	16.9217	5084.26	Liner Interface Zone	0	26.5	4364.66	11032.2	22282.9	155.762	22127.2

**Interslice Data**

Global Minimum Query (spencer) - Safety Factor: 2.52762

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	570.971	5460	1.98387e-022	0	0
2	618.579	5416.05	-146699	13710.3	-5.33928



3	666.187	5376.71	-127697	11934.4	-5.33928
4	713.795	5341.41	-93312.8	8720.9	-5.33928
5	761.402	5309.7	-62734.3	5863.08	-5.33929
6	809.01	5281.25	-50034.2	4676.14	-5.33929
7	856.618	5255.79	-65433.1	6115.3	-5.33929
8	904.226	5233.09	-116155	10855.7	-5.33928
9	951.834	5212.99	-207020	19347.9	-5.3393
10	999.442	5195.34	-340860	31856.3	-5.33927
11	1047.05	5180.02	-518810	48487.4	-5.33929
12	1066.08	5174.52	-568241	53107.1	-5.33928
13	1110.02	5163.18	-727848	68023.8	-5.33929
14	1153.96	5153.65	-924444	86397.4	-5.33928
15	1197.9	5145.89	-1.15628e+006	108064	-5.33926
16	1241.85	5139.87	-1.42067e+006	132774	-5.33928
17	1285.79	5135.57	-1.71404e+006	160192	-5.33928
18	1329.73	5132.95	-2.03189e+006	189898	-5.33929
19	1373.67	5132.02	-2.36881e+006	221386	-5.33928
20	1417.61	5132.76	-2.71843e+006	254061	-5.33928
21	1461.56	5135.18	-3.07335e+006	287232	-5.33929
22	1505.5	5139.29	-3.42512e+006	320108	-5.33929
23	1549.44	5145.11	-3.67961e+006	343892	-5.33929
24	1593.38	5152.67	-3.72686e+006	348308	-5.33929
25	1637.32	5161.99	-3.7896e+006	354171	-5.33928
26	1654.24	5166.06	6.3698e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
0	5455
490.734	5455
515.445	5450
515.2	5376
514.77	5289.93
535	5270
575	5265
610	5255
640	5235
680	5210
710	5195
732.639	5188.36
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5

1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47

**External Boundary**

X	Y
-0.000101814	4692.74
2400	4692.74
2400	5173.98
2400	5522.99
0	5522.99
0	5455
0	5200.5
0	5195.5

**Material Boundary**

X	Y
0	5195.5
42.4655	5194.88
50.3797	5194.79
51.7799	5194.77
60.2917	5194.67
61.8068	5194.65
107.644	5194
117.161	5193.89
118.878	5193.87
129.13	5193.75
130.993	5193.72
172.665	5193.13
183.844	5193.01
185.901	5192.98
197.963	5192.84
200.207	5192.81
213.269	5192.67
227.292	5192.51
242.397	5192.34
261.303	5192.12
289.21	5191.79

362.895	5190.7
388.79	5190.29
418.529	5189.82
437.446	5189.55
466.866	5189.08
474.342	5188.97
509.241	5188.41
518.548	5188.28
521.461	5188.24
535.319	5188.01
570.113	5187.45
595.286	5187.04
632.38	5186.03
647.412	5185.63
674.306	5184.92
822.972	5180.92
837.238	5180.55
933.343	5177.96
1105.74	5173.5
1115.93	5173.23
1179.29	5171.55
1185.95	5171.38
1227.44	5170.27
1468.38	5164.13
1500.38	5164.02
1608.12	5163.61
1657.48	5160.88
1670.47	5165.17
1672.7	5165.9
1675.59	5166.86
1680.63	5166.86
1680.63	5168.86
1685.27	5168.86
1688.99	5167
1692.54	5165.23
1695.8	5163.83
1708.19	5163.3
1719.82	5162.97
1727.18	5162.95
1731.2	5162.93
1734.43	5164
1745.27	5164
1754.46	5164
1769.87	5157.85
1778.92	5154.23
1783.82	5152.27

1788.35	5150.47
1789.76	5150.47
1794.29	5150.46
1798.56	5150.46
1800.71	5150.46
1804.88	5150.46
1807.87	5150.46
1811.79	5150.46
1816.58	5150.46
1819.25	5150.47
1822.8	5150.47
1827.21	5150.47
1831.13	5150.48
1835.72	5150.49
1840.51	5150.49
1844.97	5150.5
1848.9	5150.51
1852.34	5150.52
1855.93	5150.53
1859.66	5150.54
1863.56	5150.55
1867.64	5150.56
1871.9	5150.57
1876.36	5150.59
1881.03	5150.6
1885.93	5150.62
1889.81	5150.64
1891.08	5150.64
1895.17	5150.66
1896.49	5150.67
1900.82	5150.68
1902.18	5150.69
1906.77	5150.71
1908.19	5150.72
1913.04	5150.74
1914.52	5150.75
1919.68	5150.77
1921.22	5150.78
1926.7	5150.81
1928.31	5150.82
1934.14	5150.85
1935.83	5150.86
1942.04	5150.89
1943.81	5150.9
1950.44	5150.94
1952.31	5150.95

1959.4	5151
1961.37	5151.01
1968.96	5151.06
1971.06	5151.07
1979.2	5151.12
1981.43	5151.14
1990.18	5151.2
1992.56	5151.22
2001.99	5151.28
2004.53	5151.3
2014.73	5151.37
2017.46	5151.39
2028.5	5151.48
2031.44	5151.5
2043.44	5151.59
2046.62	5151.61
2049.97	5151.64
2058.45	5154.47
2063.14	5156.03
2073.87	5159.6
2079.42	5161.46
2087.05	5164
2097.36	5164
2111.05	5164
2115.66	5165.28
2117.08	5165.67
2123.79	5165.76
2138.52	5166
2164.62	5166.2
2167.74	5166.23
2170.31	5166.24
2171.83	5166.27
2176.22	5166.42
2199.06	5167.14
2202.11	5167.23
2205.69	5167.35
2207.13	5167.37
2221.2	5167.76
2224.83	5167.83
2231.54	5168
2246.4	5168.42
2264.49	5168.76
2267.13	5168.9
2271.07	5168.97
2274.87	5169.01
2277.68	5169.03

2280.46	5169.04
2283.27	5169.13
2286.68	5169.27
2296.4	5169.81
2303.08	5170.12
2323.1	5170.72
2326.63	5170.79
2329.88	5170.83
2333.96	5170.96
2340.37	5171.27
2348.81	5171.75
2353.59	5172
2382.36	5173.52
2385.05	5173.4
2388.88	5173.53
2393.87	5173.83
2396.71	5174
2400	5173.98

**Material Boundary**

X	Y
0	5455
5.2718	5455
24.9095	5455
53.5041	5455
72.0583	5455
101.782	5455
119.16	5455
150.112	5455
166.208	5455
198.502	5455
213.195	5455
246.959	5455
260.112	5455
295.495	5455
306.947	5455
344.123	5455
353.689	5455
392.858	5455
400.319	5455
441.72	5455
446.819	5455
490.734	5455
517.25	5455
518.848	5455

530.445	5455
545.445	5460
556.072	5460
575.445	5460
1536.14	5168.88

**Material Boundary**

X	Y
0	5200.5
42.5304	5199.88
50.4444	5199.79
51.8447	5199.77
60.3563	5199.67
61.8783	5199.65
107.708	5199
117.225	5198.89
118.942	5198.86
129.194	5198.75
131.064	5198.72
172.729	5198.13
183.907	5198.01
185.964	5197.98
198.026	5197.84
200.27	5197.81
213.325	5197.67
227.348	5197.51
242.453	5197.34
261.362	5197.12
289.277	5196.79
362.969	5195.7
388.869	5195.29
407.909	5194.99
418.604	5194.82
437.521	5194.55
466.941	5194.08
474.418	5193.97
509.317	5193.41
518.618	5193.28
521.537	5193.24
535.401	5193.01
570.193	5192.45
595.395	5192.04
632.517	5191.03
647.545	5190.63
674.437	5189.92

696.805	5189.32
823.107	5185.92
837.37	5185.54
933.475	5182.95
1105.87	5178.5
1116.06	5178.23
1179.42	5176.55
1186.08	5176.37
1227.57	5175.27
1468.45	5169.13
1500.4	5169.02
1536.14	5168.88
1608.27	5168.61
1650.32	5166.28
1665.01	5165.47
1672.07	5167.8
1675.27	5168.86
1680.63	5168.86

**Material Boundary**

X	Y
1670.47	5165.17
1685.26	5164.35
1695.8	5163.83

**Material Boundary**

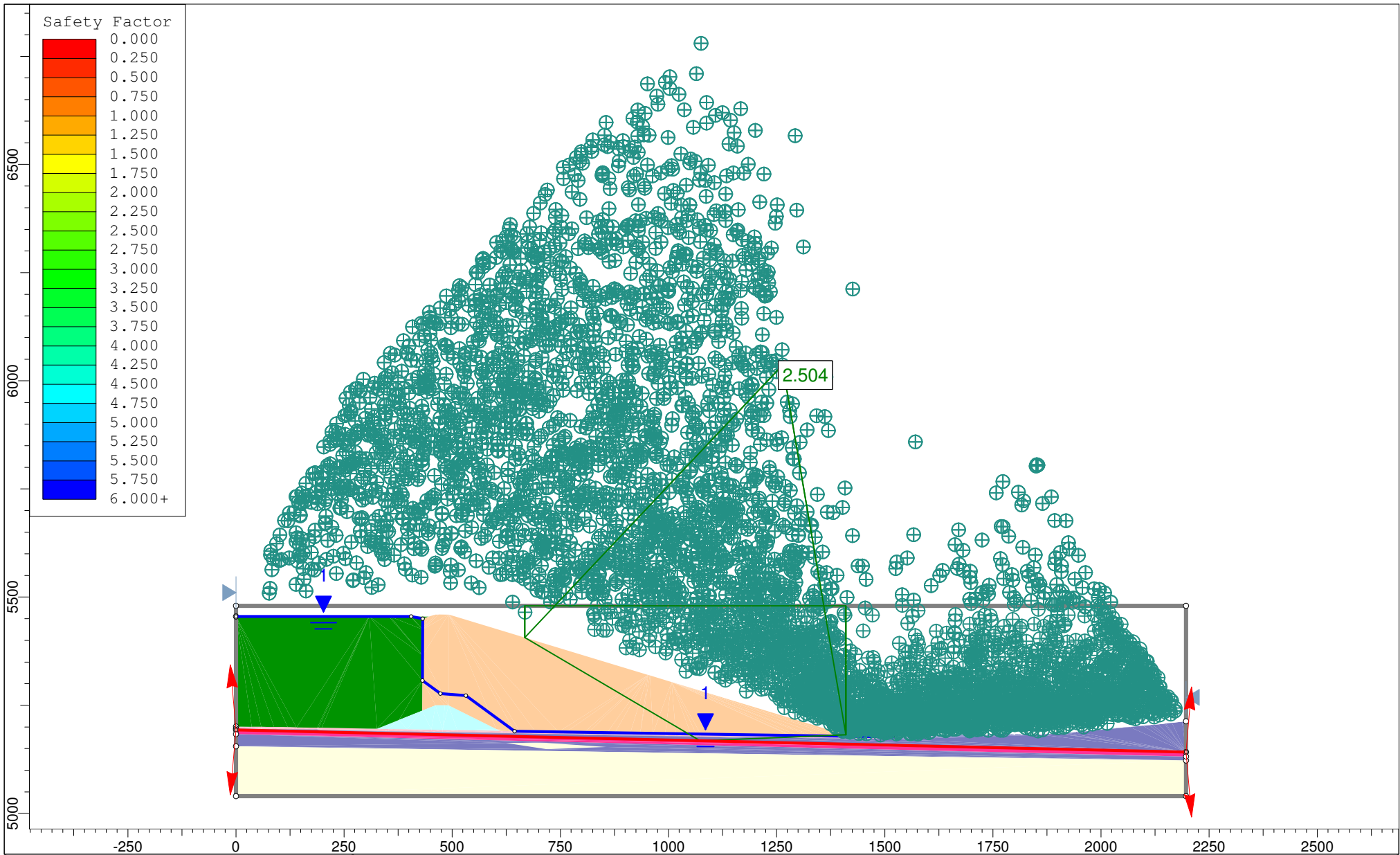
X	Y
407.909	5194.99
514.508	5237.63
514.77	5289.93
515.2	5376
515.445	5450
519.684	5451.41
530.445	5455


**Material Boundary**

X	Y
514.508	5237.63
545.445	5250
575.445	5250
696.805	5189.32



**APPENDIX H.2  
STABILITY SECTION B-B'**



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Static, Block Failure		
	Drawn By	GS	Scale	1:3686	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	1 - Section B 5460R - DS_S_B.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

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File Name: 1 - Section B 5460R - DS\_S\_B.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Static, Block Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/13

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

Spencer

Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check  $m\alpha < 0.2$ : Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>  
Advanced Groundwater Method: None

#### **Random Numbers**









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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft <sup>3</sup> ]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765

2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5

22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7

42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6

62656.7	15211.8
---------	---------

## Global Minimums

---

### Method: spencer

FS: 2.504180  
 Axis Location: 1263.734, 6036.472  
 Left Slip Surface Endpoint: 667.648, 5406.642  
 Right Slip Surface Endpoint: 1409.947, 5181.705  
 Left Slope Intercept: 667.648 5480.000  
 Right Slope Intercept: 1409.947 5480.000  
 Resisting Moment=2.81387e+009 lb-ft  
 Driving Moment=1.12367e+009 lb-ft  
 Resisting Horizontal Force=3.03106e+006 lb  
 Driving Horizontal Force=1.2104e+006 lb

## Global Minimum Coordinates

---

### Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

## Valid / Invalid Surfaces

---

### Method: spencer

Number of Valid Surfaces: 584  
 Number of Invalid Surfaces: 4416

#### Error Codes:

Error Code -106 reported for 2 surfaces  
 Error Code -107 reported for 1331 surfaces  
 Error Code -108 reported for 2123 surfaces  
 Error Code -110 reported for 70 surfaces  
 Error Code -111 reported for 595 surfaces  
 Error Code -112 reported for 295 surfaces

#### Error Codes

The following errors were encountered during the computation:

- 106 = Average slice width is less than 0.0001 \* (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the



driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient  $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$  for the final iteration of the safety factor calculation.

This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

## Slice Data

### Global Minimum Query (spencer) - Safety Factor: 2.50418

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	25.4457	63.7207	78.6884	0	78.6884
2	31.651	48443.9	Cyclone Underflow	0	39	393.5	985.395	1216.86	0	1216.86
3	31.651	80739.8	Cyclone Underflow	0	39	671.529	1681.63	2076.63	0	2076.63
4	31.651	113036	Cyclone Underflow	0	39	949.556	2377.86	2936.4	0	2936.4
5	31.651	145332	Cyclone Underflow	0	39	1227.58	3074.09	3796.18	0	3796.18
6	31.651	177628	Cyclone Underflow	0	39	1505.61	3770.32	4655.95	0	4655.95
7	31.651	209923	Cyclone Underflow	0	39	1783.64	4466.55	5515.73	0	5515.73
8	31.651	242219	Cyclone Underflow	0	39	2061.66	5162.78	6375.5	0	6375.5
9	31.651	274515	Cyclone Underflow	0	39	2339.69	5859.01	7235.27	0	7235.27
10	31.651	306847	Cyclone Underflow	0	39	2618.13	6556.28	8096.33	0	8096.33
11	31.651	339198	Cyclone Underflow	0	39	2896.61	7253.64	8957.49	0	8957.49
12	31.651	371504	Cyclone Underflow	0	39	3174.66	7949.91	9817.32	0	9817.32
13	8.75148	108572	Liner Interface Zone	0	26.5	2185.06	5471.78	11170.6	195.919	10974.6
14	16.9638	218358	Foundation Materials	150	29	2579.64	6459.89	11383.3	0	11383.3
15	30.2226	380408	Foundation Materials	150	29	2774.58	6948.05	12264	0	12264
16	30.2226	344600	Foundation Materials	150	29	2511.83	6290.08	11077	0	11077
17	30.2226	308792	Foundation Materials	150	29	2249.09	5632.12	9890.02	0	9890.02
18	30.2226	272984	Foundation Materials	150	29	1986.34	4974.16	8703	0	8703
19	30.2226	237176	Foundation Materials	150	29	1723.6	4316.2	7516.03	0	7516.03
20	30.2226	201368	Foundation Materials	150	29	1460.85	3658.23	6329.01	0	6329.01
21	31.0757	169713	Liner Interface Zone	0	26.5	967.095	2421.78	5130.02	272.678	4857.34
22	31.0757	131843	Liner Interface Zone	0	26.5	745.37	1866.54	3908.11	164.386	3743.72
23	31.0757	93991.2	Liner Interface Zone	0	26.5	523.772	1311.62	2686.8	56.0937	2630.7
24	31.1043	56349.3	Cyclone Underflow	0	39	474.407	1188	1467.06	0	1467.06
25	31.1043	18783.1	Cyclone Underflow	0	39	1844.88	4619.92	5705.13	0	5705.13

## Interslice Data

**Global Minimum Query (spencer) - Safety Factor: 2.50418**

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214017	13186.2	-3.5257
3	730.95	5369.4	-256320	15792.6	-3.5257
4	762.601	5350.78	-297148	18308.2	-3.52571
5	794.252	5332.16	-336500	20732.8	-3.52571
6	825.903	5313.53	-374377	23066.5	-3.52571
7	857.554	5294.91	-410779	25309.3	-3.5257
8	889.205	5276.29	-445705	27461.2	-3.52571
9	920.856	5257.67	-479156	29522.2	-3.5257
10	952.507	5239.05	-511131	31492.3	-3.52571
11	984.158	5220.43	-541429	33359	-3.5257
12	1015.81	5201.81	-570296	35137.6	-3.5257
13	1047.46	5183.19	-597825	36833.8	-3.52571
14	1056.21	5178.04	-590828	36402.6	-3.5257
15	1073.18	5168.06	-583080	35925.3	-3.52571
16	1103.4	5169.28	-796681	49085.8	-3.5257
17	1133.62	5170.51	-1.00613e+006	61990.4	-3.52569
18	1163.84	5171.73	-1.21142e+006	74639.1	-3.5257
19	1194.07	5172.96	-1.41256e+006	87031.9	-3.5257
20	1224.29	5174.18	-1.60955e+006	99168.8	-3.52569
21	1254.51	5175.41	-1.80238e+006	111050	-3.52571
22	1285.59	5176.66	-1.98927e+006	122565	-3.52571
23	1316.66	5177.92	-2.17327e+006	133902	-3.52572
24	1347.74	5179.18	-2.35439e+006	145061	-3.52571
25	1378.84	5180.44	-2.53815e+006	156383	-3.52571
26	1409.95	5181.7	4.449e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

**Block Search Lines**

X	Y

0	5193.11
2196.33	5141.83

**External Boundary**

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28

814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18

745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77

1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13

1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250



491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

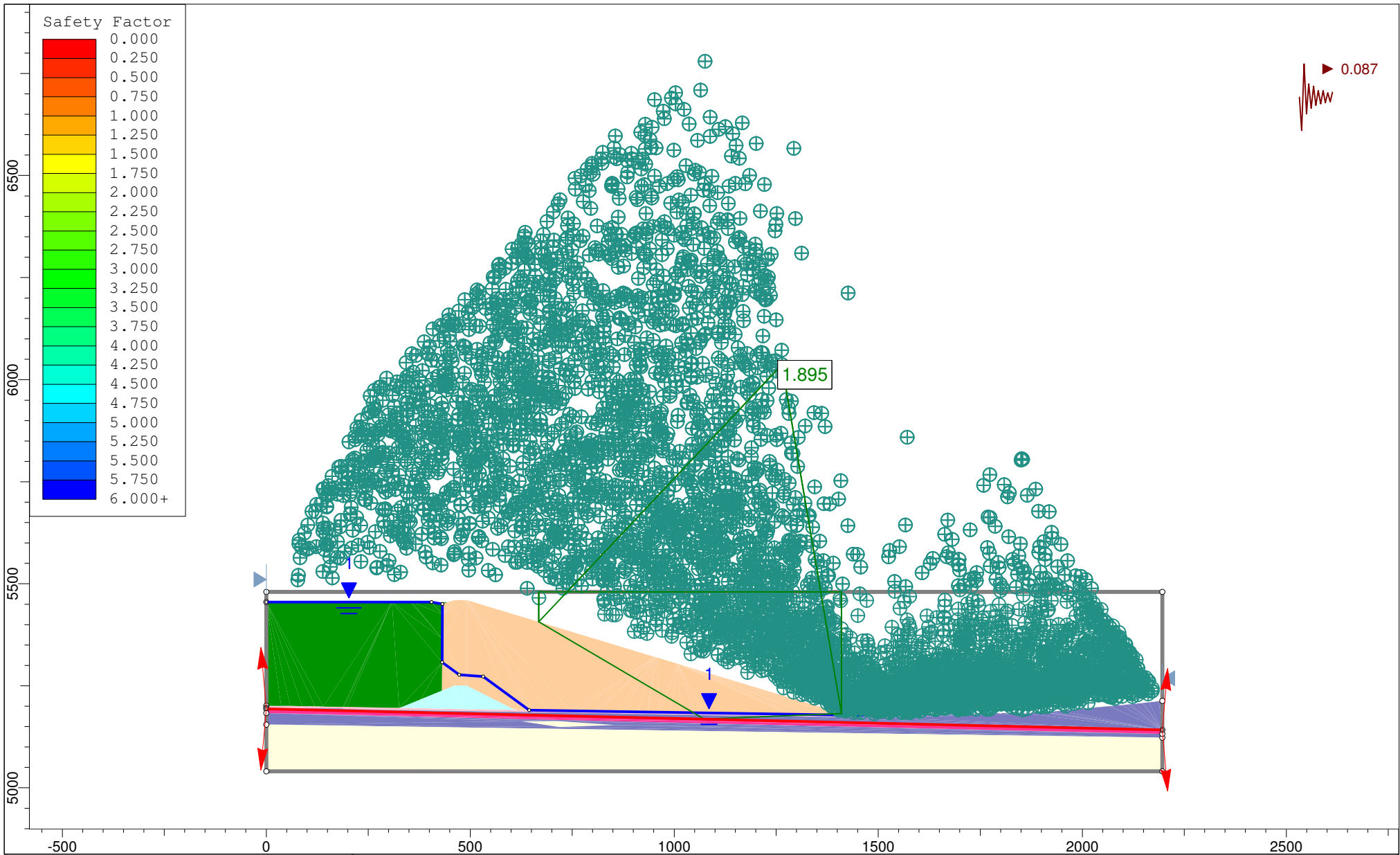
X	Y
0	5154.97
2196.33	5122.6


**Material Boundary**

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Pseudo Static, Block Failure		
	Drawn By	GS	Scale	1:3908	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	2 - Section B 5460R - DS_PS_B.slim	

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 2 - Section B 5460R - DS\_PS\_B.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Block Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/2013

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft3  
Advanced Groundwater Method: None

#### **Random Numbers**

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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3









## Surface Options

Surface Type: Non-Circular Block Search  
 Number of Surfaces: 5000  
 Pseudo-Random Surfaces: Enabled  
 Convex Surfaces Only: Disabled  
 Left Projection Angle (Start Angle): 95  
 Left Projection Angle (End Angle): 265  
 Right Projection Angle (Start Angle): 85  
 Right Projection Angle (End Angle): -85  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186

835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2

20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7

40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

## Global Minimums

---

### Method: spencer

FS: 1.895340  
 Axis Location: 1263.734, 6036.472  
 Left Slip Surface Endpoint: 667.648, 5406.642  
 Right Slip Surface Endpoint: 1409.947, 5181.705  
 Left Slope Intercept: 667.648 5480.000  
 Right Slope Intercept: 1409.947 5480.000  
 Resisting Moment=2.74873e+009 lb-ft  
 Driving Moment=1.45026e+009 lb-ft  
 Resisting Horizontal Force=2.96703e+006 lb  
 Driving Horizontal Force=1.56543e+006 lb

## Global Minimum Coordinates

---

### Method: spencer

X	Y
667.648	5406.64
1073.18	5168.06
1409.95	5181.7
1409.95	5480

## Valid / Invalid Surfaces

---

### Method: spencer

Number of Valid Surfaces: 562  
 Number of Invalid Surfaces: 4438

#### Error Codes:

Error Code -106 reported for 2 surfaces  
 Error Code -107 reported for 578 surfaces  
 Error Code -108 reported for 2726 surfaces  
 Error Code -110 reported for 101 surfaces  
 Error Code -111 reported for 671 surfaces  
 Error Code -112 reported for 360 surfaces

#### Error Codes

The following errors were encountered during the computation:



- 106 = Average slice width is less than 0.0001 \* (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge
- 112 = The coefficient M-Alpha =  $\cos(\alpha)(1+\tan(\alpha)\tan(\phi)/F) < 0.2$  for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

## Slice Data

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	31.651	16148	Cyclone Underflow	0	39	0	0	-57.4289	0	-57.4289
2	31.651	48443.9	Cyclone Underflow	0	39	484.264	917.844	1133.44	0	1133.44
3	31.651	80739.8	Cyclone Underflow	0	39	833.882	1580.49	1951.75	0	1951.75
4	31.651	113036	Cyclone Underflow	0	39	1183.5	2243.14	2770.05	0	2770.05
5	31.651	145332	Cyclone Underflow	0	39	1533.12	2905.79	3588.35	0	3588.35
6	31.651	177628	Cyclone Underflow	0	39	1882.74	3568.44	4406.65	0	4406.65
7	31.651	209923	Cyclone Underflow	0	39	2232.36	4231.08	5224.95	0	5224.95
8	31.651	242219	Cyclone Underflow	0	39	2581.98	4893.73	6043.25	0	6043.25
9	31.651	274515	Cyclone Underflow	0	39	2931.6	5556.38	6861.55	0	6861.55
10	31.651	306847	Cyclone Underflow	0	39	3281.79	6220.1	7681.19	0	7681.19
11	31.651	339198	Cyclone Underflow	0	39	3631.96	6883.8	8500.79	0	8500.79
12	31.651	371504	Cyclone Underflow	0	39	3981.57	7546.43	9319.07	0	9319.07
13	8.75148	108572	Liner Interface Zone	0	26.5	2802.54	5311.77	10849.7	195.919	10653.8
14	16.9638	218358	Foundation Materials	150	29	3293.35	6242.01	10990.3	0	10990.3
15	30.2226	380408	Foundation Materials	150	29	3636.26	6891.94	12162.8	0	12162.8
16	30.2226	344600	Foundation Materials	150	29	3288.53	6232.88	10973.8	0	10973.8
17	30.2226	308792	Foundation Materials	150	29	2940.8	5573.81	9784.82	0	9784.82
18	30.2226	272984	Foundation Materials	150	29	2593.07	4914.75	8595.82	0	8595.82
19	30.2226	237176	Foundation Materials	150	29	2245.34	4255.68	7406.86	0	7406.86
20	30.2226	201368	Foundation Materials	150	29	1897.61	3596.62	6217.86	0	6217.86
21	31.0757	169713	Liner Interface Zone	0	26.5	1250.03	2369.24	5024.65	272.678	4751.98
22	31.0757	131843	Liner Interface Zone	0	26.5	956.103	1812.14	3798.98	164.386	3634.59
23	31.0757	93991.2	Liner Interface Zone	0	26.5	662.33	1255.34	2573.92	56.0937	2517.83
24	31.1043	56349.3	Cyclone Underflow	0	39	574.915	1089.66	1345.62	0	1345.62
25	31.1043	18783.1	Cyclone Underflow	0	39	3181.77	6030.54	7447.09	0	7447.09

## Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.89534

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	667.648	5406.64	2.69073e-022	0	0
2	699.299	5388.02	-214342	17977.3	-4.79429
3	730.95	5369.4	-256846	21542.3	-4.79432
4	762.601	5350.78	-298096	25002	-4.79431
5	794.252	5332.16	-338091	28356.5	-4.79431
6	825.903	5313.53	-376832	31605.8	-4.79431
7	857.554	5294.91	-414318	34749.8	-4.79431
8	889.205	5276.29	-450549	37788.6	-4.79431
9	920.856	5257.67	-485526	40722.2	-4.79431
10	952.507	5239.05	-519248	43550.5	-4.79431
11	984.158	5220.43	-551514	46256.8	-4.79431
12	1015.81	5201.81	-582571	48861.6	-4.79431
13	1047.46	5183.19	-612510	51372.6	-4.79431
14	1056.21	5178.04	-603106	50583.9	-4.79431
15	1073.18	5168.06	-592352	49682	-4.79432
16	1103.4	5169.28	-798697	66988.6	-4.79431
17	1133.62	5170.51	-1.00144e+006	83993.1	-4.79431
18	1163.84	5171.73	-1.20058e+006	100696	-4.79434
19	1194.07	5172.96	-1.39612e+006	117096	-4.79432
20	1224.29	5174.18	-1.58806e+006	133194	-4.7943
21	1254.51	5175.41	-1.7764e+006	148990	-4.79428
22	1285.59	5176.66	-1.95716e+006	164151	-4.79429
23	1316.66	5177.92	-2.13608e+006	179158	-4.79431
24	1347.74	5179.18	-2.31317e+006	194011	-4.79431
25	1378.84	5180.44	-2.49498e+006	209260	-4.79431
26	1409.95	5181.7	4.449e-021	0	0

## List Of Coordinates

### Piezoline

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

**Block Search Lines**

X	Y
0	5193.11
2196.33	5141.83

**External Boundary**

X	Y
2196.33	5040
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	5040

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04
87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8

692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69

617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17

1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178
1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73

1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32
1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89

1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07
431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y



325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

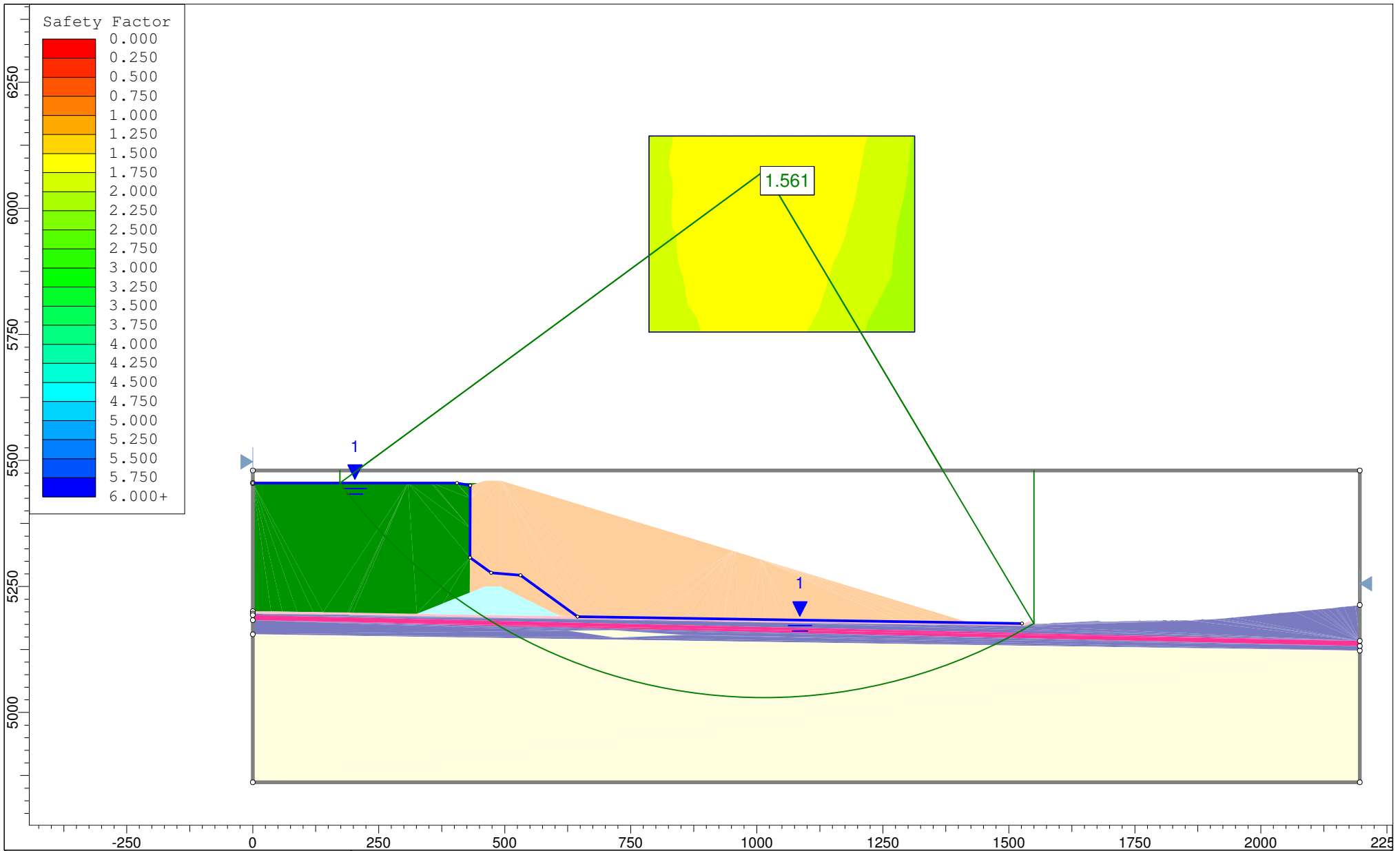
X	Y
0	5154.97
2196.33	5122.6


**Material Boundary**

X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54
712.872	5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat		
	Analysis Description			Section B-B' Stability: Downstream, Static, Circular Failure		
	Drawn By	GS	Scale	1:3149	Company	Golder Associates Inc.
	Date	11/4/2013, 11:49:04 AM		File Name	3 - Section B 5460R - DS_S_C.slim	
	SLIDEINTERPRET 6.008					

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

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File Name: 3 - Section B 5460R - DS\_S\_C.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Static, Circular Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
    103-92557  
    Material Property Edits 12/2013

#### **General Settings**

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Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

    Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft3  
Advanced Groundwater Method: None

#### **Random Numbers**









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Pseudo-random Seed: 10116  
 Random Number Generation Method: Park and Miller v.3

## Surface Options

Surface Type: Circular  
 Search Method: Grid Search  
 Radius Increment: 10  
 Composite Surfaces: Disabled  
 Reverse Curvature: Create Tension Crack  
 Minimum Elevation: Not Defined  
 Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Soft Clay	Clay	Cyclone Overflow
Color								
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Shear Normal function	Strength=F(overburden)
Unit								
Weight [lbs/ft3]	1e-025	113	120	120	120	127	127	108
Cohesion [psf]		0	0	150	0			
Friction Angle [deg]		39	29	29	26.5			
Tau/Sigma Ratio								0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated			Automatically Calculated
Ru Value	0		0	0		0	0	

## Shear Normal Functions

Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3

3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8

43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

**Global Minimums**

**Method: spencer**

FS: 1.560970  
 Center: 1015.357, 6074.806  
 Radius: 1045.828  
 Left Slip Surface Endpoint: 173.020, 5454.947  
 Right Slip Surface Endpoint: 1550.009, 5175.971  
 Left Slope Intercept: 173.020 5480.000  
 Right Slope Intercept: 1550.009 5480.000  
 Resisting Moment=1.06294e+010 lb-ft  
 Driving Moment=6.80947e+009 lb-ft  
 Resisting Horizontal Force=9.56258e+006 lb  
 Driving Horizontal Force=6.12606e+006 lb

**Valid / Invalid Surfaces**

**Method: spencer**

Number of Valid Surfaces: 2506  
 Number of Invalid Surfaces: 2246

**Error Codes:**

Error Code -103 reported for 588 surfaces  
 Error Code -108 reported for 568 surfaces  
 Error Code -110 reported for 58 surfaces  
 Error Code -111 reported for 1032 surfaces

**Error Codes**

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

**Slice Data**

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	76.7812	384179	Cyclone Overflow	421.855	0	270.252	421.855	3388.77	2894.28	494.494
2	76.7812	1.07712e+006	Cyclone Overflow	1183.95	0	758.471	1183.95	10074.5	8108.62	1965.9
3	76.7812	1.63914e+006	Cyclone Overflow	1802.07	0	1154.46	1802.07	16242.9	12337.8	3905.11
4	49.0448	1.33572e+006	Structural Fill Liner Interface	0	29	7124.22	11120.7	20062.4	0	20062.4



			Zone							
6	9.92233	312599	Foundation Materials	150	29	8606.51	13434.5	23965.9	0	23965.9
7	17.4329	567165	Soft Clay	851.163	13.3468	4488.85	7006.95	25946.3	0	25946.3
8	44.5694	1.5136e+006	Foundation Materials	150	29	9663.09	15083.8	26941.3	0	26941.3
9	67.0136	2.36955e+006	Clay	958.267	13.1388	5094.31	7952.07	29962.1	0	29962.1
10	67.0136	2.45379e+006	Clay	1025.15	13.0217	5445.36	8500.05	32321.5	0	32321.5
11	67.0136	2.50548e+006	Clay	1089.15	12.9174	5739.42	8959.07	34313.8	0	34313.8
12	67.0136	2.50972e+006	Clay	1124.79	12.8623	5943.34	9277.38	35704	0	35704
13	67.0136	2.47166e+006	Clay	1148.83	12.8262	6062.3	9463.07	36518	0	36518
14	67.0136	2.39734e+006	Clay	1148.83	12.8262	6103.92	9528.04	36803.3	0	36803.3
15	67.0136	2.28693e+006	Clay	1148.83	12.8262	6063.17	9464.42	36524	0	36524
16	67.0136	2.13981e+006	Clay	1124.79	12.8623	5933.03	9261.28	35633.4	0	35633.4
17	67.0136	1.95593e+006	Clay	1072.75	12.9435	5704.6	8904.71	34077.2	0	34077.2
18	67.0136	1.7351e+006	Clay	1017.45	13.0347	5367	8377.72	31793.1	0	31793.1
19	67.0136	1.47639e+006	Clay	923.265	13.2038	4903.36	7653.99	28688	0	28688
20	67.0136	1.1784e+006	Clay	802.785	13.4506	4288.2	6693.74	24631.1	0	24631.1
21	67.0136	839198	Clay	656.503	13.8133	3481.23	5434.09	19431.3	0	19431.3
22	67.0136	496697	Clay	415.25	14.6733	2204.49	3441.14	11555.9	0	11555.9
23	29.3922	131593	Foundation Materials	150	29	3334.74	5205.43	9120.25	0	9120.25
24	17.8998	52163.6	Soft Clay	192.084	16.2309	1043.45	1628.79	4935.28	0	4935.28
25	32.0382	41314	Foundation Materials	150	29	0	0	-38306.9	0	-38306.9

### Interslice Data

Global Minimum Query (spencer) - Safety Factor: 1.56097

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	173.02	5454.95	3.13838e-023	0	0
2	249.802	5362.29	273636	97200.6	19.556
3	326.583	5287.82	965550	342981	19.556
4	403.364	5226.74	1.86896e+006	663889	19.556
5	452.409	5193.42	2.19051e+006	778111	19.556
6	460.564	5188.26	2.26451e+006	804395	19.556
7	470.486	5182.13	2.32639e+006	826376	19.556
8	487.919	5171.72	2.51812e+006	894484	19.556
9	532.488	5147.12	2.72923e+006	969472	19.556
10	599.502	5115.21	3.28961e+006	1.16853e+006	19.556
11	666.515	5088.87	3.69589e+006	1.31285e+006	19.556
12	733.529	5067.67	3.93299e+006	1.39707e+006	19.556
13	800.543	5051.28	3.98821e+006	1.41669e+006	19.556
14	867.556	5039.47	3.85556e+006	1.36957e+006	19.556
15	934.57	5032.1	3.53467e+006	1.25558e+006	19.556
16	1001.58	5029.07	3.03059e+006	1.07652e+006	19.556
17	1068.6	5030.33	2.35334e+006	835950	19.556

18	1135.61	5035.91	1.52056e+006	540130	19.5559
19	1202.62	5045.88	558004	198213	19.5559
20	1269.64	5060.36	-497755	-176812	19.556
21	1336.65	5079.55	-1.59529e+006	-566675	19.5559
22	1403.67	5103.74	-2.6616e+006	-945449	19.556
23	1470.68	5133.3	-3.26006e+006	-1.15803e+006	19.5559
24	1500.07	5148.09	-3.50629e+006	-1.2455e+006	19.556
25	1517.97	5157.67	-3.58004e+006	-1.2717e+006	19.556
26	1550.01	5175.97	4.62169e-021	0	0

**List Of Coordinates**

**Piezoline**

X	Y
5.68e-014	5455
405.014	5455
431.537	5450
430.947	5307.07
472.744	5276.98
531.255	5272.32
644.321	5190
1526.22	5176.48

**External Boundary**

X	Y
2196.33	4861.27
2196.33	5122.6
2196.33	5131.83
2196.33	5141.83
2196.33	5213.03
2196.33	5480
0	5480
5.68434e-014	5454.95
0	5200.91
0	5195.91
0	5193.11
0	5183.11
0	5154.97
0	4861.27

**Material Boundary**

X	Y
0	5200.91
36.2762	5200.37
58.5636	5200.04

87.9203	5199.6
119.749	5199.12
139.564	5198.83
187.898	5198.1
191.209	5198.05
198.411	5197.94
325.671	5195.68
352.127	5195.21
395.214	5194.44
451.434	5193.44
477.974	5192.93
511.46	5192.3
610.156	5190.74
613.497	5190.69
617.098	5190.63
644.321	5190
655.674	5189.8
692.306	5189.18
745.298	5188.28
762.917	5188
805.444	5187.28
814.51	5187.13
858.316	5186.39
875.648	5186.09
911.187	5185.49
934.659	5185.09
964.059	5184.6
992.27	5184.12
1016.93	5183.7
1051.65	5183.11
1064.32	5182.9
1074.91	5182.72
1281.77	5180.05
1284.77	5180
1300.41	5179.72
1324.81	5179.5
1334.95	5179.32
1357.91	5179.08
1383.32	5178.96
1399.69	5178.64
1420.91	5178.38
1449.11	5178.05
1457.41	5177.99
1463.99	5177.94
1481.34	5177.51
1528.55	5176.42

**Material Boundary**

X	Y
0	5195.91
36.2013	5195.37
58.4887	5195.04
87.8455	5194.6
119.674	5194.12
139.49	5193.83
187.823	5193.1
191.134	5193.05
198.329	5192.94
352.038	5190.21
395.126	5189.44
451.342	5188.44
477.88	5187.93
511.373	5187.3
613.418	5185.69
617.004	5185.63
644.218	5185
655.588	5184.8
692.222	5184.18
745.216	5183.28
762.837	5183
805.359	5182.28
814.425	5182.13
858.231	5181.39
875.564	5181.09
911.103	5180.49
934.574	5180.09
963.974	5179.6
992.185	5179.12
1016.85	5178.7
1051.56	5178.12
1064.24	5177.9
1074.84	5177.72
1281.7	5175.05
1284.68	5175
1300.34	5174.72
1324.74	5174.5
1334.87	5174.32
1357.87	5174.08
1383.26	5173.96
1399.61	5173.64
1449.07	5173.05
1457.37	5172.99
1463.91	5172.95
1481.22	5172.51
1529.3	5171.41

**Material Boundary**

X	Y
1528.55	5176.42
1530.07	5176.92
1540.17	5180.24
1544.4	5180.22
1545.81	5180.22
1550.23	5180.21
1558.65	5175.99

**Material Boundary**

X	Y
1529.3	5171.41
1531.63	5172.17
1540.96	5175.24
1544.53	5175.72

**Material Boundary**

X	Y
5.68434e-014	5454.95
308.07	5454.95
353.349	5454.95
356.571	5454.95
403.471	5454.95
405.014	5454.95
446.545	5454.95

**Material Boundary**

X	Y
1544.53	5175.72
1551.96	5176.06
1555.99	5176
1558.65	5175.99
1560.71	5175.98
1562.45	5175.97
1564.47	5175.96
1567.8	5175.98
1568.92	5175.98
1570.4	5176
1572.7	5176.05
1575.12	5176.1
1577.64	5176.05
1584.69	5177.23
1587.52	5178
1589.43	5178

1595.33	5178.02
1598	5178.02
1599.51	5178.02
1601.92	5178.01
1604.21	5178.01
1606.06	5178.02
1611.05	5178.02
1612.65	5178.02
1615.58	5178.03
1624.1	5178.74
1634.72	5179.56
1676.06	5181.61
1677.91	5181.69
1679.85	5181.31
1689.42	5179.73
1695.55	5179.88
1719.81	5180.46
1722.1	5180.51
1724.26	5180.55
1729.33	5180.77
1736.44	5181.05
1758.17	5181.51
1760.86	5181.57
1763.13	5181.59
1770	5181.77
1776.96	5182
1778.16	5182
1781.4	5182
1786.04	5182
1790.15	5181.96
1792.18	5181.97
1793.61	5181.99
1797.47	5181.99
1801.67	5181.97
1805.2	5181.98
1807.73	5182
1809.68	5182.03
1810.98	5182.03
1812.91	5182
1815.24	5182
1817.46	5182
1820.77	5182
1825.08	5182.06
1827.32	5182.1
1830.04	5182.06
1834	5182.21
1835.24	5182.23
1837.14	5182.32

1847.73	5182.9
1853.36	5183.11
1862.09	5181.97
1863.34	5182
1868.11	5183.18
1878.92	5184
1881.28	5184.04
1882.78	5184.04
1886.78	5184.25
1895.53	5184.46
1902.77	5184.55
1904.49	5184.67
1907.81	5184.94
1915.56	5185.57
1918.57	5185.89
1922.54	5186
1936.83	5187.52
1942.58	5188
1944.05	5188.08
1945.48	5188.13
1980.15	5191.45
1985.49	5192
2003.82	5193.85
2005.48	5194.02
2024.8	5196
2030.13	5196.52
2035.72	5197.07
2041.65	5197.59
2046.56	5198.01
2056.52	5198.85
2066.29	5199.61
2077.93	5200.73
2090.84	5201.99
2097.45	5202.54
2111.64	5204
2118.78	5204.74
2132.35	5206
2146.46	5207.55
2150.18	5208
2163.74	5209.5
2187.21	5212
2196.33	5213.03

**Material Boundary**

X	Y
430.6	5237.63
430.947	5307.07

431.24	5365.55
431.537	5425
431.537	5449.95
446.545	5454.95
461.554	5460
468.536	5460
491.57	5460
952.795	5320.23
956.93	5318.98
1007.34	5303.73
1420.91	5178.38

**Material Boundary**

X	Y
325.671	5195.68
377.088	5216.23
430.6	5237.63
461.554	5250
482.872	5250
491.57	5250
601.751	5194.94
610.156	5190.74

**Material Boundary**

X	Y
0	5193.11
2196.33	5141.83

**Material Boundary**

X	Y
0	5183.11
2196.33	5131.83

**Material Boundary**

X	Y
0	5154.97
2196.33	5122.6

**Material Boundary**

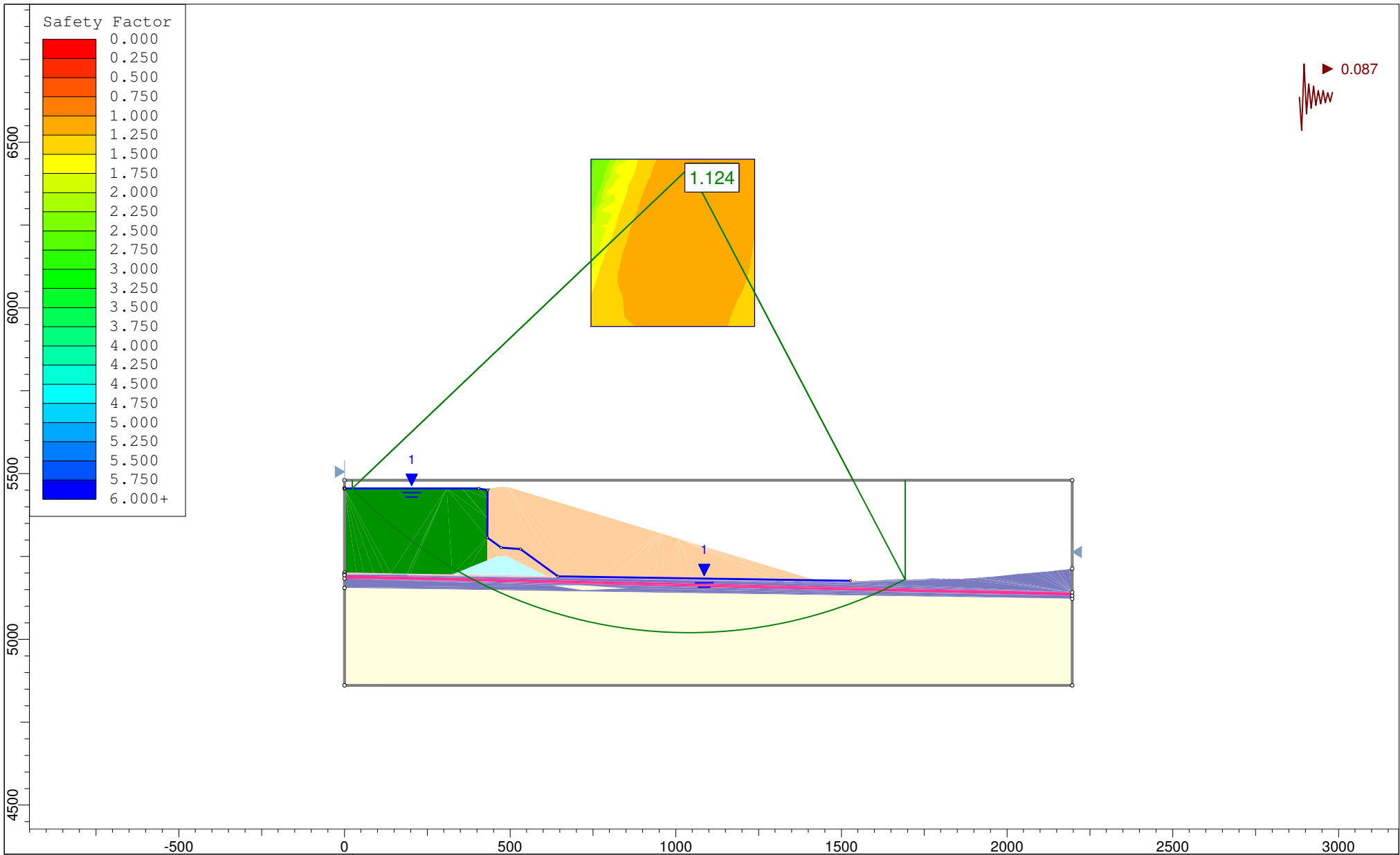
X	Y
712.872	5163.35
624.336	5162.59
718.366	5148.52
856.143	5153.54




712.872 5163.35

**Material Boundary**

X	Y
1544.4	5180.22
1544.53	5175.72



	Project			Copper Flat																			
	Analysis Description						Section B-B' Stability: Downstream, Pseudo Static, Circular Failure																
	Drawn By			GS			Scale			1:4812			Company			Golder Associates Inc.							
	Date						11/4/2013, 11:49:04 AM						File Name						4 - Section B 5460R - DS_PS_C.slim				

SLIDEINTERPRET 6.008

## **Slide Analysis Information**

### **Copper Flat**

#### **Project Summary**

---

File Name: 4 - Section B 5460R - DS\_PS\_C.slim  
Slide Modeler Version: 6.008  
Project Title: Copper Flat  
Analysis: Section B-B' Stability: Downstream, Pseudo Static, Circular Failure  
Author: GS  
Company: Golder Associates Inc.  
Date Created: 11/4/2013, 11:49:04 AM  
Comments:  
103-92557  
Material Property Edits 12/2013

#### **General Settings**

---

Units of Measurement: Imperial Units  
Time Units: days  
Permeability Units: feet/second  
Failure Direction: Left to Right  
Data Output: Standard  
Maximum Material Properties: 20  
Maximum Support Properties: 20

#### **Analysis Options**

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##### **Analysis Methods Used**

Spencer  
Number of slices: 25  
Tolerance: 0.005  
Maximum number of iterations: 50  
Check malpha < 0.2: Yes  
Initial trial value of FS: 1  
Steffensen Iteration: Yes

#### **Groundwater Analysis**

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Groundwater Method: Water Surfaces  
Pore Fluid Unit Weight: 62.4 lbs/ft3  
Advanced Groundwater Method: None

#### **Random Numbers**

---









0  
10  
20  
30  
40  
50  
60  
70  
80  
90  
100

**Slide Interpretation**

**Slide**      **Image**      **Description**

1            **Slide 1**  
 2            **Slide 2**  
 3            **Slide 3**  
 4            **Slide 4**



Element	Sample	Weight %	Standard	Concentration
Al	...	...	...	...
Ca	...	...	...	...
Fe	...	...	...	...
Mg	...	...	...	...
Mn	...	...	...	...
Ni	...	...	...	...
Pb	...	...	...	...
Si	...	...	...	...
Sr	...	...	...	...
Zn	...	...	...	...

Element	Sample	Weight %	Standard	Concentration
Al	...	...	...	...
Ca	...	...	...	...
Fe	...	...	...	...
Mg	...	...	...	...
Mn	...	...	...	...
Ni	...	...	...	...
Pb	...	...	...	...
Si	...	...	...	...
Sr	...	...	...	...
Zn	...	...	...	...

Table with 2 columns: Element, Concentration (ppm)

Element	Concentration (ppm)
Al	0.00
As	0.00
B	0.00
Be	0.00
Ba	0.00
Bi	0.00
Bk	0.00
Br	0.00
Bs	0.00
C	0.00
Ca	0.00
Ce	0.00
Cf	0.00
Cl	0.00
Co	0.00
Cs	0.00
Cu	0.00
D	0.00
Ds	0.00
Er	0.00
F	0.00
Fa	0.00
Fe	0.00
Ga	0.00
Gd	0.00
Ge	0.00
Gr	0.00
Gu	0.00
H	0.00
Hf	0.00
Hg	0.00
Ho	0.00
I	0.00
Ir	0.00
Is	0.00
Li	0.00
Lr	0.00
Lu	0.00
M	0.00
Mn	0.00
Mo	0.00
N	0.00
Nb	0.00
Nd	0.00
Ne	0.00
Ni	0.00
No	0.00
O	0.00
Os	0.00
P	0.00
Pb	0.00
Pd	0.00
Pf	0.00
Pg	0.00
Pt	0.00
R	0.00
Ra	0.00
Rb	0.00
Re	0.00
Rf	0.00
Rh	0.00
Rn	0.00
Ru	0.00
S	0.00
Sa	0.00
Sb	0.00
Sc	0.00
Se	0.00
Si	0.00
Sm	0.00
Sr	0.00
Ta	0.00
Tb	0.00
Tc	0.00
Td	0.00
Tf	0.00
Ti	0.00
Tl	0.00
Tm	0.00
Tn	0.00
U	0.00
V	0.00
Va	0.00
Vm	0.00
W	0.00
Wa	0.00
Y	0.00
Yb	0.00
Yr	0.00
Z	0.00
Zn	0.00
Zr	0.00

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99	Slide 99
100	Slide 100



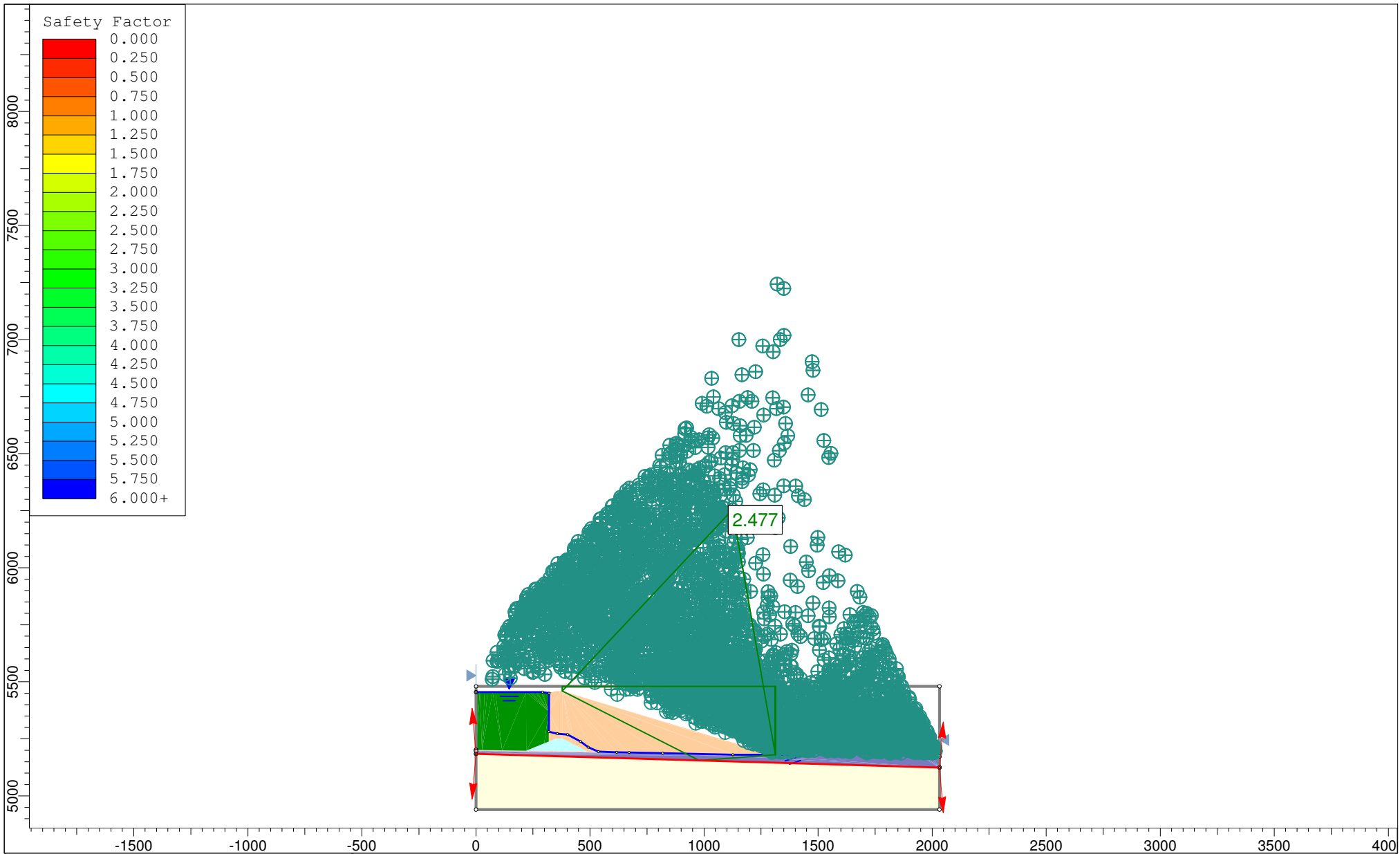
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- Slide 10
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- Slide 12
- Slide 13
- Slide 14
- Slide 15





**APPENDIX H.3  
STABILITY SECTION D-D'**



SLIDEINTERPRET 6.008

<i>Project</i>				Copper Flat	
<i>Analysis Description</i>				Section D-D' Stability: Downstream, Static, Block Failure	
<i>Drawn By</i>	GS	<i>Scale</i>	1:6984	<i>Company</i>	Golder Associates
<i>Date</i>	11/4/2013, 1:11:49 PM			<i>File Name</i>	1 - Section D 5460R - DS_S_B.slim

## Slide Analysis Information

### Copper Flat

#### Project Summary

---

- File Name: 1 - Section D 5460R - DS\_S\_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

---

- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None








## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

## Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma							0.2

Ratio							
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

### Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9

12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6

26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7

40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4

58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1388.41	5182.87
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**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04

1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01

1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45



1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

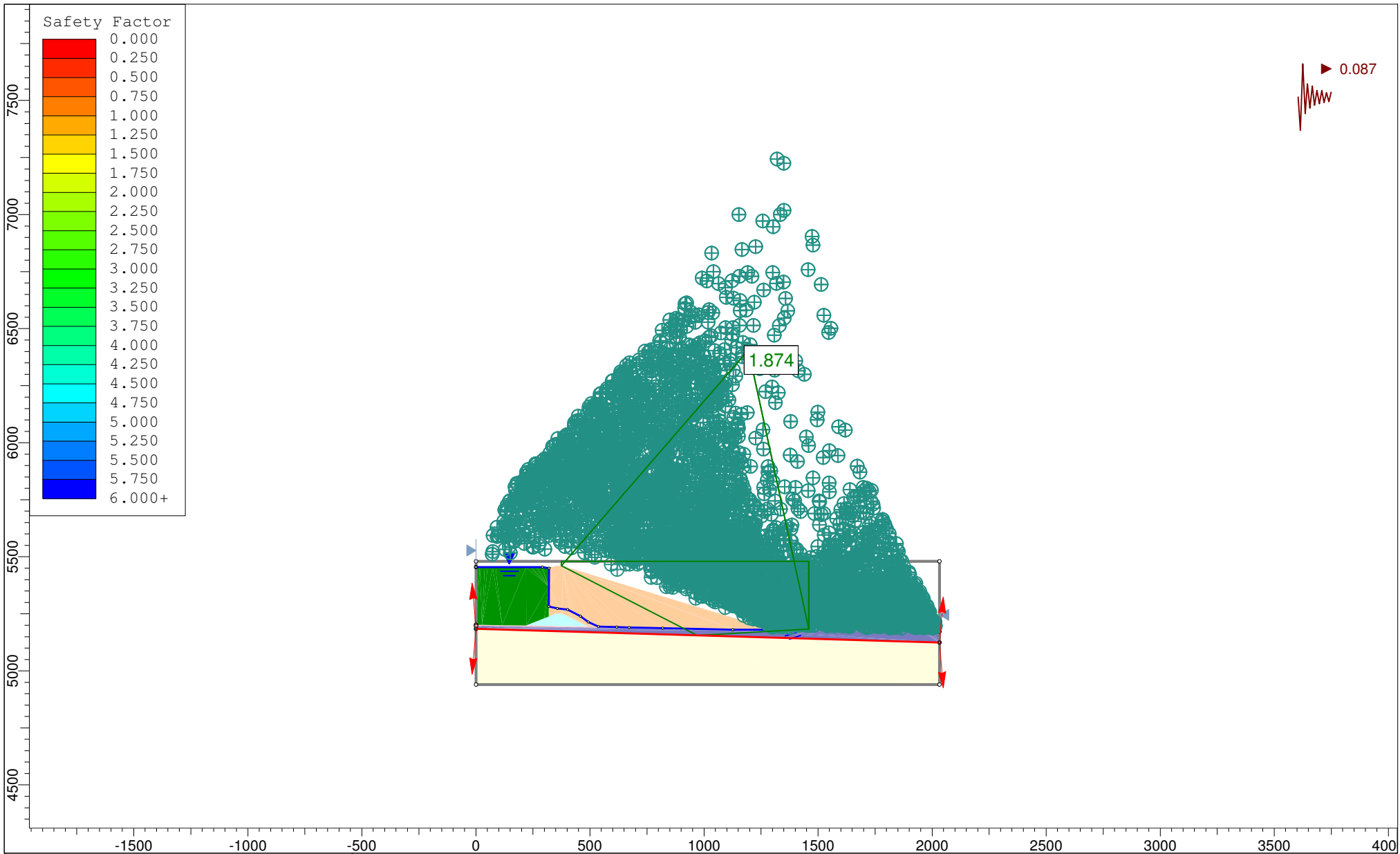
X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455



	Project			Copper Flat		
	Analysis Description			Section D-D' Stability: Downstream, Pseudo Static, Block Failure		
	Drawn By	GS	Scale	1:6984	Company	Golder Associates
	Date	11/4/2013, 1:11:49 PM		File Name	2 - Section D 5460R - DS_PS_B.slim	

SLIDEINTERPRET 6.008

## Slide Analysis Information

### Copper Flat

#### Project Summary

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- File Name: 2 - Section D 5460R - DS\_PS\_B.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Block Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material properties edits 12/2013

#### General Settings

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- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

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##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>
- Advanced Groundwater Method: None

## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








## Surface Options

- Surface Type: Non-Circular Block Search
- Number of Surfaces: 5000
- Pseudo-Random Surfaces: Enabled
- Convex Surfaces Only: Disabled
- Left Projection Angle (Start Angle): 95
- Left Projection Angle (End Angle): 265
- Right Projection Angle (Start Angle): 85
- Right Projection Angle (End Angle): -85
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Loading

- Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight	1e-025	113	120	120	120	127	108

[lbs/ft3]							
Cohesion [psf]	0	0	150	0			
Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5

9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2

21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2

32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8

50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450

-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04

1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

491.186	5194.33
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**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

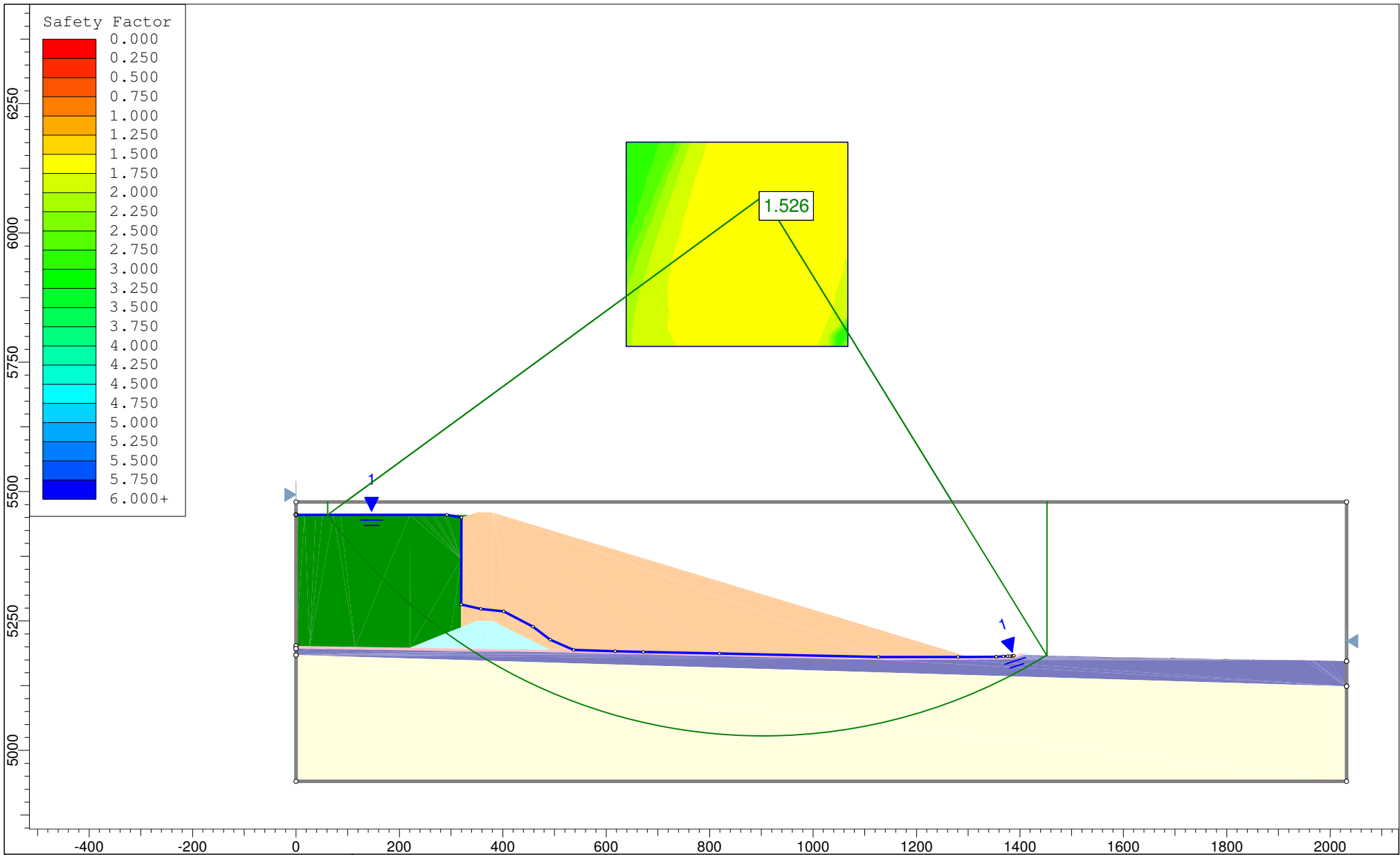
**Material Boundary**


X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52

**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76





	Project			Copper Flat		
	Analysis Description			Section D-D' Stability: Downstream, Static, Circular Failure		
	Drawn By	GS	Scale	1:3083	Company	Golder Associates
	Date	11/4/2013, 1:11:49 PM		File Name	3 - Section D 5460R - DS_S_C.slim	

SLIDEINTERPRET 6.008

## Slide Analysis Information

### Copper Flat

#### Project Summary

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- File Name: 3 - Section D 5460R - DS\_S\_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

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- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft<sup>3</sup>
- Advanced Groundwater Method: None








## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3

## Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft <sup>3</sup> ]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		
Friction Angle [deg]		39	29	29	26.5		
Tau/Sigma Ratio							0.2
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1

Hu Value	Automatically Calculated	Automatically Calculated	Automatically Calculated
Ru Value	0	0	0

**Shear Normal Functions**

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1
10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4

13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8
23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3

28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8
36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5

43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6
54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765

60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

2031.79	5123.76
---------	---------

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**Material Boundary**

X	Y
1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01

**Block Search Lines**

X	Y
0	5184.09

1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26
1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92

1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23
229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69
458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14

1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

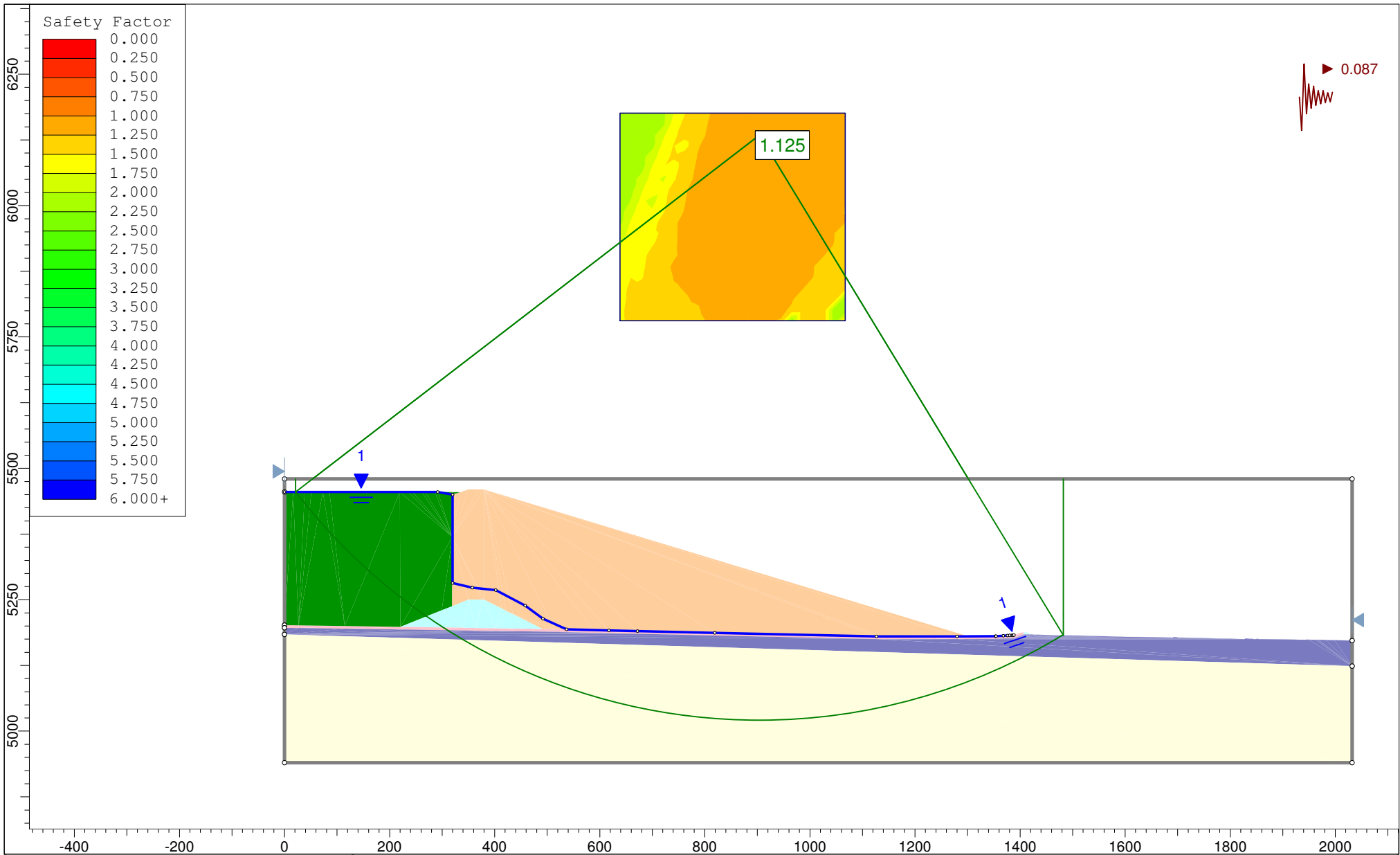
X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33


**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455
16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455



	Project			Copper Flat		
	Analysis Description					Section D-D' Stability: Downstream, Pseudo Static, Circular Failure
	Drawn By	GS	Scale	1:3035	Company	Golder Associates
	Date	11/4/2013, 1:11:49 PM		File Name	4 - Section D 5460R - DS_PS_C.slim	

SLIDEINTERPRET 6.008



## Slide Analysis Information

### Copper Flat

#### Project Summary

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- File Name: 4 - Section D 5460R - DS\_PS\_C.slim
- Last saved with Slide version: 6.008
- Project Title: Copper Flat
- Analysis: Section D-D' Stability: Downstream, Pseudo Static, Circular Failure
- Author: GS
- Company: Golder Associates
- Date Created: 11/4/2013, 1:11:49 PM
- Comments:
  - 103-92557
  - Material Property Edits 12/2013

#### General Settings

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- Units of Measurement: Imperial Units
- Time Units: days
- Permeability Units: feet/second
- Failure Direction: Left to Right
- Data Output: Standard
- Maximum Material Properties: 20
- Maximum Support Properties: 20

#### Analysis Options

---

##### Analysis Methods Used

- Spencer
- Number of slices: 50
- Tolerance: 0.005
- Maximum number of iterations: 50
- Check malpha < 0.2: Yes
- Initial trial value of FS: 1
- Steffensen Iteration: Yes

## Groundwater Analysis

- Groundwater Method: Water Surfaces
- Pore Fluid Unit Weight: 62.4 lbs/ft3
- Advanced Groundwater Method: None

## Random Numbers

- Pseudo-random Seed: 10116
- Random Number Generation Method: Park and Miller v.3








## Surface Options

- Surface Type: Circular
- Search Method: Grid Search
- Radius Increment: 10
- Composite Surfaces: Disabled
- Reverse Curvature: Create Tension Crack
- Minimum Elevation: Not Defined
- Minimum Depth: Not Defined

## Loading

- Seismic Load Coefficient (Horizontal): 0.087

## Material Properties

Property	Air	Cyclone Underflow	Structural Fill	Foundation Materials	Liner Interface Zone	Clay	Cyclone Overflow
Color							
Strength Type	No strength	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Shear Normal function	Strength=F(overburden)
Unit Weight [lbs/ft3]	1e-025	113	120	120	120	127	108
Cohesion [psf]		0	0	150	0		

Friction Angle [deg]	39	29	29	26.5			
Tau/Sigma Ratio				0.2			
Water Surface	None	Piezometric Line 1	None	None	Piezometric Line 1	None	Piezometric Line 1
Hu Value		Automatically Calculated			Automatically Calculated		Automatically Calculated
Ru Value	0		0	0		0	

### Shear Normal Functions

- Name: User Defined 1

Normal (psf)	Shear (psf)
0.417709	0.4
418.126	186
835.835	342
1253.54	488.3
1671.25	628.8
2088.96	765
2506.67	897.9
2924.38	1028.2
3342.09	1156.3
3759.8	1282.4
4177.5	1406.8
4595.21	1529.8
5012.92	1651.4
5430.63	1771.7
5848.34	1891
6266.05	2009.2
6683.76	2126.5
7101.46	2242.9
7519.17	2358.5
7936.88	2473.3
8354.59	2587.3
8772.3	2700.7
9190.01	2813.5
9607.72	2925.6
10025.4	3037.1
10443.1	3148.1

10860.8	3258.5
11278.5	3368.5
11696.3	3477.9
12114	3586.8
12531.7	3695.4
12949.4	3803.4
13367.1	3911.1
13784.8	4018.3
14202.5	4125.2
14620.2	4231.7
15037.9	4337.8
15455.6	4443.5
15873.3	4548.9
16291.1	4654
16708.8	4758.8
17126.5	4863.2
17544.2	4967.3
17961.9	5071.1
18379.6	5174.7
18797.3	5277.9
19215	5380.9
19632.7	5483.6
20050.4	5586.1
20468.1	5688.2
20885.8	5790.2
21303.6	5891.9
21721.3	5993.3
22139	6094.5
22556.7	6195.5
22974.4	6296.3
23392.1	6396.8

23809.8	6497.1
24227.5	6597.2
24645.2	6697.1
25062.9	6796.8
25480.6	6896.3
25898.4	6995.6
26316.1	7094.7
26733.8	7193.6
27151.5	7292.4
27569.2	7390.9
27986.9	7489.3
28404.6	7587.5
28822.3	7685.5
29240	7783.3
29657.7	7881
30075.4	7978.5
30493.1	8075.8
30910.9	8173
31328.6	8270
31746.3	8366.9
32164	8463.6
32581.7	8560.2
32999.4	8656.6
33417.1	8752.9
33834.8	8849
34252.5	8945
34670.2	9040.8
35087.9	9136.5
35505.6	9232.1
35923.4	9327.5
36341.1	9422.8

36758.8	9517.9
37176.5	9613
37594.2	9707.9
38011.9	9802.6
38429.6	9897.3
38847.3	9991.8
39265	10086.2
39682.7	10180.5
40100.4	10274.7
40518.2	10368.7
40935.9	10462.6
41353.6	10556.4
41771.3	10650.1
42189	10743.7
42606.7	10837.2
43024.4	10930.5
43442.1	11023.8
43859.8	11116.9
44277.5	11210
44695.2	11302.9
45112.9	11395.7
45530.7	11488.4
45948.4	11581
46366.1	11673.6
46783.8	11766
47201.5	11858.3
47619.2	11950.5
48036.9	12042.6
48454.6	12134.7
48872.3	12226.6
49290	12318.4
49707.7	12410.2
50125.4	12501.8
50543.2	12593.4
50960.9	12684.8
51378.6	12776.2
51796.3	12867.5
52214	12958.7
52631.7	13049.8
53049.4	13140.8
53467.1	13231.7
53884.8	13322.6

54302.5	13413.3
54720.2	13504
55138	13594.6
55555.7	13685.1
55973.4	13775.5
56391.1	13865.9
56808.8	13956.1
57226.5	14046.3
57644.2	14136.4
58061.9	14226.5
58479.6	14316.4
58897.3	14406.3
59315	14496.1
59732.7	14585.8
60150.5	14675.5
60568.2	14765
60985.9	14854.5
61403.6	14943.9
61821.3	15033.3
62239	15122.6
62656.7	15211.8

1126.48	5180
1279.93	5180.24
1353.21	5180.5
1354	5180.55
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1388.41	5182.87

**Block Search Lines**

X	Y
0	5184.09
2031.79	5123.76

**External Boundary**

X	Y
2031.79	4940
2031.79	5123.76
2031.79	5123.76
2031.79	5172.24
2031.79	5172.24
2031.79	5480
0	5480
-8.88178e-016	5455
-3.55271e-015	5201.58
-3.55271e-015	5201.58
-7.10543e-015	5196.58
-7.10543e-015	5196.58
0	5184.09
0	5184.09
0	4940

**List Of Coordinates**

**Piezoline**

X	Y
-8.88178e-016	5455
291.649	5455
319.83	5450
319.649	5281.62
357.393	5272.94
401.861	5268.39
458.094	5238.85
491.717	5213.7
536.883	5193.72
617.217	5191.45
671.576	5190.23
818.688	5186.92

**Material Boundary**

X	Y
---	---

1406.35	5183.84
1408.84	5183.95
1410.54	5184.06
1414.71	5184.02
1418.24	5184
1421.09	5183.84
1426.9	5183.53
1435.44	5183.45
1446.75	5183.04
1450.41	5182.94
1453.47	5182.87
1477.47	5182.4
1490.36	5182.08
1497.22	5182.06
1503.44	5182.02
1504.84	5182.01
1506.36	5181.99
1507.61	5181.99
1539.7	5181.03
1562.99	5180.38
1590.77	5179.52
1604.77	5179.2
1626.36	5179.14
1657.9	5178.67
1691.72	5178.02
1694.27	5178.02
1699.51	5178
1701.22	5177.98
1702.96	5177.97
1709.66	5177.96
1721.49	5177.64
1740.86	5177.19
1748.22	5177.04
1762.45	5176.63
1778.98	5176.04
1786.55	5175.98
1791.65	5175.94
1792.96	5175.93
1802.43	5175.92
1809.8	5175.69
1828.02	5175.27
1831.2	5175.26

1834.51	5175.24
1842.98	5175.12
1849.95	5175.02
1853.03	5175.01
1856.22	5174.99
1878.02	5174.81
1913.35	5174.36
1921.87	5174.26
1934.29	5174.01
1945.09	5173.97
1947.38	5173.97
1949.12	5173.96
1950.59	5173.96
1961.99	5173.92
1965.58	5173.92
1968.23	5173.92
1981.99	5173.38
1998.24	5173.03
2031.79	5172.24

229.978	5198.08
339.831	5196.4
363.815	5196.12
399.834	5195.69
458.245	5194.82
491.186	5194.33
513.77	5193.99
588.916	5192.09
617.217	5191.45
671.576	5190.23
818.688	5186.92
1126.48	5180
1137.58	5179.74
1279.93	5180.24
1302.77	5180.32
1345.35	5180.47
1353.6	5180.52
1367.88	5181.43
1376.15	5181.96
1380.11	5182.05
1385.14	5182.07
1386.21	5182.14
1387.79	5182.66
1392.43	5184.19
1403.23	5187.77
1406.35	5187.77
1413.26	5187.76
1416.88	5185.95
1418.29	5185.25
1421.09	5183.84

**Material Boundary**

X	Y
318.893	5237.63
319.552	5369.37
319.83	5450
325.925	5452.03
334.831	5455
349.832	5460
369.265	5460
379.833	5460
1302.77	5180.32

**Material Boundary**

X	Y
-7.10543e-015	5196.58
26.2539	5196.18
114.41	5194.85
229.902	5193.08
339.764	5191.4
363.756	5191.12
399.767	5190.69

**Material Boundary**

X	Y
-3.55271e-015	5201.58
26.3295	5201.18
114.486	5199.85
220.393	5198.23

458.17	5189.82
513.67	5188.99
588.796	5187.09
617.104	5186.46
671.463	5185.23
818.576	5181.92
1126.37	5175
1137.53	5174.74
1279.95	5175.24
1345.37	5175.47
1353.84	5175.53
1368.19	5176.44
1376.37	5176.96
1380.17	5177.05
1385.31	5177.07
1386.81	5177.14
1389.36	5177.91
1394	5179.45
1404.03	5182.77
1406.35	5182.77

16.375	5455
38.2237	5455
50.9444	5455
72.93	5455
85.5063	5455
219.596	5455
235.945	5455
243.614	5455
261.097	5455
267.631	5455
286.669	5455
291.649	5455
312.952	5455
315.667	5455
334.831	5455

**Material Boundary**

X	Y
1406.35	5182.77
1406.35	5183.84
1406.35	5187.77

**Material Boundary**

X	Y
220.393	5198.23
299.17	5229.74
318.893	5237.63
349.832	5250
375.024	5250
379.833	5250
413.352	5233.24
421.524	5229.16
434.805	5222.52
491.186	5194.33

**Material Boundary**

X	Y
0	5184.09
2031.79	5123.76

**Material Boundary**

X	Y
-8.88178e-016	5455
3.52609	5455

**APPENDIX I**  
**FOUNDATION SETTLEMENT POTENTIAL EVALUATION**

**APPENDIX I.1**  
**SETTLEMENT POTENTIAL, COPPER FLAT TAILINGS EMBANKMENT FOUNDATION**  
**REVISED JUNE 2016**



**Date:** April 23, 2013  
**Project No.:** 103-92557  
**Subject:** Settlement Potential, Copper Flat  
Tailings Embankment Foundation  
**Made by:** DP  
**Checked by:** GM  
**Reviewed by:** MJG  
**Project Short Title:** COPPER FLAT FEASIBILITY STUDY TAILINGS DISPOSAL FACILITY DESIGN

### 1.0 OBJECTIVE

Evaluate the consolidation characteristics of the soil strata, and estimate the post-construction subgrade settlement under the containment embankments for the Copper Flat Tailings Impoundment.

### 2.0 BACKGROUND

Golder is completing the feasibility level designing of the Copper Flat tailings impoundment embankment which will impound slurried tailings to an estimated maximum depth of approximately 230 feet at the impoundment embankment. An earthen starter dam will be constructed to a height of approximately 50 feet and the remainder of the dam will be constructed with sand recovered from the tailings in a cyclone plant. A geotechnical investigation was performed in the vicinity of the proposed embankment, which included standard penetration testing and sample collection from the surface to a depth of 53 feet. Drilling indicated that the tailings embankment area consists primarily of alluvial deposits that include silt, sand and gravel, underlain by clay.

For the settlement analysis, representative samples of the foundation strata were analyzed in Golder's geotechnical laboratory for index properties, gradation, and Atterberg limits. Additionally, select samples were remolded in the laboratory, and the remolded samples were subjected to one-dimensional consolidation testing. The results are summarized in the following sections.

Drill hole logs were input onto a site plan and cross-sections through the embankment were developed. Two cross-sections (B-B' and D-D') were further developed for settlement potential analyses with the addition of information from the site investigation conducted in 1980 for the former Quintana mining operation (*Tailings Dam and Disposal Area, Quintana Minerals Corporation, Copper Flats Project, Golddust, New Mexico, Sergeant Hauskins and Beckwith, 1980*). Generalized soil and rock designations were assigned to the strata.

Settlement calculations were developed for the post-construction embankment, which represents the worst-case condition. Staged settlement was not analyzed because the results for the post-construction condition demonstrate that the amount of predicted settlement (approximately 2.1 feet under the heaviest loaded portion of embankment) is tolerable, and, will be adequately mitigated by fill placement during ongoing embankment construction.

Settlement calculations were performed using the computer model SETTLE3D v. 2.0, a computer program developed by Rocscience, Inc. for the analysis of settlement and consolidation under foundations and embankments.

The results of the consolidation tests performed on the samples obtained during geotechnical investigations are included in Attachment 1. Tables summarizing the laboratory consolidation test results and the computer modeling input parameters are included in Attachment 2. The SETTLE3D computer model output files are included in Attachment 3. Figures depicting the two embankment sections

X:\Tucson\Projects\13proj\133-92505 Copper Flat TSF\30,000 TPD Report\Appendix I Foundation Settlement Potential Evaluation

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analyzed for foundation settlement, as well as a graphical representation of the analysis results, are included in Attachment 4.

### 3.0 LABORATORY ANALYSIS

Laboratory one-dimensional consolidation analyses were performed on representative samples obtained during the geotechnical investigation. The results of the geotechnical laboratory analyses are summarized in the Tables included in Attachment 1. Consolidation tests were generally conducted on the fine grain fraction of the soil- strata, on remolded drill hole samples.

As shown, the soil strata generally have compression index values ( $C_c$ ) ranging from 0.035 to 0.09, and estimated initial void ratio values ranging from 0.71 to 0.75. The compression index results were obtained from review of the laboratory graphs of void ratio versus stress, and were obtained for loads ranging from 16 to 32 kilopounds per square foot (ksf).

### 4.0 COMPUTER MODELING

Computer modeling was performed using SETTLE3D v. 2.0. The results of the modeling are included in Attachment 3.

The following parameters were incorporated into the analyses:

- Settlement Index ( $C_c$ ) values obtained from laboratory testing of soil fraction of field samples.
- Initial void ratio ( $e_0$ ) values estimated from review of the consolidation test graphs.
- Unit weights estimated from the consolidation test results, and based on previous experience with similar soils.
- An overconsolidation ratio (OCR) of 1.0 assumed.
- A Poisson's Ratio of 0.3 was assumed, based on a published range of values from 0.2 to 0.4 for sandy clays.
- The model was initialized for primary consolidation, with non-linear settlement characteristics.

A conservative value of 4,000 ksf was assigned for the constrained modulus of the coarse sand and gravel, caliche and weathered basalt layers. The constrained modulus was assigned to represent inter-particle consolidation in the gravels and sands. Settlement in these layers was demonstrated to be minimal, even with the conservative modulus value used in the model. (Reference: US Army Corps of Engineers, EM 1110-1-1904, 1990).

Modeling was performed on an individual point basis, at a minimum of 5 analysis points on each embankment cross-section. The model was initialized for 1-dimensional Boussinesq Methodology.

## 5.0 RESULTS

### 5.1 Settlement Beneath Embankment

The results of the consolidation analyses are summarized on Figures 5 and 7 in Attachment 4. As shown on the figures, calculated settlement beneath the impoundment embankment ranges from approximately 2.1 to 0.2 feet, with settlement decreasing as the weight of post-construction loading decreases towards the outer toe of the embankment. The results of the consolidation testing indicate the various soils at the site exhibit generally similar consolidation characteristics. Based on the consolidation testing and computer modeling, settlement beneath the embankment is projected to be generally uniform.

The consolidation analyses results are considered conservative, as the laboratory consolidation testing was performed on the fine-grained fraction of soil samples. The effects of inter-particle bearing contact within the gravel-rich strata, which will reduce the estimated settlement, have not been considered in the analyses. Based on the field Standard Penetration Testing (SPT) results, which showed the strata to

generally be very dense to hard, actual post-construction consolidation settlement of less than 1 foot is anticipated.

As the embankment will be constructed continuously, it is anticipated that observed settlement will be accommodated by managing dam fill placement during embankment construction.

## 5.2 Settlement Beneath Outlet Piping

Section F-F' was developed along the proposed outlet pipe alignment, as shown on Figure 9. The pipe will be comprised of 14-inch diameter Schedule 80 carbon steel, encased in a minimum 42-inch by 42-inch concrete-filled trench.

From review of Section F-F', it appears the inlet of the outlet pipe will be founded within clay overlaying a basalt rock layer. Settlement of the outlet pipe foundation may be reduced at the inlet side by the basalt, which appears to transition to soil-gravel strata at some undefined location along the pipe downstream of the inlet. Stresses induced by the limited differential settlement are expected to be negligible, and limited by the concrete backfill and steel pipe strength. The pipe currently is proposed with a vertical drop of approximately 31 feet along the 1290-foot length of pipe. Post consolidation pipe slope (assuming a worst-case settlement of 2.1 feet at the pipe inlet) would average 2.3 percent along the length of the pipe. As described above, field standard penetration tests suggest actual post-construction settlement will be less than that calculated on the basis of consolidation test results.

### **Attachments**

- Attachment 1 – Laboratory Test Summary Tables and Consolidation Test Results
- Attachment 2 – Consolidation Testing Summary and SETTLE 3D Model Input Tables
- Attachment 3 – SETTLE 3D Output Files
- Attachment 4 – Embankment Analysis Cross-sections with Graphical Output Results  
(Sections B-B' and D-D' only)

**ATTACHMENT 1**  
**LABORATORY TEST SUMMARY TABLES AND CONSOLIDATION TEST RESULTS**

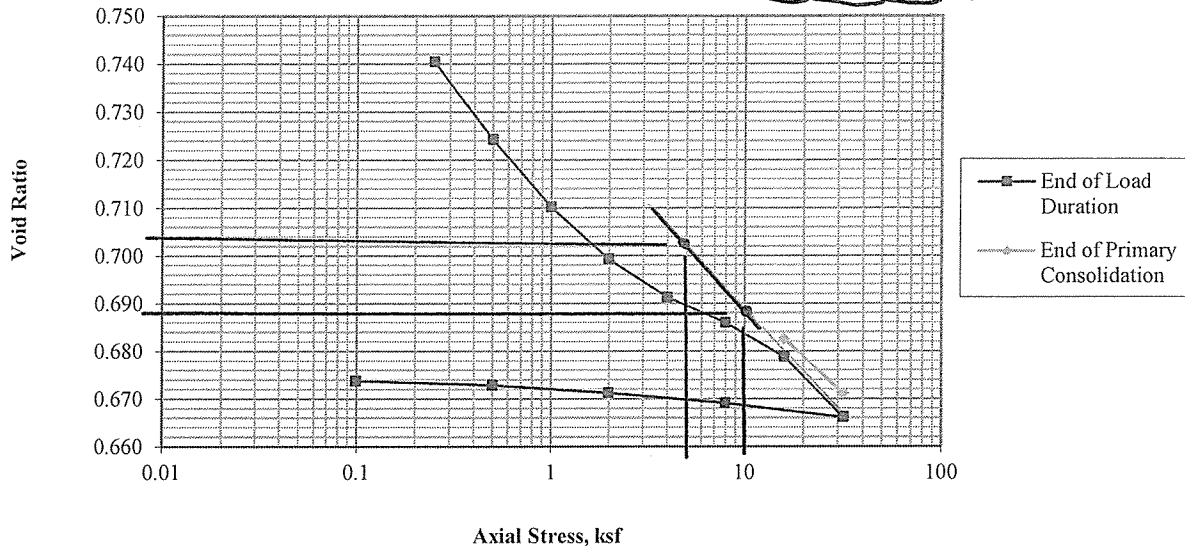
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.994 in		0.960 in	USCS description (ASTM D2487):	Lean clay with sand, yellowish red, moist		
Diameter =	2.499 in		2.499 in	Atterberg Limits (ASTM D4318):	LL = 47	PL = 20	PI = 27
Area =	4.905 in <sup>2</sup>		4.905 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 99%	No. 200 = 81%
Volume =	4.875 in <sup>3</sup>		4.709 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.4%		4.8%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.6%		
Height of Solids =	0.5641 in		0.5641 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.762		0.702	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		18.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308 lb		0.282 lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)	
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.3 pcf		103.6 pcf	Final Differential Height:	-0.0158 in		
Dry Unit Weight =	95.5 pcf		98.9 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1058					0.0000	0.9901	0.00	0.755				
1	0.25	1410					0.0083	0.9818	0.84	0.740				
2	0.50	1410					0.0174	0.9726	1.75	0.724				
3	1.00	1440					0.0254	0.9647	2.55	0.710				
4	2.00	1470					0.0315	0.9585	3.17	0.699				
5	4.00	1425					0.0361	0.9540	3.63	0.691				
6	8.00	1425					0.0390	0.9510	3.93	0.686				
7	16.00	1425	0.0410	0.9491	4.12	0.683	0.0431	0.9470	4.33	0.679	2 (Root time)	0.683	3.650	0.5
8	32.00	1440	0.0474	0.9426	4.77	0.671	0.0502	0.9398	5.05	0.666	2 (Root time)	0.672	2.520	0.5
9	8.00	95					0.0485	0.9415	4.88	0.669				
10	2.00	120					0.0473	0.9427	4.76	0.671				
11	0.50	95					0.0464	0.9436	4.67	0.673				
12	0.10	70					0.0459	0.9442	4.62	0.674				

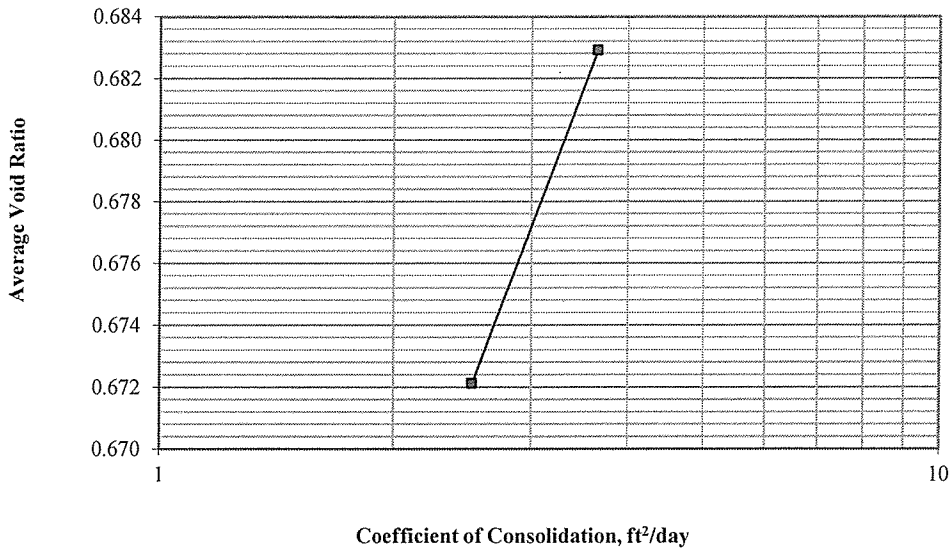
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title:  ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
Job Short Title:  Copper Flat Tailings Design Study						
Sample:  BH-10 @ 19-33 ft	Technician:  RJM	Reviewed:  CCS	Start Date:  3/11/2013	Job Number:  103-92557.006	Figure:  1	

Void Ratio vs. Axial Stress

$$C_c = \frac{.702 - .688}{\log\left(\frac{10}{5}\right)} = 0.05$$

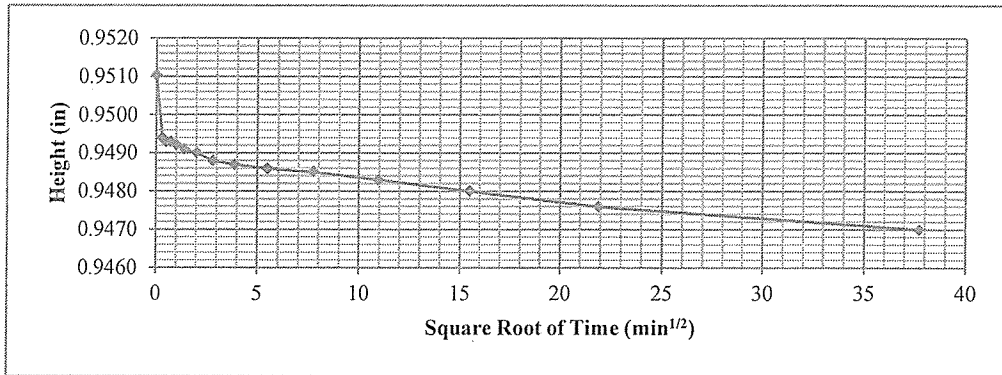


Average Void Ratio vs. Coefficient of Consolidation

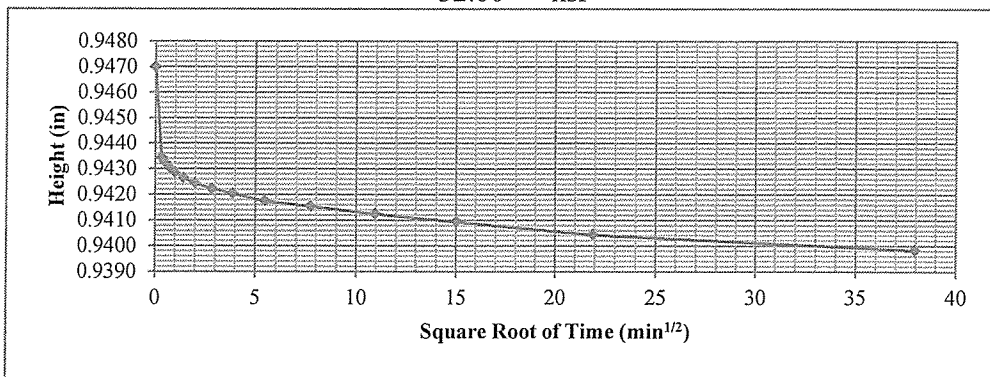


<b>Golder Associates Inc.</b> Denver, Colorado		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-10 @ 19-33 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 2	

16.00 ksf



32.00 ksf



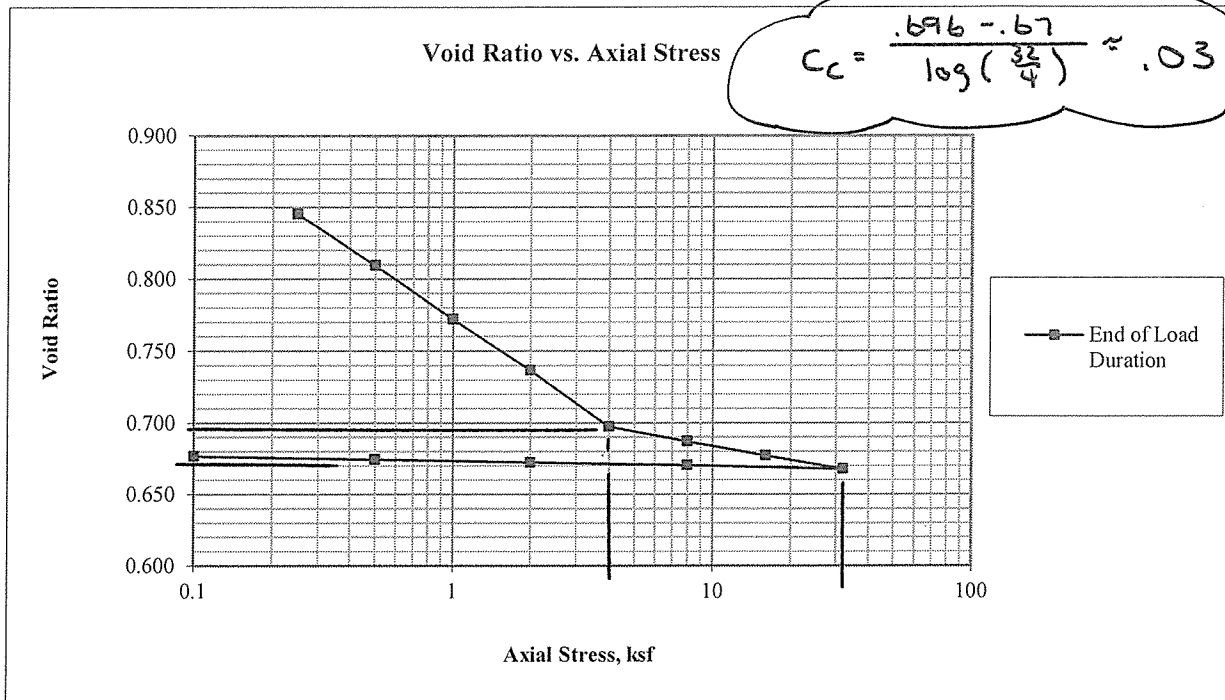
<p><b>Golder Associates Inc.</b>  <b>Denver, Colorado</b></p>		<p>Title:  <b>ASTM D2435</b>  <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b>  <b>TIME-DEFORMATION PLOTS</b></p>			
<p>Job Short Title:  <b>Copper Flat Tailings Design Study</b></p>					
<p>Sample:  <b>BH-10 @ 19-33 ft</b></p>	<p>Technician:  <b>RJM</b></p>	<p>Reviewed:  <b>CCS</b></p>	<p>Start Date:  <b>3/11/2013</b></p>	<p>Job Number:  <b>103-92557.006</b></p>	<p>Figure:  <b>3</b></p>

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.905 in	USCS description (ASTM D2487):	Fat clay, dark red, moist		
Diameter =	2.496 in		2.496 in	Atterberg Limits (ASTM D4318):	LL = 66	PL = 31	PI = 35
Area =	4.893 in <sup>2</sup>		4.893 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 100%	No. 200 = 90%
Volume =	4.878 in <sup>3</sup>		4.428 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	25.9%		6.4%	Remold Targets:	86.0 pcf (dry) at	29.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	28.8%		
Height of Solids =	0.5215 in		0.5215 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.912		0.735	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	76.7%		23.5%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.313 lb		0.264 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.248 lb		0.248 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	110.8 pcf		103.2 pcf	Final Differential Height:	-0.0304 in		
Dry Unit Weight =	88.0 pcf		97.0 pcf	Estimated Preconsolidation Stress:	Not Computed		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1019					0.0000	0.9822	0.00	0.883				
1	0.25	1410					0.0197	0.9625	1.98	0.845				
2	0.50	1410					0.0385	0.9437	3.86	0.809				
3	1.00	1430					0.0579	0.9242	5.81	0.772				
4	2.00	1470					0.0764	0.9057	7.66	0.737				
5	4.00	1415					0.0968	0.8853	9.71	0.698				
6	8.00	1420					0.1023	0.8798	10.26	0.687				
7	16.00	1425					0.1074	0.8747	10.77	0.677				
8	32.00	1440					0.1123	0.8699	11.26	0.668				
9	8.00	90					0.1109	0.8712	11.13	0.670				
10	2.00	115					0.1099	0.8722	11.02	0.672				
11	0.50	100					0.1089	0.8733	10.92	0.674				
12	0.10	70					0.1076	0.8746	10.79	0.677				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-12 @ 33.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1





<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	<b>Title:</b> ASTM D2435 <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>				
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-12 @ 33.5-48.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 2

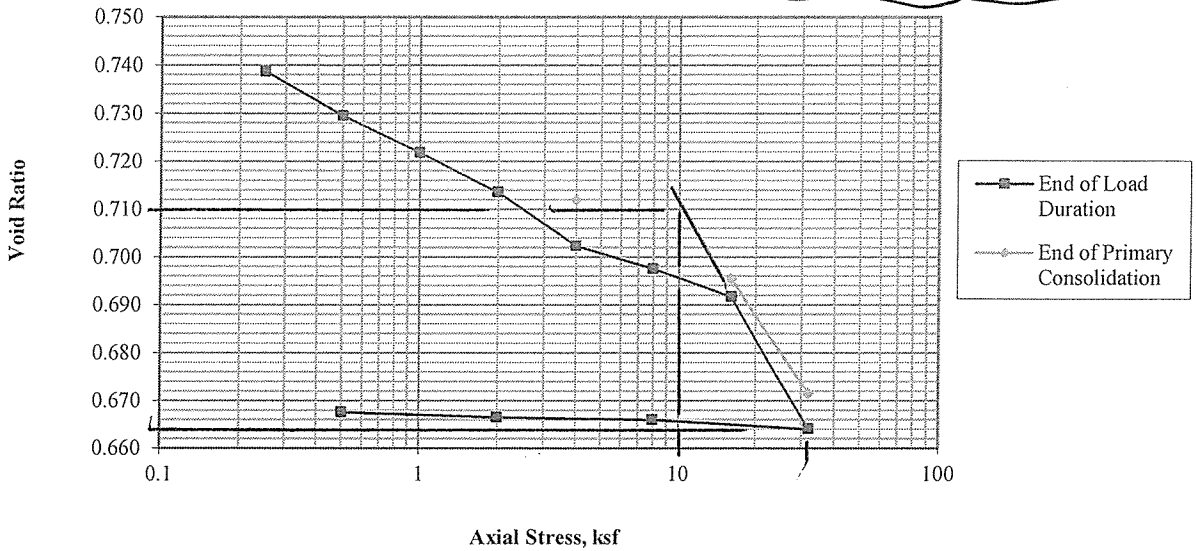
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.993	in	0.887	in	USCS description (ASTM D2487):	Clayey sand with gravel, yellowish brown, dry	
Diameter =	2.500	in	2.500	in	Atterberg Limits (ASTM D4318):	LL = 23	PL = 14
Area =	4.909	in <sup>2</sup>	4.909	in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 83%
Volume =	4.874	in <sup>3</sup>	4.354	in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted
Water Content =	14.1%		1.7%		Remold Targets:	95.0	pcf (dry) at 15.0% moisture
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%	
Height of Solids =	0.5636	in	0.5636	in	Trimming Procedure:	Specimen remolded in ring	
Void Ratio =	0.762		0.574		Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated
Degree of Saturation =	49.8%		8.0%		Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B
Wet Mass =	0.307	lb	0.274	lb	Apparatus:	Frame No. 1	(Wykeham Farrance 24251)
Dry Mass =	0.269	lb	0.269	lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial
Wet Unit Weight =	108.9	pcf	108.7	pcf	Final Differential Height:	0.0528 in	
Dry Unit Weight =	95.5	pcf	106.9	pcf	Estimated Preconsolidation Stress:	16.4 ksf	
							PI = 9
							No. 200 = 36%

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1055					0.0000	0.9877	0.00	0.752				
1	0.25	1425					0.0077	0.9800	0.78	0.739				
2	0.50	1425					0.0129	0.9747	1.30	0.730				
3	1.00	1440					0.0173	0.9704	1.74	0.722				
4	2.00	1410					0.0219	0.9657	2.21	0.714				
5	4.00	2785	0.0229	0.9647	2.31	0.712	0.0283	0.9594	2.85	0.702	2 (Root time)	0.712	8.212	0.4
6	8.00	1425					0.0309	0.9567	3.12	0.698				
7	16.00	1430	0.0321	0.9555	3.24	0.695	0.0343	0.9534	3.45	0.692	2 (Root time)	0.696	10.130	0.3
8	32.00	1440	0.0457	0.9420	4.60	0.671	0.0498	0.9378	5.02	0.664	2 (Root time)	0.675	4.980	0.4
9	8.00	105					0.0487	0.9389	4.91	0.666				
10	2.00	90					0.0484	0.9392	4.88	0.667				
11	0.50	180					0.0478	0.9398	4.82	0.668				

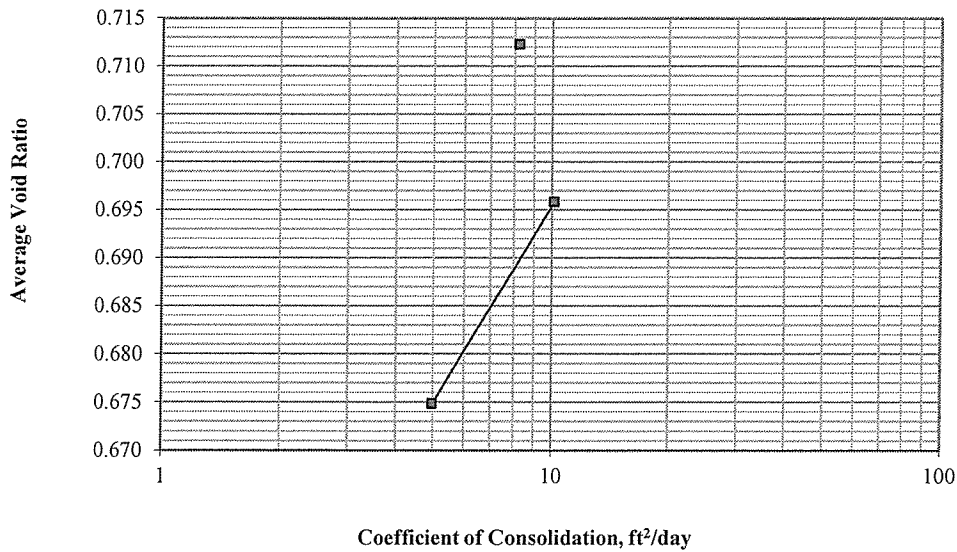
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-16 @ 29-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> I	

Void Ratio vs. Axial Stress

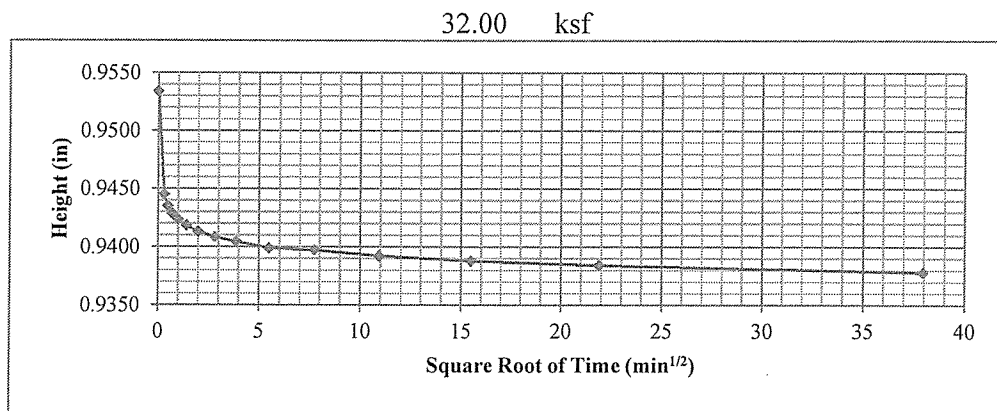
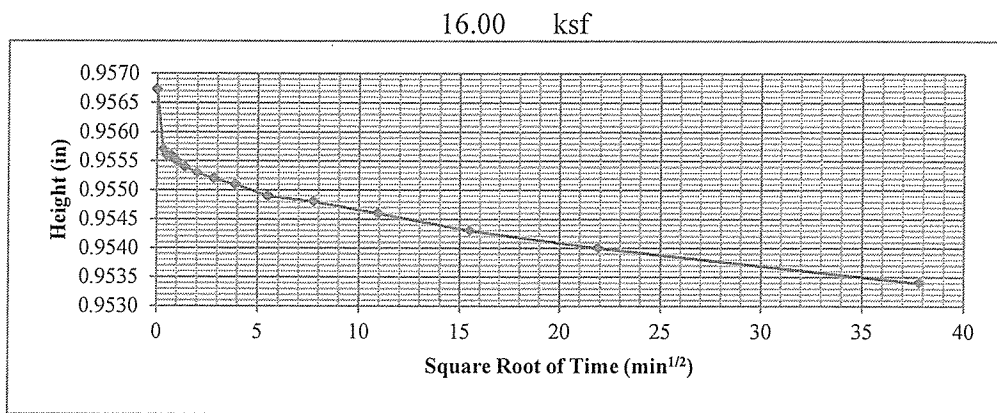
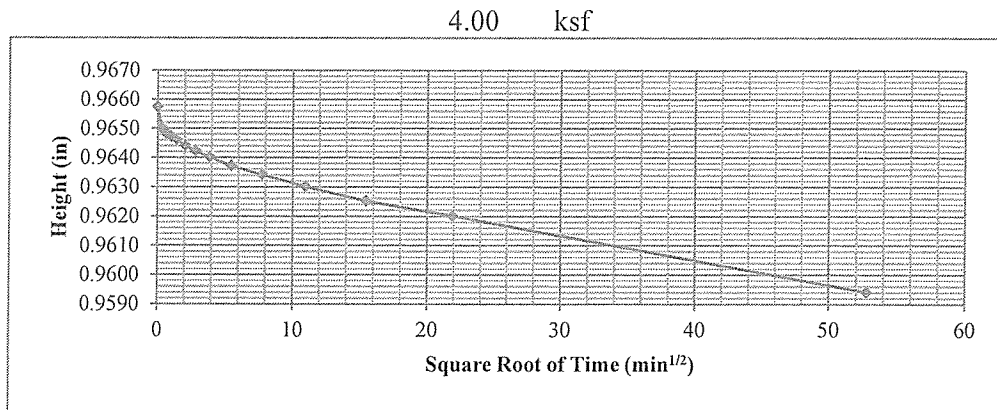
$$C_c = \frac{.71 - .664}{\log\left(\frac{32}{10}\right)} \approx .09$$



Average Void Ratio vs. Coefficient of Consolidation



<b>Golder Associates Inc.</b> Denver, Colorado	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>CONSOLIDATION PLOTS</b>					
	Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-16 @ 29-34 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
Job Short Title: <b>Copper Flat Tailings Design Study</b>					
Sample: <b>BH-16 @ 29-34 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>

	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.960 in	USCS description (ASTM D2487):	Sandy lean clay, reddish brown, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 25	PL = 14	PI = 11
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 100%	No. 4 = 95%	No. 200 = 52%
Volume =	4.886 in <sup>3</sup>		4.705 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.5%		2.7%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.7%		
Height of Solids =	0.5638 in		0.5638 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.768		0.703	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	51.1%		10.4%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.308 lb		0.276 lb	Apparatus:	Frame No. 4	(ELE C-320A)	
Dry Mass =	0.269 lb		0.269 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.0 pcf		101.5 pcf	Final Differential Height:	0.0045 in		
Dry Unit Weight =	95.1 pcf		98.8 pcf	Estimated Preconsolidation Stress:	14.7 ksf		

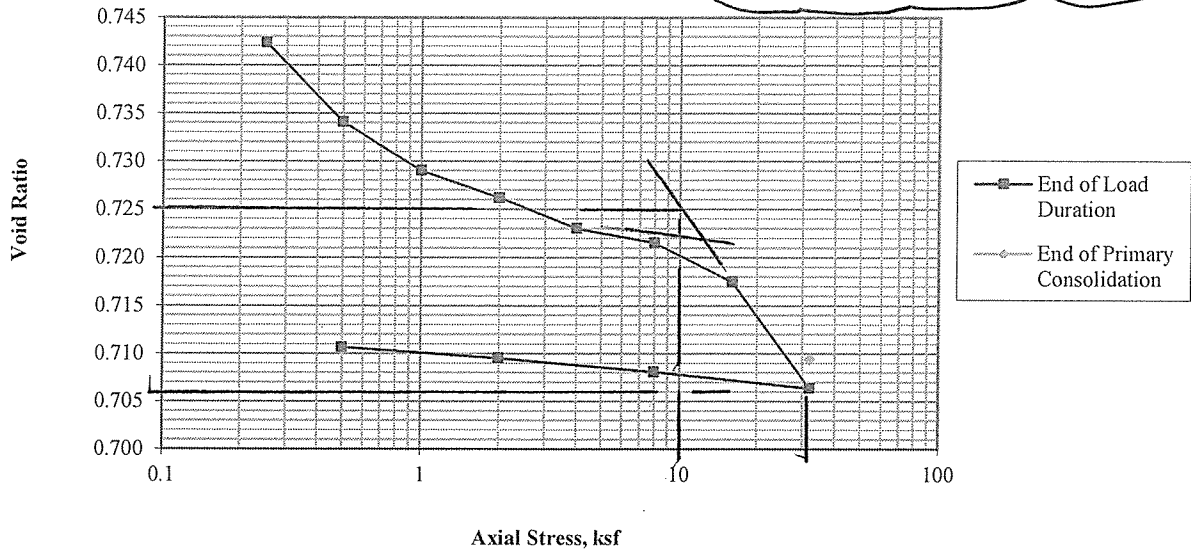
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	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	1025					0.0000	0.9894	0.00	0.755				
1	0.25	1425					0.0070	0.9824	0.70	0.742				
2	0.50	1425					0.0117	0.9777	1.17	0.734				
3	1.00	1440					0.0145	0.9748	1.46	0.729				
4	2.00	1410					0.0161	0.9732	1.62	0.726				
5	4.00	2780					0.0179	0.9714	1.80	0.723				
6	8.00	1425					0.0187	0.9706	1.88	0.722				
7	16.00	1420					0.0210	0.9683	2.11	0.717				
8	32.00	1440	0.0256	0.9638	2.57	0.709	0.0273	0.9621	2.74	0.706	2 (Root time)	0.710	5.423	0.5
9	8.00	105					0.0263	0.9630	2.64	0.708				
10	2.00	90					0.0255	0.9638	2.56	0.710				
11	0.50	180					0.0249	0.9645	2.49	0.711				

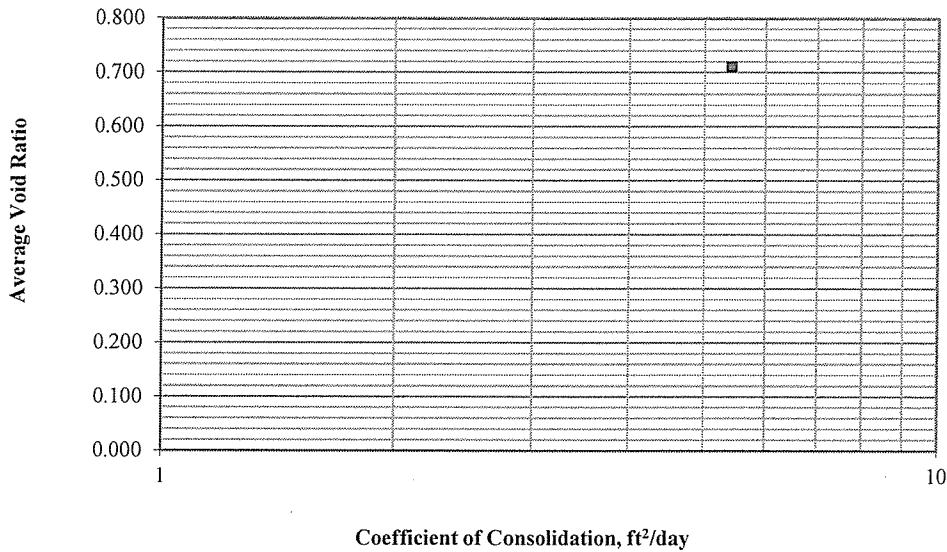
<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b>  ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

Void Ratio vs. Axial Stress

$$C_c = \frac{.725 - .706}{\log\left(\frac{32}{10}\right)} \approx .04$$

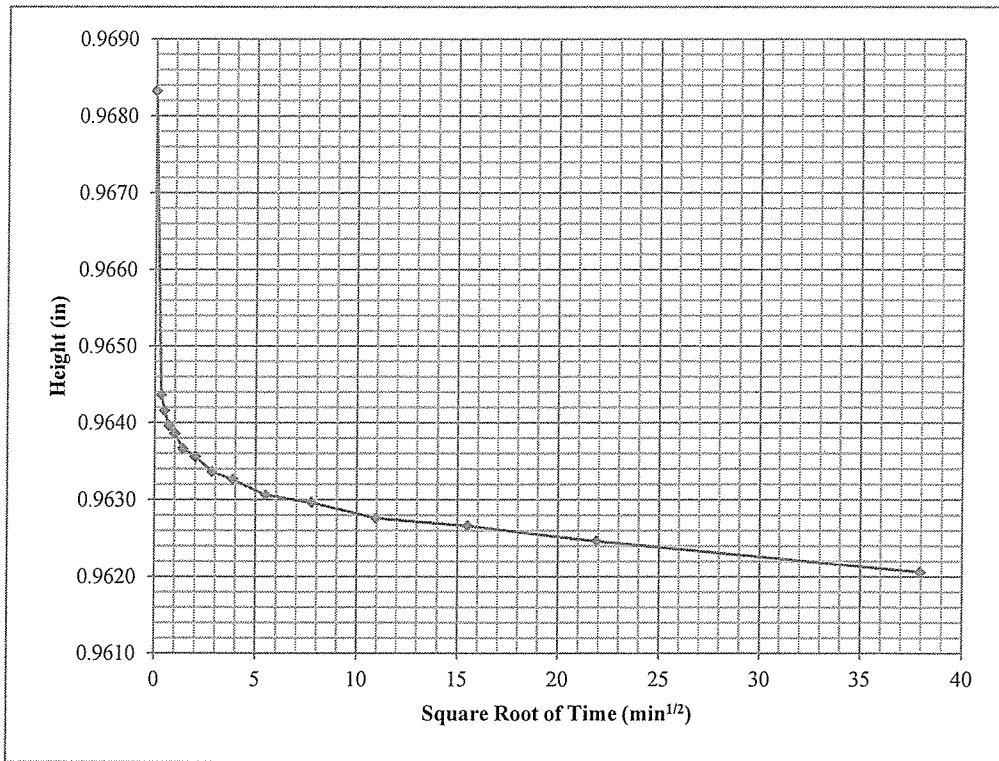


Average Void Ratio vs. Coefficient of Consolidation



<b>Golder Associates Inc.</b> Denver, Colorado		Title: <b>ASTM D2435                  ONE-DIMENSIONAL CONSOLIDATION TEST REPORT                  CONSOLIDATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 23-33.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	

32.00 ksf



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT TIME-DEFORMATION PLOTS				
<b>Job Short Title:</b> Copper Flat Tailings Design Study						
<b>Sample:</b> BH-18 @ 23-33.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 3	

	Initial		Final	
Height =	0.994	in	0.924	in
Diameter =	2.498	in	2.498	in
Area =	4.901	in <sup>2</sup>	4.901	in <sup>2</sup>
Volume =	4.871	in <sup>3</sup>	4.528	in <sup>3</sup>
Water Content =	28.9%		9.0%	
Specific Gravity =	2.70	(Assumed)	2.70	(Assumed)
Height of Solids =	0.5085	in	0.5085	in
Void Ratio =	0.955		0.817	
Degree of Saturation =	81.7%		29.7%	
Wet Mass =	0.313	lb	0.264	lb
Dry Mass =	0.243	lb	0.243	lb
Wet Unit Weight =	110.9	pcf	100.9	pcf
Dry Unit Weight =	86.1	pcf	92.6	pcf

Notes

USCS description (ASTM D2487):

Atterberg Limits (ASTM D4318):

Percent Finer (ASTM D422):

Specimen Type:

Remold Targets:

Water Content of Trimmings (ASTM D2216):

Trimming Procedure:

Inundation:

Test Method:

Apparatus:

Final Water Content Specimen:

Final Differential Height:

Estimated Preconsolidation Stress:

Fat clay with sand, dark red, wet

LL = 62                      PL = 20                      PI = 42

3/4 in. = 100%                      No. 4 = 100%                      No. 200 = 82%

Intact                       Reconstituted

86.0 pcf (dry) at                      29.0% moisture

29.1%

Specimen remolded in ring

Not inundated                       Inundated

A                       B

Frame No. 5                      (ELE C-320A)

Entire                       Partial

-0.0246 in

-- ksf

-- indicates test was not performed

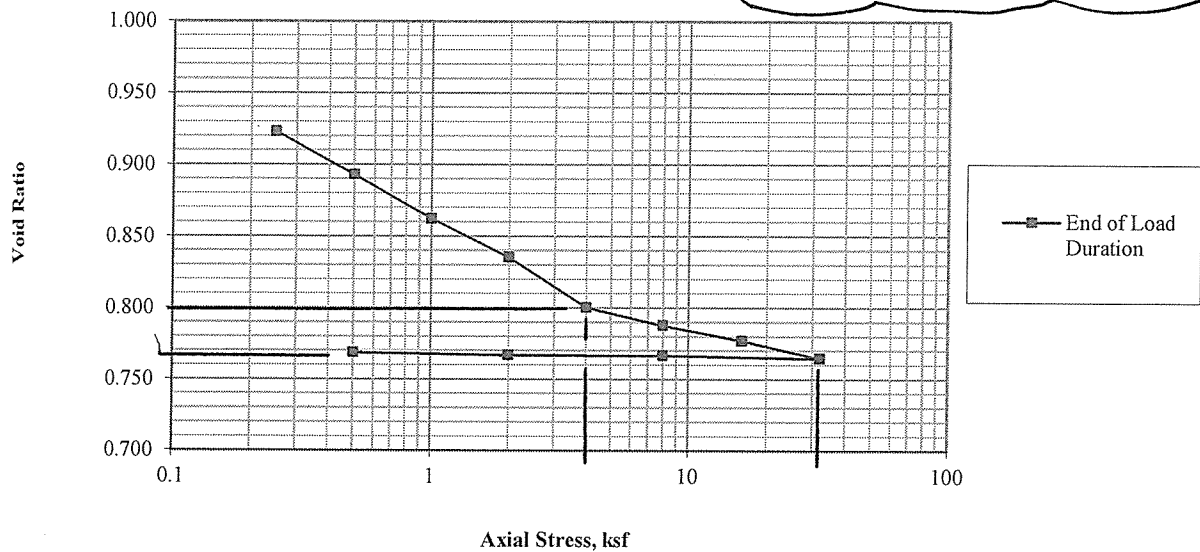
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	990					0.0000	0.9933	0.00	0.953				
1	0.25	1425					0.0154	0.9779	1.55	0.923				
2	0.50	1825					0.0307	0.9626	3.09	0.893				
3	1.00	1440					0.0463	0.9470	4.66	0.862				
4	2.00	1410					0.0600	0.9333	6.04	0.835				
5	4.00	2775					0.0778	0.9155	7.82	0.800				
6	8.00	1425					0.0843	0.9090	8.48	0.788				
7	16.00	1425					0.0897	0.9036	9.02	0.777				
8	32.00	1440					0.0960	0.8973	9.65	0.765				
9	8.00	105					0.0950	0.8983	9.55	0.767				
10	2.00	105					0.0948	0.8985	9.54	0.767				
11	0.50	165					0.0939	0.8994	9.45	0.769				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title:			
Job Short Title: Copper Flat Tailings Design Study		ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
Sample: BH-18 @ 43.5-48.5 ft		Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006
					Figure: 1



Void Ratio vs. Axial Stress

$$C_c \approx \frac{.80 - .768}{\log\left(\frac{32}{4}\right)} \approx .04$$



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT CONSOLIDATION PLOTS				
Job Short Title: Copper Flat Tailings Design Study						
Sample: BH-18 @ 43.5-48.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/25/2013	Job Number: 103-92557.006	Figure: 2	

	Initial	Final
Height =	1.000 in	0.981 in
Diameter =	2.497 in	2.497 in
Area =	4.897 in <sup>2</sup>	4.897 in <sup>2</sup>
Volume =	4.897 in <sup>3</sup>	4.804 in <sup>3</sup>
Water Content =	9.7%	1.1%
Specific Gravity =	2.70 (Assumed)	2.70 (Assumed)
Height of Solids =	0.5363 in	0.5363 in
Void Ratio =	0.865	0.829
Degree of Saturation =	30.3%	3.6%
Wet Mass =	0.281 lb	0.259 lb
Dry Mass =	0.256 lb	0.256 lb
Wet Unit Weight =	99.0 pcf	93.0 pcf
Dry Unit Weight =	90.2 pcf	92.0 pcf

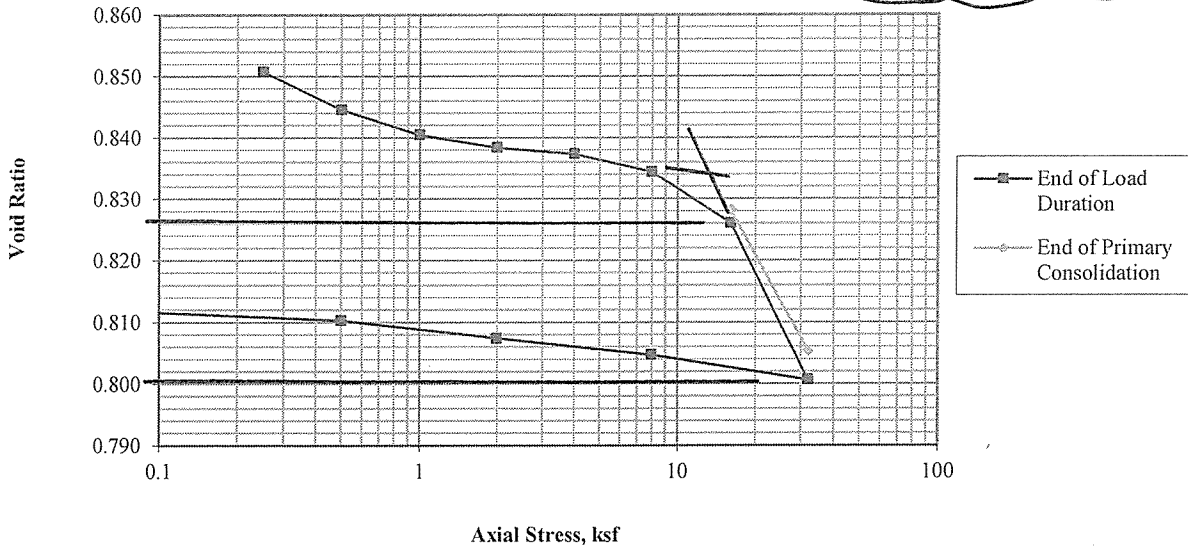
**Notes**  
USCS description (ASTM D2487): Sandy silty clay, brownish yellow, moist  
Atterberg Limits (ASTM D4318): LL = 25 PL = 21 PI = 4  
Percent Finer (ASTM D422): 3/4 in. = 99% No. 4 = 99% No. 200 = 52%  
Specimen Type:  Intact  Reconstituted  
Remold Targets: 90.0 pcf (dry) at 10.0% moisture  
Water Content of Trimmings (ASTM D2216): 9.4%  
Trimming Procedure: Specimen remolded in ring  
Inundation:  Not inundated  Inundated  
Test Method:  A  B  
Apparatus: Frame No. 6 (ELE C-320A)  
Final Water Content Specimen:  Entire  Partial  
Final Differential Height: -0.0094 in  
Estimated Preconsolidation Stress: 13.0 ksf

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	944					0.0000	0.9957	0.00	0.857				
1	0.25	1410					0.0031	0.9926	0.31	0.851				
2	0.50	1410					0.0064	0.9893	0.64	0.845				
3	1.00	1440					0.0086	0.9871	0.86	0.841				
4	2.00	1470					0.0097	0.9860	0.97	0.838				
5	4.00	1410					0.0103	0.9854	1.03	0.837				
6	8.00	1410					0.0119	0.9838	1.19	0.834				
7	16.00	1425	0.0152	0.9805	1.52	0.828	0.0164	0.9794	1.64	0.826	2 (Root time)	0.829	23.310	0.3
8	32.00	1470	0.0275	0.9682	2.75	0.805	0.0300	0.9657	3.00	0.801	2 (Root time)	0.807	3.521	0.4
9	8.00	75					0.0278	0.9679	2.78	0.805				
10	2.00	95					0.0263	0.9694	2.63	0.807				
11	0.50	1315					0.0248	0.9709	2.48	0.810				
12	0.10	195					0.0241	0.9716	2.41	0.812				

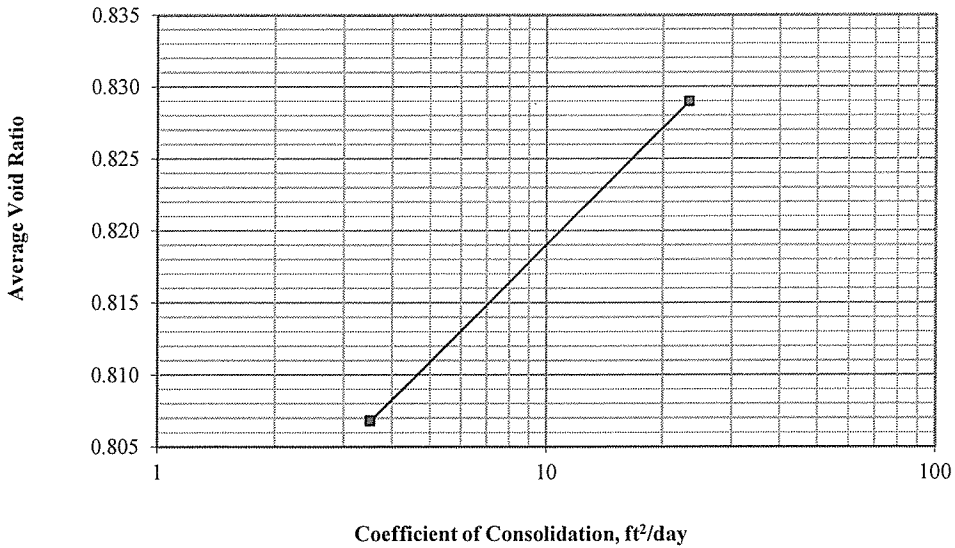
<b>Golder Associates Inc.</b> Denver, Colorado		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-22 @ 0-8.5 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

Void Ratio vs. Axial Stress

$$C_c = \frac{.826 - .80}{\log\left(\frac{32}{16}\right)} = 0.09$$



Average Void Ratio vs. Coefficient of Consolidation



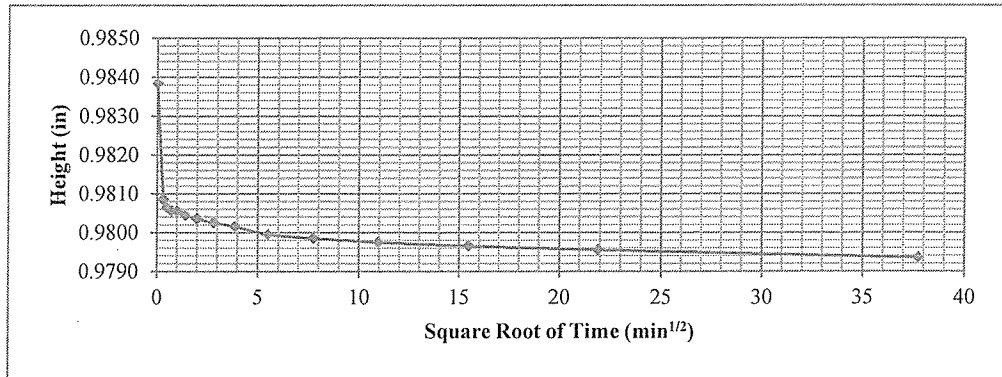
**Golder Associates Inc.**  
**Denver, Colorado**

Job Short Title:  
 Copper Flat Tailings Design Study

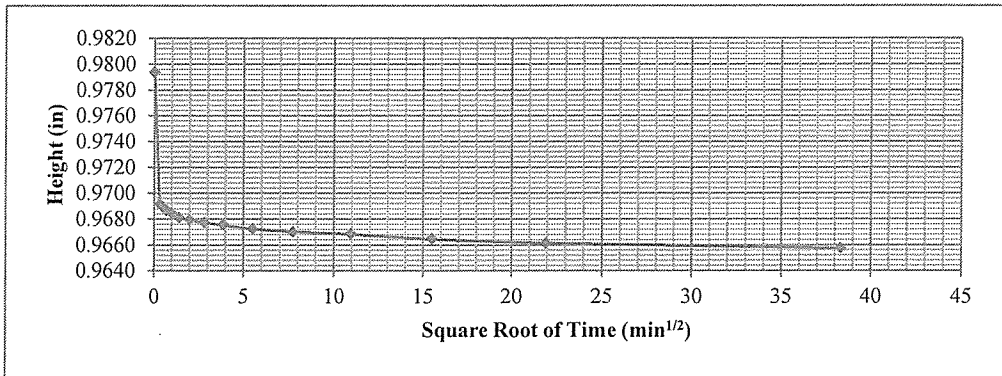
Title:  
 ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 CONSOLIDATION PLOTS

Sample:	Technician:	Reviewed:	Start Date:	Job Number:	Figure:
BH-22 @ 0-8.5 ft	RJM	CCS	3/11/2013	103-92557.006	2

16.00 ksf



32.00 ksf

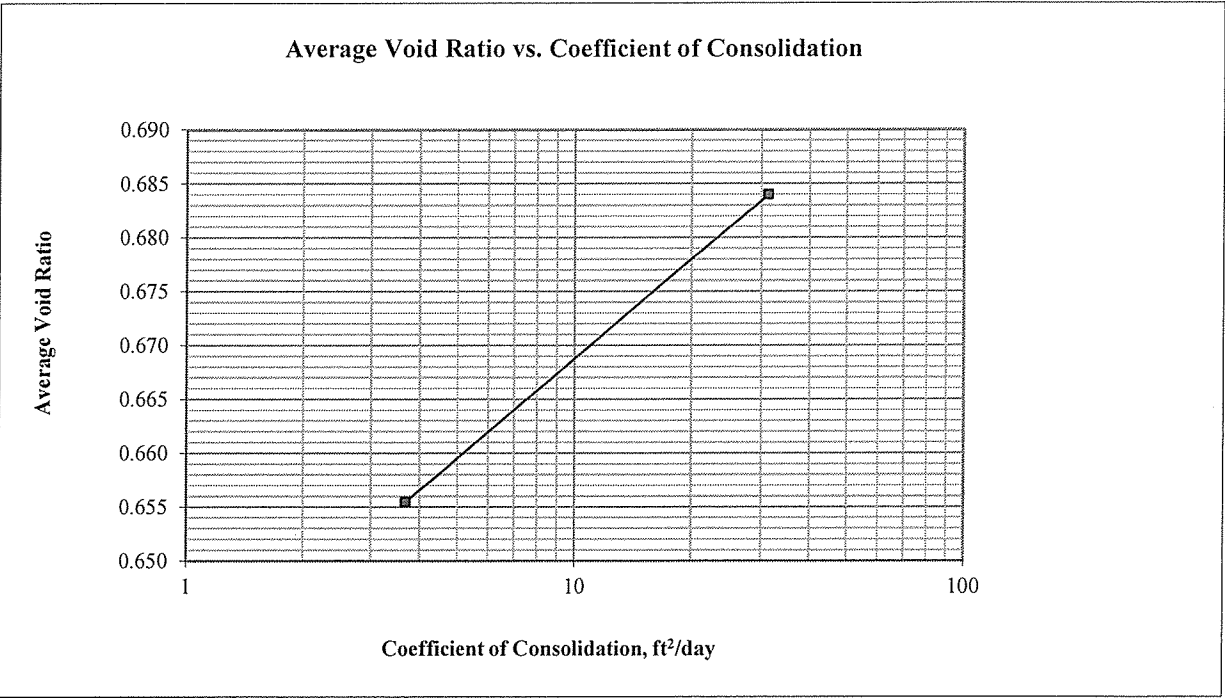
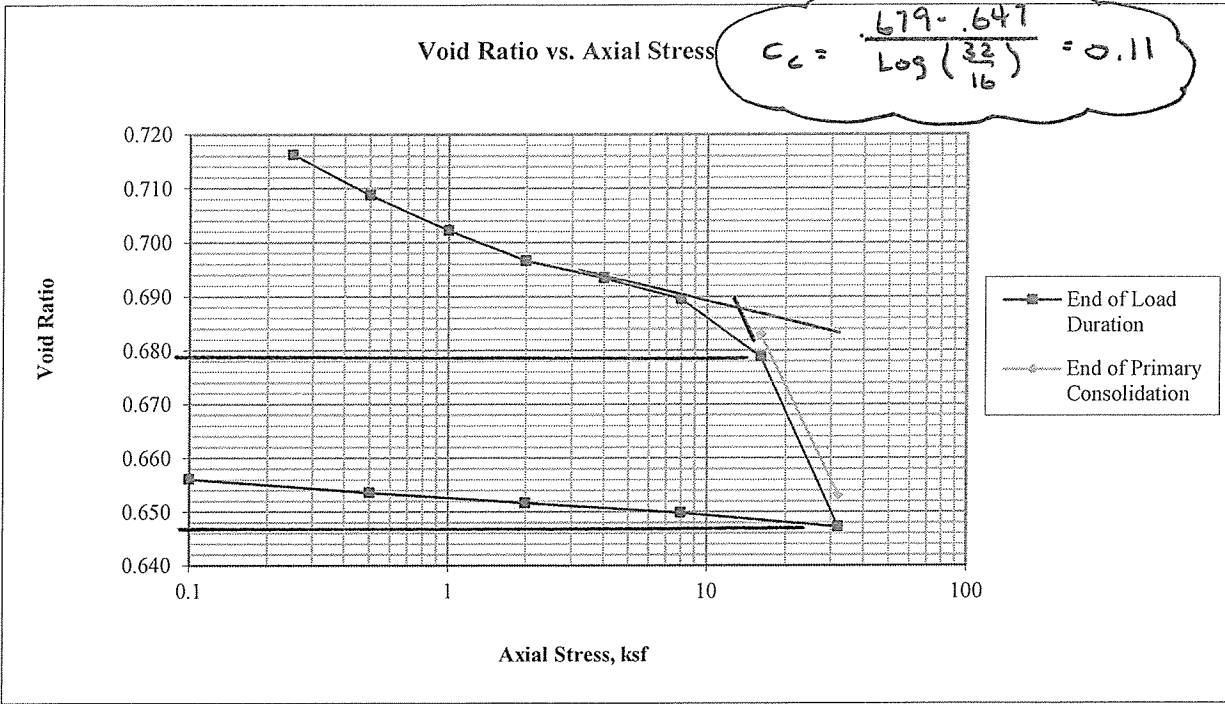


<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>			
Job Short Title: Copper Flat Tailings Design Study					
Sample: BH-22 @ 0-8.5 ft	Technician: RJM	Reviewed: CCS	Start Date: 3/11/2013	Job Number: 103-92557.006	Figure: 3

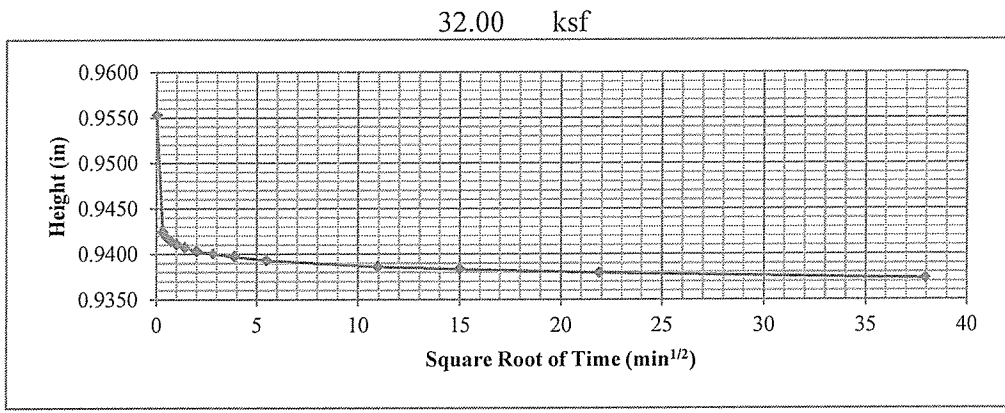
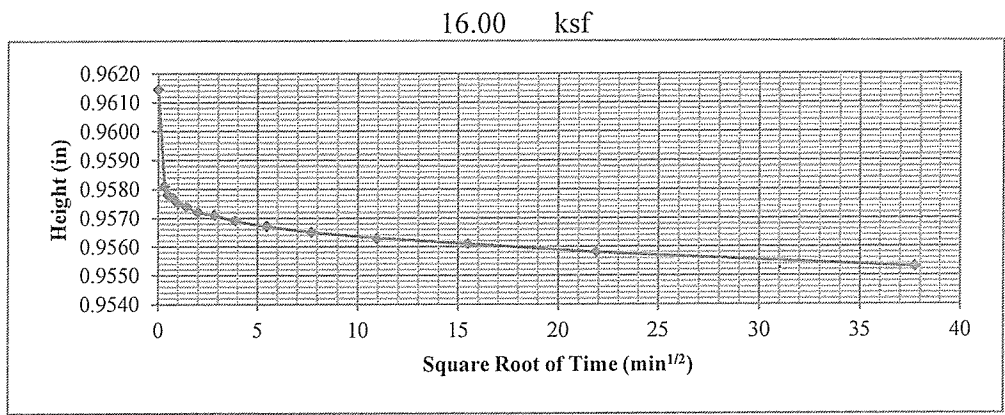
	<b>Initial</b>		<b>Final</b>	<b>Notes</b>			
Height =	0.997 in		0.924 in	Visual description (Golder procedure):	CLAYEY SAND, pale red, moist		
Diameter =	2.498 in		2.498 in	Atterberg Limits (ASTM D4318):	LL = 36	PL = 18	PI = 18
Area =	4.901 in <sup>2</sup>		4.901 in <sup>2</sup>	Percent Finer (ASTM D422):	3/4 in. = 98%	No. 4 = 82%	No. 200 = 37%
Volume =	4.886 in <sup>3</sup>		4.528 in <sup>3</sup>	Specimen Type:	<input type="checkbox"/> Intact	<input checked="" type="checkbox"/> Reconstituted	
Water Content =	14.2%		2.9%	Remold Targets:	95.0 pcf (dry) at	15.0% moisture	
Specific Gravity =	2.70 (Assumed)		2.70 (Assumed)	Water Content of Trimmings (ASTM D2216):	14.5%		
Height of Solids =	0.5690 in		0.5690 in	Trimming Procedure:	Specimen remolded in ring		
Void Ratio =	0.752		0.624	Inundation:	<input checked="" type="checkbox"/> Not inundated	<input type="checkbox"/> Inundated	
Degree of Saturation =	50.8%		12.6%	Test Method:	<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	
Wet Mass =	0.310 lb		0.279 lb	Apparatus:	Frame No. 5	(ELE C-320A)	
Dry Mass =	0.272 lb		0.272 lb	Final Water Content Specimen:	<input checked="" type="checkbox"/> Entire	<input type="checkbox"/> Partial	
Wet Unit Weight =	109.6 pcf		106.6 pcf	Final Differential Height:	0.0184 in		
Dry Unit Weight =	96.0 pcf		103.6 pcf	Estimated Preconsolidation Stress:	14.5 ksf		

	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	980					0.0000	0.9843	0.00	0.730				
1	0.25	1410					0.0077	0.9766	0.77	0.716				
2	0.50	1410					0.0119	0.9724	1.20	0.709				
3	1.00	1440					0.0156	0.9687	1.57	0.702				
4	2.00	1470					0.0188	0.9655	1.89	0.697				
5	4.00	1410					0.0207	0.9636	2.07	0.693				
6	8.00	1415					0.0229	0.9614	2.29	0.690				
7	16.00	1425	0.0267	0.9576	2.67	0.683	0.0290	0.9553	2.91	0.679	2 (Root time)	0.684	31.272	0.3
8	32.00	1440	0.0436	0.9407	4.37	0.653	0.0470	0.9373	4.71	0.647	2 (Root time)	0.655	3.684	0.4
9	8.00	75					0.0455	0.9388	4.56	0.650				
10	2.00	130					0.0444	0.9399	4.46	0.652				
11	0.50	100					0.0433	0.9410	4.34	0.654				
12	0.10	70					0.0419	0.9424	4.20	0.656				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA					
<b>Job Short Title:</b> Copper Flat Tailings Design Study							
<b>Sample:</b> BH-22 @ 28-30 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/11/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1		



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435          ONE-DIMENSIONAL CONSOLIDATION TEST REPORT          CONSOLIDATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study					
Sample: <b>BH-22 @ 28-30 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/11/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>2</b>



**Golder Associates Inc.**  
**Denver, Colorado**

Title: **ASTM D2435**  
**ONE-DIMENSIONAL CONSOLIDATION TEST REPORT**  
**TIME-DEFORMATION PLOTS**

Job Short Title:  
 Copper Flat Tailings Design Study

Sample: **BH-22 @ 28-30 ft**

Technician:  
 RJM

Reviewed:  
 CCS

Start Date:  
 3/11/2013

Job Number:  
 103-92557.006

Figure:  
 3

	<b>Initial</b>		<b>Final</b>
Height =	1.000 in	0.973 in	
Diameter =	2.498 in	2.498 in	
Area =	4.901 in <sup>2</sup>	4.901 in <sup>2</sup>	
Volume =	4.901 in <sup>3</sup>	4.769 in <sup>3</sup>	
Water Content =	9.8%	4.2%	
Specific Gravity =	2.70 (Assumed)	2.70 (Assumed)	
Height of Solids =	0.5558 in	0.5558 in	
Void Ratio =	0.799	0.751	
Degree of Saturation =	33.0%	15.1%	
Wet Mass =	0.291 lb	0.276 lb	
Dry Mass =	0.265 lb	0.265 lb	
Wet Unit Weight =	102.7 pcf	100.1 pcf	
Dry Unit Weight =	93.5 pcf	96.1 pcf	

**Notes**  
USCS description (ASTM D2487):  
Atterberg Limits (ASTM D4318):  
Percent Finer (ASTM D422):  
Specimen Type:  
Remold Targets:  
Water Content of Trimmings (ASTM D2216):  
Trimming Procedure:  
Inundation:  
Test Method:  
Apparatus:  
Final Water Content Specimen:  
Final Differential Height:  
Estimated Preconsolidation Stress:

Clayey sand, reddish brown, moist  
LL = 37 PL = 17 PI = 20  
3/4 in. = 100% No. 4 = 95% No. 200 = 39%  
Intact  Reconstituted  
93.0 pcf (dry) at 10.0% moisture  
9.9%  
Specimen remolded in ring  
 Not inundated  Inundated  
 A  B  
Frame No. 6 (ELE C-320A)  
 Entire  Partial  
-0.0136 in  
13.8 ksf

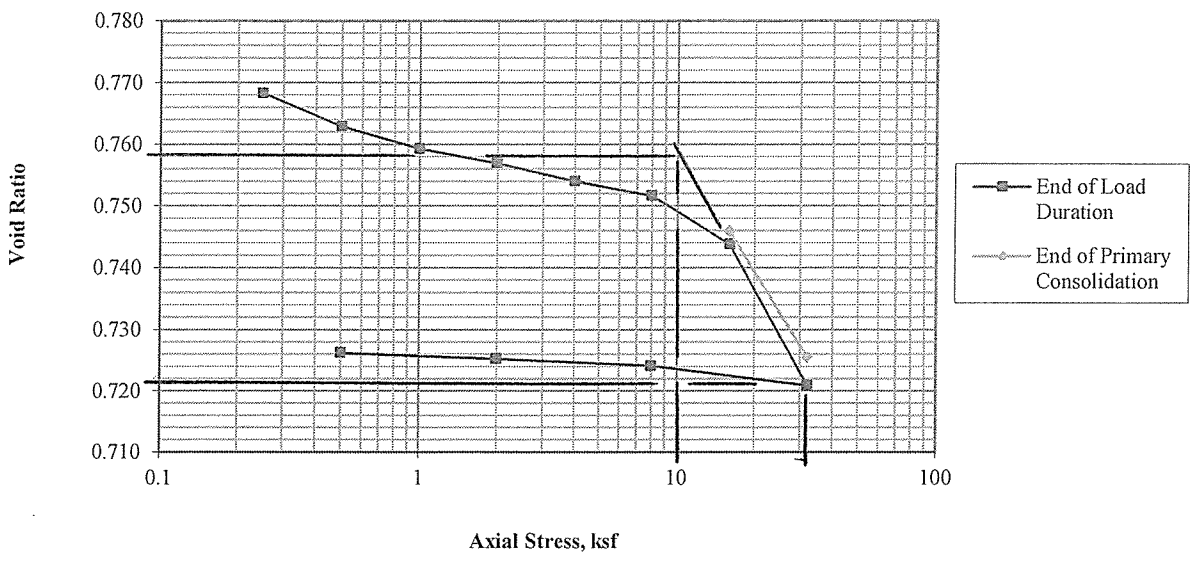
	Axial Stress (ksf)	Load Duration (min)	At End of Primary Consolidation				At End of Load Duration				Time Deformation Method	Average Void Ratio	Coefficient of Consolidation (ft <sup>2</sup> /day)	Time to 50% Consolidation (min)
			Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio	Deformation (in)	Specimen Height (in)	Axial Strain (%)	Void Ratio				
Seating	0.10	960					0.0000	0.9894	0.00	0.780				
1	0.25	1440					0.0066	0.9828	0.66	0.768				
2	0.50	1425					0.0096	0.9798	0.96	0.763				
3	1.00	1440					0.0116	0.9778	1.16	0.759				
4	2.00	1410					0.0129	0.9765	1.29	0.757				
5	4.00	2770					0.0146	0.9748	1.46	0.754				
6	8.00	1410					0.0159	0.9735	1.59	0.752				
7	16.00	1410	0.0191	0.9703	1.91	0.746	0.0203	0.9692	2.03	0.744	2 (Root time)	0.746	1.313	0.9
8	32.00	1440	0.0304	0.9590	3.04	0.726	0.0330	0.9564	3.30	0.721	2 (Root time)	0.727	3.137	0.5
9	8.00	80					0.0312	0.9582	3.12	0.724				
10	2.00	100					0.0305	0.9589	3.05	0.725				
11	0.50	180					0.0300	0.9594	3.00	0.726				

<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>		<b>Title:</b> ASTM D2435 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT SPECIMEN AND SUMMARY DATA			
<b>Job Short Title:</b> Copper Flat Tailings Design Study					
<b>Sample:</b> BH-25 @ 22-34 ft	<b>Technician:</b> RJM	<b>Reviewed:</b> CCS	<b>Start Date:</b> 3/25/2013	<b>Job Number:</b> 103-92557.006	<b>Figure:</b> 1

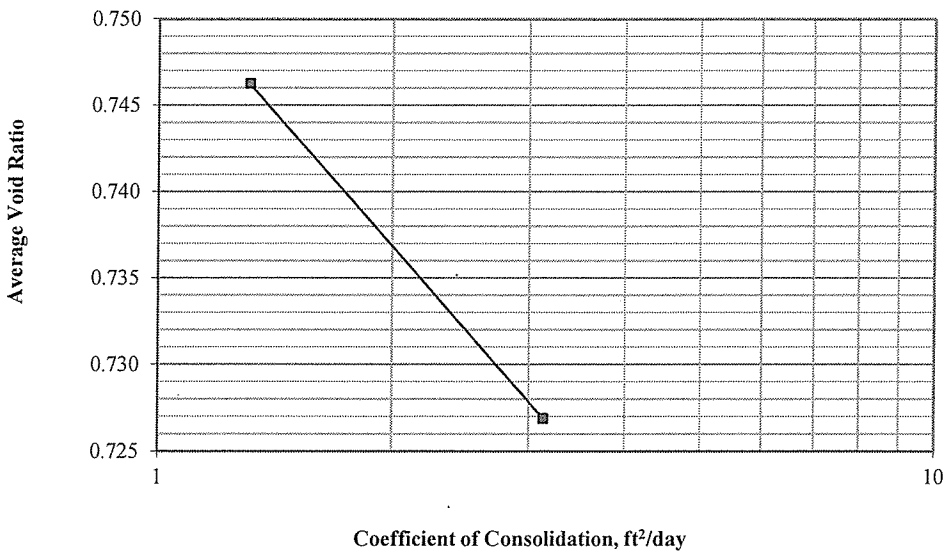


Void Ratio vs. Axial Stress

$$C_c = \frac{.758 - .721}{\log\left(\frac{32}{10}\right)} \approx .07$$



Average Void Ratio vs. Coefficient of Consolidation



**Golder Associates Inc.**  
**Denver, Colorado**

Title:

ASTM D2435  
 ONE-DIMENSIONAL CONSOLIDATION TEST REPORT  
 CONSOLIDATION PLOTS

Job Short Title:

Copper Flat Tailings Design Study

Sample:

BH-25 @ 22-34 ft

Technician:

RJM

Reviewed:

CCS

Start Date:

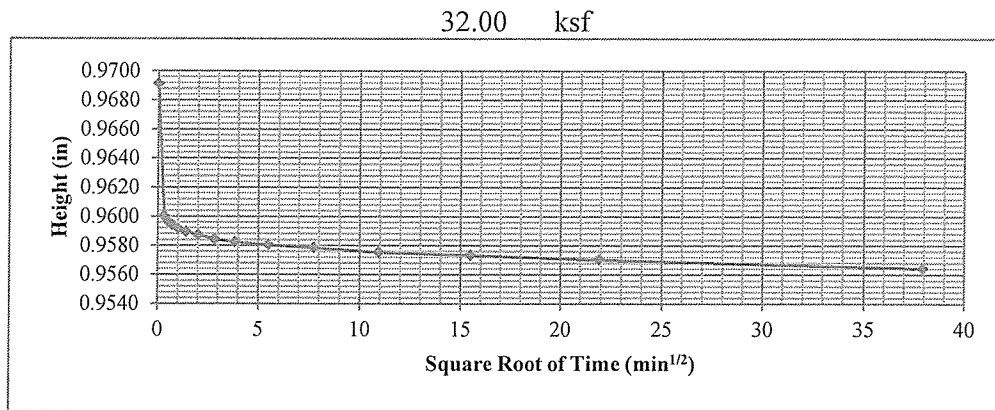
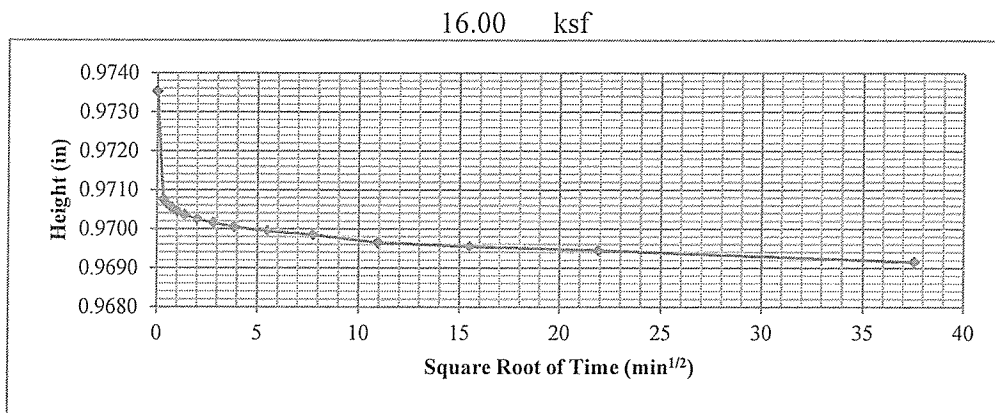
3/25/2013

Job Number:

103-92557.006

Figure:

2



<b>Golder Associates Inc.</b> <b>Denver, Colorado</b>	Title: <b>ASTM D2435</b> <b>ONE-DIMENSIONAL CONSOLIDATION TEST REPORT</b> <b>TIME-DEFORMATION PLOTS</b>				
Job Short Title: Copper Flat Tailings Design Study					
Sample: <b>BH-25 @ 22-34 ft</b>	Technician: <b>RJM</b>	Reviewed: <b>CCS</b>	Start Date: <b>3/25/2013</b>	Job Number: <b>103-92557.006</b>	Figure: <b>3</b>

**ATTACHMENT 2**  
**CONSOLIDATION TESTING SUMMARY AND SETTLE 3D MODEL INPUT TABLES**

**Table 1: Summary Results Of Consolidation Tests -- Copper Flat, Sierra County, New Mexico**

Boring No.	Sample Depth	In-situ Overburden Pressure at Midpoint of Tested Stratum (kips, Note 1)	Material Classification	Void Ratio, min	Void Ratio, max	Void Ratio, mean	Estimated Initial Void Ratio (from consolidation curve)	Cc for Analysis (Note 2)	Estimated Moist Density (final) (pcf) (Note 3)
BH-10	19-33	2.86	CL	0.666	0.74	0.703	0.69	0.05	104
BH-12	33.5-48.5	4.51	CH	0.67	0.845	0.7575	0.69	0.3	104
BH-16	29-34	3.465	SC	0.664	0.738	0.701	0.704	0.09	110
BH-18	23-33.5	3.1075	CL	0.706	0.742	0.724	0.724	0.04	103
BH-18	43.5-48.5	5.06	CH	0.767	0.925	0.846	0.79	0.04	102
BH-22	0-8.5	0.4675	ML	0.8	0.85	0.825	0.844	0.09	98
BH-22	28-30	3.19	SC	0.646	0.716	0.681	0.695	0.11	108
BH-25	22-34	3.08	SC	0.727	0.746	0.7365	0.756	0.07	101

**Notes**

1. Based on an assumed unit weight of 110 pcf.
2. Generally based on portion of curve developed for loading between 16 and 32 kips.
3. Moist Density estimated for water content ranging from 3 to 6 percent.

**Table 2: Stratum Parameters For Settlement Analysis -- Copper Flat, Sierra County, New Mexico**

Stratum Designation	Soil Types	Assumed Initial Void Ratio	Assumed Cc Value	Referenced Consolidation Tests	Assumed Unit Weight (pcf)	Young's Modulus (ksf)
1	ML, CL-ML, SM	0.74	0.09	BH-16, BH-22 (2), BH-25	106	NA
2	CL	0.71	0.045	BH-10, BH-18	104	NA
3	CH, MH	0.75	0.035	BH-12, BH-18	103	NA
4 (Note 1)	Gravel, Sand	NA	NA	Ref. 1	110	4000
5 (Note 2)	Basalt, Caliche	NA	NA	Ref. 1	125	4000
6 (Note 3)	Embankment Fill	NA	NA	NA	97	NA
7 (Note 4)	Tailings	NA	NA	NA	97	NA

**Notes**

1. Settlement within embankment fill not calculated for foundation settlement evaluation.
2. Settlement within tailings not analyzed for foundation settlement evaluation.

**References**

1. US Army Corps of Engineer, EM-1110-1904, 1990.

**Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico**

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

**Notes**

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.

**Table 3A: SETTLE3D Input Parameters -- Copper Flat, Sierra County, New Mexico**

Cross-Section	Analysis Section	Stratum Number	Thickness (ft)	Unit Weight (kcf)	Loading Pressure (rectangular, ksf)	Assumed Poisson Ratio (Note 1)	Cc	Cr (Note 2)	e0	OCR
B-B'	1	6	234.4	0.097	22.7	na	na	na	na	na
" "	" "	2	19.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	39.8	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	27.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	2	6	143	0.097	13.9	na	na	na	na	na
" "	" "	2	66.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	2	9	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	12	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	14	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	3	6	137	0.097	13.3	na	na	na	na	na
" "	" "	2	15.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	2.1	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	28.3	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28.6	0.103		0.3	0.035	0.1	0.75	1
B-B'	4	6	75.2	0.097	7.3	na	na	na	na	na
" "	" "	2	12.5	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	4.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	23	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	28	0.103		0.3	0.035	0.1	0.75	1
B-B'	5	6	9.8	0.097	1.0	na	na	na	na	na
" "	" "	1	18.8	0.106		0.3	0.09	0.1	0.74	1
" "	" "	3	5.9	0.103		0.3	0.035	0.1	0.75	1
" "	" "	2	19.2	0.104		0.3	0.045	0.1	0.71	1
" "	" "	3	25.1	0.103		0.3	0.035	0.1	0.75	1

**Notes**

1. Poisson Ratio of 0.3 is approximation, based on published values ranging from 0.2 to 0.4 for sandy clays.
2. Cr value of 0.1 loaded into program, although rebound is not projected for the project.

**ATTACHMENT 3  
SETTLE 3D OUTPUT FILES**



# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 1.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.49066 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.97303
Consolidation Settlement [ft]	0	1.97303
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	22.7
Total Stress [ksf]	0	31.6782
Total Strain	-0	0.10667
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.020072	31.6754
Over-consolidation Ratio	1	1
Void Ratio	0.527594	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 22.7 ksf  
 Depth: 0 ft

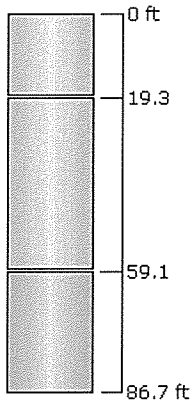
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	19.3	0
2	Soil Stratum 2	39.8	19.3
3	Soil Stratum 3	27.6	59.1



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 2.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.646645 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98619
Consolidation Settlement [ft]	0	1.98619
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.9
Total Stress [ksf]	0	27.3058
Total Strain	-0	0.0869309
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.006916	27.303
Over-consolidation Ratio	1	1
Void Ratio	0.561348	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 13.9 ksf  
 Depth: 0 ft

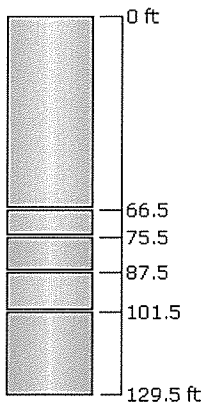
Installation Stage: Stage 1

**Coordinates**



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	66.5	0
2	Soil Stratum 2	9	66.5
3	Soil Stratum 3	12	75.5
4	Soil Stratum 2	14	87.5
5	Soil Stratum 3	28	101.5



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 59

## Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 3.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.561914 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.4179
Consolidation Settlement [ft]	0	1.4179
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	13.3
Total Stress [ksf]	0	21.0132
Total Strain	-0	0.103067
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.01612	21.0103
Over-consolidation Ratio	1	1
Void Ratio	0.533756	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 13.3 ksf  
 Depth: 0 ft

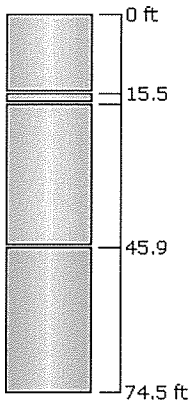
Installation Stage: Stage 1

**Coordinates**



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	15.5	0
2	Soil Stratum 3	2.1	15.5
3	Soil Stratum 2	28.3	17.6
4	Soil Stratum 3	28.6	45.9



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

**Field Point Grid**



Number of points: 289  
Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 4.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.491452 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.00705
Consolidation Settlement [ft]	0	1.00705
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	7.3
Total Stress [ksf]	0	14.379
Total Strain	-0	0.0723724
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.065	14.3761
Over-consolidation Ratio	1	1
Void Ratio	0.586243	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 7.3 ksf  
 Depth: 0 ft

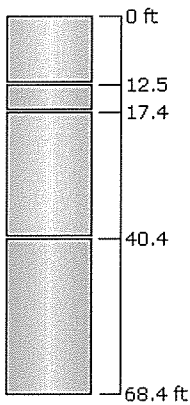
Installation Stage: Stage 1

**Coordinates**



X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 2	12.5	0
2	Soil Stratum 3	4.9	12.5
3	Soil Stratum 2	23	17.4
4	Soil Stratum 3	28	40.4



**Soil Properties**

Property	Soil Stratum 3	Soil Stratum 2
Color		
Unit Weight [kips/ft <sup>3</sup> ]	0.103	0.104
Primary Consolidation	Enabled	Enabled
Material Type	Non-Linear	Non-Linear
Cc	0.035	0.045
Cr	0.1	0.1
e0	0.75	0.71
OCR	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 53

**Field Point Grid**

Number of points: 289  
Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section B-B', Line 5.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.562388 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.508677
Consolidation Settlement [ft]	0	0.508677
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	1
Total Stress [ksf]	0	8.18236
Total Strain	-0	0.139728
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019928	8.17977
Over-consolidation Ratio	1	1
Void Ratio	0.496874	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 1 ksf  
 Depth: 0 ft

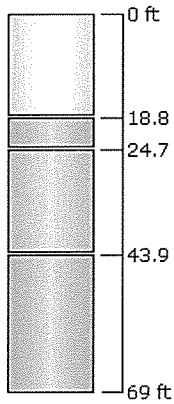
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.8	0
2	Soil Stratum 3	5.9	18.8
3	Soil Stratum 2	19.2	24.7
4	Soil Stratum 3	25.1	43.9



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 61

**Field Point Grid**

Number of points: 289  
Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 1.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.499416 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	2.11072
Consolidation Settlement [ft]	0	2.11072
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	23.4
Total Stress [ksf]	0	28.7145
Total Strain	-0	0.211235
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.019292	28.7127
Over-consolidation Ratio	1	1
Void Ratio	0.372451	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 23.4 ksf  
 Depth: 0 ft



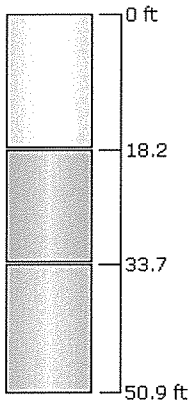
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	18.2	0
2	Soil Stratum 3	15.5	18.2
3	Soil Stratum 2	17.2	33.7



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 2.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.498987 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.98358
Consolidation Settlement [ft]	0	1.98358
Immediate Settlement [ft]	0	0
Loading Stress [ksf]	0	18.5
Total Stress [ksf]	0	24.1716
Total Strain	-0	0.206331
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.018974	24.1698
Over-consolidation Ratio	1	1
Void Ratio	0.380985	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 18.5 ksf  
 Depth: 0 ft

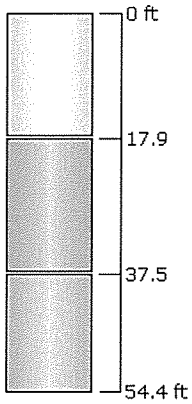
Installation Stage: Stage 1

**Coordinates**




X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	17.9	0
2	Soil Stratum 3	19.6	17.9
3	Soil Stratum 2	16.9	37.5



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104
Primary Consolidation	Enabled	Enabled	Enabled
Material Type	Non-Linear	Non-Linear	Non-Linear
Cc	0.09	0.035	0.045
Cr	0.1	0.1	0.1
e0	0.74	0.75	0.71
OCR	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 55

**Field Point Grid**

Number of points: 289

Expansion Factor: 2

**Grid Coordinates**

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 3.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.576466 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	1.40443
Consolidation Settlement [ft]	0	1.35522
Immediate Settlement [ft]	0	0.0492125
Loading Stress [ksf]	0	12.7
Total Stress [ksf]	0	18.5709
Total Strain	-0	0.203888
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.014522	18.5692
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 12.7 ksf  
 Depth: 0 ft

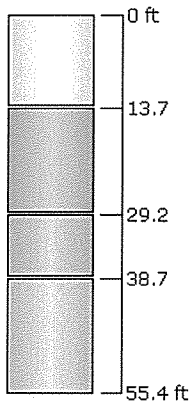
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	13.7	0
2	Soil Stratum 4	15.5	13.7
3	Soil Stratum 3	9.5	29.2
4	Soil Stratum 2	16.7	38.7



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4
Color				
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11
Immediate Settlement	Disabled	Disabled	Disabled	Enabled
Es [ksf]				4000
Esur [ksf]				4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear	
Cc	0.09	0.035	0.045	
Cr	0.1	0.1	0.1	
e0	0.74	0.75	0.71	
OCR	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
1	15.349, 19.014	Auto: 65

## Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01



# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

---

Document Name: Section D-D', Line 4.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

---

Stage #	Name
1	Stage 1

### Results

---

Time taken to compute: 0.636294 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.948079
Consolidation Settlement [ft]	0	0.914599
Immediate Settlement [ft]	0	0.03348
Loading Stress [ksf]	0	6.2
Total Stress [ksf]	0	12.7328
Total Strain	-0	0.141231
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.05777	12.7313
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

---

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 6.2 ksf  
 Depth: 0 ft

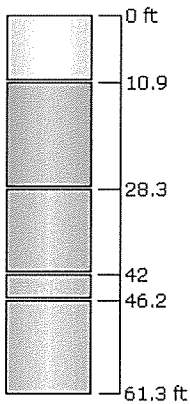
Installation Stage: Stage 1

**Coordinates**






X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	10.9	0
2	Soil Stratum 4	17.4	10.9
3	Soil Stratum 3	13.7	28.3
4	Soil Stratum 5	4.2	42
5	Soil Stratum 2	15.1	46.2



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	4000
Esur [ksf]				4000	4000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 71

### Field Point Grid

---

Number of points: 289  
Expansion Factor: 2

#### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

# Settle3D Analysis Information

## Copper Flat Embankment Post-Construction Settlement

### Project Settings

Document Name: Section D-D', Line 5.s3z  
 Project Title: Copper Flat Embankment Post-Construction Settlement  
 Analysis: 1-D Boussinesq  
 Author: David Poe, P.E.  
 Company: Golder Associates  
 Date Created: 4/25/2013, 2:08:28 PM  
 Stress Computation Method: Boussinesq  
 Use average properties to calculate layered stresses

### Stage Settings

Stage #	Name
1	Stage 1

### Results

Time taken to compute: 0.583683 seconds

#### Stage: Stage 1

Data Type	Minimum	Maximum
Total Settlement [ft]	0	0.175852
Consolidation Settlement [ft]	0	0.173272
Immediate Settlement [ft]	0	0.00257999
Loading Stress [ksf]	0	0.4
Total Stress [ksf]	0	7.63261
Total Strain	-0	0.0875971
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.04134	7.61899
Over-consolidation Ratio	1	1
Void Ratio	0	0.75
Hydroconsolidation Settlement [ft]	0	0

### Loads

#### 1. Rectangular Load

Length: 2000 ft  
 Width: 2000 ft  
 Rotation angle: 0 degrees  
 Load Type: Flexible  
 Area of Load: 4e+006 ft<sup>2</sup>  
 Load: 0.4 ksf  
 Depth: 0 ft

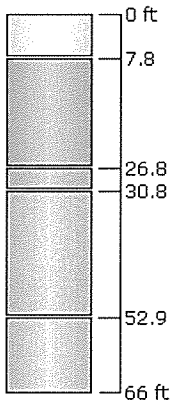
Installation Stage: Stage 1

**Coordinates**

X [ft]	Y [ft]
-984.651	-980.986
1015.35	-980.986
1015.35	1019.01
-984.651	1019.01

**Soil Layers**

Layer #	Type	Thickness [ft]	Depth [ft]
1	Soil Stratum 1	7.8	0
2	Soil Stratum 4	19	7.8
3	Soil Stratum 3	4	26.8
4	Soil Stratum 5	22.1	30.8
5	Soil Stratum 2	13.1	52.9



**Soil Properties**

Property	Soil Stratum 1	Soil Stratum 3	Soil Stratum 2	Soil Stratum 4	Soil Stratum 5
Color					
Unit Weight [kips/ft <sup>3</sup> ]	0.106	0.103	0.104	0.11	0.115
Immediate Settlement	Disabled	Disabled	Disabled	Enabled	Enabled
Es [ksf]				4000	13000
Esur [ksf]				4000	13000
Primary Consolidation	Enabled	Enabled	Enabled	Disabled	Disabled
Material Type	Non-Linear	Non-Linear	Non-Linear		
Cc	0.09	0.035	0.045		
Cr	0.1	0.1	0.1		
e0	0.74	0.75	0.71		
OCR	1	1	1	1	1

**Query Points**

Point #	(X,Y) Location	Number of Divisions
---------	----------------	---------------------

1 15.349, 19.014 Auto: 63

### Field Point Grid

---

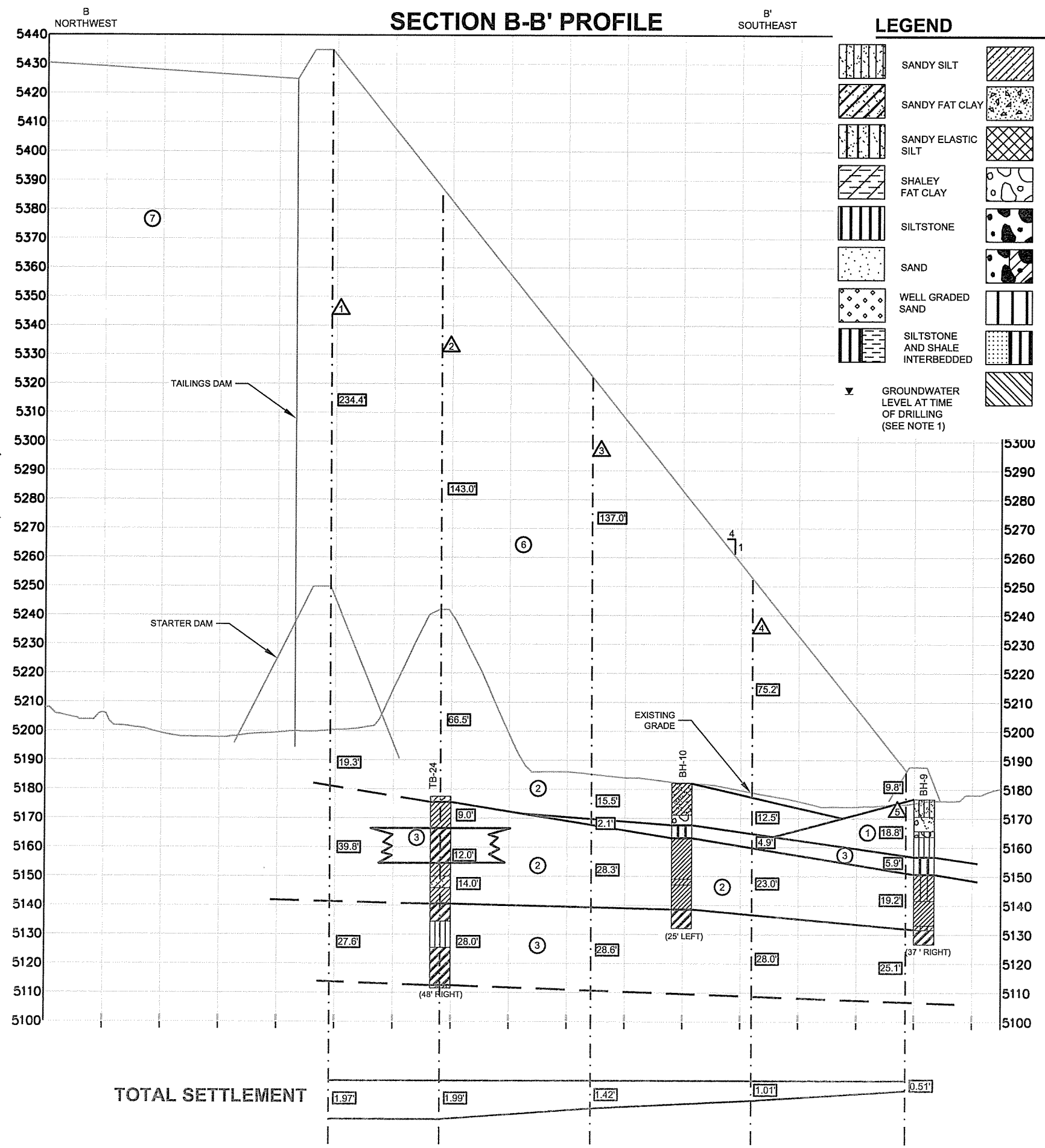
Number of points: 289  
Expansion Factor: 2

#### Grid Coordinates

X [ft]	Y [ft]
2015.35	2019.01
2015.35	-1989.31
-1989.58	-1989.31
-1989.58	2019.01

**ATTACHMENT 4**  
**EMBANKMENT ANALYSIS CROSS-SECTIONS WITH GRAPHICAL OUTPUT RESULTS**  
**(SECTIONS B-B' AND D-D' ONLY)**

# SECTION B-B' PROFILE



## LEGEND

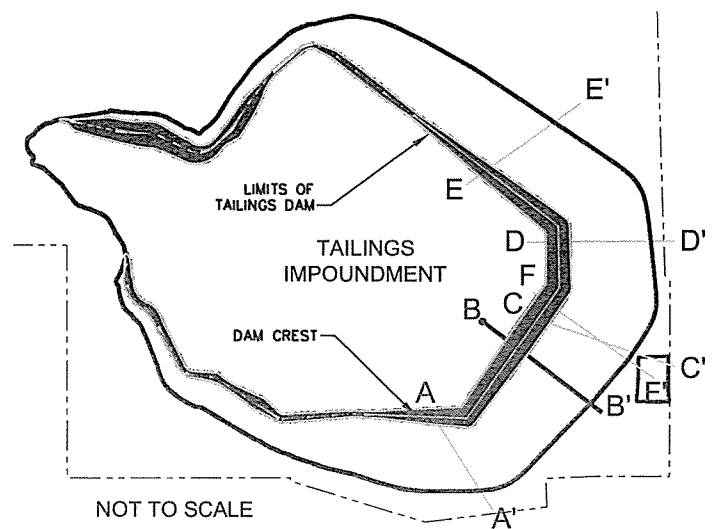
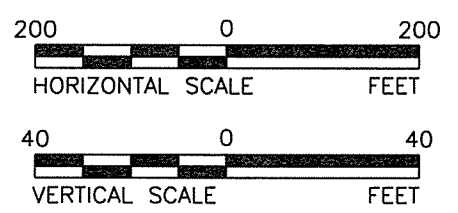
	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		USCS CL-CH		POORLY GRADED SAND WITH SILT

### STRATUM DESCRIPTORS

①	ML, CL-ML, SM
②	CL
③	CH, MH
④	GRAVEL, SAND
⑤	BASALT, CALICHE
⑥	EMBANKMENT FILL
⑦	TAILINGS (CONSOLIDATED)
△	ANALYSIS SECTION
12.0'	STRATUM/FILL THICKNESS

## NOTES

- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.

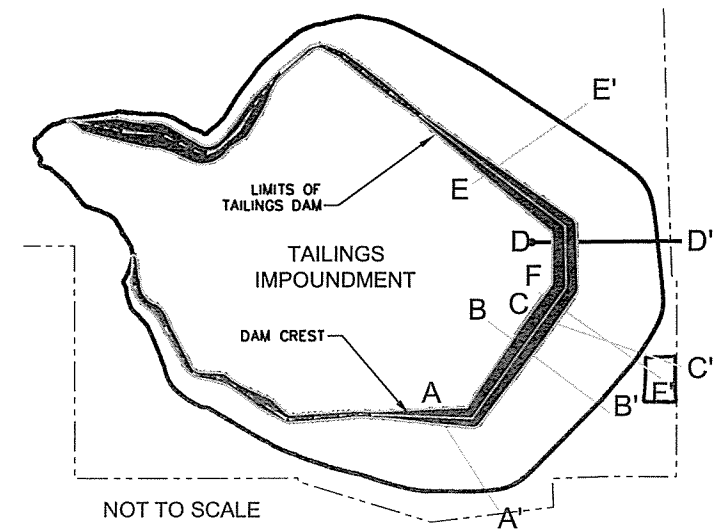
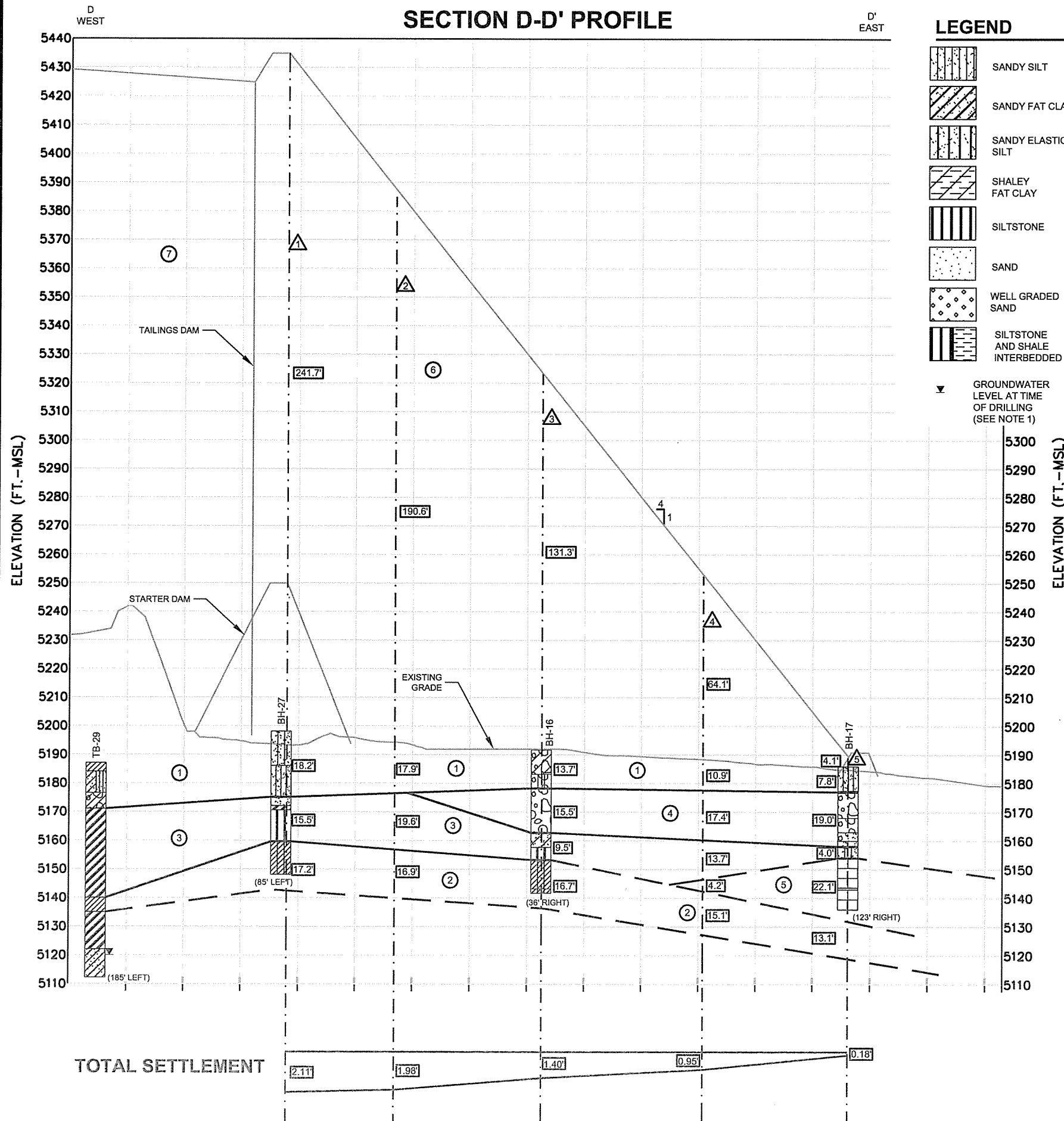


**DRAFT**

PROJECT		COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO			
TITLE		GEOLOGIC CROSS SECTION B-B'			
PROJECT No.		10392557		FILE No. 10392557A003	
DESIGN	CMT	2/21/13	SCALE	AS SHOWN	
CADD	CMT	2/21/13	FIGURE	5	
CHECK	DEP	2/21/13			
REVIEW	GM	2/21/13			



# SECTION D-D' PROFILE



**DRAFT**

PROJECT				COPPER FLAT PROJECT TAILINGS STORAGE FACILITY SIERRA COUNTY, NEW MEXICO			
TITLE				GEOLOGIC CROSS SECTION D-D'			
PROJECT No.		10392557		FILE No.		10392557A005	
DESIGN	CMT	2/21/13	SCALE	AS SHOWN			
CADD	CMT	2/21/13	FIGURE	7			
CHECK	DEP	2/21/13					
REVIEW	GM	2/21/13					



**APPENDIX I.2  
SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS**

**REVISED JUNE 2016  
REVISED NOVEMBER 2016**

Made By: JL  
 Checked by: GM  
 Reviewed by: MP  
 Revised by: TS

## SETTLEMENT & GEOMEMBRANE STRAIN ANALYSIS

### 1.0 OBJECTIVE

Estimate the tensile strain caused by differential settlement of the in-situ subsurface materials inferred below the proposed Copper Flat tailing facility.

### 2.0 METHODOLOGY

The proposed geomembrane liner system may experience tensile strain because of differential settlement caused from the loading (tailings and embankment) of the subsurface soils.

#### 2.1 Settlement Analysis

Settlement was calculated using the finite element software SigmaW from the 2012 GeoStudio package. Cross sections A and B (both shown in plan view on Figure 1 and in profile view in Figure 2) showing the proposed tailing facility and tailings embankment layout/dimensions, inferred subsurface soils and boundaries were imported into the software for analyses. Geotechnical properties for each subsurface material layer were selected from previous reports (Refs. 1 and 2) and from experience with similar soils. The geotechnical properties were incorporated into the software and used for the settlement analyses. Figure 3 provides information on the geologic units and geologic structures (Refs. 4, 5 and 6) associated with cross sections A and B along with the the proposed tailing facility and tailings embankment layout/dimensions. Table 1 below provides a list of the geotechnical subsurface material layers and properties.

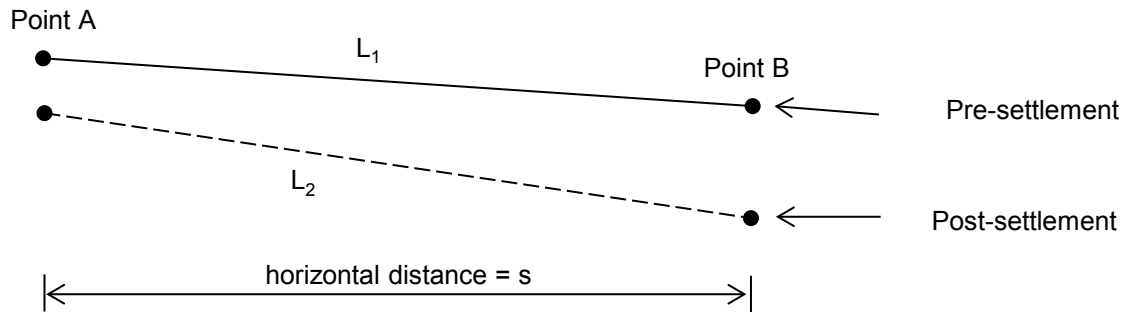
**Table 1: Geotechnical Subsurface Material Layers and Properties**

Material No.	Material Name	Geotechnical Properties		
		Unit Weight (lb/ft <sup>3</sup> )	Poisson's Ratio (-)	Effective Modulus (lb/ft <sup>2</sup> )
0	Tailings/Embankment	97	0.49	10,000,000
1	Well-Graded Gravel	110	0.30	4,000,000
2	Well-Graded Sand with Silt and Gravel	110	0.30	4,000,000
3	Conglomerate	130	0.30	5,000,000
4	Basalt	160	0.30	100,000,000
5	Lean Clay, Fat Clay, Silty Clay	104	0.30	790,600
6	Silt	106	0.30	671,400
7	Caliche	125	0.30	100,000,000
8	Bedrock	175	0.30	100,000,000

## 2.1 Tensile Strain from Differential Settlement

Settlement results from the SigmaW runs were used to calculate the induced strain in the geomembrane liner system along Cross Section A and B shown in Figure 1 and Figure 2.

The tensile strain of a base liner system caused by differential settlement can be estimated by the following equation:



**Illustration: Liner Differential Settlement**

$$\varepsilon = \frac{L_2 - L_1}{L_1}$$

$$L = \sqrt{(Elev.A - Elev.B)^2 + s^2}$$

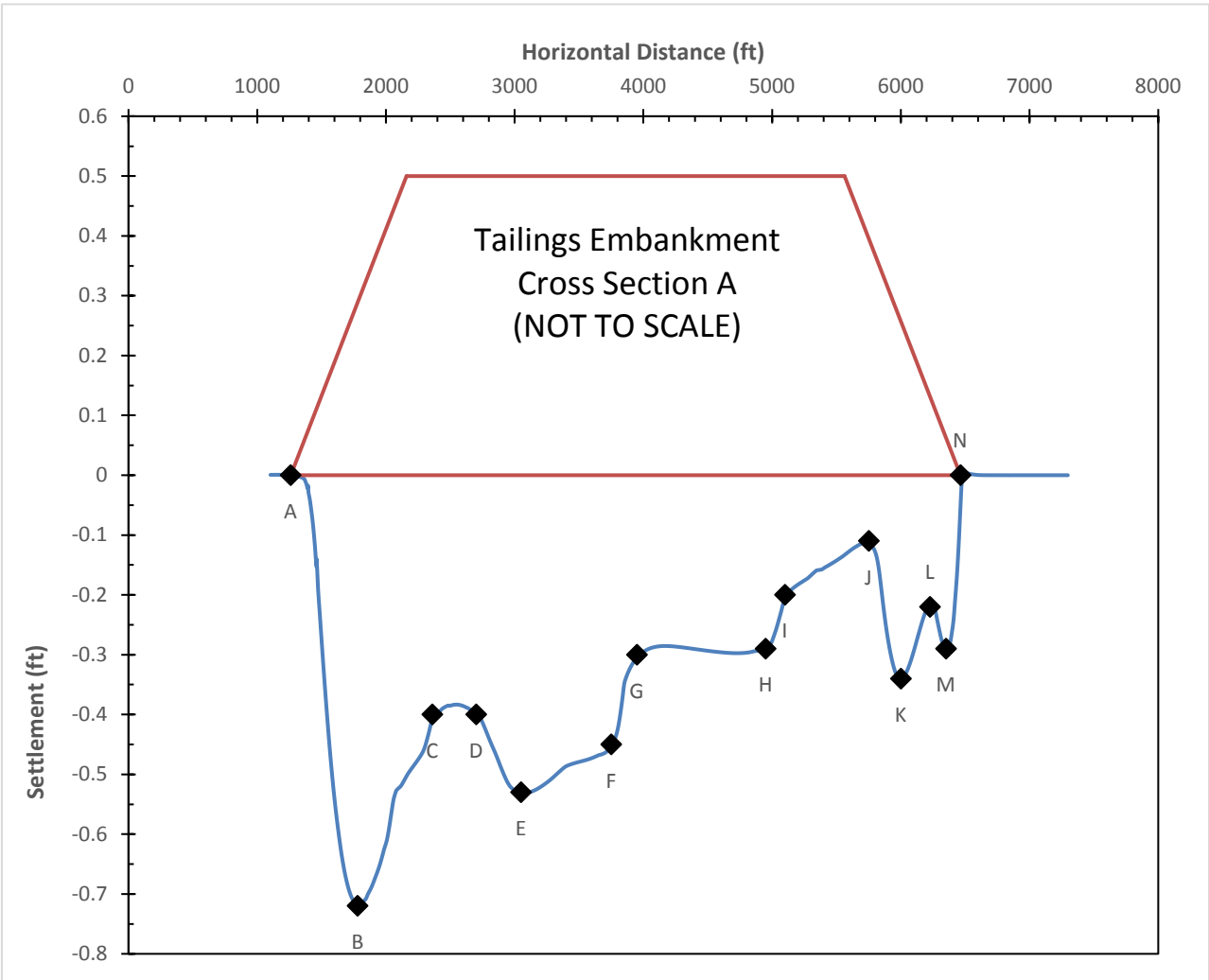
Where:

- e** = Tensile strain in liner system between Points A and B
- $L_1$  = Distance between Points A and B, pre-settlement
- $L_2$  = Distance between Points A and B, post-settlement
- $s$  = Horizontal distance between Points A and B

## 3.0 CALCULATIONS AND RESULTS

### 3.1 Tensile Strain from Differential Settlement

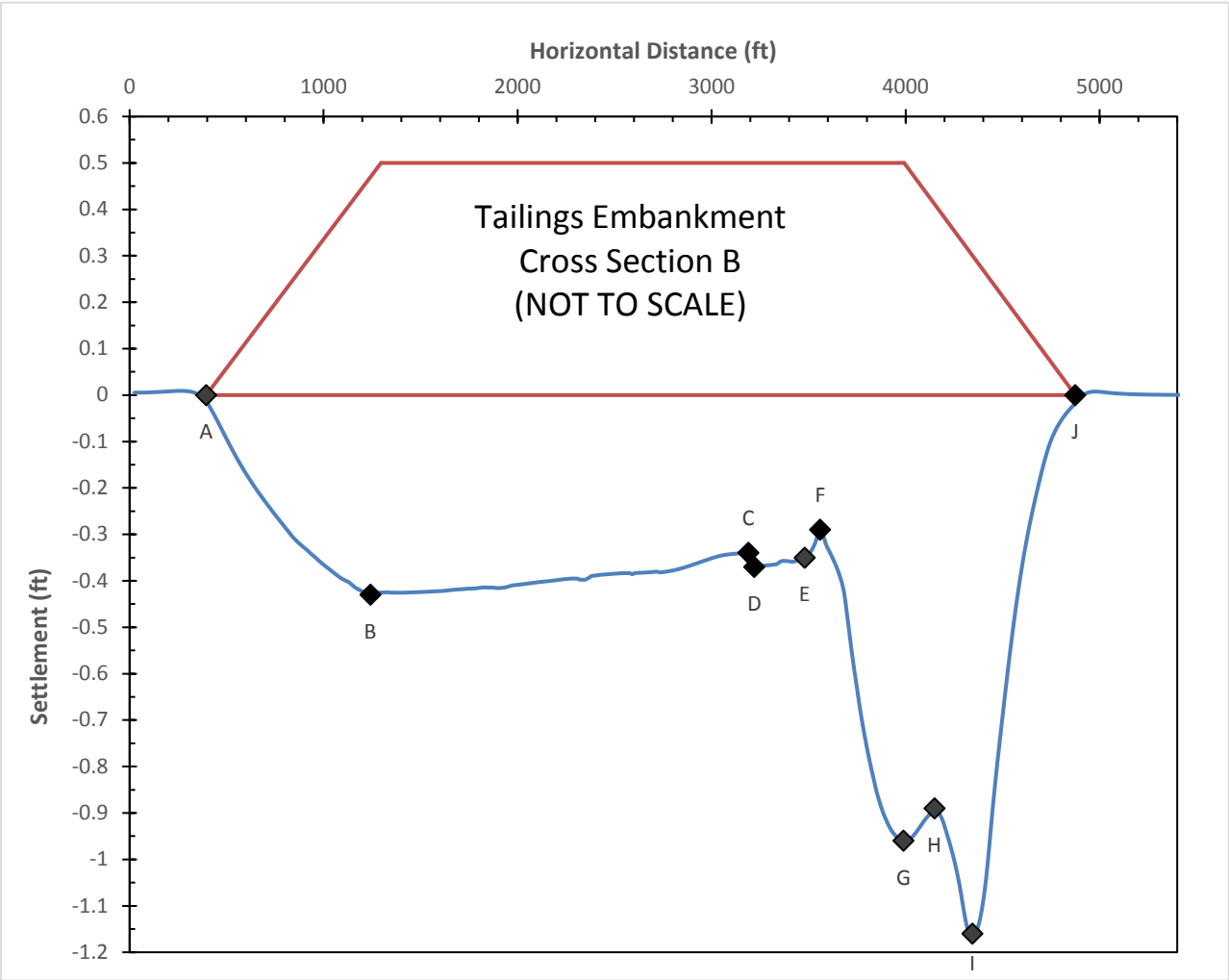
The settlement results for Cross Section A and Cross Section B are illustrated below. Points for liner strain evaluation were selected at locations where peaks or valleys were observed in the results. The liner strain evaluations due to differential settlement of the subsurface materials are summarized in Table 2 and Table 3.



**Illustration: Settlement Profile - Cross Section A**  
(refer to Figure 2 for location along horizontal distance)

**Table 2: Liner Integrity Analysis Results - Cross Section A**

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L <sub>1</sub> ), feet	Post-settlement Dist. (L <sub>2</sub> ), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5383.4	5383.4	0.00	520.00	524.920	525.019	0.0188%
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
B	5311.7	5311.0	0.72	580.00	581.129	581.109	Under Compression
C	5275.5	5275.1	0.40	350.00	350.352	350.358	0.0017%
D	5267.5	5267.1	0.40	350.00	350.352	350.358	0.0017%
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
E	5251.8	5251.3	0.53	700.00	700.339	700.337	Under Compression
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
F	5230.0	5229.6	0.45	200.00	200.000	200.000	0.0001%
G	5230.2	5229.9	0.30	150.00	155.850	155.826	Under Compression
H	5239.6	5239.3	0.29	150.00	155.850	155.826	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
I	5197.3	5197.1	0.20	650.00	650.113	650.111	Under Compression
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
J	5185.2	5185.1	0.11	250.00	250.046	250.051	0.0018%
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
K	5180.4	5180.1	0.34	225.00	225.030	225.028	Under Compression
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
L	5176.7	5176.5	0.22	125.00	125.046	125.048	0.0015%
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
M	5173.3	5173.0	0.29	113.00	113.102	113.090	Under Compression
N	5168.5	5168.5	0.00				
Maximum Tensile Strain due to Differential Settlement =							0.0188%



**Illustration: Settlement Profile - Cross Section B**  
(refer to Figure 2 for location along horizontal distance)

**Table 3: Liner Integrity Analysis Results - Cross Section B**

Points	Elevations		Settlement, feet	Horizontal Distance (s), feet	Pre-settlement Dist. (L <sub>1</sub> ), feet	Post-settlement Dist. (L <sub>2</sub> ), feet	Tensile Strain
	Pre-settlement, feet	Post-settlement, feet					
A	5280.2	5280.2	0.00	847.00	848.707	848.734	0.0032%
B	5226.4	5226.0	0.43				
C	5213.7	5213.4	0.34	30.00	34.000	33.986	Under Compression
D	5229.7	5229.3	0.37				
E	5239.3	5239.0	0.35	79.00	83.716	83.696	Under Compression
F	5211.6	5211.3	0.29				
F	52116.0	52115.7	0.29	430.00	46921.770	46922.440	0.0014%
G	5196.2	5195.2	0.96				
G	5196.2	5195.2	0.96	160.00	160.004	160.004	0.0003%
H	5197.3	5196.4	0.89				
H	5197.3	5196.4	0.89	195.00	195.004	195.003	Under Compression
I	5198.6	5197.4	1.16				
I	5198.6	5197.4	1.16	530.00	530.000	530.003	0.0005%
J	5199.3	5199.3	0.00				
Maximum Tensile Strain due to Differential Settlement =						0.0032%	

#### 4.0 DISCUSSION AND CONCLUSIONS

It is understood that the liner system will consist of HDPE 80 mil geomembrane liner between a liner bedding fill layer and tailings. The minimum allowable tensile strain for geomembrane is 10% (Refs. 3). Based on the analysis performed herein and available information at the time of this calculation, the estimated tensile strain along Cross Section A and Cross Section B are less than the allowable tensile strain. The allowable strain is presented in Table 4.

**Table 4: Summary of Allowable Liner Strains**

Cross Section	Max. Tensile Strain from Differential Settlement	Liner Component	Allowable Tensile Strain	Tensile Strain less than Allowable?
A	0.0188%	Geomembrane	10%	Yes
B	0.0032%			Yes

The potential strain of the geomembrane liner system was analyzed for overall differential settlement along two cross sections (Cross Section A and B) within the proposed Copper Flat tailing facility. Based on the available information, experience with similar subsurface materials and conservative assumptions, the maximum liner strain is estimated to be 0.02%, from differential settlement which is less than the allowable strain for geomembrane liners.



## 5.0 REFERENCES

1. - Golder 2013, Feasibility Study Copper Flat Project, Sierra County, New Mexico, Volume 1 - Tailings Storage Facility, report dated July 2013, Golder Project No. 103-92557.011
2. - SHB (Sergent, Hauskins and Beckwith), 1980. Tailings Dam and Disposal Area - Quintana Minerals Corporation - Copper Flats Project - Golddust, New Mexico. October 14, 1980
3. - Robert M. Koerner (2005) Designing with Geosynthetics, Fifth Edition, Pearson/Prentice Hall.
4. - A.J Jochems and Others (2014) DRAFT Geologic Map of the Hisslboro 7.5-Minute Quadrangle, Sierra County, New Mexico: New Mexico Bureau of Geoloty and Mineral Resources pen-File Geologic Map 242
5. - D.J. Koenig and Others (2015) DRAFT Geologic Map of the Skute Stone Arroyo 7.5-Minute Quadrangle, Sierra County, New Mexico: New Mexico Bureau of Geoloty and Mineral Resources pen-File Geologic Map 252
6. - JSAI (John Shomaker and Associates, Inc.) 2014. Figure 11K-2b of Copper Flat Mine Discharge Permit Application,dated December 2015.

### **GeoStudio File Names**

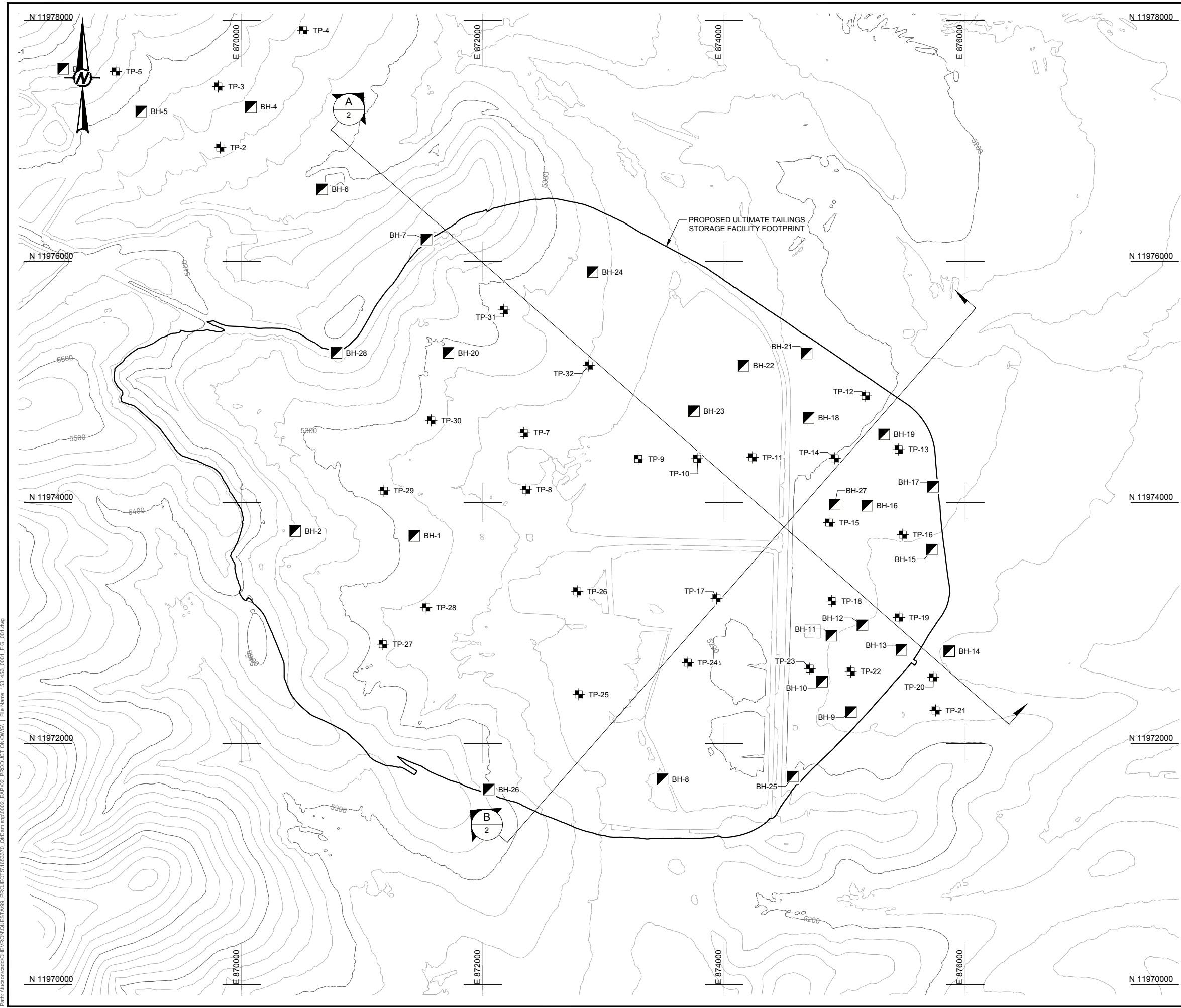
Full Cross Section A.gsz  
Full Cross Section B.gsz

### **Description**

Cross Section A Settlement Analysis  
Cross Section B Settlement Analysis

### **Attachments**

Figure 1: Cross Section Location Plan  
Figure 2: Geologic Cross Sections  
Figure 3: Geologic Cross-Sections Near Tailings Storage Facility



**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- BH-11 HOLLOW STEM AUGER (HSA) BOREHOLE
- TP-8 TEST PIT
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



CLIENT  
**THEMAC** RESOURCES | NEW MEXICO COPPER CORPORATION  
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PROJECT  
**COPPER FLAT**

SIERRA COUNTY, NEW MEXICO

TITLE  
**CROSS-SECTION LOCATION PLAN**

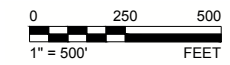
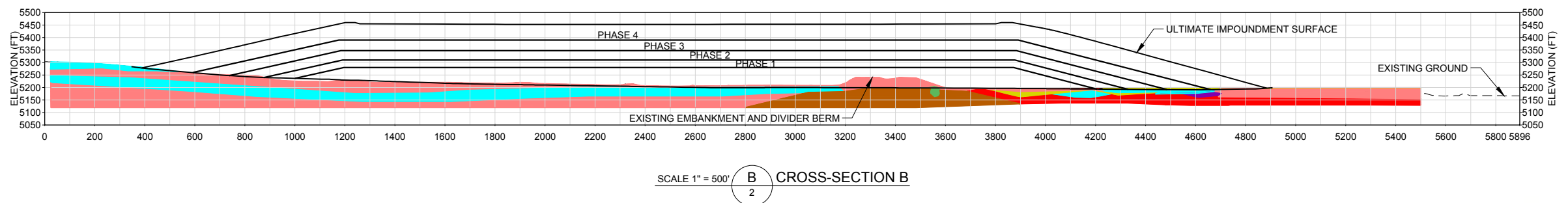
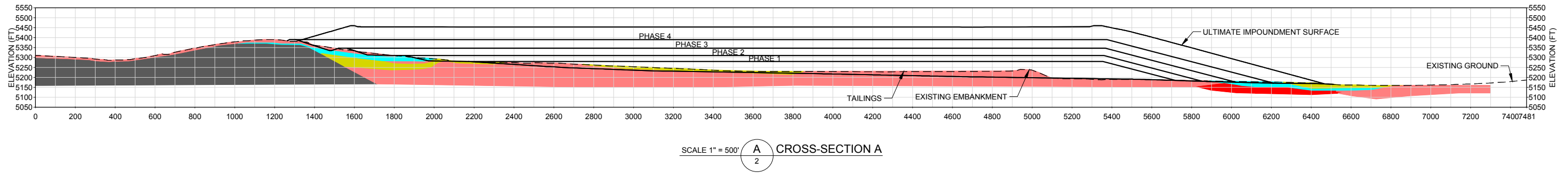
CONSULTANT	YYYY-MM-DD	2016-05-11
	DESIGNED	JS
	PREPARED	JHR
	REVIEWED	JL
	APPROVED	GM

PROJECT NO. 1531453 CONTROL 0001 REV. A FIGURE 1

Path: \\ussas01\usd4\cfe\PROJECTS\1531453\CAD\DWG\002\_EAP\02\_PROD\DRAWING\1 File Name: 1531453\_0001\_FIG\_001.dwg

1 in. IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B

Path: \\nas01\share\1531453\1531453\_NMCopper\Supp\0001\_Sediment2\_PRODUCT\DWG\1 | File Name: 1531453\_0001\_FIG\_002.dwg



**LEGEND**

	WELL-GRADED GRAVEL
	WELL-GRADED SAND WITH SILT AND GRAVEL
	CONGLOMERATE
	BASALT
	LEAN CLAY, FAT CLAY, SILTY CLAY
	SILT
	CALICHE
	BEDROCK

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PROJECT  
**COPPER FLAT**

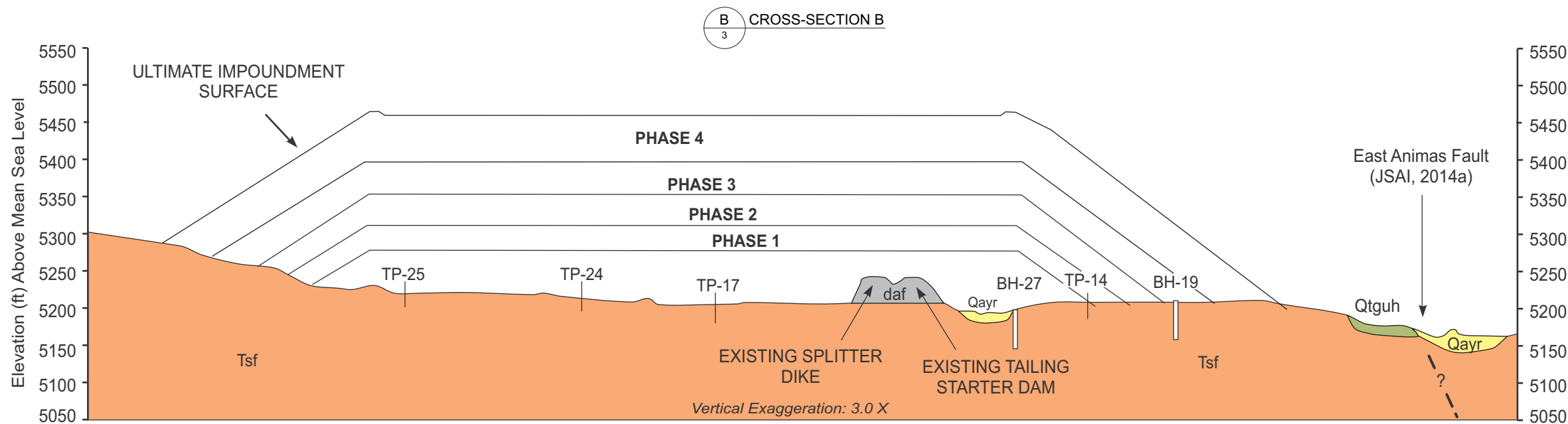
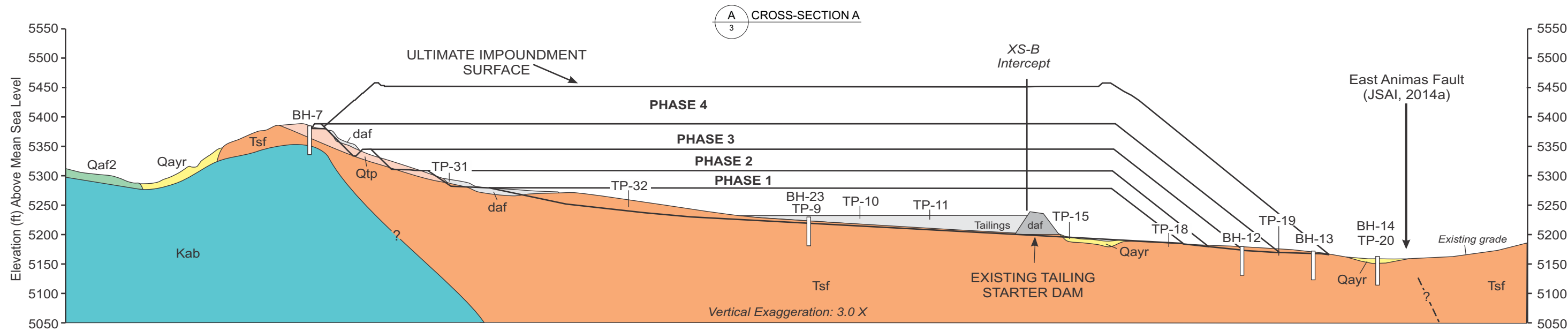
SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTIONS**

CONSULTANT	YYYY-MM-DD	2016-05-11
	DESIGNED	JS
	PREPARED	JHR
	REVIEWED	JL
	APPROVED	GM



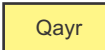

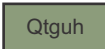

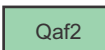
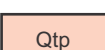
PROJECT NO. 1531453      CONTROL 0001      REV. A      FIGURE 2

1 in. IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B



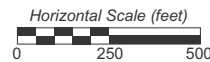
LEGEND

Description of Geologic Units

- |   |   |  |   |
|---|---|--|---|
|  daf   | Historic mining-related disturbed materials, including borrow materials, dredge materials, excavations and fill   |  Tsf | Santa Fe Group predating the Palomas Fm. (Miocene to Pliocene). Pebbly sandstone and conglomerate |
|  Qayr  | Young and recent (modern and historic) alluvium, undivided (present to middle Holocene)   |  Kab | Laharic flow breccia of Copper Flat   |
|  Qtguh | High level terrace deposit associated with smaller canyons, undivided   |      | Inferred fault location   |
|  Qaf2  | Alluvial fans graded to the level of level of Qao2 stream terraces (middle Pleistocene?)  |  |   |
|  Qtp   | Palomas Formation of the Santa Fe Group (lower Pliocene to mid Pleistocene), differentiated from underlying Santa Fe Gp. strata by more cobbles, boulders |  |   |

Sources of data for surficial geologic units:

- (1) New Mexico Bureau of Geology and Mineral Resources  
\*DRAFT Open-File Geologic Map 242 (A.P. Jochems and Others, 2014)  
\*DRAFT Open-File Geologic Map 252 (D.J. Koning and Others, 2015)  
*\*has not been field verified and may not depict site-specific geology with accuracy*
- (2) Updated geologic data from existing test pit and borehole logs
- (3) Regional surface geology and location of East Animas Fault presented on Figure 11K-2b of discharge permit application package, taken from JSAI, 2014a



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PROJECT  
**COPPER FLAT**

SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTIONS NEAR TAILINGS STORAGE FACILITY**

CONSULTANT	YYYY-MM-DD	2016-11-2
	DESIGNED	LCK
	PREPARED	LCK
	REVIEWED	TS
	APPROVED	

PROJECT NO. 1531453 CONTROL 0001 REV. FIGURE 3

**APPENDIX J  
DRAWINGS**



**NEW MEXICO  
COPPER  
CORPORATION**

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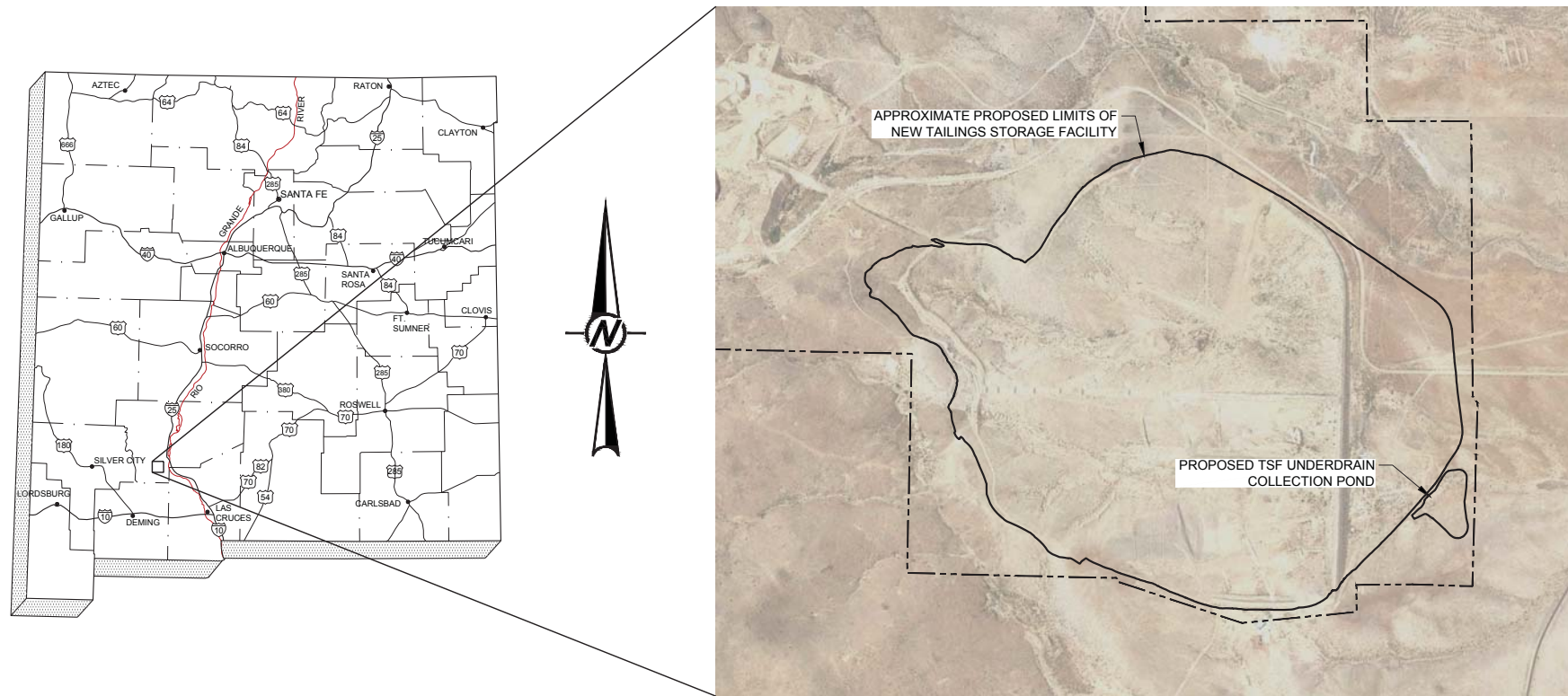
# COPPER FLAT PROJECT

## 30K TPD TAILINGS STORAGE FACILITY

### FEASIBILITY DESIGN

## SIERRA COUNTY, NEW MEXICO

## NOVEMBER 2015



**STATE OF NEW MEXICO**  
NOT TO SCALE

**LOCATION MAP**  
NOT TO SCALE

**DRAWING LIST**

No.	DRAWING TITLE
1	TITLE SHEET AND LOCATION MAP
2	EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN
3	GEOTECHNICAL TEST AND GEOLOGIC CROSS-SECTION LOCATIONS
4	GEOLOGIC CROSS-SECTION A-A'
5	GEOLOGIC CROSS-SECTION B-B'
6	GEOLOGIC CROSS-SECTION C-C'
7	GEOLOGIC CROSS-SECTION D-D'
8	GEOLOGIC CROSS-SECTION E-E'
9	GEOLOGIC CROSS-SECTION F-F'
10	TAILINGS STORAGE FACILITY GRADING PLAN
11	TAILINGS STORAGE FACILITY PHASE 1 PLAN
12	TAILINGS STORAGE FACILITY AT FINAL BUILDOUT
13	TAILINGS STORAGE FACILITY CROSS-SECTIONS
14	TAILINGS STORAGE FACILITY DETAILS
15	DAM AND IMPOUNDMENT UNDERDRAIN PLAN
16	DAM UNDERDRAIN DETAILS (1 OF 2)
17	DAM UNDERDRAIN DETAILS (2 OF 2)
18	TSF UNDERDRAIN COLLECTION POND PLAN
19	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)
20	TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(2 OF 2)
21	CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN
22	SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS
23	GENERAL PROCESS FLOW DIAGRAM
24	TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN
25	TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)
26	TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2)
27	WATER RECLAIM SYSTEM PIPING PLAN
28	WATER RECLAIM SYSTEM DETAILS
29	TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS

**GENERAL REFERENCES**

- 2-FOOT TOPOGRAPHY DEVELOPED BY COOPER AERIAL SURVEY CO. BASED ON JUNE 18, 2011 AERIAL SURVEY AND PROVIDED BY THEMAC RESOURCES.
- COORDINATE SYSTEM IS IN UTM ZONE 13 NAD83 U.S. FOOT.

**NOTES**

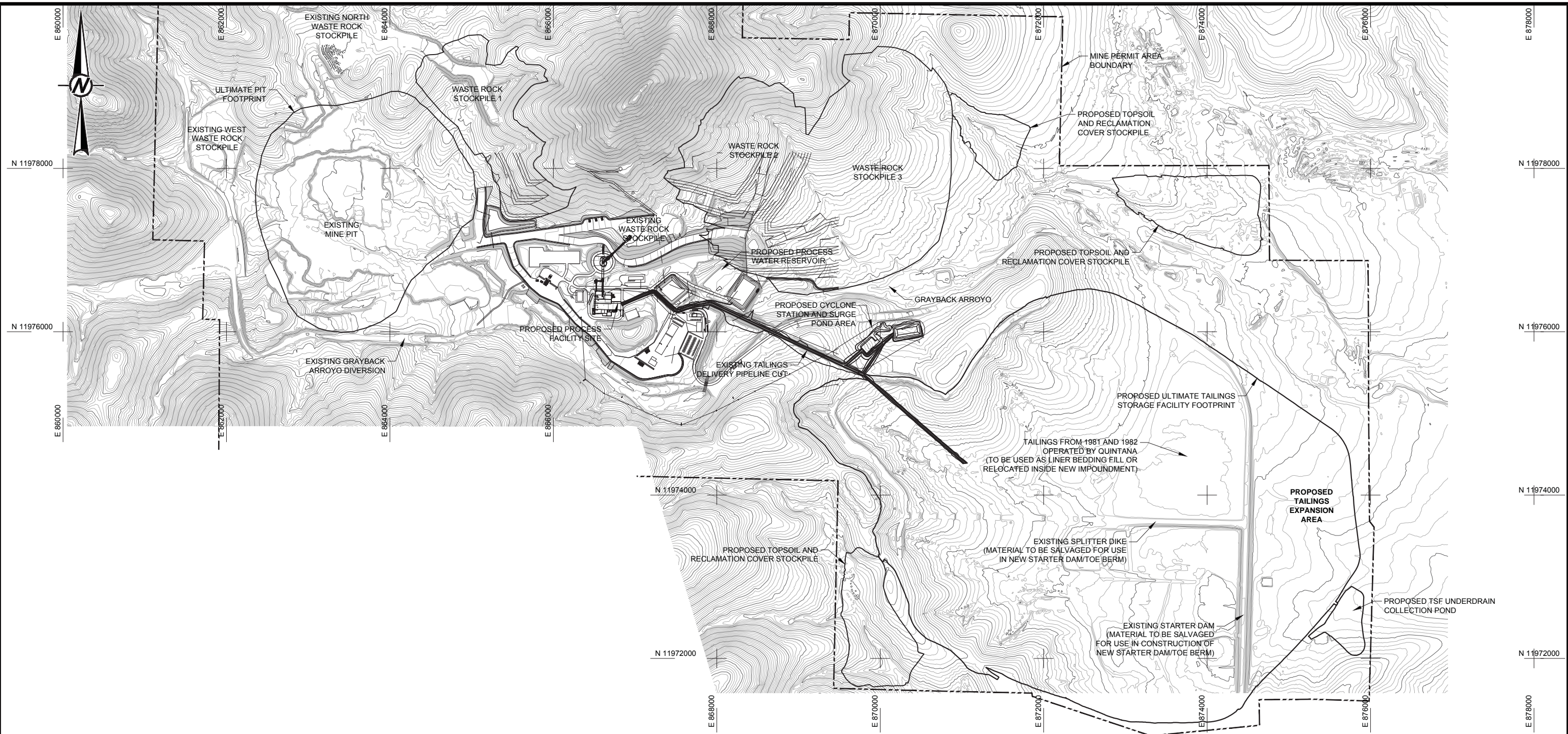
- DRAWINGS TO BE READ IN CONJUNCTION WITH FEASIBILITY LEVEL DESIGN, 30,000 TPD TAILINGS STORAGE FACILITY, SIERRA COUNTY, NEW MEXICO.

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

<b>DRAWING USE</b>	<b>PRELIMINARY FOR AGENCY REVIEW</b>		
<b>TITLE</b>	<b>COPPER FLAT PROJECT</b> <b>30K TPD TAILINGS STORAGE FACILITY</b> <b>FEASIBILITY DESIGN</b> <b>SIERRA COUNTY, NEW MEXICO</b>		
<b>PROJECT</b>	103-92557	<b>FILE No.</b>	10392557K001
<b>DESIGN</b>	DW	2013-04-08	<b>SCALE</b> NOT TO SCALE
<b>CADD</b>	JHR	2013-07-10	<b>DRAWING</b>
<b>CHECK</b>	GM	2013-07-16	<b>1</b>
<b>REVIEW</b>	DAK	2013-07-17	



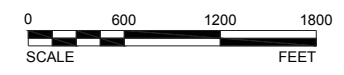
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**NOTES**

1. EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
2. TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
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DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

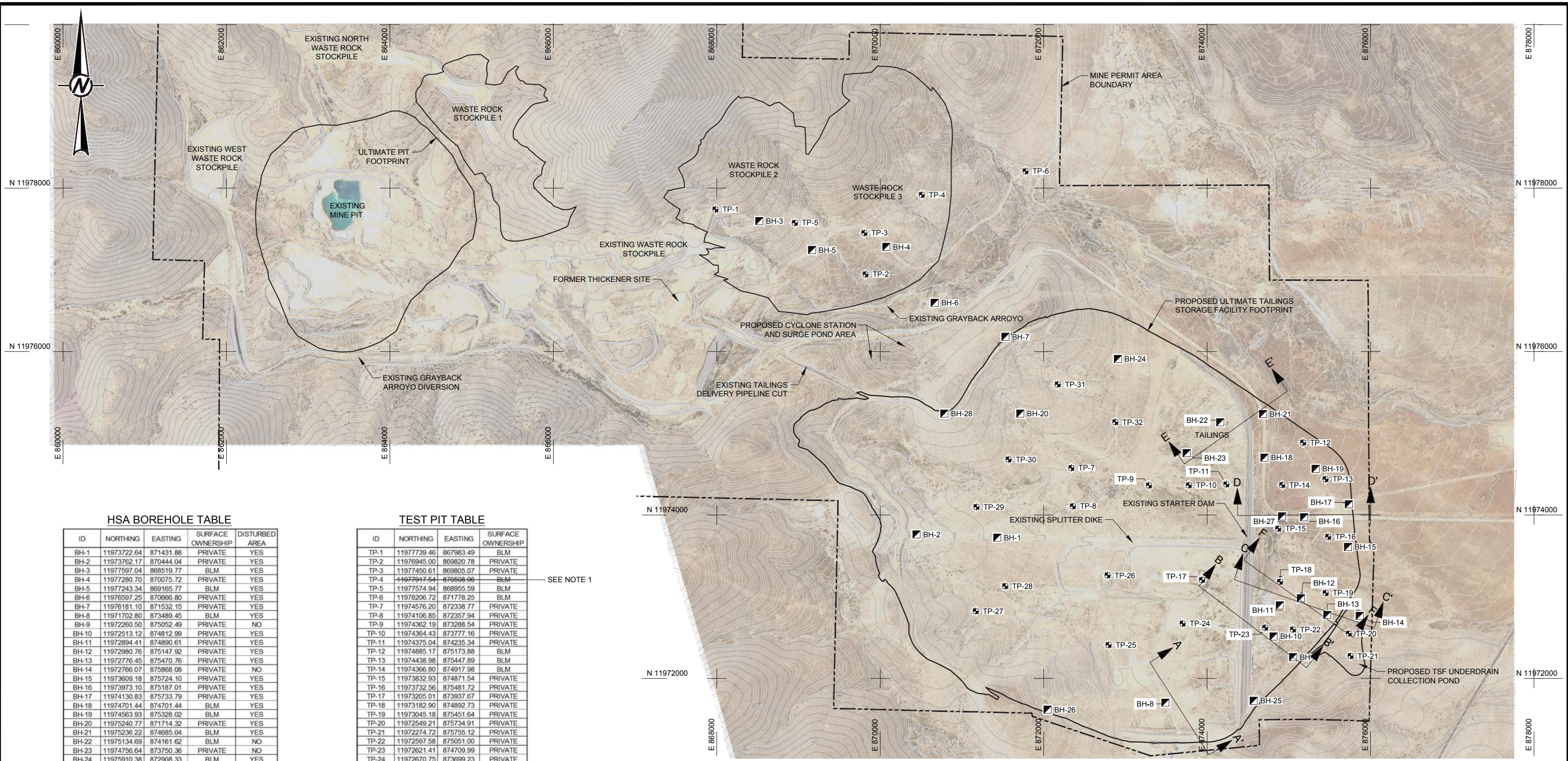
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

PROJECT No. 103-92557 FILE No. 10392557K002  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**EXISTING CONDITIONS AND PROPOSED FACILITIES PLAN**

DESIGN	DW	2013-04-08	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		

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**HSA BOREHOLE TABLE**

ID	NORTHING	EASTING	SURFACE OWNERSHIP	DISTURBED AREA
BH-1	11973722.64	871431.88	PRIVATE	YES
BH-2	11973762.17	870444.04	PRIVATE	YES
BH-3	11977597.04	868519.77	BLM	YES
BH-4	11977280.70	870075.72	PRIVATE	YES
BH-5	11977243.34	869165.77	BLM	YES
BH-6	11976597.25	870666.80	PRIVATE	YES
BH-7	11976181.10	871532.15	PRIVATE	YES
BH-8	11971702.80	873489.45	BLM	YES
BH-9	11972260.50	875052.49	PRIVATE	NO
BH-10	11972513.12	874812.99	PRIVATE	YES
BH-11	11972894.41	874890.61	PRIVATE	YES
BH-12	11972980.76	875147.92	PRIVATE	YES
BH-13	11972776.45	875470.76	PRIVATE	YES
BH-14	11972766.07	875868.08	PRIVATE	NO
BH-15	11973609.18	875724.10	PRIVATE	YES
BH-16	11973973.10	875187.01	PRIVATE	YES
BH-17	11974130.83	875733.79	PRIVATE	YES
BH-18	11974701.44	874701.44	BLM	YES
BH-19	11974563.93	875328.02	BLM	YES
BH-20	11975240.77	871714.32	PRIVATE	YES
BH-21	11975236.22	874685.04	BLM	YES
BH-22	11975134.69	874161.62	BLM	NO
BH-23	11974756.64	873750.36	PRIVATE	NO
BH-24	11975910.38	872908.33	BLM	YES
BH-25	11971725.72	874570.21	BLM	YES
BH-26	11971618.08	872048.45	BLM	YES
BH-27	11973982.94	874917.98	PRIVATE	YES
BH-28	11975241.04	870785.44	PRIVATE	YES

**TEST PIT TABLE**

ID	NORTHING	EASTING	SURFACE OWNERSHIP
TP-1	11977739.46	867983.49	BLM
TP-2	11976945.00	869820.78	PRIVATE
TP-3	11977450.61	869805.07	PRIVATE
TP-4	11977917.64	870568.96	BLM
TP-5	11977574.94	868955.59	BLM
TP-6	11978206.72	871778.25	BLM
TP-7	11974576.20	872338.77	PRIVATE
TP-8	11974106.85	872357.94	PRIVATE
TP-9	11974362.19	873288.54	PRIVATE
TP-10	11974364.43	873777.16	PRIVATE
TP-11	11974375.04	874235.34	PRIVATE
TP-12	11974885.17	875173.88	BLM
TP-13	11974438.98	875447.89	BLM
TP-14	11974366.80	874917.98	BLM
TP-15	11973832.93	874871.54	PRIVATE
TP-16	11973732.56	875481.72	PRIVATE
TP-17	11973205.01	873937.67	PRIVATE
TP-18	11973182.90	874892.73	PRIVATE
TP-19	11973045.18	875451.64	PRIVATE
TP-20	11972549.21	875734.91	PRIVATE
TP-21	11972274.72	875755.12	PRIVATE
TP-22	11972597.58	875051.00	PRIVATE
TP-23	11972621.41	874709.99	PRIVATE
TP-24	11972670.75	873699.23	PRIVATE
TP-25	11972408.78	872794.31	PRIVATE
TP-26	11973262.64	872782.60	PRIVATE
TP-27	11972823.18	871169.18	PRIVATE
TP-28	11973129.48	871528.89	PRIVATE
TP-29	11974098.42	871178.07	PRIVATE
TP-30	11974680.06	871571.56	PRIVATE
TP-31	11975597.72	872172.62	BLM
TP-32	11975136.61	872876.80	BLM

SEE NOTE 1

**LEGEND**

- MINE PERMIT AREA BOUNDARY
- BH-1 HOLLOW STEM AUGER (HSA) BOREHOLE
- ⊕ TP-29 TEST PIT

**NOTES**

- TP-4 TEST PIT NOT EXCAVATED DUE TO LACK OF ACCESS.

**REFERENCES**

- EXISTING FIVE (5) FOOT TOPOGRAPHY AND PERMIT BOUNDARY PROVIDED BY NEW MEXICO COPPER CORPORATION.
- TOPOGRAPHY IN THE MINE AREA AND TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.

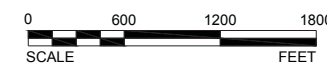
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

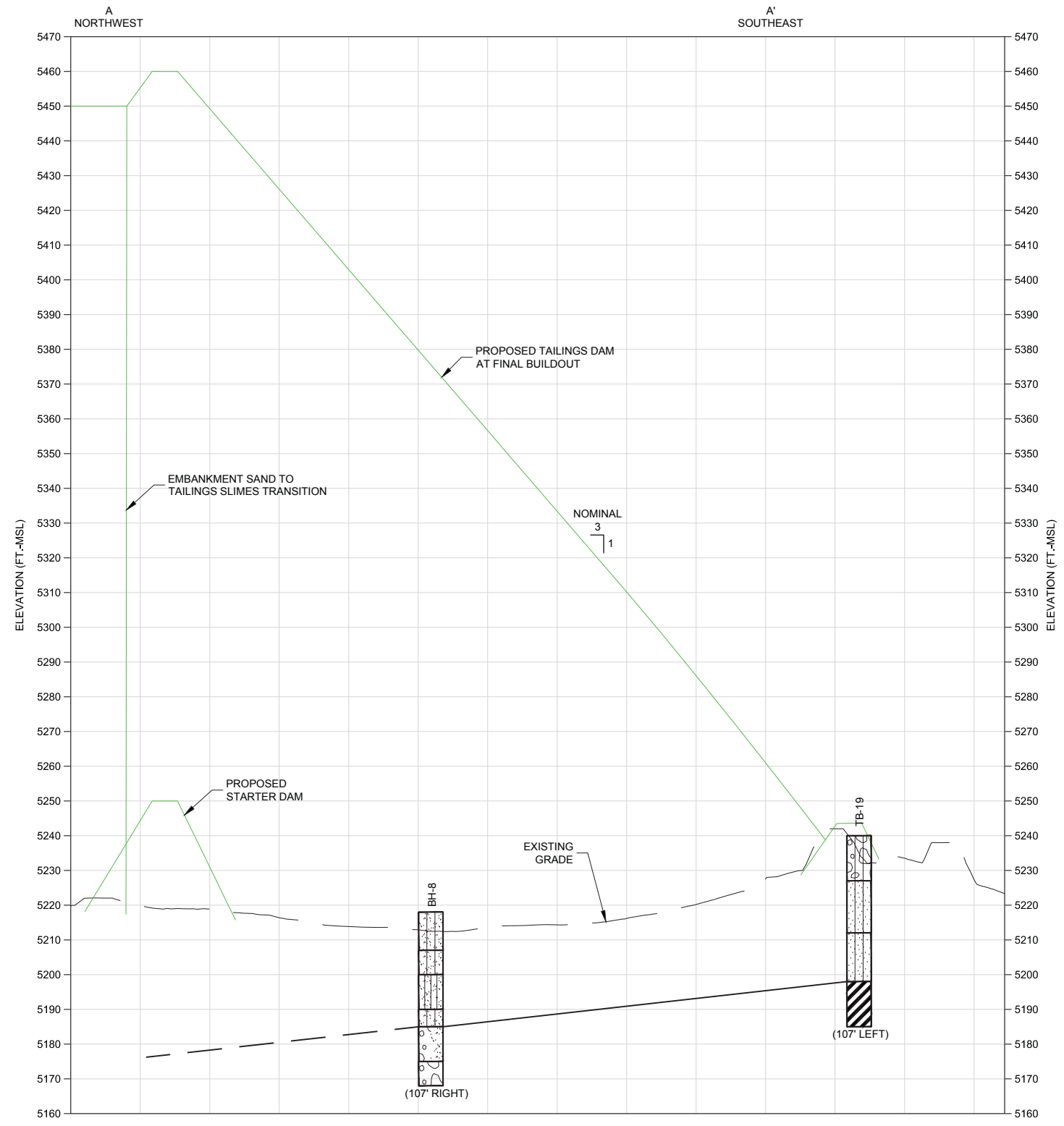
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FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE	PROJECT No.	FILE No.	SCALE
<b>GEOTECHNICAL TEST AND GEOLOGIC CROSS-SECTION LOCATIONS</b>	DESIGN	DW	2013-04-08
	CADD	JHR	2013-07-10
	CHECK	GM	2013-07-16
	REVIEW	DAK	2013-07-17





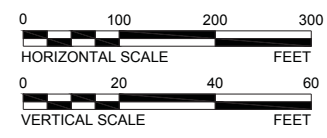
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- NOTES**
- FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  - PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  - PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  - SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

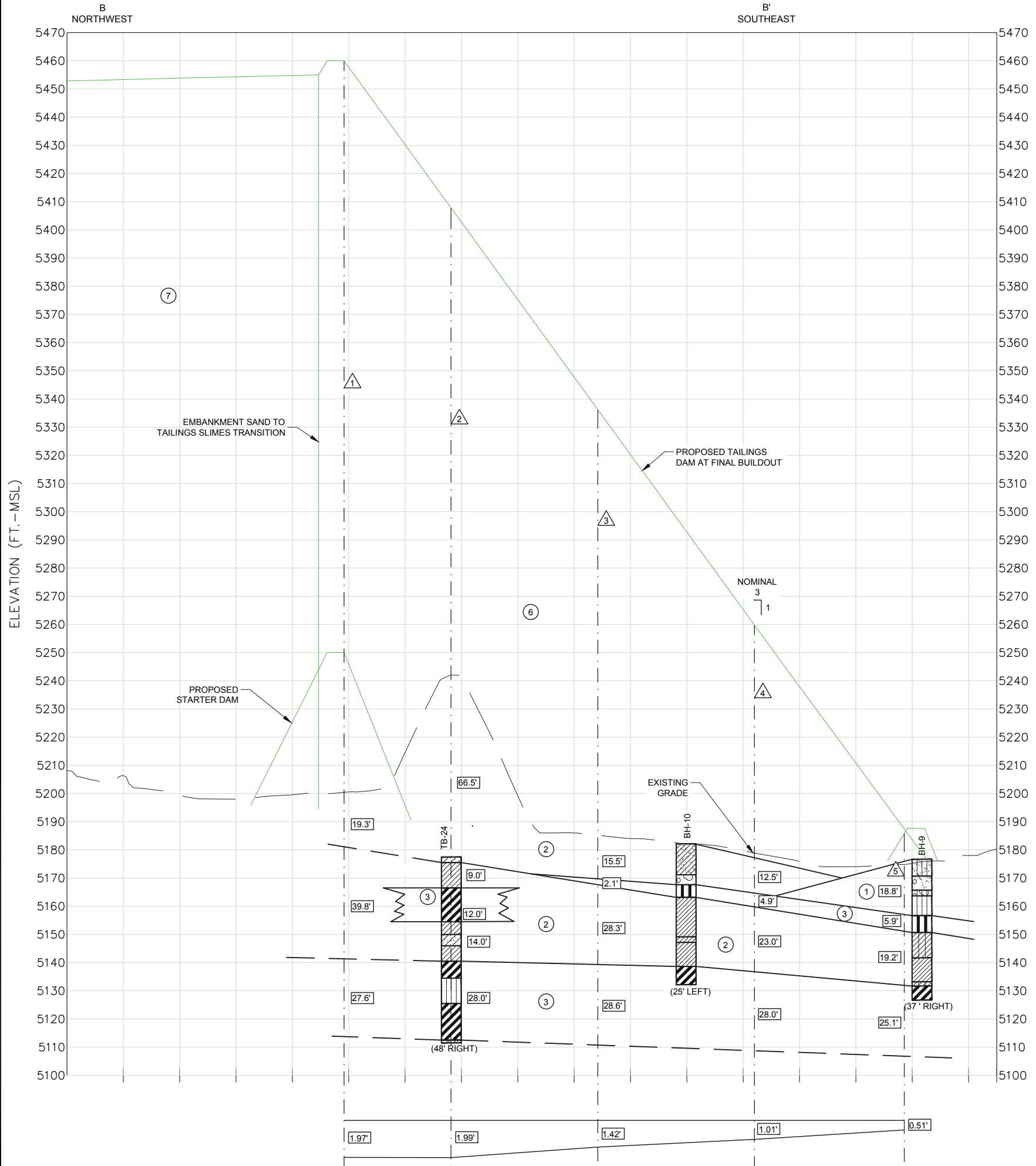
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	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
2013-11-15		ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE <b>PRELIMINARY</b> FOR AGENCY REVIEW	PROJECT <b>THEMAC</b> RESOURCES NEW MEXICO COPPER CORPORATION Environmentally Responsible. Community Minded. Local Opportunities.		COPPER FLAT PROJECT 30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN SIERRA COUNTY, NEW MEXICO	
	TITLE <b>GEOLOGIC CROSS-SECTION A-A'</b>			
PROJECT No. 103-92557		FILE No. 10392557K004		
DESIGN DW	2013-04-08	SCALE	AS SHOWN	
CADD JHR	2013-07-10	DRAWING		
CHECK GM	2013-07-16	<b>4</b>		
REVIEW DAK	2013-07-17			

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**STRATUM DESCRIPTORS**

- ① ML, CL-ML, SM
- ② CL
- ③ CH, MH
- ④ GRAVEL, SAND
- ⑤ BASALT, CALICHE
- ⑥ EMBANKMENT FILL
- ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

**NOTES**

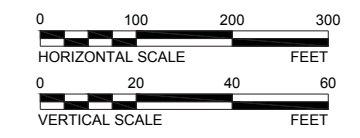
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2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

▼ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

△ SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
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△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
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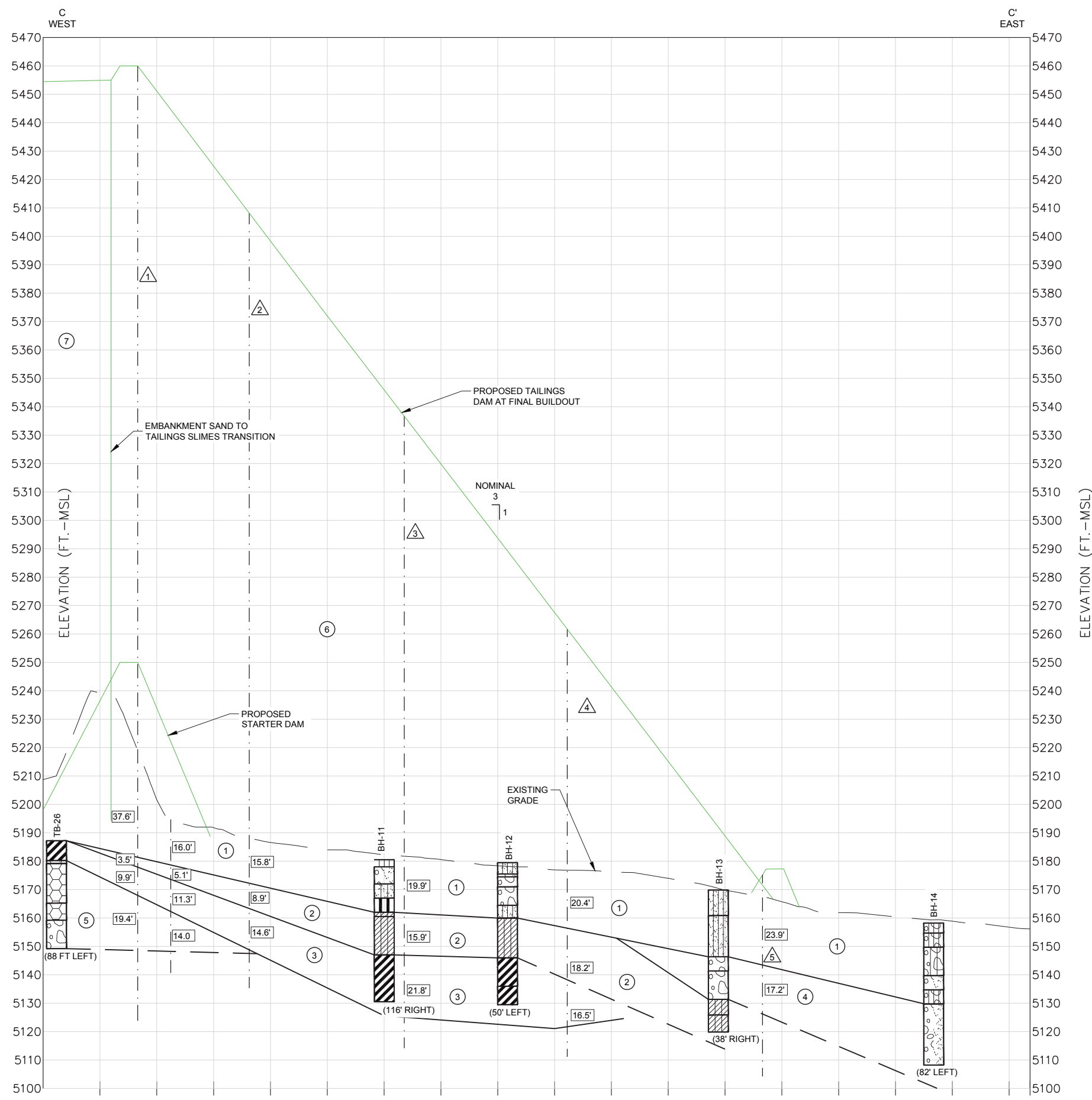
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**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT: COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

TITLE <b>GEOLOGIC CROSS-SECTION B-B'</b>			
PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>5</b>
REVIEW	DAK	2013-07-17	

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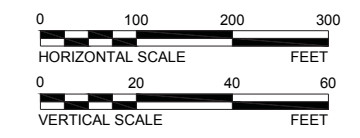
- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
  - 12.0' STRATUM THICKNESS

- NOTES**
1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
  2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)  
 SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
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△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
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DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

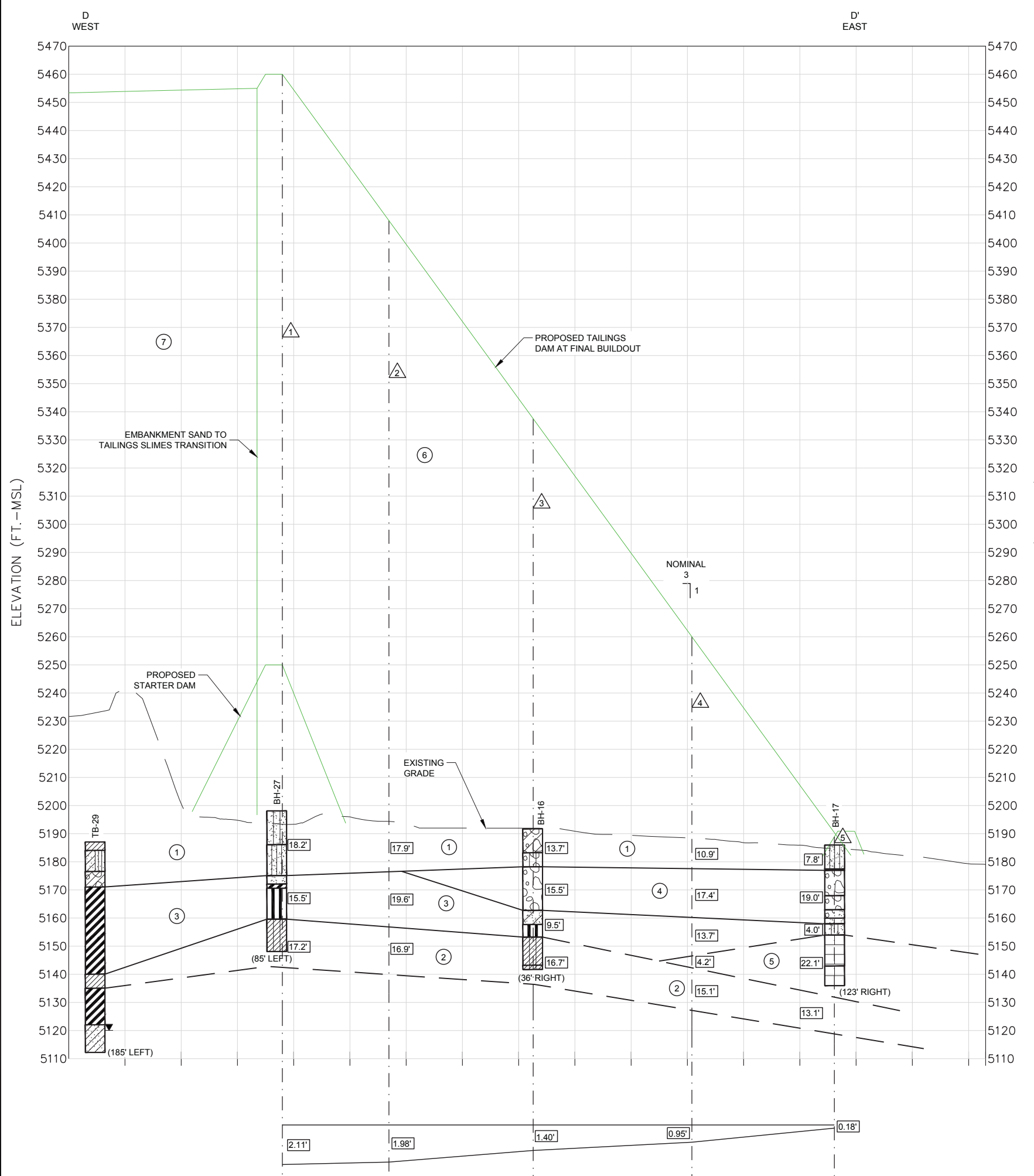
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTION C-C'**

PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	<b>6</b>

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- STRATUM DESCRIPTORS**
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
  - 12.0' STRATUM THICKNESS

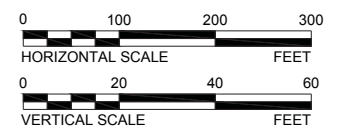
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  2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
  3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
  4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	BASALT		POORLY GRADED SAND WITH SILT		

▼ GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)

△ SETTLEMENT SECTION LINE AND IDENTIFIER



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
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2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

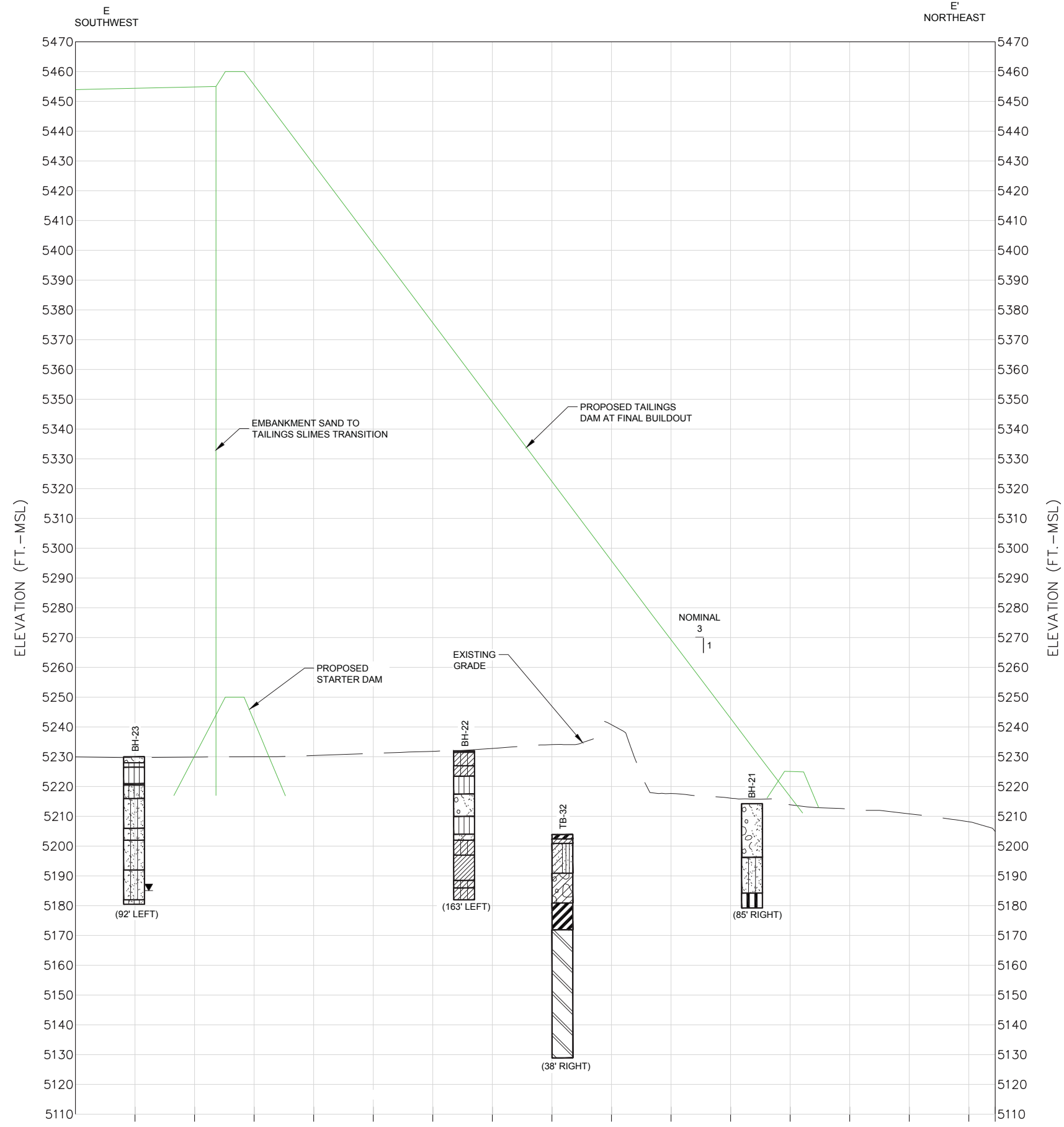
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Resources  
Environmentally Responsible. Community Minded. Local Opportunities.

COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**GEOLOGIC CROSS-SECTION D-D'**

PROJECT No.	103-92557	FILE No.	10392557K004
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	<b>7</b>

K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K004.dwg | Layout: 8 GEOLOGIC CROSS-SECTION E-E | Modified: mcozaco 11/11/2015 1:44 PM | Plotted: Range 11/13/2015

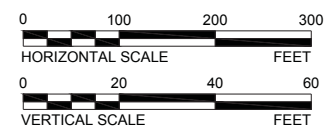


**NOTES**

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
1	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
3	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
4	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
5	2013-05-03	ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

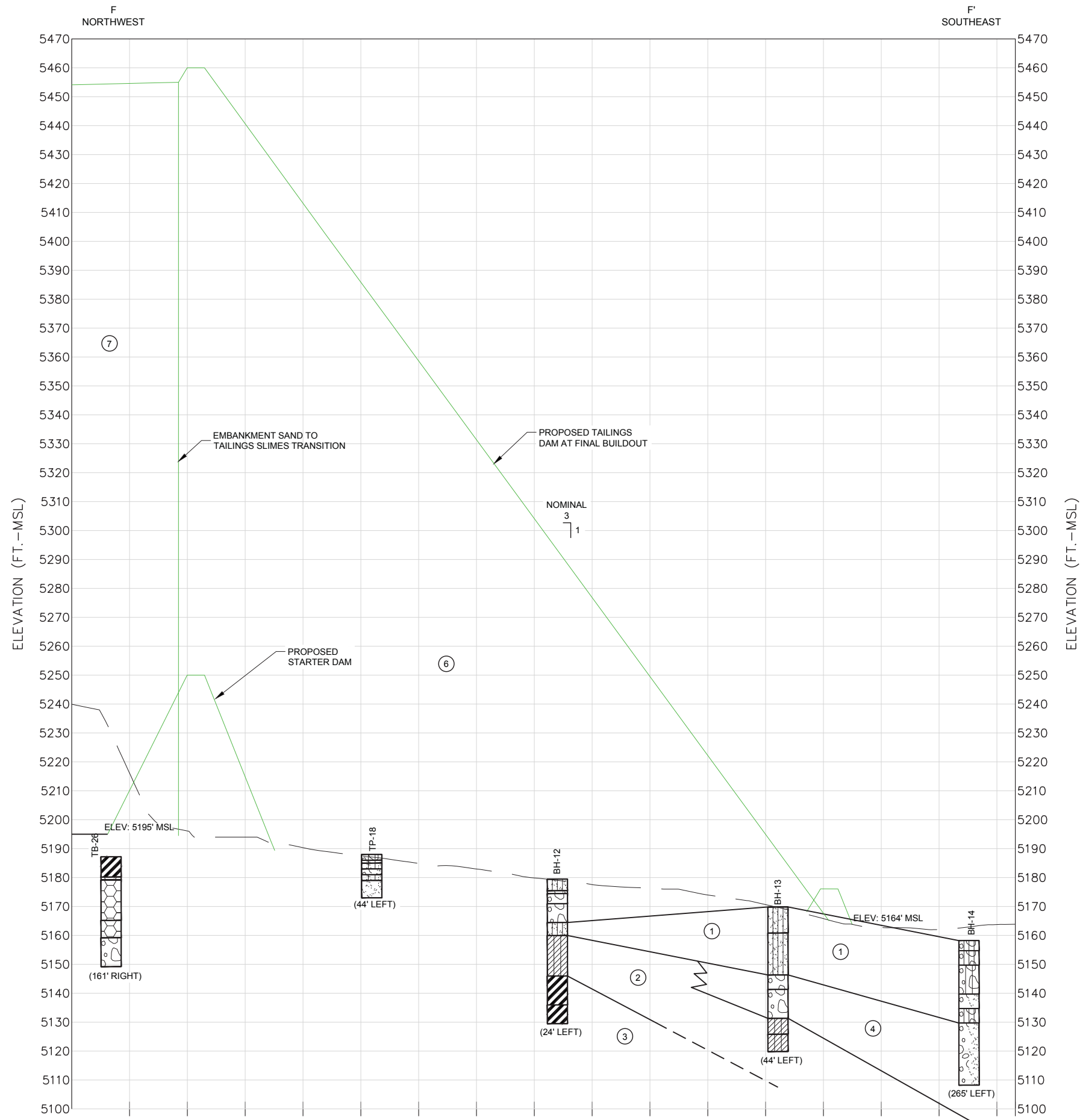
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

**COPPER FLAT PROJECT**  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**GEOLOGIC CROSS-SECTION E-E'**

	PROJECT No.	103-92557	FILE No.	10392557K004	
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
	REVIEW	DAK	2013-07-17		

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**STRATUM DESCRIPTORS**

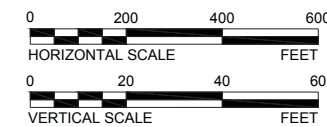
- ① ML, CL-ML, SM
  - ② CL
  - ③ CH, MH
  - ④ GRAVEL, SAND
  - ⑤ BASALT, CALICHE
  - ⑥ EMBANKMENT FILL
  - ⑦ TAILINGS (CONSOLIDATED)
- 12.0' STRATUM THICKNESS

**NOTES**

1. FOR BORINGS WITHOUT STATIC AND/OR INITIAL WATER LEVELS, NO WATER LEVEL OBSERVATIONS WERE MADE AT THE TIME OF THE INVESTIGATIONS.
2. PROFILE OF EXISTING GROUND BASED ON FIVE (5) FOOT TOPOGRAPHY PROVIDED BY NEW MEXICO COPPER CORPORATION.
3. PROFILE IN THE TAILINGS STORAGE FACILITY REPRESENTS EXISTING CONDITIONS AND DISTURBANCE ASSOCIATED WITH QUINTANA 1981-82 MINING OPERATIONS.
4. SEE DRAWING 3 FOR LOCATION OF INDIVIDUAL CROSS-SECTION LINES.

**LEGEND**

	SANDY SILT		USCS LOW PLASTICITY SANDY CLAY		CLAYEY SAND
	SANDY FAT CLAY		CONCRETE		USCS HIGH PLASTICITY CLAY
	SANDY ELASTIC SILT		FILL (MADE GROUND)		USCS LOW PLASTICITY CLAY
	SHALEY FAT CLAY		USCS POORLY-GRADED GRAVEL		SANDSTONE
	SILTSTONE		USCS WELL-GRADED GRAVEL		SANDSTONE AND SHALE
	SAND		USCS WELL-GRADED GRAVEL WITH CLAY		WELL GRADED SAND WITH CLAY
	WELL GRADED SAND		USCS ELASTIC SILT		SHALE
	SILTSTONE AND SHALE INTERBEDDED		SANDSTONE AND SILTSTONE INTERBEDDED		SHALEY ELASTIC SILT
	GROUNDWATER LEVEL AT TIME OF DRILLING (SEE NOTE 1)		BASALT		POORLY GRADED SAND WITH SILT



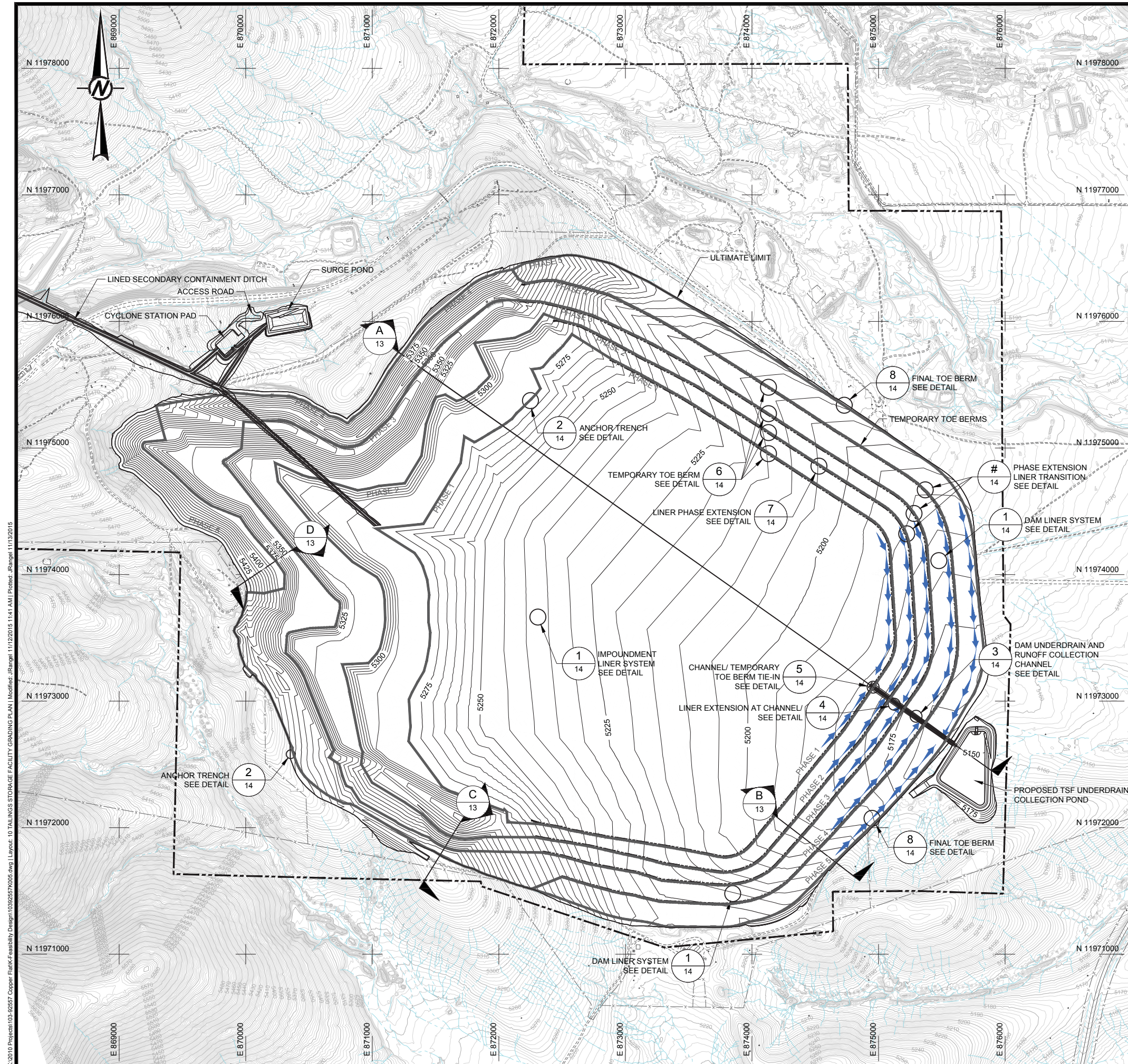
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

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**COPPER FLAT PROJECT**  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

<b>GEOLOGIC CROSS-SECTION F-F'</b>					
PROJECT No.	103-92557	FILE No.	10392557K004		
DESIGN	DW	2013-04-08	SCALE	AS SHOWN	
CADD	JHR	2013-07-10	DRAWING	<b>9</b>	
CHECK	GM	2013-07-16			
REVIEW	DAK	2013-07-17			



**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- DRAINAGE
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
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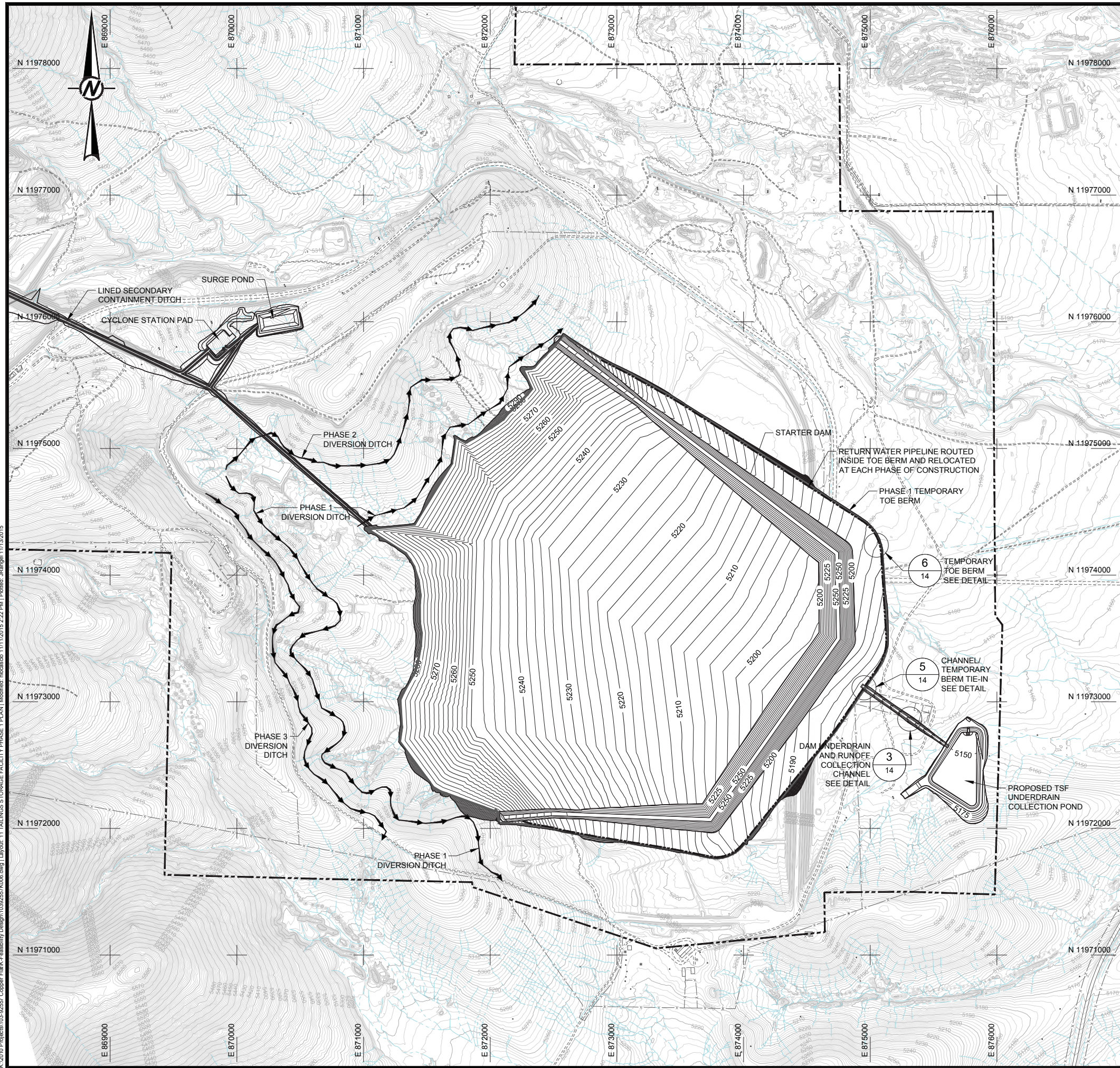
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS STORAGE FACILITY  
GRADING PLAN**

PROJECT No.	103-92557	FILE No.	10392557K005
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>10</b>
REVIEW	DAK	2013-07-17	

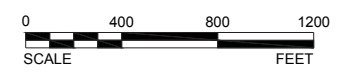
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K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K006.dwg | Layout: 11 TAILINGS STORAGE FACILITY PHASE 1 PLAN | Modified: inccasco 11/11/2015 2:22 PM | Plotter: Range 11/13/2015



**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
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△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
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△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
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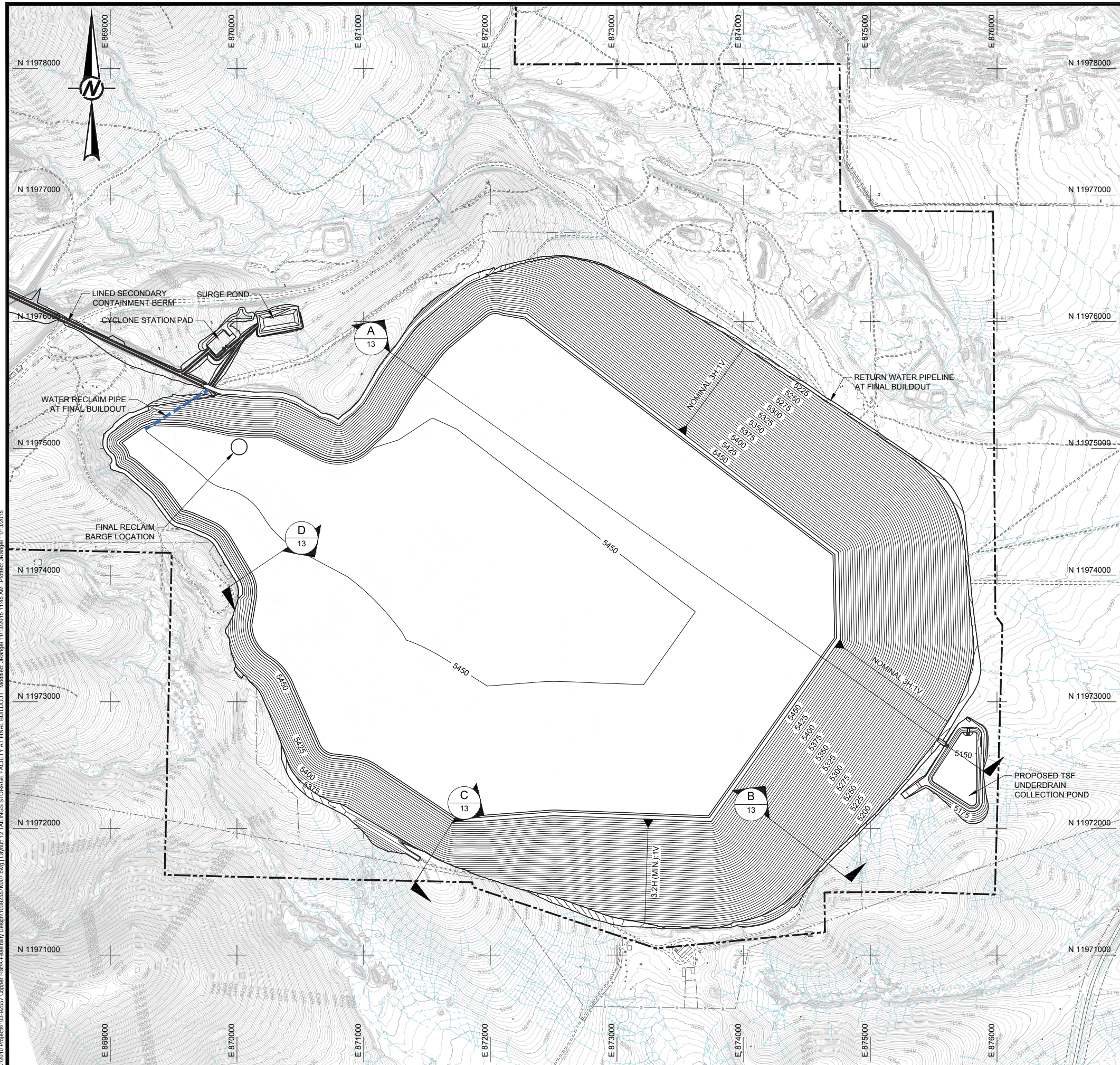
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS STORAGE FACILITY PHASE 1 PLAN**

PROJECT No.	103-92557	FILE No.	10392557K006
DESIGN	DW	2013-04-08	SCALE AS SHOWN
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CHECK	GM	2013-07-16	<b>11</b>
REVIEW	DAK	2013-07-17	

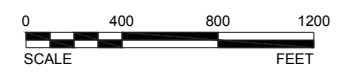


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**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1**  
14  
DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- A**  
13  
CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
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△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
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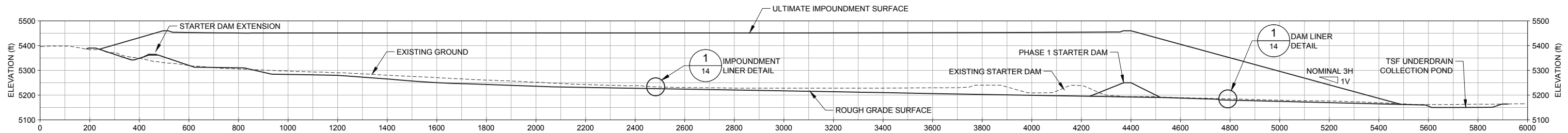
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

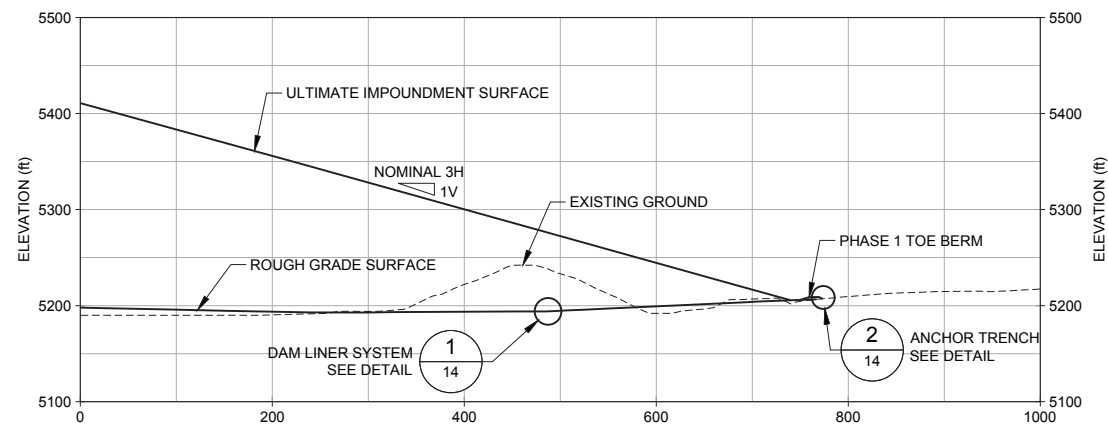
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS STORAGE FACILITY AT FINAL BUILDOUT**

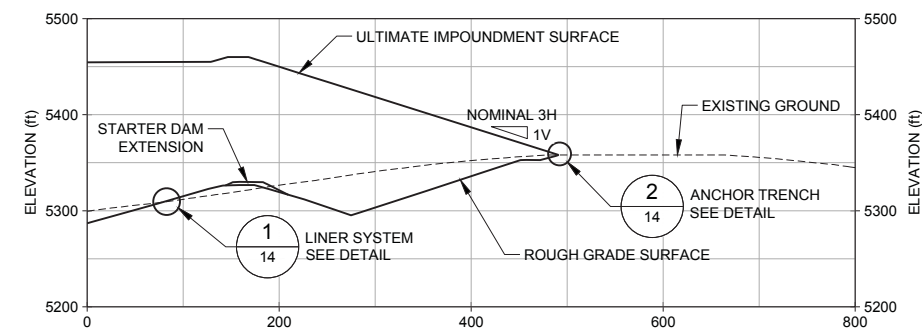
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REVIEW	DAK	2013-07-17	



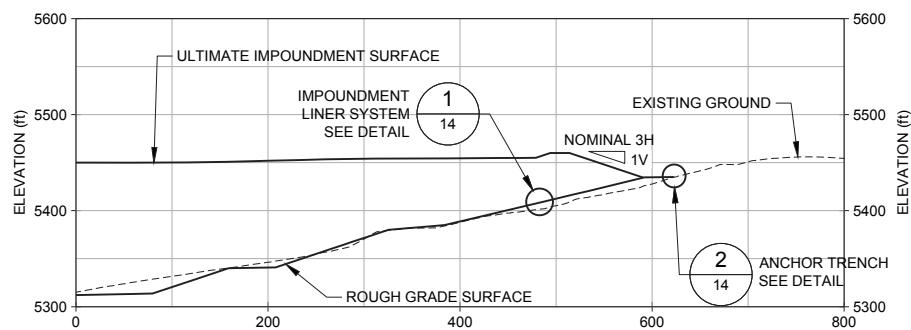
SCALE A **A** CROSS-SECTION A  
13



SCALE B **B** CROSS-SECTION B  
13

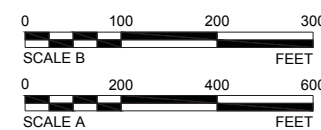


SCALE B **C** CROSS-SECTION C  
13



NOTE: STARTER DAM EXTENSION TO BE SHOWN ON DETAILED DESIGN DRAWINGS.

SCALE B **D** CROSS-SECTION D  
13



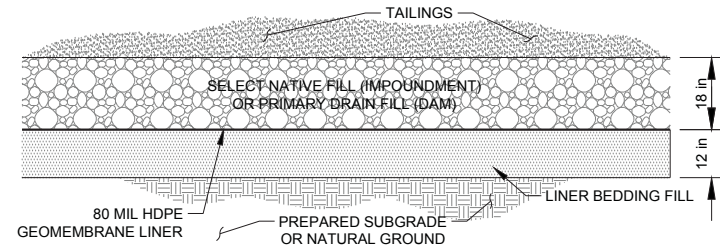
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

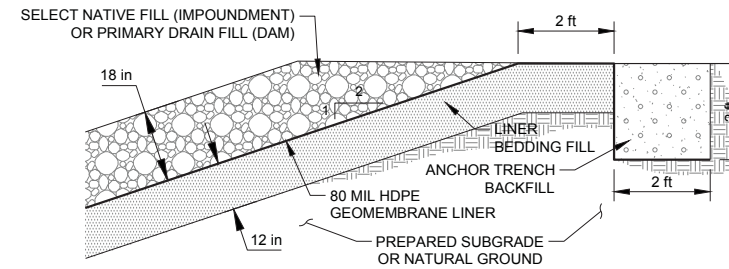
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE			
<b>TAILINGS STORAGE FACILITY CROSS-SECTIONS</b>			
PROJECT No.	103-92557	FILE No.	10392557K008
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>13</b>
REVIEW	DAK	2013-07-17	

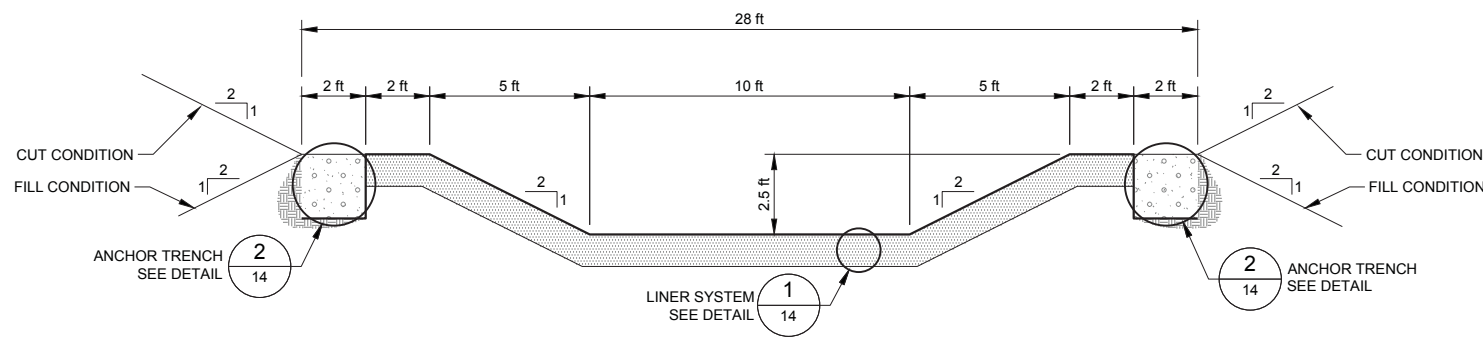




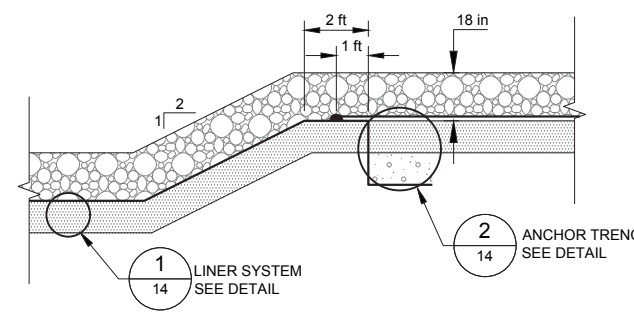
SCALE: N.T.S. 1 IMPOUNDMENT LINER SYSTEM DETAIL



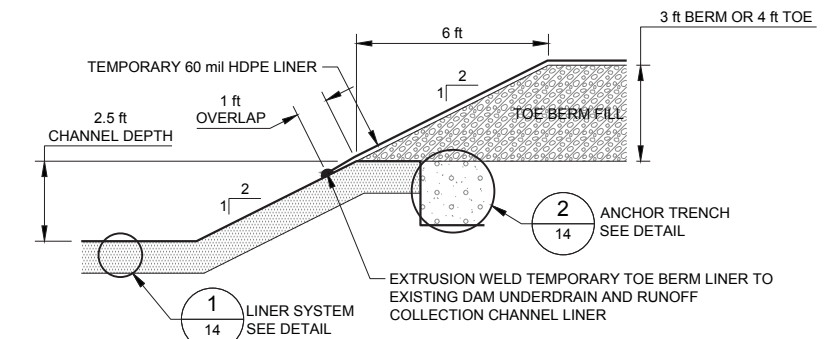
SCALE: N.T.S. 2 ANCHOR TRENCH DETAIL



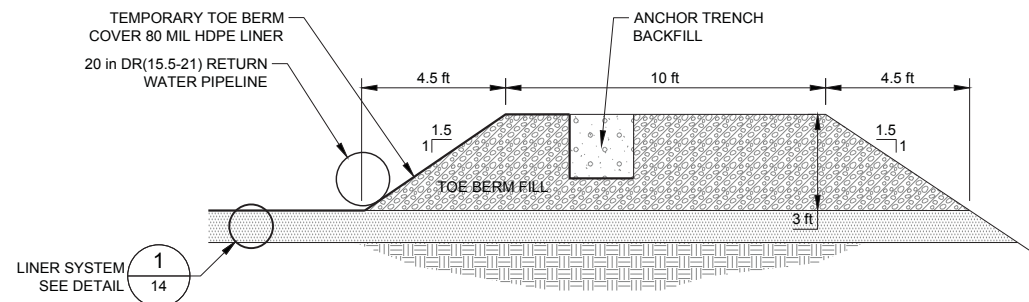
SCALE: N.T.S. 3 DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL DETAIL



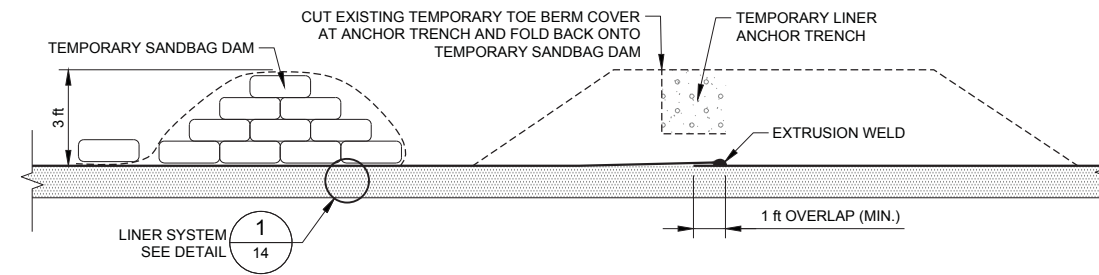
SCALE: N.T.S. 4 LINER EXTENSION WITH CHANNEL ANCHOR DETAIL



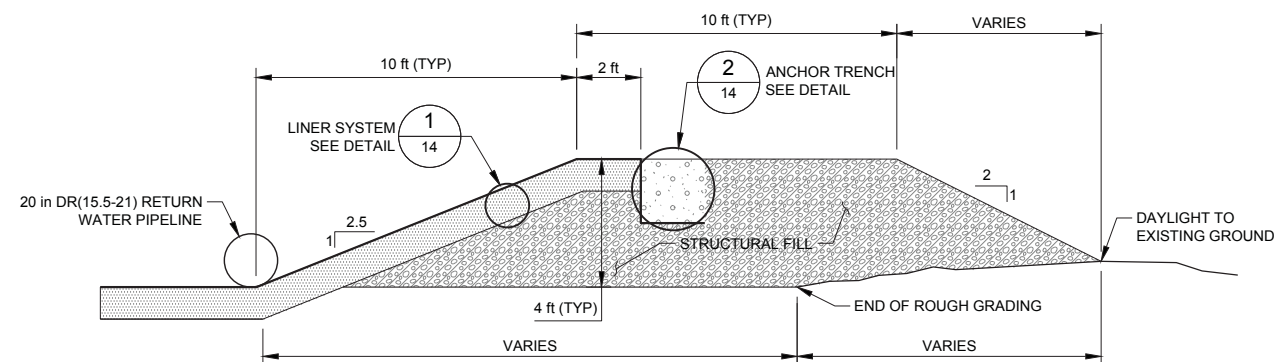
SCALE: N.T.S. 5 CHANNEL/TEMPORARY BERM TIE-IN DETAIL



SCALE N.T.S. 6 TEMPORARY TOE BERM DETAIL



SCALE N.T.S. 7 PHASE EXTENSION LINER TIE-IN AT TOE BERM DETAIL



SCALE: N.T.S. 8 FINAL TOE BERM DETAIL

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

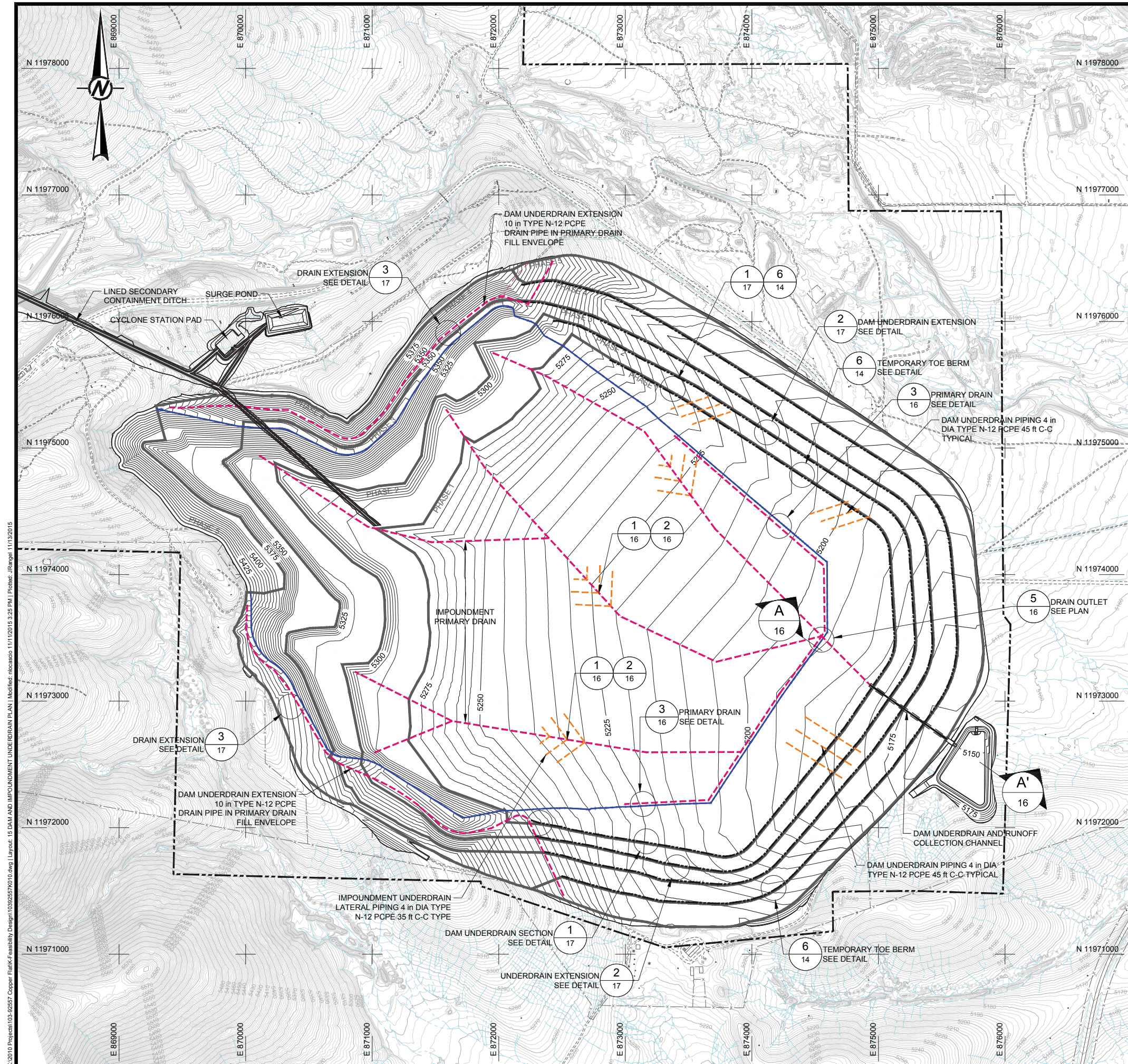
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
RESOURCES  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE					
<b>TAILINGS STORAGE FACILITY DETAILS</b>					
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DESIGN	DW	2013-04-08	SCALE	NOT TO SCALE	
CADD	JHR	2013-07-10	DRAWING		
CHECK	GM	2013-07-16			
REVIEW	DAK	2013-07-17			



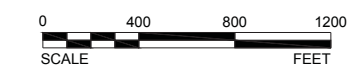
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**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 10 IN. DIA. N-12 PCPE PIPE
- 12 IN. DIA. N-12 PCPE PIPE
- 14 IN SCH80 STEEL
- 4 IN. DIA. N-12 PCPE PIPE
- LIMIT IMPOUNDMENT UNDERDRAIN SYSTEMS
- REGRADED CONTOURS (ft -MSL)
- TEMPORARY TOE BERM CENTERLINE
- PHASE 1 PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or 3H:1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION

- NOTES**
- 1.) TYPICAL ORIENTATION FOR 4 IN UNDERDRAIN PIPING SHOWN. COVERAGE WILL BE AT 35 FT C-C SPACING OVER IMPOUNDMENT UNDERDRAIN AREA. COVERAGE WILL BE 45 FT C-C OVER DAM UNDERDRAIN AREA.
  - 2.) THE MAIN BODY OF THE EMBANKMENT WILL BE UNDERLAIN WITH A BLANKET DRAIN WITH INTERNAL 4 IN PCPE PIPE NETWORK. DAM UNDERDRAIN EXTENSION IN PHASE 2 THROUGH PHASE 5 WILL INCLUDE A 10 IN DIAMETER PCPE DRAIN CONSTRUCTED ON THE OUTER TOE OF THE STARTER DAM



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

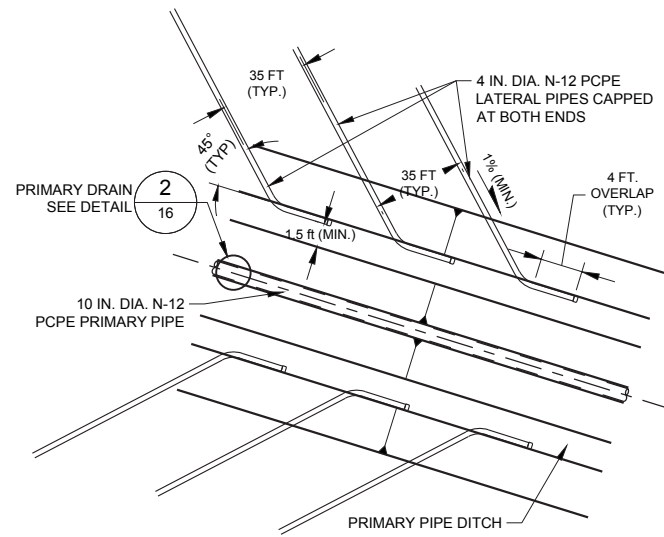
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**DAM AND IMPOUNDMENT UNDERDRAIN PLAN**

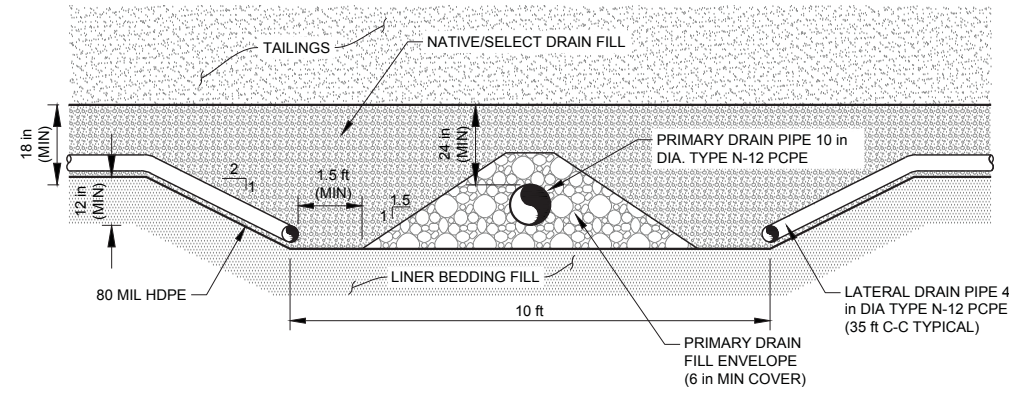
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CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>15</b>
REVIEW	DAK	2013-07-17	



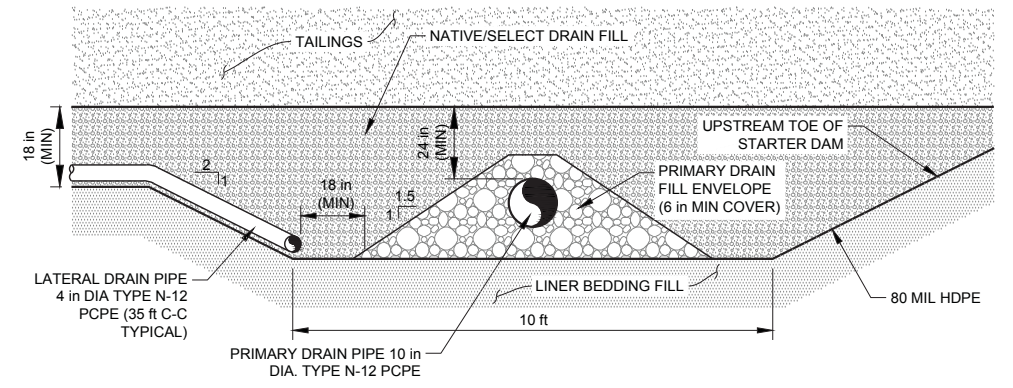
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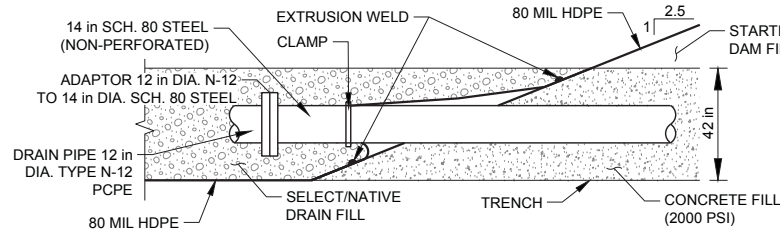
NTS 1 IMPOUNDMENT DRAIN PLACEMENT  
16



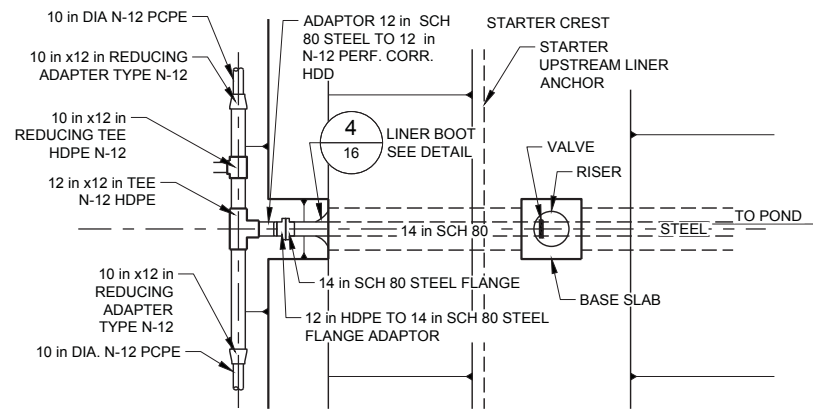
NTS 2 PRIMARY DRAIN PLACEMENT  
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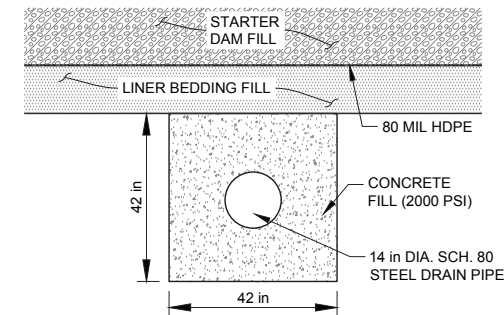
NTS 3 PRIMARY DRAIN PLACEMENT AT STARTER DAM TOE  
16



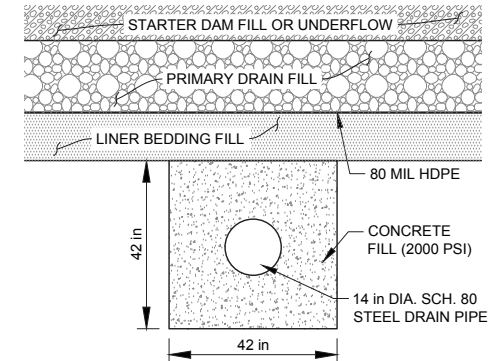
NTS 4 DETAIL DRAIN PIPE TO STARTER DAM ENTRY  
16



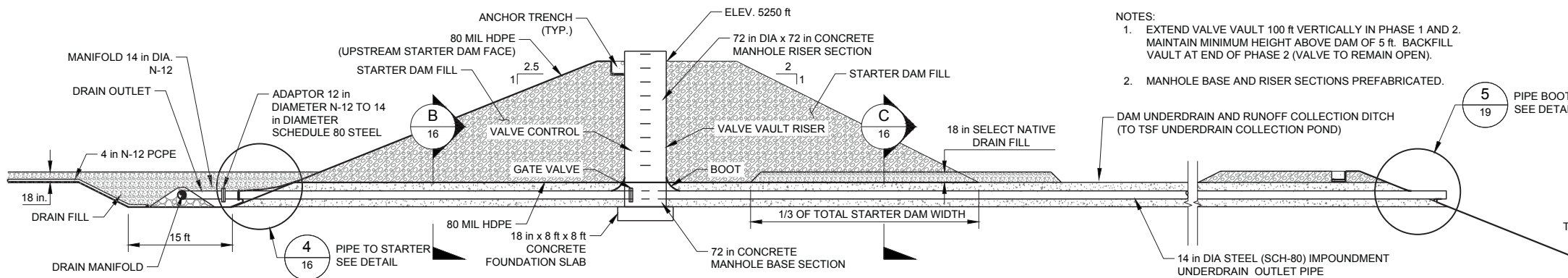
NTS 5 DRAIN OUTLET PLAN  
16



NTS B SECTION B-B  
16



NTS C SECTION C-C  
16



NTS A SECTION A-A DRAIN OUTLET  
16

- NOTES:
1. EXTEND VALVE VAULT 100 ft VERTICALLY IN PHASE 1 AND 2. MAINTAIN MINIMUM HEIGHT ABOVE DAM OF 5 ft. BACKFILL VAULT AT END OF PHASE 2 (VALVE TO REMAIN OPEN).
  2. MANHOLE BASE AND RISER SECTIONS PREFABRICATED.

5 PIPE BOOT SEE DETAIL  
19

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

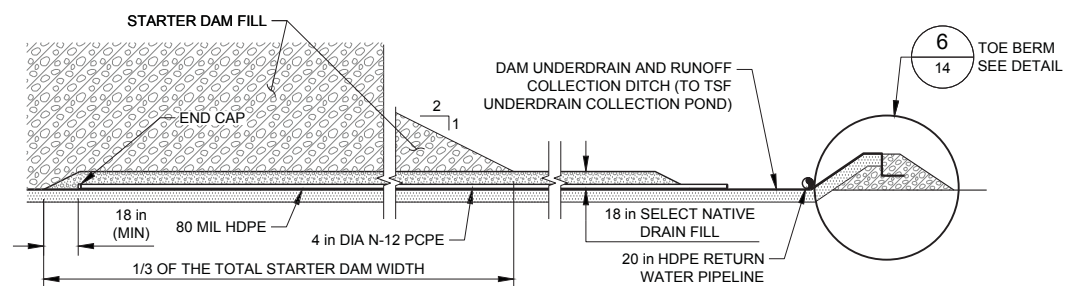
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

**DAM UNDERDRAIN DETAILS (1 of 2)**

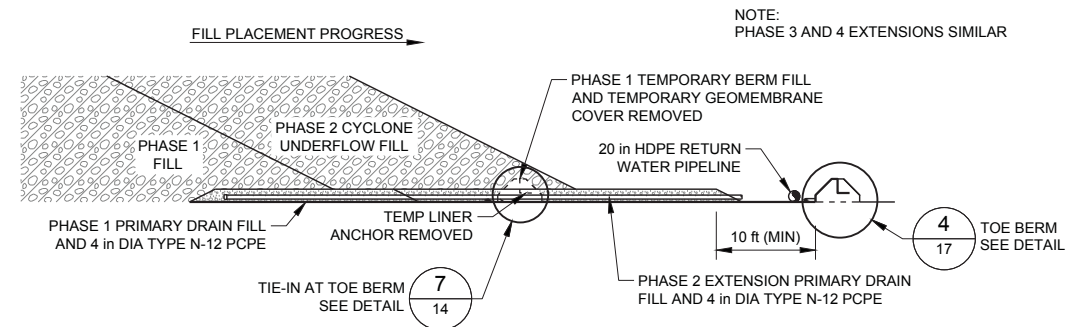
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CADD	JHR 2013-07-10	DRAWING	
CHECK	GM 2013-07-16		
REVIEW	DAK 2013-07-17		





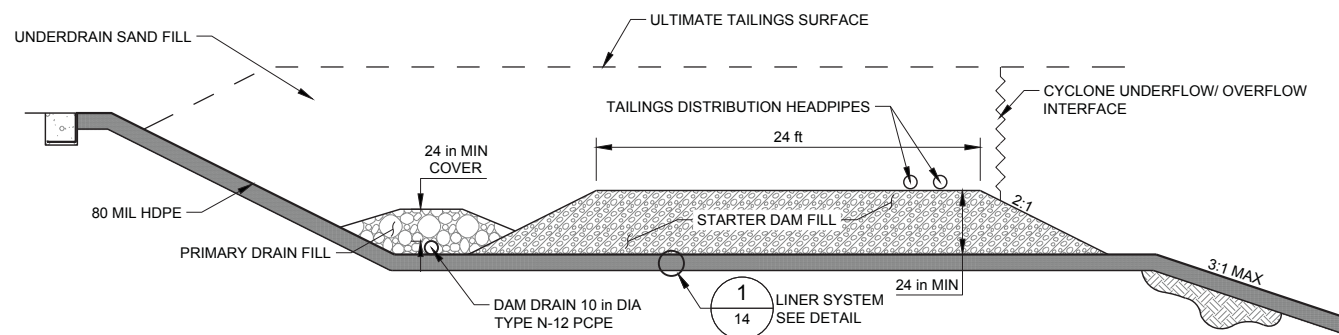
NTS 1 DAM UNDERDRAIN SECTION

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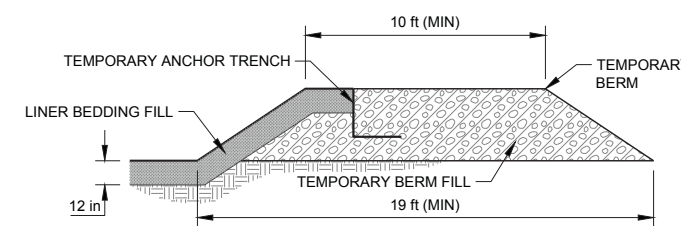
NTS 2 PHASE 1 TO PHASE 2 TSF LINER AND DAM UNDERDRAIN EXTENSION

17



NTS 3 DAM UNDERDRAIN 10 IN PCPE DRAIN EXTENSION

17






NTS 4 TEMPORARY PERIMETER BERM

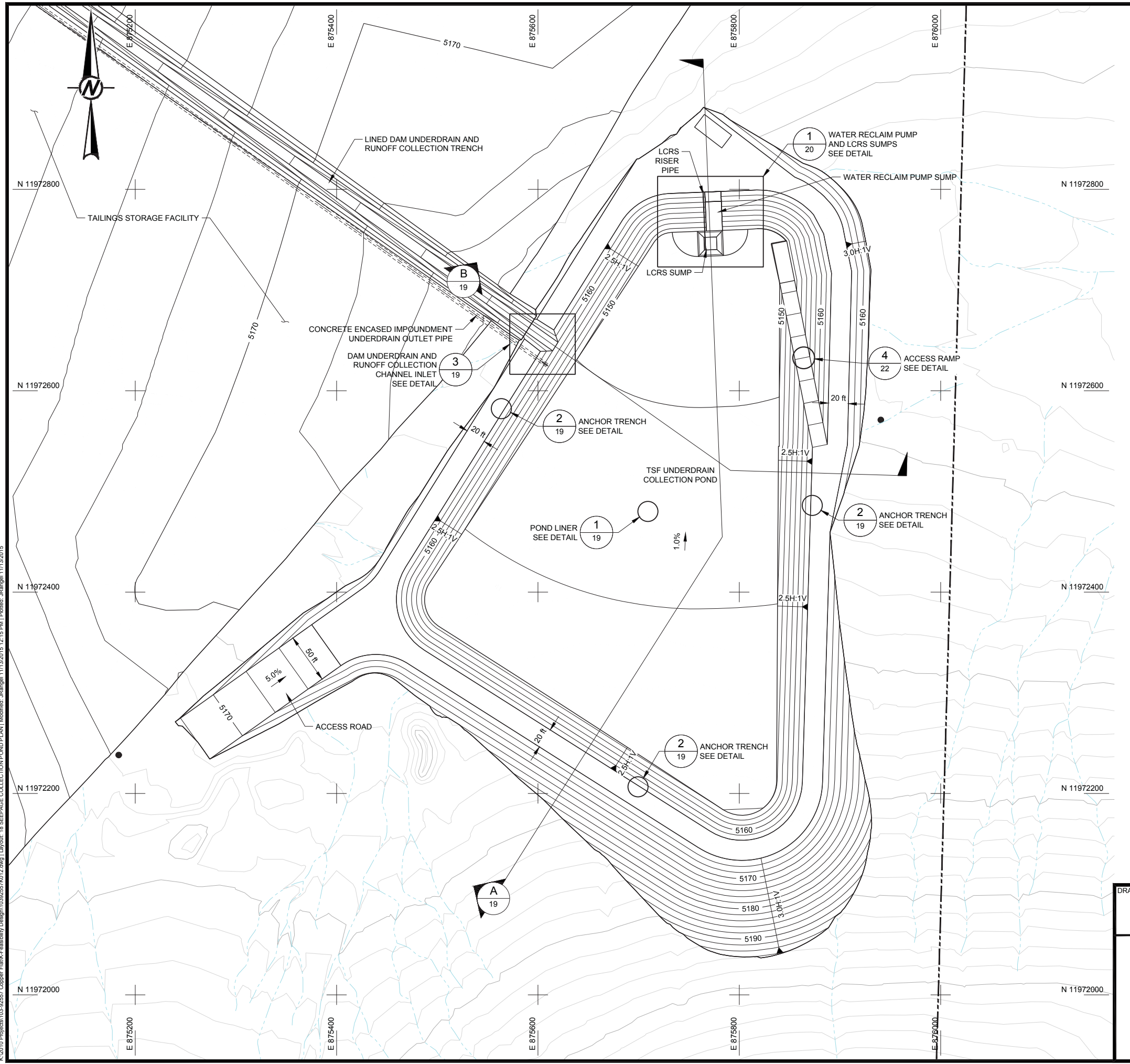
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REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE <b>PRELIMINARY</b> FOR AGENCY REVIEW	PROJECT		COPPER FLAT PROJECT		
	30K TPD TAILINGS STORAGE FACILITY		FEASIBILITY DESIGN		
TITLE		SIERRA COUNTY, NEW MEXICO			
 		PROJECT No.	103-92557	FILE No.	10392557K011
		DESIGN	DW	2013-04-08	SCALE
		CADD	JHR	2013-07-10	DRAWING
		CHECK	GM	2013-07-16	<b>17</b>
REVIEW	DAK	2013-07-17			

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**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3H:1V or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

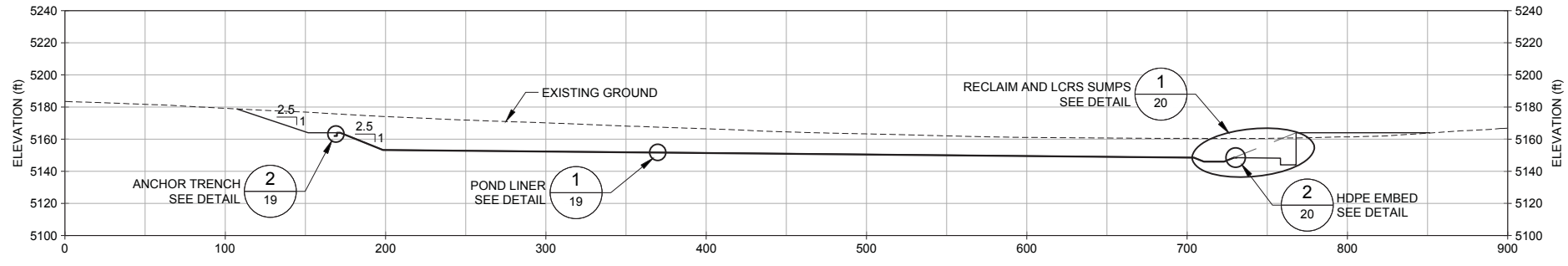
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

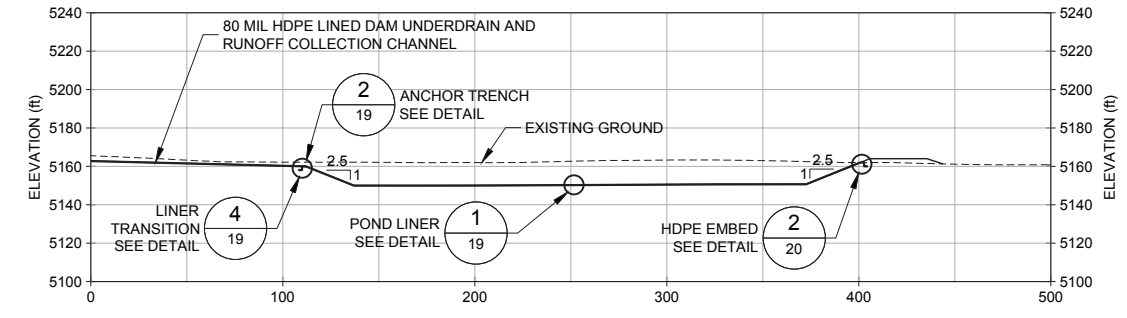
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TSF UNDERDRAIN COLLECTION POND PLAN**

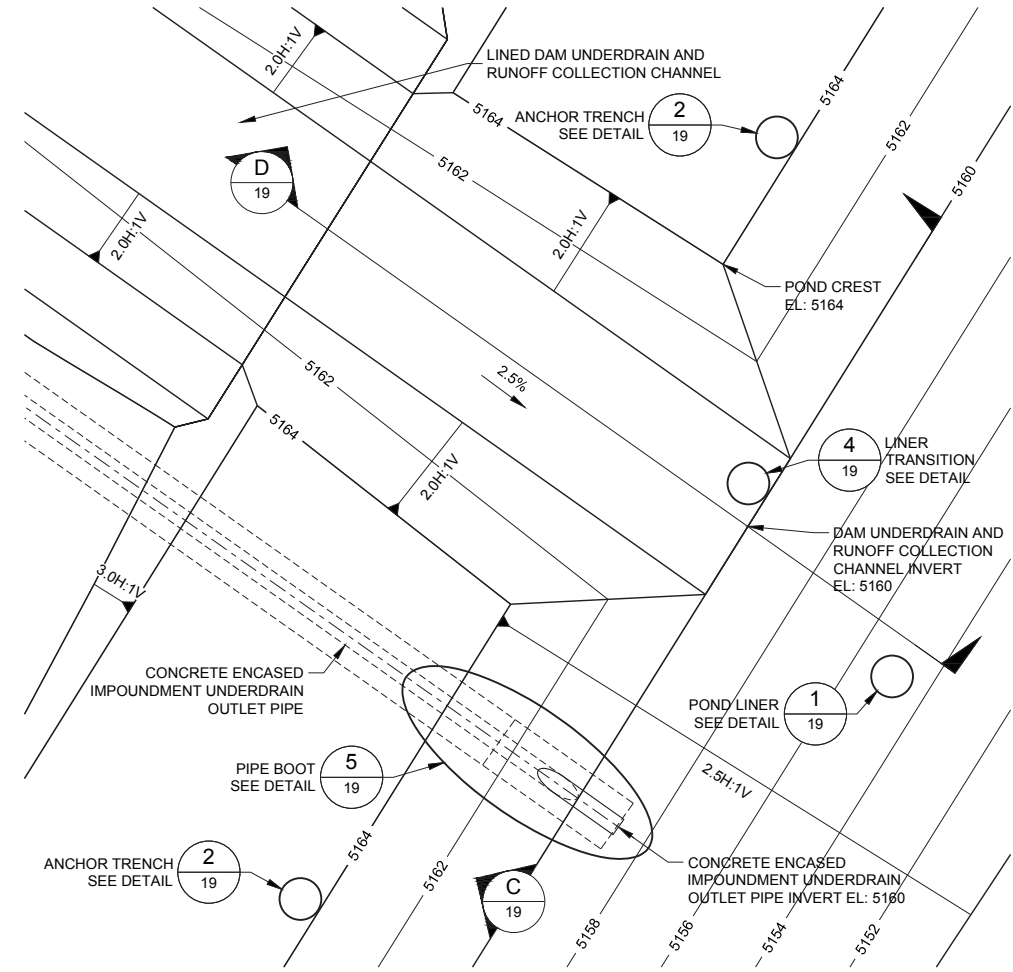
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REVIEW	DAK	2013-07-17	



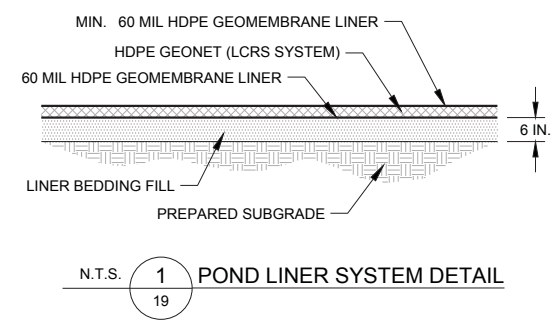
SCALE A **A** CROSS-SECTION A  
19



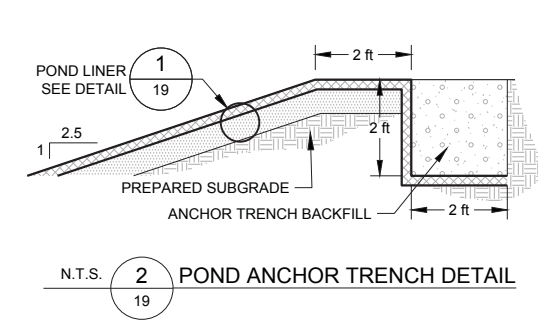
SCALE A **B** CROSS-SECTION B  
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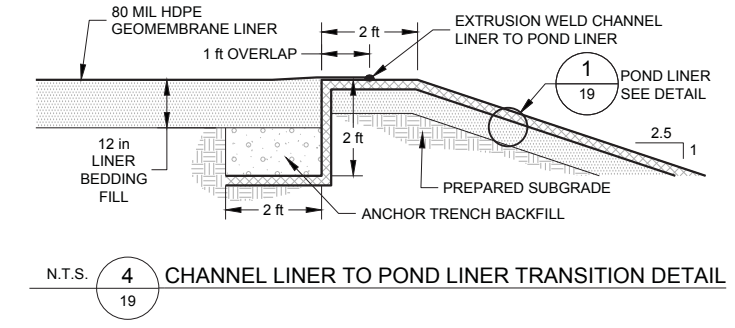
SCALE B **3** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET TO POND  
19



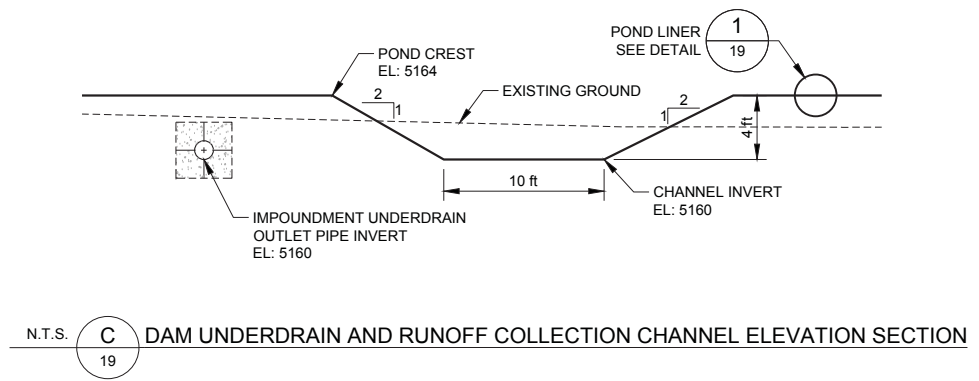
N.T.S. **1** POND LINER SYSTEM DETAIL  
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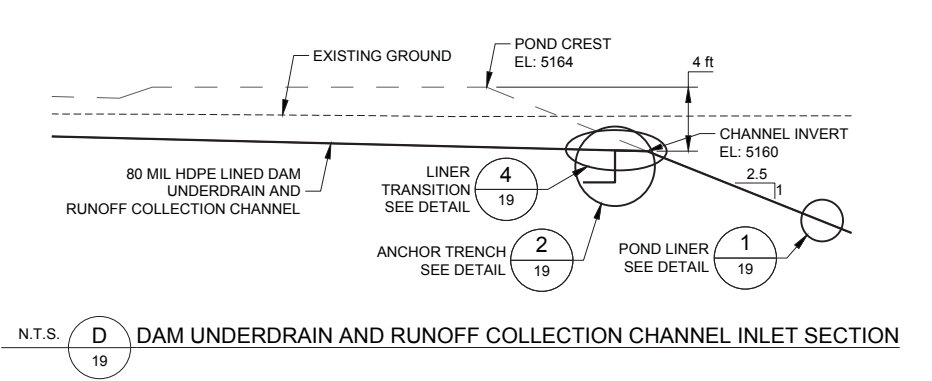
N.T.S. **2** POND ANCHOR TRENCH DETAIL  
19



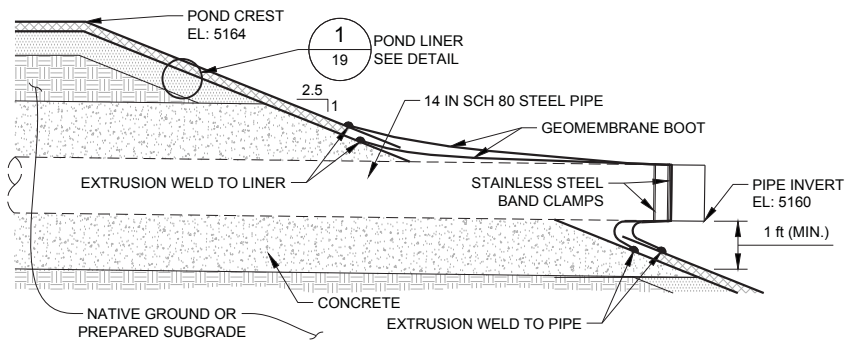
N.T.S. **4** CHANNEL LINER TO POND LINER TRANSITION DETAIL  
19



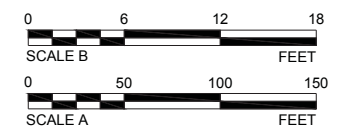
N.T.S. **C** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL ELEVATION SECTION  
19



N.T.S. **D** DAM UNDERDRAIN AND RUNOFF COLLECTION CHANNEL INLET SECTION  
19



N.T.S. **5** IMPOUNDMENT UNDERDRAIN OUTLET PIPE BOOT DETAIL  
19



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

**COPPER FLAT PROJECT**  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

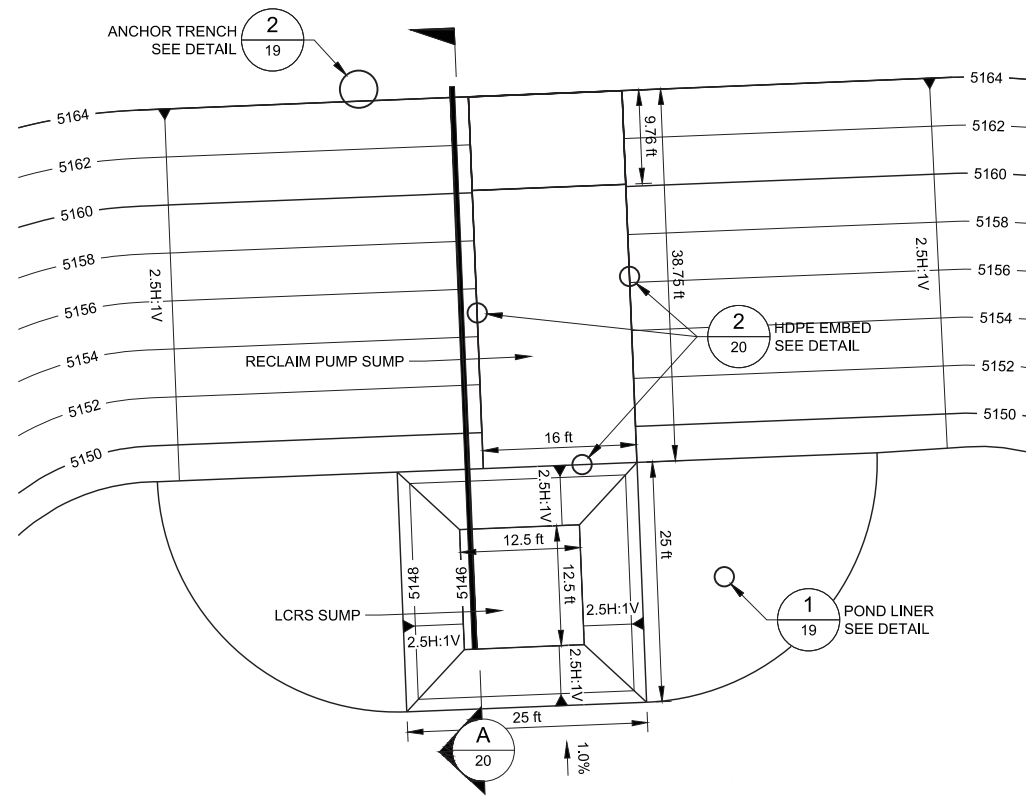
**TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(1 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K013
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CHECK	GM	2013-07-16	
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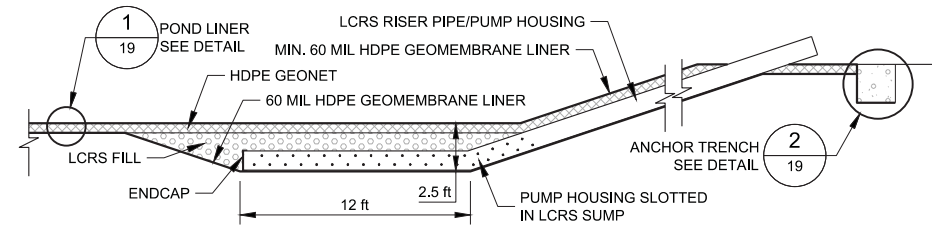


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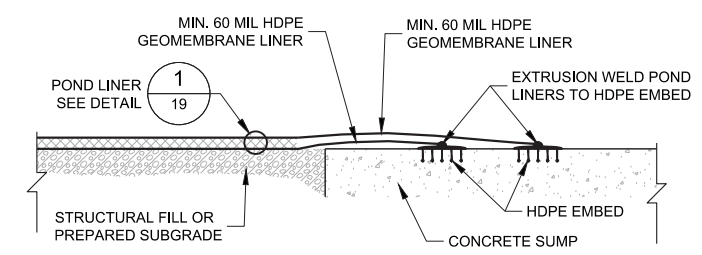




SCALE A **1** RECLAIM PUMP SUMP AND LCRS SUMP DETAIL  
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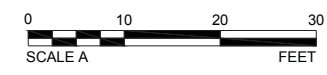


N.T.S. **A** LCRS SUMP SECTION  
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N.T.S. **2** CONCRETE DOUBLE EMBED DETAIL  
20

K:\2010 Projects\103-9257 Copper Flat\Feasibility Design\1039257K013.dwg | Layout: 20 SEEPAGE COLLECTION POND CROSS-SECTIONS AND DETAILS (2 OF 2) | Modified: nccasdo 11/11/2015 4:21 PM | Plotted: jRangel 11/13/2015



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2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK	
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

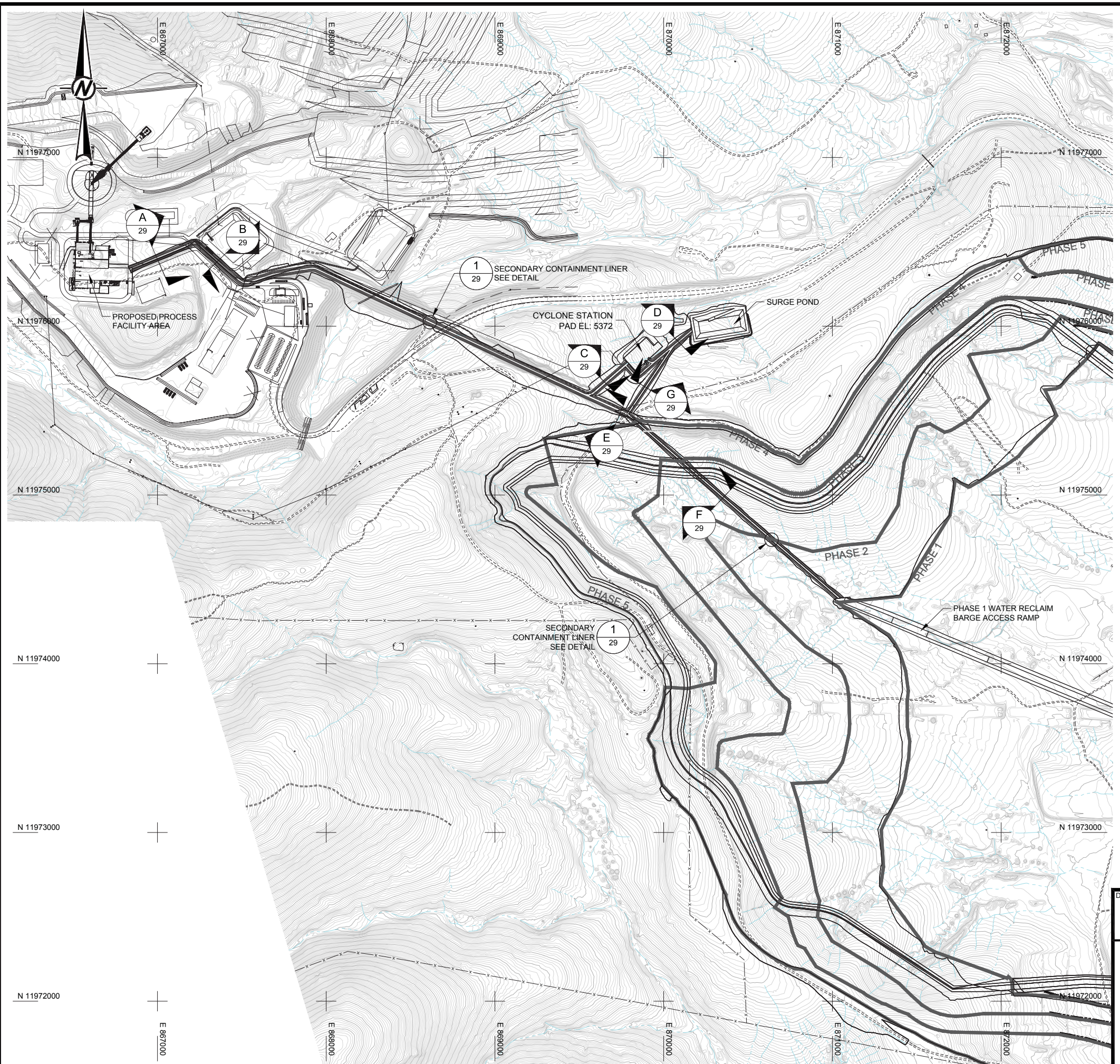
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

**30K TPD TAILINGS STORAGE FACILITY FEASIBILITY DESIGN**  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TSF UNDERDRAIN COLLECTION POND CROSS-SECTION AND DETAILS(2 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K013
DESIGN	DW	2013-04-08	SCALE AS SHOWN
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CHECK	GM	2013-07-16	<b>20</b>
REVIEW	DAK	2013-07-17	

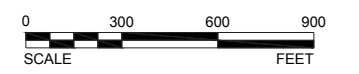




**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- REGRADED CONTOURS (ft -MSL)
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
- CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION

K:\2010 Projects\103-9257\Copper Flat\Facility Design\1039257K014.dwg | Layout: 21 CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN | Modified: nbcasico 11/11/2016 4:45 PM | Plotted: JRange 11/13/2015



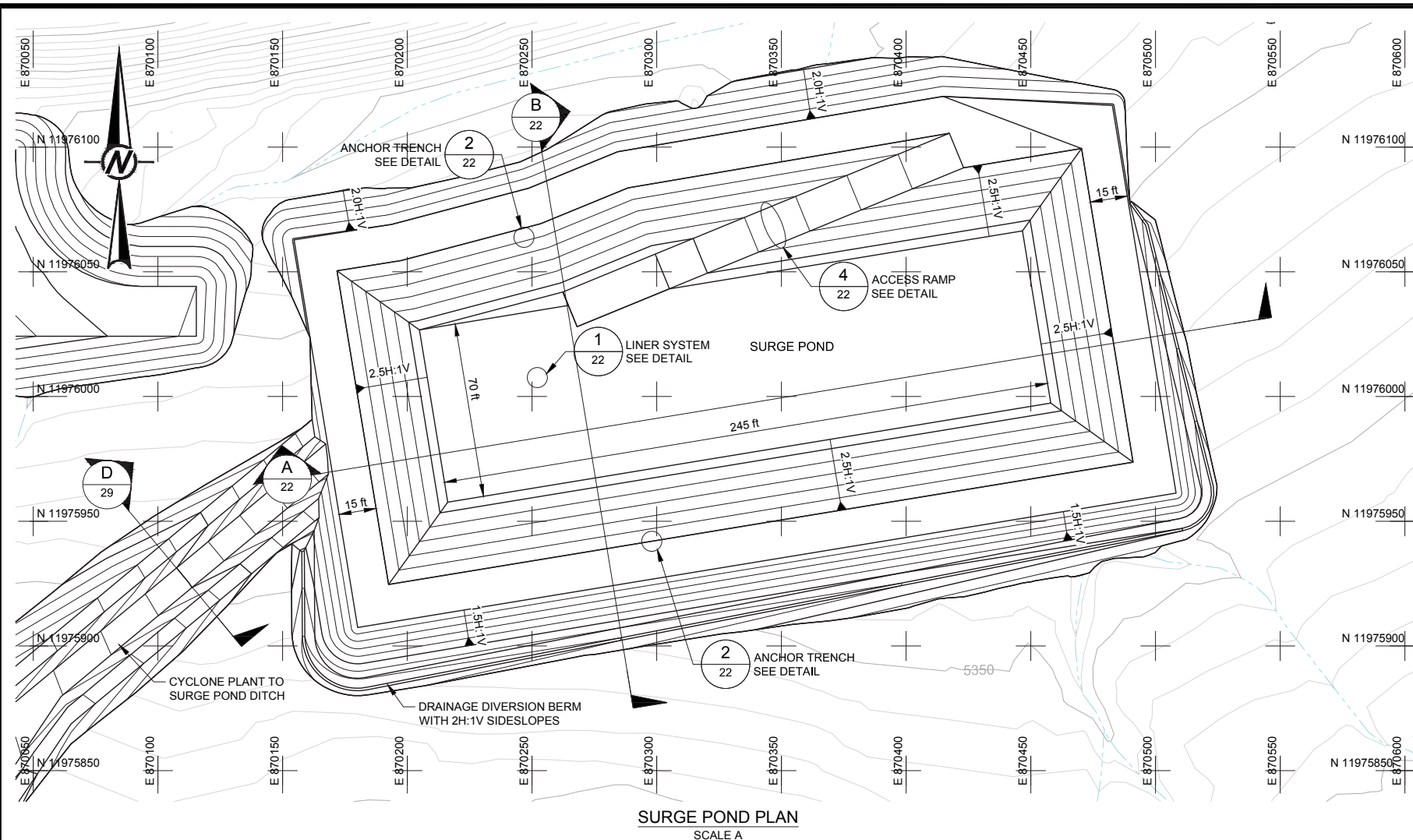
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△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

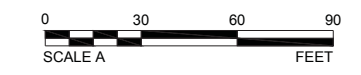
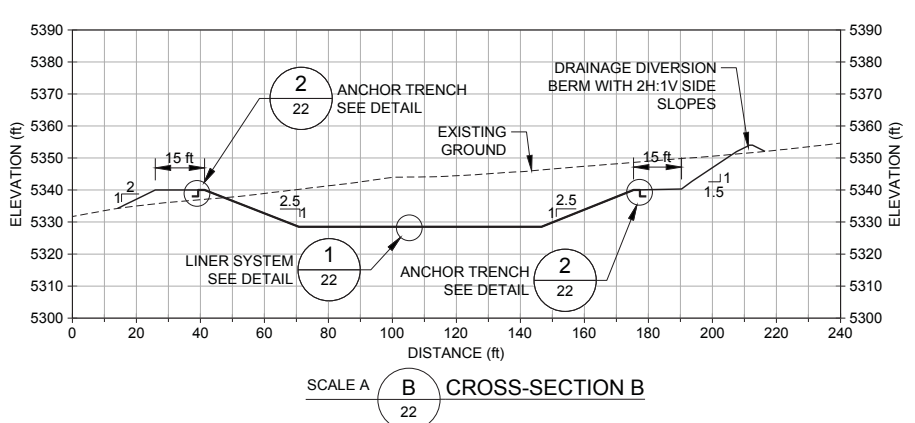
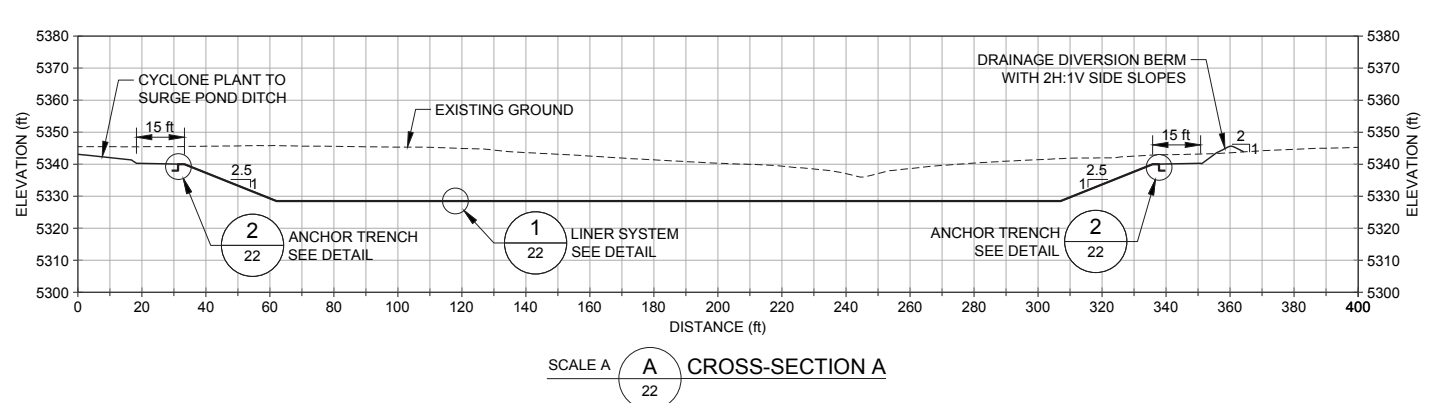
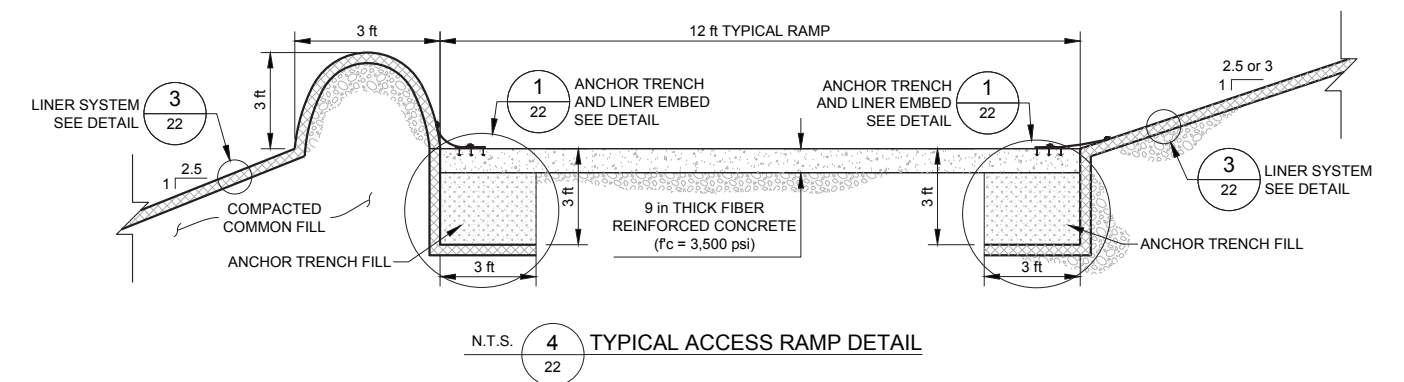
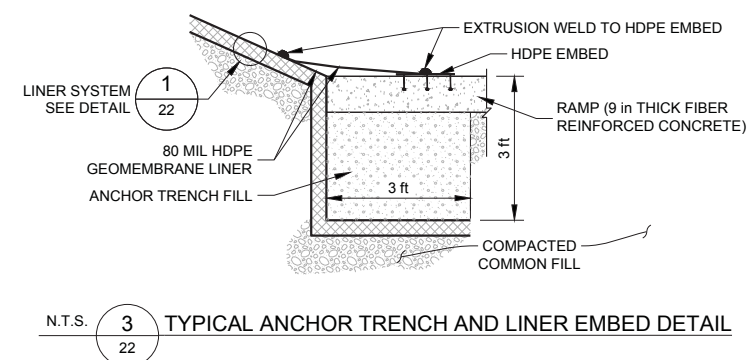
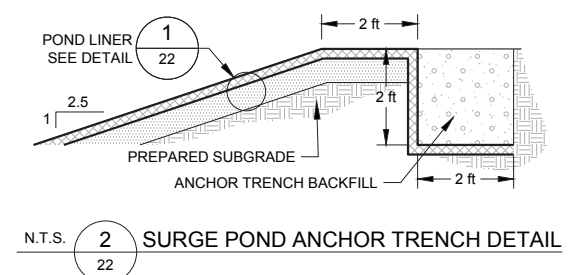
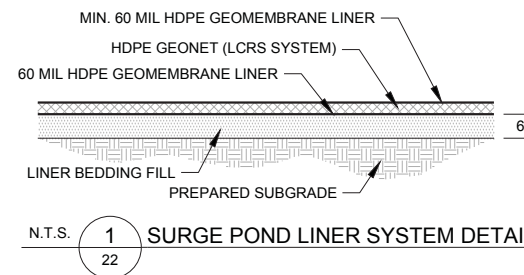
PROJECT  
 NEW MEXICO COPPER CORPORATION  
 Environmentally Responsible. Community Minded. Local Opportunities.  
 COPPER FLAT PROJECT  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**CYCLONE STATION, SURGE POND AND PROCESS AREA PLAN**

PROJECT No.	103-9257	FILE No.	1039257K014
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>21</b>
REVIEW	DAK	2013-07-17	



- LEGEND**
- 3600 EXISTING GROUND CONTOUR (ft -MSL)
  - 3600 REGRADED CONTOURS (ft -MSL)
  - GRADE BREAK
  - SLOPE INDICATOR
  - $3H:1V$  or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
  - 5% GRADE INDICATOR
  - 1  
2-315  
DETAIL CALLOUT  
DETAIL ID  
DRAWING SHEET LOCATION
  - A  
2-313  
CROSS-SECTION CALLOUT  
SECTION ID  
DRAWING SHEET LOCATION



2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK
REV	DATE	DES	CADD	CHK	RVW

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

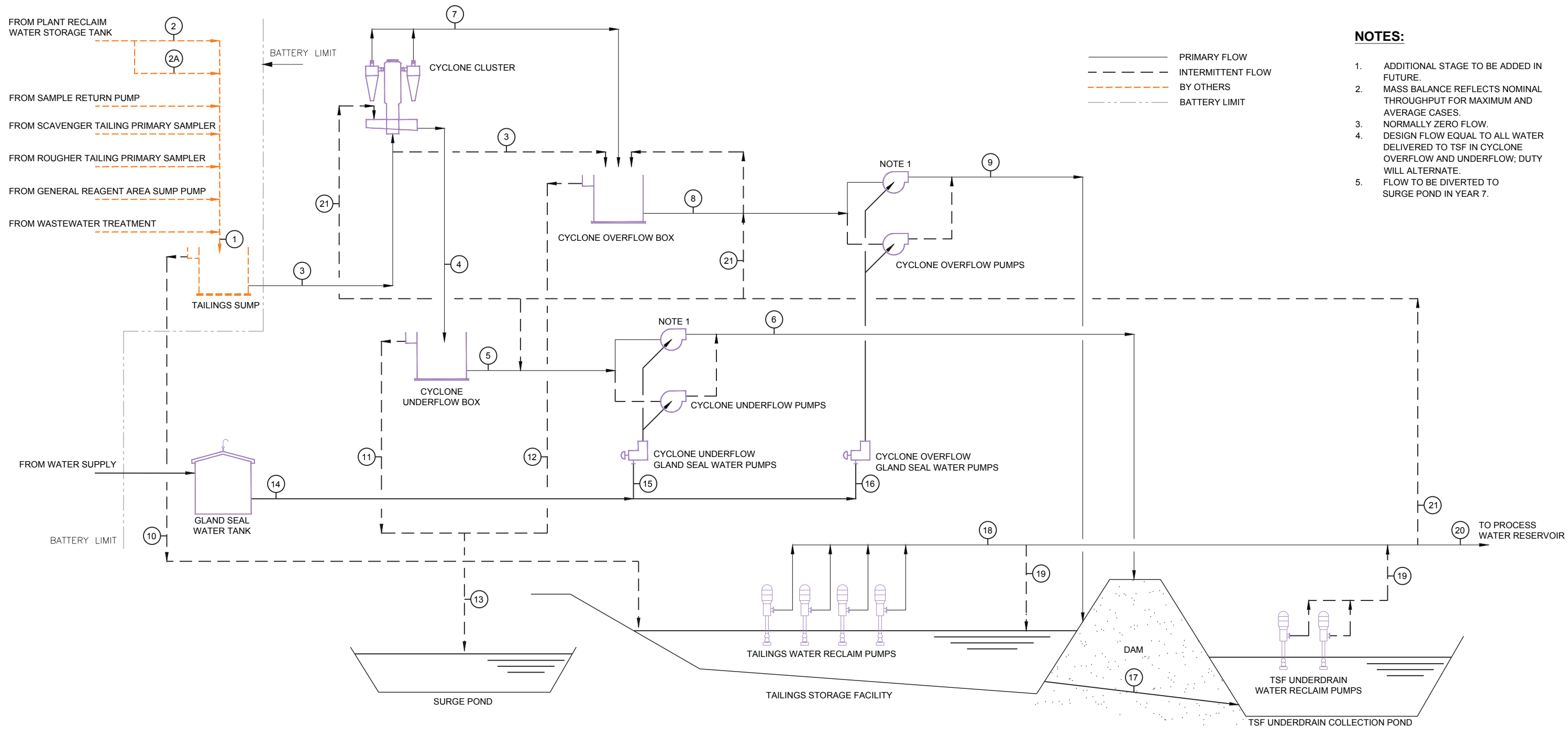
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**SURGE POND PLAN, CROSS-SECTIONS, AND DETAILS**

PROJECT No.	103-92557	FILE No.	10392557K015	
DESIGN	DW	2013-05-03	SCALE	AS SHOWN
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		



K:\2010 Projects\103-92557\Copper Flat\Feasibility Design\10392557K015.dwg | Layout: 22 SURGE POND PLAN CROSS-SECTIONS AND DETAILS | Modified: tccasocio 11/12/2015 7:55 AM | Plotter: JRange | 11/13/2015



- NOTES:**
1. ADDITIONAL STAGE TO BE ADDED IN FUTURE.
  2. MASS BALANCE REFLECTS NOMINAL THROUGHPUT FOR MAXIMUM AND AVERAGE CASES.
  3. NORMALLY ZERO FLOW.
  4. DESIGN FLOW EQUAL TO ALL WATER DELIVERED TO TSF IN CYCLONE OVERFLOW AND UNDERFLOW; DUTY WILL ALTERNATE.
  5. FLOW TO BE DIVERTED TO SURGE POND IN YEAR 7.

K:\2010 Projects\103-92557\Copper Flat\Facility Design\10392557K023.dwg | Layout: 23 GENERAL PROCESS FLOW DIAGRAM | Modified: nicascos 11/11/2015 6:09 PM | Plotted: J.Rangel 11/13/2015

Note 2

	Tailings Sump Feed	Tailings Sump Flush Water Note 3	Dilution Water Note 3	Tailings Sump Outlet/ Cyclone Inlet	Cyclone Underflow w	Cyclone Underflow Box Discharge	Cyclone Underflow Pump Discharge	Cyclone Overflow	Cyclone Overflow Box Discharge	Cyclone Overflow Pump Discharge	Tailings Sump Overflow Note 3, 5	Cyclone Underflow Box Overflow Note 3	Cyclone Overflow Box Overflow Note 3	Cyclone Area Surge Discharge Note 3	Gland Seal Water Supply Header	Cyclone Underflow Pump Gland Seal Water	Cyclone Overflow Pump Gland Seal Water	TSF Underflow	TSF Reclaim Water Note 4	TSF Underdrain Collection Pond Reclaim Water Note 4	Reclaim Water to Plant	Cyclone Area Flush Water from TSF Note 3
Stream	1	2	2A	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21
TPD (solids)	32000			32000	14550	14550	14550	17450	17450	17450	32000	14550	32000	32000								
Solids (t/h)	1333			1333	606	606	606	727	727	727	1333	606	1333	1333								
Solution (t/h)	3249	3755	0	3249	260	260	264	2989	2989	3001	3249	260	3249	3249	17	4	13	3267	3267	1001	3267	490
Slurry (t/h)	4582			4582	866	866	870	3716	3716	3728	4582	866	4582	4582								
Solids (%) Cw	29.1%			29.1%	70.0%	70.0%	69.7%	19.6%	19.6%	19.5%	29.1%	70.0%	29.1%	29.1%								
Solids (gpm)	2018			2018	918	918	918	1101	1101	1101	2018	918	2018	2018								
Solution (gpm)	12981	14999	0	12981	1038	1038	1054	11943	11943	11993	12981	1038	12981	12981	66	16	50	13047	13047	4000	13047	1956
Slurry (gpm)	14999			14999	1956	1956	1972	13044	13044	13094	14999	1956	14999	14999								
Solids (%) Cv	13.5%			13.5%	46.9%	46.9%	46.5%	8.4%	8.4%	8.4%	13.5%	46.9%	13.5%	13.5%								
Solids SG	2.64			2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64	2.64								
Solution SG	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Slurry SG	1.22			1.22	1.77	1.77	1.76	1.14	1.14	1.14	1.22	1.77	1.77	1.22								

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	MJG
REV	DATE	DES	CADD	CHK	RWW

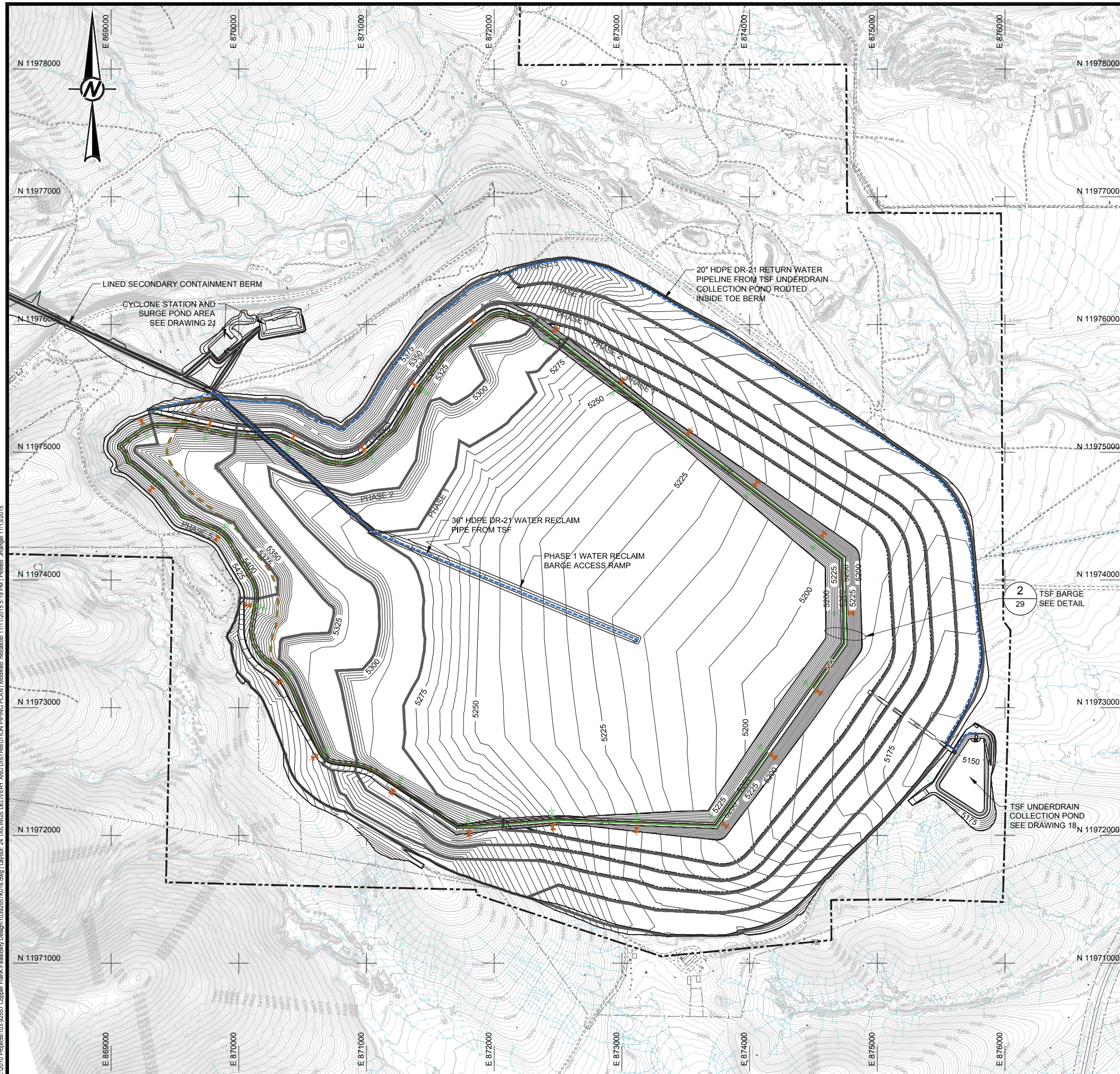
PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

<b>GENERAL PROCESS FLOW DIAGRAM</b>				
PROJECT No.	103-92557	FILE No.	10392557K023	
DESIGN	DMW	2013-04-08	SCALE	NTS
CADD	JHR	2013-07-10	DRAWING	
CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17		



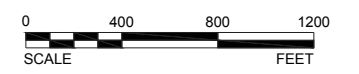
K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K016.dwg | Layout: 24 TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN | Modified: 11/11/2015 5:19 PM | Plotter: JRange1 11/13/2015



**LEGEND**

	EXISTING GROUND CONTOUR (ft -MSL)		PINCH VALVES
	EXISTING ROADS		KNIFE GATES
	EXISTING DRAINAGE		GRADE BREAK
	EXISTING FENCELINE		SLOPE INDICATOR
	MINE PERMIT AREA BOUNDARY		3H:1V or 3H/1V 3 HORIZONTAL TO 1 VERTICAL SLOPE
	REGRADED CONTOURS (ft -MSL)		5% GRADE INDICATOR
	PHASE 1 PHASE BOUNDARY		DETAIL CALLOUT DETAIL ID DRAWING SHEET LOCATION
	12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - PERMANENT ALIGNMENT		CROSS-SECTION CALLOUT SECTION ID DRAWING SHEET LOCATION
	12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - TEMPORARY ALIGNMENT		
	12 IN. DIA. DR-9 HDPE CYCLONE UNDERFLOW PIPELINE - FINAL ALIGNMENT		
	30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - PERMANENT ALIGNMENT		
	30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - TEMPORARY ALIGNMENT		
	30 IN. DIA. DR-17 HDPE CYCLONE OVERFLOW PIPELINE - FINAL ALIGNMENT		

- NOTES**
1. PHASE 1 TAILINGS DISTRIBUTION BEGINS AT EL: 5250 AND ENDS AT EL: 5280.
  2. KNIFE GATE VALVES ON CYCLONE UNDERFLOW AND CYCLONE OVERFLOW DISTRIBUTION PIPES AT 2000 FT SPACING. (SHOWN SPACED FOR CLARITY)
  3. SPIGOTS ON CYCLONE OVERFLOW PIPE AT 660 FT SPACING. (SHOWN SPACED FOR CLARITY)
  4. SPIGOTS ON CYCLONE UNDERFLOW PIPE AT 330 FT SPACING. (SHOWN SPACED FOR CLARITY)



2  
29  
TSE BARGE  
SEE DETAIL

2  
18  
TSE UNDERDRAIN  
COLLECTION POND  
SEE DRAWING 18

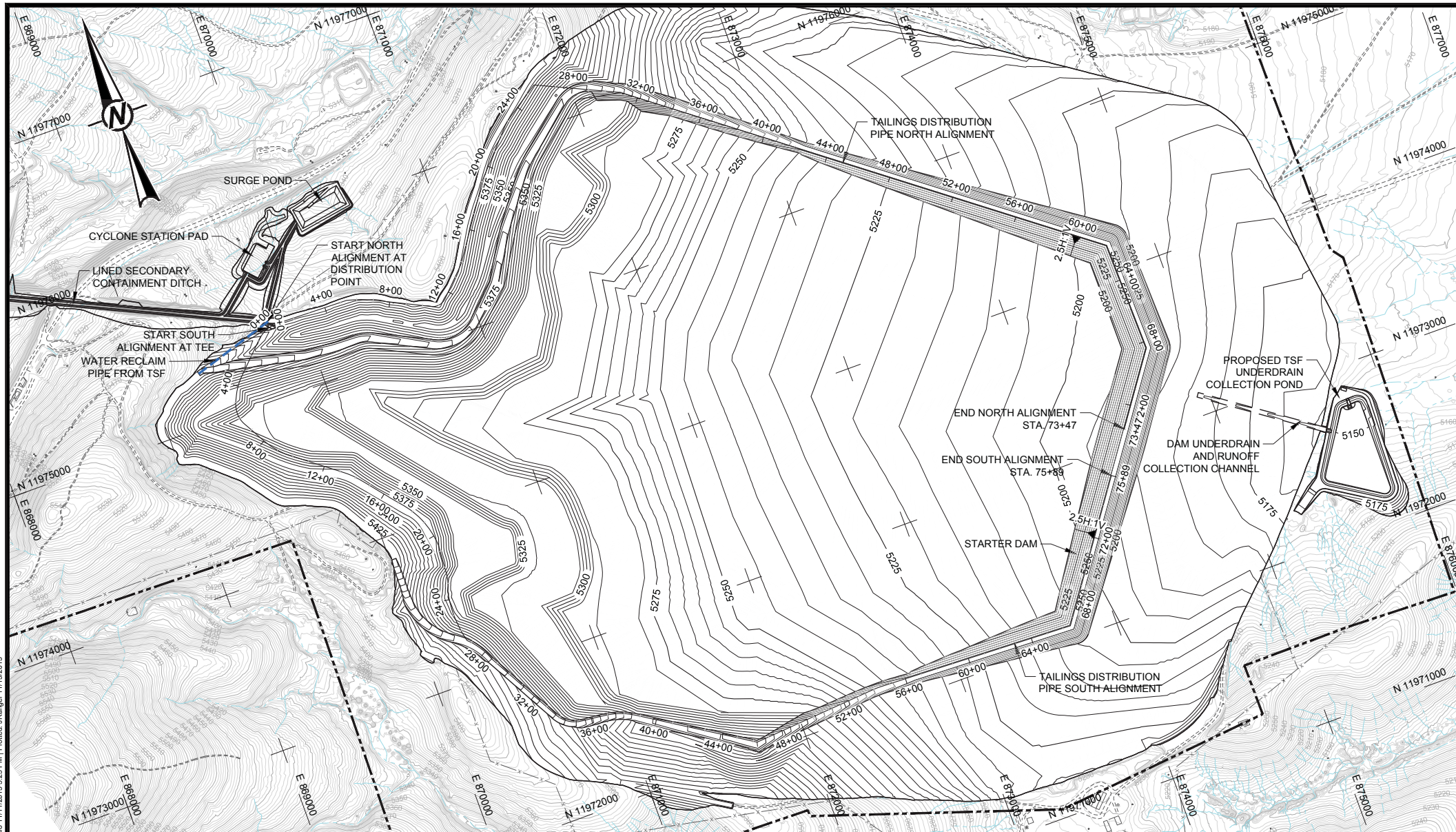
DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.  
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

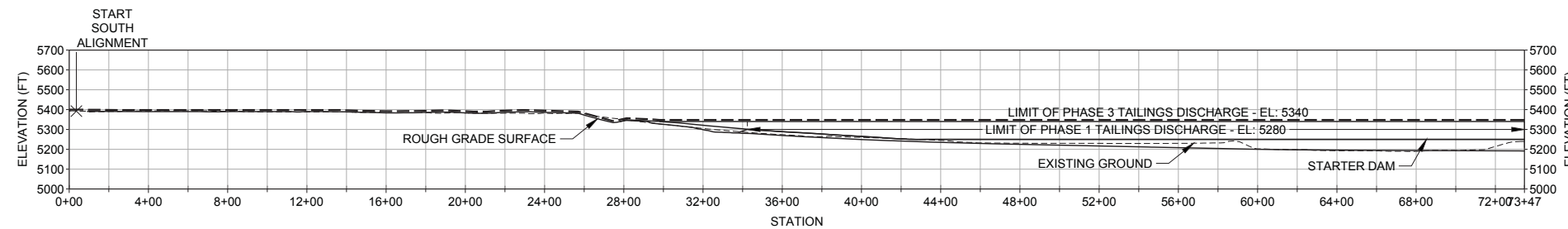
TITLE  
**TAILINGS DELIVERY AND DISTRIBUTION PIPING PLAN**

	PROJECT No.	103-92557	FILE No.	10392557K016	
	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17			

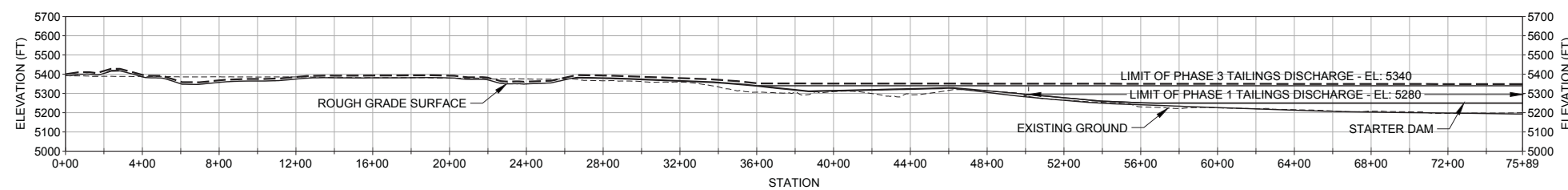


- LEGEND**
- 3600 EXISTING GROUND CONTOUR (ft -MSL)
  - EXISTING ROADS
  - EXISTING DRAINAGE
  - EXISTING FENCELINE
  - MINE PERMIT AREA BOUNDARY
  - 3600 REGRADED CONTOURS (ft -MSL)
  - GRADE BREAK
  - SLOPE INDICATOR
  - $3H:1V$  or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
  - 5% GRADE INDICATOR
  - TAILINGS DELIVERY AND DISTRIBUTION PIPE AT PHASE 3 LIMITS
  - WATER RECLAIM PIPE

PHASES 1-3 TAILINGS DISTRIBUTION PLAN  
SCALE A



PHASES 1-3 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE  
SCALE A



PHASES 1-3 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE  
SCALE A



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVV
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

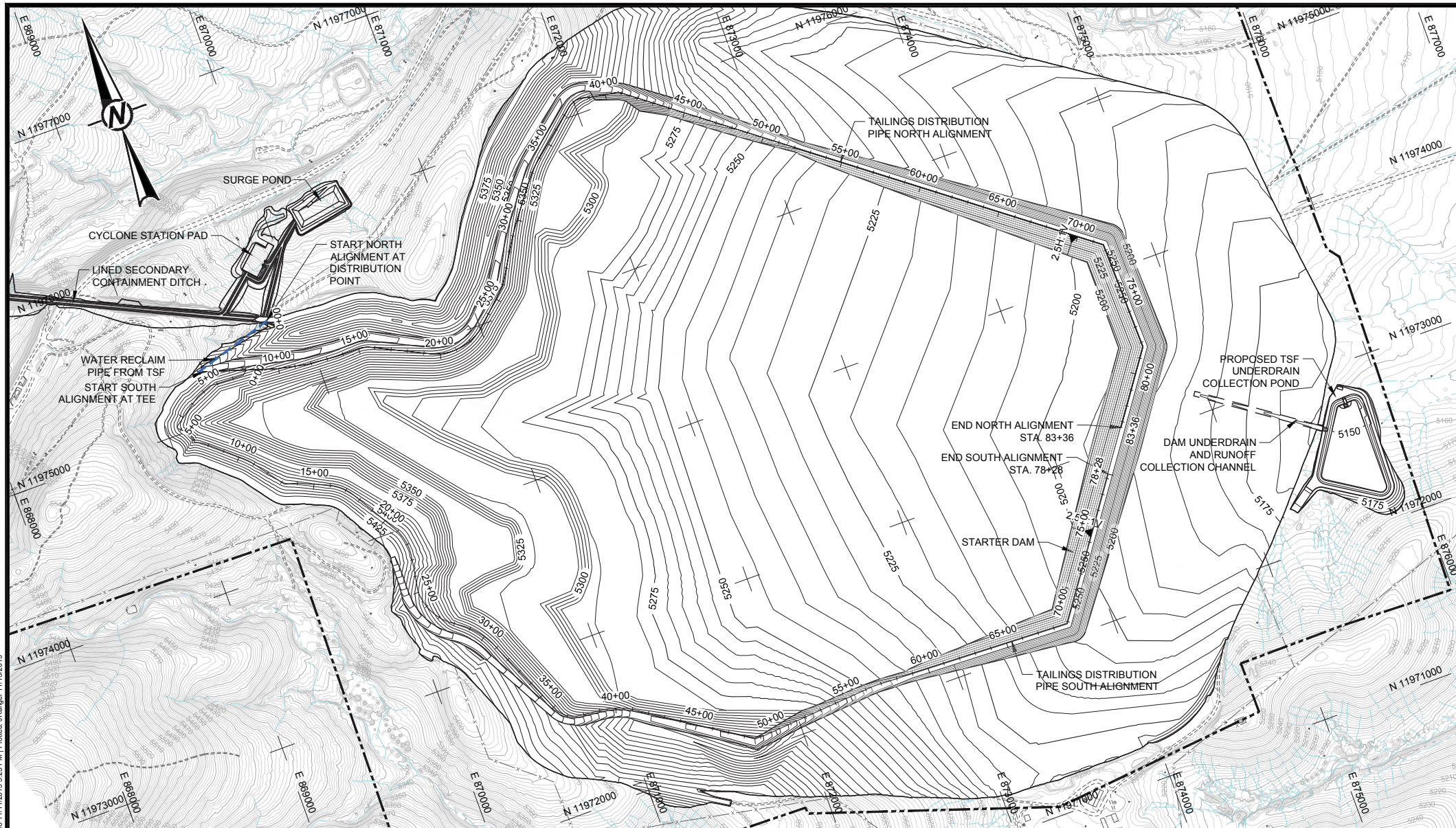
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K017
DESIGN	DW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

**25**

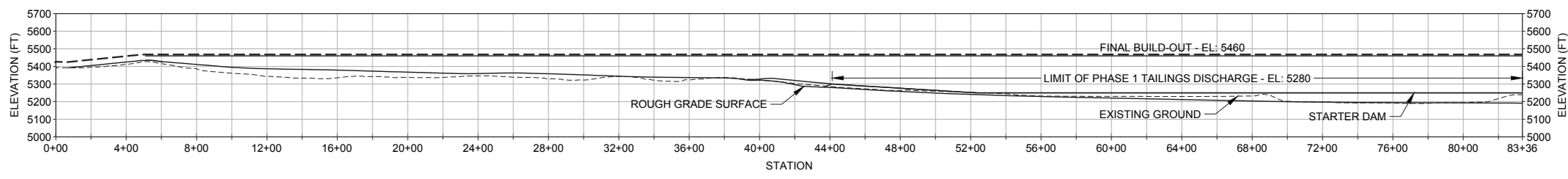
K:\2010 Projects\103-92557 Copper Flat\Feasibility Design\10392557K017.dwg | Layout: 25 TAILINGS DISTRIBUTION PLAN AND PROFILE (1 OF 2) | Modified: abcascas 11/11/2015 5:25 PM | Plotted: J.Rangel 11/13/2015



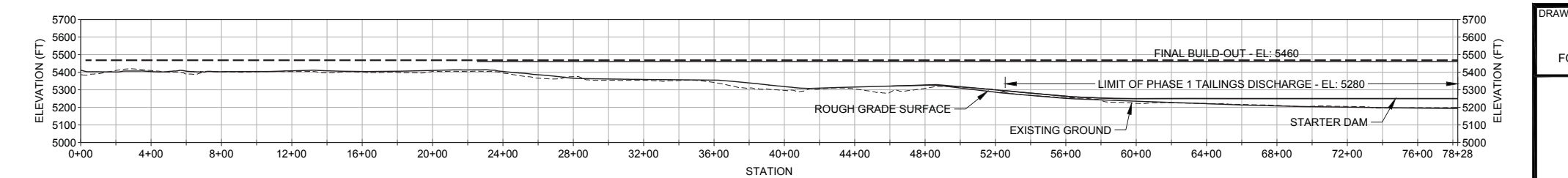
PHASES 4-5 TAILINGS DISTRIBUTION PLAN  
SCALE A

**LEGEND**

- EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- REGRADED CONTOURS (ft -MSL)
- PHASE BOUNDARY
- GRADE BREAK
- SLOPE INDICATOR
- 3 HORIZONTAL TO 1 VERTICAL SLOPE
- GRADE INDICATOR
- TAILINGS DELIVERY AND DISTRIBUTION PIPE AT FINAL BUILD-OUT
- WATER RECLAIM PIPE



PHASES 4-5 TAILINGS DISTRIBUTION NORTH ALIGNMENT PROFILE  
SCALE A



PHASES 4-5 TAILINGS DISTRIBUTION SOUTH ALIGNMENT PROFILE  
SCALE A



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWV
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

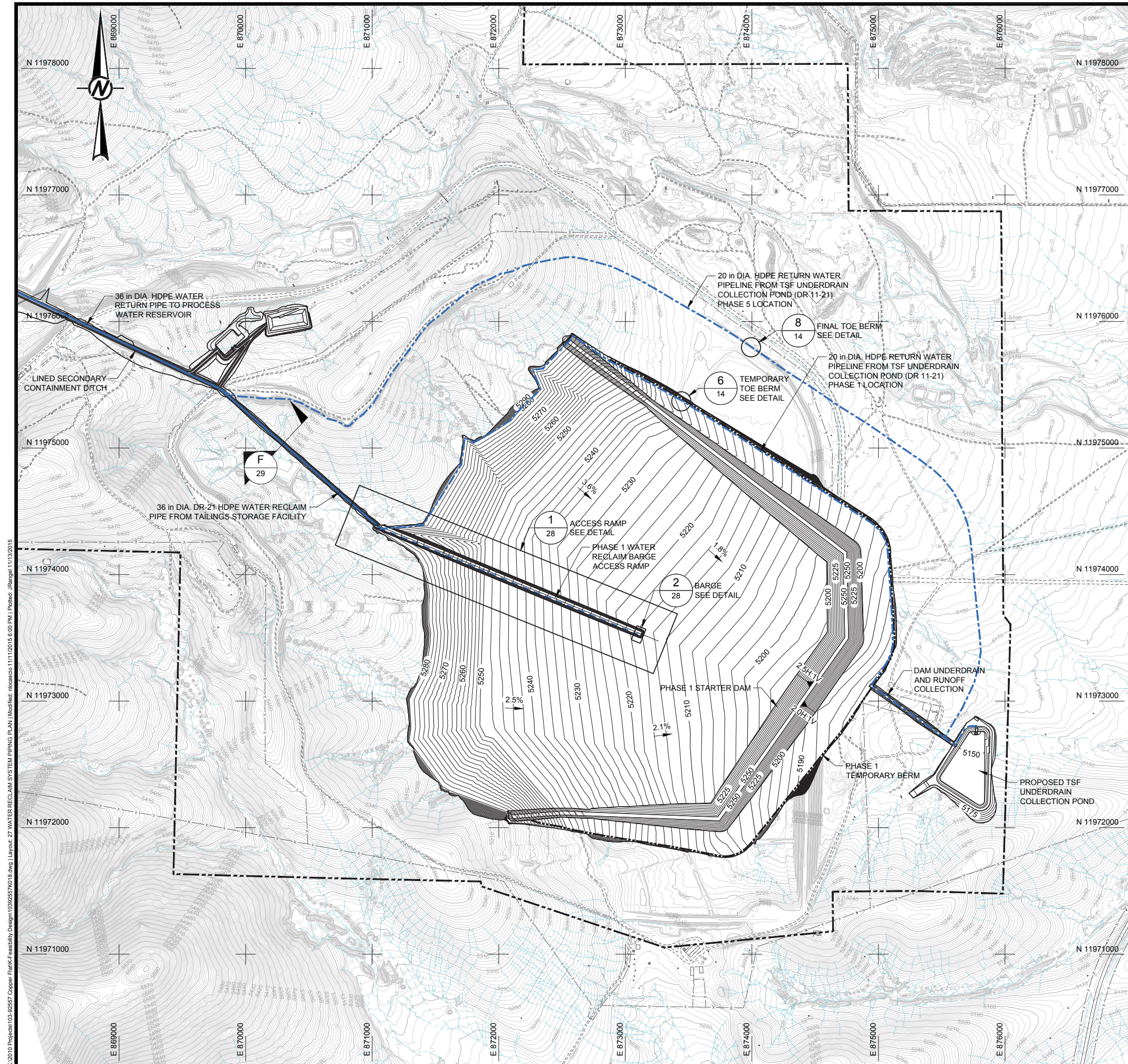
PROJECT: **COPPER FLAT PROJECT**  
**THEMAC** RESOURCES  
 NEW MEXICO COPPER CORPORATION  
 Environmentally Responsible. Community Minded. Local Opportunities.  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2)**

PROJECT No.	103-92557	FILE No.	10392557K017
DESIGN	DMW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	



K:\2010 Projects\103-92557\Copper Flat\FEASIBILITY Design\10392557K017.dwg | Layout: 26 TAILINGS DISTRIBUTION PLAN AND PROFILE (2 OF 2) | Modified: 11/11/2015 5:25 PM | Plotted: JRange | 11/13/2015

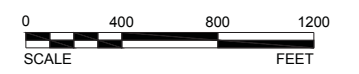


**LEGEND**

- 3600 EXISTING GROUND CONTOUR (ft -MSL)
- EXISTING ROADS
- EXISTING DRAINAGE
- EXISTING FENCELINE
- MINE PERMIT AREA BOUNDARY
- 3600 REGRADED CONTOURS (ft -MSL)
- INTERMEDIATE TOE BERM CENTERLINE
- GRADE BREAK
- SLOPE INDICATOR
- $3H:1V$  or  $\frac{3H}{1V}$  3 HORIZONTAL TO 1 VERTICAL SLOPE
- 5% GRADE INDICATOR
- 1  
14

**DETAIL CALLOUT**  
 1 14  
 2 28  
 6 14  
 8 14

- NOTES**
- SEE DRAWING 29 FOR RETURN WATER PIPE DITCH SECTIONS AND DETAILS.
  - RETURN WATER PIPELINE FROM TSF UNDERDRAIN AND RUNOFF COLLECTION POND WILL BE ROUTED INSIDE THE EMBANKMENT TOE BERM AND WILL REQUIRE RELOCATION AT EACH CONSTRUCTION PHASE.
  - RETURN WATER PIPELINE WILL BE RELOCATED TO THE TOE BERM AT EACH PHASE OF BUILDOUT. PHASE 1 AND PHASE 5 LOCATIONS ARE SHOWN ON DRAWING.



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
△	2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
△	2014-01-06	ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	MJG
△	2013-11-15	ISSUED FOR 30,000 TPD M3 USE	DMW	NIL	GM	MJG
△	2013-07-17	ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
△	2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
 FOR AGENCY REVIEW

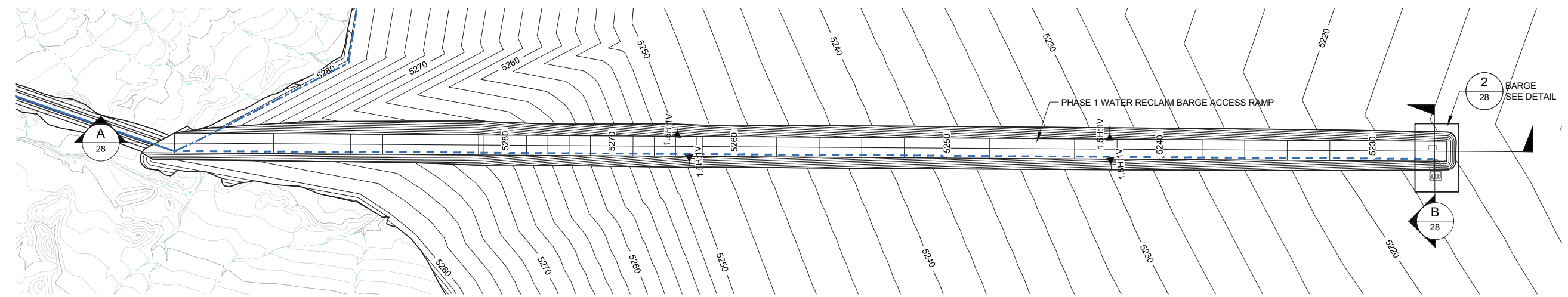
PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
 Environmentally Responsible. Community Minded. Local Opportunities.  
 30K TPD TAILINGS STORAGE FACILITY  
 FEASIBILITY DESIGN  
 SIERRA COUNTY, NEW MEXICO

TITLE  
**WATER RECLAIM SYSTEM PIPING PLAN**

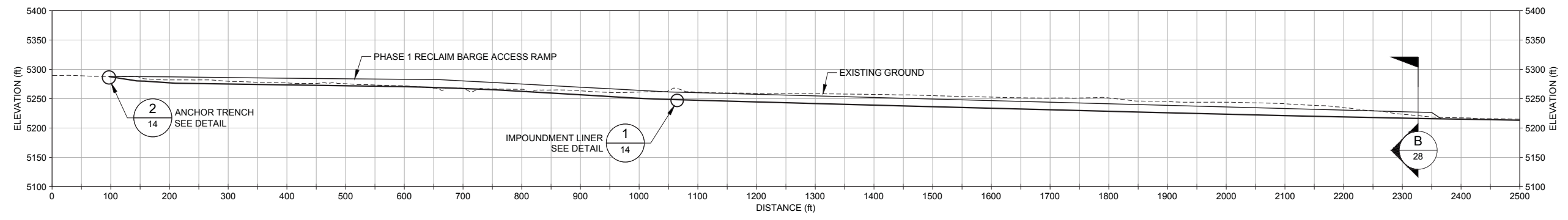
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	DESIGN	DW	2013-04-08	SCALE	AS SHOWN
	CADD	JHR	2013-07-10	DRAWING	
	CHECK	GM	2013-07-16		
REVIEW	DAK	2013-07-17			

K:\2010 Projects\103-92557 Copper Flat\KFeasibility Design\10392557K018.dwg | Layout: 27 WATER RECLAIM SYSTEM PIPING PLAN | Modifier: nbarcasco 11/11/2016 6:00 PM | Plotter: J:\Rangel 11/13/2015

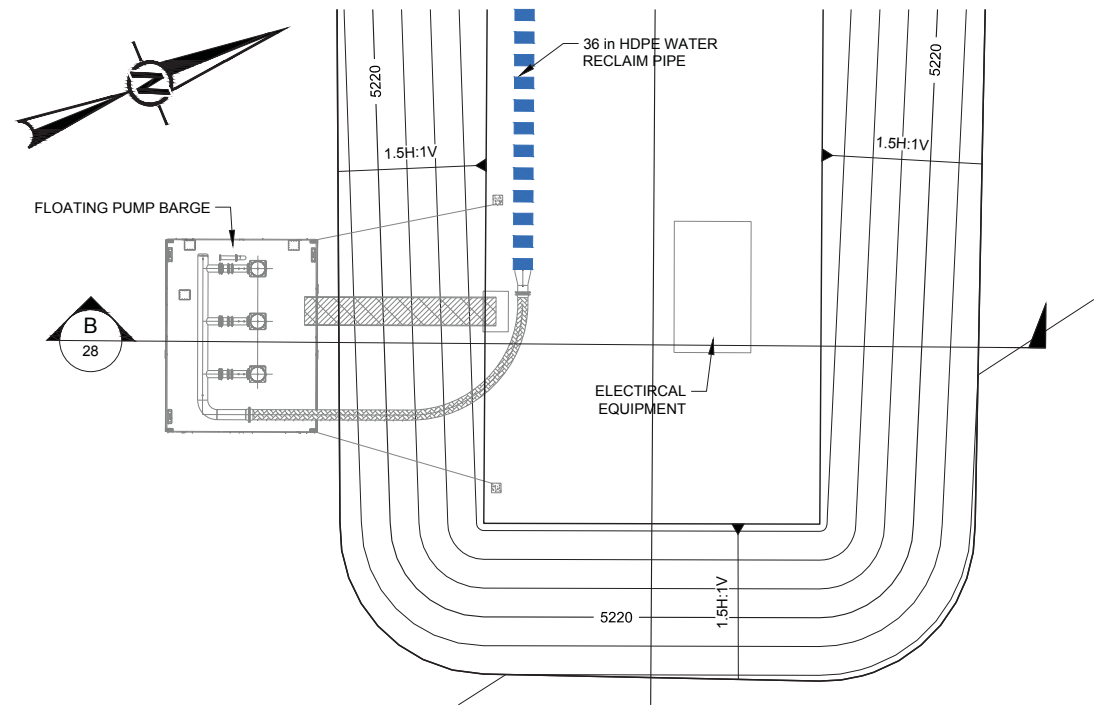




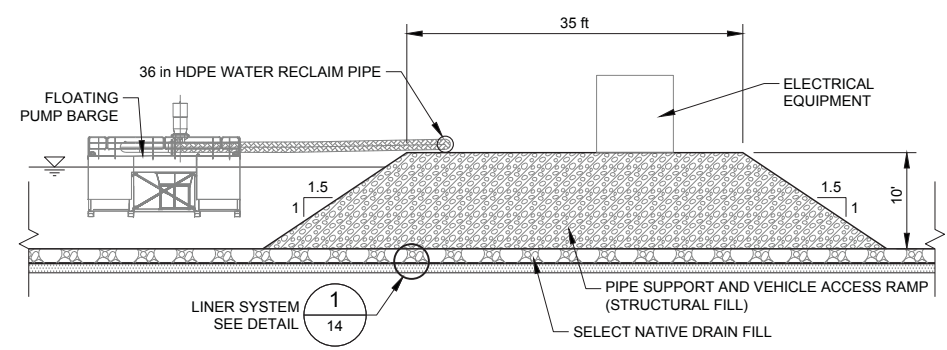
SCALE A **1** PIPE RAMP DETAIL  
28



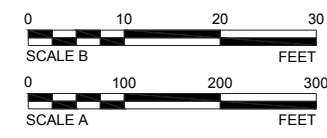
SCALE A **A** PIPE RAMP SECTION  
28



SCALE B **2** RECLAIM BARGE DETAIL  
28



N.T.S. **B** RECLAIM BARGE SECTION  
28



REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RVVW
2015-11-12		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2015-10-27		ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG
2014-01-06		ISSUED FOR FEASIBILITY REPORT (30,000 TPD)	DMW	JHR	GM	DAK
2013-07-17		ISSUED FOR FEASIBILITY STUDY	DMW	NIL	GM	DAK
2013-05-03		ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	DAK

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** RESOURCES  
NEW MEXICO COPPER CORPORATION  
Environmentally Responsible. Community Minded. Local Opportunities.

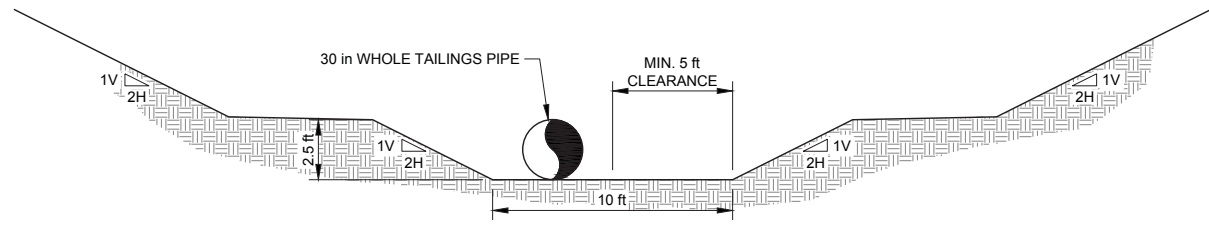
COPPER FLAT PROJECT  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE			
<b>WATER RECLAIM SYSTEM DETAILS</b>			
PROJECT No.	103-92557	FILE No.	10392557K018
DESIGN	DW	2013-04-08	SCALE NOT TO SCALE
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	
REVIEW	DAK	2013-07-17	

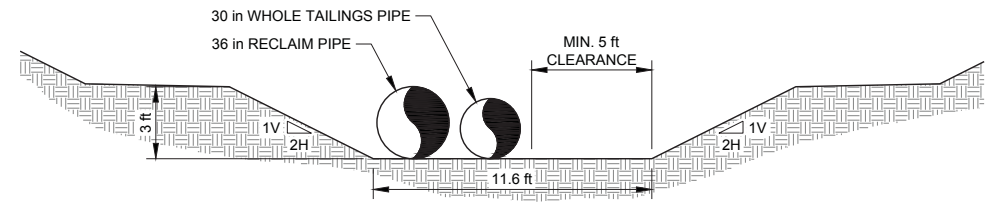


K:\2010 Projects\103-92557\Copper Flat\Fee\Feasibility Design\10392557K018.dwg | Layout: 28 WATER RECLAIM SYSTEM DETAILS | Modified: inccasoo 11/11/2015 6:00 PM | Plotted: \_Rangel 11/13/2015

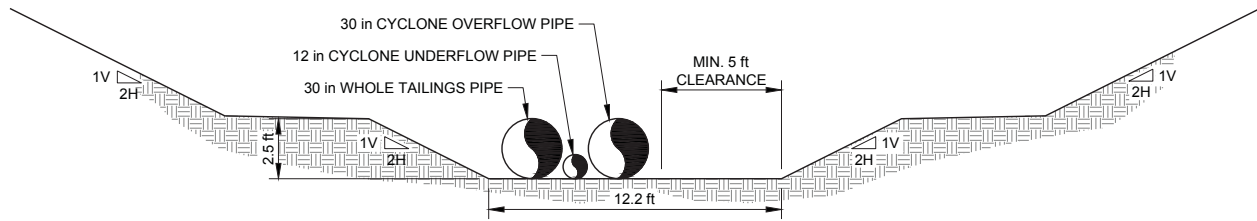
K:\2010 Projects\103-9257-Copper-Flat\Feasibility Design\1039257K022.dwg | Layout: 20 TAILINGS DISTRIBUTION SECONDARY CONTAINMENT DETAILS AND SECT | Modified: mcaasoo 11/11/2015 6:08 PM | Plotted: Rangal 11/13/2015



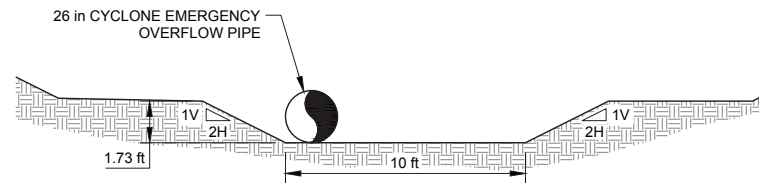
SCALE: N.T.S. **A** 29  
SECONDARY CONTAINMENT MAIN DITCH ABOVE PROCESS WATER RESEVOIR



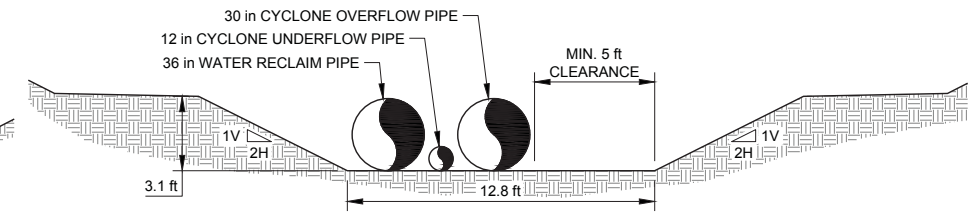
SCALE: N.T.S. **B** 29  
MAIN DITCH BELOW PROCESS WATER RESEVOIR



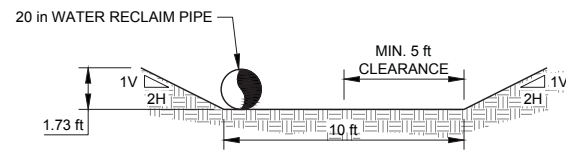
SCALE: N.T.S. **C** 29  
MAIN DITCH TO CYCLONE PLANT



SCALE: N.T.S. **D** 29  
CYCLONE PLANT TO SURGE POND DITCH



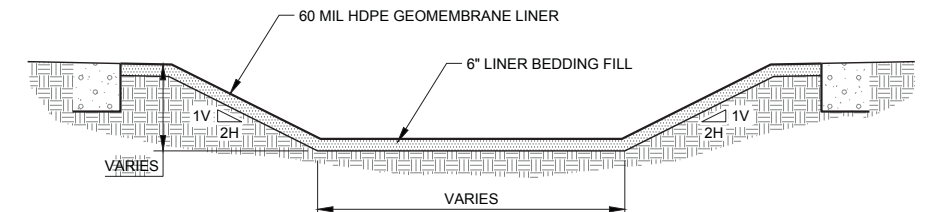
SCALE: N.T.S. **E** 29  
MAIN DITCH BELOW CYCLONE PLANT



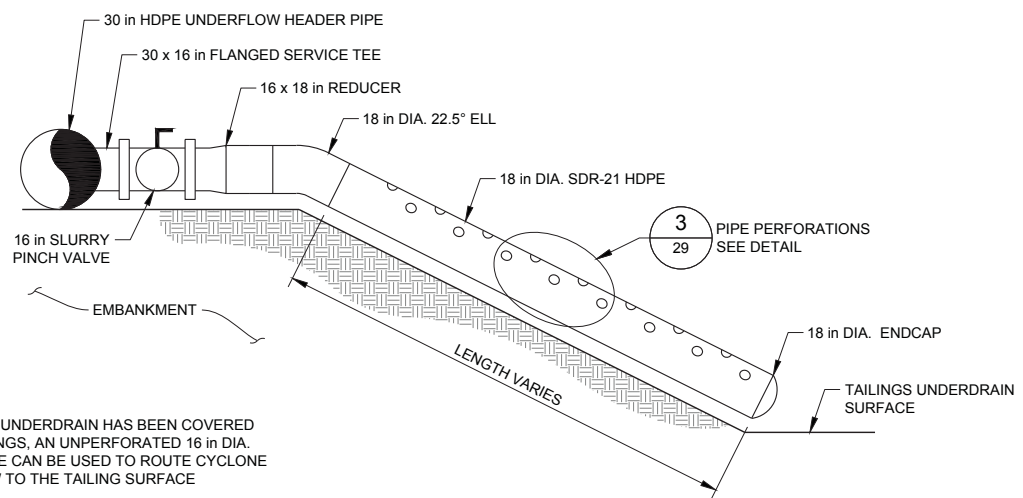
SCALE: N.T.S. **F** 29  
IN TSF RECLAIM PIPE DITCH



SCALE: N.T.S. **G** 29  
POST YEAR 6 SURGE POND DITCH

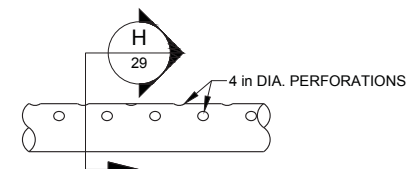


SCALE: N.T.S. **1** 29  
TYPICAL SECONDARY CONTAINMENT LINER

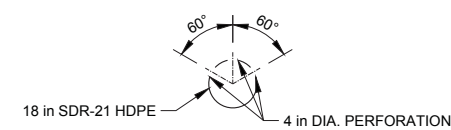


NOTE:  
WHEN THE UNDERDRAIN HAS BEEN COVERED WITH TAILINGS, AN UNPERFORATED 16 in DIA. SDR-32 PIPE CAN BE USED TO ROUTE CYCLONE OVERFLOW TO THE TAILING SURFACE

SCALE: N.T.S. **2** 29  
ENERGY DISSIPATOR/SCOUR PROTECTION CYCLONE OVERFLOW DISTRIBUTION SYSTEM



SCALE: N.T.S. **3** 29  
PIPE PERFORATION DETAIL



SCALE: N.T.S. **H** 29  
PIPE PERFORATION SECTION

2015-11-12	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2015-10-27	ISSUED FOR CLIENT AND AGENCY REVIEW	GM	NIL	GM	MJG	
2013-05-03	ISSUED FOR CLIENT REVIEW	DMW	NIL	GM	MJG	
REV	DATE	REVISION DESCRIPTION	DES	CADD	CHK	RWW

DRAWING USE  
**PRELIMINARY**  
FOR AGENCY REVIEW

PROJECT  
**THEMAC** NEW MEXICO COPPER CORPORATION  
30K TPD TAILINGS STORAGE FACILITY  
FEASIBILITY DESIGN  
SIERRA COUNTY, NEW MEXICO

TITLE  
**TAILINGS DISTRIBUTION AND SECONDARY CONTAINMENT DETAILS AND SECTIONS**

PROJECT No.	103-92557	FILE No.	10392557K022
DESIGN	DMW	2013-04-08	SCALE AS SHOWN
CADD	JHR	2013-07-10	DRAWING
CHECK	GM	2013-07-16	<b>29</b>
REVIEW	DAK	2013-07-17	



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# Copper Flat Project



## Impoundment Design Report

Prepared For:

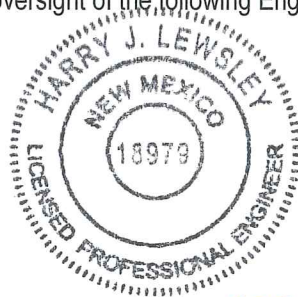
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This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.

Harry Lewsley  
Signature



Exp. 12/31/2017  
Date 12/7/2015

IMPOUNDMENT DESIGN REPORT  
COPPER FLAT PROJECT

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*Harry J. Lewisley* 12/7/2015  
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*Harry J. Lewsley* 12/7/2015  
  
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## 1 INTRODUCTION

The Copper Flat Project is located in South Central New Mexico, near the town of Hillsboro, approximately 150 miles south of Albuquerque, and approximately 20 miles southwest of Truth or Consequences (straight-line distances) (Figure 1). The Project is owned and operated by New Mexico Copper Corporation (NMCC), a wholly owned subsidiary of THEMAC Resources Group Limited.

The State of New Mexico has promulgated regulations pertaining to groundwater protection at copper mining facilities (New Mexico Administrative Code Title 20, Chapter 6, Part 7 [20.6.7 NMAC], the "Copper Rule"), the stated purpose of which is "to control discharges of water contaminants specific to copper mine facilities and their operations to prevent water pollution."

This report provides the design criteria, location, purpose, operation, and performance of certain elements of the project identified in Section 2 of this report to comply with 20.6.7 NMAC. This report excludes the design considerations for the Tailings Storage Facility (TSF), i.e., the tailings impoundment, underdrain collection pond, surge pond and the secondary containment trench from the processing facility to the TSF, which have been completed by others and are reported separately.



## 2 SYSTEM DESCRIPTIONS

Impacted Stormwater Impoundments are designed to receive surface drainage that potentially has come in contact with water contaminants on a copper mine facility. These systems consist of a network of diversion channels designed to convey to the impoundment at minimum the peak from a 100-year-return-interval storm with at least 6 inches of freeboard per 20.6.7.17.D.(2).(f). The Impacted Stormwater Impoundments are designed to store impacted stormwater for less than 30 days and include an engineered liner system, as described in Section 3.2.

The Process Water Reservoir is designed to receive reclaimed process water from a variety of sources including the TSF, impacted stormwater impoundments and freshwater supply system conveyed via pipelines. The reservoir also receives direct precipitation to the pond surface and embankment crest area. The Process Water Reservoir is designed with an engineered liner system, leak collection system, and subgrade bedding, as described in Section 3.3.

3 BASIS OF DESIGN

3.1 GENERAL

All impoundments for the Copper Flat Project will be considered "new" impoundments as defined by NMAC 20.6.7.17 (D).

Outside Slopes	20.6.7.17.D.(1).(a) NMAC	Maximum 2:1 (H:V)
Static factor of safety	20.6.7.17.D.(1).(a) NMAC	Minimum 1.3
Liner Sidewall seams	20.6.7.17.D.(1).(e) NMAC	Vertical only
Capacity	20.6.7.17.D.(2) NMAC	Contain 100-year return interval storm event plus minimum 2 ft of freeboard

3.2 IMPACTED STORMWATER IMPOUNDMENTS

Impacted stormwater impoundments are designed to hold impacted stormwater for less than 30 days in accordance with NMAC 20.6.7.17 (D) (4) and (7).

Liner system	20.6.7.17.D.(4).(a) NMAC	Compacted minimum 6-inch subbase overlain by 60 mil HDPE liner system
Wind protection	20.6.7.17.D.(4).(d) NMAC	Weighting system to limit liner damage in high winds
Spillway design	20.6.7.17.D.(7) NMAC	Safely discharge peak flow from 24-hour storm event with 25-year return

3.3 PROCESS WATER RESERVOIR

Process Water Impoundments/Reservoirs are designed to hold process water at design capacity plus impacted stormwater for more than 30 days in accordance with NMAC 20.6.7.17 (D) (3).

Liner system	20.6.7.17.D.(3).(a) and 20.6.7.17.D.(3).(c) NMAC	Primary 60 mil HDPE liner over a secondary 60 mil HDPE liner with drainage layer over a compacted minimum 6-inch subbase
Leakage collection system	20.6.7.17.D.(3).(d) NMAC	Drainage layer between primary and secondary liners with fluid removal system
Drainage layer	20.6.7.17.D.(3).(d) NMAC	Granular soil material or geosynthetic drainage net
Drainage layer slope	20.6.7.17.D.(3).(d) NMAC	At least 2 percent
Drainage layer permeability	20.6.7.17.D.(3).(d) NMAC	At least $1 \times 10^{-2}$ centimeters per second (cm/s)
Collection sump	20.6.7.17.D.(3).(d) NMAC	At confluence drainage layer with dedicated automatic pump system with totalizing flow meter and automated failure alarm system
Spillway design	20.6.7.17.D.(7) NMAC	No discharge to ground surface, safely discharge peak process flows

## 4 DESIGN AND CONSTRUCTION OF IMPACTED STORMWATER IMPOUNDMENTS AND PROCESS WATER RESERVOIR

The Copper Flat Project permit boundary (Figure 2) incorporates the mine pit, processing plant area, waste rock stockpiles (WRSPs), and the TSF. The TSF and related facilities including the cyclone plant, surge pond, and underdrain collection pond are described by others. The mine and process plant area includes five developed watershed (WS) areas (Figure 3) that are managed as part of this plan. The facilities described below are designed to manage process and impacted stormwater to prevent releases from the site to surface water and groundwater (Figure 4).

### 4.1 FACILITIES

WS A includes the process plant, maintenance, and administrative areas of the Copper Flat Project. It also includes WS E, which is the Process Water Reservoir as a separate, internal area of stormwater and process water management (Sec. 4.1.5). During precipitation events, sheet flow of stormwater is directed (Figure 5) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment A (Figures 6 and 7). The impoundment is designed with a spillway that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment A are presented in Table 1.

WS B includes runoff from the western flank of Animas Peak and proposed new waste rock stockpiles (WRSP-1) (Figure 3). During precipitation events, sheet flow of stormwater is directed (Figure 3) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment B (Figures 8 and 9). The impoundment is designed with a spillway to the mine pit that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment B are presented in Table 1.

WS C includes runoff from the eastern flank of Animas Peak and proposed new waste rock stockpiles (WRSP-2 and 3) (Figure 3). During precipitation events, sheet flow of stormwater is directed (Figure 3) to open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to Impacted Stormwater Impoundment C (Figures 10 and 11). The impoundment is designed with a spillway that is capable of safely discharging the peak flow from a 25-year, 24-hour precipitation event with a 90 percent chance of not being exceeded during the design life of the impoundment. Design criteria for Impacted Stormwater Impoundment C are presented in Table 1.

WS D includes runoff from uphill slopes and existing waste rock stockpiles (EWRSP-1 and -2b) to the mine pit (Figure 3). During precipitation events, sheet flow of stormwater is directed by natural drainage channels and open channel conveyances designed to convey the peak flow from a 100-year return interval storm event with at least 6 inches of freeboard to the mine pit.

WS E consists of direct precipitation onto the lined surfaces of the Process Water Reservoir and unlined perimeter road that is directed to the reservoir (Figure 3). The amount of direct precipitation to the pond is small (9.5 cubic feet per second [cfs]) in comparison to the design throughput of process solutions through the pond (100,000 cfs). The design freeboard of 2 ft is more than adequate to handle the additional flux from a precipitation event. The design capacity of the pond is 726,400 cubic feet (ft<sup>3</sup>) with 2 ft of freeboard and the ultimate capacity is 938,000 ft<sup>3</sup> (Figures 12 and 13). Overtopping of the reservoir is controlled by an alarm system and emergency shutoff system. Overtopping flows, in the event of system failure, are directed to the lined tailings conveyance trench to the lined tailings impoundment. Design criteria for the Process Water Reservoir are presented in Table 1.

#### **4.2 SURFACE WATER CONTROL**

Surface areas draining to the Impacted Stormwater Impoundments (A, B, and C), mine pit, and Process Water Reservoir will be shielded from run-on surface drainage by site diversions as described in a separate report.

#### **4.3 GEOLOGIC HAZARDS**

No geologic hazards are known to exist in the vicinities of the Impacted Stormwater Impoundments or Process Water Reservoir. Impacted Stormwater Impoundment B is located on the eastern wall of the ultimate mine pit (Figure 11J-3). In the event of a pit slope failure, any liquids contained in Impacted Stormwater Impoundment B would be contained in the mine pit.

#### **4.4 SOLUTION CHARACTERIZATION**

Liquids routinely expected to enter the Impacted Stormwater Impoundments (A, B, and C) are direct precipitation and stormwater runoff from areas impacted by mining activities including mining, hauling, waste rock stockpiling, mineral processing, and shipping and receiving of goods and products. The Impacted Stormwater Impoundments will be typically empty. Impacted stormwater collected in the impoundments will be pumped out and used as process makeup water.

Liquids routinely expected to enter the Process Water Reservoir include direct precipitation, water reclaimed from the Copper-Moly (Cu-Mo) Thickener, fresh make-up water from the water supply wellfield, and reclaimed water from the Tailings Impoundment and Underdrain Collection Pond. The Process Water Reservoir is typically maintained at a nearly full operational level at all times to ensure continuity of the process during short-term interruptions of return or makeup flows. The physical characteristics of these constituents are expected to be neutral to slightly alkaline and completely compatible with the liner materials. Flows from upset conditions in the concentrator do not flow directly to the Process Water Reservoir, but would eventually contribute to the water reclaimed from the Tailings Impoundment and Underdrain Collection Pond.

#### **4.5 CAPACITY AND STORAGE DESIGN**

The capacity and storage design of the subject impoundments and reservoir are provided in Table 1. The impacted water impoundments are designed to contain the runoff from a 100-year, 24-hour storm event with a minimum of 2 ft of freeboard.

The Process Water Reservoir is designed to contain the maximum design process flow plus stormwater runoff from the reservoir catchment area with a minimum of 2 ft of freeboard.

#### **4.6 SPILLWAY DESIGN**

Spillways for Impacted Stormwater Impoundments A, B, and C are designed to safely discharge the peak runoff of a 25-year, 24-hour precipitation event. The spillways for Impacted Stormwater Impoundments A and C are designed as open channel spillways with slopes that are suitable for vehicle access on the perimeter road. The spillway for Impacted Stormwater Impoundment B is designed as a culvert beneath the haul road. The culvert(s) will have sufficient capacity to safely pass peak runoff from the prescribed precipitation event.

Overflow protection for the process water reservoir is accomplished via a designed solution conveyance to the lined tailings conveyance trench, which conveys any upset flows that exceed the maximum capacity without compromising the integrity of the structure.

#### 4.7 SITE PREPARATION

The pond areas will be cleared and grubbed of vegetation. Any unsuitable foundation materials within the pond footprint will be excavated and replaced. Bedding soil will be placed, moisture conditioned, and compacted pursuant to 20.6.7.17.D.(3) and (4). The bedding soil must be free of sharp rock, vegetation, and stubble to a depth of at least 6 inches. The bedding surface must be smooth to ensure good contact between the liner and the bedding. The liner must be placed on a layer of sand or fine soil. The floor of the bedding surface will be sloped to collection sump at grades of up to 1 percent to facilitate removal of the contents. Side slopes will be less than 2H:1V to permit proper installation of the liner system. The liner bedding shall have an acceptance certificate prior to installation of the liner.

#### 4.8 LINER SYSTEMS

Pursuant to 20.6.7.17.D.(4), the liner system of the Impacted Stormwater Impoundments consists of a single 60 mil HDPE textured geomembrane liner that is certified as UV resistant in accordance with a Construction Quality Assurance and Construction Quality Control (CQA/CQC) Plan, which will be generated and approved prior to construction. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature. These impoundments are typically empty and the liner will be weighted to prevent wind damage. The liner shall be secured in an anchor trench (Figure 14, Detail 3).

Pursuant to 20.6.7.17.D. (3), the liner system for the Process Water Reservoir consists of a secondary liner, drainage layer, and primary liner. The drainage layer connects directly to the fluid collection sump and fluid removal system to alleviate the need for fluid collection pipes. This reservoir typically contains solution and will not require the liner to be weighted unless there is a prolonged period when the reservoir will be empty and susceptible to wind damage. The liner system shall be secured in an anchor trench (Figure 14, Detail 1).

The lower (secondary) liner consists of a single 60 mil HDPE geomembrane AGRU® drainage liner, or equivalent, that is installed in accordance with an approved CQA/CQC Plan. This type of secondary liner, paired with a primary liner, doubles as a drainage layer with a coefficient of permeability of  $1 \times 10^{-2}$  cm/s on a design slope of 2 percent. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature.

The primary liner for the Process Water Reservoir consists of a single 60 mil HDPE textured geomembrane liner that is certified as UV resistant and installed in accordance with an approved CQA/CQC Plan. Liner panels shall be oriented such that the seams on the sidewall of the impoundments are vertical. Sufficient slack in the liner will be maintained to accommodate expansion and contraction of the liner material due to changes in temperature.

A CQA/CQC plan will be developed by the design engineer and the liner installation contractor and for approval by the appropriate agency as part of the final design prior to construction. The plan includes the following elements.

- Identification of persons and entities responsible for overseeing the program.
- Inspection protocols for subgrade, materials, placement, anchoring, welding, testing, and repairing.
- Identification of field and laboratory testing equipment and testing entities.
- Procedures for observing and testing liner, subgrade, bedding, etc.
- Verification protocol for manufacturer's QC testing.
- Procedures for reviewing results of testing and inspection.
- Corrective actions for material repair, subgrade and bedding deficiencies, weld testing failures, or other construction defects.
- Seaming procedures, qualification, testing, and inspection.
- QA/QC reporting procedures, schedules, and certifications.

- Guidelines, schedules, contents, and certifications for submission of a CQA/CQC report.

#### 4.9 LEAK COLLECTION SYSTEM

Pursuant to 20.6.7.17.D.(3).(d), the liner drainage layer of the process water reservoir discharges directly into a leakage collection sump (Figure 14, Detail 6) which is part of the fluid removal system. The sump contains granular fill materials to convey the drainage fluid to the fluid removal pipe and pump system. The fluid removal pipe consists of a 6" Sch. 80 polyvinyl chloride (PVC) pipe with 3 ft of slotted screen at the bottom for water collection. The fluid removal pipe can be cleaned using conventional pipe cleaning equipment. An automated fluid removal pump is installed at the bottom of the pipe to enable removal of leakage. The pump is activated in the presence of drainage fluid in the sump and is turned off when the fluid has been removed. A totalizing flow meter records the volume of fluid removed from the sump. The pump also has an alarm system to notify the operator of system failure.

#### 4.10 PERFORMANCE INSPECTIONS AND OPERATIONAL MONITORING

Routine inspections of the Process Water Reservoir and Impacted Stormwater Impoundments begin at the time of construction and proceed quarterly. Additional inspections are prescribed in the event of a process upset or a significant stormwater flow event. Inspections include visual assessment of integrity and physical assessment of pond capacity. Water levels in the ponds are noted with respect to the freeboard. Totalizing meter readings are recorded from fluid removal pumps from the leakage collection sump and from the impacted stormwater impoundment pumps.

## TABLES

Table 1: Impoundment Design Criteria

Impoundment ID	Stormwater Impoundment A	Stormwater Impoundment B	Stormwater Impoundment C	Process Water Reservoir
Catchment Area (ac)	91.06	98.52	198.66	1.80
Peak Flow, Q100 (cfs) <sup>1</sup>	200.25	176.88	315.76	9.54
Pond Size - Approx, Surface area (ac)	1.98	2.12	6.37	1.80
Freeboard Requirement (ft)	2.0	2.0	2.0	2.0
Capacity at Freeboard (ft <sup>3</sup> )	976,800	748,400	1,405,500	726,400
Design Capacity at spillway/crest (ft <sup>3</sup> )	1,280,500	913,200	1,802,100	938,000
Primary Liner Specifications <sup>2</sup>	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent 6" fine soil subgrade Certified UV resistant	60 mil HDPE or equivalent Certified UV resistant
Secondary Liner Specifications <sup>2</sup>	N/A	N/A	N/A	60 mil HDPE or equivalent 6" fine soil subgrade
Drainage Layer Specifications <sup>3</sup>	N/A	N/A	N/A	Geonet drainage layer Slope min. 2% Perm. min. 1 x 10 <sup>-2</sup> cm/s
Perforated Fluid Collection System <sup>4</sup>	N/A	N/A	N/A	Geonet drainage layer
Fluid Removal System <sup>2</sup>	N/A	N/A	N/A	Automatic pump Totalizing flow meter Automated failure alarm
Design Flow for Conveyance Structures (cfs)	Q100 = 200.25	Q100 = 176.88	Q100 = 315.76	Q100 = 9.54
Design Storm for Pond & Source	100-yr, 24hr rainfall event, WS A	100-yr, 24hr rainfall event, WS B	100-yr, 24hr rainfall event, WS C	100-yr, 24hr rainfall event, WS E
Design Storm for Spillway & Source	200-yr, 24hr rainfall event, WS A	200-yr, 24hr rainfall event, WS B	200-yr, 24hr rainfall event, WS C	N/A
Peak Flow, Q200 for Spillway (cfs) <sup>5</sup>	6.16	6.37	8.80	N/A
Bank Slopes	2:1 (H:V) Max	2:1 (H:V) Max	2:1 (H:V) Max	2:1 (H:V) Max

<sup>1</sup> Precipitation data is per NOAA Atlas 14, Volume 1, Version 5; Hillsboro, NM, Station ID: 29-4009

<sup>2</sup> Specifications are per 20.6.7.17.D.(3) and (4)(c)

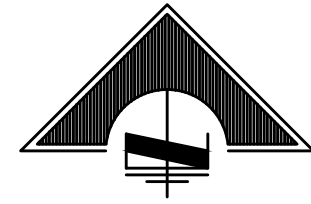
<sup>3</sup> Specifications are per 20.6.7.17.D.(3)(d)

<sup>4</sup> Geonet layer drains directly into collection sump and fluid removal system

<sup>5</sup> Design Flow for spillway is approximate flow from pond to spillway during the 200-yr event assuming the spillway elevation is at the 100-yr WSEL



## FIGURES



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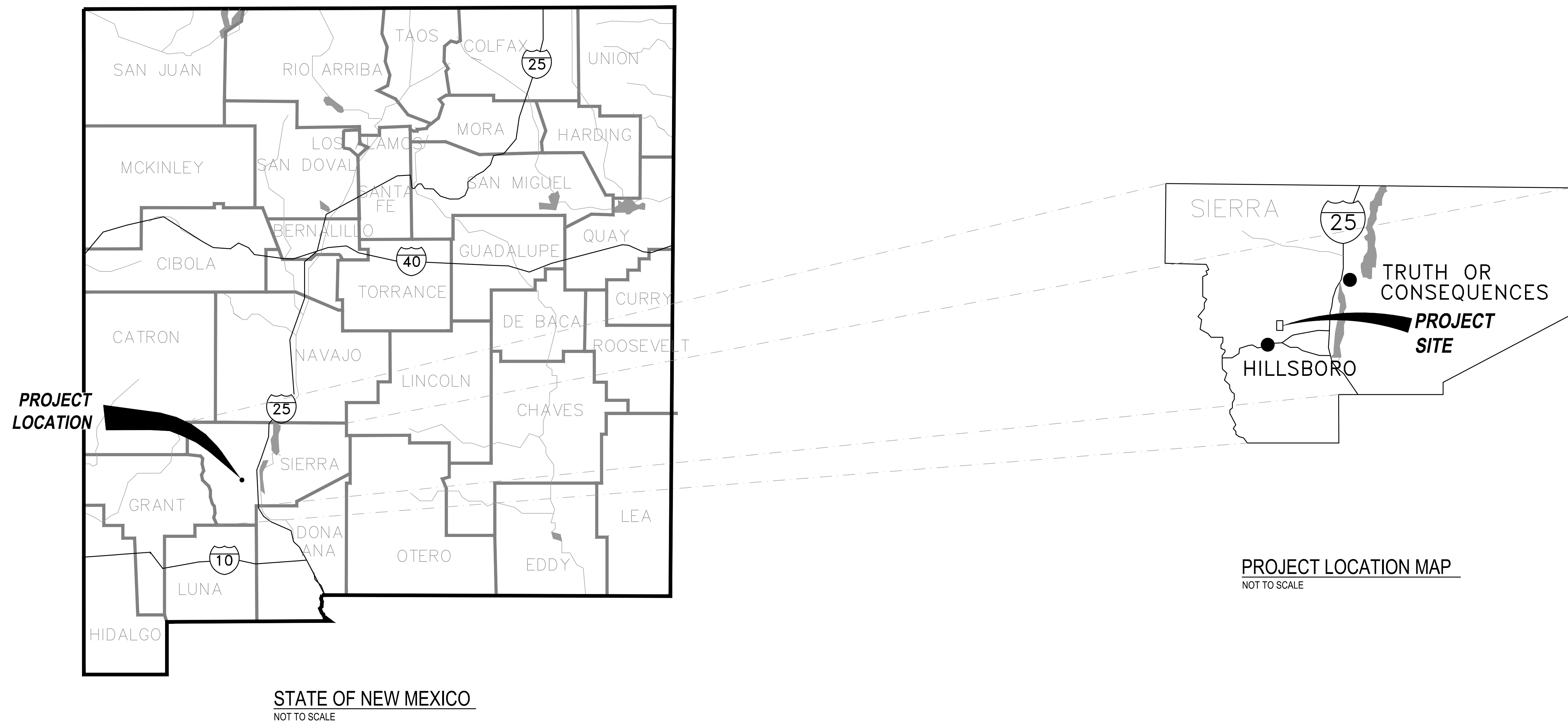


FIGURE 1

**PRELIMINARY**  
FOR AGENCY REVIEW



DO NOT SCALE 11x17 DRAWINGS

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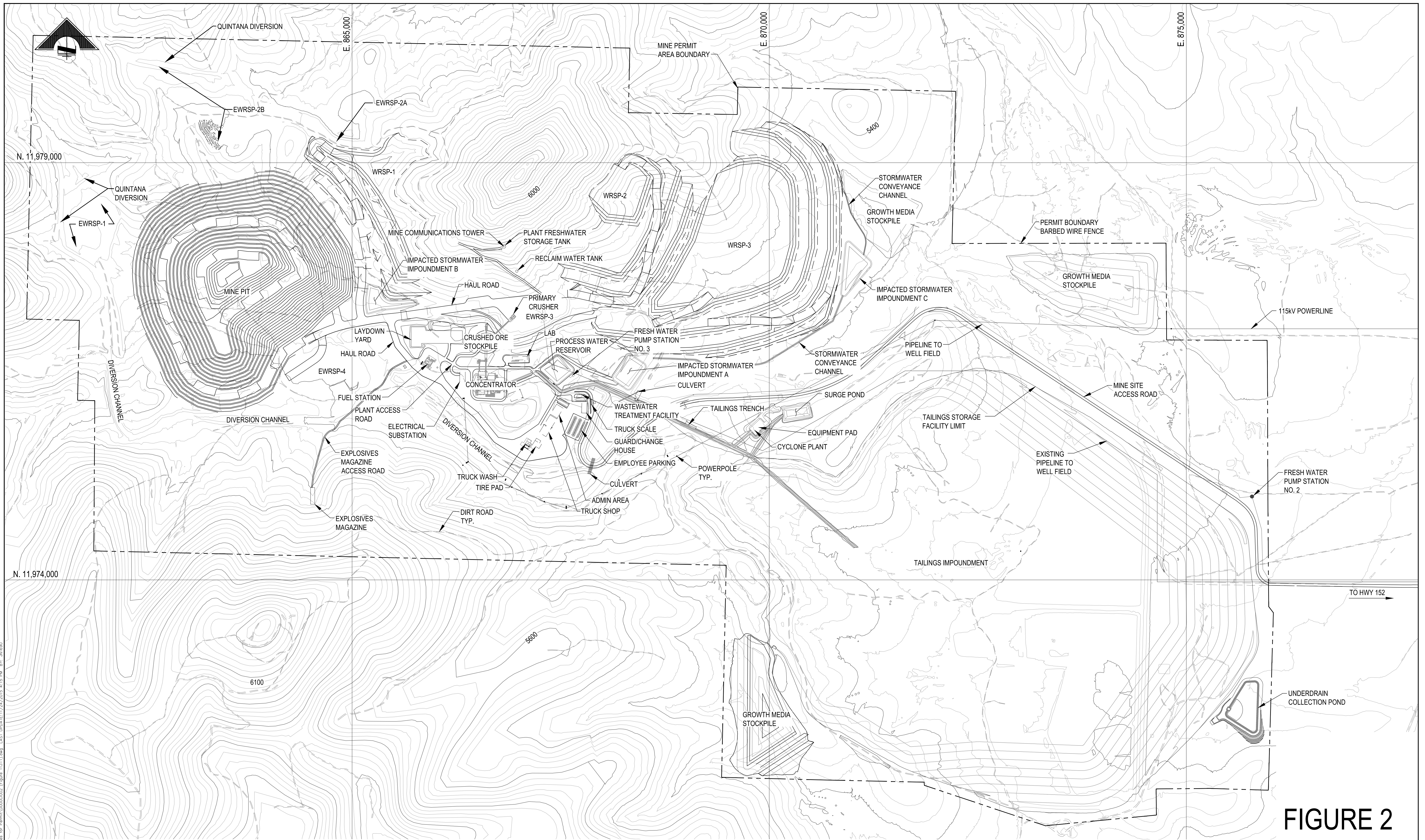
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**COPPER FLAT PROJECT**

**SITE GENERAL  
CIVIL  
PROJECT AREA  
SITE LOCATION PLAN**

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-001**  
REV. NO. P4 DATE 05 MAR 13

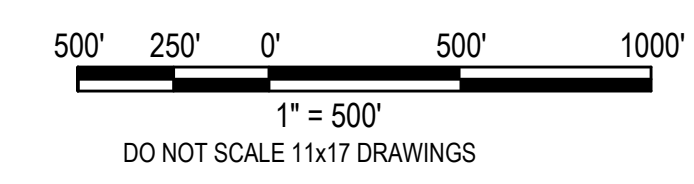
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**FIGURE 2**

**EWRSP = EXISTING WASTE ROCK STOCKPILE**  
**WRSP = WASTE ROCK STOCKPILE**

**SITE PLAN**  
 SCALE: 1:500



**PRELIMINARY**  
 FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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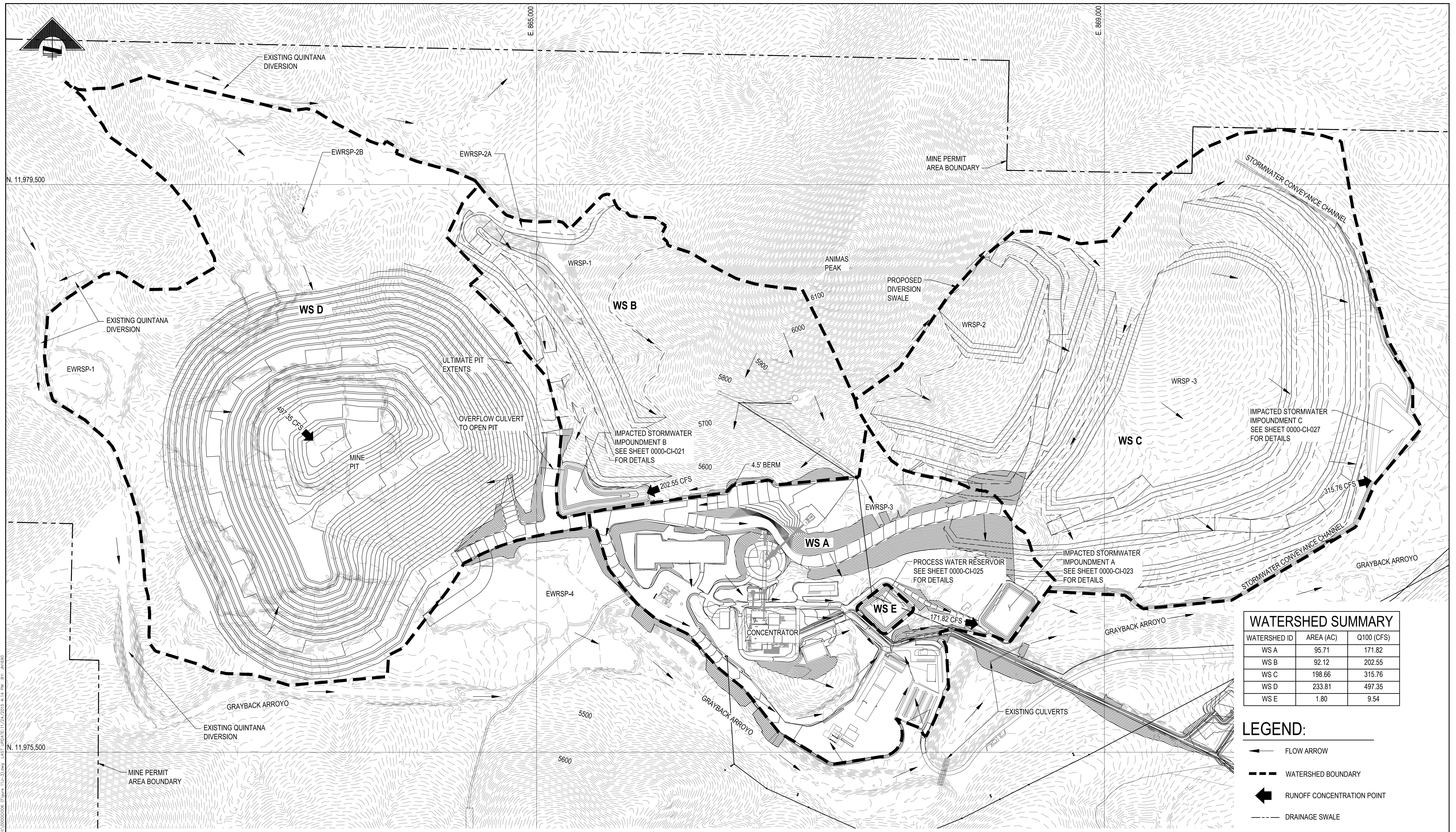
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**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL PROJECT AREA PROPOSED SITE PLAN**

JOB NO. M3 PN-120085  
 DWG. NO. **FIGURE 11J-1**  
 REV. NO. P18 DATE 16 NOV 15

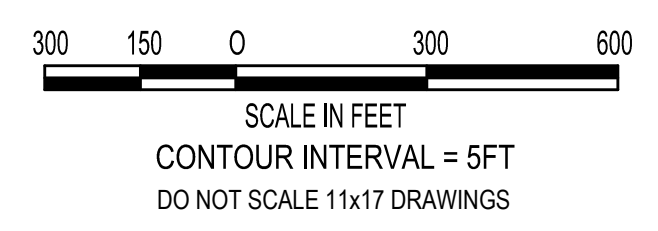
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WATERSHED SUMMARY		
WATERSHED ID	AREA (AC)	Q100 (CFS)
WS A	95.71	171.82
WS B	92.12	202.55
WS C	198.66	315.76
WS D	233.81	497.35
WS E	1.80	9.54

- LEGEND:**
- FLOW ARROW
  - - - WATERSHED BOUNDARY
  - ◀ RUNOFF CONCENTRATION POINT
  - - - DRAINAGE SWALE

**PLAN VIEW**  
SCALE: 1" = 300'



**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 3**



**EWRSP = EXISTING WASTE ROCK STOCKPILE**  
**WRSP = WASTE ROCK STOCKPILE**

REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT
0000-CI-021	IMPACTED STORMWATER IMPOUNDMENT A PLAN VIEW														
0000-CI-023	IMPACTED STORMWATER IMPOUNDMENT B PLAN VIEW														
0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT D PLAN VIEW														

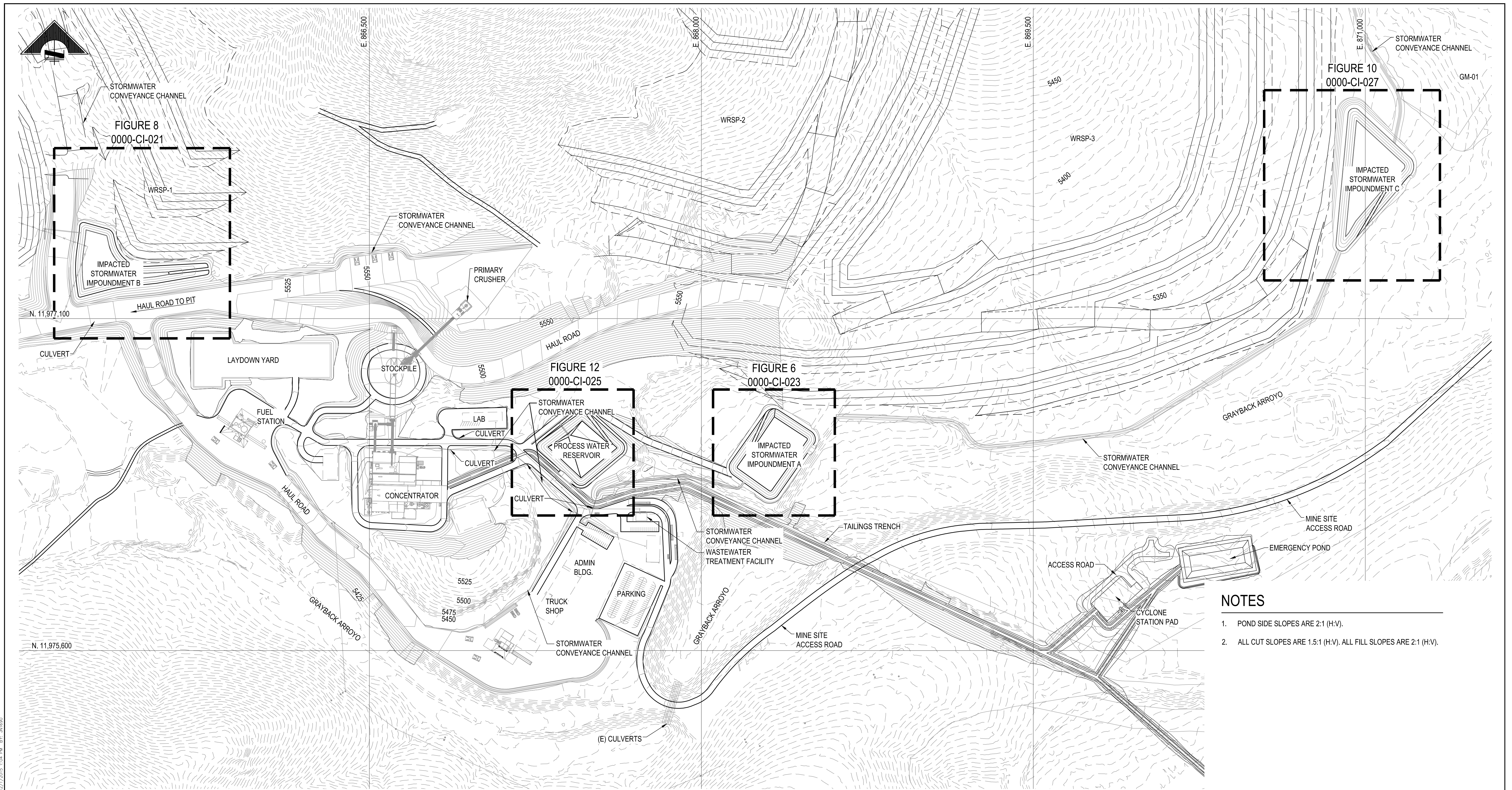
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**DEVELOPED WATERSHED AREAS**

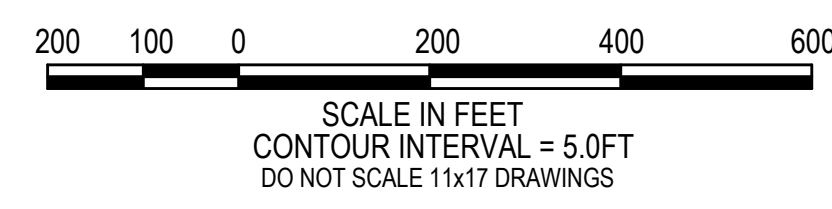
JOB NO. M3 PN-120085  
DWG NO. **FIGURE 11J-3**  
REV. NO. P10 DATE 29 OCT 15

File: E:\2012\200808\Civil\0443\442 - Design\Drawings for Permit\0000000006 - Figure 11J-3.dwg, LAST UPDATED: 11/29/2015, 4:14 PM, BY: ANR89



**SITE PLAN**  
SCALE: 1:200

- NOTES**
1. POND SIDE SLOPES ARE 2:1 (H:V).
  2. ALL CUT SLOPES ARE 1.5:1 (H:V); ALL FILL SLOPES ARE 2:1 (H:V).



**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 4**



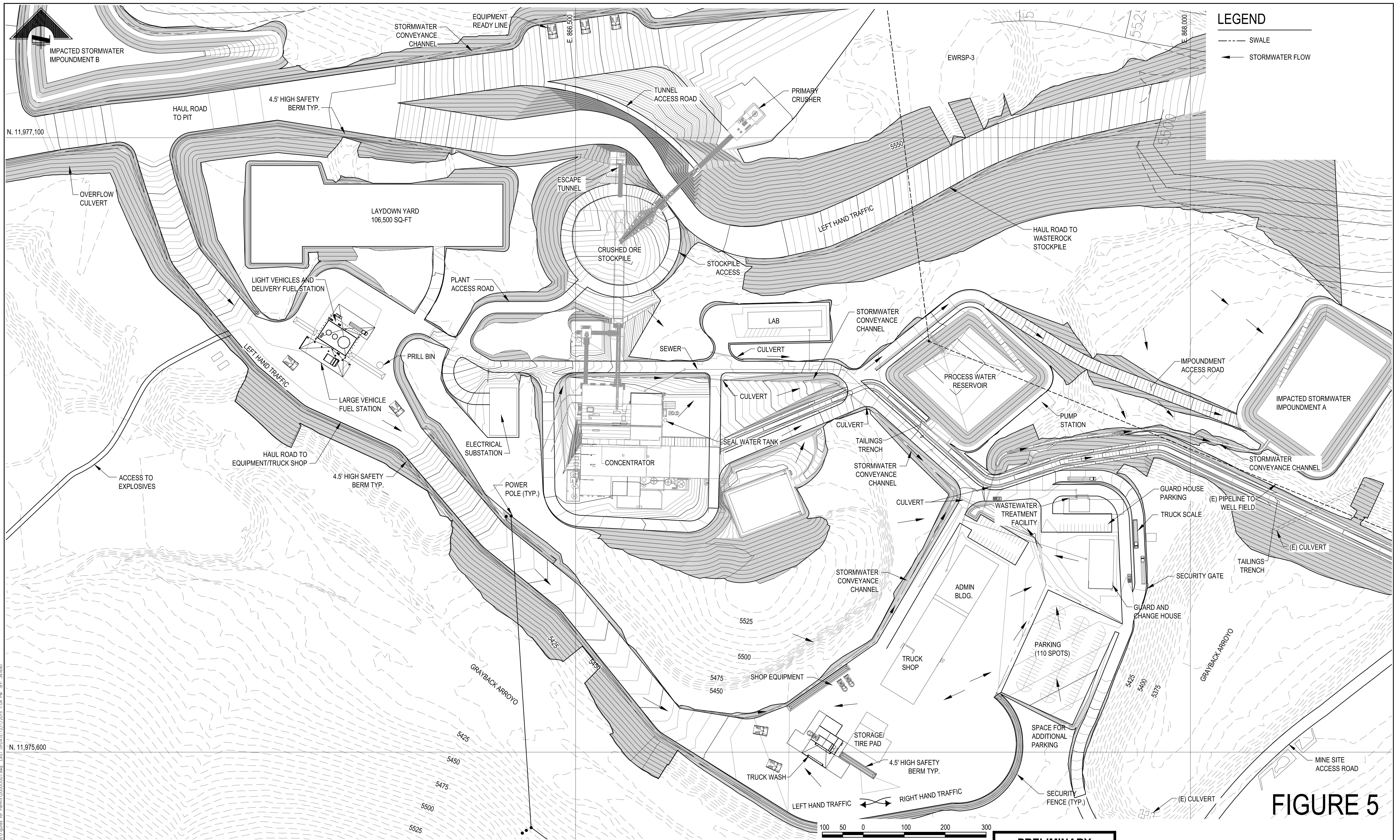
DWG. NO.	TITLE
0000-CV-021	IMPACTED STORMWATER IMPOUNDMENT B, PLAN VIEW
0000-CV-023	IMPACTED STORMWATER IMPOUNDMENT A, PLAN VIEW
0000-CV-025	PROCESS WATER RESERVOIR PLAN VIEW
0000-CV-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW

REFERENCES		REFERENCES		REVISIONS							REVISIONS						
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT		

SCALE: AS NOTED  
 DESIGNED BY: SAM JUL 15  
 DRAWN BY: SAM JUL 15  
 CHECKED BY: JPN JUL 15  
 PROJECT MGR: RKZ JUL 15  
 CLIENT APPR:   
 ARCHITECTURE  
 ENGINEERING  
 CONSTRUCTION MANAGEMENT  
 www.m3eng.com

**COPPER FLAT PROJECT**  
**SITE GENERAL CIVIL**  
**STORMWATER & PROCESS WATER PONDS OVERALL PLAN**  
 JOB NO. M3 PN-120085  
 DWG. NO. **0000-CI-020**  
 REV. NO. P3 DATE 09 OCT 15

File: E:\3000\300000\Civil\04415442 - Design\Drawings for Permit\00000000.dwg, LAST UPDATE: 12/7/2015 1:04 PM, BY: JNK90



**LEGEND**  
 --- SWALE  
 ← STORMWATER FLOW

**SITE PLAN**  
 SCALE: 1:100

SCALE IN FEET  
 PROPOSED CONTOUR INTERVAL = 1.0FT  
 EXISTING CONTOUR INTERVAL = 5.0FT  
 DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
**FOR AGENCY REVIEW**

**FIGURE 5**

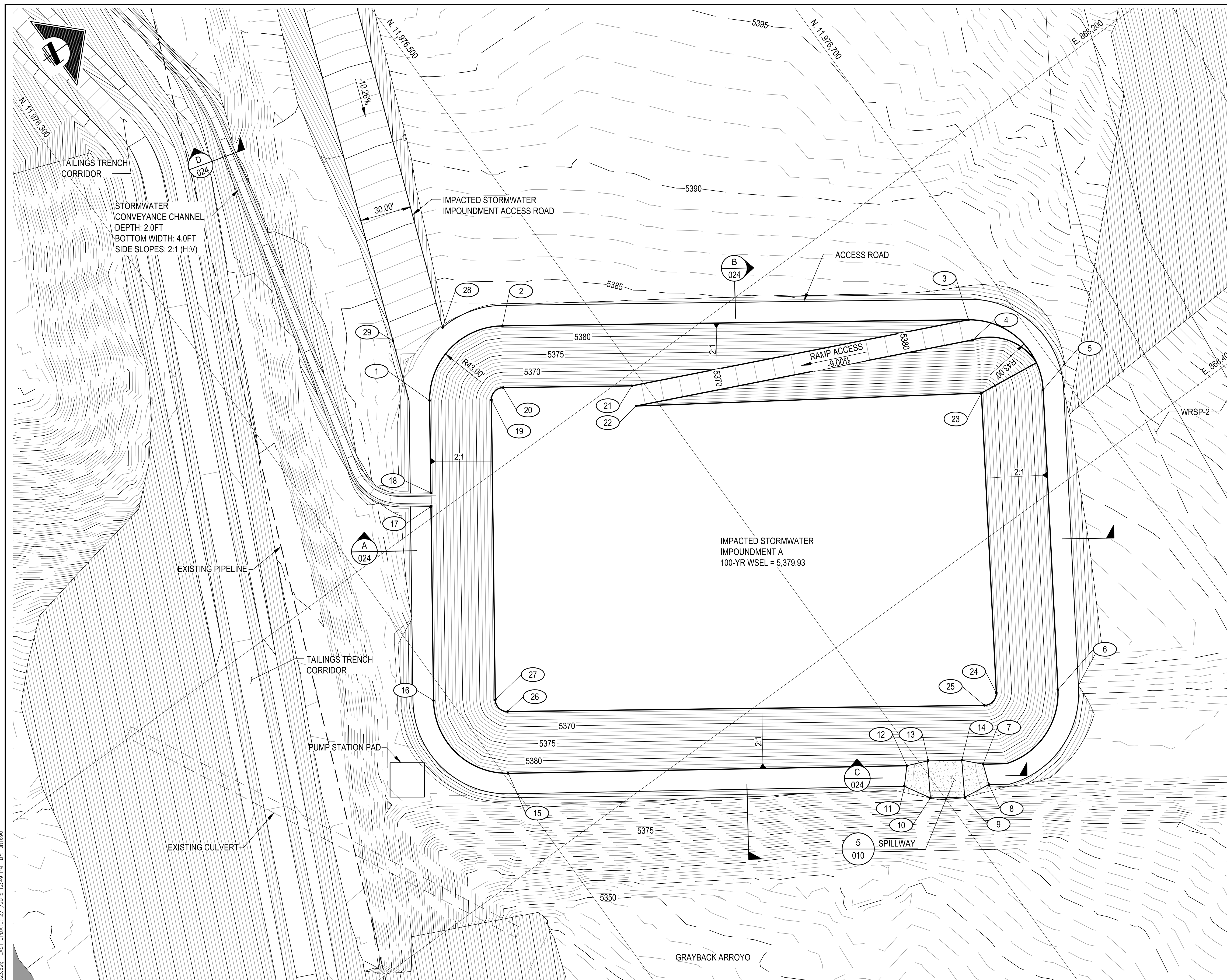
**THEMAC**  
 RESOURCES

REFERENCES		REFERENCES		REVISIONS				REVISIONS							
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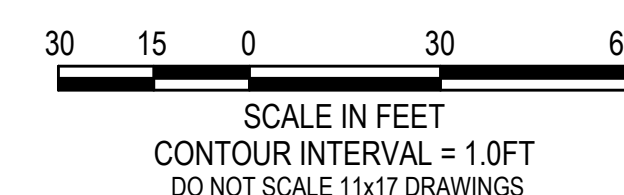
	ARCHITECTURE
	ENGINEERING
	CONSTRUCTION MANAGEMENT
	www.m3eng.com

<b>COPPER FLAT PROJECT</b>		JOB NO. M3 PN-12085
<b>SITE GENERAL CIVIL PROJECT AREA PROCESS AREA SITE PLAN</b>		DWG NO. <b>0000-CI-007</b>
		REV NO. P9 DATE 09 OCT 15

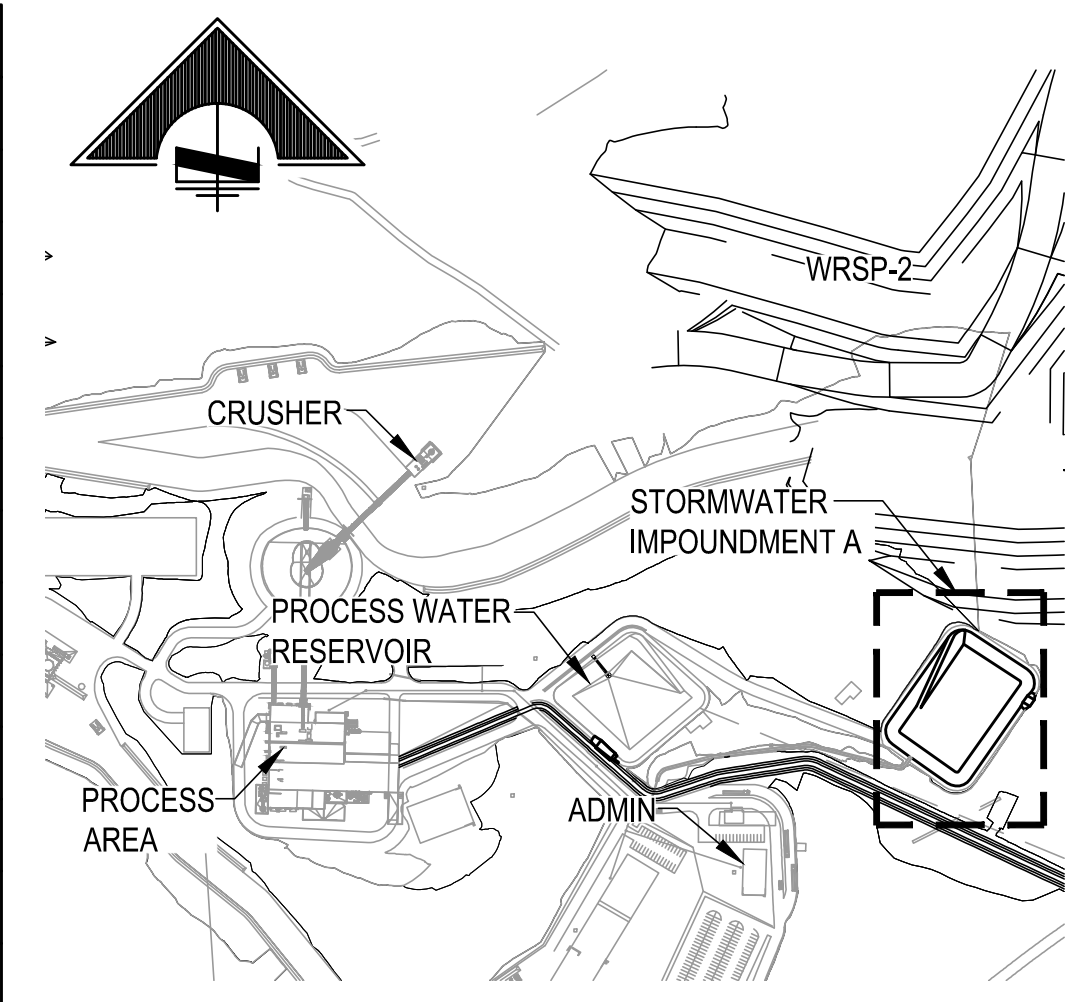
File: P:\2013\200805\Civil\_0441\5442 - Design\Drawings for Permit\00000007.dwg, LAST UPDATE: 12/7/2015 1:04 PM, BY: JNK90



**PLAN**  
SCALE: 1" = 30'



POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION
1	11976393.13	868144.02	5383.50
2	11976453.06	868133.72	5383.50
3	11976675.02	868290.57	5383.50
4	11976670.08	868301.51	5383.50
5	11976686.19	868349.21	5383.50
6	11976590.39	868495.80	5383.50
7	11976529.56	868505.28	5383.50
8	11976525.25	868516.90	5383.50
9	11976509.58	868514.53	5381.96
10	11976493.13	868503.16	5381.96
11	11976484.86	868488.73	5383.50
12	11976493.10	868479.86	5383.50
13	11976504.45	868484.69	5382.18
14	11976520.85	868496.13	5382.18
15	11976302.43	868346.86	5383.50
16	11976292.13	868286.93	5383.50
17	11976357.47	868194.48	5383.50
18	11976362.08	868187.95	5383.50
19	11976422.53	868164.79	5365.50
20	11976432.28	868163.12	5365.50
21	11976493.65	868206.42	5365.50
22	11976488.60	868217.31	5365.50
23	11976656.01	868329.59	5365.50
24	11976560.26	868476.10	5365.50
25	11976550.36	868477.99	5365.50
26	11976323.21	868317.47	5365.50
27	11976321.53	868307.71	5365.50
28	11976424.35	868113.97	5383.50
29	11976396.34	868103.21	5383.50



**KEY MAP**  
SCALE: 1" = 500'

**NOTES:**

1. STORMWATER IMPOUNDMENT A IS INTENDED TO CAPTURE STORMWATER RUNOFF FROM THE MINE SITE PROCESS AREA.
2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
3. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60ML HDPE LINER, PER DETAIL 4 ON SHEET 0000-CI-010, OR APPROVED EQUAL.
4. STORMWATER SPILLWAY IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT MINIMUM.
5. SPILLWAY IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

**IMPOUNDMENT SUMMARY**

CAPACITY AT 100-YR WSEL	976,772	CU-FT
ULTIMATE CAPACITY	1,280,516	CU-FT

**FIGURE 6**

**PRELIMINARY**  
FOR AGENCY REVIEW



File: P:\2013\20130803\Civil - (441)\441 - Dept\Updates for Permit\0000-CI-023.dwg LAST UPDATE: 12/7/2015 12:49 PM BY: JN0690

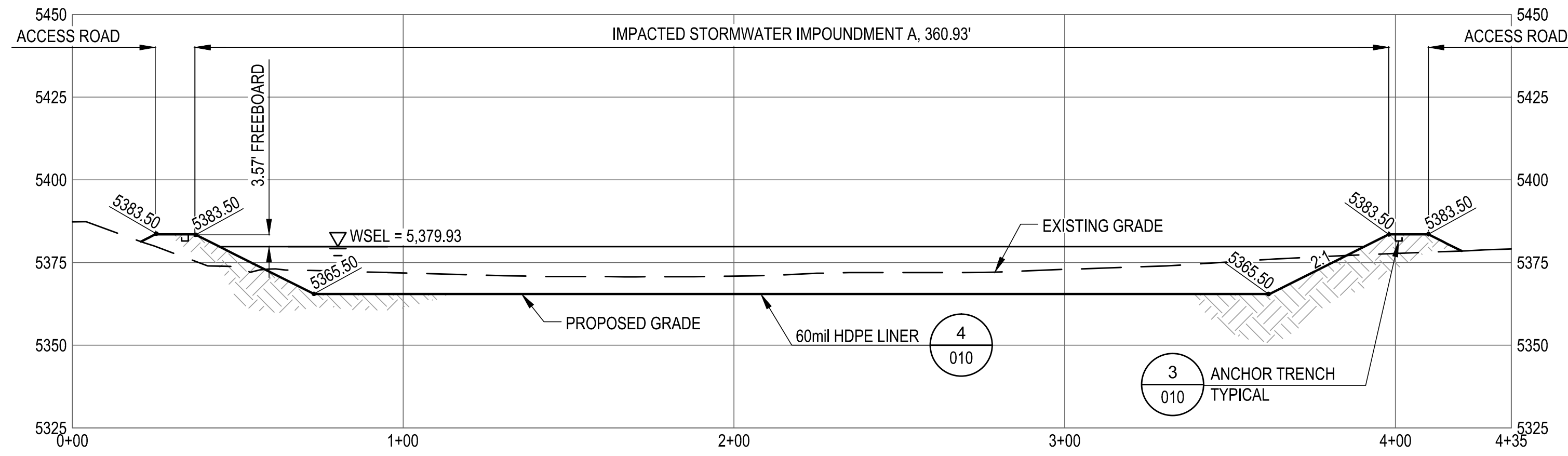
REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED			
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0000-CI-024	IMPACTED STORMWATER IMPOUNDMENT A, SECTIONS														
0000-CI-010	STANDARD DETAILS SHEET 3														

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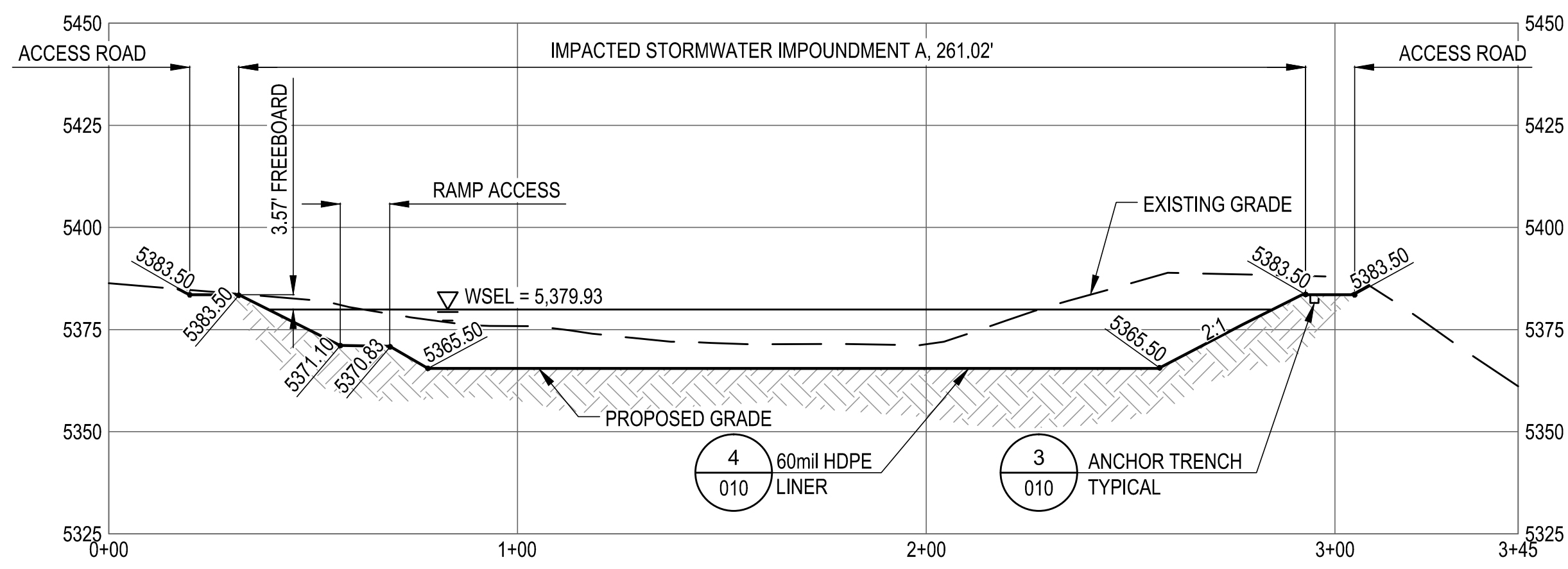
**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**IMPACTED STORMWATER IMPOUNDMENT A**  
**PLAN VIEW**

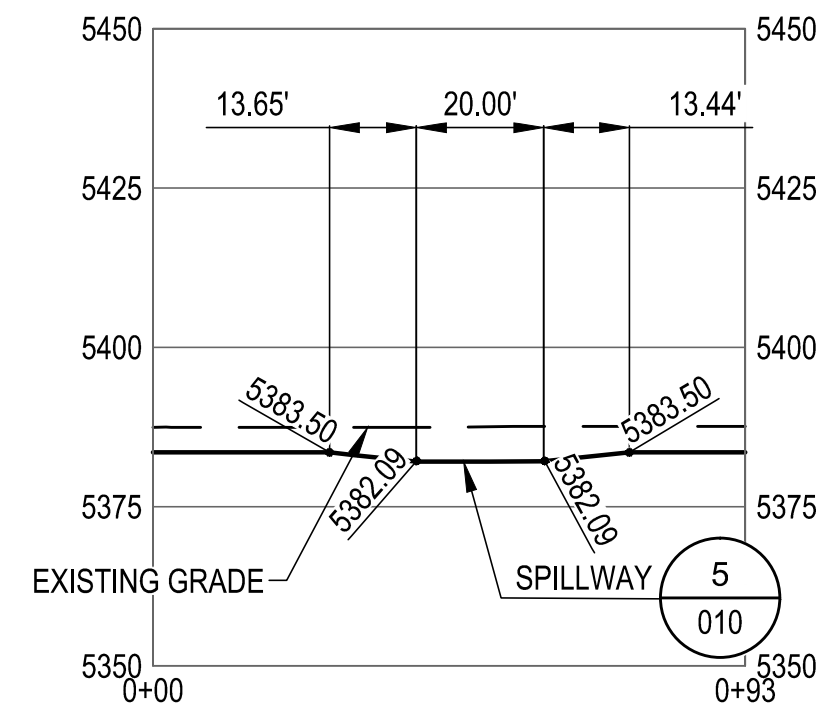
JOB NO. M3 PN-120085  
DWG NO. **0000-CI-023**  
REV NO. P3 DATE 09 OCT 15



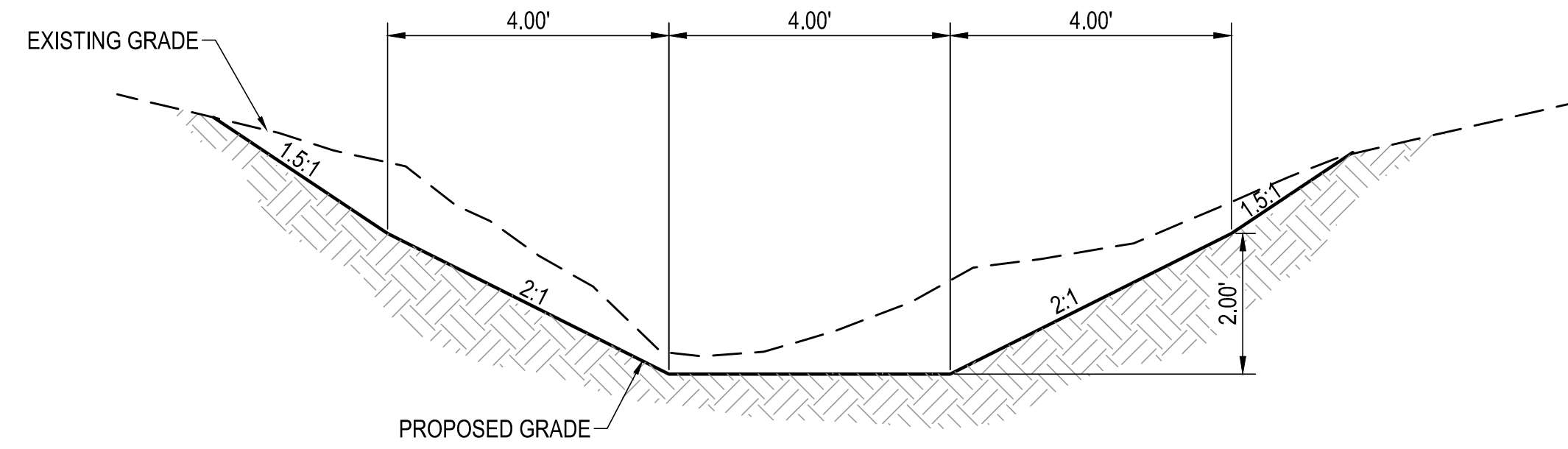
SECTION A  
SCALE: 1" = 30'



SECTION B  
SCALE: 1" = 30'



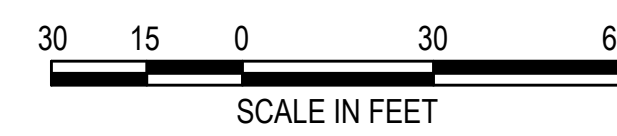
SECTION C  
SCALE: 1" = 30'



SECTION D  
SCALE: NTS

**NOTES:**

1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 7**



File: P:\2023\230808\Civil - (644)\544.2 - Dept\Updates for Permit\0000-CI-024.dwg, LAST UPDATE: 12/7/2015 11:20 AM, BY: AN090

REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED			
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0000-CI-010	STANDARD DETAILS SHEET 3														

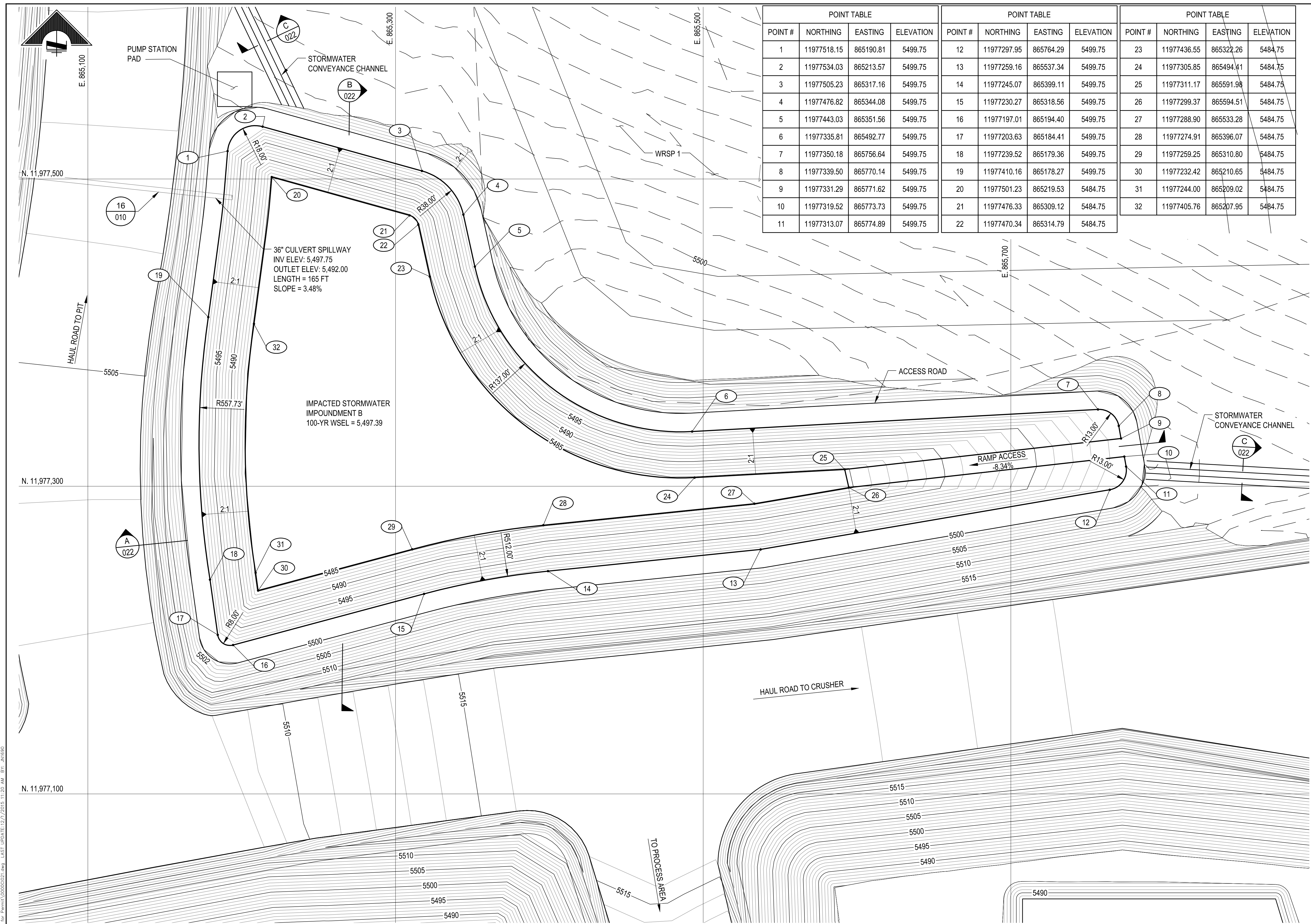
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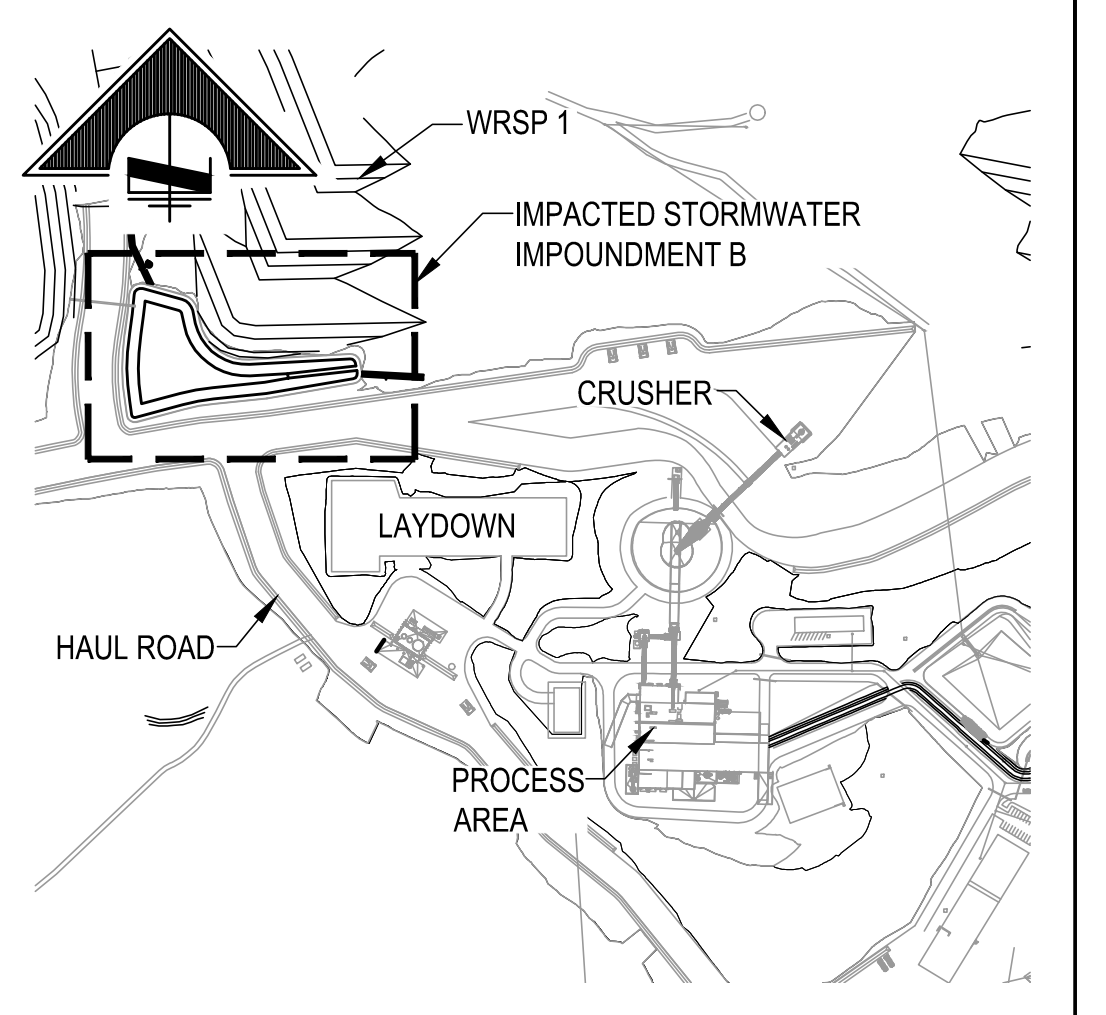
**GENERAL SITE CIVIL**  
**IMPACTED STORMWATER IMPOUNDMENT A SECTIONS**

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-024**  
REV. NO. P3 DATE 09 OCT 15





POINT TABLE				POINT TABLE				POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION	POINT #	NORTHING	EASTING	ELEVATION	POINT #	NORTHING	EASTING	ELEVATION
1	11977518.15	865190.81	5499.75	12	11977297.95	865764.29	5499.75	23	11977436.55	865322.26	5484.75
2	11977534.03	865213.57	5499.75	13	11977259.16	865537.34	5499.75	24	11977305.85	865494.41	5484.75
3	11977505.23	865317.16	5499.75	14	11977245.07	865399.11	5499.75	25	11977311.17	865591.98	5484.75
4	11977476.82	865344.08	5499.75	15	11977230.27	865318.56	5499.75	26	11977299.37	865594.51	5484.75
5	11977443.03	865351.56	5499.75	16	11977197.01	865194.40	5499.75	27	11977288.90	865533.28	5484.75
6	11977335.81	865492.77	5499.75	17	11977203.63	865184.41	5499.75	28	11977274.91	865396.07	5484.75
7	11977350.18	865756.64	5499.75	18	11977239.52	865179.36	5499.75	29	11977259.25	865310.80	5484.75
8	11977339.50	865770.14	5499.75	19	11977410.16	865178.27	5499.75	30	11977232.42	865210.65	5484.75
9	11977331.29	865771.62	5499.75	20	11977501.23	865219.53	5484.75	31	11977244.00	865209.02	5484.75
10	11977319.52	865773.73	5499.75	21	11977476.33	865309.12	5484.75	32	11977405.76	865207.95	5484.75
11	11977313.07	865774.89	5499.75	22	11977470.34	865314.79	5484.75				



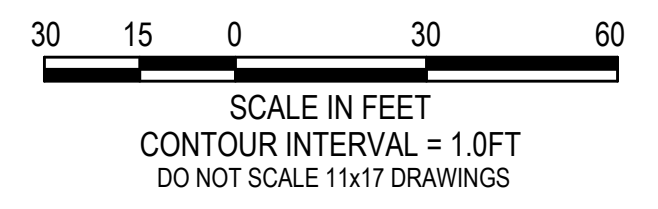
**KEY MAP**  
SCALE: 1" = 500'

**NOTES:**

1. STORMWATER IMPOUNDMENT B IS INTENDED TO CAPTURE STORMWATER RUNOFF FROM THE WASTE ROCK STOCK PILE 1 (WRSP-1).
2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
3. STORMWATER CULVERT SPILLWAY IS DESIGNED FOR THE 25-YR, 24HR RAINFALL EVENT AT MINIMUM.
4. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60mil HDPE LINER PER DETAIL 4 ON DWG. 0000-CI-010, OR APPROVED EQUIVALENT.

IMPOUNDMENT SUMMARY		
CAPACITY AT 100-YR WSEL	748,445	CU-FT
ULTIMATE CAPACITY	913,160	CU-FT

**PLAN**  
SCALE: 1" = 30'



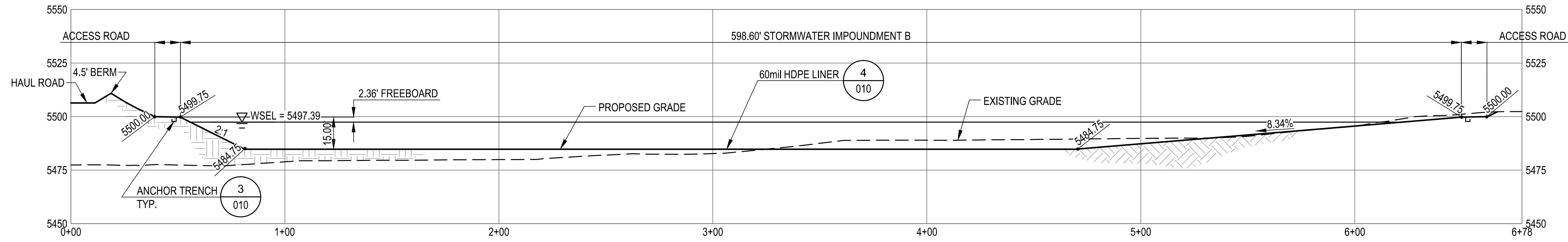
**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 8**

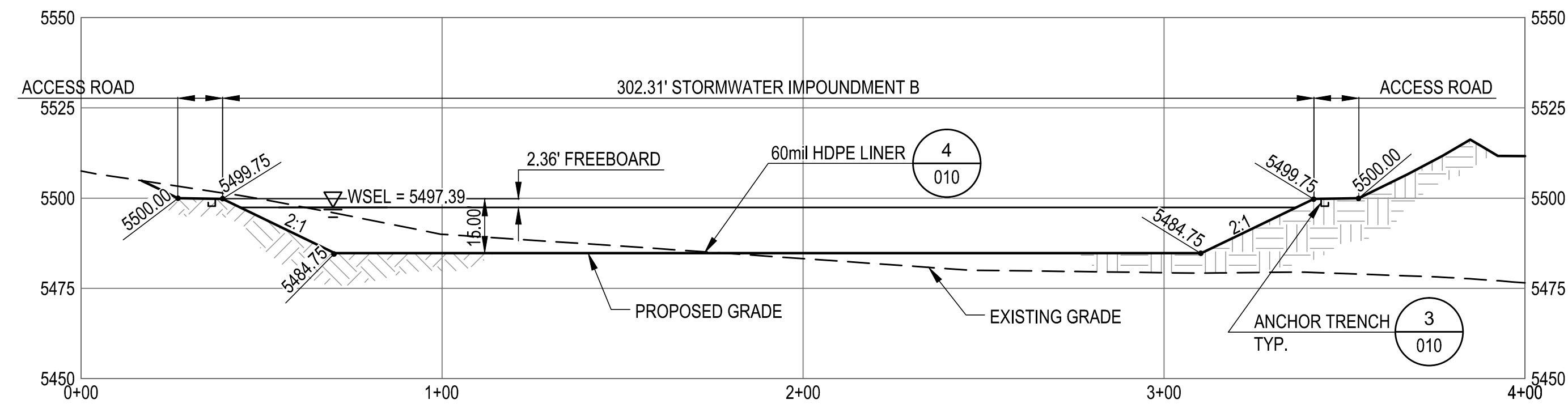


REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED		DATE		ARCHITECTURE		ENGINEERING		CONSTRUCTION MANAGEMENT		COPPER FLAT PROJECT		JOB NO. M3 PN-120085	
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	DESIGNED BY	DATE	ARCHITECTURE	ENGINEERING	CONSTRUCTION MANAGEMENT	GENERAL SITE CIVIL	IMPACTED STORMWATER IMPOUNDMENT B	PLAN VIEW	DWG. NO.	DATE
0000-CI-022	IMPACTED STORMWATER IMPOUNDMENT B SECTIONS															SAM	JUN 15	3						0000-CI-021	27 OCT 15
0000-CI-005	STANDARD DETAILS SHEET 2															SAM	JUN 15								
																JPN	JUN 15								
																RKZ	JUN 15								

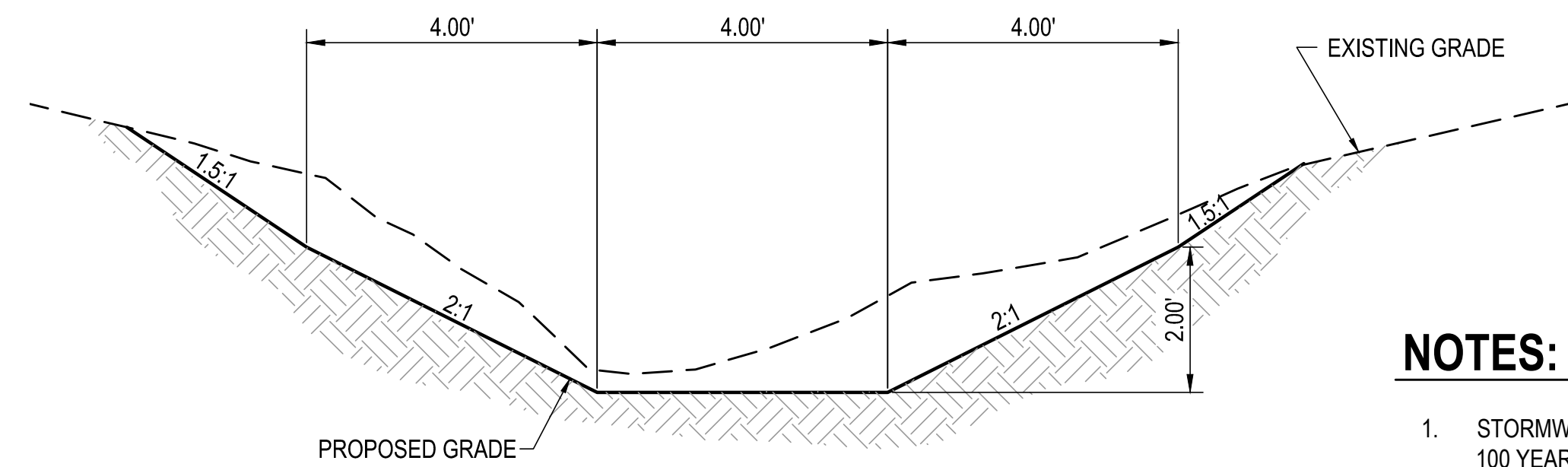
File: P:\2013\200808\Civil\0441\544.2 - Imp\Updates for Permit\00000201.dwg LAST UPDATE: 12/17/2015 11:20 AM BY: JN1090



SECTION A  
SCALE: 1" = 30'



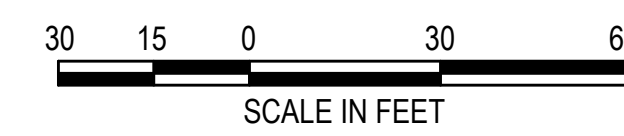
SECTION B  
SCALE: 1" = 30'



SECTION C  
SCALE: 1" = 30'

**NOTES:**

1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
NOT FOR CONSTRUCTION

**FIGURE 9**

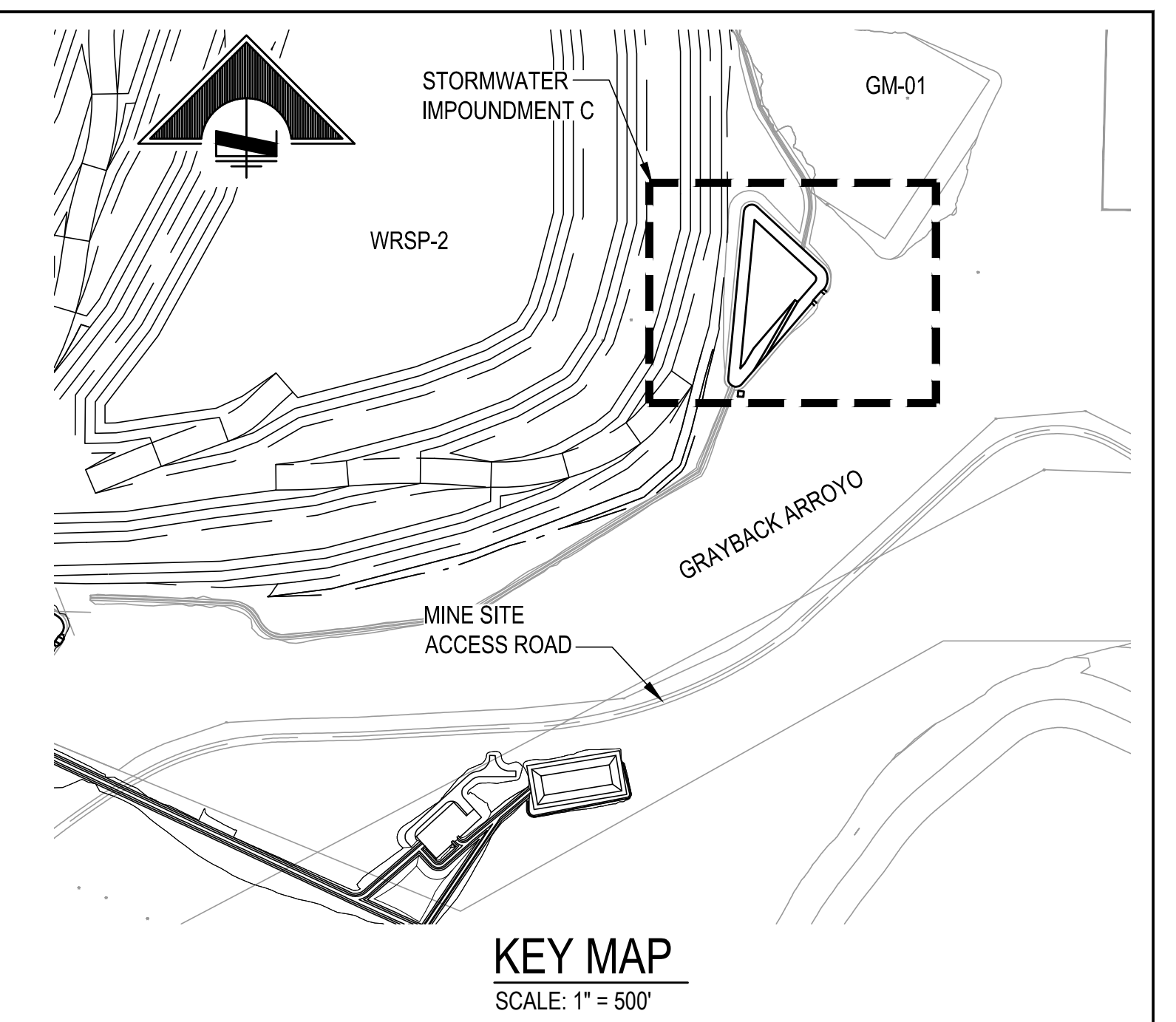
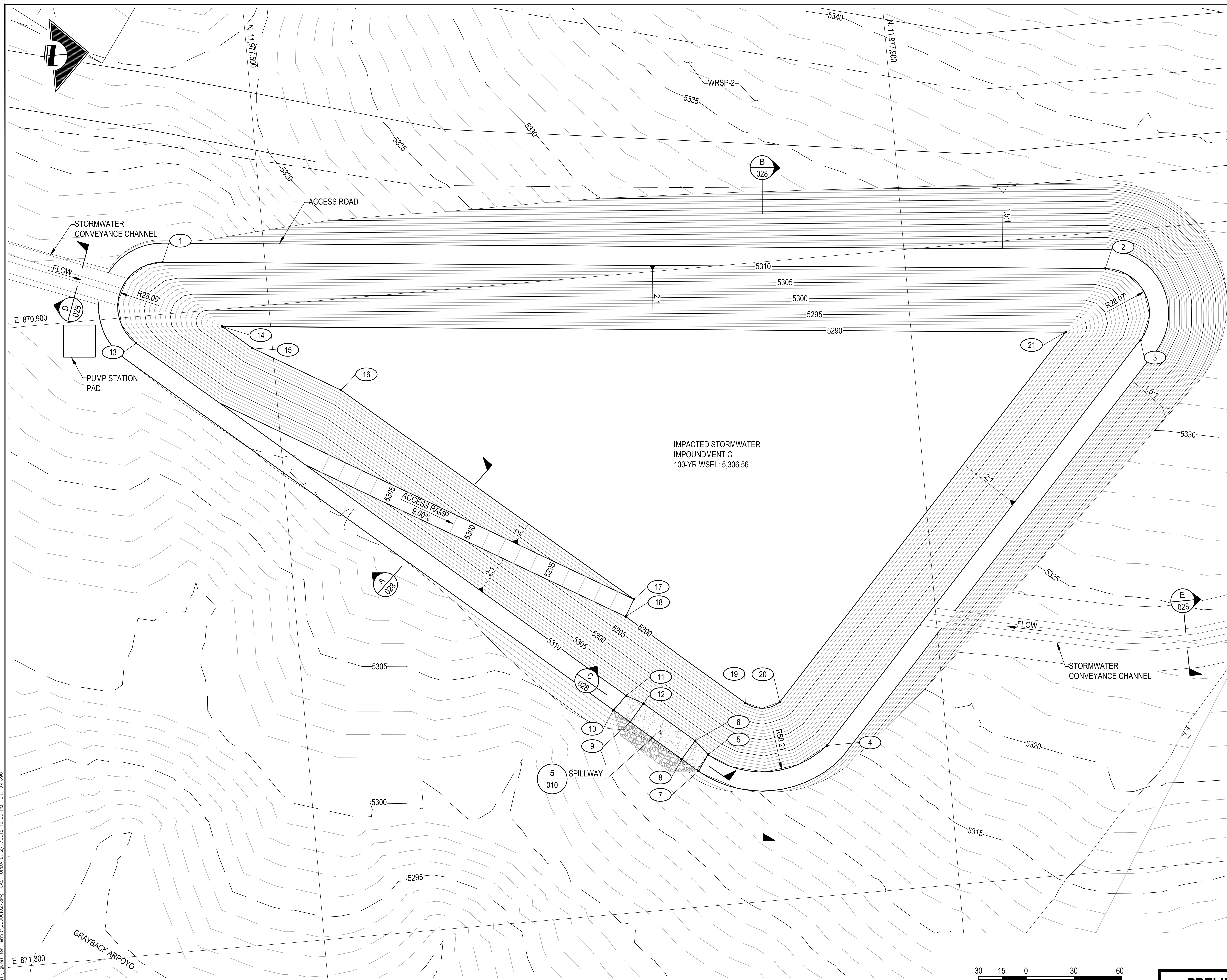


File: P:\2013\200809\Civil (644)\544.2 - Impacted Stormwater Impoundment B.dwg, LAST UPDATE: 12/7/2015 11:20 AM, BY: AN090

REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: AS NOTED			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT
0000-CI-021	IMPACTED STORMWATER IMPOUNDMENT B PLAN VIEW														
0000-CI-010	STANDARD DETAILS SHEET 3														

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**COPPER FLAT PROJECT**  
**GENERAL SITE CIVIL IMPACTED STORMWATER IMPOUNDMENT B SECTIONS**  
JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-022**  
REV. NO. P3 DATE 09 OCT 15

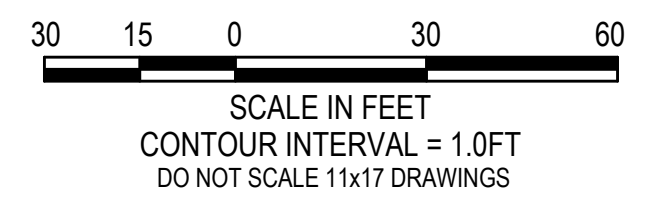


- NOTES:**
1. IMPACTED STORMWATER IMPOUNDMENT C IS INTENDED TO COLLECT AND RETAIN STORMWATER RUNOFF FROM WASTE ROCK STOCKPILE 2 (WRSP-2) AND WASTE ROCK STOCKPILE 3 (WRSP-3).
  2. STORMWATER IMPOUNDMENT IS SIZED TO CONTAIN THE 100-YR, 24-HR RAINFALL EVENT WITH A MINIMUM OF 2.0 FEET OF FREEBOARD.
  3. STORMWATER IMPOUNDMENT SHALL BE SINGLE LINED WITH 60mil HDPE PER DETAIL 4 ON SHEET 0000-CI-010, OR APPROVED EQUIVALENT.
  4. STORMWATER SPILLWAY IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT MINIMUM.
  5. SPILLWAY IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

IMPOUNDMENT SUMMARY			
CAPACITY AT 100-YR WSEL	1,405,507	CU-FT	
ULTIMATE CAPACITY	1,802,067	CU-FT	

POINT TABLE				POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION	POINT #	NORTHING	EASTING	ELEVATION
1	11977436.02	870867.41	5310.00	12	11977712.41	871169.49	5308.81
2	11978024.85	870922.90	5310.00	13	11977415.12	870916.50	5310.00
3	11978042.98	870969.57	5310.00	14	11977469.75	870910.76	5290.00
4	11977824.76	871205.86	5310.00	15	11977487.16	870925.76	5290.00
5	11977750.03	871205.03	5310.00	16	11977540.59	870957.08	5290.00
6	11977742.72	871195.59	5308.81	17	11977712.07	871103.95	5290.00
7	11977743.12	871214.89	5310.00	18	11977706.00	871114.30	5290.00
8	11977733.33	871206.49	5308.67	19	11977776.14	871174.73	5290.00
9	11977703.03	871180.38	5308.67	20	11977797.74	871176.15	5290.00
10	11977693.25	871171.96	5310.00	21	11977996.59	870960.41	5290.00
11	11977702.00	871163.65	5310.00				

**PLAN**  
SCALE: 1" = 30'



**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 10**



REFERENCES			REFERENCES			REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT		
0000-CI-028	IMPACTED STORMWATER IMPOUNDMENT C SECTIONS																
0000-CI-005	STANDARD DETAILS SHEET 2																
0000-CI-010	STANDARD DETAILS SHEET 3																

SCALE: AS NOTED	DATE:
DESIGNED BY: SAM	JUN 15
DRAWN BY: SAM	JUN 15
CHECKED BY: JPN	AUG 15
PROJECT MGR: RKZ	AUG 15
CLIENT APPR:	

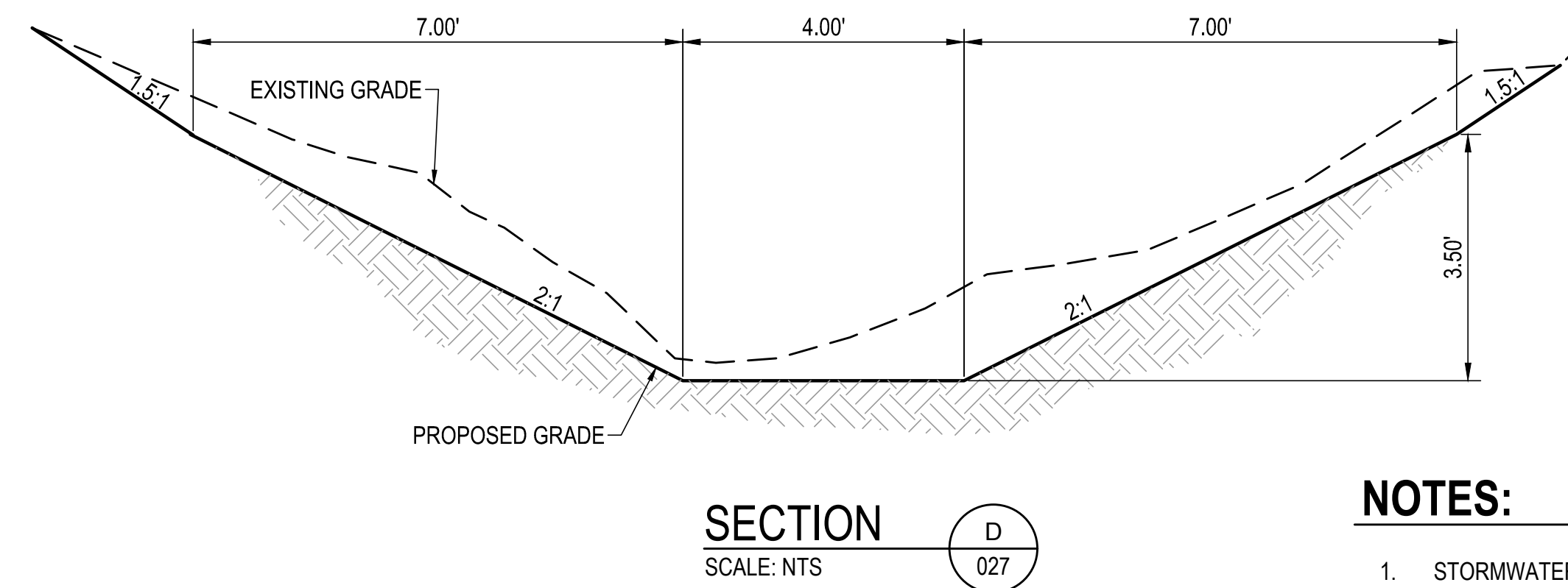
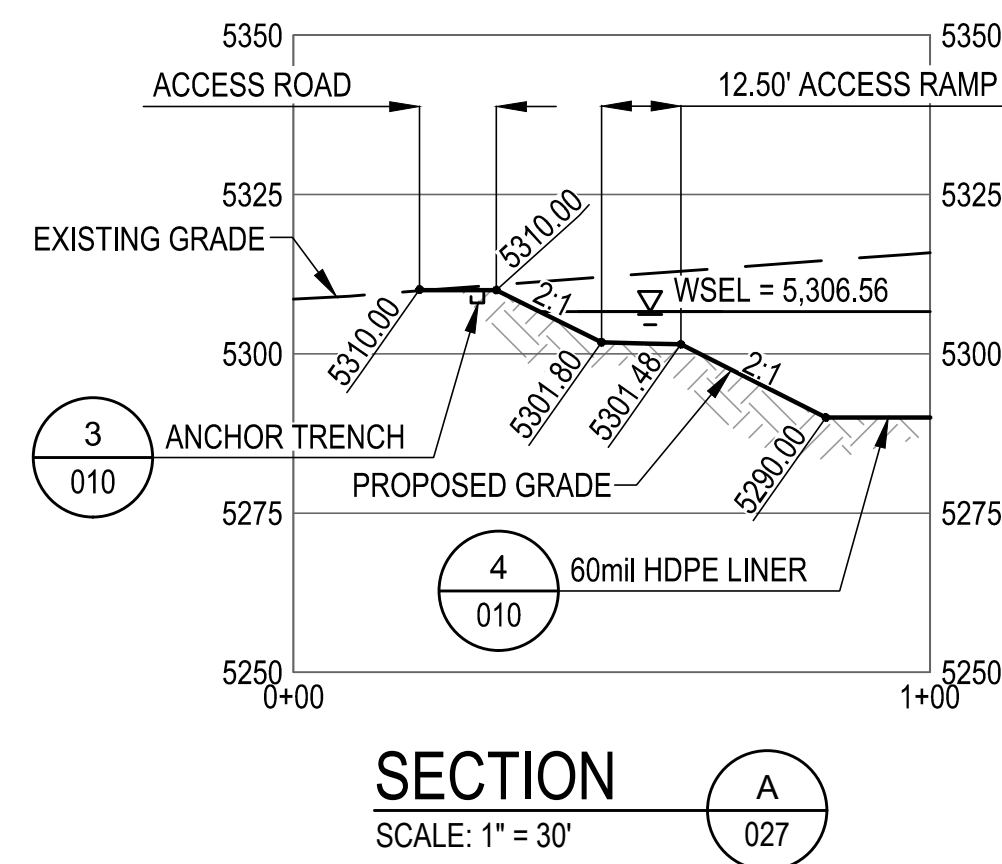
**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**IMPACTED STORMWATER IMPOUNDMENT C**  
**PLAN VIEW**

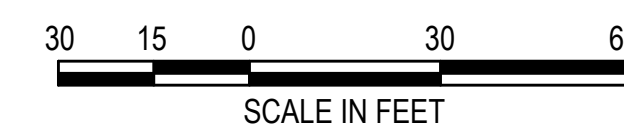
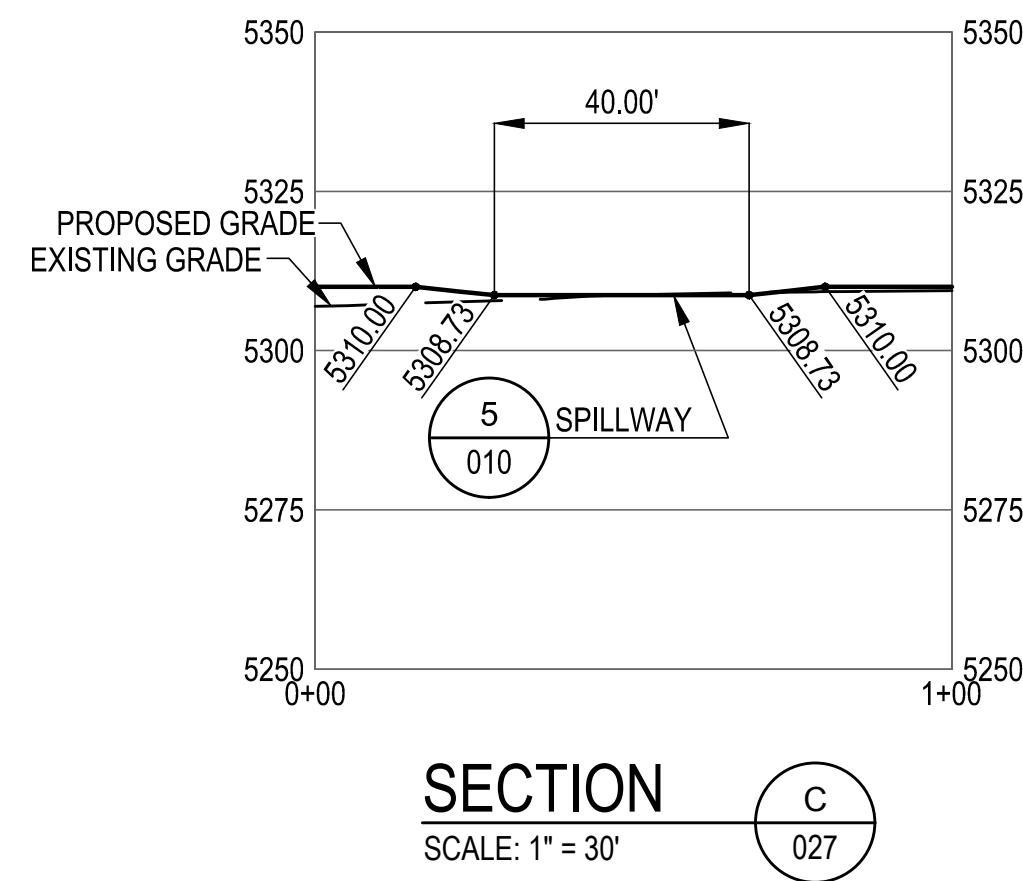
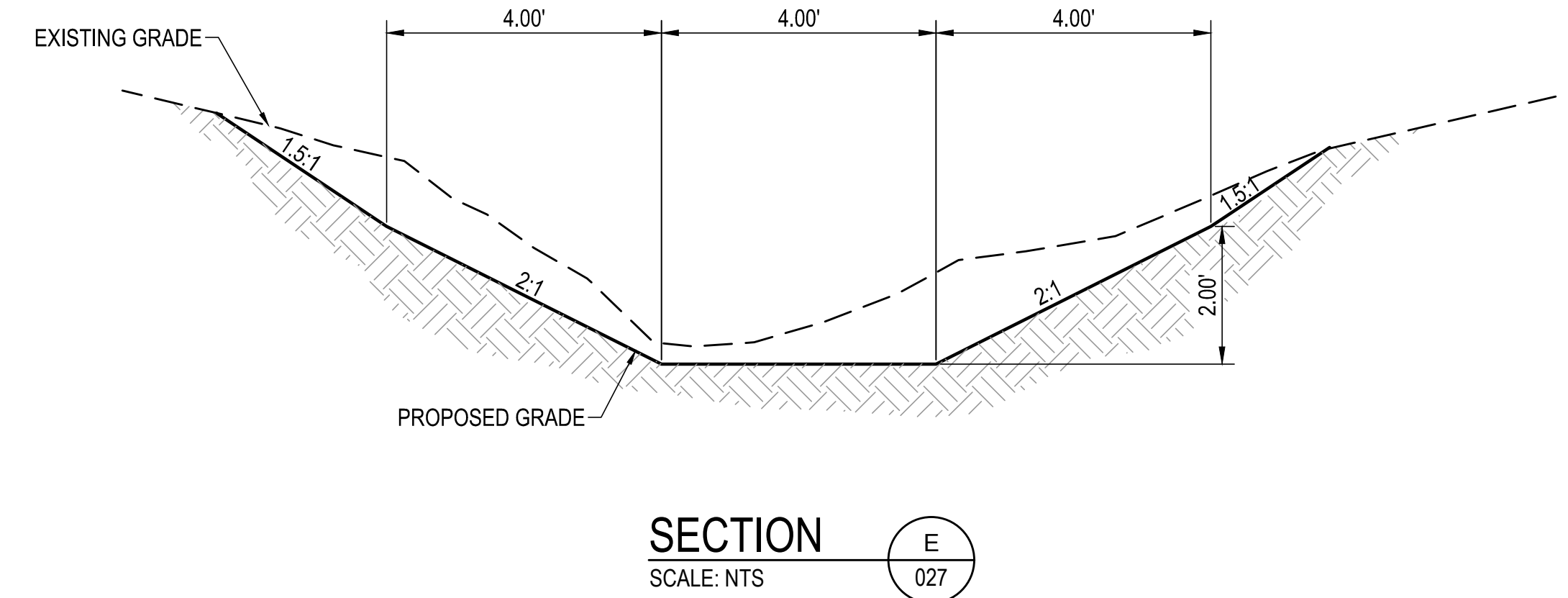
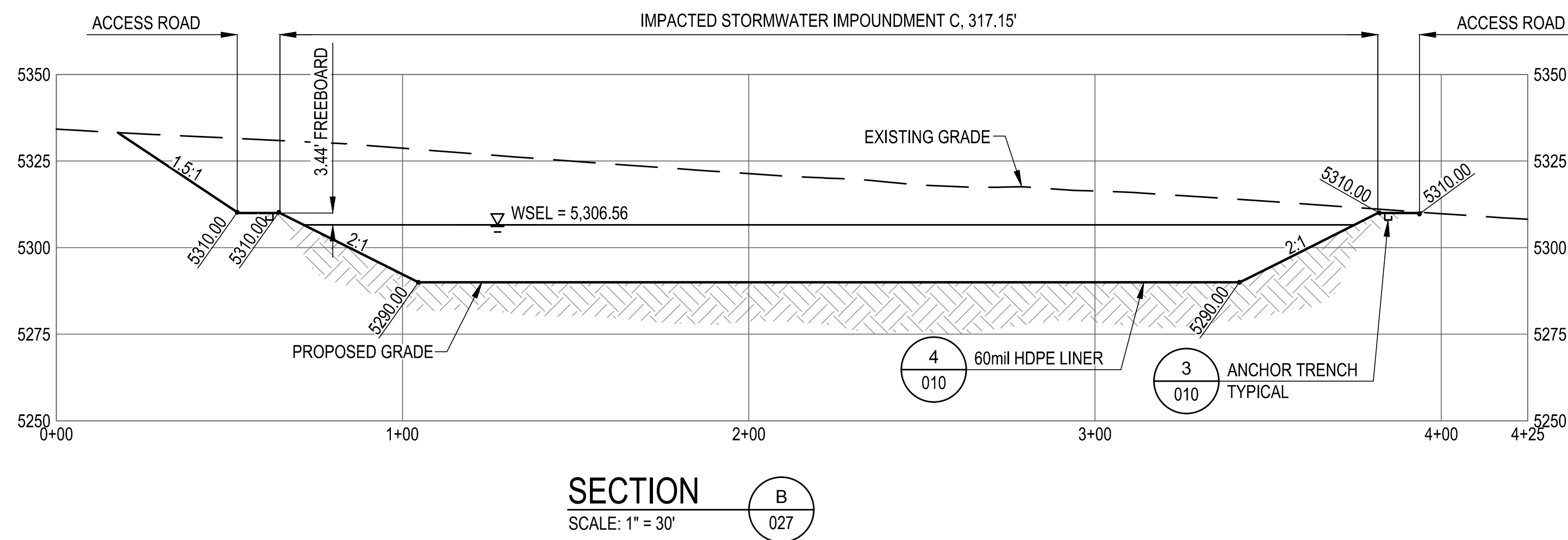
JOB NO. M3 PN-120085  
DWG NO. **0000-CI-027**  
REV. NO. P3 DATE 09 OCT 15

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File: E:\3003\3003A\Civil\_04\3003A.dwg, LAST UPDATE: 12/7/2015 12:33 PM BY: JN069



- NOTES:**
1. STORMWATER CONVEYANCE CHANNEL WILL BE DESIGNED TO CONVEY, AT A MINIMUM, THE PEAK FLOW FROM A 100 YEAR RETURN INTERVAL STORM EVENT WHILE PRESERVING NO LESS THAN 6 INCHES OF FREEBOARD.
  2. CONVEYANCE STRUCTURE WILL BE DESIGNED TO MINIMIZE PONDING AND INFILTRATION OF STORMWATER.



DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW

**FIGURE 11**



File: P:\2024\202408\Civil - (441)\44.2 - Dept\Updates for Permit\0000CI028.dwg, LAST UPDATE: 12/7/2015 12:38 PM, BY: JN090

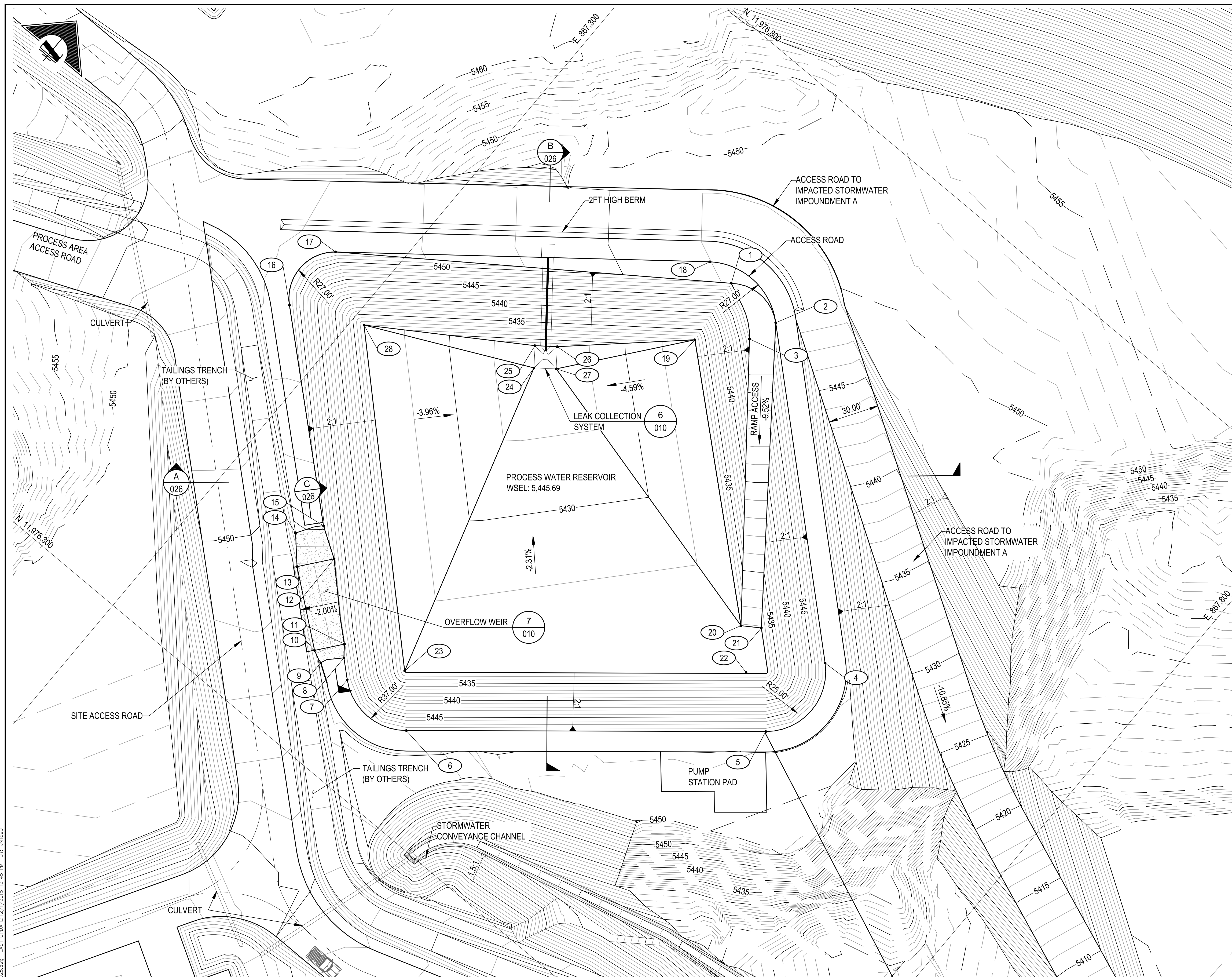
REFERENCES		REFERENCES		REVISIONS						REVISIONS					
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0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW														
0000-CI-010	STANDARD DETAILS SHEET 3														

SCALE: AS NOTED	DATE:
DESIGNED BY: SAM	JUN 15
DRAWN BY: SAM	JUN 15
CHECKED BY: JPN	
PROJECT MGR: RKZ	
CLIENT APPR:	

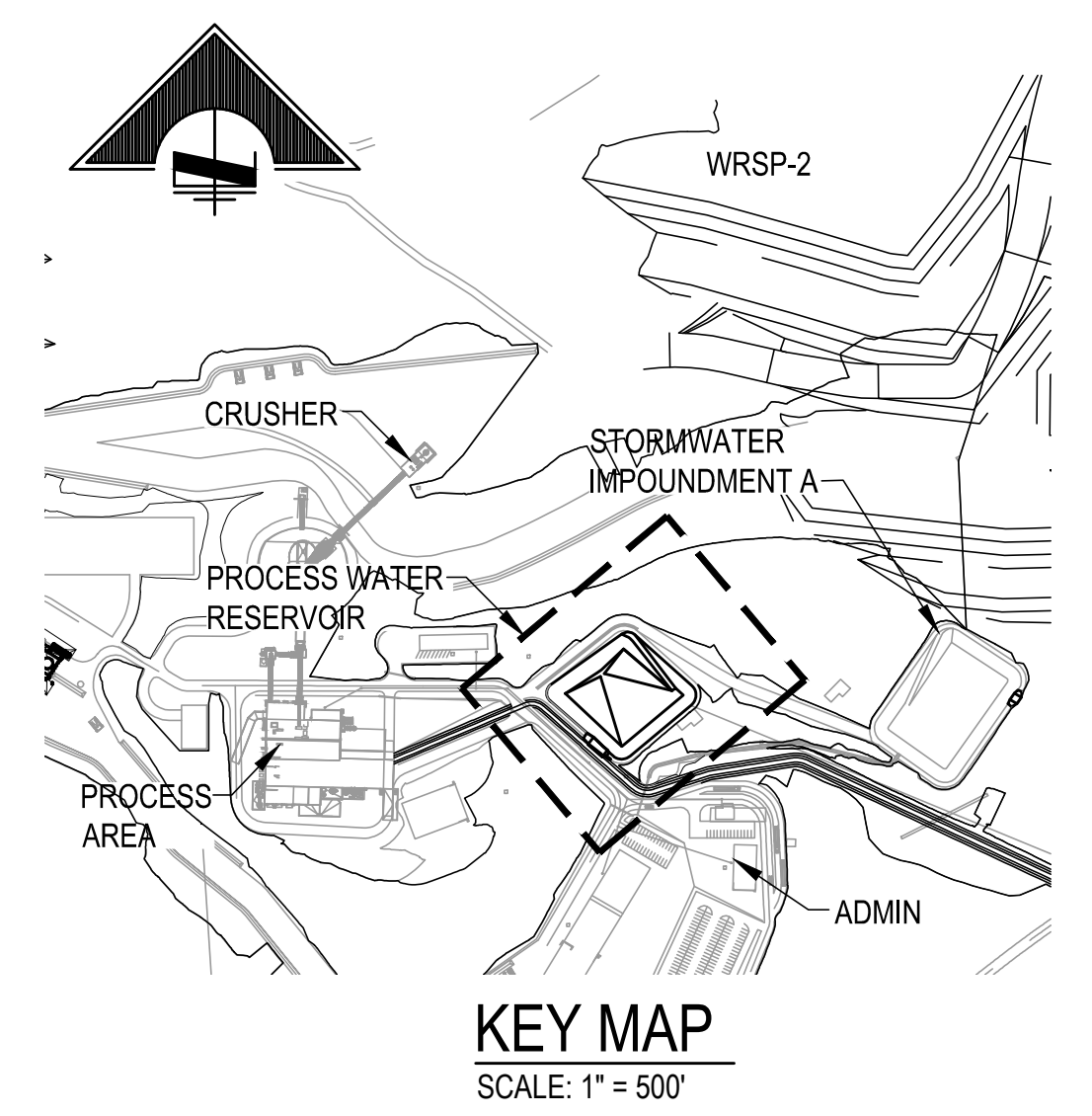
  

	ARCHITECTURE	<b>COPPER FLAT PROJECT</b> GENERAL SITE CIVIL IMPACTED STORMWATER IMPOUNDMENT C SECTIONS	JOB NO. M3 PN-120085
	ENGINEERING		DWG. NO. 0000-CI-028
CONSTRUCTION MANAGEMENT	www.m3eng.com	REV. NO. P3	DATE 09 OCT 15



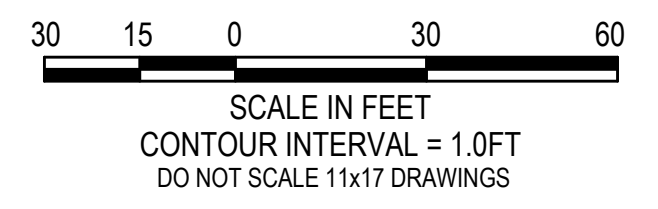
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POINT TABLE			
POINT #	NORTHING	EASTING	ELEVATION
1	11976677.01	867456.22	5448.90
2	11976676.01	867490.99	5448.90
3	11976658.90	867485.30	5448.00
4	11976525.20	867641.13	5444.30
5	11976488.75	867638.96	5448.61
6	11976354.25	867478.64	5448.90
7	11976354.80	867433.15	5448.90
8	11976363.32	867423.40	5448.90
9	11976352.45	867414.83	5448.90
10	11976355.69	867407.41	5447.80
11	11976369.69	867418.41	5448.16
12	11976404.09	867381.76	5448.24
13	11976386.62	867368.04	5447.80
14	11976401.16	867354.74	5449.81
15	11976414.90	867364.55	5450.22
16	11976500.94	867266.34	5452.92
17	11976542.17	867266.96	5452.92
18	11976678.55	867438.45	5449.02
19	11976637.98	867461.15	5431.90
20	11976519.41	867579.10	5431.84
21	11976526.05	867589.10	5431.90
22	11976508.16	867609.15	5431.90
23	11976380.38	867455.56	5431.90
24	11976564.87	867399.57	5428.10
25	11976575.19	867391.67	5428.10
26	11976583.10	867401.99	5428.10
27	11976572.78	867409.89	5428.10
28	11976520.09	867307.24	5431.90



- NOTES:**
1. THE PROCESS WATER RESERVOIR IS INTENDED TO RETAIN PROCESS WATER, STORMWATER THAT FALLS DIRECTLY ON THE POND SURFACE, AND STORMWATER TRANSFERRED FROM OTHER IMPACTED STORMWATER IMPOUNDMENTS.
  2. THE PROCESS WATER RESERVOIR IS SIZED TO RETAIN 12 HRS OF 7,200GPM INFLOW AND THE 100-YR, 24-HR RAINFALL EVENT PLUS 2 FEET OF FREEBOARD.
  3. THE PROCESS WATER WATER RESERVOIR SHALL BE DOUBLE LINED WITH 60mil HDPE PER DETAIL 2 SHEET 0000-CI-010.
  4. THE PROCESS WATER RESERVOIR OVERFLOW WEIR IS DESIGNED FOR THE 25-YR, 24-HR RAINFALL EVENT AT CAPACITY (SEE NOTE 2) AT MINIMUM. THE WEIR CONVEYS PROCESS WATER INTO THE TAILINGS TRENCH AND TO THE TAILINGS IMPOUNDMENT.
  5. OVERFLOW WEIR IS DESIGNED TO ALLOW FOR VEHICULAR TRAFFIC.

IMPOUNDMENT SUMMARY		
CAPACITY AT 100-YR WSEL	726,365	CU-FT
ULTIMATE CAPACITY	937,998	CU-FT



**PRELIMINARY**  
FOR AGENCY REVIEW

FIGURE 12



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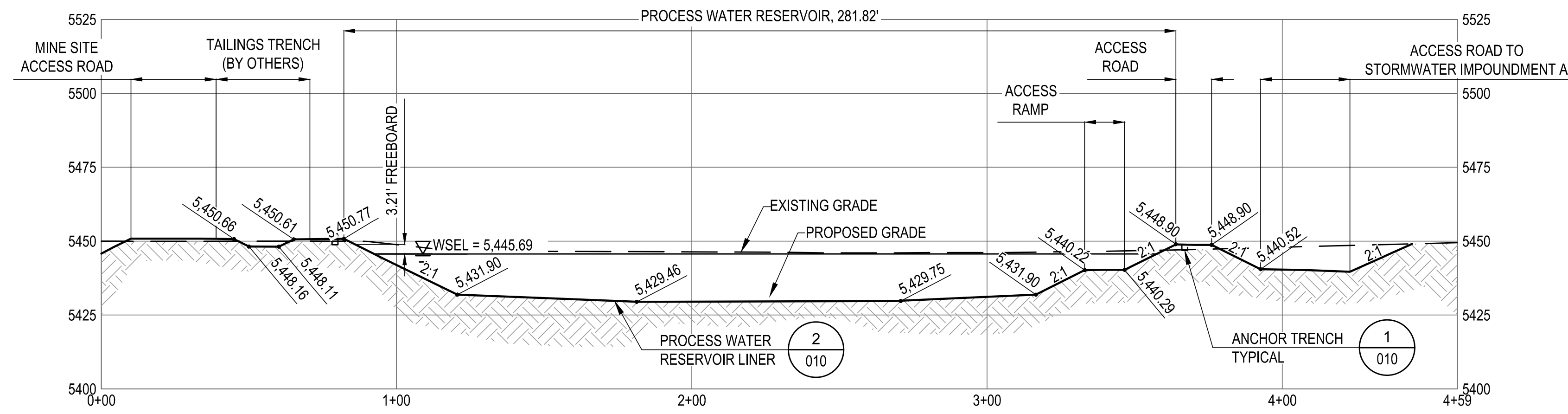
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0000-CI-026	PROCESS WATER RESERVOIR SECTIONS														
0000-CI-010	STANDARD DETAILS SHEET 3														

**3** ARCHITECTURE  
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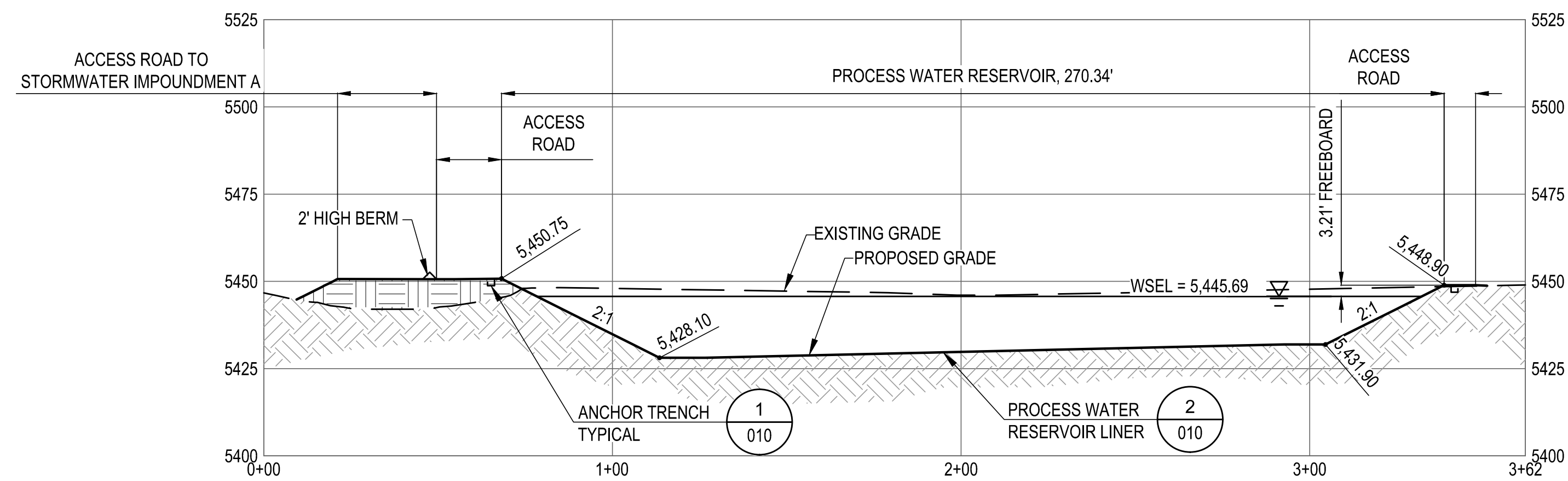
**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**PROCESS WATER RESERVOIR**  
**PLAN VIEW**

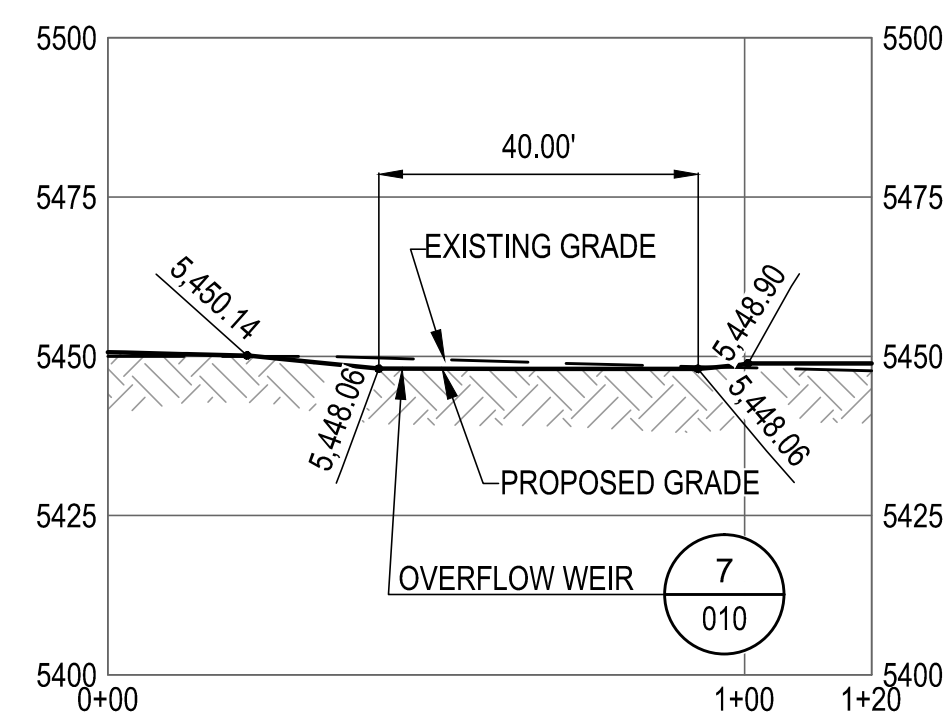
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DWG NO. **0000-CI-025**  
REV. NO. P3 DATE 09 OCT 15



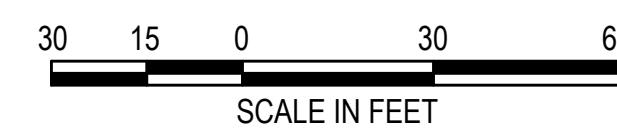
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SECTION B  
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SECTION C  
SCALE: 1" = 30'



DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



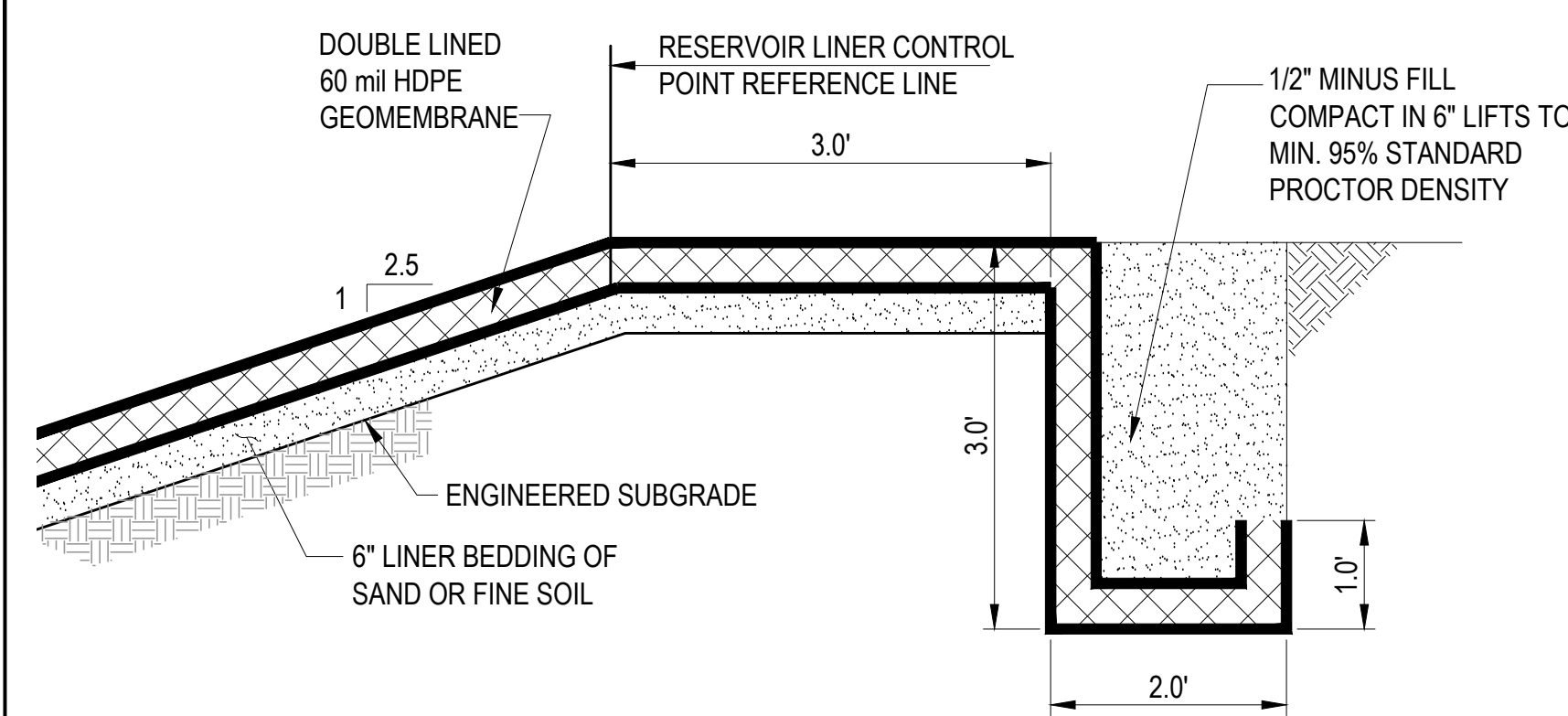
FIGURE 13

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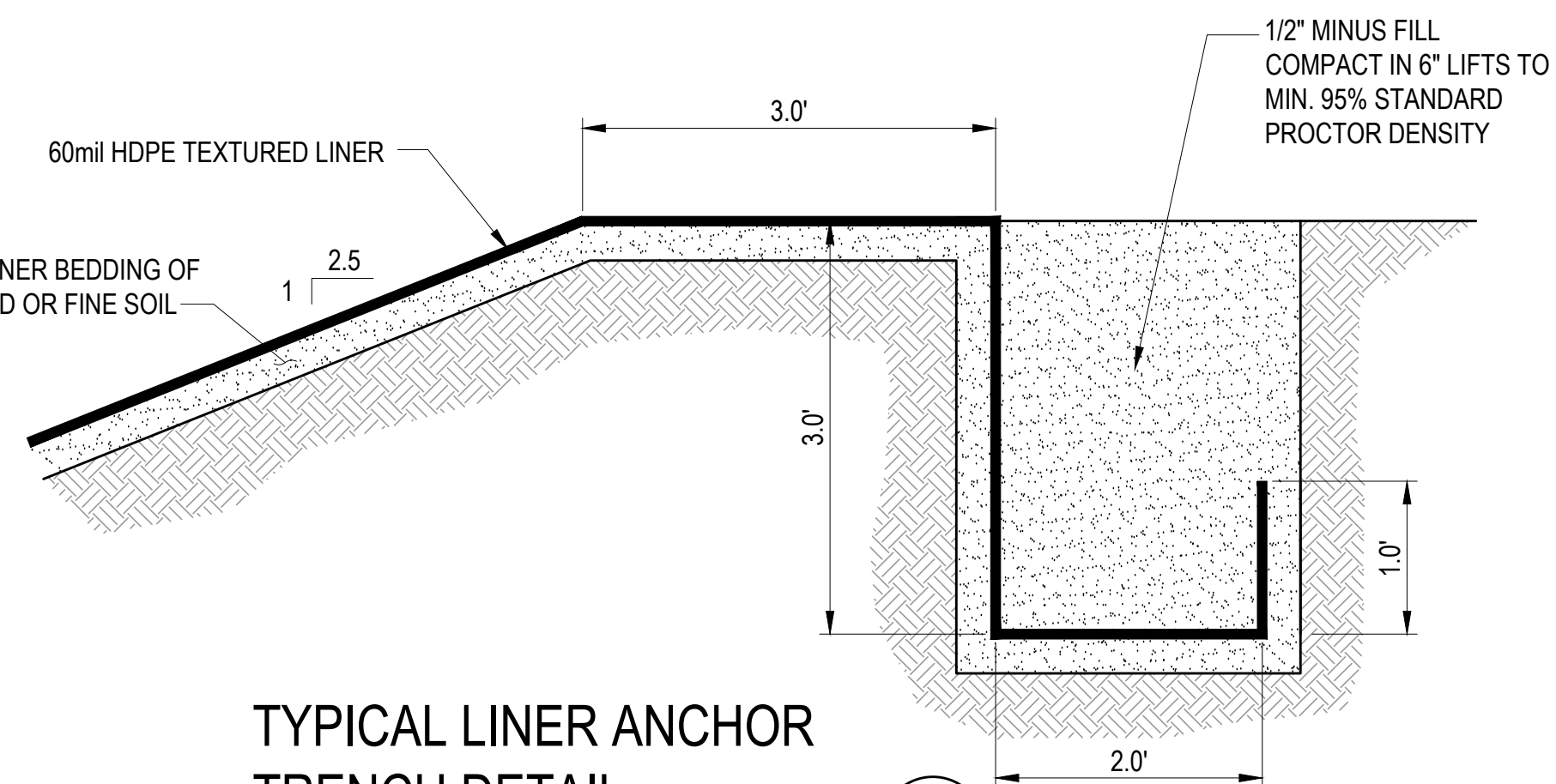
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CHECKED BY:	JPN		
PROJECT MGR:	RKZ		
CLIENT APPR:			

<b>COPPER FLAT PROJECT</b>		JOB NO. M3 PN-120085
<b>GENERAL SITE CIVIL PROCESS WATER RESERVOIR SECTIONS</b>		DWG. NO. <b>0000-CI-026</b>
P3	DATE	09 OCT 15



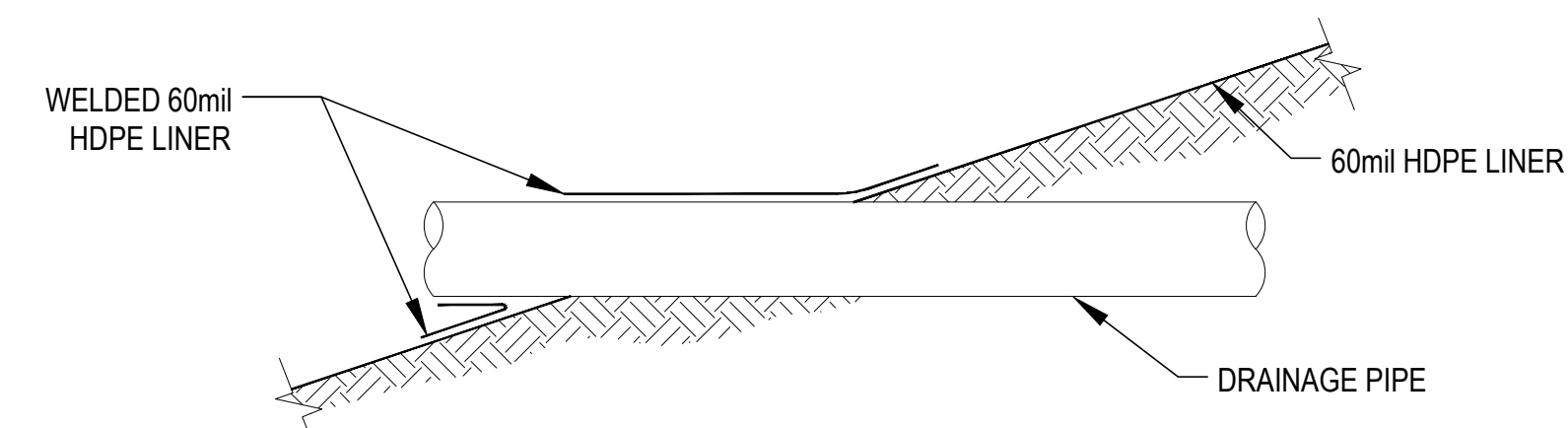
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N.T.S. 1 026



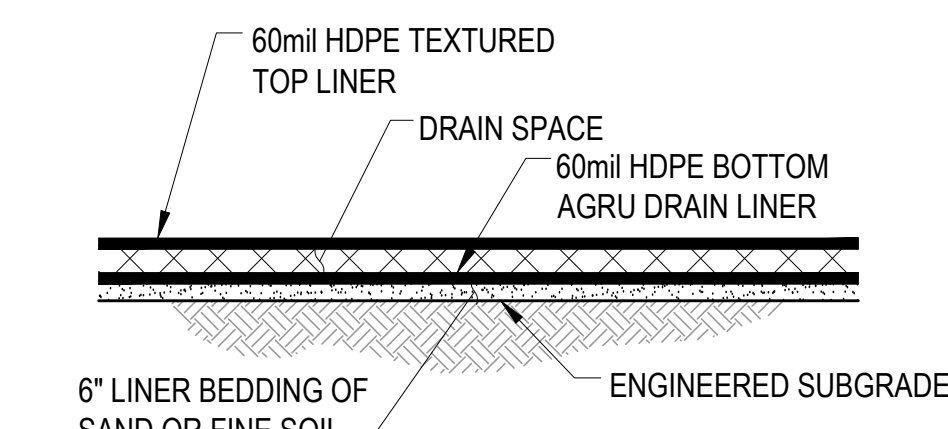
**TYPICAL LINER ANCHOR TRENCH DETAIL**

N.T.S. 3 022, 024, 028



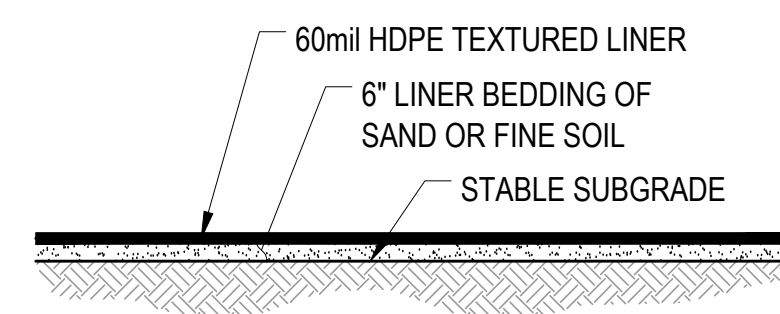
**PIPE PENETRATING LINER**

N.T.S. 16



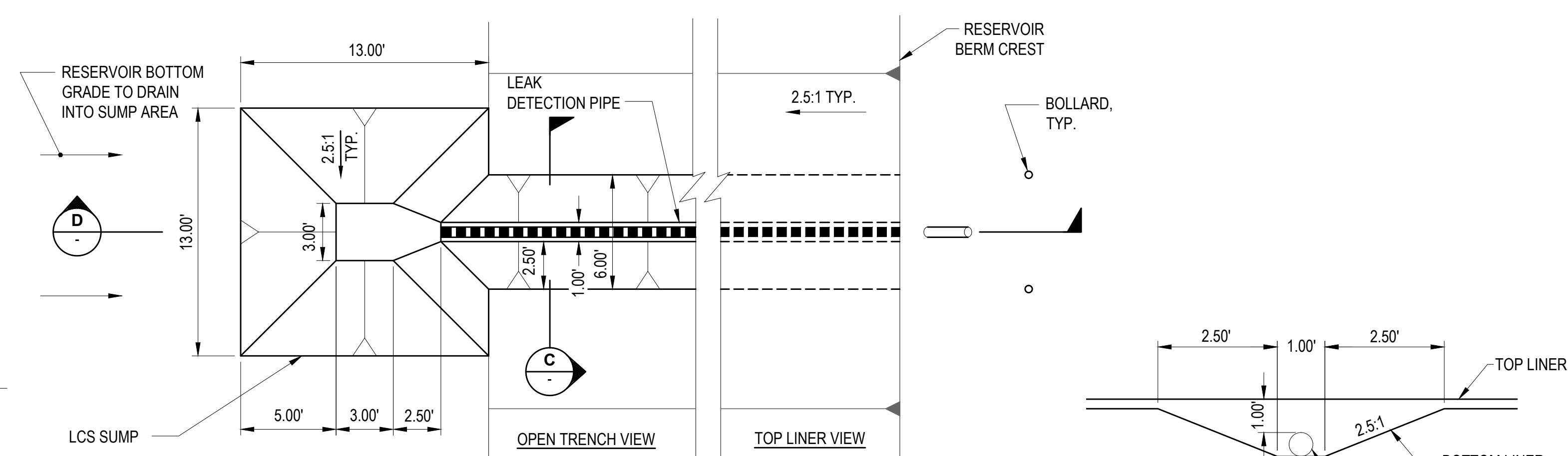
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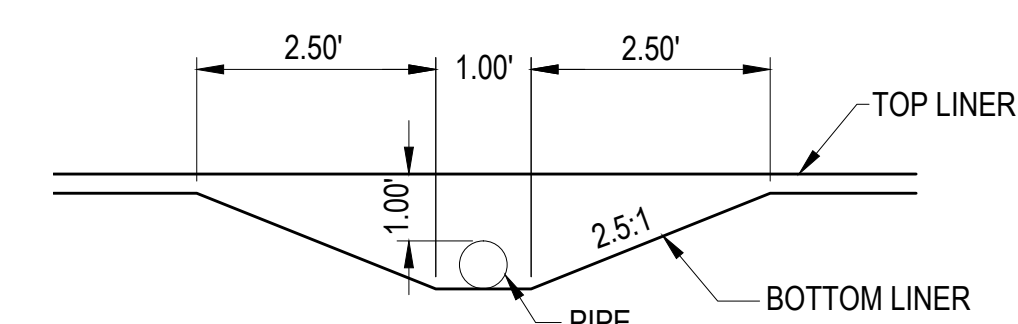
**TYPICAL STORMWATER IMPOUNDMENT LINER**

N.T.S. 4 022, 024, 028

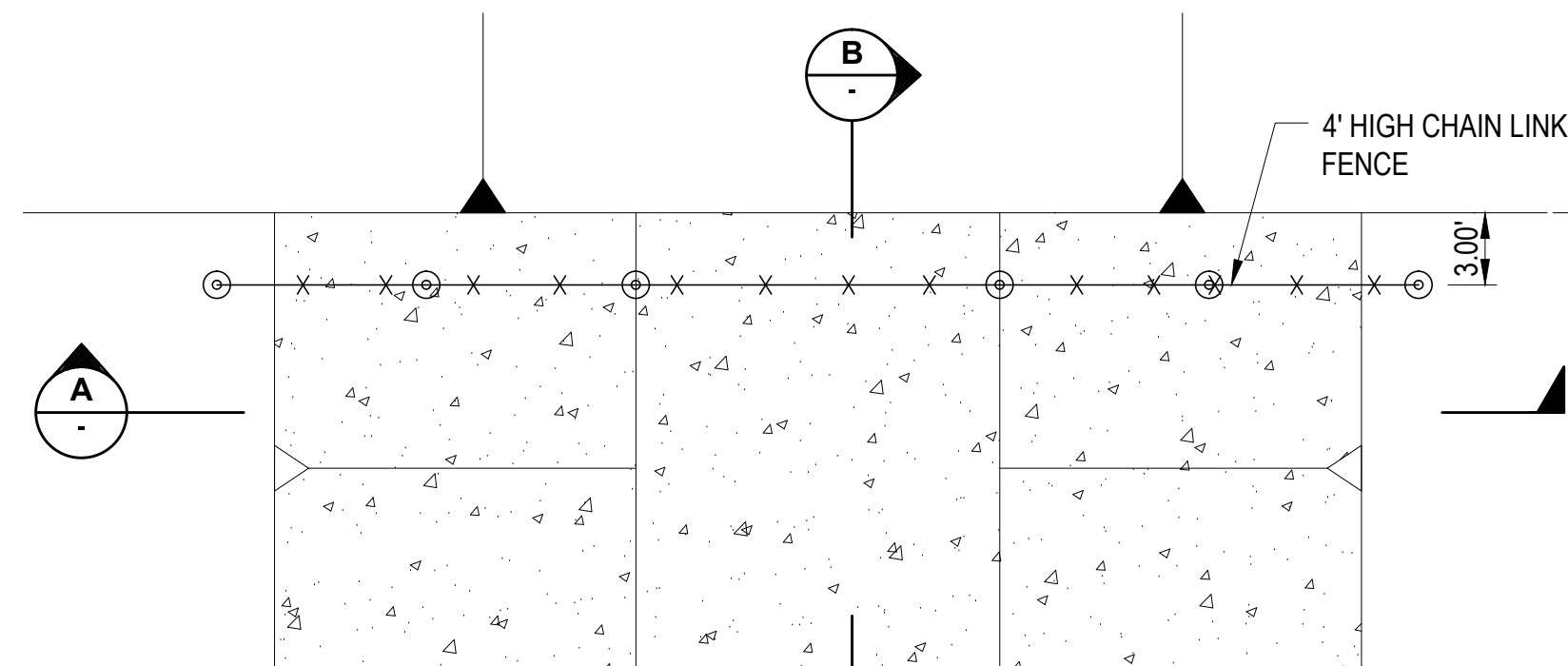


**PROCESS WATER RESERVOIR - LCS PLAN**

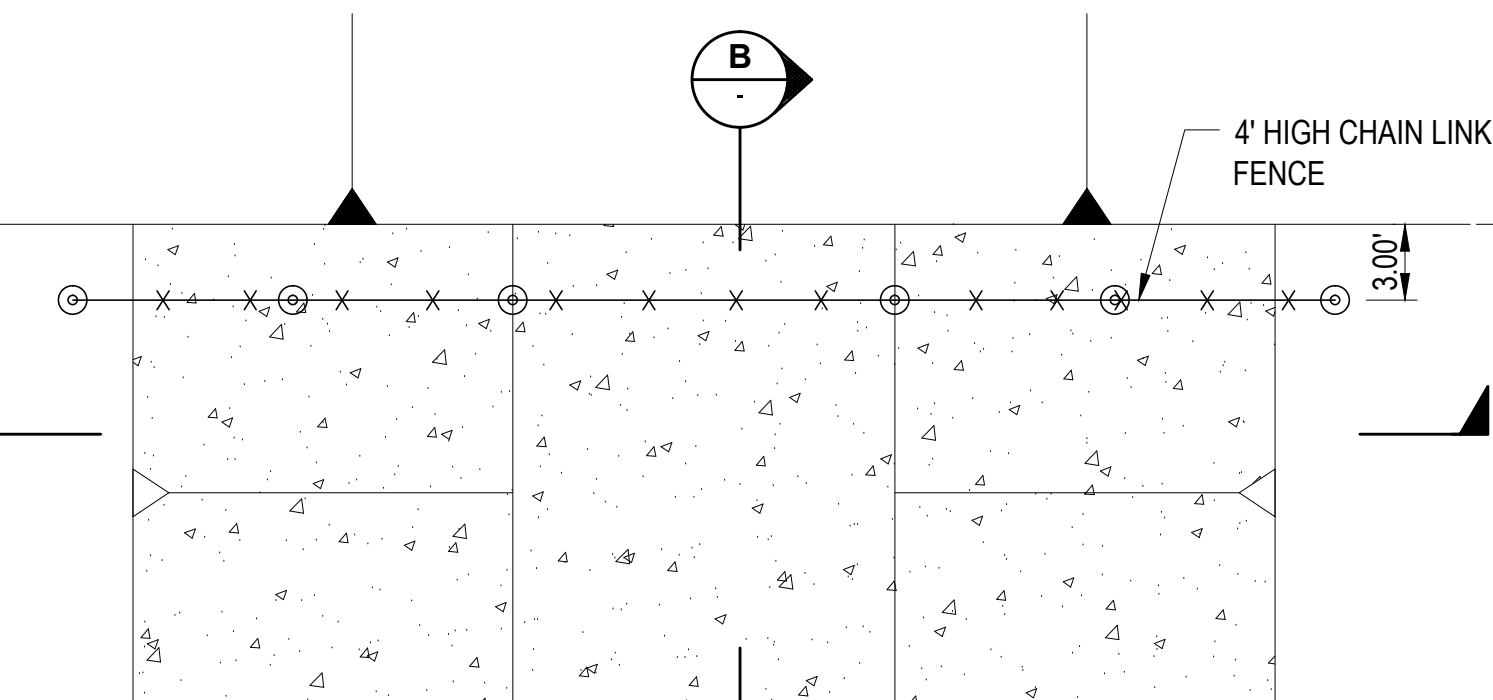
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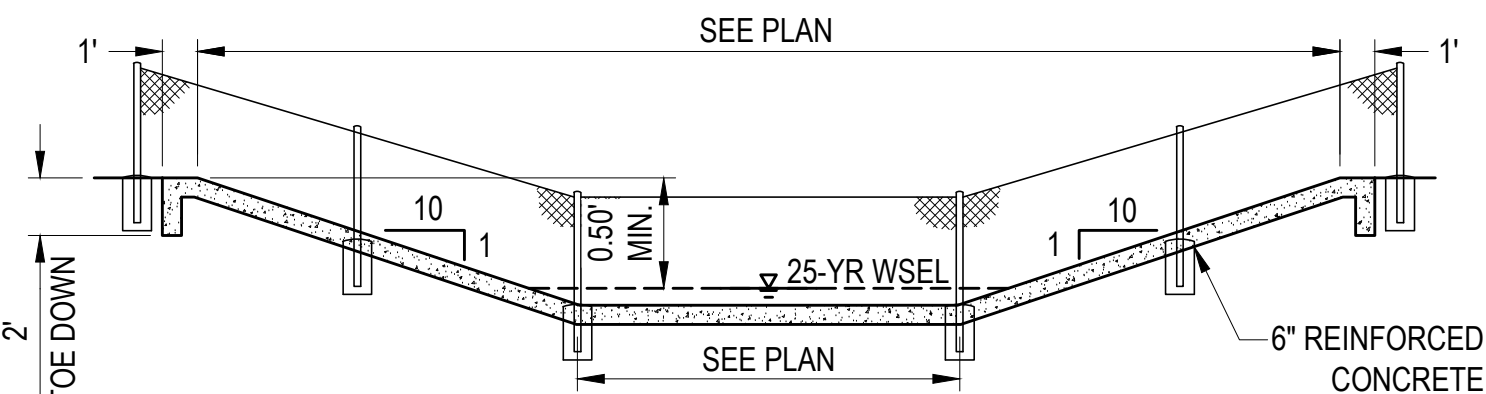
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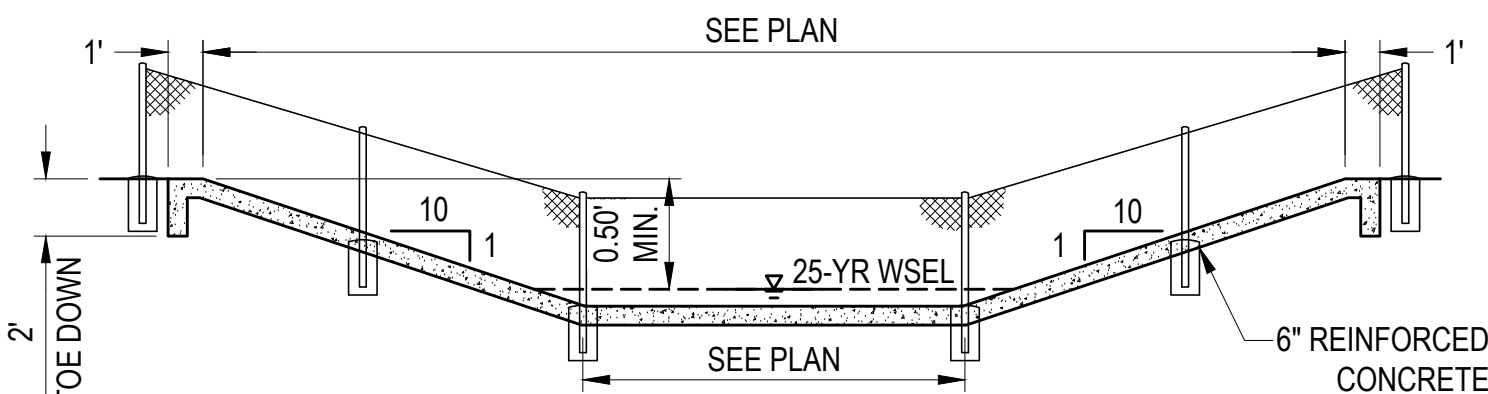
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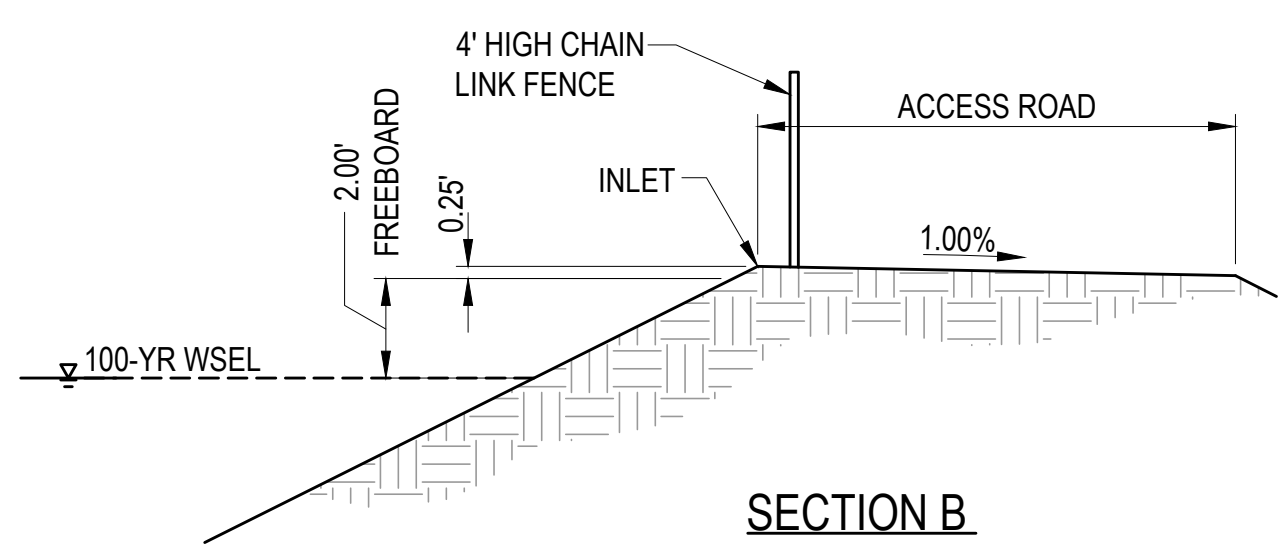
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**SECTION A**



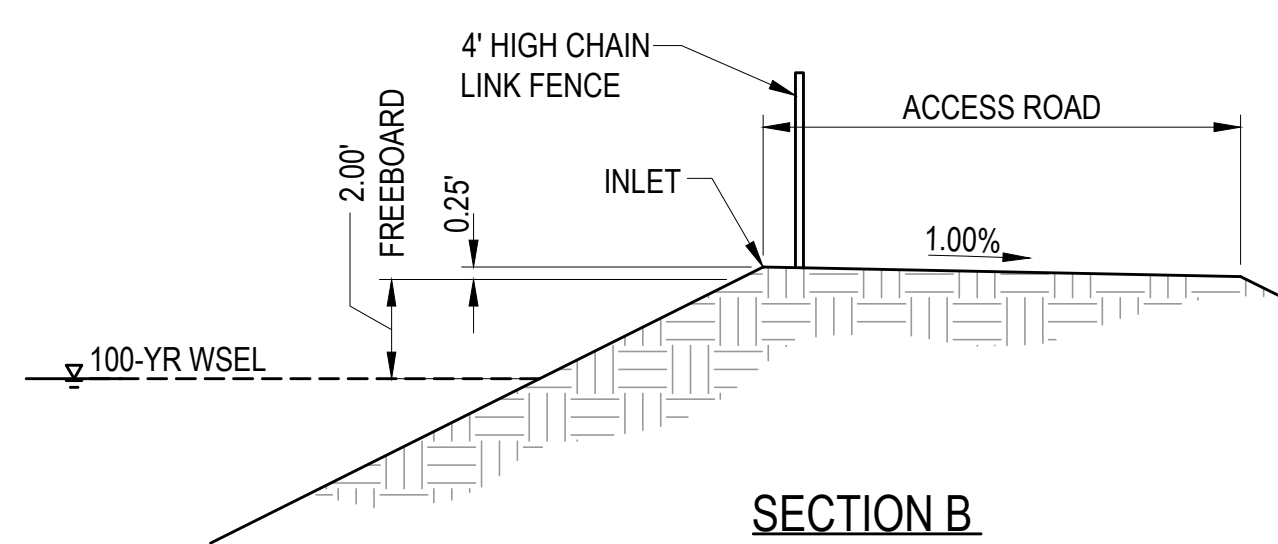
**SECTION A**



**SECTION B**

**SPILLWAY**

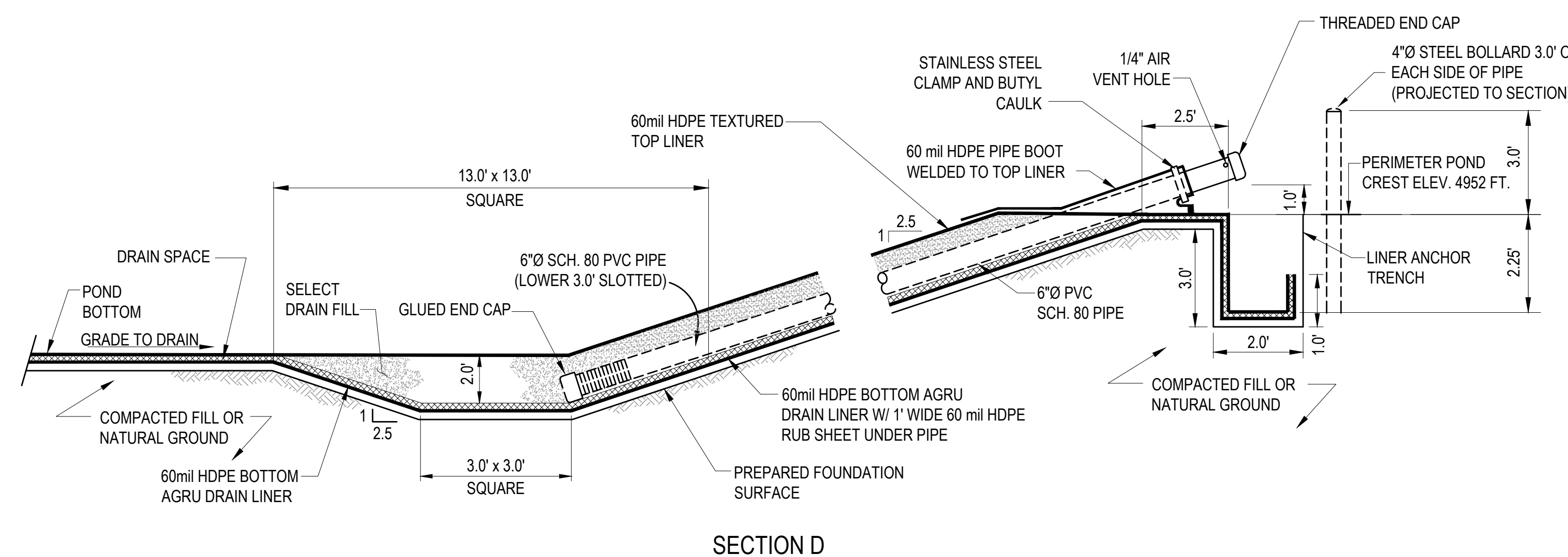
N.T.S. 5 021, 023, 027



**SECTION B**

**OVERFLOW WEIR**

N.T.S. 7 025



**SECTION D**

**FIGURE 14**

**PRELIMINARY FOR AGENCY REVIEW**

**THEMAC RESOURCES**

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REFERENCES		REFERENCES		REVISIONS				REVISIONS				SCALE: NONE		DATE	
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0000-CI-025	PROCESS WATER RESERVOIR PLAN VIEW														
0000-CI-027	IMPACTED STORMWATER IMPOUNDMENT C, PLAN VIEW														

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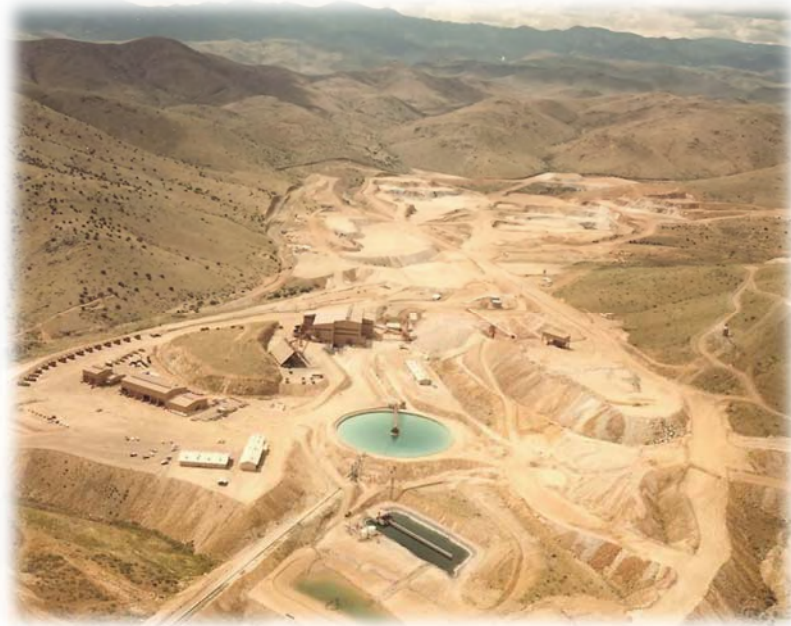
**COPPER FLAT PROJECT**

GENERAL/STANDARDS CIVIL STANDARD DETAILS SHEET 3

JOB NO. M3 PN-120085  
DWG NO. 0000-CI-010  
REV. NO. P3  
DATE 09 OCT 15

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# Copper Flat Project



## Process Facility Containment Report

Prepared For:

**THEMAC**  
RESOURCES 



**Certified Professional Engineer Seal**

This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.

  
Signature




Exp. 12/31/2017

Date 12/7/2015

PROCESS FACILITY CONTAINMENT REPORT  
COPPER FLAT PROJECT

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*Harry J. Lewisley* 12/7/2015  
  
Exp. 12/31/2017

PROCESS FACILITY CONTAINMENT REPORT  
COPPER FLAT PROJECT

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LIST OF DRAWINGS

<u>Drawings</u>	<u>Title</u>	<u>Drawings follow Tables</u>
0000-CI-001	Site Location Map	
0000-CI-008	Process Facility Containment Areas	
3000-FS-000	Overall Process Flowsheet	
0000-GA-050	Concentrator Area Containment Plan	
1010-AR-012	Truck Shop Tank Farm Containment	
1010-GA-010	Fuel Station Containment Plan	
1010-GA-001	Truck Wash Floor Plan & Section	
3010-AR-003	Laboratory Floor Plan	

*Harry J. Lewisley* 12/7/2015  
  
Exp. 12/31/2017

## 1 INTRODUCTION

The Copper Flat Project is located in South Central New Mexico, near the town of Hillsboro, approximately 150 miles south of Albuquerque, and approximately 20 miles southwest of Truth or Consequences (straight-line distances) as illustrated in Drawing (Dwg.) 000-CI-001. The Project is owned and operated by New Mexico Copper Corporation (NMCC), a wholly owned subsidiary of THEMAC Resources Group Limited.

The State of New Mexico has promulgated regulations pertaining to groundwater protection at copper mining facilities (New Mexico Administrative Code Title 20, Chapter 6, Part 7 [20.6.7 NMAC], the "Copper Rule"), the stated purpose of which is "to control discharges of water contaminants specific to copper mine facilities and their operations to prevent water pollution."

This report provides the design criteria, location, and capacity of containment systems for the process areas identified in Section 2 of this report to comply with 20.6.7 NMAC, Sections 22, 23, and 26. This report excludes design considerations for the Impacted Stormwater Impoundments, Process Water Reservoir, and Tailings Storage Facility (TSF), i.e., the tailings impoundment, underdrain collection pond, surge pond and the secondary containment trench from the processing facility to the TSF, which have been completed by others and are reported separately.

## 2 PROCESS AREA IDENTIFICATION

Pursuant to 20.6.17.22 and .23, tanks and pipelines for new construction as proposed for the Copper Flat Project are required to be designed with adequate containment to prevent migration of process solutions to groundwater. Specific areas of the project for which the containment designs are described in this report include the following areas with tanks that require secondary containment.

- Concentrator Area
- Truck Shop
- Fuel Station
- Process Water Storage Tank

This report also describes containment for the following areas that contain process equipment and/or liquids that require containment to prevent release to the environment.

- Crusher
- Coarse Ore Stockpile and Ore Reclaim Tunnel
- Truck and Equipment Washing Unit
- Analytical Laboratory
- Domestic Sewage Treatment Facility

Discussion of containment of facilities related to the tailings storage facility (TSF) (tailings pipelines, tailings storage facility), impacted stormwater impoundments, and the process water reservoir is not included in this report.

The following containment areas will be covered by a roof and therefore will not collect appreciable precipitation.

- Grinding
- Copper Flotation
- Molybdenum Flotation
- Copper Filtration
- Diesel Reagents
- Flotation Reagents
- Sodium Hydrosulfide Reagents
- Truck Shop Tank Farm
- Assay Laboratory
- Wastewater Treatment Facility

The following areas will be uncovered and will therefore collect precipitation. Monthly inspections of these facilities are required by 2.6.7.22 and .23 NMAC. However, after occurrence of a significant precipitation event, the collection sumps will be inspected and collected water will be evacuated within 30 days to maintain required containment capacity. Stormwater evacuated from these areas will be added to the process reservoir or process stream for disposal.

- Copper-Moly Thickening
- Copper Thickening
- Lime Reagents
- Wheel Wash
- Fuel Station
- Truck and Equipment Washing Unit
- Process Water Storage Tank

### 3 BASIS OF DESIGN

Tanks and pipelines for the Copper Flat Project are designed in accordance with NMAC 20.6.7.23 (A). Most process pipelines are located within buildings with concrete floors sloped to drain to containment sumps. Process pipelines located outside of buildings will have secondary containment for management of leaks and spills. Pipelines are designed in accordance with the following specifications.

Construction Materials	20.6.7.23.A.(1).(a) NMAC	Impermeable materials compatible with contents and resistant to corrosion or degradation
Leak Detection	20.6.7.23.A.(1).(b) NMAC	Mechanism for monitoring integrity
Containment	20.6.7.23.A.(1).(c) NMAC	Secondary containment provided by building floors, double-walled piping, or lined trenches.

Tanks containing process solutions, reagents, chemicals, and petroleum products are located within a building area or on concrete containment pads with curbs or stem walls that are designed to be impermeable to the contents of the contained tanks. Joints between containment curbs and floors will be sealed with approved water stops to seal against leaks. Tanks are designed to contain the stored material, prevent overflows, and are resistant to corrosion or degradation. Tanks for this project (Table 1) are above ground and designed in accordance with the following specifications, except where specifically noted.

Construction Materials	20.6.7.23.A.(2).(a) NMAC	Impermeable materials compatible with contents and resistant to corrosion or degradation
Foundation	20.6.7.23.A.(2).(b) NMAC	Constructed concrete foundation
Containment	20.6.7.23.A.(2).(c) and 20.6.7.23.A.(2).(d) NMAC	Concrete containment to prevent run-on. Containment volume sized to contain 110% of volume of largest tank in the enclosed area while accounting for unusable volume with the containment due to the presence of other tanks, equipment, pedestals, etc.

The majority of process solution pipelines will be placed inside secondary containment for tanks and process buildings. When process pipelines cannot be positioned within secondary containment for tanks or process buildings, the pipe will either be double walled and positioned to drain to an established containment area, or secondary containment specific to the pipeline will be constructed.

Containment sumps will be sealed with water stops at joints and coatings applied where necessary to provide a watertight seal in order to prevent leaking solution to the environment. A list of containment sumps and the destination of liquids collecting in the sumps is provided as Table 2.

## 4 PROCESS FACILITY CONTAINMENT AREAS

Containment of process solutions, reagents, and other potential groundwater contaminants associated with the Copper Flat Project is delineated below by process area. The areas described include the primary mineral processing circuit and other ancillary areas that are essential to the operation of the project, such as vehicle washing and maintenance, fueling operations, assaying, and sanitary wastewater treatment. The portions of the project area addressed in this report are identified in Dwg. 0000-CI-008. Containment of process solutions is discussed below by process area and by location of ancillary facilities. A list of tanks designed for installation at the Copper Flat Project is presented as Table 1. Table 1 provides location, dimensions, material of construction, and capacity of the tanks in gallons. These tank capacities are compared with the capacities of containments in which they are located to demonstrate adequate containment.

Containment is provided around tanks and in areas with process piping to prevent loss of contents, control leaks and spills, and prevent release of solutions to the surface or ground water. Concrete containment walls are used to contain leaks and spills where tanks are present. The net volume of the containment is designed to contain at least 110 percent of the volume of the largest tank or series of connected tanks in the system. Any leakage, spillage, or wash water within a containment area is directed by sloped concrete floors to a watertight drainage sump. A list of the sumps present in each of the containment areas and the disposition of the captured solutions is presented in Table 2. The sumps listed in Table 2 are containment sumps.

### 4.1 CONCENTRATOR AREA

Drawing 3000-FS-000 summarizes the Copper Flat ore process flowsheet. Processing of the ore begins with the Crusher. Crushed ore is conveyed to the Coarse Ore Stockpile to be used as mill feed. A concrete tunnel beneath the Coarse Ore Stockpile is used to reclaim the crushed ore and convey it to the Grinding Area. Coarse ore is combined with water in the grinding mills to create a slurry that flows by gravity to Copper Flotation. Concentrate containing copper (Cu) and molybdenum (Moly) is pumped to the Copper-Molybdenum Thickening area and then to the Molybdenum Flotation area for separation of Copper and Moly. Moly Concentrates are filtered, dried, and bagged for shipment in this area. The remaining Copper concentrate is pumped to the Copper Thickening area where excess water is removed. Thickened Copper concentrates are pumped to the Copper Filtration area for dewatering prior to loading onto trucks for offsite shipment. The wheels of trucks and trailers loaded with Copper concentrate are washed in the Wheel Wash area to prevent the loss of concentrate and contamination of roadways.

Chemicals used in the process described above are stored in the Reagents area. Several containment areas are present in the Reagent area to isolate chemicals that may be incompatible or require special handling. Containment areas in the Reagents area include Lime Containment, Diesel Containment, General Reagents Containment, and Sodium Hydrosulfide (NaHS) Containment. The sumps are equipped with a dedicated sump pump to remove the contained liquids and transfer them to an appropriate location in the process.

An overall plan of the Concentrator Area is provided in Dwg. 0000-GA-050. Locations of tanks and sumps are shown on the drawing along with flow arrows depicting the slope of the floor surfaces to demonstrate flows to the collection sumps for the identified areas. A table on the drawing provides the volume of the largest tank in a given area and compares it with the capacity of the containment sump in which it resides.

The concentrator perimeter will be enclosed by curbing to prevent migration of process solutions away from the concentrator building. Unless specified below, general containment curbing at the perimeter of the concentrator will be at least 4 inches tall.

#### 4.1.1 Crushing Area

The Crusher Area (Dwg. 0000-CI-008) does not contain any tanks. The crusher takes large blocks of ore recovered from the mine pit and crushes it to sizes that can be efficiently handled in the grinding area. Crushing is a dry process, but the crusher dump pocket is open to precipitation. Water is used in the crusher to suppress dust during the transfer of crushed ore to the conveying system to feed the Coarse Ore Stockpile. Water is used to wash down areas within the crusher that accumulate dust and dirt. Precipitation and the moisture content of the ore also contribute minor amounts of water that will collect at the bottom of the crusher structure.

A collection sump located in the lowest level of the concrete crusher facility collects water within the primary crusher and pump it to the Coarse Ore Reclaim Tunnel sump using a dedicated sump pump. The sump has a capacity of approximately 560 gallons. The sump pump will be configured for automatic start when solution is detected and the capacity of the crusher sump is appropriate for the potential solution flows.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Crushing Area</b>				
No Storage Tanks	na	na	na	na
Containment Volume (gallons)			560	

#### 4.1.2 Coarse Ore Stockpile and Ore Reclaim Tunnel Area

The Coarse Ore Stockpile and Reclaim Tunnel Area (Dwg. 0000-CI-008) does not contain any tanks. The Coarse Ore Stockpile accumulates crushed ore for delivery to the grinding area of the concentrator for processing. An existing concrete Reclaim Tunnel lies beneath this area and will be used to recover crushed ore at a controllable rate to supply feed to the grinding circuit. Process water and fresh water piping to the Concentrator pass through the Reclaim Tunnel.

Water used for wash down and dust suppression, as well as the water pumped from Crusher sump, drains to a sump in the bottom of the Reclaim Tunnel. Process water or fresh water solution coming from piping in the tunnel and moisture from precipitation or excess water contained in the crushed ore will drain to the Reclaim Tunnel sump.

A dedicated sump pump will be used to transfer collected water from the Reclaim Tunnel sump to the concentrator through the Primary Cyclone Feed system. The Reclaim Tunnel Sump has a capacity of approximately 6,400 gallons. The sump pump will be configured for automatic start when solution is detected and the capacity of the Reclaim Tunnel sump is appropriate for the potential solution flows. The installed pumping capacity will be sufficient to evacuate the sump at a flow rate equal to the rate that may be generated by a broken water line.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Reclaim Tunnel Area</b>				
No Storage Tanks	na	na	na	na
Containment Volume (gallons)			6,400	



#### 4.1.3 Grinding Area

A conveyor from the Reclaim Tunnel delivers crushed ore to the semi-autogenous grinding (SAG) mill. Water is added to the SAG mill to begin the grinding process. Discharge from the SAG mill is pumped to the Primary Cyclone classifiers in closed circuit with the Ball Mill. The slurry is circulated through the Ball Mill until the particles are fine enough to flow out of the top of the cyclones and on the Rougher Flotation Conditioning Tank. A portion of the circulating slurry will go through a gravity concentration circuit to recover separable gold prior to fine grinding.

The only tank in the Grinding Area (Dwg. 0000-GA-050) is the Gravity Concentrate Tank. This tank accumulates concentrate from gravity gold separation and will typically be filled with 8,000 gallons of wet solids. The Grinding Area containment volume is 200,000 gallons. The large containment volume in this area is not based on required capacity but rather results from requirements for setup and configuration of the grinding equipment. Therefore, the resulting containment capacity is more than adequate to contain the contents of the gravity concentrate tank and any operating upsets or maintenance conditions that may occur in the grinding circuit or elsewhere in the concentrator area.

All leakage or spillage of liquids or water used to wash down equipment and floors in this area reports to the Grinding Area Containment sump. The sump is equipped with a dedicated pump and automatic start/stop controls. Liquids accumulated in this sump are pumped back into the Primary Cyclone Feed system.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Grinding Area Containment</b>				
Gravity Concentrate Tank	12.00	9.50	8,000	Carbon Steel
Containment Volume (gallons)			200,000	

#### 4.1.4 Copper Flotation Area

Copper Flotation (Dwg. 0000-GA-050) includes the Rougher Conditioning Tank, rougher flotation cells, and other equipment to recover the sulfide minerals in the process slurry. The floor in the Copper Flotation area is sloped to facilitate gravity flow from one flotation cell to the next.

The floor slopes are directed to the Copper Flotation Area sump. Solutions accumulated in the Copper Flotation Area sump are returned to the Rougher Flotation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest tank the Copper Flotation Area is the Rougher Flotation Conditioning Tank with a capacity of 69,300 gallons, which requires a containment capacity of approximately 76,200 gallons at 110% of capacity. The net containment volume in the Copper Flotation Area is 79,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Copper Flotation Area Containment</b>				
Rougher Flotation Conditioning Tank	22.00	25.00	69,300	Carbon Steel
Containment Volume (gallons)			79,000	

#### 4.1.5 Copper-Molybdenum Thickening Area

The Copper-molybdenum (Copper-Moly) Thickening Area (Dwg. 0000-GA-050) recovers water from the rougher flotation concentrate in preparation for separating the moly into a saleable product. The area includes the flocculant mixing and distribution system, Copper-Moly thickener, and Copper-Moly thickener overflow tank.

The floor slopes are directed to the Copper-Moly Thickening Area sump. Solutions accumulated in the Copper-Moly Thickening Area sump are returned to the Copper-Moly Separation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Copper-Moly Concentrate Thickener at a capacity of 17,700 gallons., which requires a containment capacity of approximately 19,500 gallons at 110% of capacity. The designed containment volume is 29,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Copper-Moly Thickening Area Containment</b>				
Flocculant Mix Tank	12.00	7.25	6,100	Carbon Steel
Flocculant Distribution Tank	12.00	7.25	6,100	Carbon Steel
Copper-Moly Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Copper-Moly Concentrate Thickener Overflow Tank	4.00	10.67	1,000	Carbon Steel
Containment Volume (gallons)			29,000	

#### 4.1.6 Molybdenum Flotation Area

Molybdenum (Moly) Flotation (Dwg. 0000-GA-050) separates the moly from the copper concentrate and refines it through a series of flotation and cleaning steps into a saleable product that is filtered, dried, and bagged for shipment to the buyer. The area includes the copper-moly separation conditioning tank, moly filter feed tank, and moly filter cloth wash tank.

The floor slopes are directed to the Moly Flotation Area sump. Solutions accumulated in the Moly Flotation Area sump are returned to the Copper-Moly Separation Conditioning Tank. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Moly Filter Feed Tank at a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons at 110% of capacity. The designed containment volume is 8,300 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Moly Flotation Area Containment</b>				
Copper-Moly Separation Conditioning Tank	4.00	10.67	1,000	Carbon Steel
Moly Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Moly Filter Feed Tank	8.00	6.00	2,000	Carbon Steel

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Containment Volume (gallons)	8,300
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**4.1.7 Copper Thickening Area**

The Copper Thickening Area (Dwg. 0000-GA-050) receives the copper concentrate from the moly flotation circuit and dewateres it in preparation for filtration to make a shippable concentrate. The area includes a stock tank for storage of thickened concentrates to act as surge capacity for production. Copper Filtrate tanks receive water recovered from the filters for recycling through the thickener.

The floor slopes are directed to the Copper Thickening Area sump. Solutions accumulated in the Copper Thickening Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Copper Concentrate Stock Tank at a capacity of 41,760 gallons., which requires a containment capacity of approximately 46,000 gallons at 110% of capacity. The designed containment volume is 50,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Copper Thickening Area Containment</b>				
Copper Concentrate Thickener Overflow Tank	4.00	4.00	375	Carbon Steel
Copper Concentrate Stock Tank	17.00	25.00	41,760	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Concentrate Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			50,000	

**4.1.8 Copper Filtration Area**

The Copper Filtration Area (Dwg. 0000-GA-050) does not contain any tanks. Thickened copper concentrate from the stock tank is pumped to a pair of plate-and-frame filters that produce a filter cake suitable for bulk shipment. The water recovered from the filters returns to the copper thickener via the Copper Filtrate Tanks that are located with the thickener. The filter cake drops to the floor in the Concentrate Loadout area and is loaded into concentrate trailers for shipment offsite.

The floor slopes are directed to the Copper Loadout Area sump. Solutions accumulated in the Copper Loadout Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The designed containment in the Copper Filtration Area is approximately 25,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Copper Filtration and Concentrate Loadout Area Containment</b>				

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No Storage Tanks	na	na	na	na
Containment Volume (gallons)			25,000	

**4.1.9 Wheel Wash Area**

The Wheel Wash Area (Dwg. 0000-GA-050) consists of the wheel wash tank and pump area and a concrete containment area in which the wheels of the concentrate trucks are washed to remove adhering copper concentrates before driving off onto the site roads.

The floor slopes are directed to the Wheel Wash Area sump. Solutions accumulated in the Wheel Wash Area sump are returned to the Copper Thickener feed box. The sump is equipped with a dedicated pump and automatic start/stop control.

The largest vessel is the Wheel Wash Surge Tank at a capacity of 560 gallons., which requires a containment capacity of approximately 620 gallons at 110% of capacity. The designed containment volume is 15,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Wheel Wash Area Containment</b>				
Wheel Wash Surge Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			15,000	

**4.1.10 Reagents Area**

The Reagents Area (Dwg. 0000-GA-050) includes equipment and storage tanks to contain reagents used in the process. This area is configured into four separate containments to separate and manage Lime Reagent, Diesel Reagent, Flotation Reagent, and Sodium Hydrosulfide (NaHS) Reagent.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Concentrator: Lime Reagent Containment</b>				
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Containment Volume (gallons)			30,000	
<b>Concentrator: Diesel Reagent Containment</b>				
No. 2 Diesel Storage Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gallons)			4,800	
<b>Concentrator: Flotation Reagent Containment</b>				

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Pax Mix Tank	8.00	10.67	4,000	Carbon Steel
Pax Distribution Tank	8.00	10.67	4,000	Carbon Steel
MIBC Storage Tank	8.00	6.00	2,000	Carbon Steel
AERO 238 Mix Tank	8.00	10.67	4,000	Carbon Steel
AERO 238 Distribution Tank	8.00	10.67	4,000	Carbon Steel
Moly Collector Mix Tank	8.00	6.00	2,300	Carbon Steel
Moly Collector Distribution Tank	8.00	6.00	2,300	Carbon Steel
Containment Volume (gallons)			10,200	
<b>Concentrator: Sodium Hydrosulfide Reagent Containment</b>				
NaHS Mix Tank	8.00	10.67	4,000	Stainless Steel
NaHS Distribution Tank	8.00	10.67	4,000	Stainless Steel
NaHS Stock Tank	8.00	10.67	4,000	Stainless Steel
Containment Volume (gallons)			4,900	

#### 4.1.10.1 Lime Reagent Area

The Lime Reagent Area (Dwg. 0000-GA-050) consists of a Lime Silo, Lime Slaker, and Milk of Lime tanks to receive pebble lime from the supplier and convert it to a lime slurry that can be used to raise the pH of process solutions.

The floor slopes are directed to the Lime Reagent Area sump. Solutions accumulated in the Lime Reagent Area sump are returned to the milk of lime distribution box. The containment will be inspected on a regular basis per regulatory requirements and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The largest vessels are the Milk of Lime Tanks at a capacity of 20,000 gallons., which requires a containment capacity of approximately 22,000 gallons at 110% of capacity. The designed containment volume is 30,000 gallons.

#### 4.1.10.2 Diesel Containment Area

Diesel fuel is used as a reagent in the flotation process. It is stored in the Diesel Tank in the Diesel Containment (Dwg. 0000-GA-050) and has its own containment area and sump.

The floor slopes are directed to the Diesel Containment Area sump. Leaked or spilled diesel fuel accumulating in the sump is pumped out using a portable pump and deposited in the Used Oil Storage Tank (Sec. 4.2). The containment will be inspected on a regular basis per regulatory requirements and solutions evacuated as soon as observed, and the source will be investigated and addressed.

The Diesel Containment Tank has a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons. The designed containment volume is 4,800 gallons.

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**4.1.10.3 Flotation Reagent Area**

The Flotation (General) Reagents Area contains tanks with compatible aqueous reagents that are used in the process.

The floor slopes are directed to the Flotation Reagent Area sump. Solutions accumulating in the sump are pumped to the tailings box to be combined with tailings and report to the TSF. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. The sump is equipped with a dedicated pump and manual start/stop control.

The largest vessels are the Aero 238 Tanks at a capacity of 4,000 gallons., which requires a containment capacity of approximately 4,400 gallons. The designed containment volume is 10,200 gallons.

**4.2 TRUCK SHOP**

The Truck Shop is located south east of the concentrator (Dwg. 0000-CI-008) and provides maintenance services for the mining and mobile equipment fleet. Tanks will be constructed at the truck shop (Dwg. 1010-AR-012) to store required maintenance fluids such as: motor oil, gear oil, hydraulic oil, engine coolant, and used oil and coolant for recycling.

The floor slopes are directed to the Truck Shop Tank Farm Area sump. Leaked or spilled diesel fuel accumulating in the sump is pumped out using a portable pump and deposited in the Used Oil Storage Tank for recycling. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The Used Oil Storage Tank has a capacity of 2,000 gallons., which requires a containment capacity of approximately 2,200 gallons. The designed containment volume is 5,600 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Truck Shop Tank Farm Containment</b>				
Used Oil Storage Tank	8.00	6.00	2,000	Carbon Steel
Used Anti-Freeze Storage Tank	8.00	6.00	2,000	Carbon Steel
ATF Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Hydraulic Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Engine Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Gear Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Anti-Freeze/Coolant Storage Tank	6.00	6.00	1,000	Carbon Steel
Containment Volume (gallons)			5,600	

**4.3 FUEL STATION**

A Fuel Station with secondary containment for project vehicles will be constructed on the haulage road west of the concentrator (Dwg. 0000-CI-008). Tanks, pumps, and piping associated with the fuel station will be positioned inside

**PROCESS FACILITY CONTAINMENT REPORT  
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the containment facility (Dwg. 1010-GA-010). During fueling and fuel offloading operations, vehicles and equipment will be parked on concrete pads that are sloped to drain into solution containment sumps. Liquid from the sumps will be evacuated into a portable used oil tank for transfer to the used oil holding facility for recycling.

The floor slopes at the fueling areas and within the tank containment are directed to the Fuel Station Area sumps. Solutions accumulated in the Fuel Station Area sumps are pumped out and transported offsite by a Certified used oil recycler. The containment will be inspected on a regular basis per regulatory requirement and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The largest vessel is the Off Road Diesel Fuel Storage Tank at a capacity of 100,000 gallons., which requires a containment capacity of approximately 110,000 gallons at 110% of capacity. The designed containment volume is 119,000 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Fuel Station Containment</b>				
Off Road Diesel Fuel Storage Tank	28.00	24.00	100,000	Carbon Steel
On Road Diesel Storage Tank	12.00	12.00	10,000	Carbon Steel
Gasoline Storage Tank	12.00	12.00	10,000	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gallons)			119,000	

**4.4 TRUCK AND EQUIPMENT WASHING UNIT**

A Truck and Equipment Washing Unit (Truck Wash) will be constructed along the haulage road from the mine to the Truck Shop (Dwg. 0000-CI-008). This facility is designed in accordance with 20.6.7.26 NMAC. The washing unit includes a concrete pad on which equipment will be parked for washing (Dwg. 1010-GA-001). The pad will be sloped to drain into a concrete basin for separating water, solids, oil, and grease. The water from the basin will be recycled and reused for washing equipment. Oil and grease will be skimmed from the settling basin and stored in sealed drums for proper disposal. After draining, sediment will be removed from the basin and either loaded directly into a dump truck for disposal at tailings or stored on a concrete pad adjacent to the facility for re-handling and disposal at the TSF. The settling basin will be designed to provide at least 12 inches of freeboard during operation. During periods of precipitation, excess water will be removed from the settling basin by a water truck and transported to the a process water impoundment for reuse.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Equipment Wash Pad</b>				
Settling Basin	40' W x 50' L x 3' D		50,000	Concrete
Containment Volume (gallons)			50,000	

4.5 ASSAY LABORATORY

The Assay Laboratory (Dwg. 0000-CI-008) uses a variety of reagents for sample process and analytical testing of mine and process samples. Sinks and drains in areas with potential chemical use are piped to a below-ground holding tank (Dwg. 3010-AR-003) so that they are not commingled with sanitary wastes from the building's restrooms and breakroom area. The chemical holding tank will be installed in a concrete, water tight vault as secondary containment.

The floor slopes in the Assay Lab Chemical Waste tank containment are directed to a collection sump. Solutions accumulated in the Assay Lab Chemical Waste sump are pumped out and taken to the TSF or transferred to a portable tank and removed from site by a certified disposal contractor. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

The only vessel is the Chemical Waste Tank at a capacity of 1,200 gallons., which requires a containment capacity of approximately 1,300 gallons at 110% of capacity. The designed containment volume is 7,500 gallons.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Assay Laboratory</b>				
Assay Lab Chemical Waste Tank	4.00	14.00	1,200	Polypropylene
Containment Volume (gallons)			7,700	

4.6 DOMESTIC WASTEWATER TREATMENT FACILITY

A packaged wastewater treatment facility (WWTF) for treatment of domestic sanitary will be installed on an existing concrete slab located near the Guardhouse at the main entrance (Dwg. 0000-CI-008). Concrete, water tight curbing will be installed at the perimeter of the foundation slab to provide a minimum volume of 5,000 gallons of secondary containment for the facility. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
<b>Sanitary Wastewater Treatment Facility</b>				
Sewage Treatment Equalization Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Treatment Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Solids Holding Tank	9.60	8.00	4,300	Carbon Steel
Containment Volume (gallons)			5,000	



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**4.7 PROCESS WATER STORAGE TANK**

Process water from the Process Water Reservoir will be pumped to the Process Water Storage Tank (Dwg. 0000-CI-008) on the slopes of Animas Peak to provide consistent line pressure using gravity head to supply water for use in the process. The tank will be installed within a lined earthen basin with a secondary containment capacity of at least 190,000 gallons, plus 2 feet of freeboard. The containment basin will be lined with 60 mil HDPE liner or equivalent. The containment will be inspected on a regular basis and solutions evacuated immediately as observed and the source will be investigated and addressed. Solutions will be evacuated from the containment using a portable pump. Any water collecting in the basin will be pumped out within 30 days and deposited in the Process Water Reservoir.

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gallons)	TANK MATERIAL
Miscellaneous Site				
Process Water Head Tank	30.00	32.00	170,000	Carbon Steel

## TABLES

**Table 1: List of Tanks and Process Solution Containment Areas**

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
<b>Concentrator: Grinding Area Containment</b>				
Gravity Concentrator Concentrate Tank	12.00	9.50	8,000	Carbon Steel
Containment Volume (gal)			200,000	
<b>Concentrator: Copper Flotation Area Containment</b>				
Rougher Flotation Conditioning Tank	22.00	25.00	69,300	Carbon Steel
Containment Volume (gal)			79,000	
<b>Concentrator: Copper Regrind Area Containment</b>				
Copper 1st Cleaner Conditioning Tank	12.00	7.25	6,000	Carbon Steel
Containment Volume (gal)			8,900	
<b>Concentrator: Moly Flotation Area Containment</b>				
Cu-Mo Separation Conditioning Tank	4.00	10.67	1,000	Carbon Steel
Moly Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Moly Filter Feed Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gal)			8,300	
<b>Concentrator: Copper-Moly Thickening Area Containment</b>				
Flocculant Mix Tank	12.00	7.25	6,100	Carbon Steel
Flocculant Distribution Tank	12.00	7.25	6,100	Carbon Steel
Cu-Mo Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Cu-Mo Concentrate Thickener Overflow Tank	4.00	10.67	1,000	Carbon Steel
Containment Volume (gal)			13,000	
<b>Concentrator: Copper Thickening Area Containment</b>				
Cu-Mo Concentrate Thickener	20.00	7.50	17,700	Carbon Steel
Cu Concentrate Thickener Overflow Tank	4.00	4.00	375	Carbon Steel
Cu Concentrate Stock Tank	17.00	25.00	41,760	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Copper Filtrate Tank	4.00	4.00	375	Carbon Steel
Cu Concentrate Filter Cloth Wash Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			50,000	
<b>Concentrator: Copper Filtration and Concentrate Loadout Area Containment</b>				
No Storage Tanks				
Containment Volume (gal)			25,000	

**Table 1: List of Tanks and Process Solution Containment Areas**

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
<b>Concentrator: Copper Filtration Wheel Wash Area Containment</b>				
Wheel Wash Surge Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			15,000	
<b>Concentrator: Lime Reagent Containment</b>				
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Milk of Lime	12.00	23.67	20,000	Carbon Steel
Containment Volume (gal)			30,000	
<b>Concentrator: Diesel Reagent Containment</b>				
No. 2 Diesel Storage Tank	8.00	6.00	2,000	Carbon Steel
Containment Volume (gal)			4,800	
<b>Concentrator: General Reagent Containment</b>				
Pax Mix Tank	8.00	10.67	4,000	Carbon Steel
Pax Distribution Tank	8.00	10.67	4,000	Carbon Steel
MIBC Storage Tank	8.00	6.00	2,000	Carbon Steel
AERO 238 Mix Tank	8.00	10.67	4,000	Carbon Steel
AERO 238 Distribution Tank	8.00	10.67	4,000	Carbon Steel
Moly Collector Mix Tank	8.00	6.00	2,300	Carbon Steel
Moly Collector Distribution Tank	8.00	6.00	2,300	Carbon Steel
Containment Volume (gal)			10,200	
<b>Concentrator: Sodium Hydrosulfide Reagent Containment</b>				
NaHS Mix Tank	8.00	10.67	4,000	Stainless Steel
NaHS Distribution Tank	8.00	10.67	4,000	Stainless Steel
NaHS Stock Tank	8.00	10.67	4,000	Stainless Steel
Containment Volume (gal)			4,900	

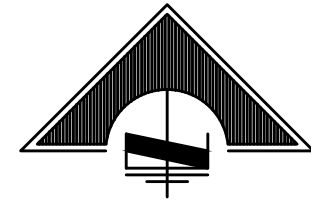
**Table 1: List of Tanks and Process Solution Containment Areas**

TANK DESCRIPTION	TANK DIAMETER (ft)	TANK HEIGHT (ft)	TANK VOLUME (gal)	TANK MATERIAL
<b>Truck Shop Tank Farm Containment</b>				
Used Oil Storage Tank	8.00	6.00	2,000	Carbon Steel
Used Anti-Freeze Storage Tank	8.00	6.00	2,000	Carbon Steel
ATF Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Hydraulic Fluid Storage Tank	6.00	6.00	1,000	Carbon Steel
Engine Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Gear Oil Storage Tank	6.00	6.00	1,000	Carbon Steel
Anti-Freeze/Coolant Storage Tank	6.00	6.00	1,000	Carbon Steel
Containment Volume (gal)			5,600	
<b>Fuel Station Containment</b>				
Off Road Diesel Fuel Storage Tank	28.00	24.00	100,000	Carbon Steel
On Road Diesel Storage Tank	12.00	12.00	10,000	Carbon Steel
Gasoline Storage Tank	12.00	12.00	10,000	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Urea Tank	4.00	6.00	560	Carbon Steel
Containment Volume (gal)			119,000	
<b>Equipment Wash Pad</b>				
Settling Basin	40' W x 50' L x 3' D		50,000	Concrete
Containment Volume (gal)			50,000	
<b>Miscellaneous Site</b>				
Process Water Head Tank	30.00	32.00	170,000	Carbon Steel
Assay Lab Chemical Waste Tank	4.00	14.00	1,200	Polypropylene
Sewage Treatment Equalization Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Treatment Tank	9.60	8.00	4,300	Carbon Steel
Sewage Treatment Solids Holding Tank	9.60	8.00	4,300	Carbon Steel

Table 2: Containment and Cleanout Sumps

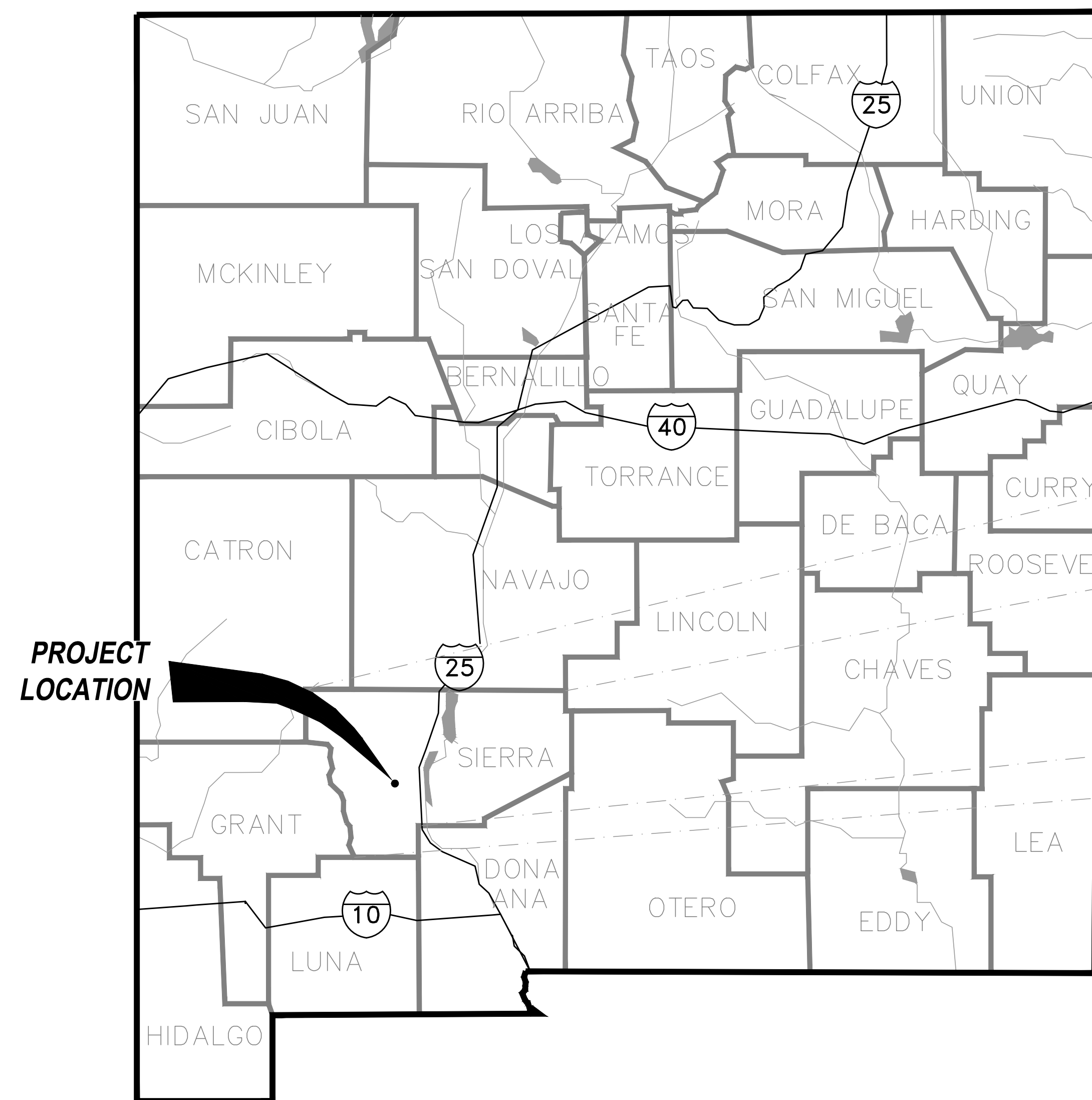
Location	Reports To
Crusher	Recalim Tunnel Sump
Reclaim Tunnel	Primary Cyclone Feed System
Grinding Area	Primary Cyclone Feed System
Copper Flotation Area	Rougher Flotation Conditioning Tank
Copper Regrind Area	Copper Regrind Cyclone Feed Box
Mo Flotation Area	Copper-Molybdenum Separation Conditioning Tank
Copper-Molybdenum Thickening Area	Copper-Molybdenum Thickener
Copper Thickening Area	Copper Concentrate Thickener
Copper Filtration and Concentrate Loadout Area	Copper Concentrate Thickener
Copper Filtration Wheel Wash Area	Copper Concentrate Thickener
Lime Containment Area	Lime Distribution Box
Diesel Reagent Containment Area	Used Oil Tank
General Reagent Containment Area	Tailings Sump
Sodium Hydrosulfide Reagent Containment Area	Copper-Molybdenum Separation Conditioning Tank
Mobile Equipment Shop Containment	Certified Used Oil Recycler
Fuel Station Containment	Certified Used Oil Recycler
Heavy Equipment Fuel Pad	Certified Used Oil Recycler
Light Vehicle Fuel/Fuel Offload Pad	Certified Used Oil Recycler
Equipment Wash Pad Settling Basin	Closed Basin; Zero Discharge
Process Water Head Tank Containment	Process Water Reservoir
Sewage Treatment Plant	Sewage Treatment Plant or Sanitary Waste Disposal Contractor
Chemical Waste Containment	TSF or Certified Contractor Disposal

## DRAWINGS

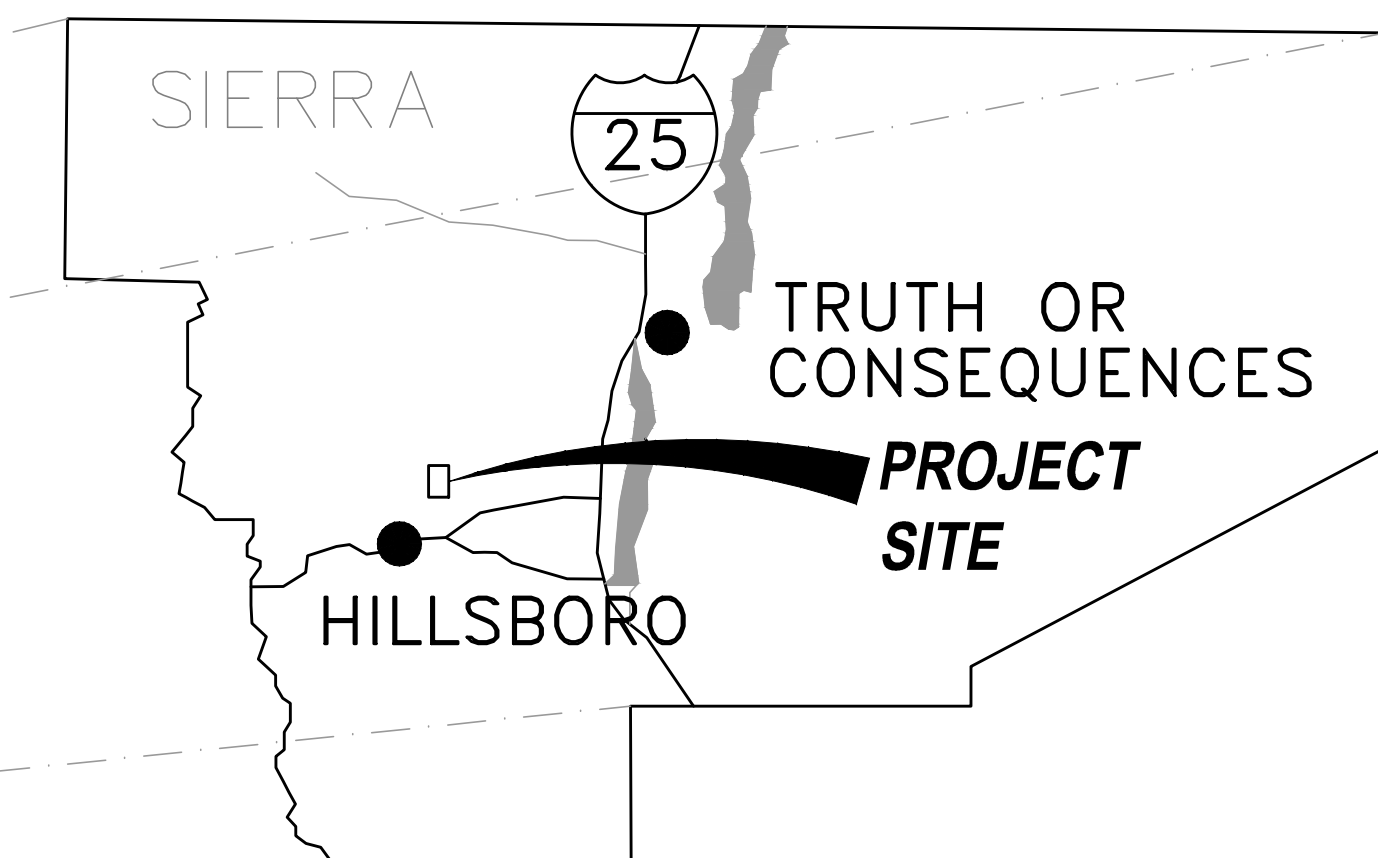


# NEW MEXICO COPPER CORPORATION

## COPPER FLAT PROJECT SIERRA COUNTY, NEW MEXICO



**STATE OF NEW MEXICO**  
NOT TO SCALE



**PROJECT LOCATION MAP**  
NOT TO SCALE

File: P:\2013\20080\Civil (644)\544.2 - Dept\Updates for Permit\00000000.dwg LAST UPDATE: 12/22/2015 10:37 AM BY: JN699

DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



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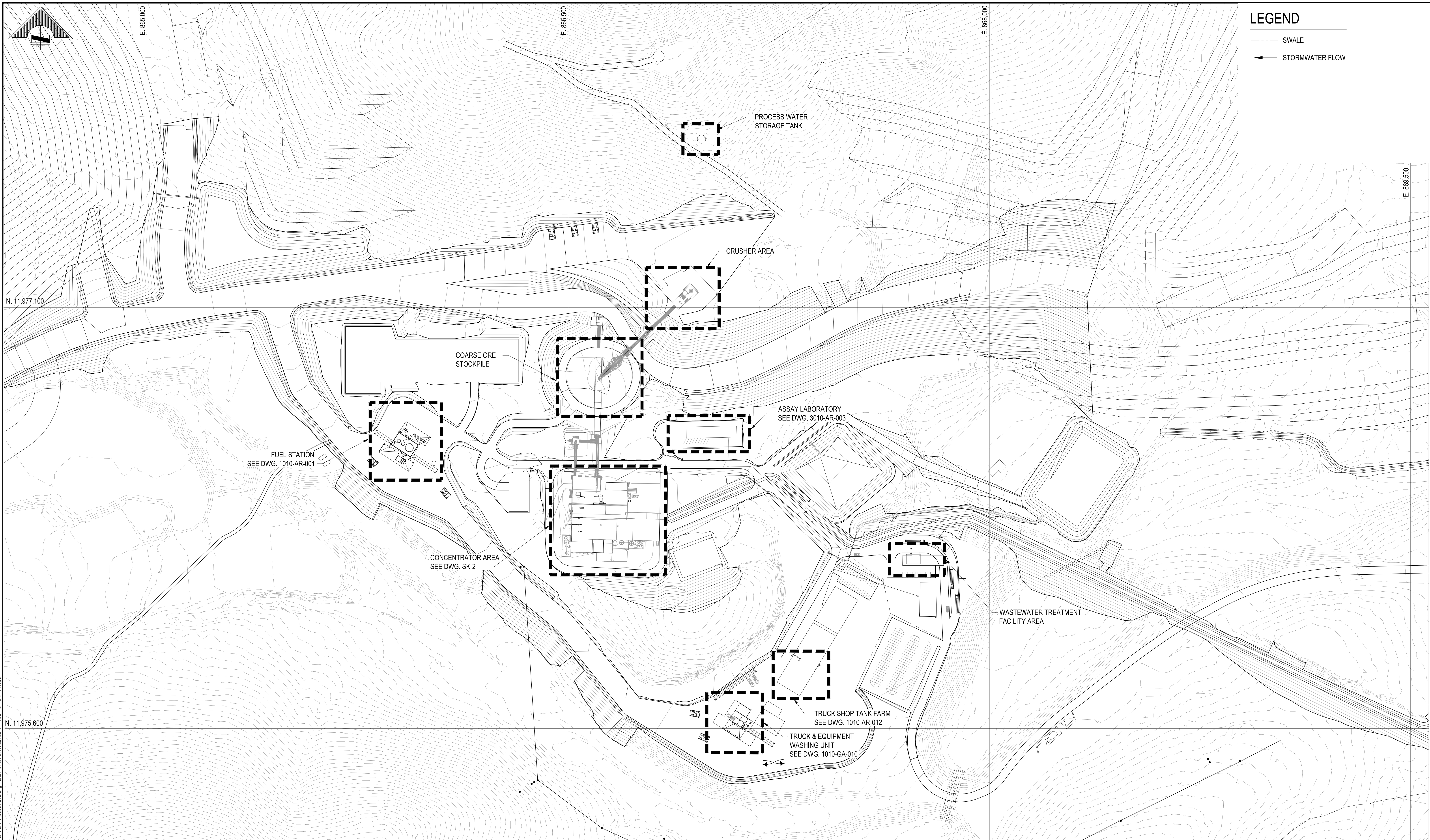
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ENGINEERING  
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**COPPER FLAT PROJECT**

**SITE GENERAL  
CIVIL  
PROJECT AREA  
SITE LOCATION PLAN**

JOB NO. M3 PN-120085  
DWG. NO. **0000-CI-001**  
REV. NO. P4 DATE 05 MAR 13





**LEGEND**  
 - - - - - SWALE  
 ← STORMWATER FLOW

N. 11,977,100

N. 11,975,600

E. 865,000

E. 866,500

E. 868,000

E. 869,500

**SITE PLAN**  
 SCALE: 1:150



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**PRELIMINARY**  
 FOR AGENCY REVIEW



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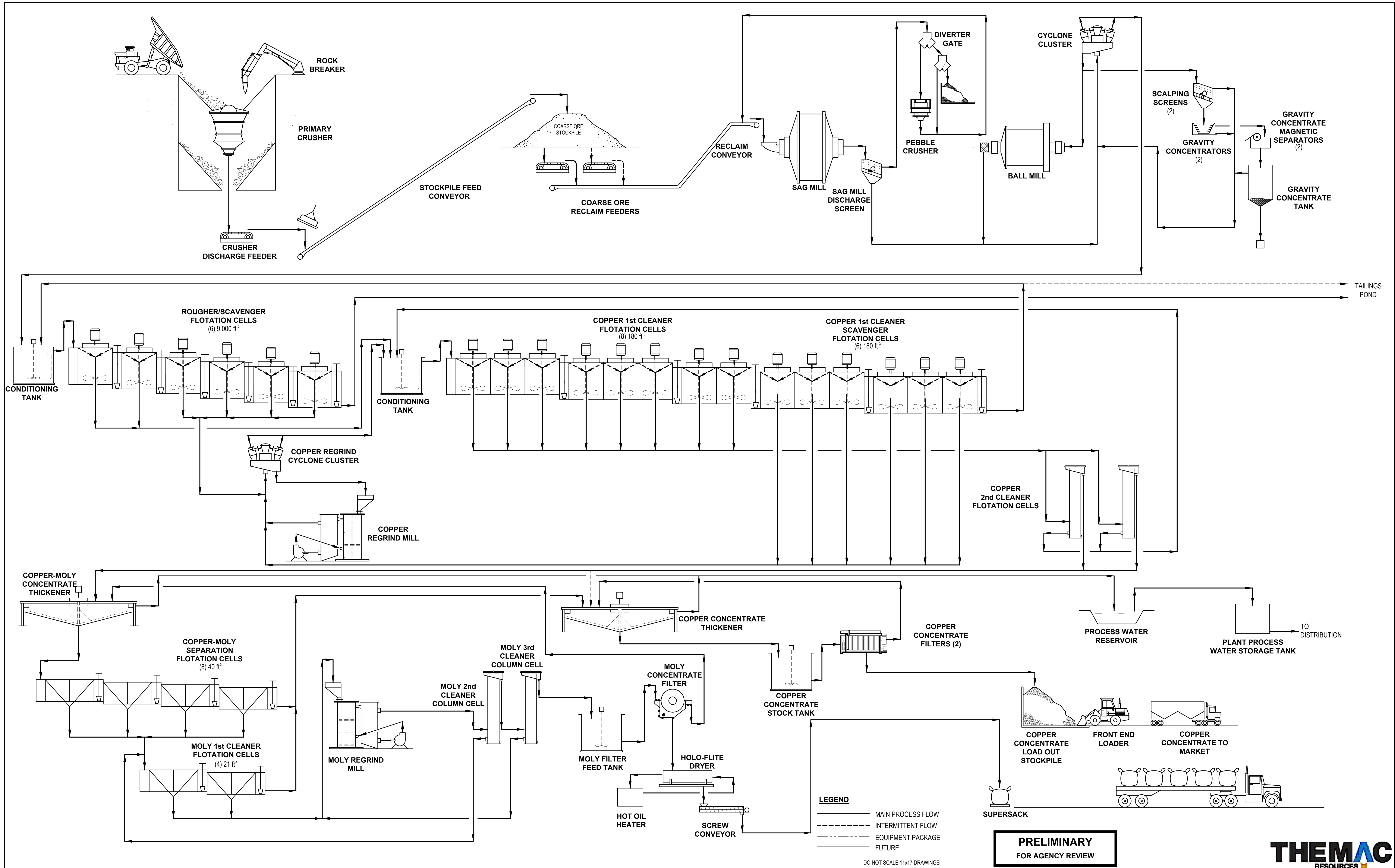
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CLIENT APPR.		

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**COPPER FLAT PROJECT**  
 SITE GENERAL  
 CIVIL  
 PROJECT AREA  
 PROCESS FACILITY CONTAINMENT AREAS

JOB NO. M3 PN-120085  
 DWG NO. **0000-CI-008**  
 REV NO. p1  
 DATE 25 NOV 15

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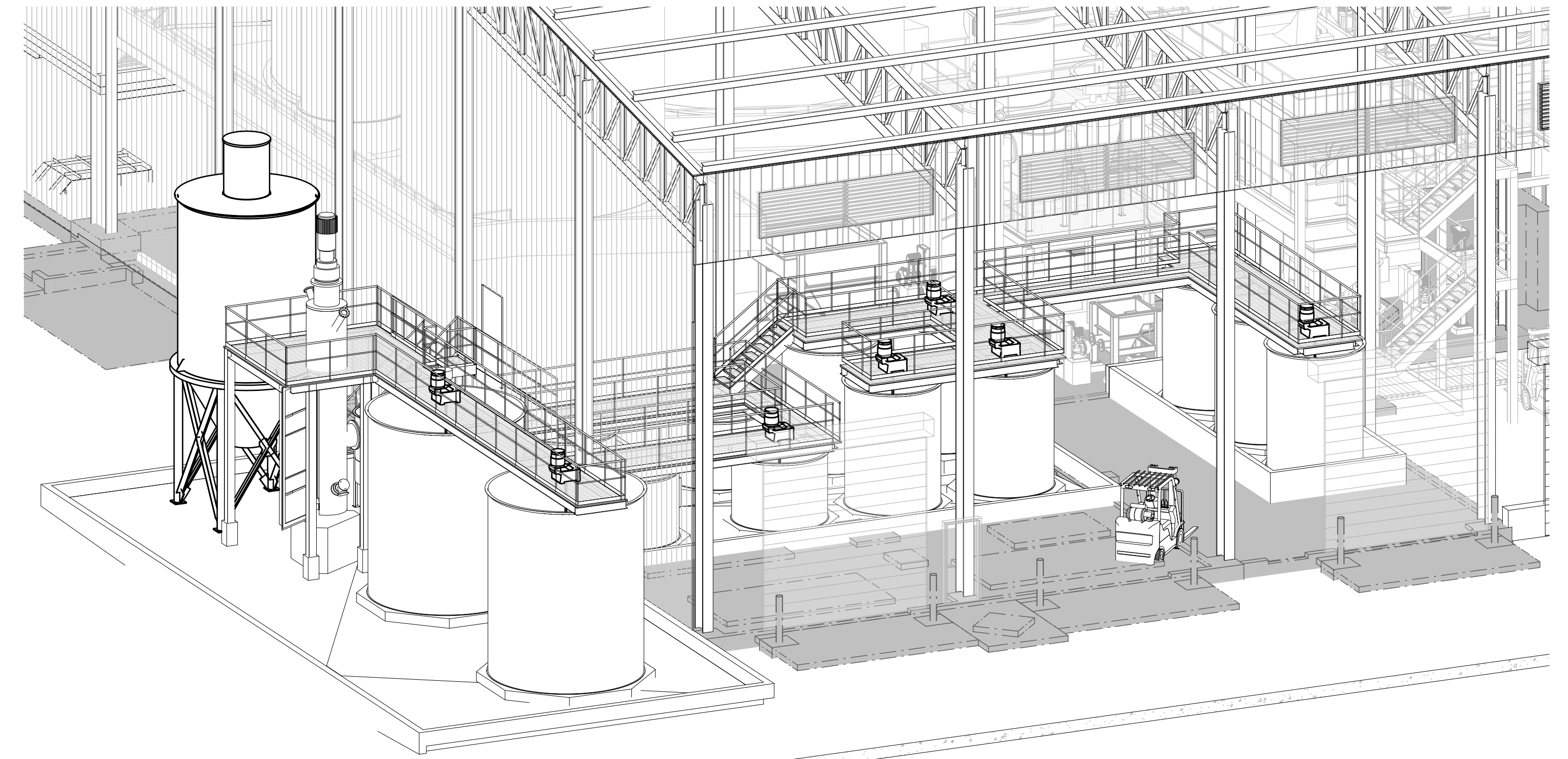
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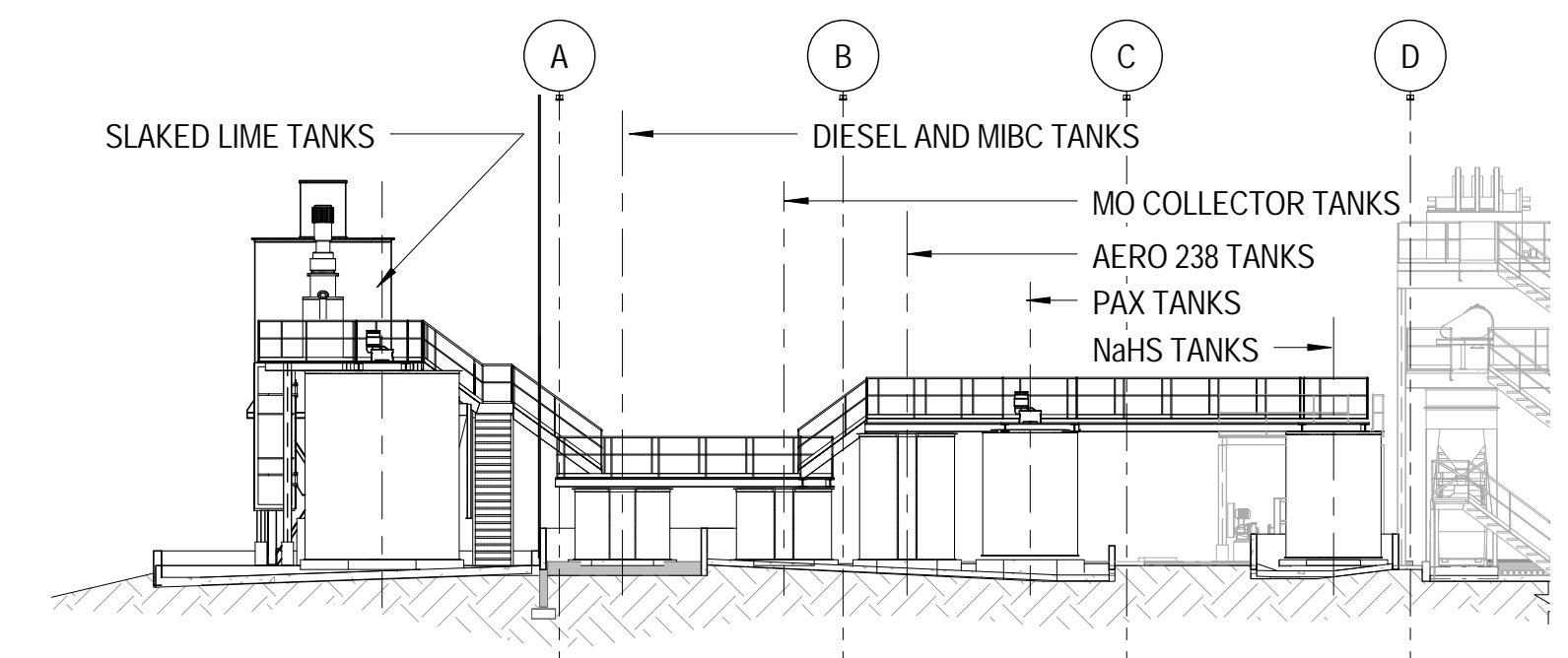
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PROCESSING PLANT  
30,000 TPD  
CONVENTIONAL TAILINGS  
OVERALL FLOW SHEET

JOB NO. M3 PN-120065  
DWG NO. 3000-FS-000  
REV NO. P6  
DATE 18 MAR 13

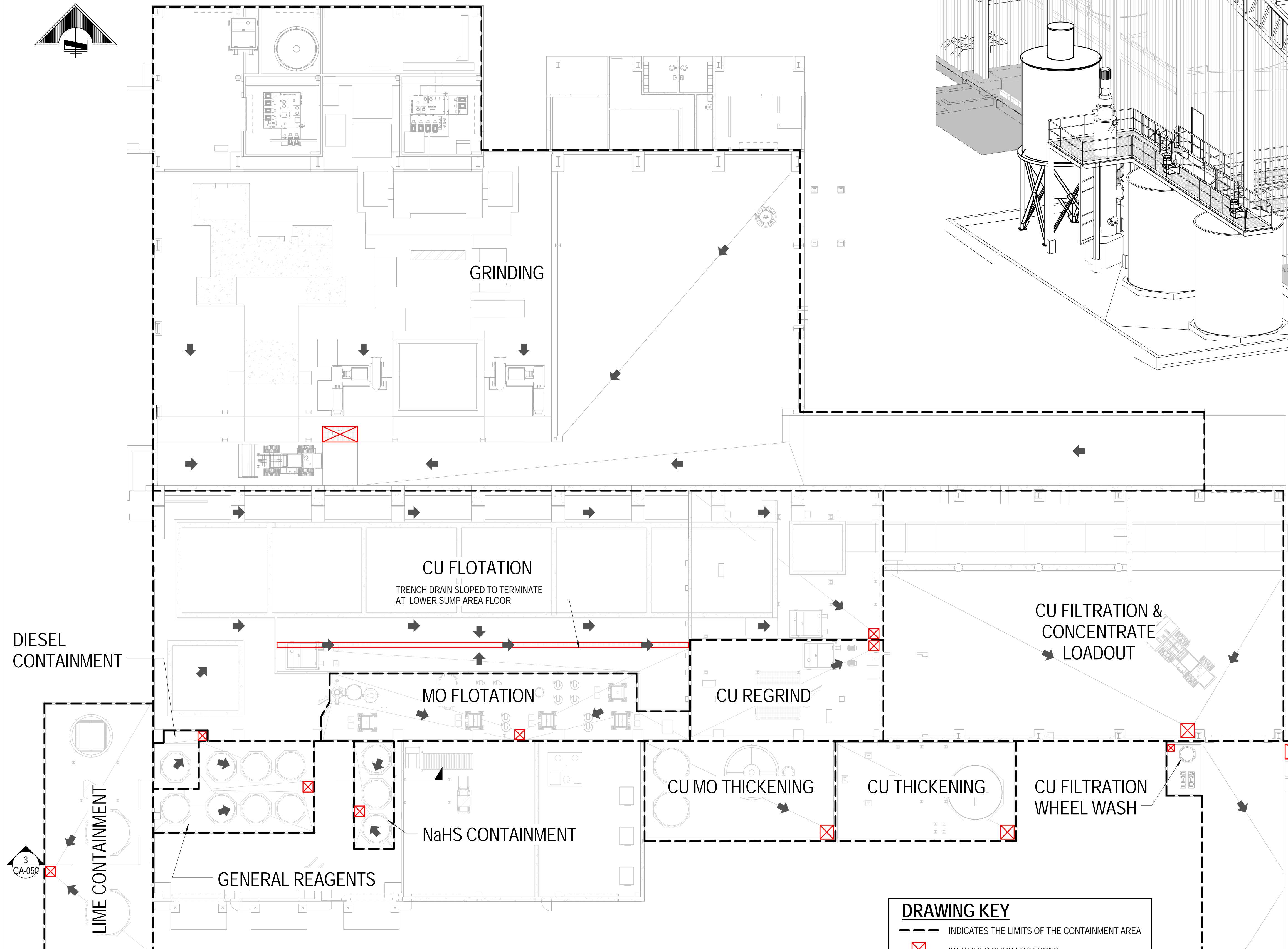


**REAGENTS AREA ISOMETRIC**



**REAGENTS SECTION**  
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3  
GA-050



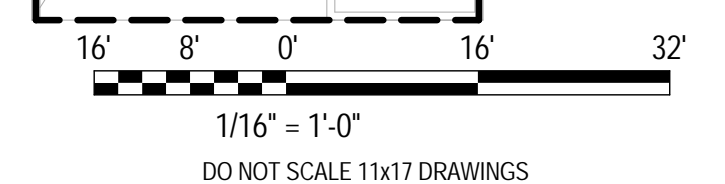
**CONCENTRATOR CONTAINMENT PLAN**

**DRAWING KEY**

- INDICATES THE LIMITS OF THE CONTAINMENT AREA
- ⊠ IDENTIFIES SUMP LOCATIONS
- ➔ DEMONSTRATES FLOW DIRECTION

Containment Area	Tank Name	Tank Volume (gal)	Containment Volume	
			Required (gal)	Provided (gal)
General Reagents	PAX Mix Tank*	4,000	4,400	10,200
	PAX Distribution Tank	4,000		
	MIBC Storage Tank	2,000		
	Moly Collector Mix Tank	2,300		
	Moly Collector Distribution Tank	2,300		
	AERO 238 Mix Tank	4,000		
	AERO 238 Distribution Tank	4,000		
Diesel Containment	No. 2 Diesel Storage Tank*	2,000	2,200	4,800
Lime Containment	Milk of Lime Storage Tank*	20,000	22,000	30,000
	Milk of Lime Storage Tank	20,000		
NaHS Containment	NaHS Distribution Tank*	4,000	4,400	4,900
	NaHS Mix Tank	4,000		
	NaHS Stock Tank	4,000		
Cu Flotation	Rougher Flotation Tank*	66,000	72,600	79,000
Cu Regrind	Cu 1st Cleaner Conditioning Tank*	1,000	1,100	8,900
Mo Flotation	Filter Feed Tank*	2,000	2,200	8,300
Cu Thickening	Cu Concentrate Stock Tank*	41,800	46,000	50,000
Cu-Mo Thickening	Cu-Mo Concentrate Thickener*	17,700	19,500	29,000
Cu Filtration-Wheel Wash	Wheel Wash Surge Tank*	560	620	750

\* - Denotes largest tank in containment area



**PRELIMINARY**  
NOT FOR CONSTRUCTION



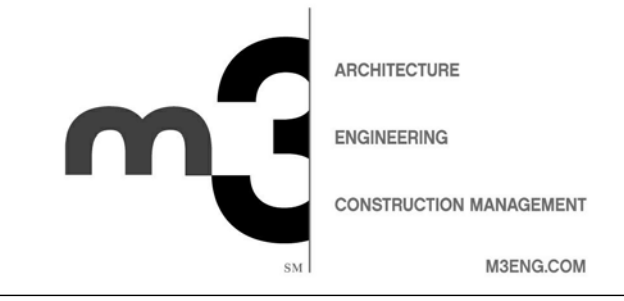
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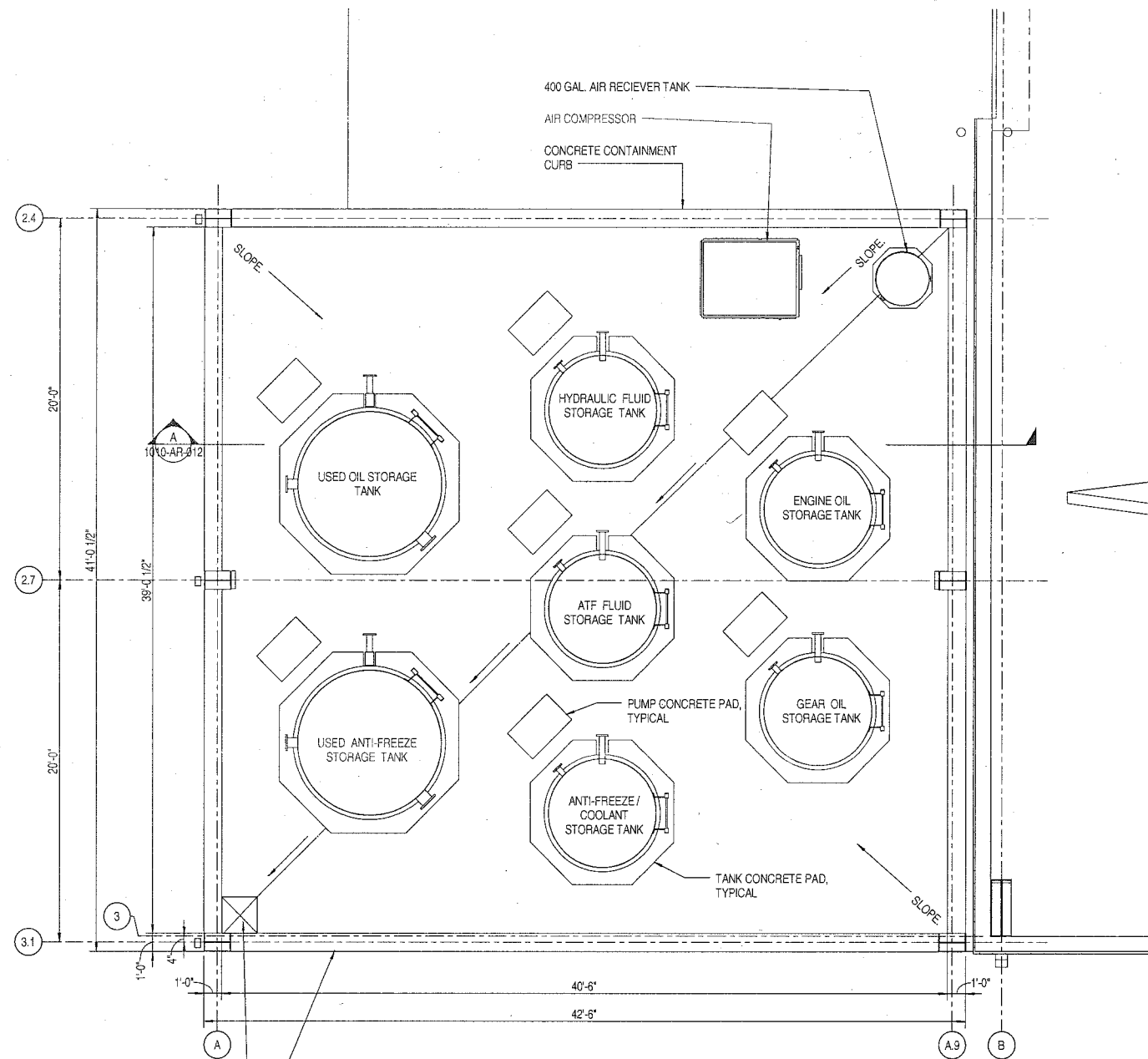
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GENERAL ARRANGEMENT  
CONCENTRATOR AREA  
CONTAINMENT PLAN**

PROJECT NO.	M3-PN120085
DWG NO.	0000-GA-050
REV NO.	P1
DATE	11/30/15

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CHECKED BY:	AF	NOV 15	
PROJECT MGR:			
CLIENT APPR:			



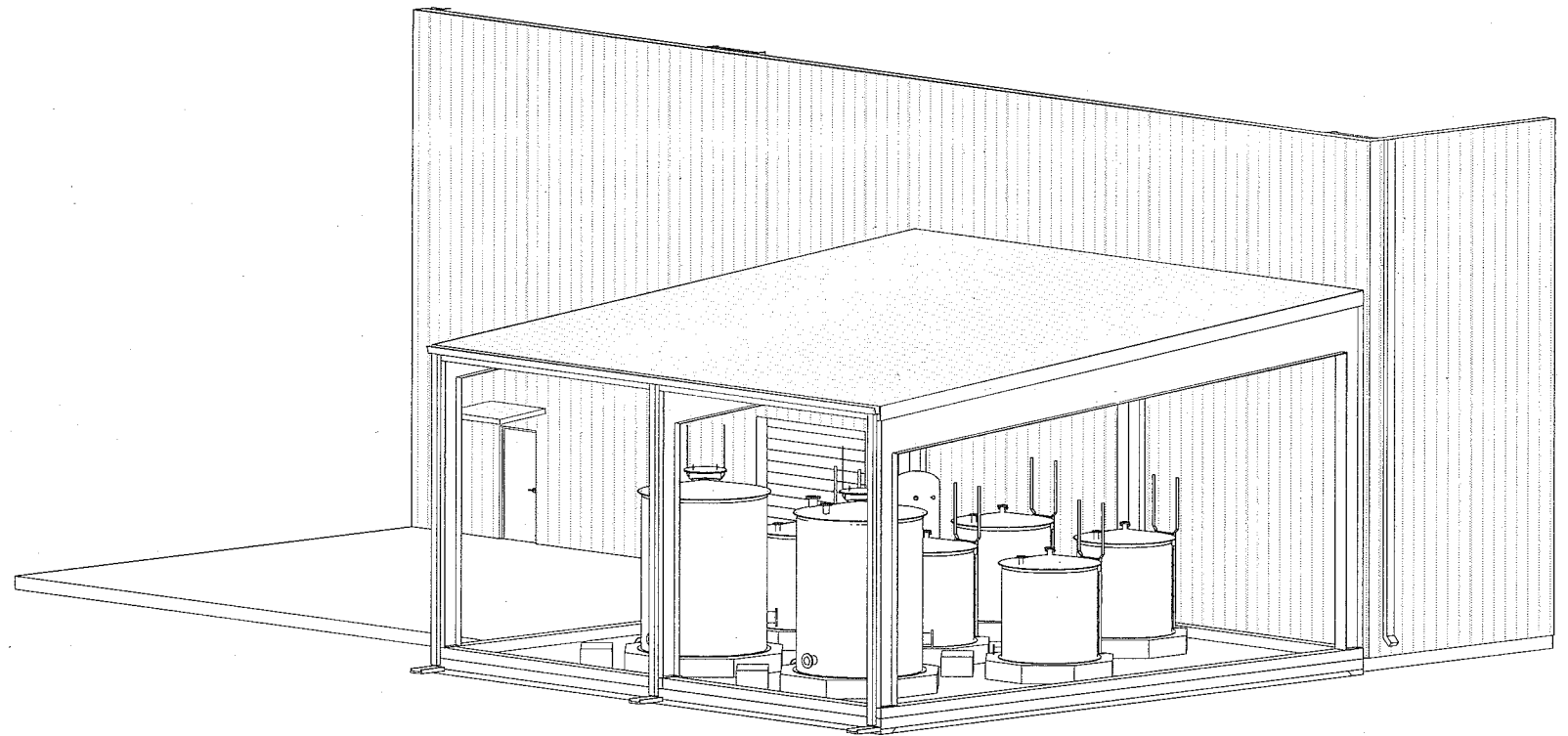


**TANK FARM CONTAINMENT FLOOR PLAN**

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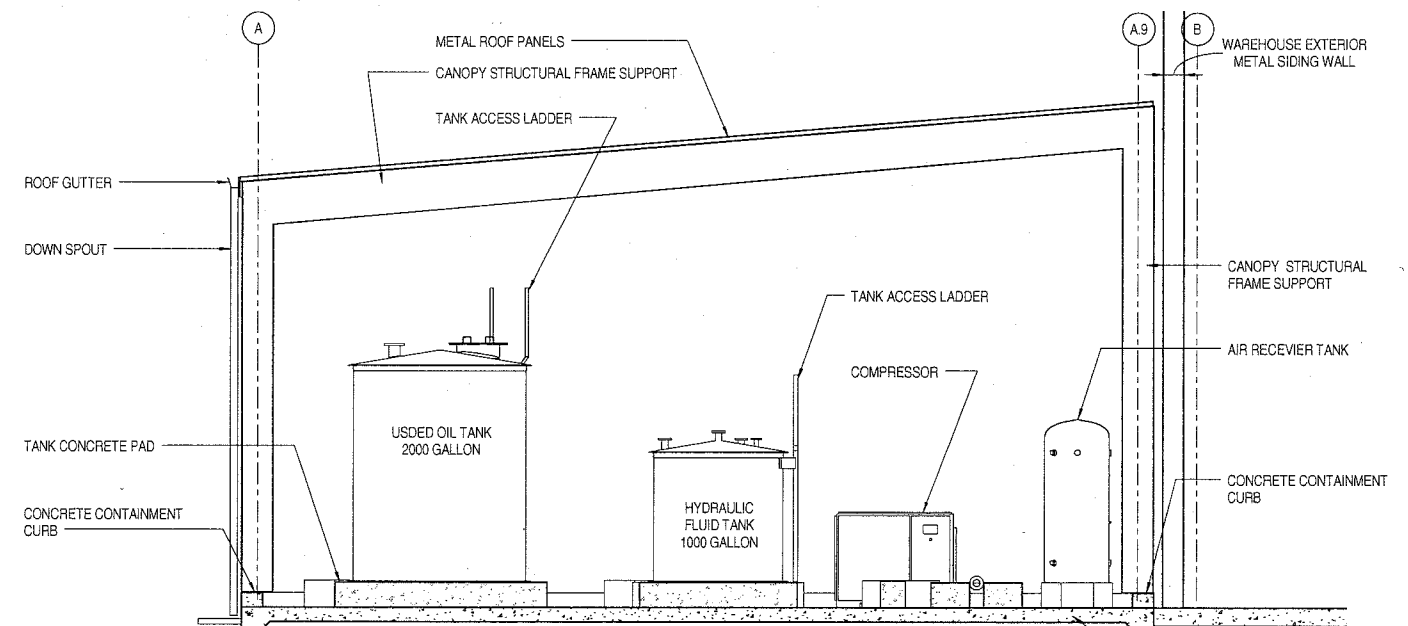
CONTAINMENT AREA	TANK NAME	TANK VOLUME (GAL.)	CONTAINMENT VOLUME	
			REQUIRED (GAL.)	PROVIDED (GAL.)
TRUCK SHOP TANK FARM.	USED OIL STORAGE TANK *	2,000	2,200	5,600
	USED ANTI-FREEZE STORAGE TANK	2,000		
	ATF FLUID STORAGE TANK	1,000		
	HYDRAULIC FLUID STORAGE TANK	1,000		
	ENGINE OIL STORAGE TANK	1,000		
	GEAR OIL STORAGE TANK	1,000		
	ANTI-FREEZE/COOLANT STORAGE TANK	1,000		

\* DENOTES LARGEST TANK IN CONTAINMENT AREA



**TRUCK SHOP - TANK FARM CONTAINMENT**

SCALE:



**CROSS SECTION**

SCALE: 1/4" = 1'-0"

PRELIMINARY  
NOT FOR CONSTRUCTION

**THEMAC**  
RESOURCES

REFERENCES		REFERENCES		REVISIONS						REVISIONS					
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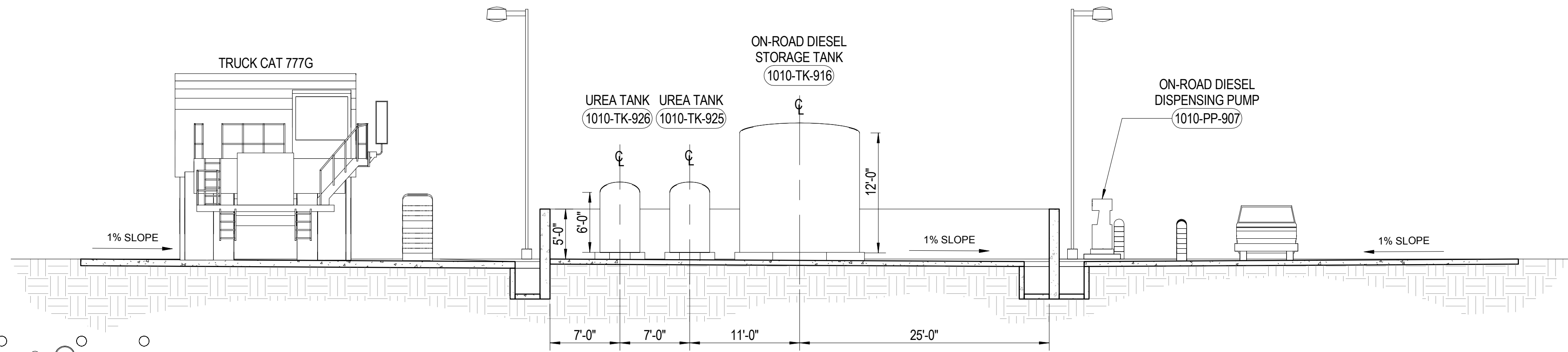
DO NOT SCALE 1/4"=1'-0" DRAWINGS

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ENGINEERS  
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**COPPER FLAT PROJECT**

ADMINISTRATION AREA  
ARCHITECTURAL  
TRUCK SHOP  
TANK FARM CONTAINMENT

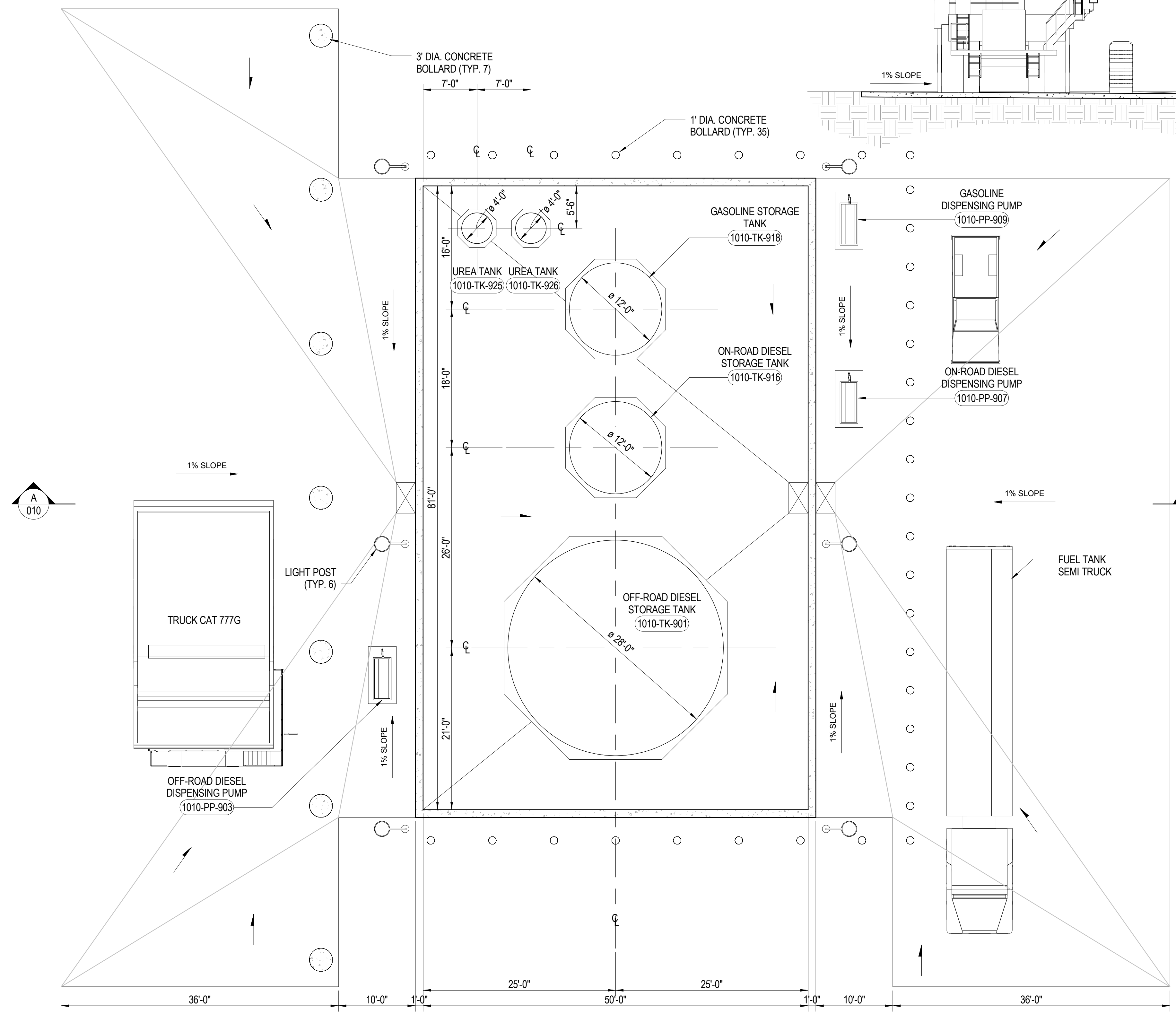
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DWG NO. **1010-AR-012**  
REV NO. P2 DATE 11/18/15



**SECTION**

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010



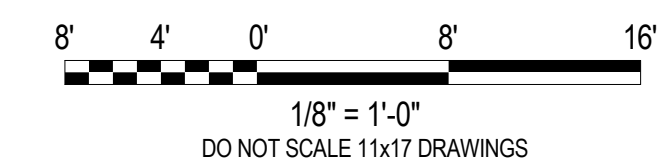
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	DEMONSTRATES FLOW DIRECTION

CONTAINMENT AREA	TANK NAME	TANK VOLUME	CONTAINMENT VOLUME	
		( GAL.)	REQUIRED ( GAL.)	PROVIDED ( GAL.)
FUEL STATION TANK FARM	OFF-ROAD DIESEL STORAGE TANK *	100,000	110,000	119,000
	ON-ROAD DIESEL STORAGE TANK	10,000		
	GASOLINE STORAGE TANK	10,000		
	UREA TANK	560		
	UREA TANK	560		

\* DENOTES LARGEST TANK IN CONTAINMENT AREA

**FUEL STATION CONTAINMENT PLAN**

SCALE: 1/8" = 1'-0"



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**THE MAC**  
RESOURCES

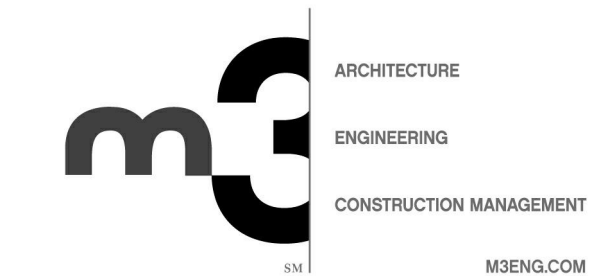
**COPPER FLAT PROJECT**

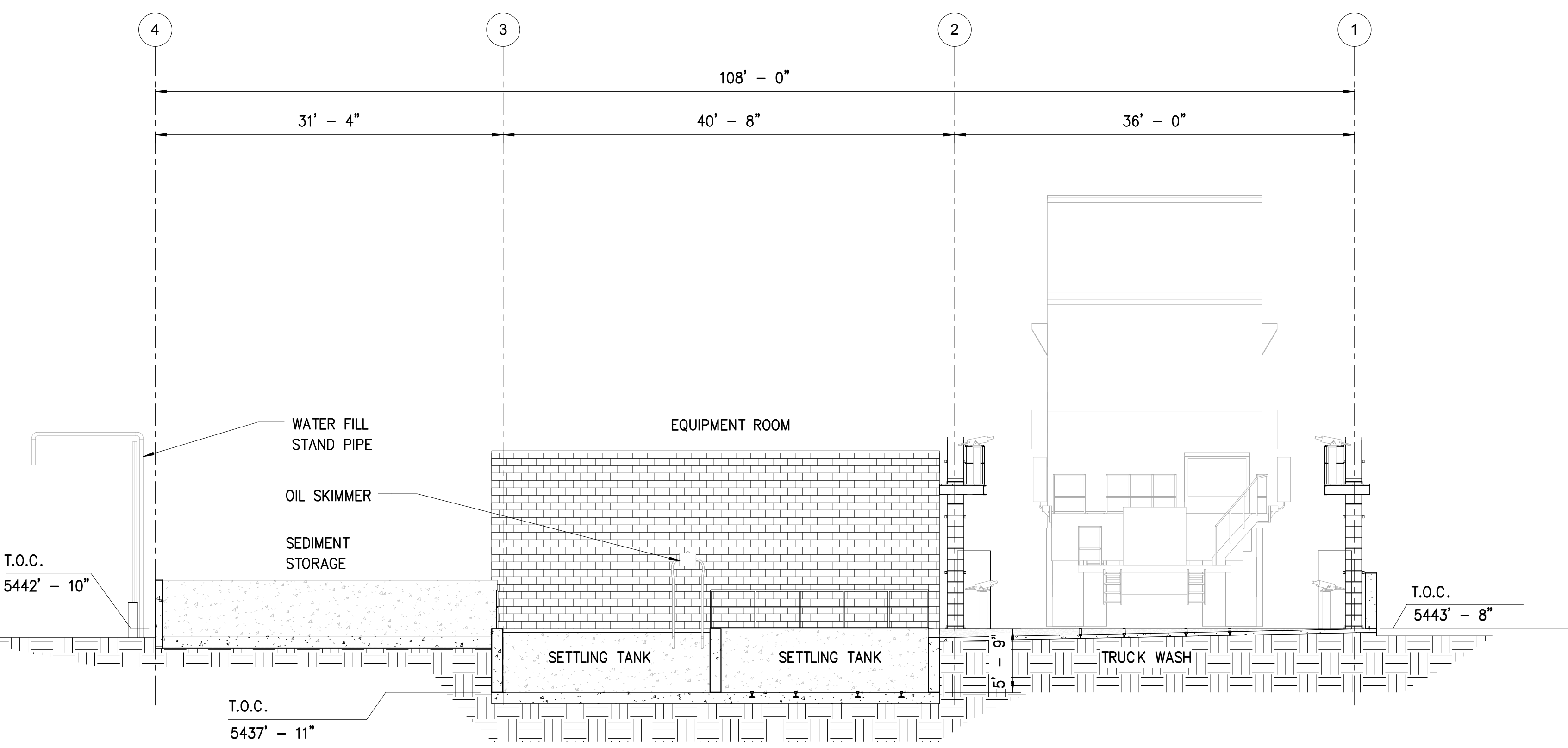
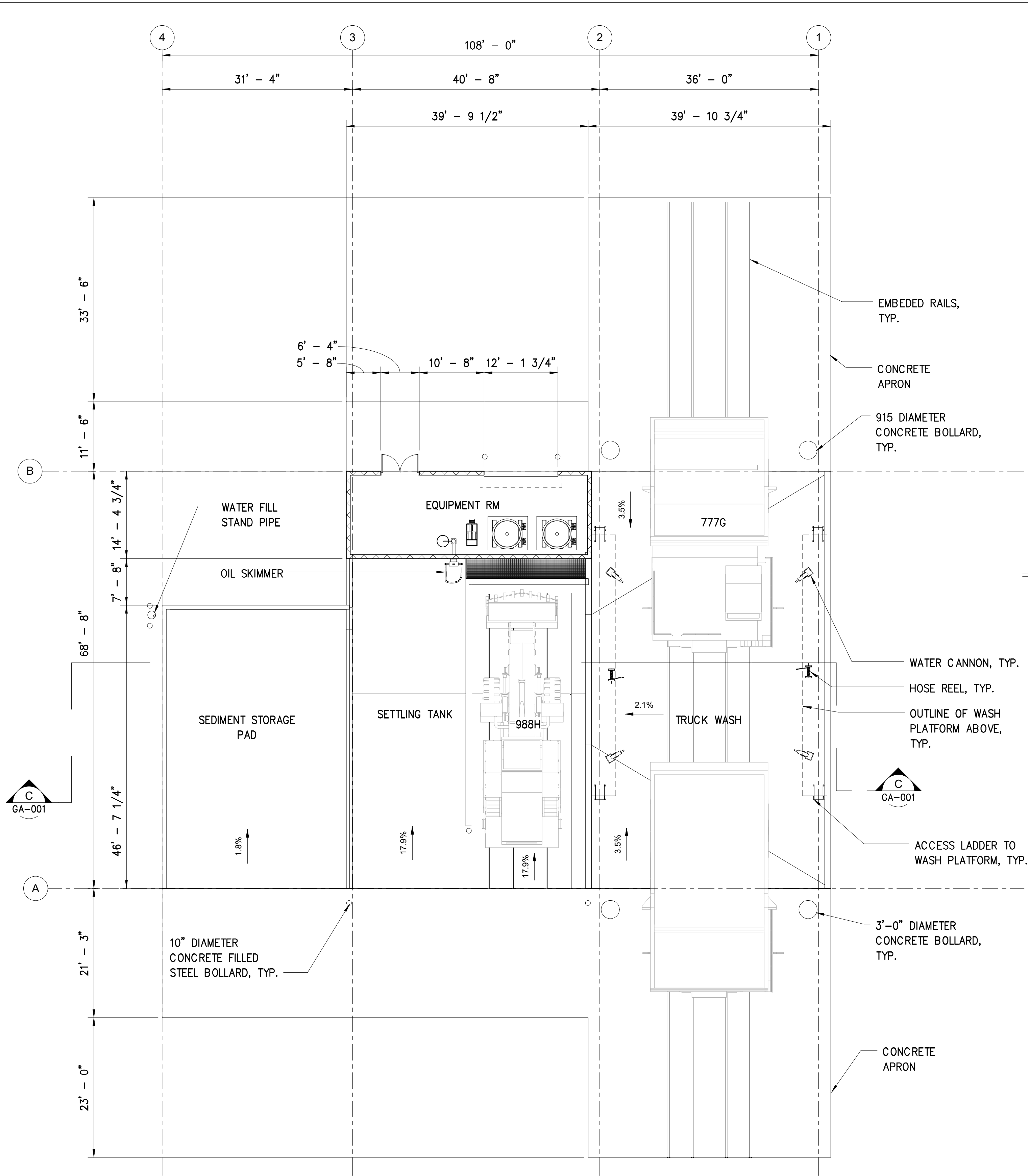
**ADMINISTRATION AREA  
GENERAL ARRANGEMENT  
FUEL STATION  
CONTAINMENT PLAN**

PROJECT NO.	M3-PN120085
DWG NO.	<b>1010-GA-010</b>
REV. NO.	P1
DATE	19 NOV 15

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DESIGNED BY:	RZ	DATE:	NOV 15
DRAWN BY:	JC	DATE:	NOV 15
CHECKED BY:			
PROJECT MGR:			
CLIENT APPR:			





**FLOOR PLAN**  
SCALE: 3/32" = 1'-0"

**SECTION C-C**  
SCALE: 1/8" = 1'-0"

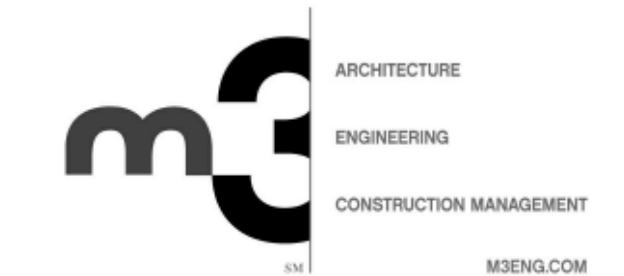
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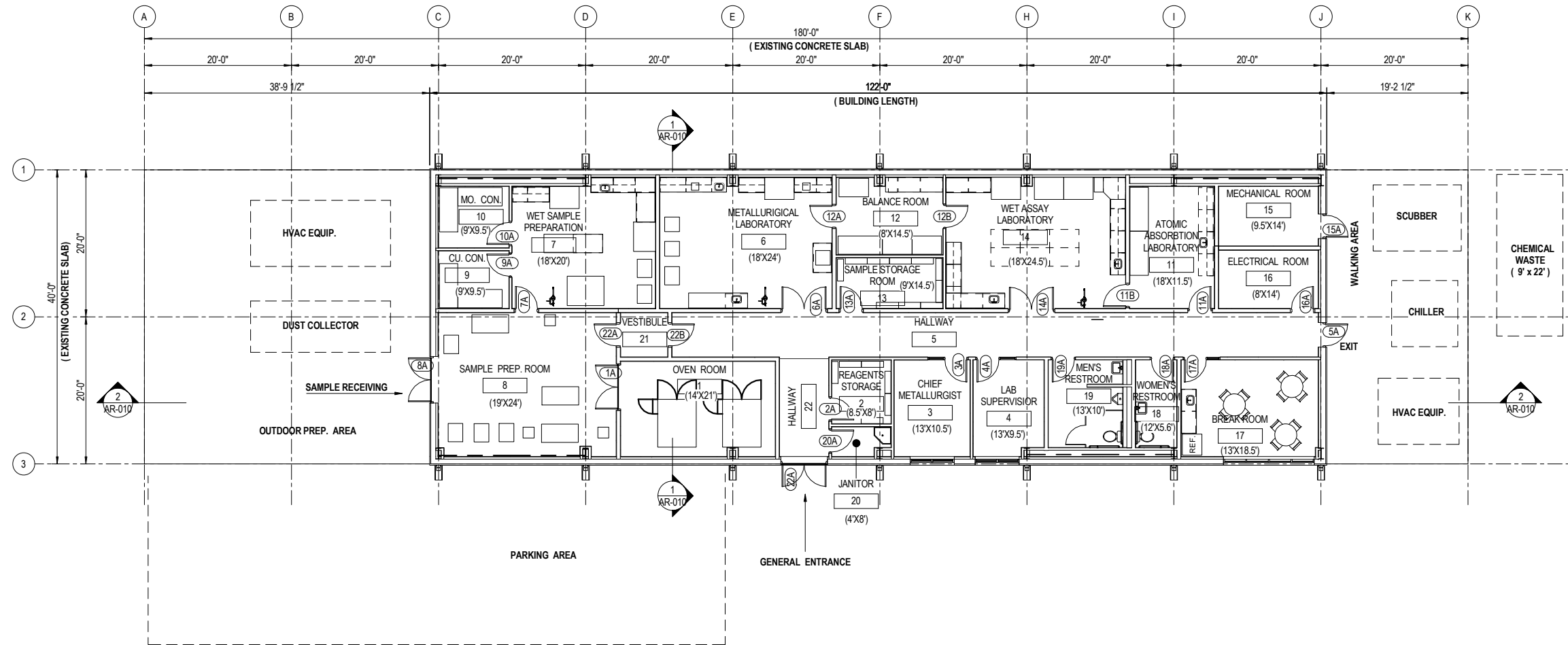
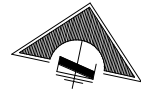
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DESIGNED BY SDI	NOV 15
DRAWN BY SDI	NOV 15
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PROJECT MGR	
CLIENT APPR	



<b>COPPER FLAT PROJECT</b>		PROJECT NO. M3-PN120085
<b>ADMINISTRATION AREA GENERAL ARRANGEMENT TRUCK WASH FLOOR PLAN &amp; SECTION</b>		DWG NO. <b>1010-GA-001</b>
REV NO. P1	DATE 20 NOV 15	

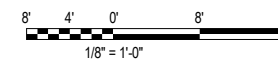


**FLOOR PLAN**

SCALE: 1/8" = 1'-0"

**ANALYSIS:**

1. VENDOR ENGINEERED PREFABRICATED METAL BUILDING.
2. NEW BUILDING SIZE: 122'-0" x 40'-0" = 4,880 SQ. FT.
3. EXISTING CONCRETE SLAB SQUARE FOOTAGE = 7,200 SQ. FT.
4. COLUMN LINE SPACING SAME AS PRIOR BUILDING, UTILIZING EXISTING FOUNDATIONS.



PRELIMINARY  
NOT FOR CONSTRUCTION



REFERENCES		REFERENCES		REVISIONS						REVISIONS					
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT	NO.	DESCRIPTION	BY	APPD	DATE	CLIENT

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DESIGNED BY R. RAMOS	NOV. 12
DRAWN BY R. RAMOS	NOV. 12
CHECKED BY R.S.	NOV. 12
PROJECT MGR	
CLIENT APPR	

**COPPER FLAT PROJECT**  
**CONCENTRATOR AREA**  
**ARCHITECTURAL**  
**LABORATORY**  
**FLOOR PLAN**

PROJECT NO. M3-PN120085
DWG. NO. <b>3010-AR-003</b>
REV. NO. P2
DATE 5 DEC. 12

**Appendix D**

**Copper Flat**

**Site Diversion Analysis**

**M3 Engineering & Technology Corporation**

**Revised, June 2016**



# Copper Flat Project



## Site Diversion Analysis Report

Prepared For:

**THEMAC**  
RESOURCES 

**Certified Professional Engineer Seal**

This report documents work conducted under the oversight of the following Engineer:

Harry Lewsley, P.E.



Harry Lewsley  
Signature


Exp. 12/31/2017  
Date 1/4/2015

SITE DIVERSION ANALYSIS  
COPPER FLAT PROJECT

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6 RESULTS.....	6
7 CONCLUSIONS.....	8
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*Harry J. Lewsley*  
*12/7/2015*



HARRY J. LEWSLEY  
NEW MEXICO  
18979  
LICENSED PROFESSIONAL ENGINEER  
Exp. 12/31/2017

LIST OF FIGURES

<u>LIST OF DRAWINGS</u>	<u>DRAWING DESCRIPTION</u>	<u>Drawings Follow Text</u>
Drawing 0000-CI-100	Greenhorn Arroyo Pre-Mining Watershed Basin	
Drawing 0000-CI-101	Existing Hydrology Pre-Quintana Mining	
Drawing 0000-CI-102	Existing Hydrology Post-Quintana Mining	
Drawing 0000-CI-103	Grayback Arroyo Diversion Through NMCC Project Site	
Drawing 0000-CI-104	Pre-Quintana Existing Watershed Areas	
Drawing 0000-CI-105	Post-Quintana Existing Watershed Areas	

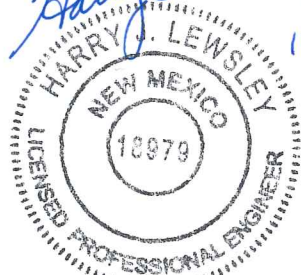
*Harry Lewisley*  
*12/7/2015*

Exp. 12/31/2017

LIST OF TABLES

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*Harry Lewsley*  
*12/7/2015*



Exp. 12/31/2017

LIST OF APPENDICES

APPENDIX	DESCRIPTION
A	Existing Watershed Summary (Pre and Post-Quintana Watersheds)
B	HydroCAD Results for Pre-Quintana Watersheds
C	HydroCAD Results for Post-Quintana Watersheds
D	HydroCAD Results for Post-Quintana Culvert Analysis
E	HY-8 Culvert Analysis Results

*Harry J. Lewisley*  
*12/7/2015*



NEW MEXICO  
HARRY J. LEWSLEY  
18979  
LICENSED PROFESSIONAL ENGINEER  
Exp. 12/31/2017

**1 PURPOSE AND SCOPE OF ANALYSIS**

The Copper Flat Project proposed for mine development by THEMAC Resources, Inc. through its subsidiary, New Mexico Copper Company (NMCC), lies within the Greenhorn Arroyo sub-basin in Sierra County, New Mexico (Dwg. 0000-CI-100).

Previous development of the mining property was conducted by Quintana Minerals Company (Quintana). The natural drainage pattern in the area (Dwg. 0000-CI-101) was modified by Quintana to divert surface drainages away from the proposed mine pit and protect the operations from flooding (Dwg. 0000-CI-102). In addition to the diversions, Quintana placed large diameter culverts beneath two crossings of Grayback Arroyo near the mine entrance. These structures are still in place and will be used by NMCC to limit stormwater run-on and prevent the site impacted areas from interacting with uncontained surface water flows for the planned mining operation (Dwg. 0000-CI-103).

The analysis completed was to evaluate existing diversions and water conveyance features at the Copper Flat Project with regard to their adequacy in conveying flows from storm events and protecting the site from flooding. In order to evaluate the adequacy of the existing diversions it was necessary to identify and evaluate surface water flow prior to the existence of the diversions (Dwg. 000-CI-104). Analytical methods were used to calculate the peak discharges, runoff volumes, and to determine if the existing site diversions and Grayback Arroyo culverts had the adequate capacity to convey the peak discharges and runoff around and away from the proposed site. Peak flows were analyzed for each sub-basin contributing to surface water flows upstream of the project area using the the HydroCAD program which uses both Soil Conservation Service (SCS) TR-20 methods (SCS, 1982) and TR-55 methods (USDA, 1986). An SCS 24-hour Type II storm was selected for simulation due to the project's location. Upstream watersheds were delineated as depicted on Dwg. 0000-CI-104.

The result of this evaluation demonstrates that existing diversions and culverts are adequate to prevent run-on or flooding of the mine site.

## **2 TYPE OF ANALYSIS**

### **2.1 HYDROLOGY**

Peak discharge and volume analysis was performed for drainage areas contributing to the Grayback Arroyo located within the Copper Flat project area. The return periods used were:

- 100-year, 24-hour storm, (Q100)
- 200-year, 24-hour storm, (Q200)
- 500-year, 24-hour storm, (Q500)

### **2.2 HYDRAULICS**

Culvert and channel capacity analysis was conducted for the Grayback Arroyo to determine water surface elevations during the design storm for the two existing culvert crossings.



**3 KEY ASSUMPTIONS**

- Minimum of 10 percent impervious area assumed for all watersheds.
- Minimum time of concentration set to 5 minutes for developed watersheds and 10 minutes for natural watersheds.
- Soil Class D assumed because fine to medium sand and clay dominate beneath the top layer of soil, which will likely be excavated, in accordance with THEMAC Conceptual Model of Groundwater Flow.
- Surface description used for natural watersheds is “Desert Shrub Range” with a curve number (CN) of 86.
- Existing culverts were modeled assuming that they are in good condition and free of any debris.

**4 PROCEDURES AND METHODS USED:**

- Peak Discharges:
  - Calculated by the SCS methods using HydroCAD.
  - Calculated the existing discharges for the 100-, 200-, and 500-year, 24-hour storm events.
  - The peak discharges were found using an antecedent moisture condition (AMC) of II to represent normal conditions.
  - Time of concentration was calculated using the SCS Lag time method.
  - The length of longest watercourse,  $L_c$ , and mean watershed slope,  $Sc$ , were determined by using a combination of site surveys and USGS topographic maps.
  
- Culvert Capacity Calculations:
  - Calculated the existing culvert capacity for the 100-, 200-, and 500-year, 24-hour storm events.
  - Culvert capacity calculations completed using the Federal Highway Administration (FHWA) HY-8 program for Crossing #1. HY-8 was chosen for the first existing culvert crossing as the culverts will be subject to the peak discharge of the entire watershed reporting to them.
  - Culvert capacity calculations for Crossing #2 were computed utilizing HydroCAD due to the upstream ponding, which will mitigate the peak discharge and thus make the existing culvert at Crossing #2 act as a orifice outlet instead of a standard culvert.
  - Combined peak discharges at each crossing are less than the cumulative respective discharges due to the “routing” effect of Grayback Arroyo.

**5 TECHNICAL DATA**

- Precipitation depths are per the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 Precipitation Frequency Estimates (NOAA, 2006) near Hillsboro, NM.
- The watershed characteristic summary for the existing watersheds can be found in Appendix A.
- The files for the HydroCAD analysis for Pre-Quintana & Post-Quintana watersheds can be found in Appendices B and C, respectively.
- The HydroCAD analysis for the culvert crossings can be found in Appendix D.
- The HY-8 culvert analysis can be found in Appendix E.

**6 RESULTS**

Analysis of the pre-mine development topography and drainage pattern identified sixteen watersheds (Dwg. 0000-CI-105) contributing to Grayback Arroyo in the project area. These watersheds were analyzed using HydroCAD and the results are shown in Table 1 in cubic feet per second (cfs) for each watershed. The upstream drainages merged in the central portion of the current Copper Flat Project area and passed through to the eastern boundary via Grayback Arroyo.

**Table 1: Pre-Quintana Watershed Characteristics**

<b>Watershed ID</b>	<b>Area (ac)</b>	<b>Lc (ft)</b>	<b>Avg. Sc (ft/ft)</b>	<b>Tc (min)</b>	<b>Q100 (cfs)</b>	<b>Q200 (cfs)</b>	<b>Q500 (cfs)</b>
1	352.00	7,396	0.164	30.70	708.72	813.94	952.23
2	117.79	3,175	0.231	13.10	377.27	432.11	504.05
3	100.96	4,242	0.194	18.10	277.70	318.42	371.87
4	85.03	3,389	0.304	12.10	280.90	321.62	375.03
5	107.36	4,088	0.346	13.10	343.86	393.85	459.42
6	377.55	9,338	0.267	29.00	788.24	906.94	1060.97
7	144.47	5,064	0.277	17.40	405.48	464.86	542.79
8	92.01	3,617	0.273	13.40	291.83	334.29	389.99
9	235.91	7,005	0.267	23.00	567.91	651.70	761.76
10	333.62	10,278	0.262	31.50	660.28	758.38	887.32
11	397.83	7,149	0.182	28.30	843.39	968.42	1132.73
12	236.95	6,590	0.292	21.00	600.47	688.82	804.86
13	282.46	7,744	0.258	25.30	641.72	736.61	861.28
14	161.96	5,608	0.160	24.90	371.20	426.07	498.15
15	55.11	2,582	0.184	12.50	179.83	205.93	240.16
16	83.15	3,076	0.078	22.10	204.89	235.08	274.73

Lc = Length of longest waterpath  
Sc = Slope

Tc = Time of concentration  
Q = Flowrate

Diversion of surface drainages away from the prospective mining area was accomplished by Quintana using a number of diversions shown in Dwg. 0000-CI-105. Diversion 1 is an earthen diversion that re-routed a portion of Watershed 14 (Dwg. 0000 CI 105) to the east into an existing drainage that wraps around the north side of Animas Peak. Diversion 2 consists of earthen diversions routing Watersheds 12 and 13 southward to Diversion 3. Diversion 3 is a composite earthen diversion and bedrock cut that re-routes Watershed 10, 12, and 13 south into Watershed 6, where it joins the ancestral Grayback Arroyo channel south of the mine area (Dwg. 0000-CI-105). Diversion of surface drainages away from the mine area allows for surface water in the mine area to be managed in a manner that prevents impacted stormwater from migrating offsite or impacting groundwater.

Table 2 lists the watersheds that exist after the Quintana diversions, as altered by the NMCC development plan. The Quintana diversions and NMCC development plan alters the drainage pattern in the project area to facilitate control of impacted stormwater. As a result, Watersheds 15 and 16 (Dwg. 0000-CI-104) are completely within the project stormwater control area and are eliminated as tributaries to Grayback Arroyo and portions of Watersheds 1, 2, 3, and 14 are included in the project stormwater control area and isolated from interaction with Grayback Arroyo.

**Table 2: Final Watershed Characteristics**

Watershed ID	Area (ac)	Lc (ft)	Avg. Sc (ft/ft)	Tc (min)	Q100 (cfs)	Q200 (cfs)	Q500 (cfs)
1	28.21	N/A	0.249	5.00	117.20	133.97	155.94
2	106.56	3,068	0.314	11.00	363.66	415.35	484.31
3	34.99	1,603	0.177	8.70	129.54	148.20	172.66
4	75.02	3,047	0.339	10.50	259.92	297.56	346.92
5	124.19	5,976	0.340	18.00	342.49	392.70	458.61
6	331.22	8,173	0.310	24.20	774.13	888.50	1038.74
7	144.47	5,064	0.296	16.90	411.06	471.20	550.14
8	92.01	3,617	0.274	13.40	291.83	334.29	389.99
9	235.91	7,005	0.268	23.00	567.91	651.70	761.76
10	330.41	10,278	0.276	30.80	663.73	762.30	891.85
11	397.83	7,149	0.182	28.30	843.39	968.42	1132.73
12	227.42	6,590	0.315	20.20	588.30	674.80	789.72
13	275.51	7,744	0.293	23.80	649.87	745.85	871.93
14	79.97	3,545	0.224	14.60	243.90	279.49	326.19

Lc = Length of longest waterpath  
Sc = Slope

Tc = Time of concentration  
Q = Flowrate

In addition to the Diversions described above, Quintana installed culverts to convey water beneath two earthen crossings over the Grayback Arroyo channel southeast of the process plant area (Dwg. 0000-CI-103). The characteristics of these culverts were evaluated with respect to the calculated flows from the diverted and natural drainages upstream from the proposed mine area. Those characteristics and flows are presented in Tables 3 and 4. The modeled flows through the composite Grayback Arroyo drainage upstream of the culverts were evaluated to ensure that the culverts were adequate to transmit the necessary flows without risk of damage to the structures and flooding of the proposed mine site.

**Table 3: Existing Culvert Characteristics and Flows**

Crossing No.	Culvert CP Location	Description	End Treatment	Q100 (cfs)	Q200 (cfs)	Q500 (cfs)
1	2	3 – 177" CMPs	Projecting	3,552.39	4,191.95	5,046.09
2	1	1 – 177" CMP	Projecting	3,066.00	3,418.33	3,856.73

**Table 4: Existing Culvert Elevations, Lengths, and Slopes**

Crossing No.	Inlet Elev. (ft)	Outlet Elev. (ft)	Length (ft)	Slope (ft/ft)	Q100 Headwater Elev. (ft)	Q200 Headwater Elev. (ft)	Q500 Headwater Elev. (ft)	Top of Roadway Elev. (ft)
1	5372.0	5370.0	194.40	0.0103	5391.64	5397.05	5406.01	5419.0
2	5352.0	5344.0	255.4	0.0313	5386.54	5391.04	5396.71	5403.0

## **7 CONCLUSIONS**

The 24-hour storm flows for 100-year, 200-year, and 500-year return periods were evaluated for the watersheds as they exist at present. Analysis of the results of the hydrologic analyses presented in this report demonstrates that the existing diversions and control structures are adequate to protect the site from flooding during the modeled flow events.

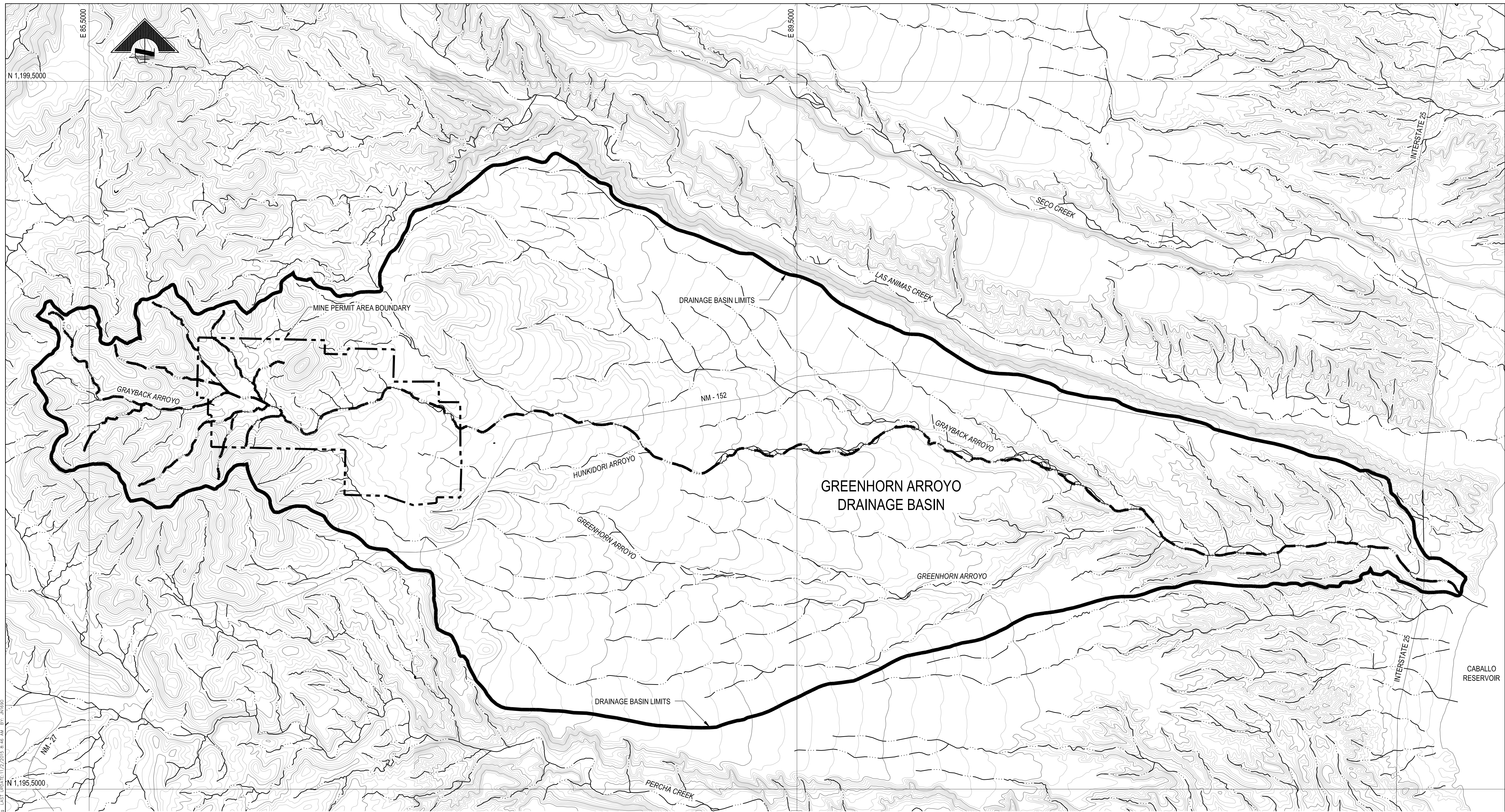
The analyses conducted also determined that both culvert crossings are capable of conveying the Q500 without overtopping the adjacent roadway or pipeline corridor. This conclusion is based on the assumption that the culverts analyzed are in good working condition. Field inspection of the culverts completed by M3 found the body of the existing culverts to be in good condition; repair will be required at the upstream inlets and vegetation removal undertaken at culvert inlets and outlets in order to meet the conditions of this analysis.

**8 REFERENCES**

- Bonin, GM, Martin, D., Lin, B., Parzybok, T., Yekta, M., and Riley, D., 2000. Precipitation Frequency Atlas of the United States, NOAA Atlas 14 Addendum, Volume 1, Version 4.0: Semiarid Southwest (Arizona, Southeast California, Nevada, New Mexico, Utah) Addendum – Update to Version 3.0. U.S. Department of Commerce, National Oceanic and Atmospheric Administration, National Weather Service, Silver Spring, Maryland, 2004 revised 2006.
- USDA, 1986. Urban Hydrology for Small Watersheds. U.S. Department of Agriculture, National Resources Conservation Service, Conservation Engineering Division. Technical Release (TR) 55. June, SCS, 1986.
- SCS, 1982. [Draft] Computer Program Co. Project Formulation – Hydrology. Soil Conservation Service Technical Release 20. Washington, DC.

## DRAWINGS





**LEGEND**

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

**SITE PLAN**

SCALE: 1" = 2500'



SCALE IN FEET  
CONTOUR INTERVAL = 50'  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



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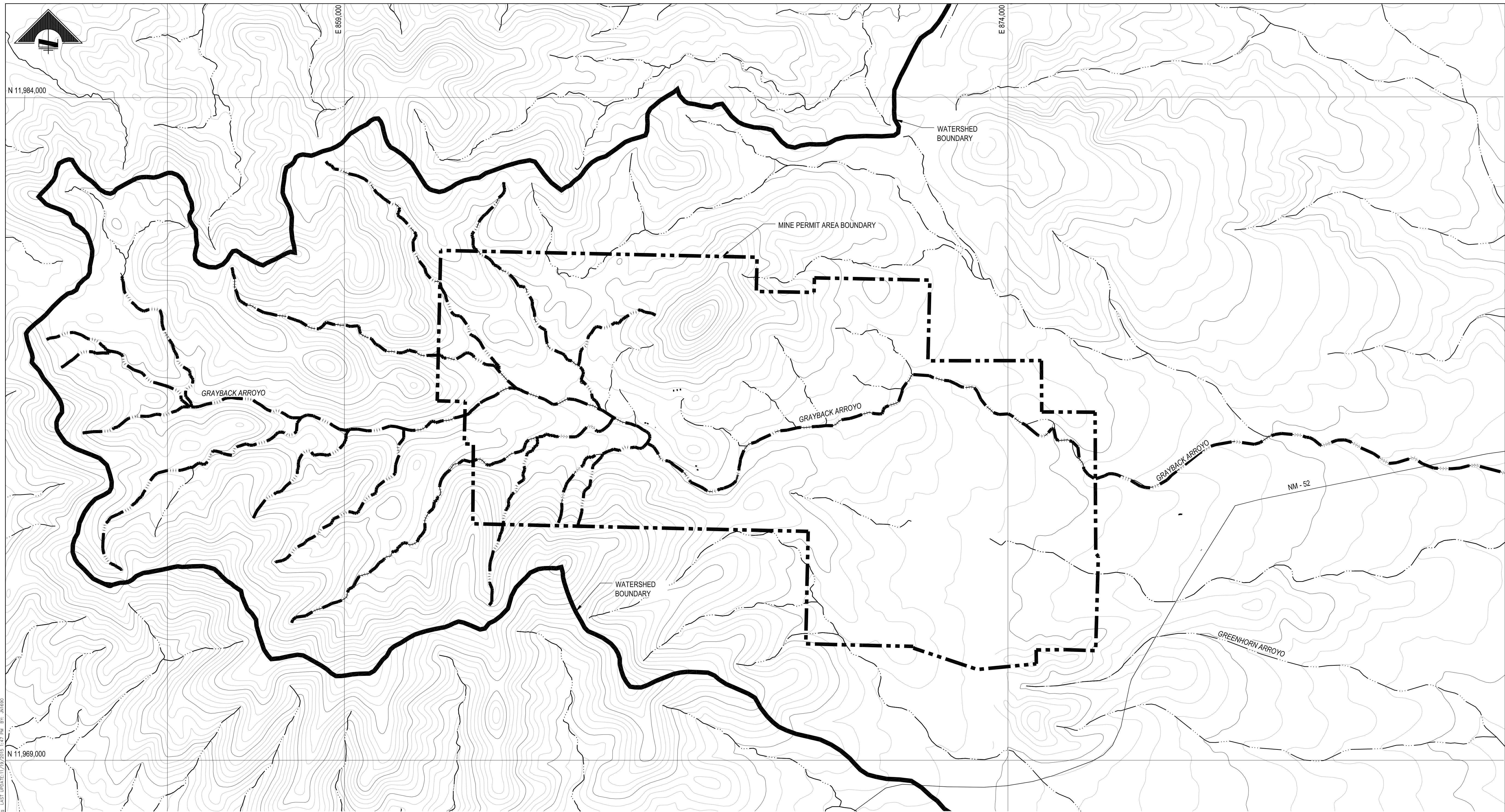
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**COPPER FLAT PROJECT**

**GENERAL SITE  
CIVIL  
GREENHORN ARROYO  
PRE-MINING WATERSHED BASIN**

JOB NO. M3 PN-12085  
DWG. NO. **0000-CI-100**  
REV. NO. P2 DATE 20 NOV 15



**LEGEND**

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

**SITE PLAN**

SCALE: 1" = 1000'



SCALE IN FEET  
CONTOUR INTERVAL = 20 FT  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



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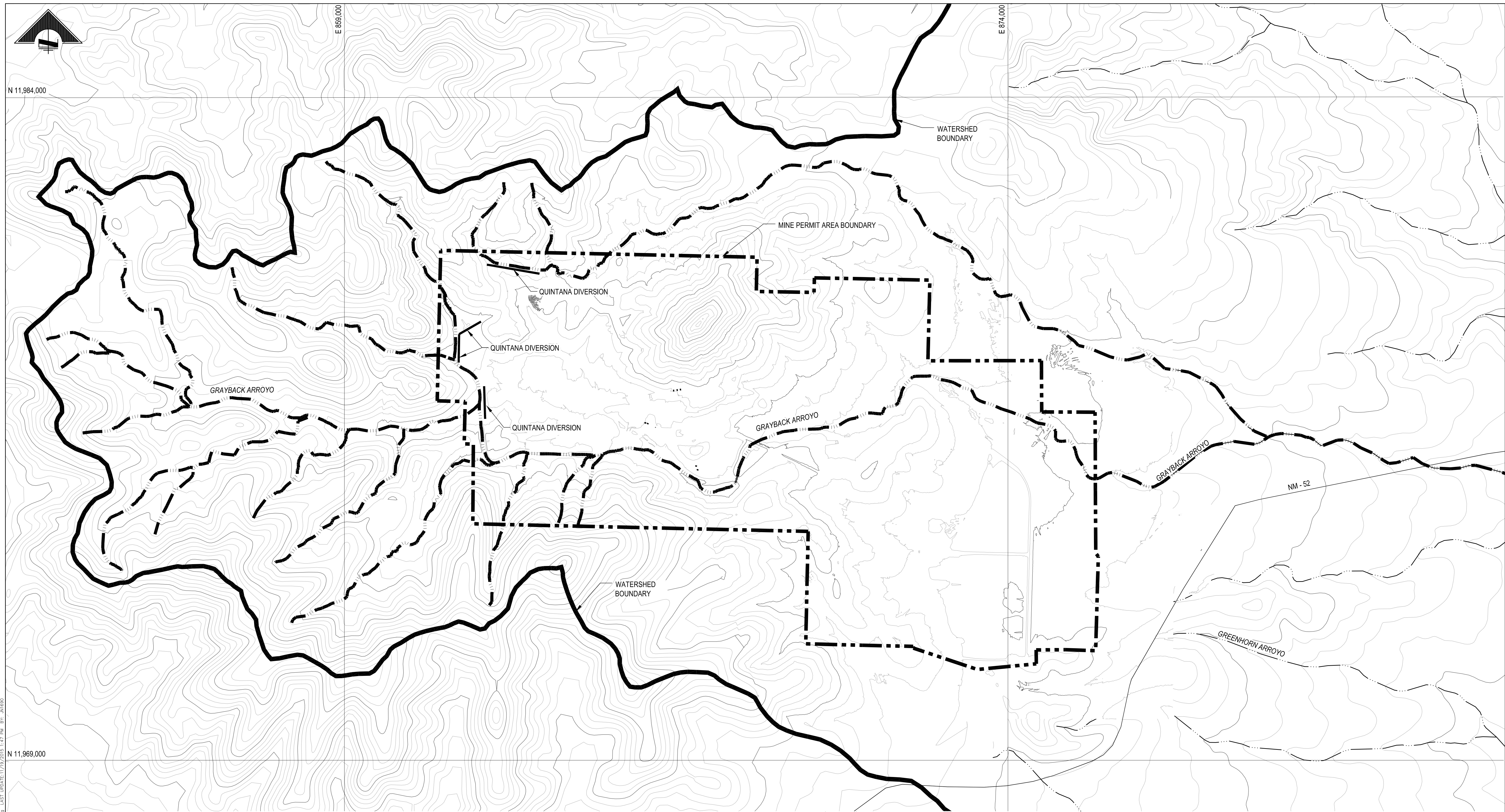
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**COPPER FLAT PROJECT**

**GENERAL SITE  
COPPER FLAT SITE  
EXISTING HYDROLOGY  
PRE-QUINTANA MINING**

JOB NO. M3 PN-120065  
DWG. NO. **0000-CI-101**  
REV. NO. P1 DATE 20 NOV 15

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**LEGEND**

- DRAINAGE BASIN BOUNDARY
- MINE PROPERTY BOUNDARY
- GRAYBACK ARROYO
- FLOWLINE

**SITE PLAN**

SCALE: 1" = 1000'



SCALE IN FEET  
CONTOUR INTERVAL = 20 FT  
DO NOT SCALE 11x17 DRAWINGS

**PRELIMINARY**  
FOR AGENCY REVIEW



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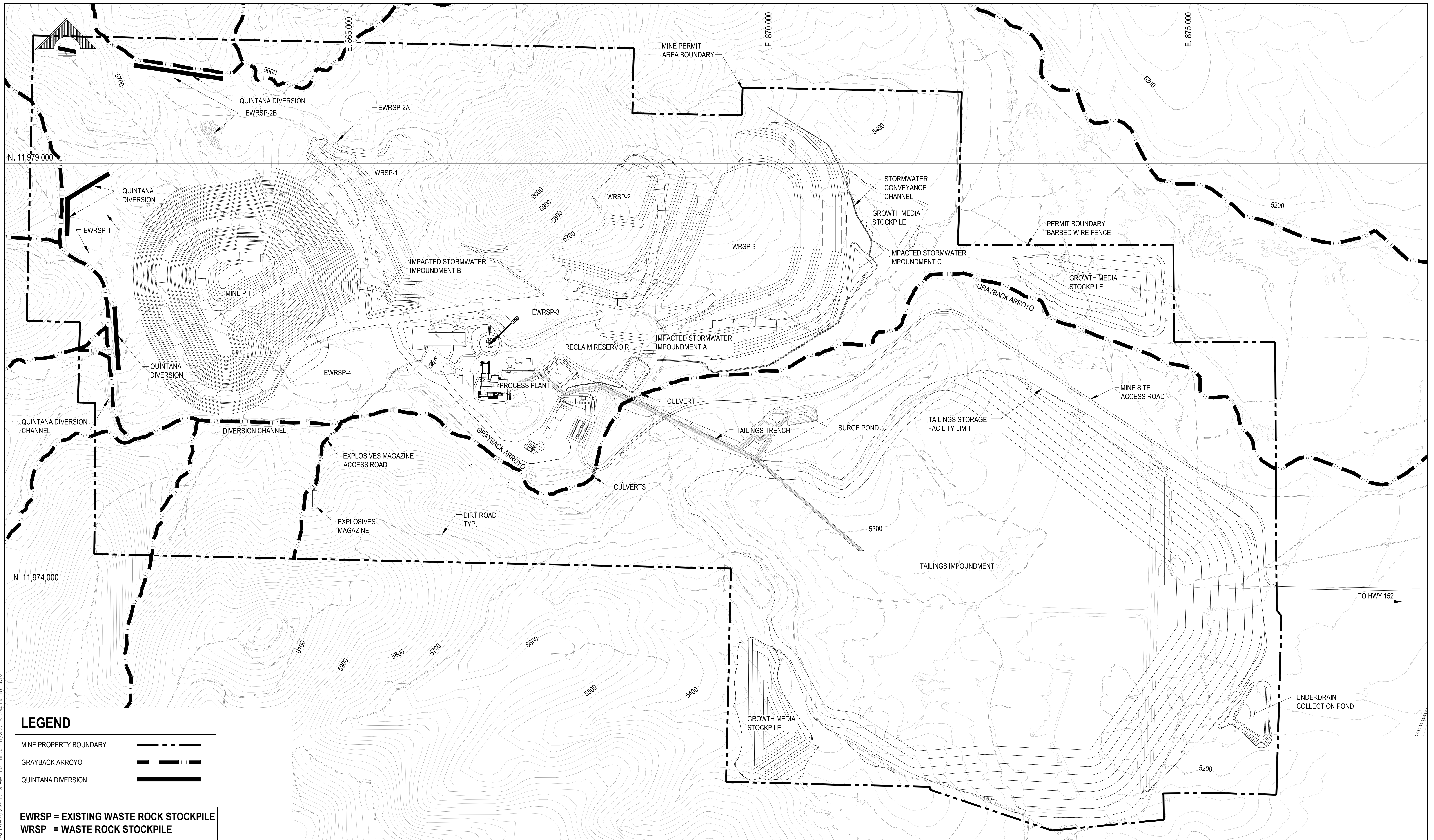
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**COPPER FLAT PROJECT**

**GENERAL SITE  
COPPER FLAT SITE  
EXISTING HYDROLOGY  
POST QUINTANA MINING**

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DWG. NO. **0000-CI-102**  
REV. NO. P1 DATE 20 NOV 15

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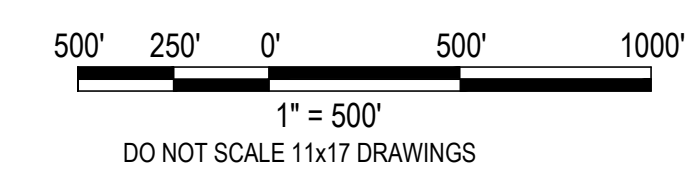
MINE PROPERTY BOUNDARY

GRAYBACK ARROYO

QUINTANA DIVERSION

**EWRSP = EXISTING WASTE ROCK STOCKPILE**  
**WRSP = WASTE ROCK STOCKPILE**

**SITE PLAN**  
SCALE: 1:500



**PRELIMINARY**  
FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
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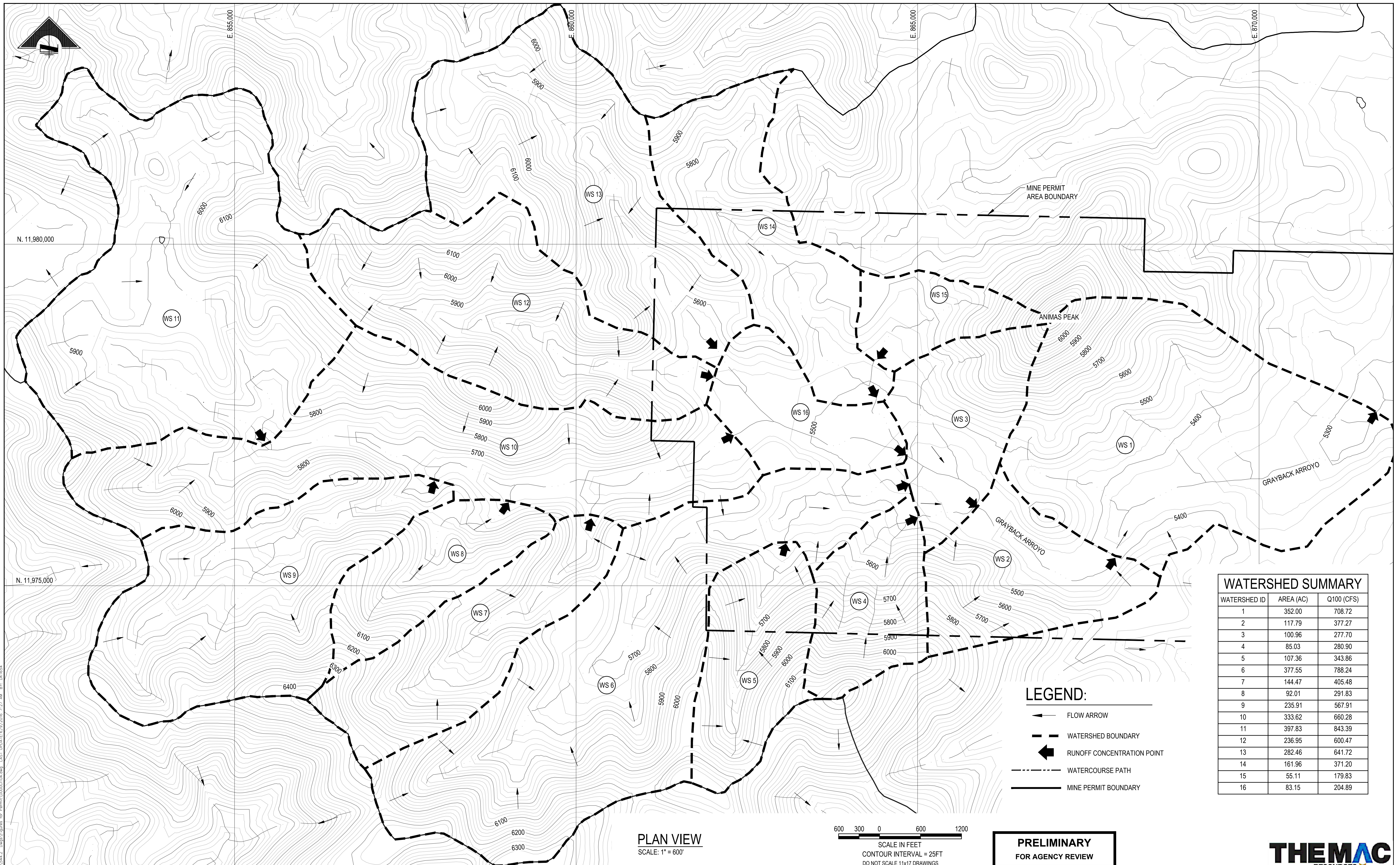
**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL**  
**GRAYBACK ARROYO DIVERSION THROUGH NMCC PROJECT SITE**

JOB NO. M3 PN-12085  
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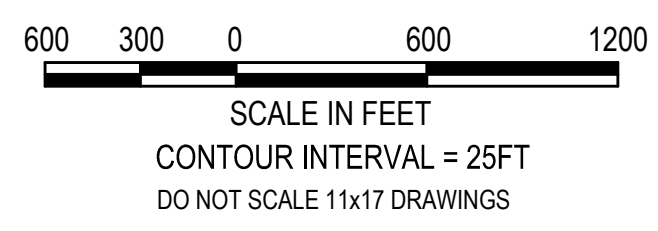
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**WATERSHED SUMMARY**

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1	352.00	708.72
2	117.79	377.27
3	100.96	277.70
4	85.03	280.90
5	107.36	343.86
6	377.55	788.24
7	144.47	405.48
8	92.01	291.83
9	235.91	567.91
10	333.62	660.28
11	397.83	843.39
12	236.95	600.47
13	282.46	641.72
14	161.96	371.20
15	55.11	179.83
16	83.15	204.89

**PLAN VIEW**  
SCALE: 1" = 600'



**PRELIMINARY**  
FOR AGENCY REVIEW



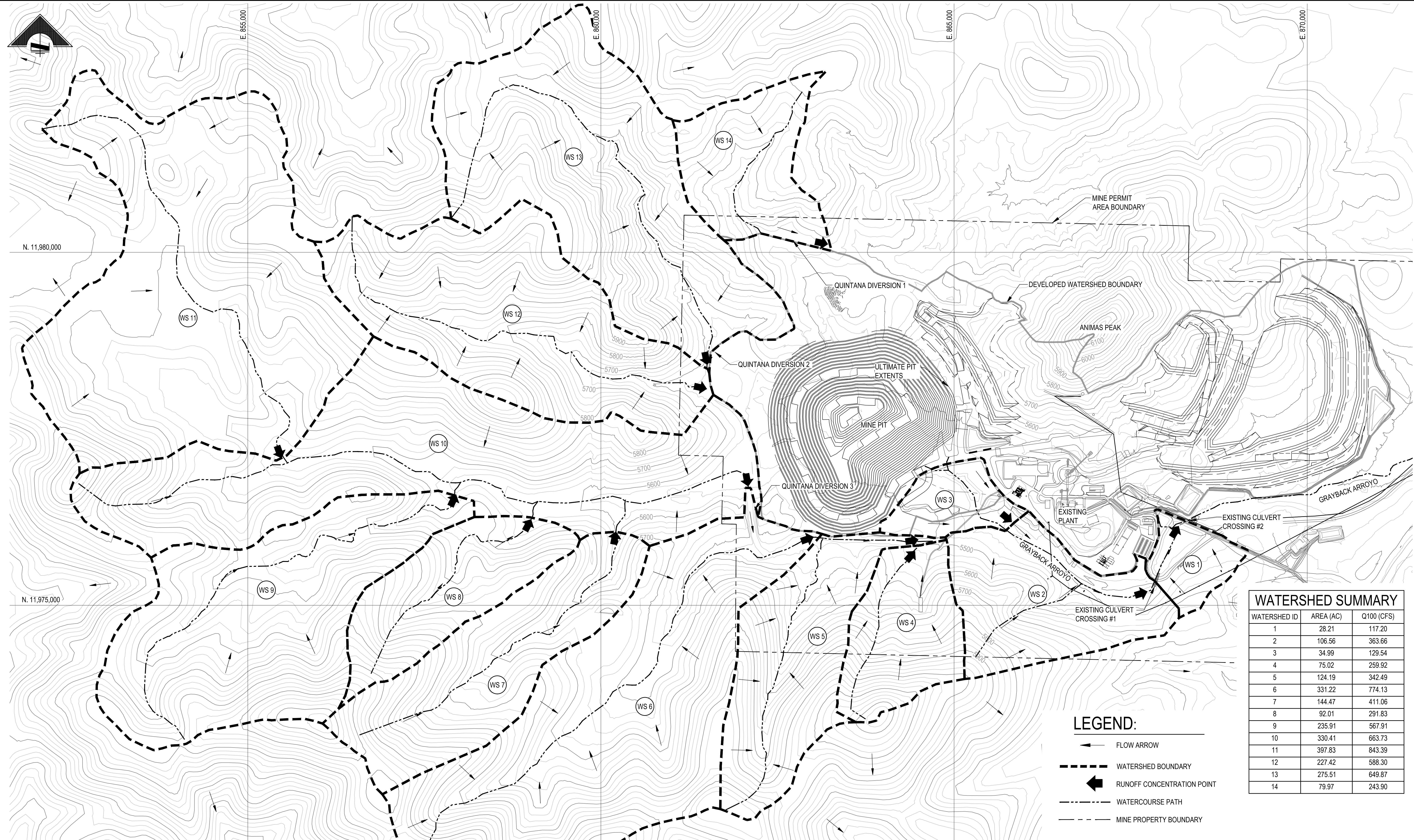
**COPPER FLAT PROJECT**  
GENERAL SITE  
CIVIL  
PRE QUINTANA  
EXISTING WATERSHED AREAS

JOB NO. M3 PN-12085  
DWG NO. **0000-CI-104**  
REV NO. P2 DATE 09 JUN 16

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N. 11,980,000

N. 11,975,000

E. 865,000

E. 860,000

E. 865,000

E. 870,000

**PLAN VIEW**  
SCALE: 1" = 600'



SCALE IN FEET  
CONTOUR INTERVAL = 25FT  
DO NOT SCALE 11x17 DRAWINGS

- LEGEND:**
- ← FLOW ARROW
  - WATERSHED BOUNDARY
  - ◀ RUNOFF CONCENTRATION POINT
  - WATERCOURSE PATH
  - MINE PROPERTY BOUNDARY

**WATERSHED SUMMARY**

WATERSHED ID	AREA (AC)	Q100 (CFS)
1	28.21	117.20
2	106.56	363.66
3	34.99	129.54
4	75.02	259.92
5	124.19	342.49
6	331.22	774.13
7	144.47	411.06
8	92.01	291.83
9	235.91	567.91
10	330.41	663.73
11	397.83	843.39
12	227.42	588.30
13	275.51	649.87
14	79.97	243.90

**PRELIMINARY**  
**FOR AGENCY REVIEW**



**COPPER FLAT PROJECT**

**GENERAL SITE CIVIL**  
**POST QUINTANA**  
**EXISTING WATERSHED AREAS**

JOB NO. M3 PN-12085  
DWG NO. **0000-CI-105**  
REV NO. P2 DATE 09 JUN 16

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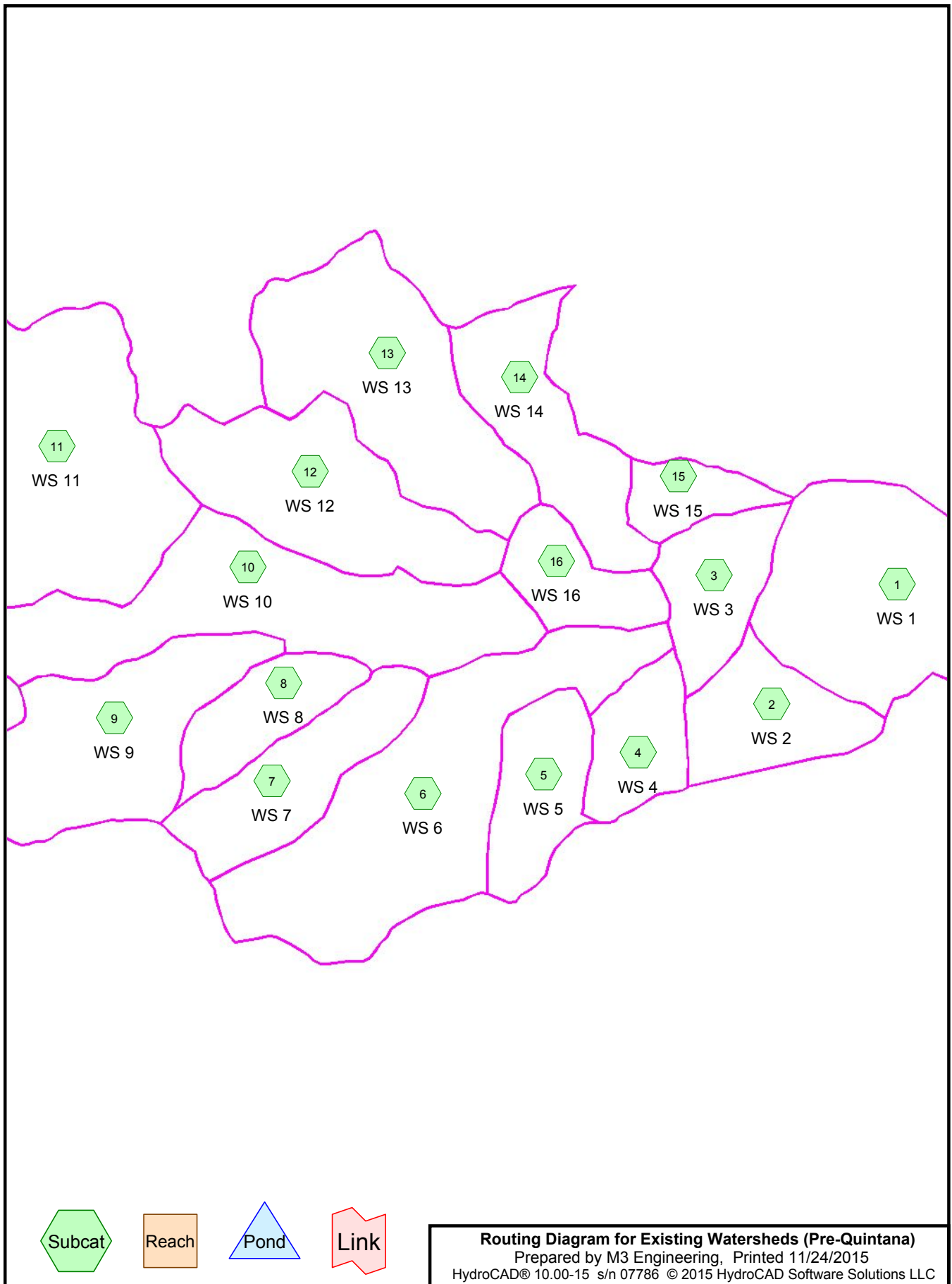
## APPENDIX A

Pre-Quintana Existing												
Watershed ID	Area	Total Area	Desert Shrub CN=86	Impervious Area CN=98	Pit Material CN=80	L <sub>c</sub>	Avg. S <sub>c</sub>	Q <sub>100</sub> - SCS	Tc Method	Tc	Weighted SCS CN	Soil Type
--	<i>sq. feet</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>ft</i>	<i>ft/ft</i>	<i>cfs</i>	--	<i>min</i>	--	--
1	15,333,000	352.00	316.80	35.20	0.00	7,396	0.164	708.72	SCS Lag	30.70	87	D
2	5,131,142	117.79	106.01	11.78	0.00	3,175	0.231	377.27	SCS Lag	13.10	87	D
3	4,397,931	100.96	90.87	10.10	0.00	4,242	0.194	277.70	SCS Lag	18.10	87	D
4	3,703,864	85.03	76.53	8.50	0.00	3,389	0.304	280.9	SCS Lag	12.10	87	D
5	4,676,541	107.36	96.62	10.74	0.00	4,088	0.346	343.86	SCS Lag	13.10	87	D
6	16,446,115	377.55	339.79	37.75	0.00	9,338	0.267	788.24	SCS Lag	29.00	87	D
7	6,292,945	144.47	130.02	14.45	0.00	5,064	0.277	405.48	SCS Lag	17.40	87	D
8	4,008,126	92.01	82.81	9.20	0.00	3,617	0.273	291.83	SCS Lag	13.40	87	D
9	10,276,128	235.91	212.32	23.59	0.00	7,005	0.267	567.91	SCS Lag	23.00	87	D
10	14,532,660	333.62	300.26	33.36	0.00	10,278	0.262	660.28	SCS Lag	31.50	87	D
11	17,329,719	397.83	358.05	39.78	0.00	7,149	0.182	843.39	SCS Lag	28.30	87	D
12	10,321,490	236.95	213.25	23.69	0.00	6,590	0.292	600.47	SCS Lag	21.00	87	D
13	12,303,894	282.46	254.21	28.25	0.00	7,744	0.258	641.72	SCS Lag	25.30	87	D
14	7,054,905	161.96	145.76	16.20	0.00	5,608	0.160	371.2	SCS Lag	24.90	87	D
15	2,400,616	55.11	49.60	5.51	0.00	2,582	0.184	179.83	SCS Lag	12.50	87	D
16	3,621,863	83.15	74.83	8.31	0.00	3,076	0.078	204.89	SCS Lag	22.10	87	D

Post-Quintana Existing												
Watershed ID	Area	Total Area	Desert Shrub CN=86	Impervious Area CN=98	Pit Material CN=80	L <sub>c</sub>	Avg. S <sub>c</sub>	Q <sub>100</sub> - SCS	Tc Method	Tc	Weighted SCS CN	Soil Type
--	<i>sq. feet</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>acres</i>	<i>ft</i>	<i>ft/ft</i>	<i>cfs</i>	--	<i>min</i>	--	--
1 (pond)	1,228,747	28.21	25.39	2.82	0.00	0	0.249	<b>117.20</b>	Minimum	5.00	87	D
2	4,641,978	106.56	95.91	10.66	0.00	3,068	0.314	<b>363.66</b>	SCS Lag	11.00	87	D
3	1,523,968	34.99	31.49	3.50	0.00	1,603	0.177	<b>129.54</b>	SCS Lag	8.70	87	D
4	3,267,841	75.02	67.52	7.50	0.00	3,047	0.339	<b>259.92</b>	SCS Lag	10.50	87	D
5	5,409,568	124.19	111.77	12.42	0.00	5,976	0.340	<b>342.49</b>	SCS Lag	18.00	87	D
6	14,427,914	331.22	298.10	33.12	0.00	8,173	0.310	<b>774.13</b>	SCS Lag	24.20	87	D
7	6,292,945	144.47	130.02	14.45	0.00	5,064	0.296	<b>411.06</b>	SCS Lag	16.90	87	D
8	4,008,126	92.01	82.81	9.20	0.00	3,617	0.274	<b>291.83</b>	SCS Lag	13.40	87	D
9	10,276,128	235.91	212.32	23.59	0.00	7,005	0.268	<b>567.91</b>	SCS Lag	23.00	87	D
10	14,392,904	330.41	297.37	33.04	0.00	10,278	0.276	<b>663.73</b>	SCS Lag	30.80	87	D
11	17,329,719	397.83	358.05	39.78	0.00	7,149	0.182	<b>843.39</b>	SCS Lag	28.30	87	D
12	9,906,558	227.42	204.68	22.74	0.00	6,590	0.315	<b>588.30</b>	SCS Lag	20.20	87	D
13	12,001,134	275.51	247.96	27.55	0.00	7,744	0.293	<b>649.87</b>	SCS Lag	23.80	87	D
14	3,483,496	79.97	71.97	8.00	0.00	3,545	0.224	<b>243.90</b>	SCS Lag	14.60	87	D



## APPENDIX B



**Existing Watersheds (Pre-Quintana)**

Prepared by M3 Engineering

Printed 11/24/2015

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Page 2

**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
2,847.730	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16)
316.410	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16)
<b>3,164.140</b>	<b>87</b>	<b>TOTAL AREA</b>

**Existing Watersheds (Pre-Quintana)**

Prepared by M3 Engineering

Printed 11/24/2015

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Page 3

**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
3,164.140	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
0.000	Other	
<b>3,164.140</b>		<b>TOTAL AREA</b>

**Existing Watersheds (Pre-Quintana)**

Prepared by M3 Engineering

Printed 11/24/2015

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Page 4

**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,847.730	0.000	2,847.730	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
0.000	0.000	0.000	316.410	0.000	316.410	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16
<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>3,164.140</b>	<b>0.000</b>	<b>3,164.140</b>	<b>TOTAL AREA</b>	

**Existing Watersheds (Pre-Quintana)***Type II 24-hr 100-yr Event Rainfall=3.70"*

Prepared by M3 Engineering

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Page 5

Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=708.72 cfs	69.315 af	
<b>Subcatchment2: WS 2</b>	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=377.27 cfs	23.195 af	
<b>Subcatchment3: WS 3</b>	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=277.70 cfs	19.883 af	
<b>Subcatchment4: WS 4</b>	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=280.90 cfs	16.744 af	
<b>Subcatchment5: WS 5</b>	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=343.86 cfs	21.141 af	
<b>Subcatchment6: WS 6</b>	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=788.24 cfs	74.344 af	
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=405.48 cfs	28.449 af	
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=291.83 cfs	18.118 af	
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=567.91 cfs	46.455 af	
<b>Subcatchment10: WS 10</b>	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=660.28 cfs	65.695 af	
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=843.39 cfs	78.339 af	
<b>Subcatchment12: WS 12</b>	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=600.47 cfs	46.657 af	
<b>Subcatchment13: WS 13</b>	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=641.72 cfs	55.621 af	
<b>Subcatchment14: WS 14</b>	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=371.20 cfs	31.893 af	
<b>Subcatchment15: WS 15</b>	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=179.83 cfs	10.852 af	
<b>Subcatchment16: WS 16</b>	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=2.36"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=204.89 cfs	16.372 af	

**Existing Watersheds (Pre-Quintana)**

*Type II 24-hr 100-yr Event Rainfall=3.70"*

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**Total Runoff Area = 3,164.140 ac   Runoff Volume = 623.071 af   Average Runoff Depth = 2.36"**  
**90.00% Pervious = 2,847.730 ac   10.00% Impervious = 316.410 ac**

**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

Prepared by M3 Engineering

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**Summary for Subcatchment 1: WS 1**

Runoff = 708.72 cfs @ 12.25 hrs, Volume= 69.315 af, Depth= 2.36"

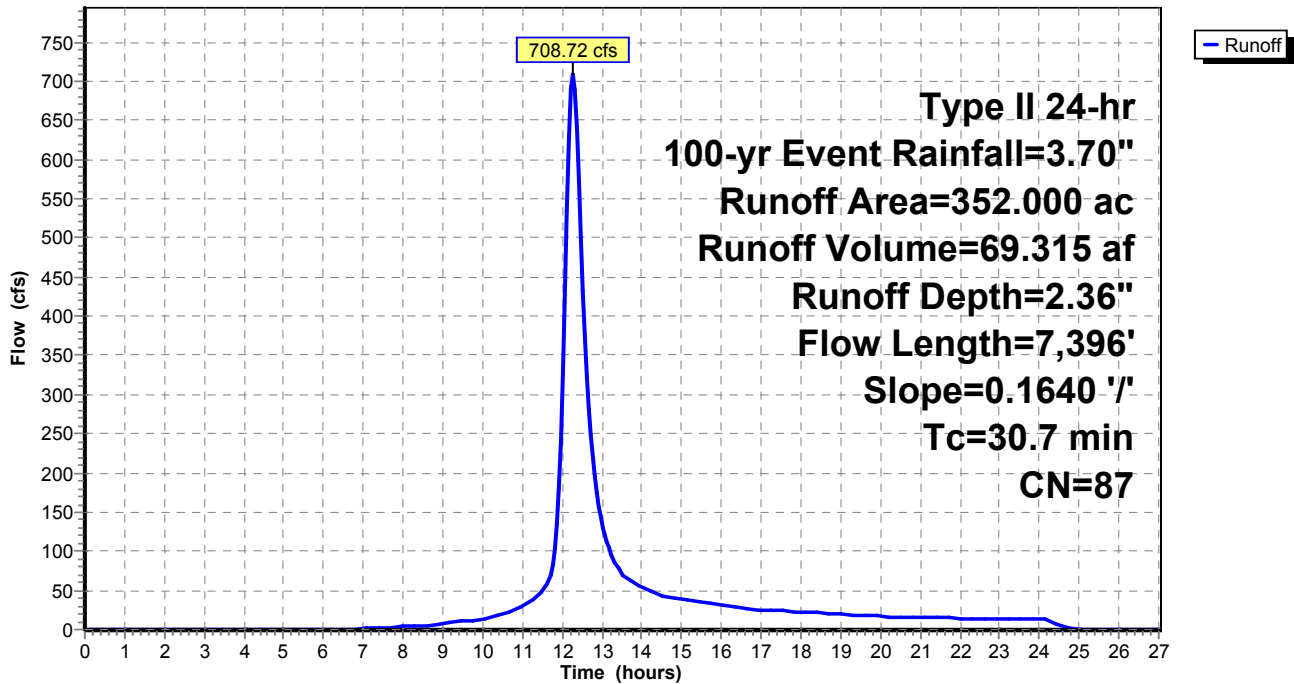
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

**Subcatchment 1: WS 1**

Hydrograph





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 2: WS 2**

Runoff = 377.27 cfs @ 12.05 hrs, Volume= 23.195 af, Depth= 2.36"

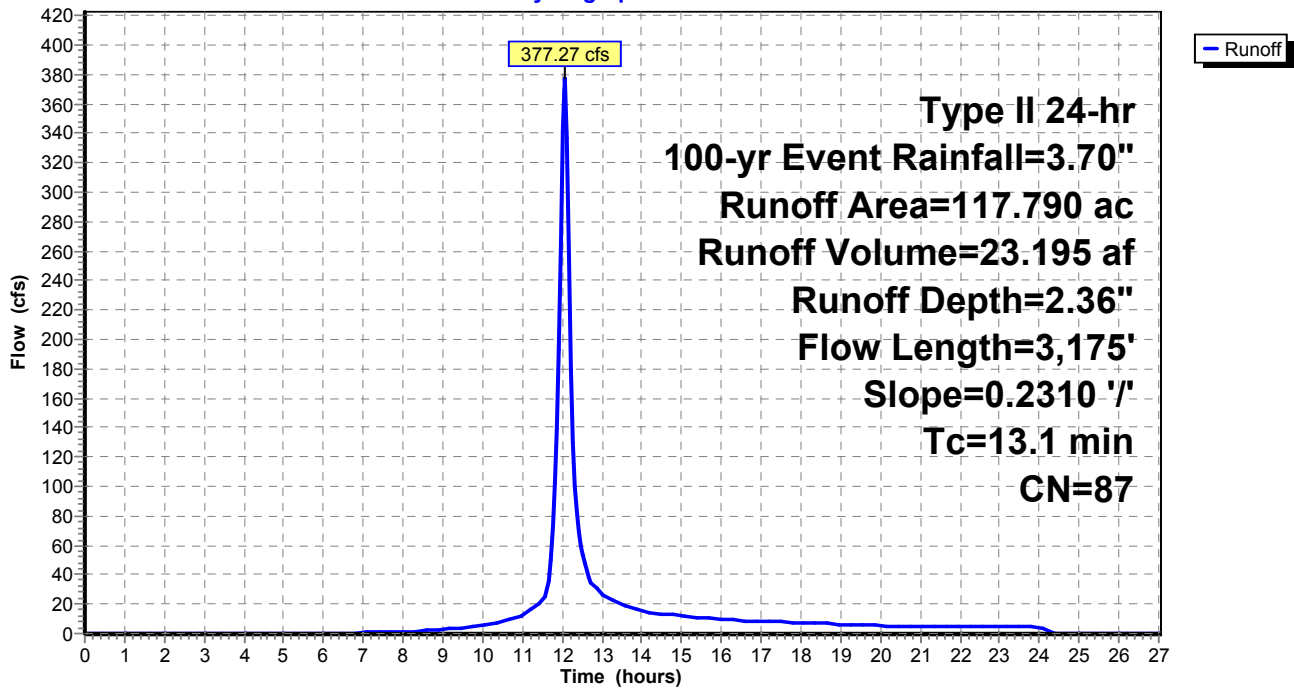
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 3: WS 3**

Runoff = 277.70 cfs @ 12.10 hrs, Volume= 19.883 af, Depth= 2.36"

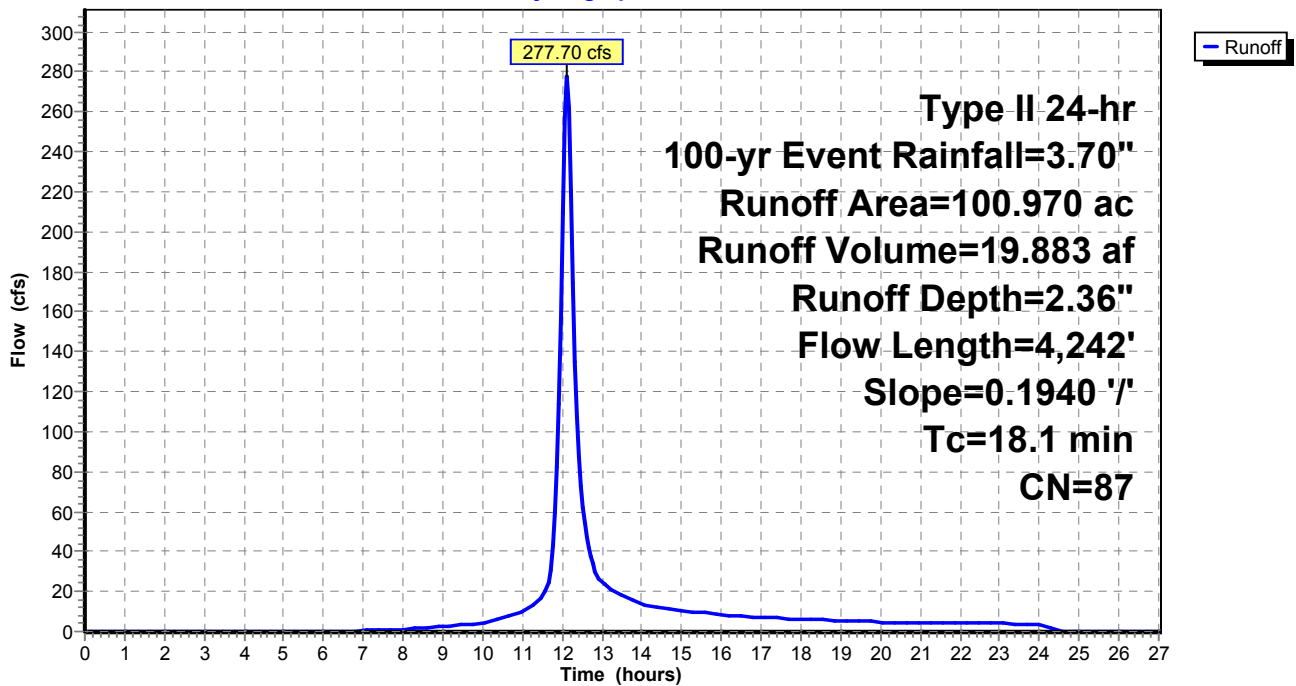
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 4: WS 4**

Runoff = 280.90 cfs @ 12.04 hrs, Volume= 16.744 af, Depth= 2.36"

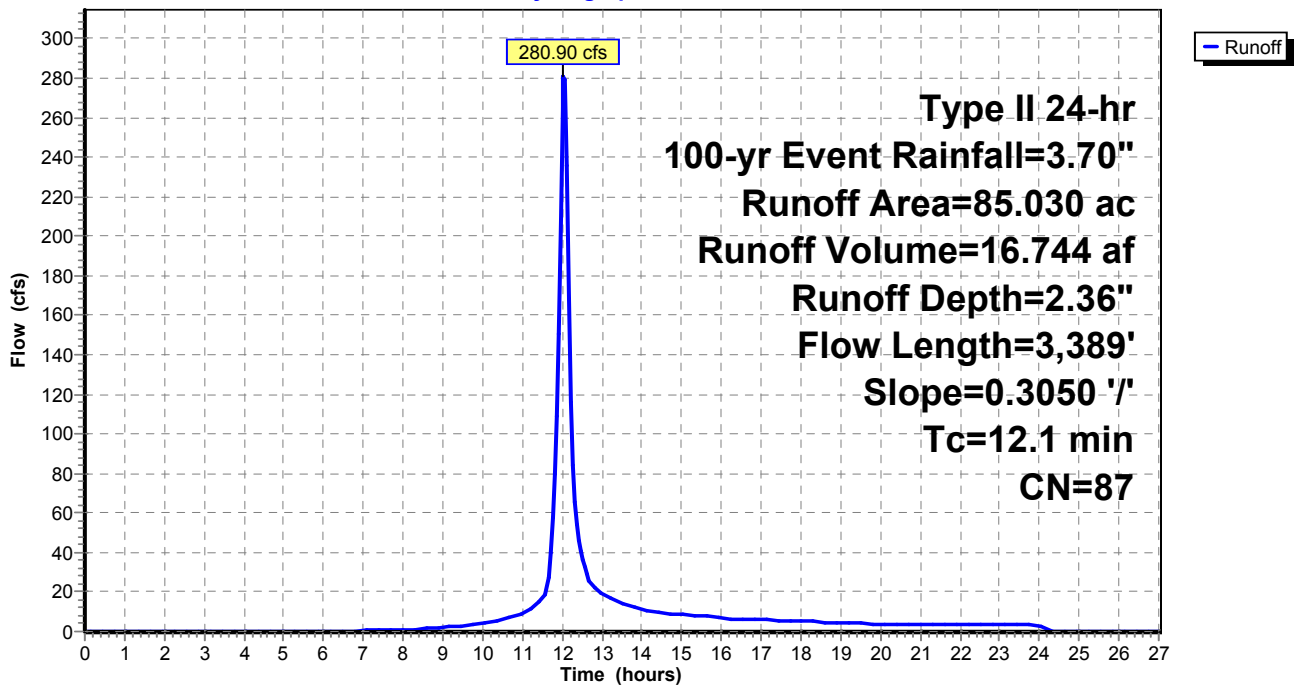
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 5: WS 5**

Runoff = 343.86 cfs @ 12.05 hrs, Volume= 21.141 af, Depth= 2.36"

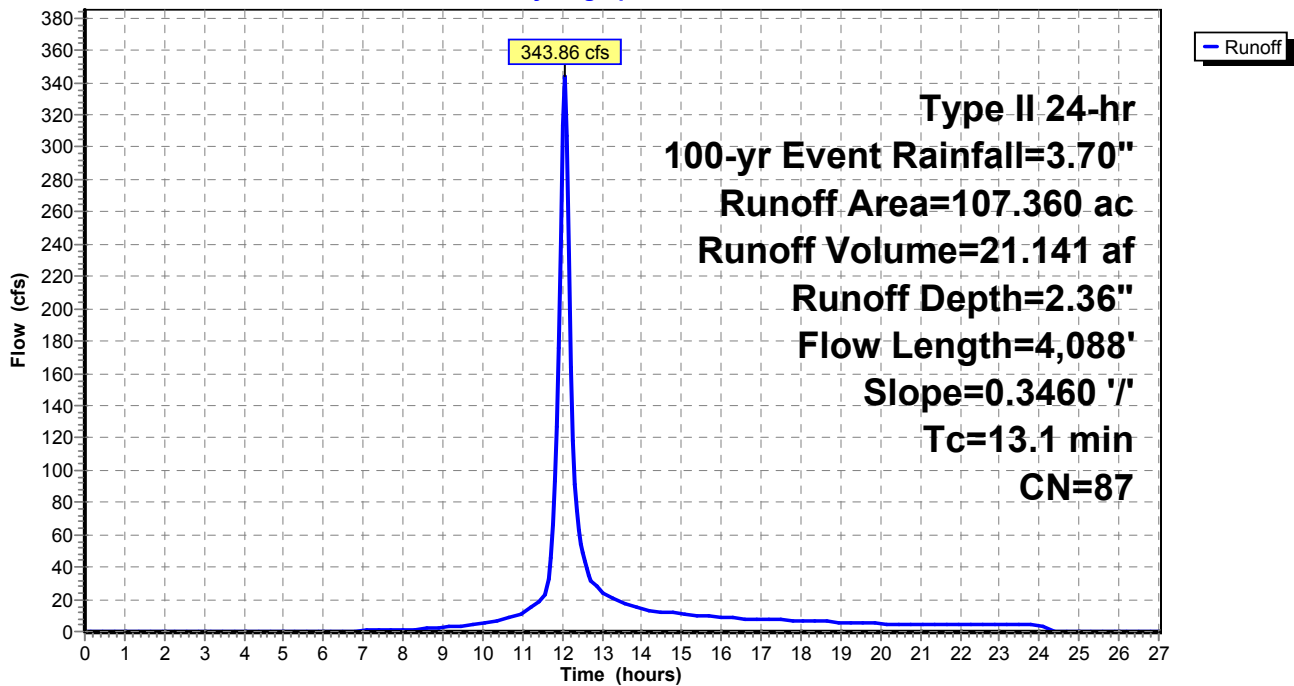
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 6: WS 6**

Runoff = 788.24 cfs @ 12.23 hrs, Volume= 74.344 af, Depth= 2.36"

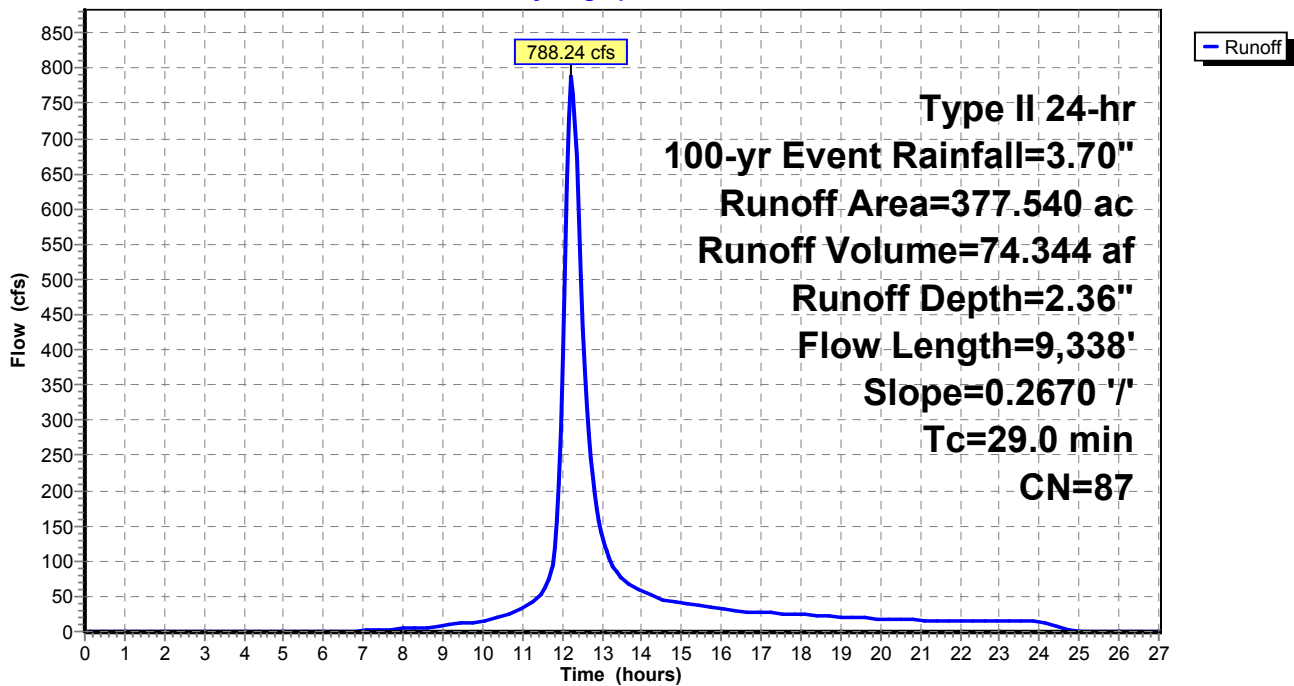
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 7: WS 7**

Runoff = 405.48 cfs @ 12.10 hrs, Volume= 28.449 af, Depth= 2.36"

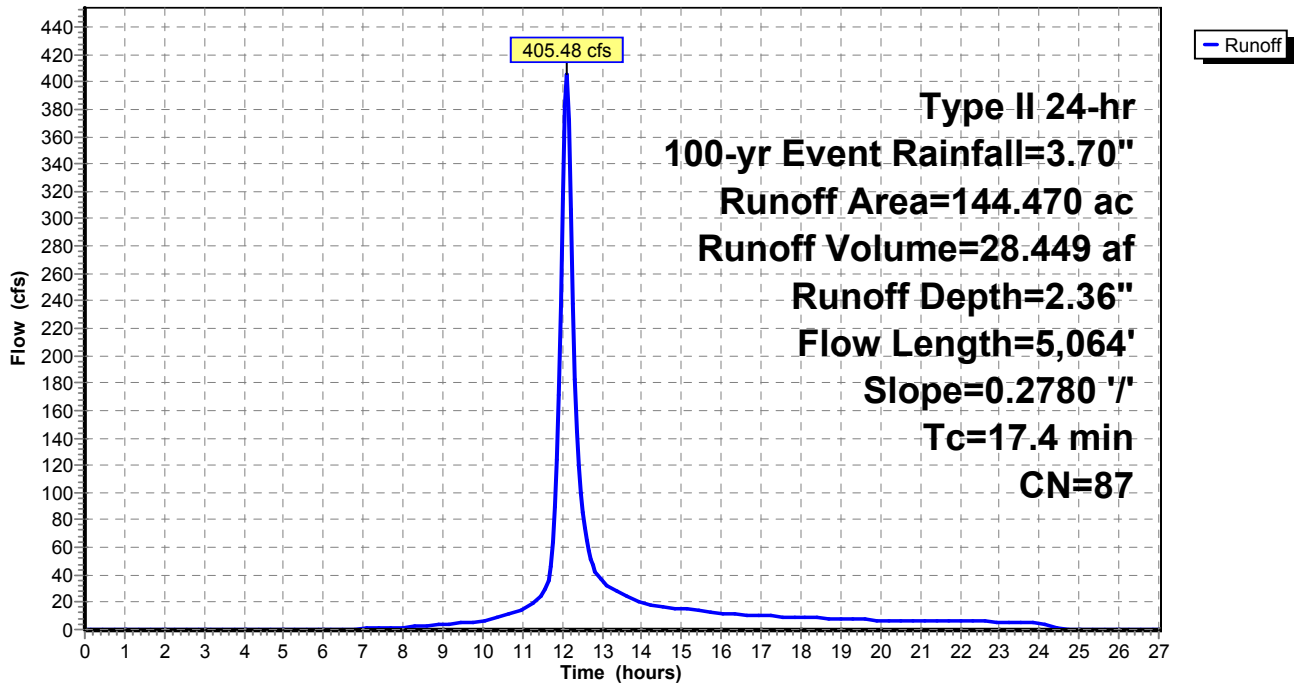
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 8: WS 8**

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

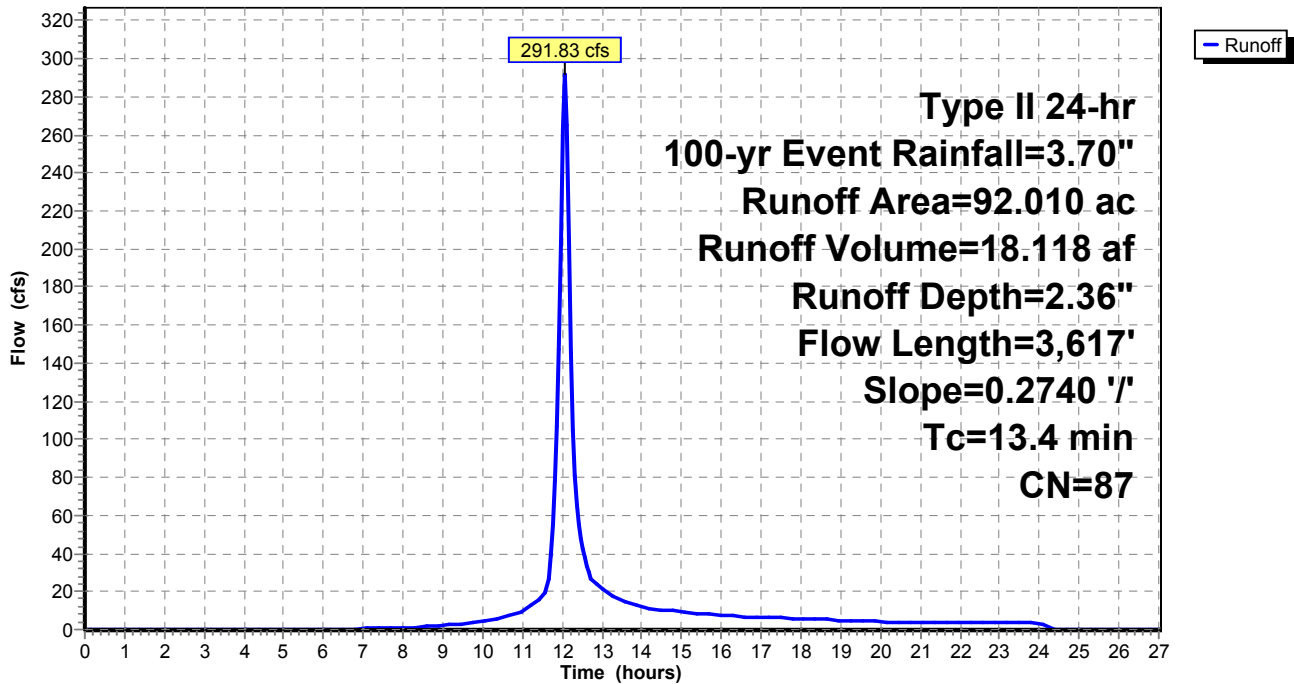
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 9: WS 9**

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

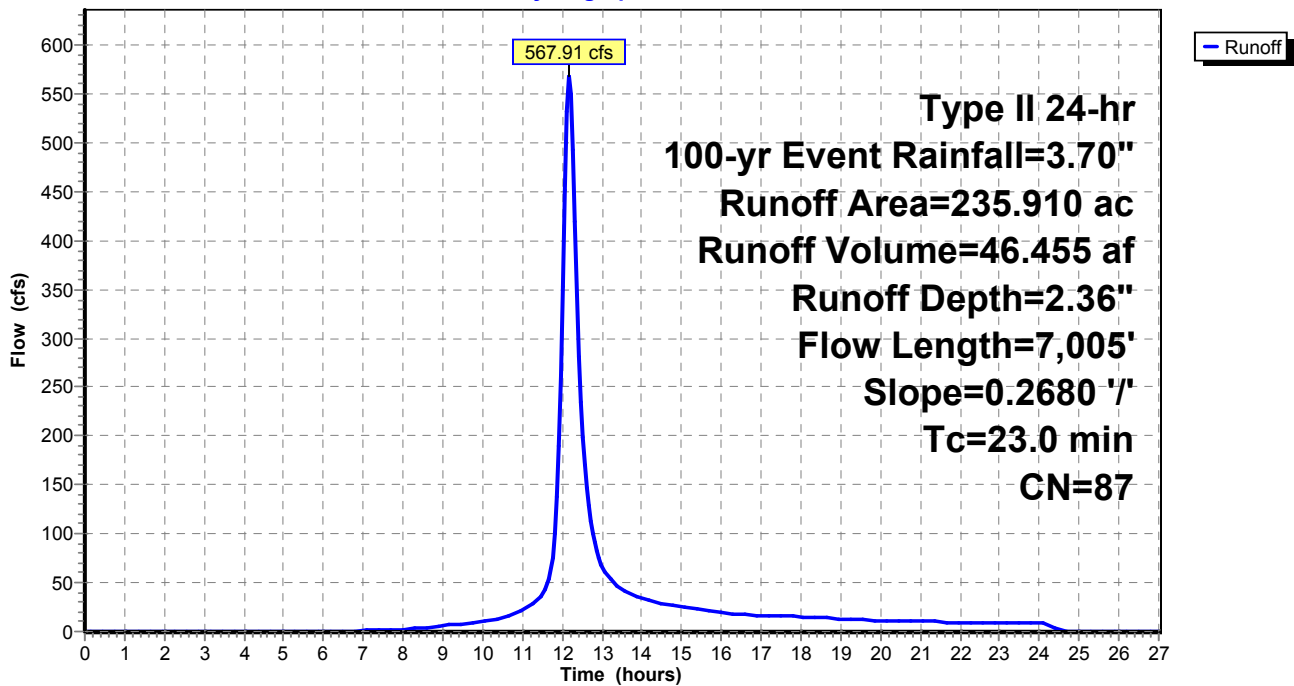
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 10: WS 10**

Runoff = 660.28 cfs @ 12.26 hrs, Volume= 65.695 af, Depth= 2.36"

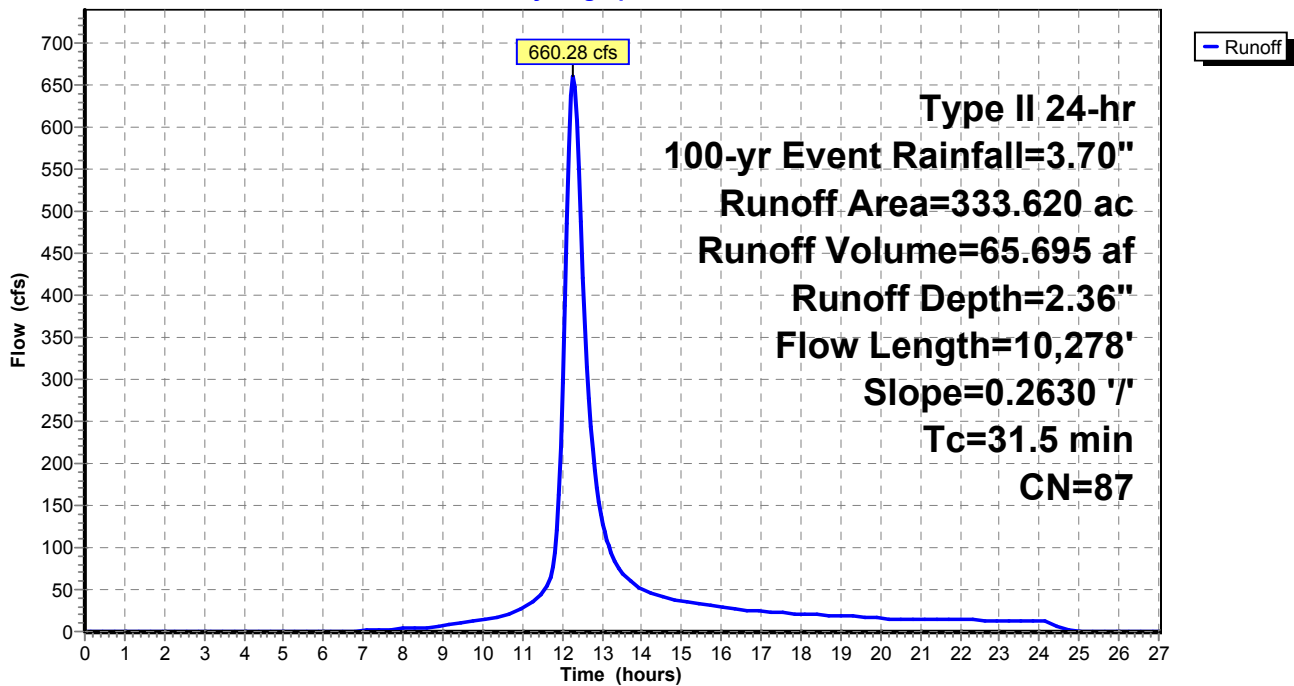
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 11: WS 11**

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

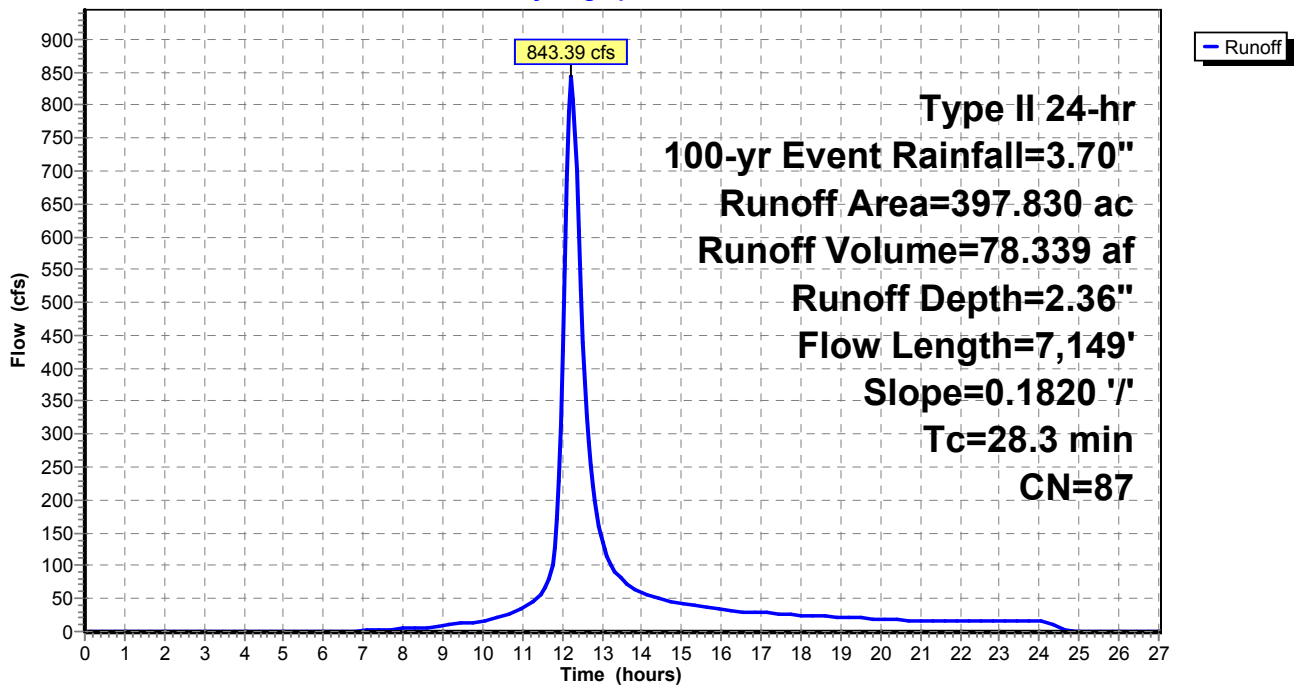
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 12: WS 12**

Runoff = 600.47 cfs @ 12.14 hrs, Volume= 46.657 af, Depth= 2.36"

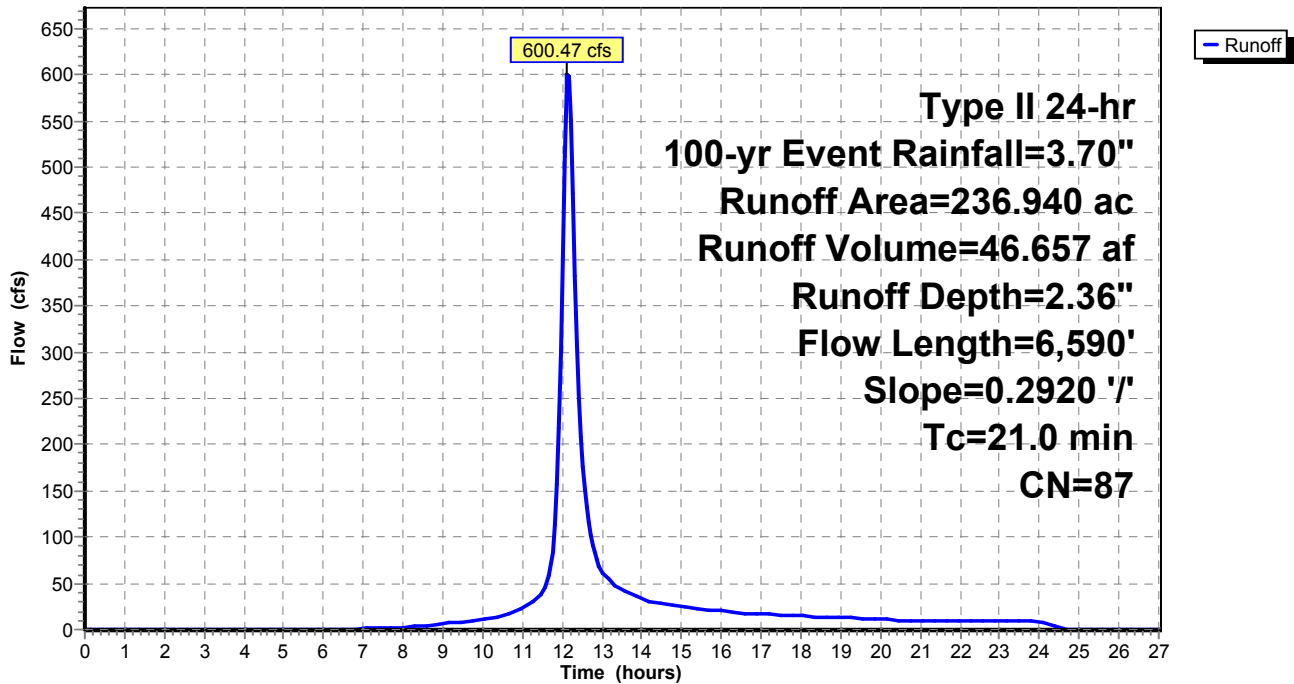
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

**Subcatchment 12: WS 12**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 13: WS 13**

Runoff = 641.72 cfs @ 12.18 hrs, Volume= 55.621 af, Depth= 2.36"

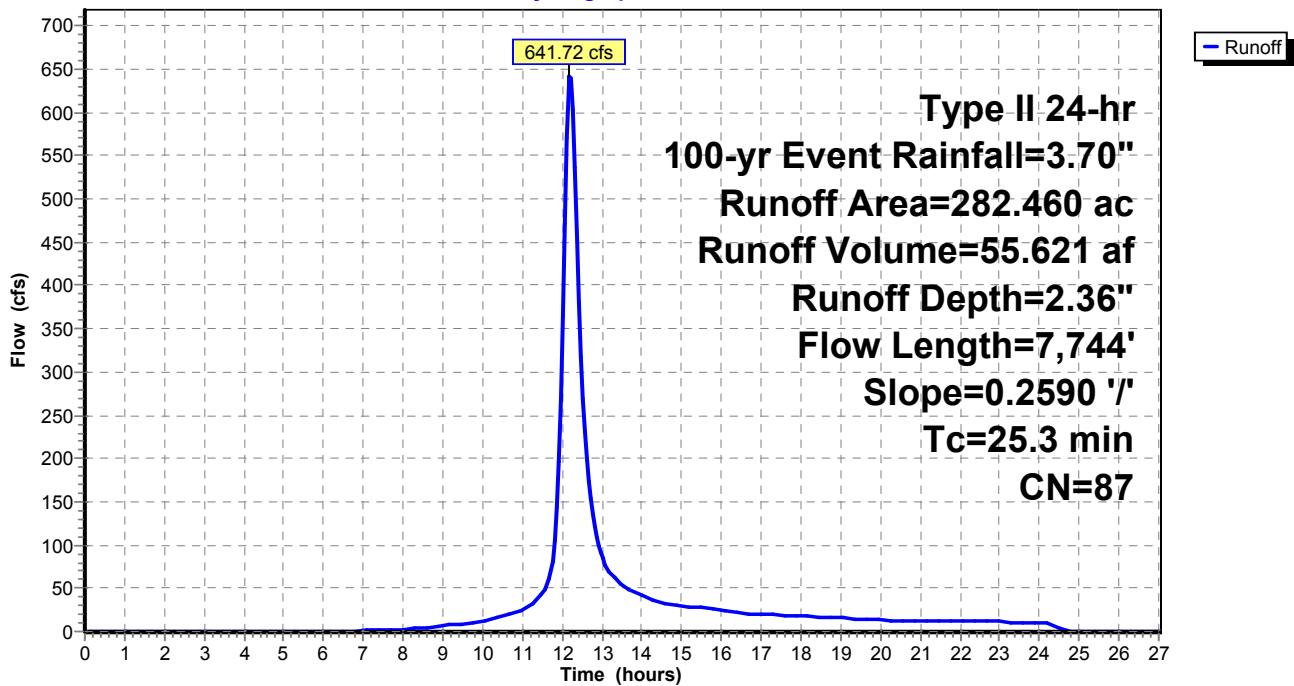
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 14: WS 14**

Runoff = 371.20 cfs @ 12.18 hrs, Volume= 31.893 af, Depth= 2.36"

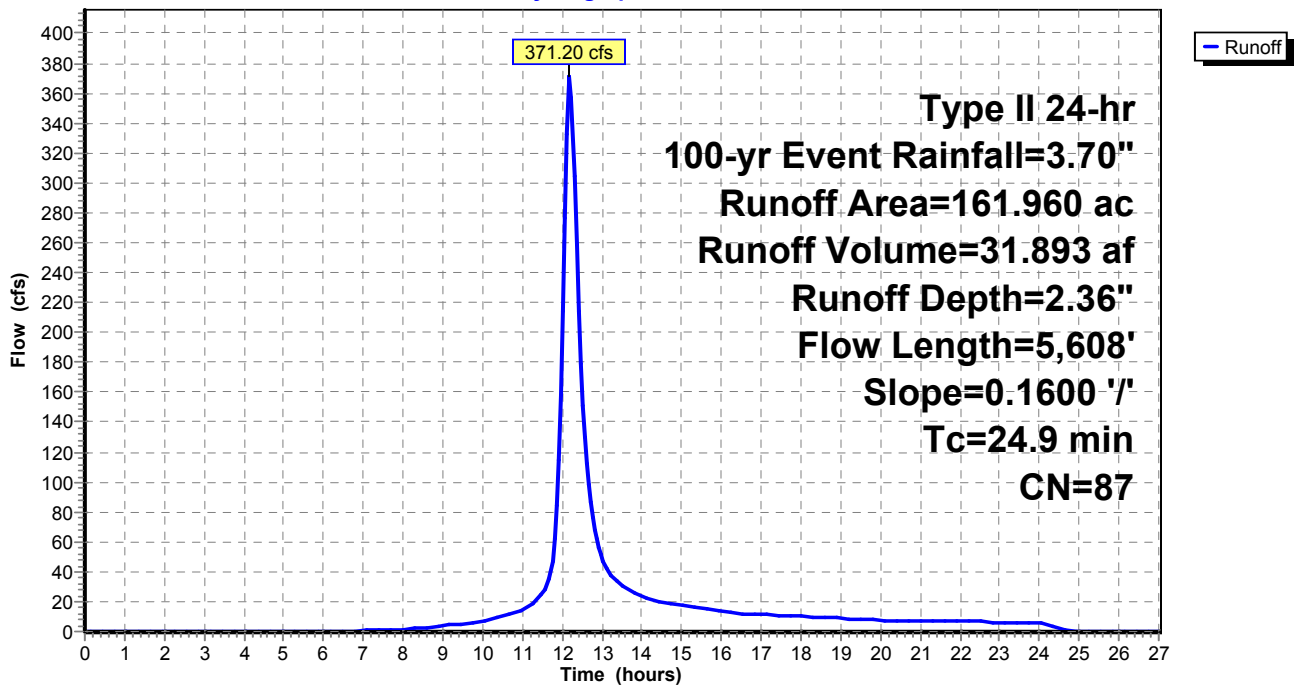
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 15: WS 15**

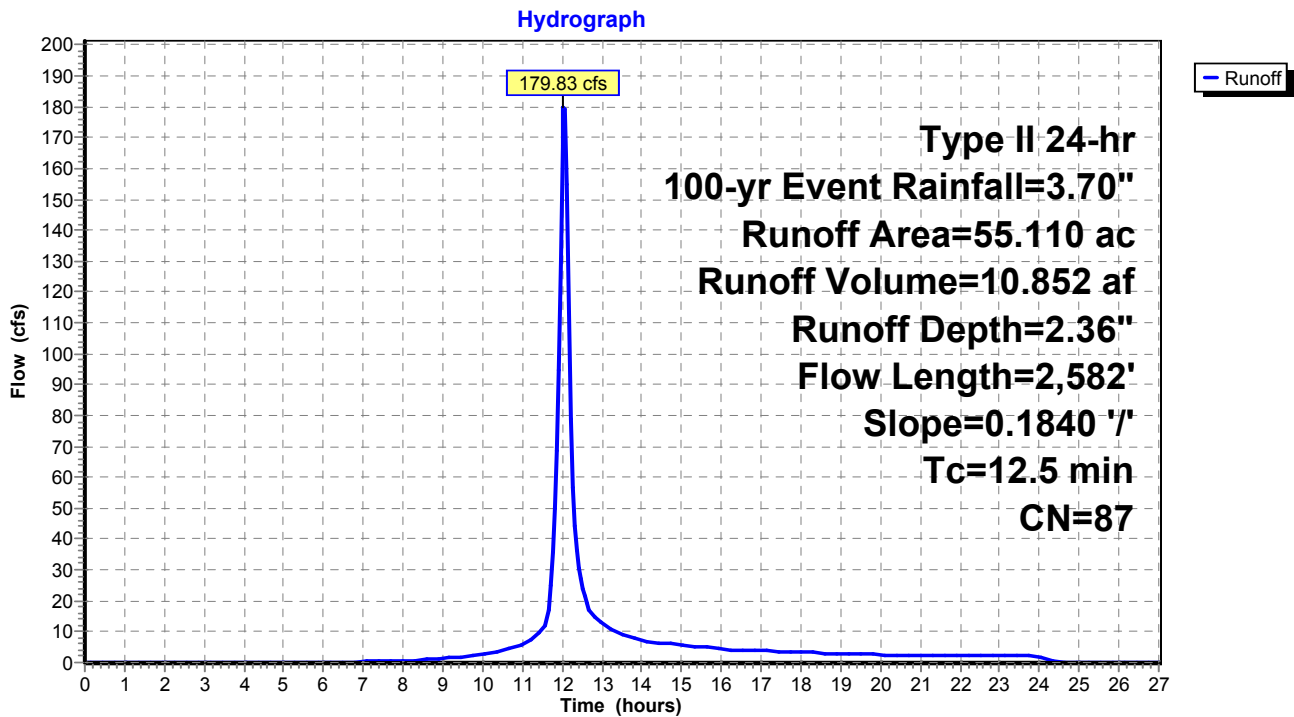
Runoff = 179.83 cfs @ 12.04 hrs, Volume= 10.852 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

**Subcatchment 15: WS 15**



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 16: WS 16**

Runoff = 204.89 cfs @ 12.15 hrs, Volume= 16.372 af, Depth= 2.36"

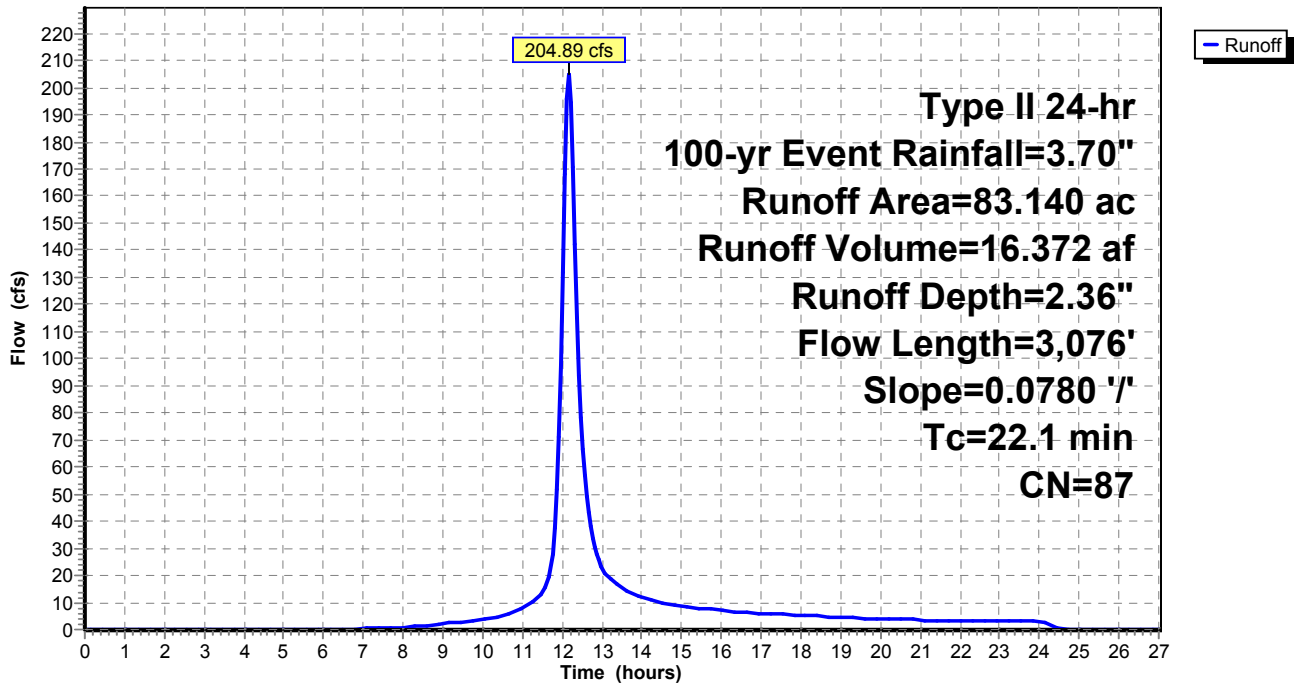
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

**Subcatchment 16: WS 16**

Hydrograph



**Existing Watersheds (Pre-Quintana)***Type II 24-hr 200-yr Event Rainfall=4.09"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=813.94 cfs	79.767 af	
<b>Subcatchment2: WS 2</b>	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=432.11 cfs	26.693 af	
<b>Subcatchment3: WS 3</b>	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=318.42 cfs	22.881 af	
<b>Subcatchment4: WS 4</b>	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=321.62 cfs	19.269 af	
<b>Subcatchment5: WS 5</b>	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=393.85 cfs	24.329 af	
<b>Subcatchment6: WS 6</b>	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=906.94 cfs	85.555 af	
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=464.86 cfs	32.739 af	
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=334.29 cfs	20.851 af	
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=651.70 cfs	53.460 af	
<b>Subcatchment10: WS 10</b>	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=758.38 cfs	75.602 af	
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=968.42 cfs	90.153 af	
<b>Subcatchment12: WS 12</b>	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=688.82 cfs	53.693 af	
<b>Subcatchment13: WS 13</b>	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=736.61 cfs	64.009 af	
<b>Subcatchment14: WS 14</b>	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=426.07 cfs	36.702 af	
<b>Subcatchment15: WS 15</b>	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=205.93 cfs	12.489 af	
<b>Subcatchment16: WS 16</b>	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=2.72"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=235.08 cfs	18.841 af	



**Existing Watersheds (Pre-Quintana)**

*Type II 24-hr 200-yr Event Rainfall=4.09"*

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**Total Runoff Area = 3,164.140 ac   Runoff Volume = 717.031 af   Average Runoff Depth = 2.72"**  
**90.00% Pervious = 2,847.730 ac   10.00% Impervious = 316.410 ac**

**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 1: WS 1**

Runoff = 813.94 cfs @ 12.25 hrs, Volume= 79.767 af, Depth= 2.72"

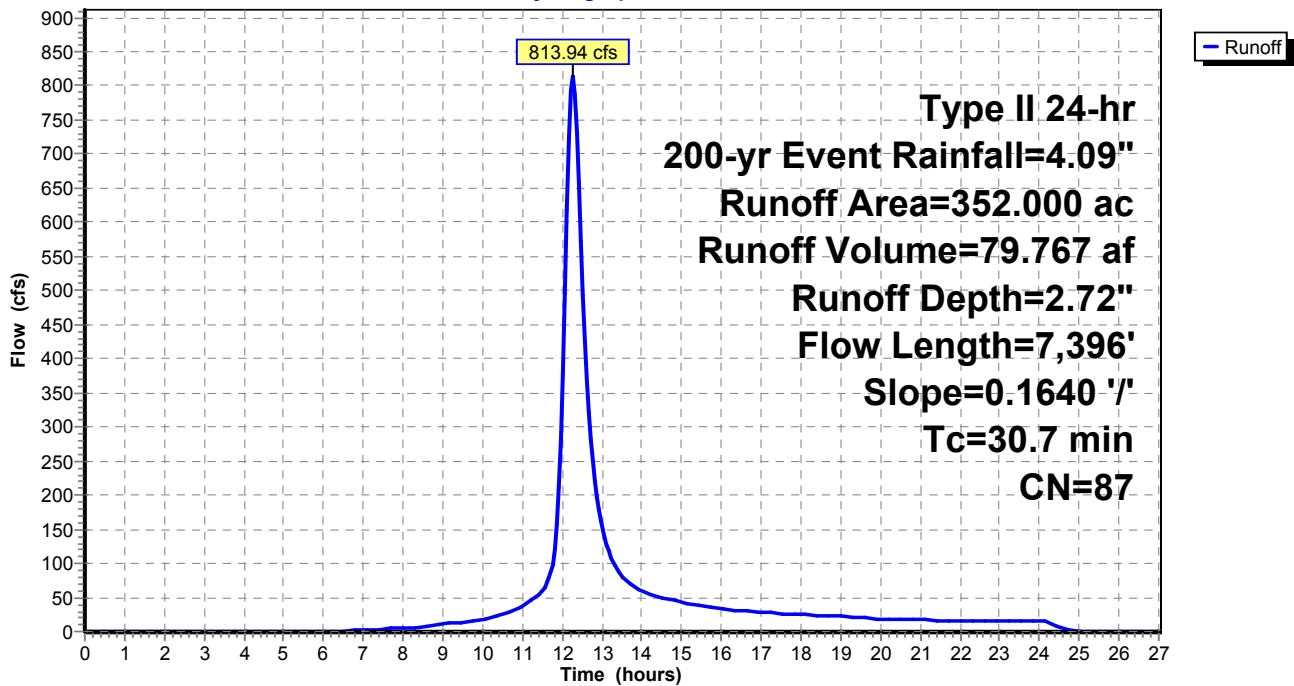
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

**Subcatchment 1: WS 1**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 2: WS 2**

Runoff = 432.11 cfs @ 12.05 hrs, Volume= 26.693 af, Depth= 2.72"

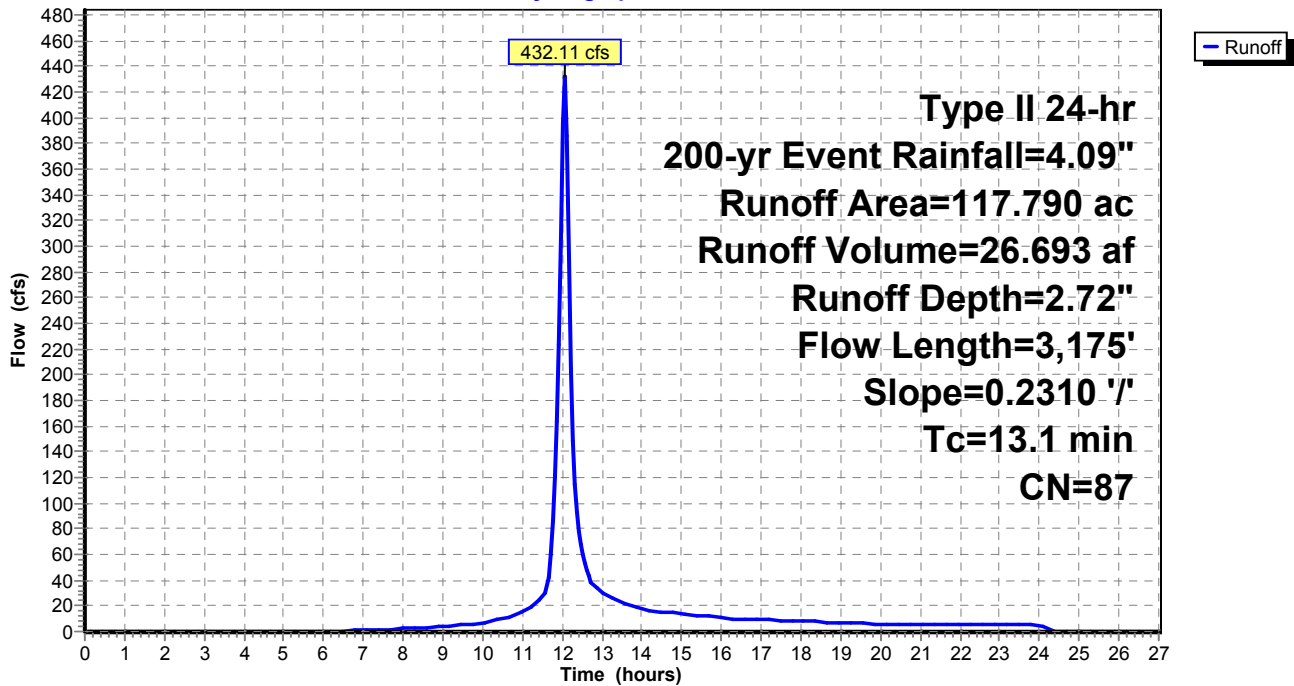
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 3: WS 3**

Runoff = 318.42 cfs @ 12.10 hrs, Volume= 22.881 af, Depth= 2.72"

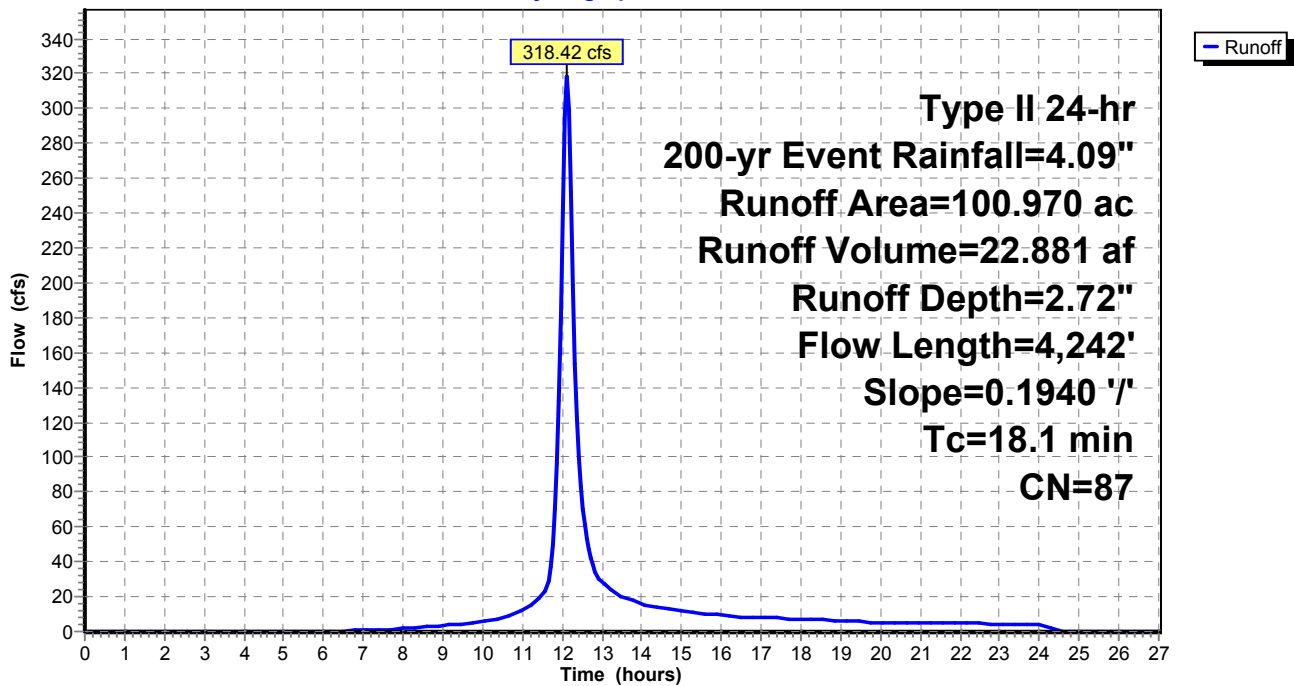
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 4: WS 4**

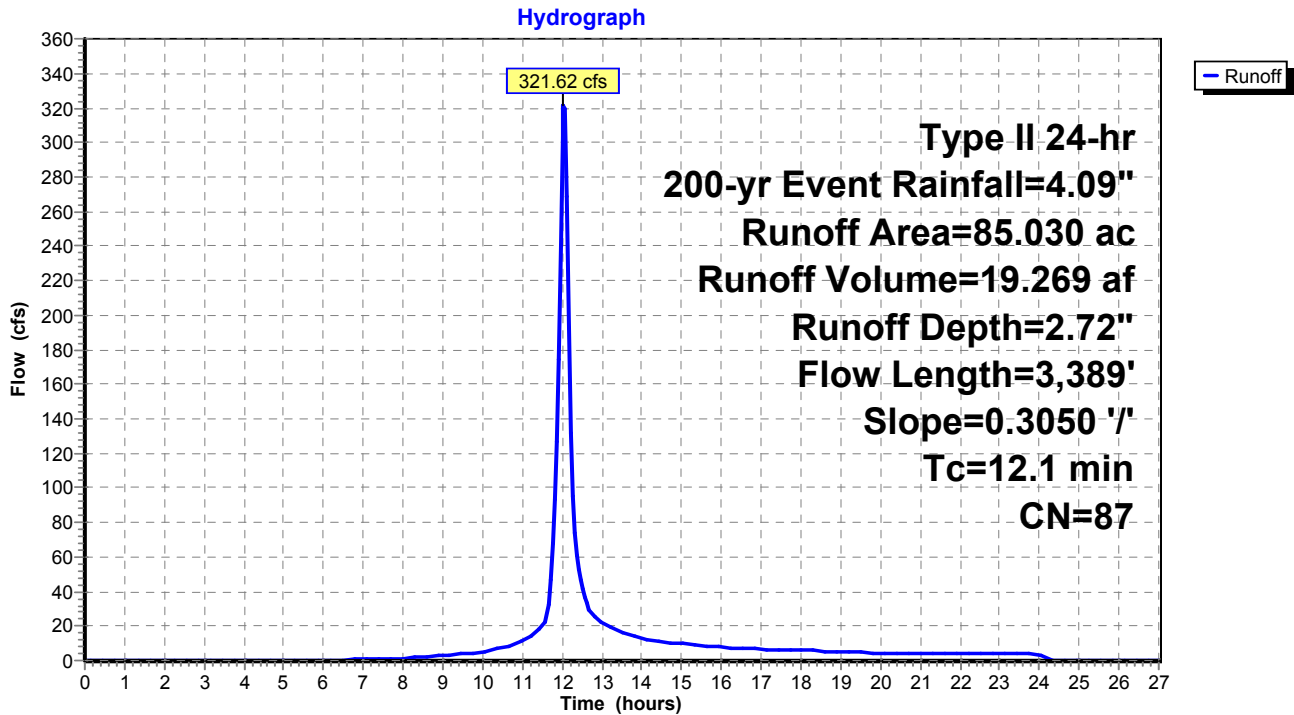
Runoff = 321.62 cfs @ 12.04 hrs, Volume= 19.269 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

**Subcatchment 4: WS 4**



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 5: WS 5**

Runoff = 393.85 cfs @ 12.05 hrs, Volume= 24.329 af, Depth= 2.72"

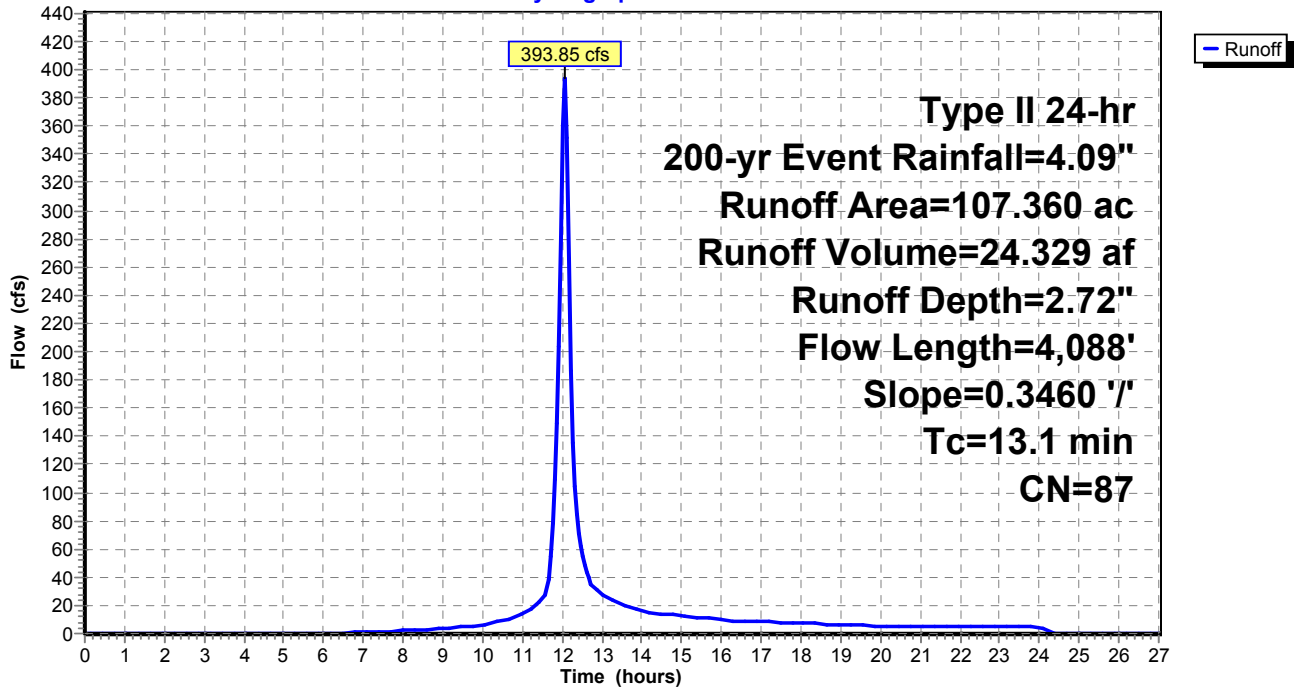
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 6: WS 6**

Runoff = 906.94 cfs @ 12.22 hrs, Volume= 85.555 af, Depth= 2.72"

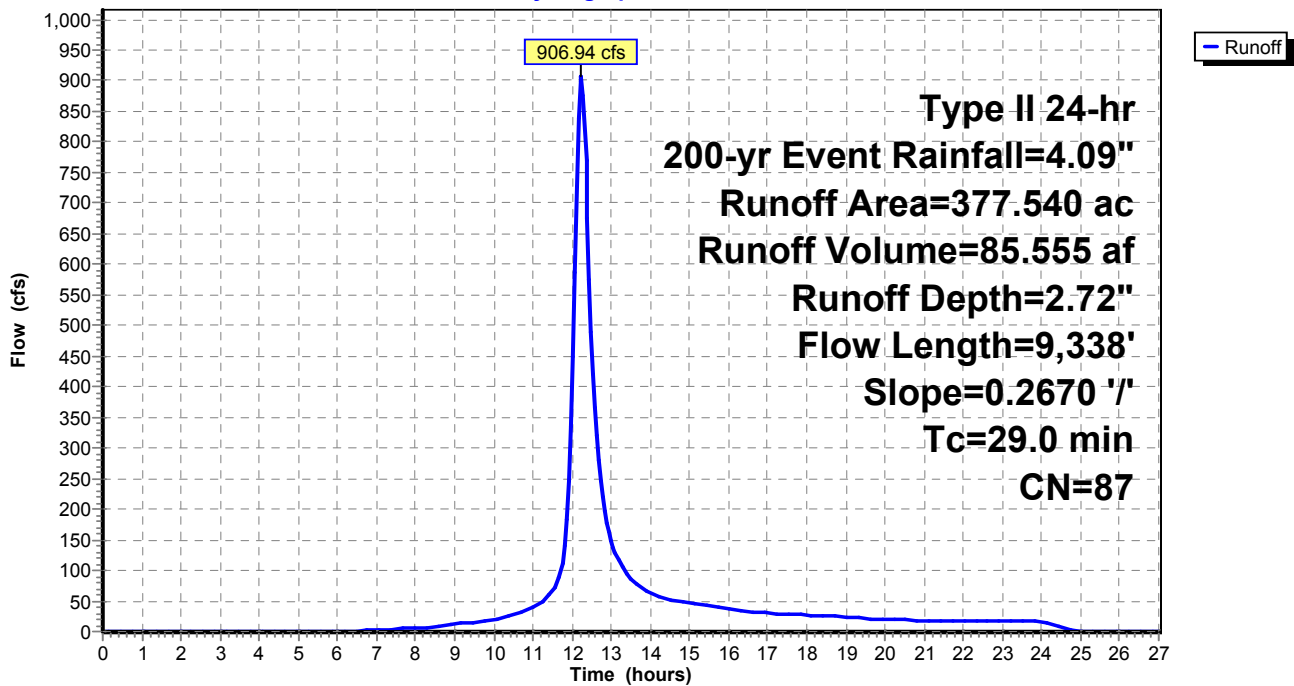
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 7: WS 7**

Runoff = 464.86 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

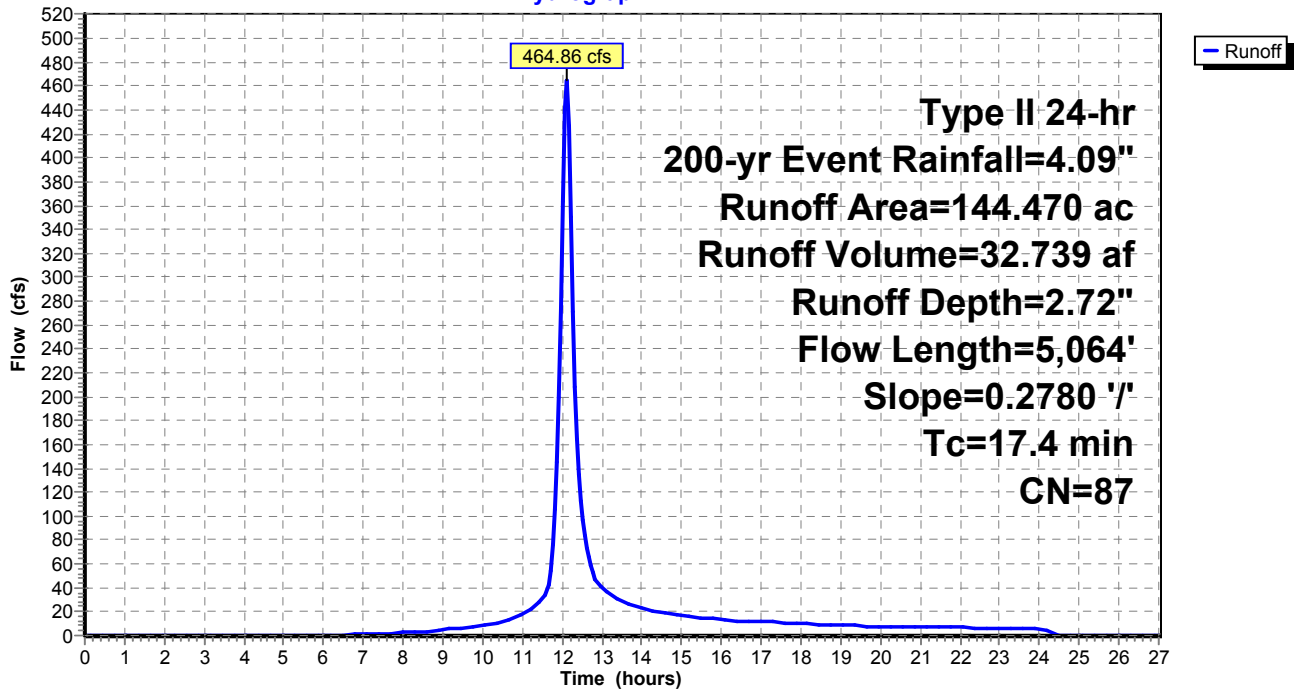
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 8: WS 8**

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

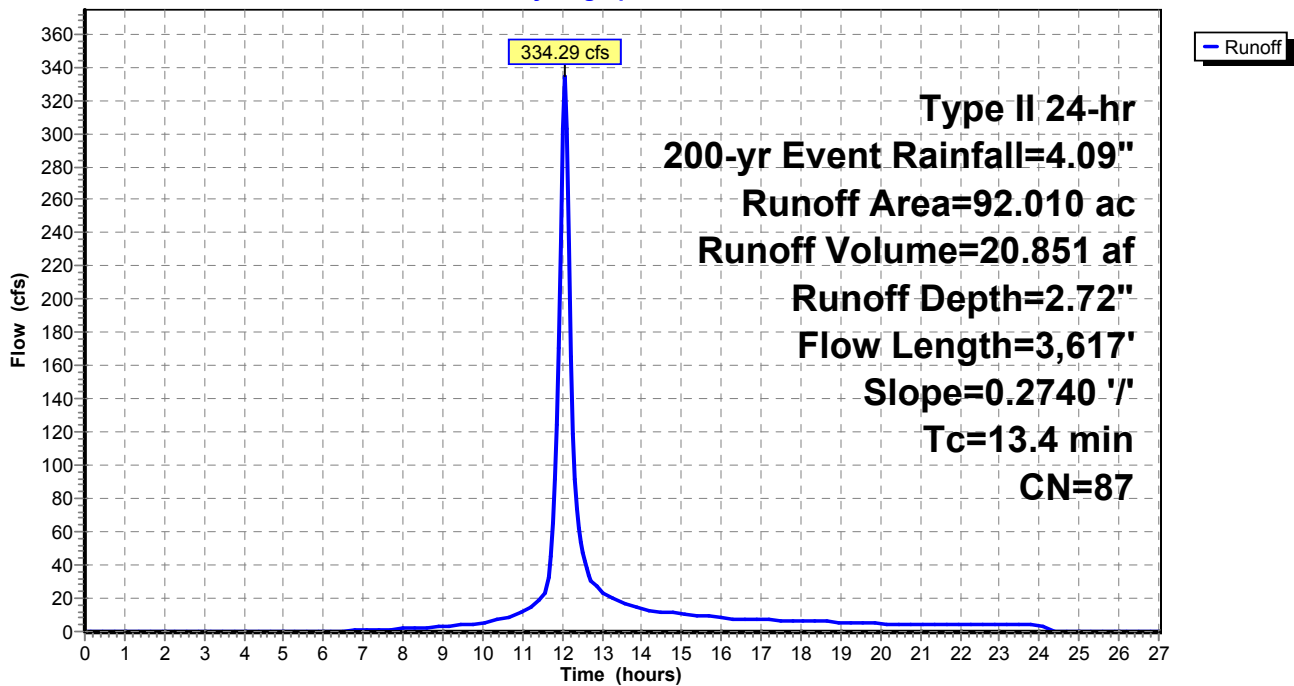
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 9: WS 9**

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

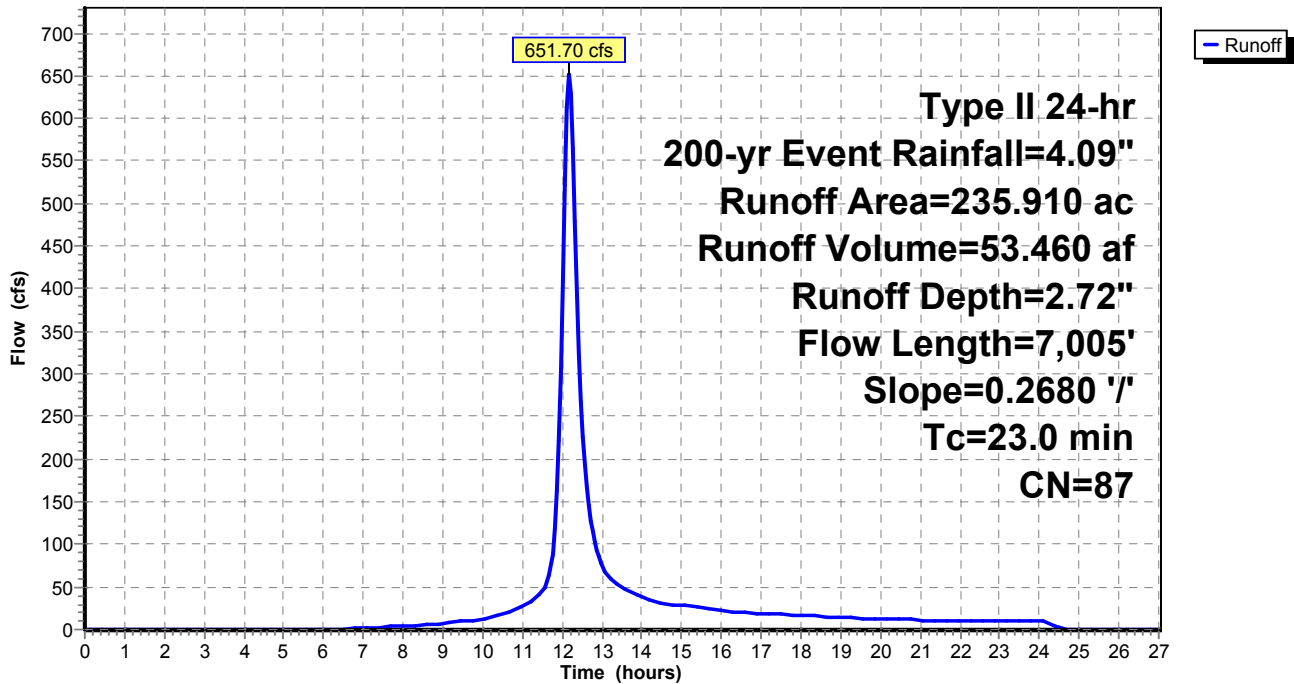
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 10: WS 10**

Runoff = 758.38 cfs @ 12.26 hrs, Volume= 75.602 af, Depth= 2.72"

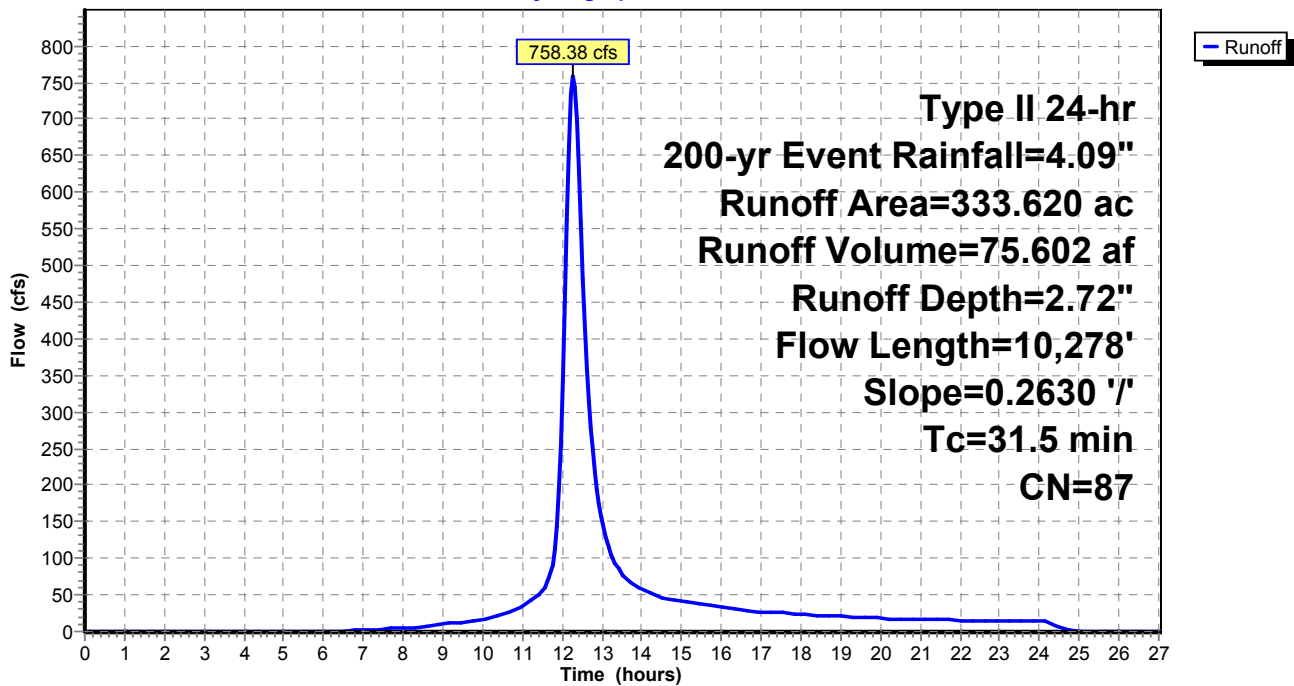
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 11: WS 11**

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

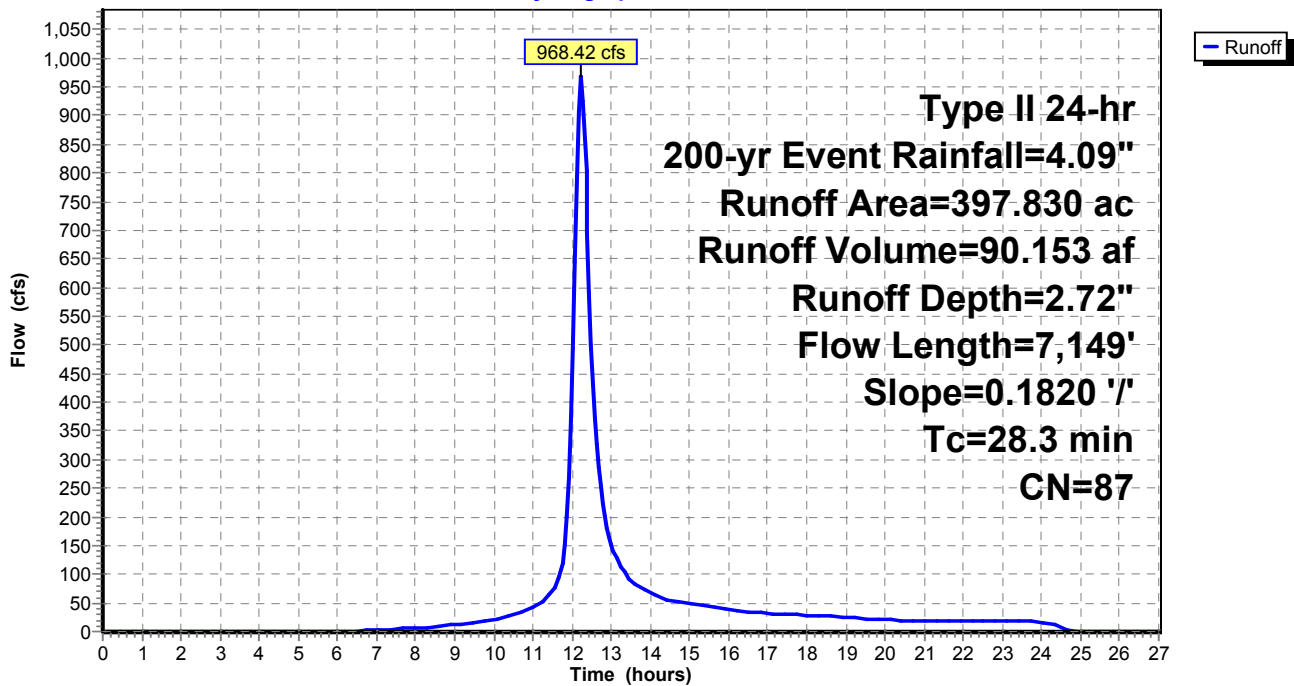
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 12: WS 12**

Runoff = 688.82 cfs @ 12.13 hrs, Volume= 53.693 af, Depth= 2.72"

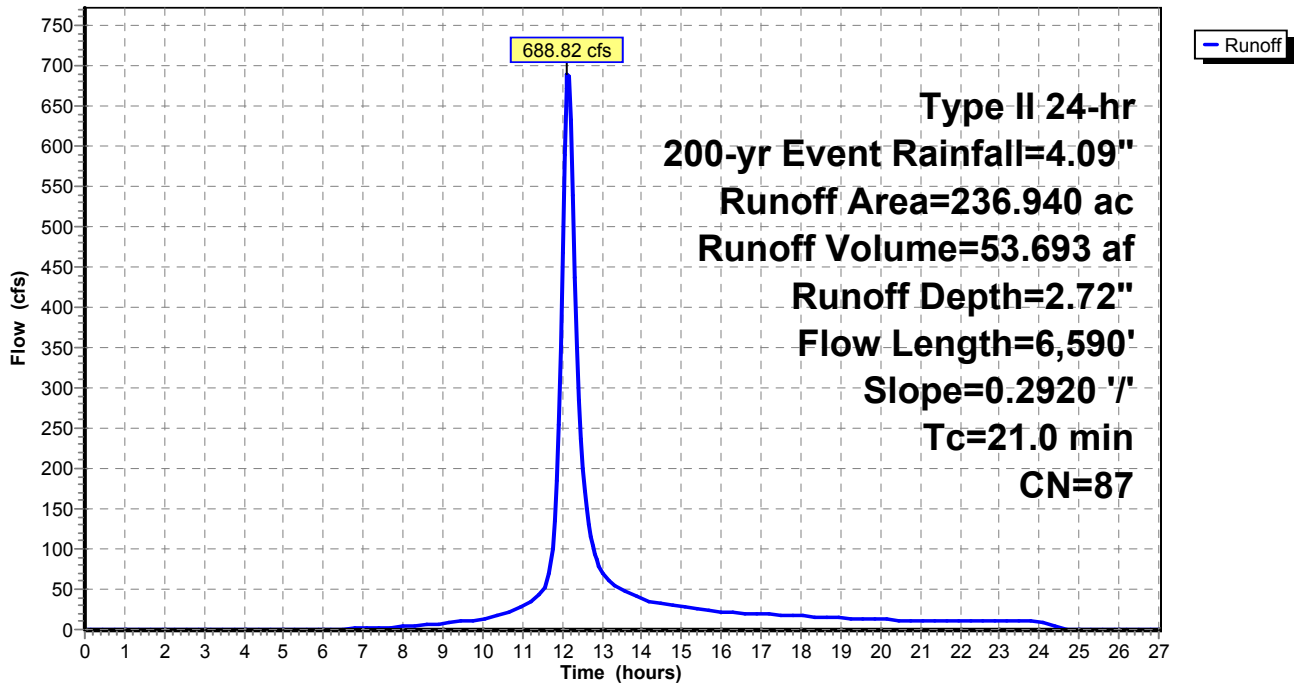
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

**Subcatchment 12: WS 12**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 13: WS 13**

Runoff = 736.61 cfs @ 12.18 hrs, Volume= 64.009 af, Depth= 2.72"

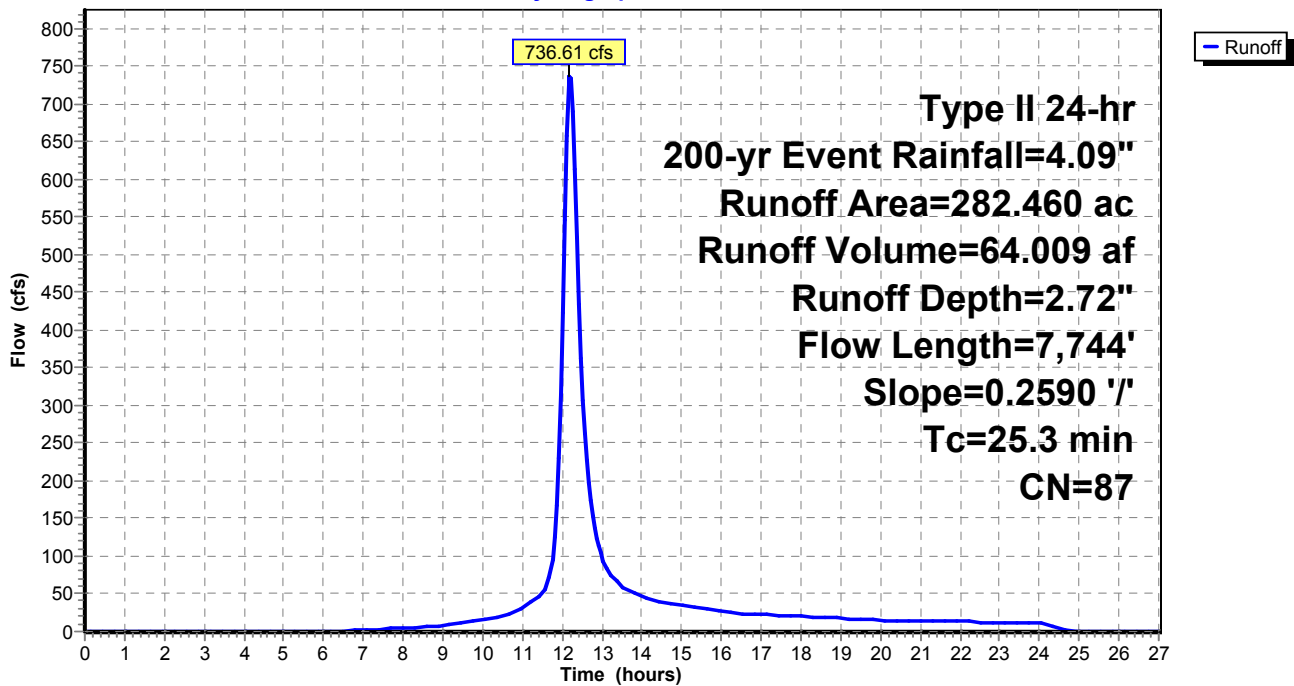
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 14: WS 14**

Runoff = 426.07 cfs @ 12.18 hrs, Volume= 36.702 af, Depth= 2.72"

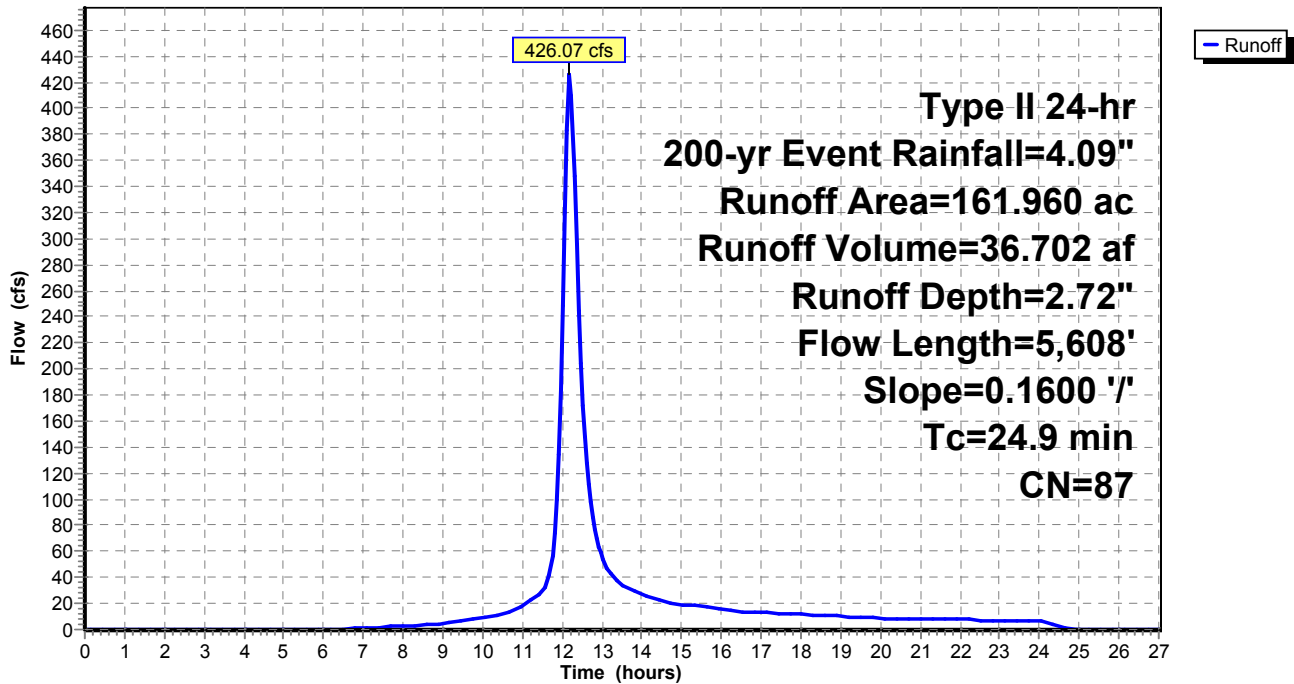
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 15: WS 15**

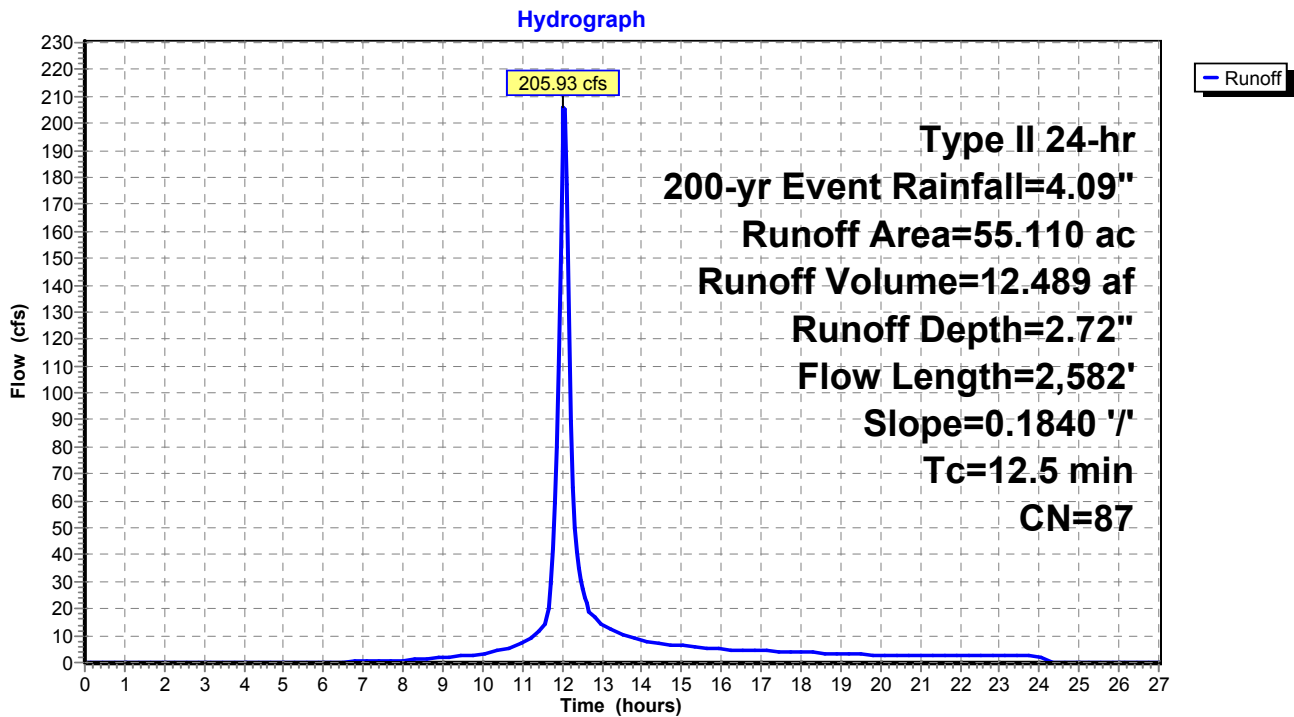
Runoff = 205.93 cfs @ 12.04 hrs, Volume= 12.489 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

**Subcatchment 15: WS 15**





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 16: WS 16**

Runoff = 235.08 cfs @ 12.15 hrs, Volume= 18.841 af, Depth= 2.72"

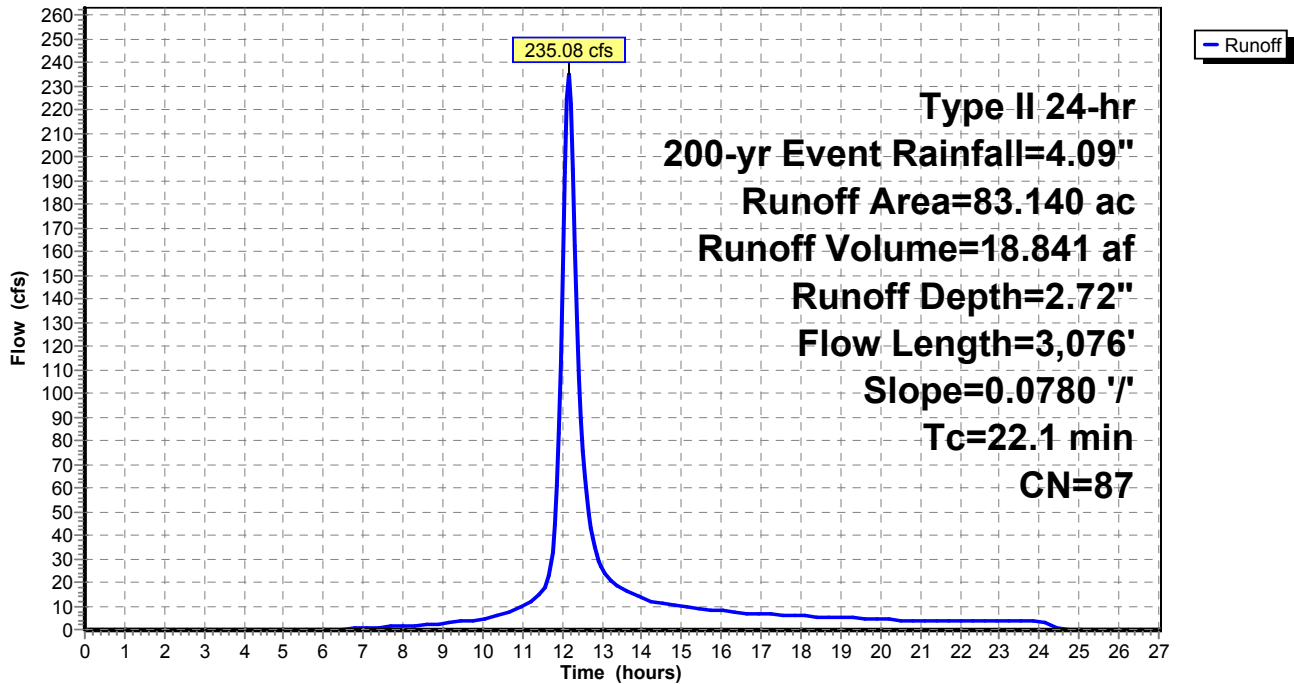
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

**Subcatchment 16: WS 16**

Hydrograph



**Existing Watersheds (Pre-Quintana)***Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=352.000 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,396'	Slope=0.1640 '/'	Tc=30.7 min	CN=87
	Runoff=952.23 cfs	93.637 af	
<b>Subcatchment2: WS 2</b>	Runoff Area=117.790 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,175'	Slope=0.2310 '/'	Tc=13.1 min	CN=87
	Runoff=504.05 cfs	31.334 af	
<b>Subcatchment3: WS 3</b>	Runoff Area=100.970 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=4,242'	Slope=0.1940 '/'	Tc=18.1 min	CN=87
	Runoff=371.87 cfs	26.859 af	
<b>Subcatchment4: WS 4</b>	Runoff Area=85.030 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,389'	Slope=0.3050 '/'	Tc=12.1 min	CN=87
	Runoff=375.03 cfs	22.619 af	
<b>Subcatchment5: WS 5</b>	Runoff Area=107.360 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=4,088'	Slope=0.3460 '/'	Tc=13.1 min	CN=87
	Runoff=459.42 cfs	28.559 af	
<b>Subcatchment6: WS 6</b>	Runoff Area=377.540 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=9,338'	Slope=0.2670 '/'	Tc=29.0 min	CN=87
	Runoff=1,060.97 cfs	100.431 af	
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=5,064'	Slope=0.2780 '/'	Tc=17.4 min	CN=87
	Runoff=542.79 cfs	38.431 af	
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,617'	Slope=0.2740 '/'	Tc=13.4 min	CN=87
	Runoff=389.99 cfs	24.476 af	
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,005'	Slope=0.2680 '/'	Tc=23.0 min	CN=87
	Runoff=761.76 cfs	62.755 af	
<b>Subcatchment10: WS 10</b>	Runoff Area=333.620 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=10,278'	Slope=0.2630 '/'	Tc=31.5 min	CN=87
	Runoff=887.32 cfs	88.748 af	
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,149'	Slope=0.1820 '/'	Tc=28.3 min	CN=87
	Runoff=1,132.73 cfs	105.828 af	
<b>Subcatchment12: WS 12</b>	Runoff Area=236.940 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=6,590'	Slope=0.2920 '/'	Tc=21.0 min	CN=87
	Runoff=804.86 cfs	63.029 af	
<b>Subcatchment13: WS 13</b>	Runoff Area=282.460 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=7,744'	Slope=0.2590 '/'	Tc=25.3 min	CN=87
	Runoff=861.28 cfs	75.138 af	
<b>Subcatchment14: WS 14</b>	Runoff Area=161.960 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=5,608'	Slope=0.1600 '/'	Tc=24.9 min	CN=87
	Runoff=498.15 cfs	43.084 af	
<b>Subcatchment15: WS 15</b>	Runoff Area=55.110 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=2,582'	Slope=0.1840 '/'	Tc=12.5 min	CN=87
	Runoff=240.16 cfs	14.660 af	
<b>Subcatchment16: WS 16</b>	Runoff Area=83.140 ac	10.00% Impervious	Runoff Depth=3.19"
Flow Length=3,076'	Slope=0.0780 '/'	Tc=22.1 min	CN=87
	Runoff=274.73 cfs	22.116 af	

**Existing Watersheds (Pre-Quintana)***Type II 24-hr 500-yr Event Rainfall=4.60"*

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**Total Runoff Area = 3,164.140 ac   Runoff Volume = 841.705 af   Average Runoff Depth = 3.19"**  
**90.00% Pervious = 2,847.730 ac   10.00% Impervious = 316.410 ac**

**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

Prepared by M3 Engineering

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**Summary for Subcatchment 1: WS 1**

Runoff = 952.23 cfs @ 12.25 hrs, Volume= 93.637 af, Depth= 3.19"

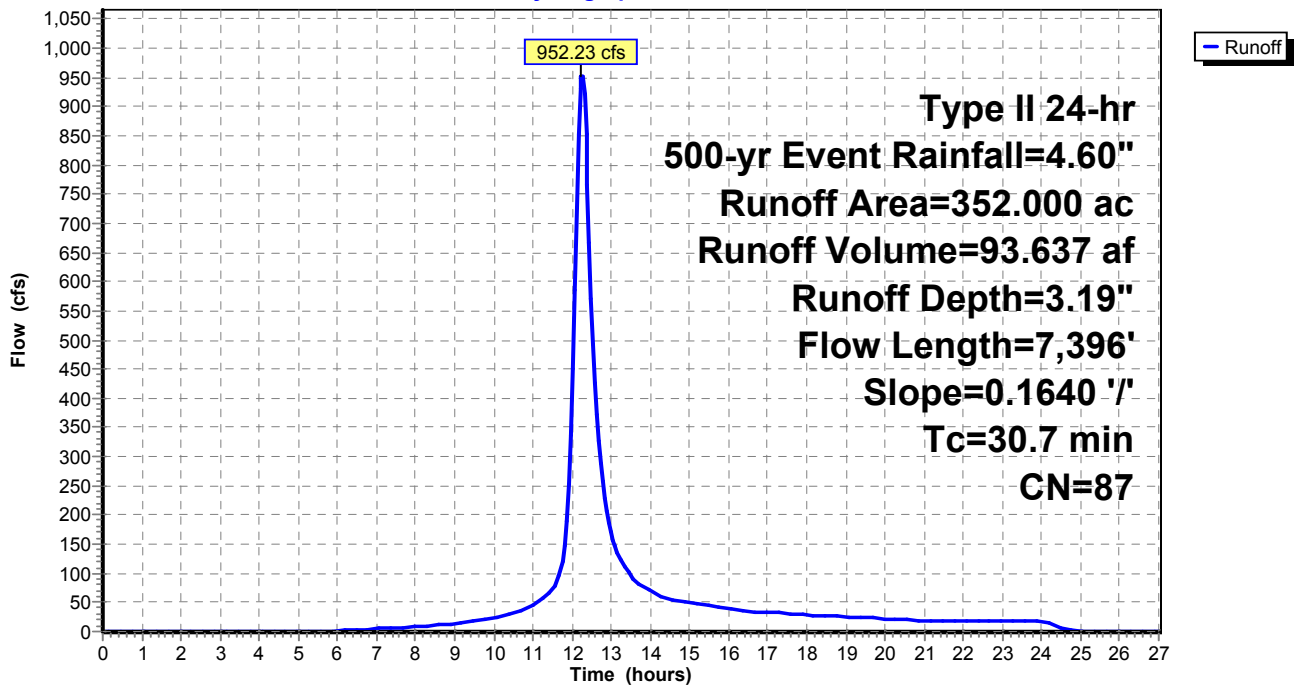
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
316.800	86	Desert shrub range, Fair, HSG D
* 35.200	98	Impervious, HSG D
352.000	87	Weighted Average
316.800		90.00% Pervious Area
35.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.7	7,396	0.1640	4.02		Lag/CN Method,

**Subcatchment 1: WS 1**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 2: WS 2**

Runoff = 504.05 cfs @ 12.05 hrs, Volume= 31.334 af, Depth= 3.19"

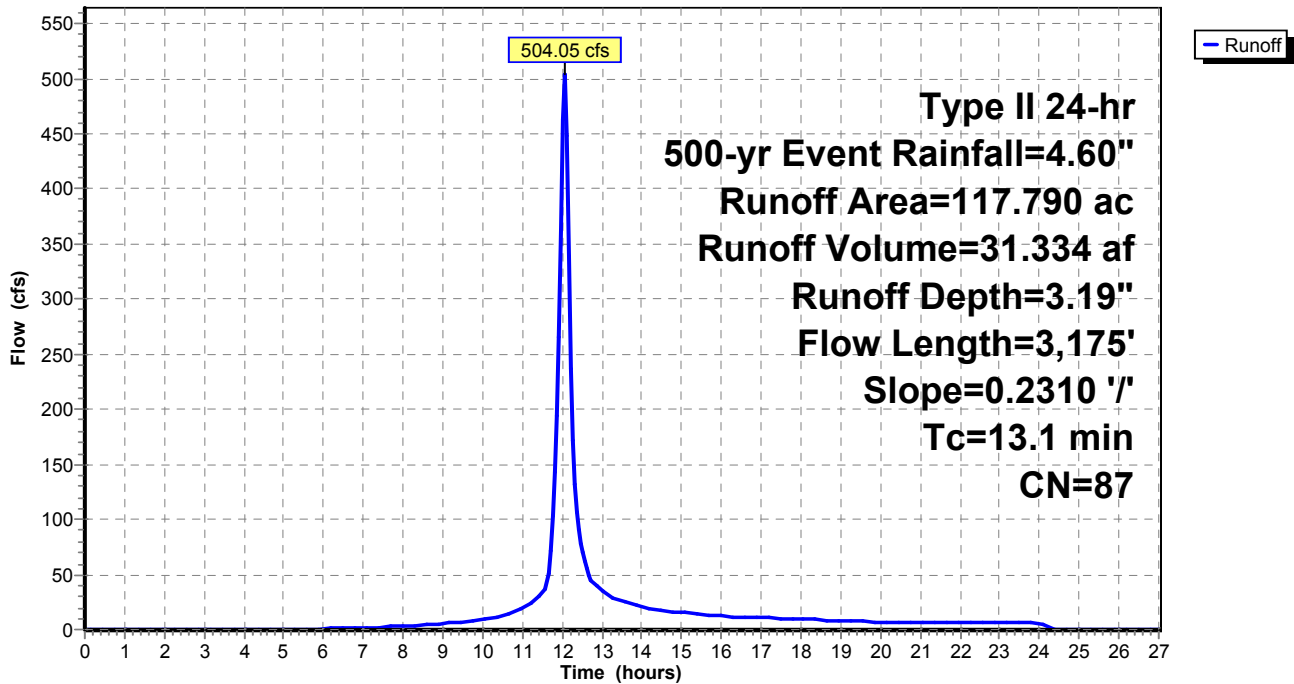
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
106.010	86	Desert shrub range, Fair, HSG D
* 11.780	98	Impervious, HSG D
117.790	87	Weighted Average
106.010		90.00% Pervious Area
11.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	3,175	0.2310	4.03		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 3: WS 3**

Runoff = 371.87 cfs @ 12.10 hrs, Volume= 26.859 af, Depth= 3.19"

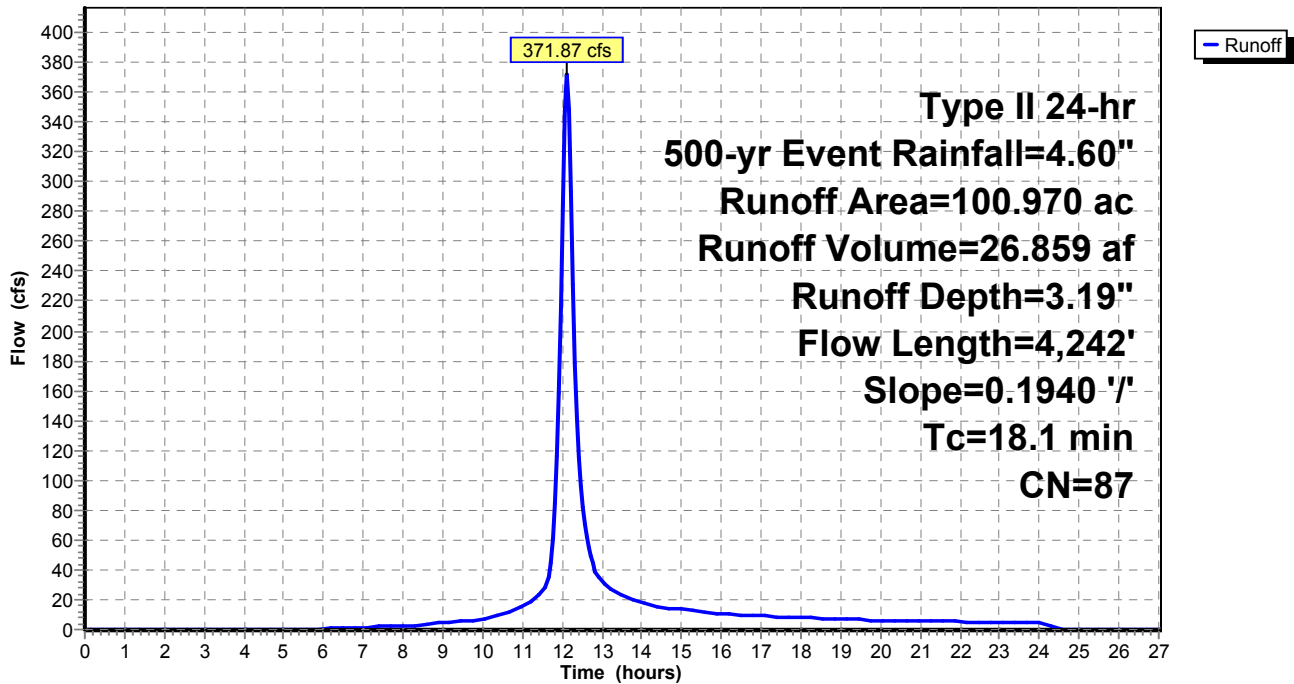
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
90.870	86	Desert shrub range, Fair, HSG D
* 10.100	98	Impervious, HSG D
100.970	87	Weighted Average
90.870		90.00% Pervious Area
10.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.1	4,242	0.1940	3.91		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 4: WS 4**

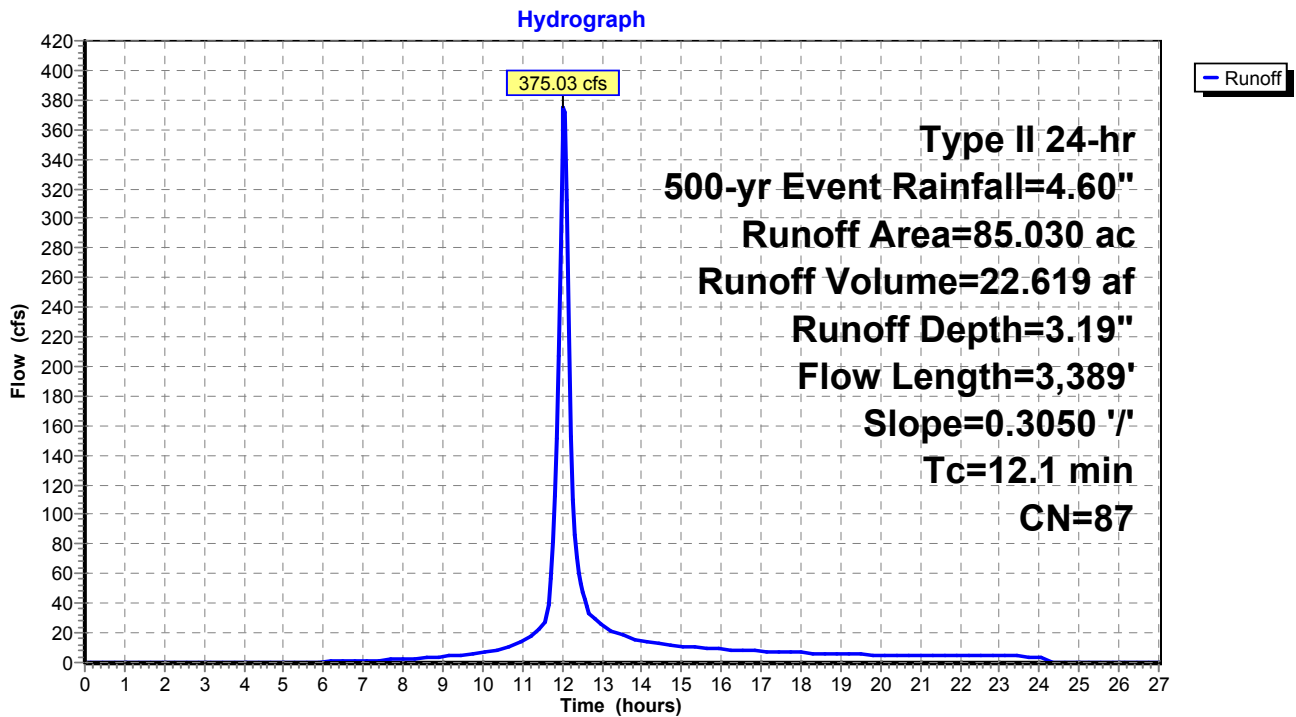
Runoff = 375.03 cfs @ 12.04 hrs, Volume= 22.619 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
76.530	86	Desert shrub range, Fair, HSG D
* 8.500	98	Impervious, HSG D
85.030	87	Weighted Average
76.530		90.00% Pervious Area
8.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1	3,389	0.3050	4.69		Lag/CN Method,

**Subcatchment 4: WS 4**



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 5: WS 5**

Runoff = 459.42 cfs @ 12.05 hrs, Volume= 28.559 af, Depth= 3.19"

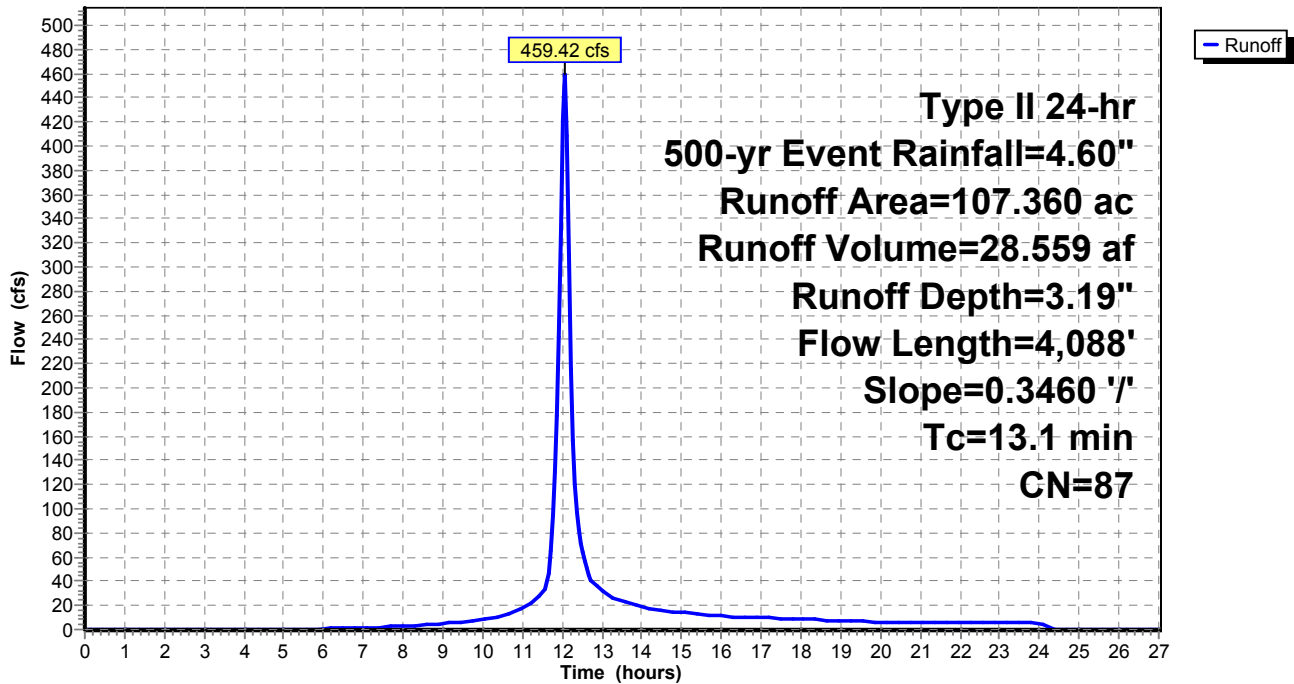
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
96.620	86	Desert shrub range, Fair, HSG D
* 10.740	98	Impervious, HSG D
107.360	87	Weighted Average
96.620		90.00% Pervious Area
10.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	4,088	0.3460	5.18		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 6: WS 6**

Runoff = 1,060.97 cfs @ 12.22 hrs, Volume= 100.431 af, Depth= 3.19"

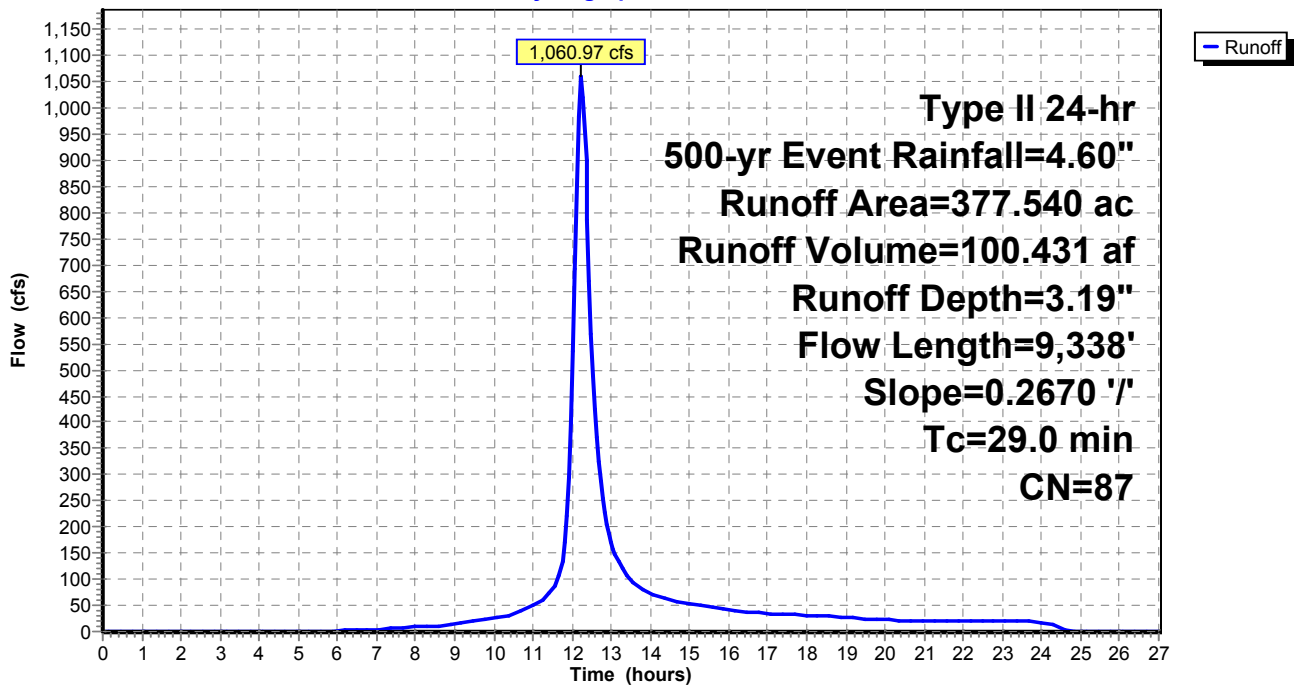
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
339.790	86	Desert shrub range, Fair, HSG D
* 37.750	98	Impervious, HSG D
377.540	87	Weighted Average
339.790		90.00% Pervious Area
37.750		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.0	9,338	0.2670	5.37		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 7: WS 7**

Runoff = 542.79 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

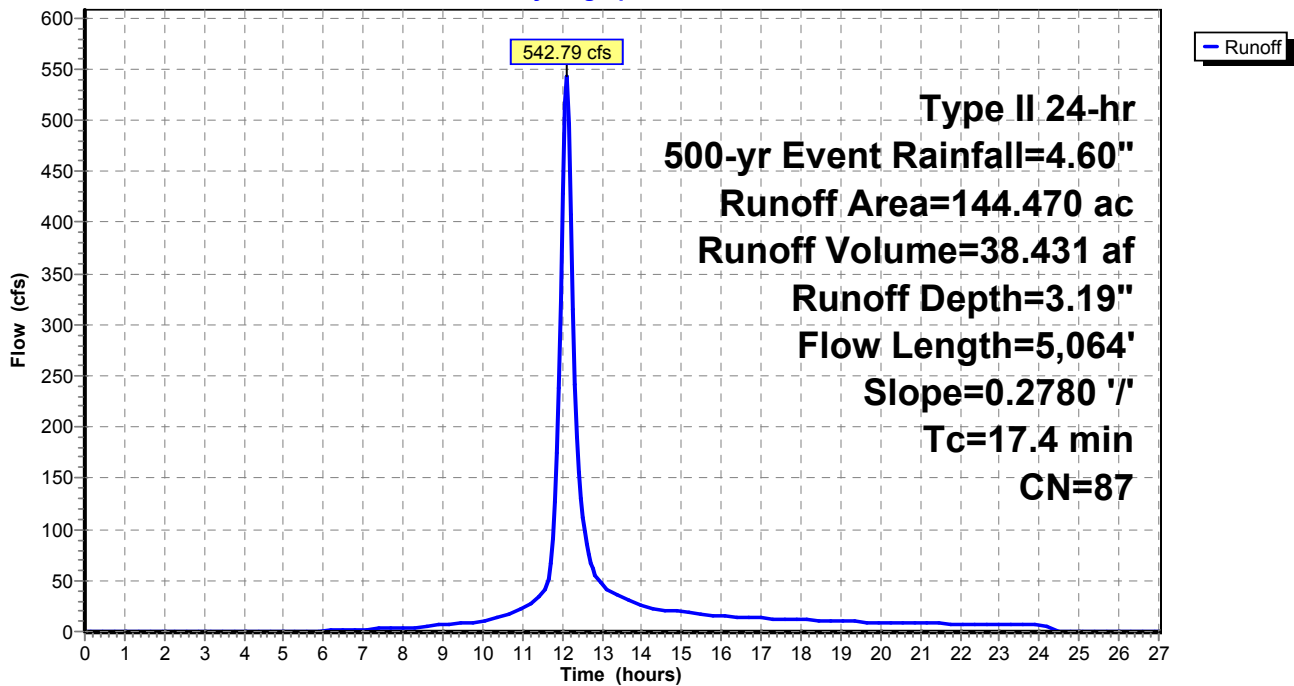
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.4	5,064	0.2780	4.85		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 8: WS 8**

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

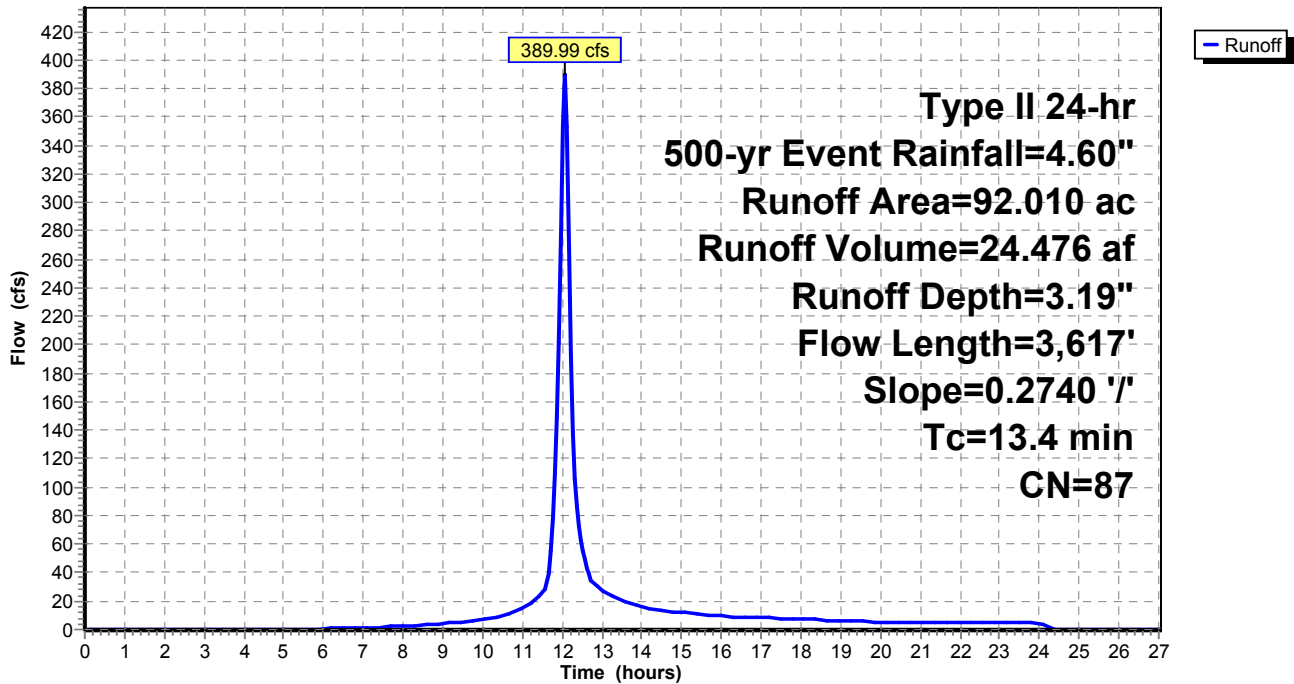
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 9: WS 9**

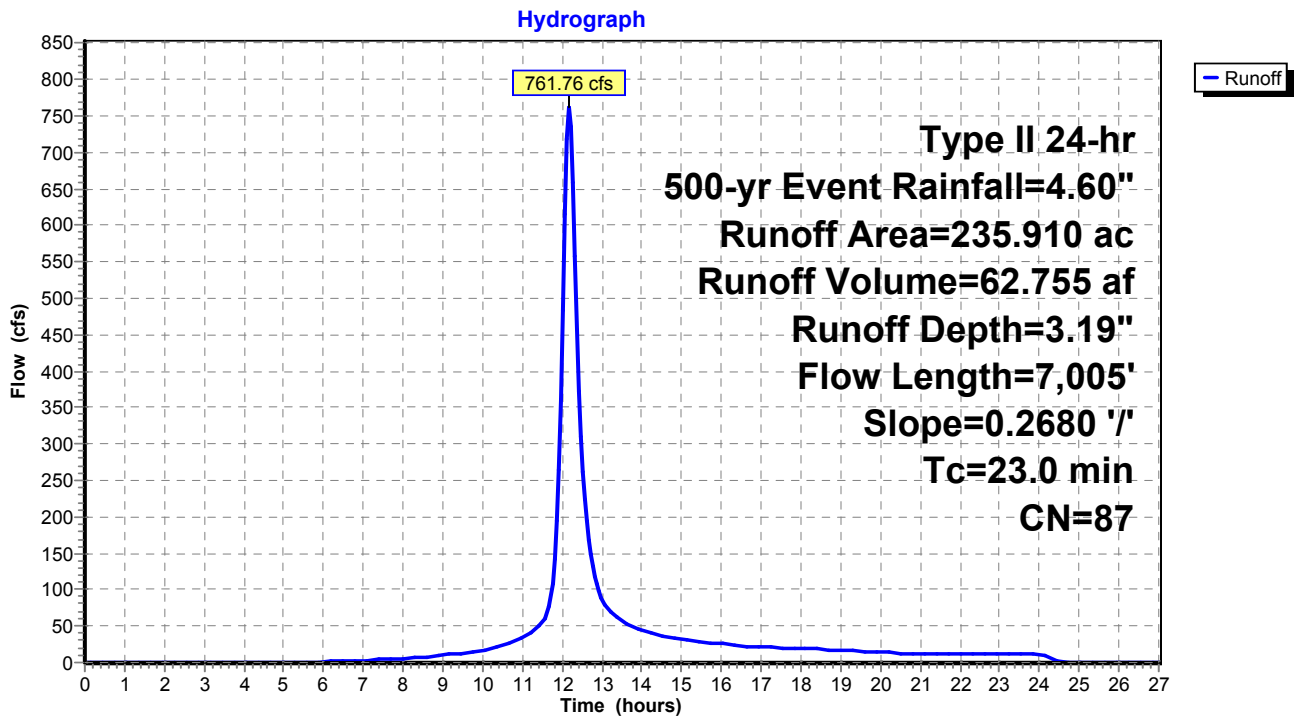
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 10: WS 10**

Runoff = 887.32 cfs @ 12.26 hrs, Volume= 88.748 af, Depth= 3.19"

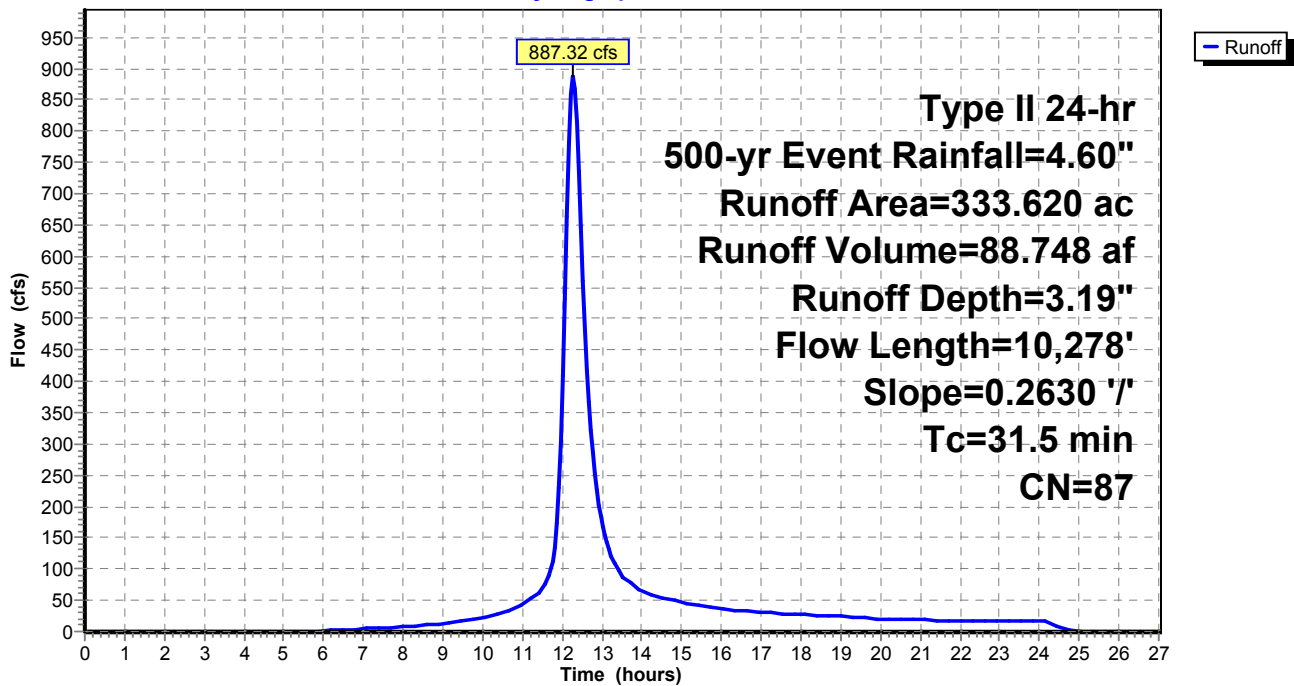
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
300.260	86	Desert shrub range, Fair, HSG D
* 33.360	98	Impervious, HSG D
333.620	87	Weighted Average
300.260		90.00% Pervious Area
33.360		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	10,278	0.2630	5.43		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 11: WS 11**

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

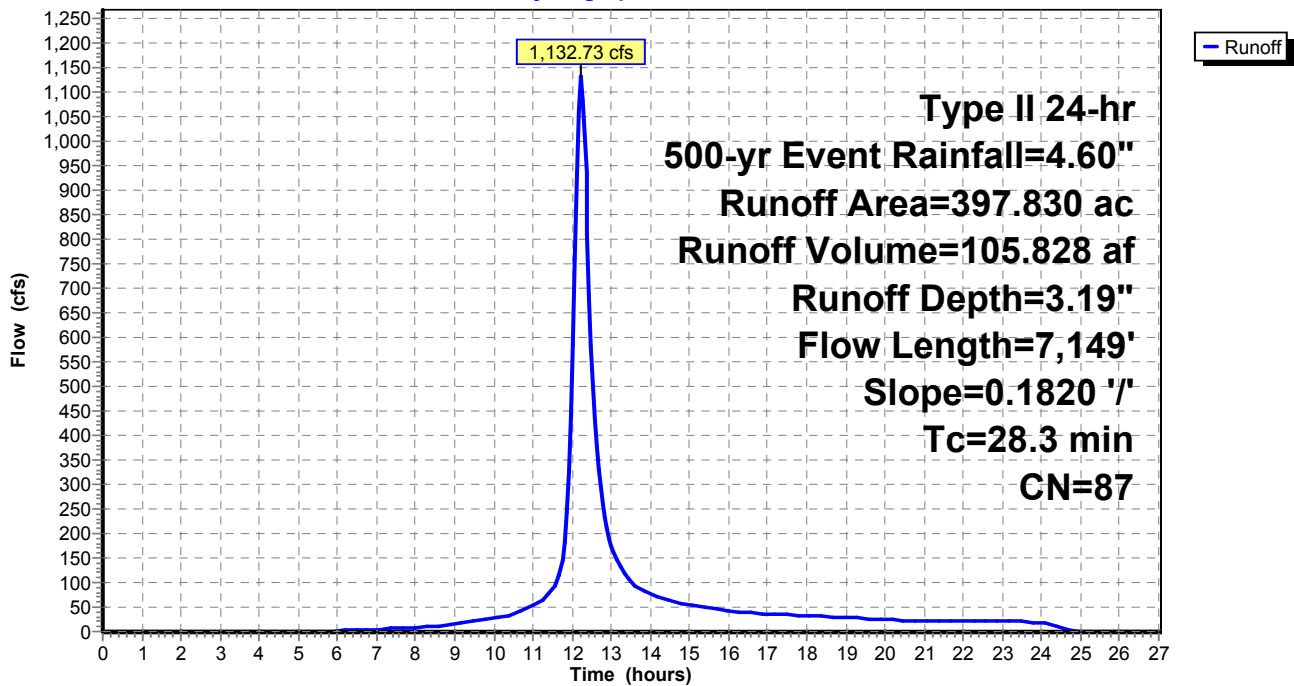
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 12: WS 12**

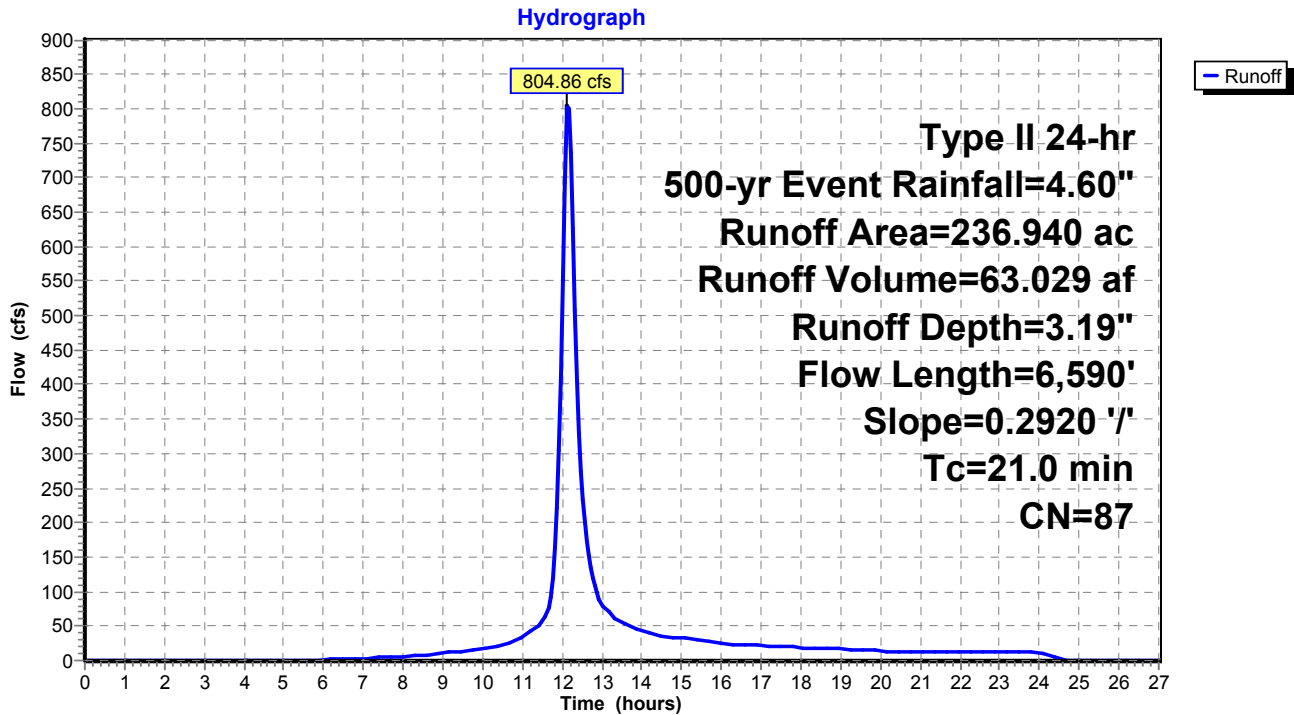
Runoff = 804.86 cfs @ 12.13 hrs, Volume= 63.029 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
213.250	86	Desert shrub range, Fair, HSG D
* 23.690	98	Impervious, HSG D
236.940	87	Weighted Average
213.250		90.00% Pervious Area
23.690		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.0	6,590	0.2920	5.24		Lag/CN Method,

**Subcatchment 12: WS 12**



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 13: WS 13**

Runoff = 861.28 cfs @ 12.18 hrs, Volume= 75.138 af, Depth= 3.19"

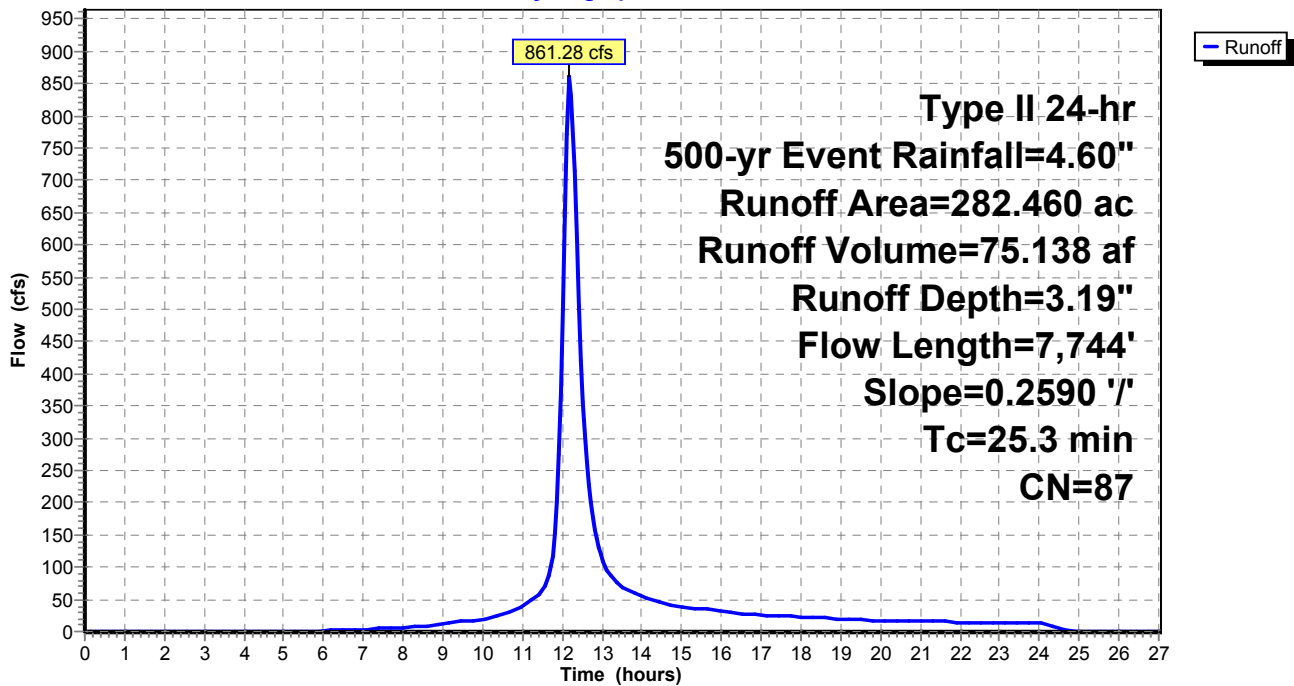
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
254.210	86	Desert shrub range, Fair, HSG D
* 28.250	98	Impervious, HSG D
282.460	87	Weighted Average
254.210		90.00% Pervious Area
28.250		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.3	7,744	0.2590	5.10		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph





**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 14: WS 14**

Runoff = 498.15 cfs @ 12.18 hrs, Volume= 43.084 af, Depth= 3.19"

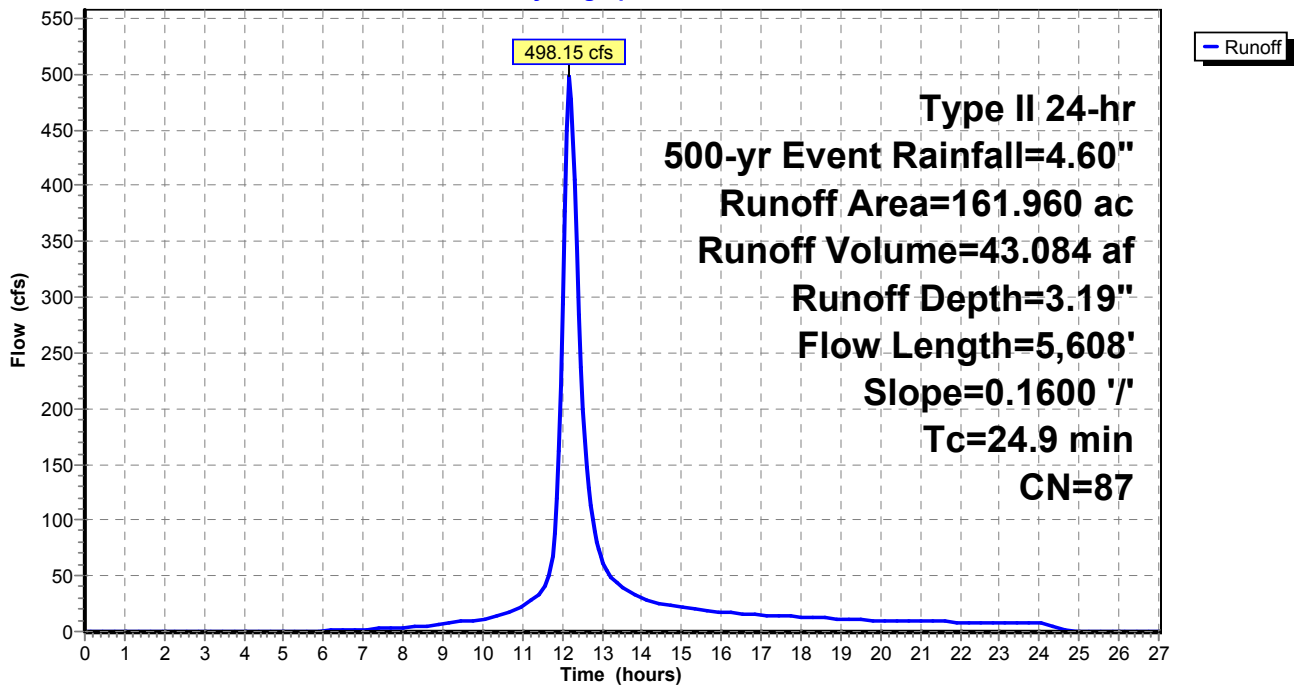
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
145.760	86	Desert shrub range, Fair, HSG D
* 16.200	98	Impervious, HSG D
161.960	87	Weighted Average
145.760		90.00% Pervious Area
16.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.9	5,608	0.1600	3.75		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 15: WS 15**

Runoff = 240.16 cfs @ 12.04 hrs, Volume= 14.660 af, Depth= 3.19"

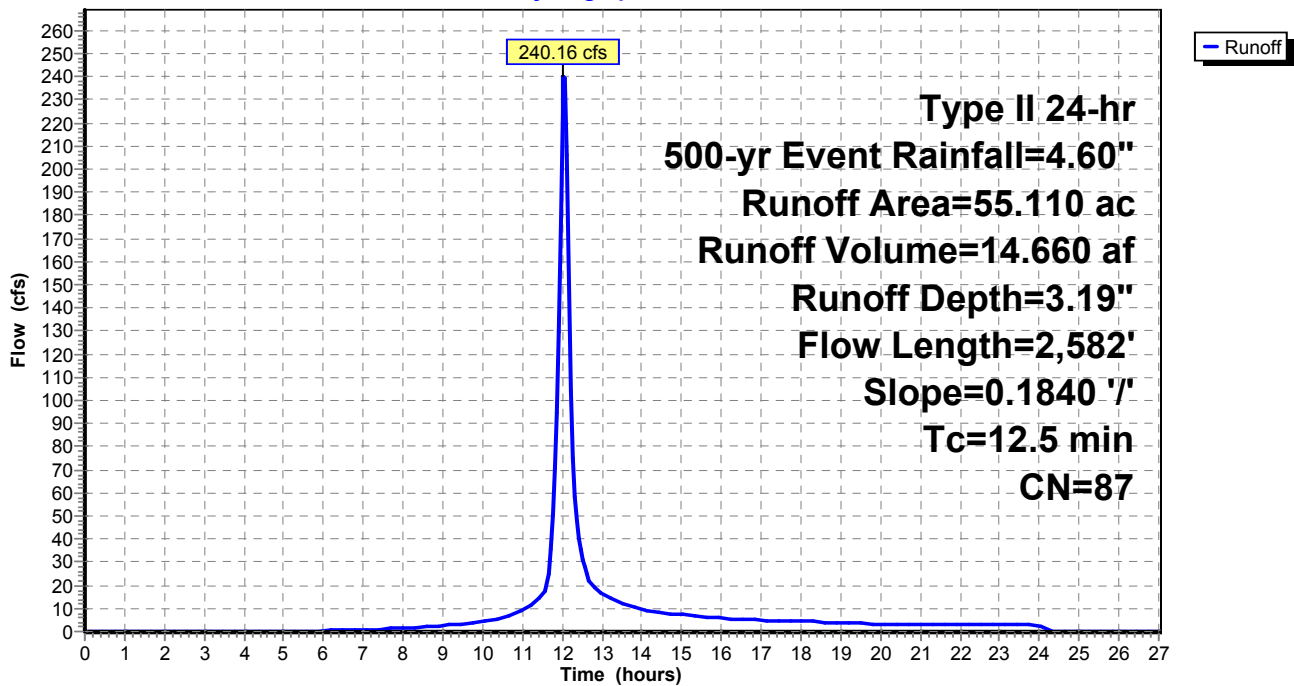
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
49.600	86	Desert shrub range, Fair, HSG D
* 5.510	98	Impervious, HSG D
55.110	87	Weighted Average
49.600		90.00% Pervious Area
5.510		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	2,582	0.1840	3.45		Lag/CN Method,

**Subcatchment 15: WS 15**

Hydrograph



**Existing Watersheds (Pre-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 16: WS 16**

Runoff = 274.73 cfs @ 12.15 hrs, Volume= 22.116 af, Depth= 3.19"

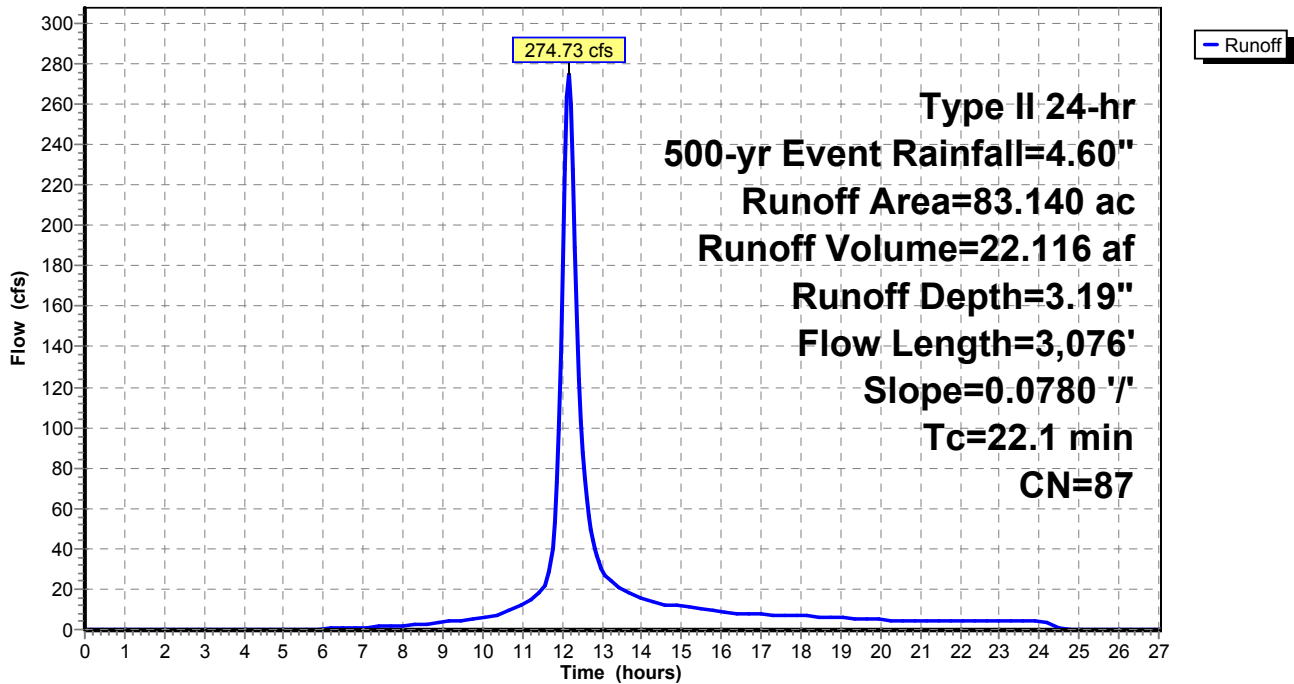
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
74.830	86	Desert shrub range, Fair, HSG D
* 8.310	98	Impervious, HSG D
83.140	87	Weighted Average
74.830		90.00% Pervious Area
8.310		10.00% Impervious Area

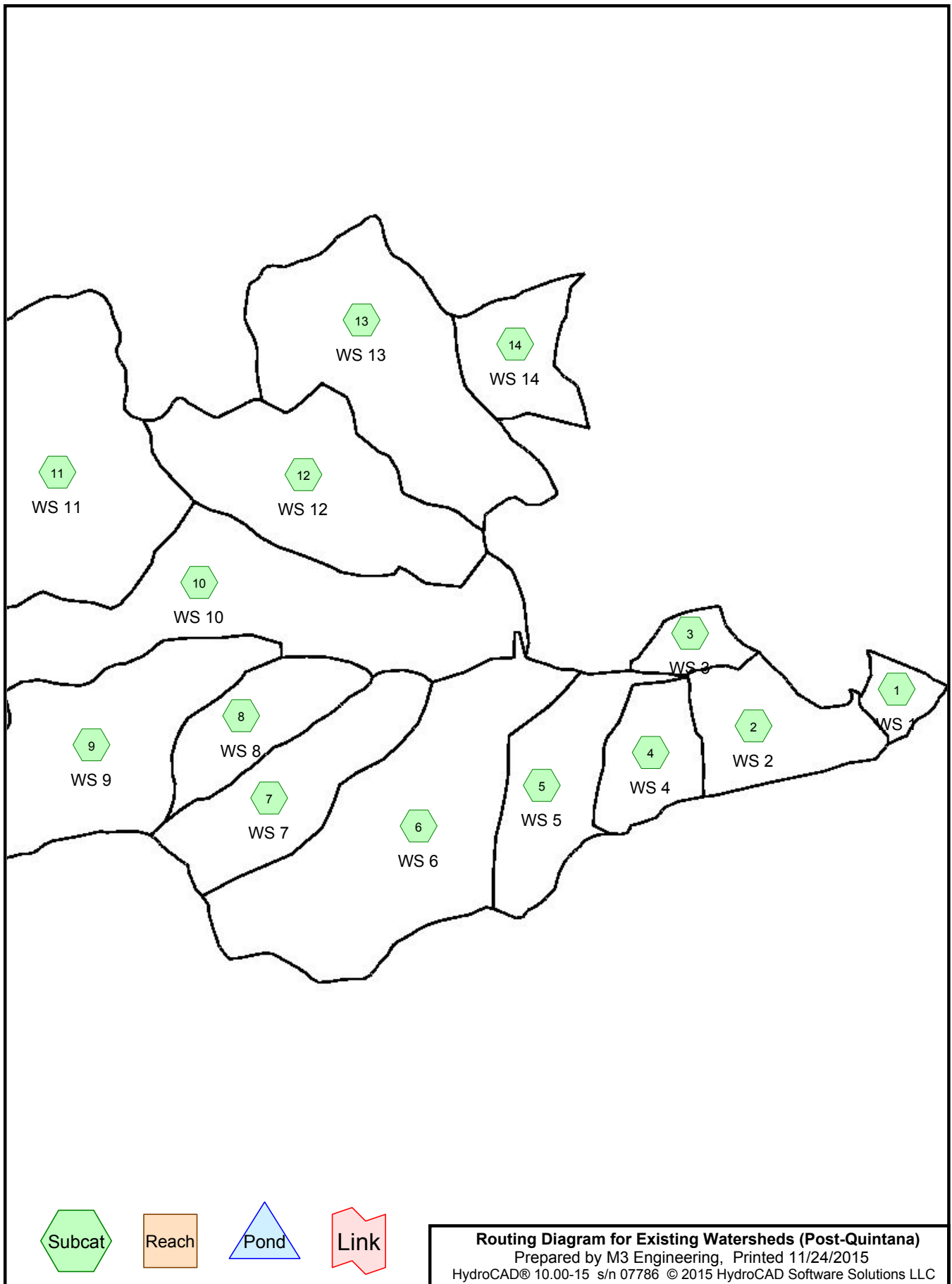
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.1	3,076	0.0780	2.32		Lag/CN Method,

**Subcatchment 16: WS 16**

Hydrograph



## APPENDIX C



**Existing Watersheds (Post-Quintana)**

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**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
2,235.360	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14)
248.370	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14)
<b>2,483.730</b>	<b>87</b>	<b>TOTAL AREA</b>

**Existing Watersheds (Post-Quintana)**

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**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2,483.730	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
0.000	Other	
<b>2,483.730</b>		<b>TOTAL AREA</b>

**Existing Watersheds (Post-Quintana)**

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**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,235.360	0.000	2,235.360	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
0.000	0.000	0.000	248.370	0.000	248.370	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14
<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>2,483.730</b>	<b>0.000</b>	<b>2,483.730</b>	<b>TOTAL AREA</b>	



**Existing Watersheds (Post-Quintana)***Type II 24-hr 100-yr Event Rainfall=3.70"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.36" Tc=5.0 min CN=87 Runoff=117.20 cfs 5.555 af
<b>Subcatchment2: WS 2</b>	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=363.66 cfs 20.985 af
<b>Subcatchment3: WS 3</b>	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=129.54 cfs 6.890 af
<b>Subcatchment4: WS 4</b>	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=259.92 cfs 14.773 af
<b>Subcatchment5: WS 5</b>	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=342.49 cfs 24.455 af
<b>Subcatchment6: WS 6</b>	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=774.13 cfs 65.223 af
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=411.06 cfs 28.449 af
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=291.83 cfs 18.118 af
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=567.91 cfs 46.455 af
<b>Subcatchment10: WS 10</b>	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=663.73 cfs 65.063 af
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=843.39 cfs 78.339 af
<b>Subcatchment12: WS 12</b>	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=588.30 cfs 44.783 af
<b>Subcatchment13: WS 13</b>	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=649.87 cfs 54.252 af
<b>Subcatchment14: WS 14</b>	Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=243.90 cfs 15.747 af

**Total Runoff Area = 2,483.730 ac Runoff Volume = 489.087 af Average Runoff Depth = 2.36"**  
**90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac**

**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 117.20 cfs @ 11.95 hrs, Volume= 5.555 af, Depth= 2.36"

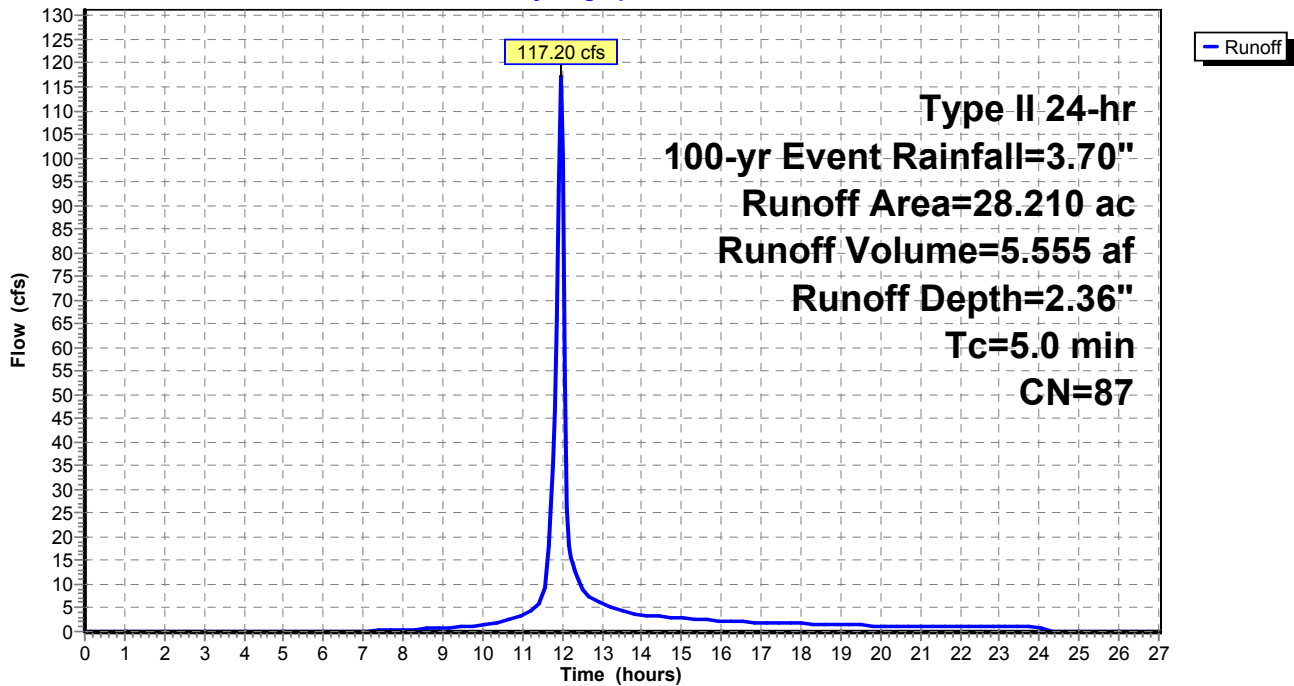
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 2: WS 2**

Runoff = 363.66 cfs @ 12.03 hrs, Volume= 20.985 af, Depth= 2.36"

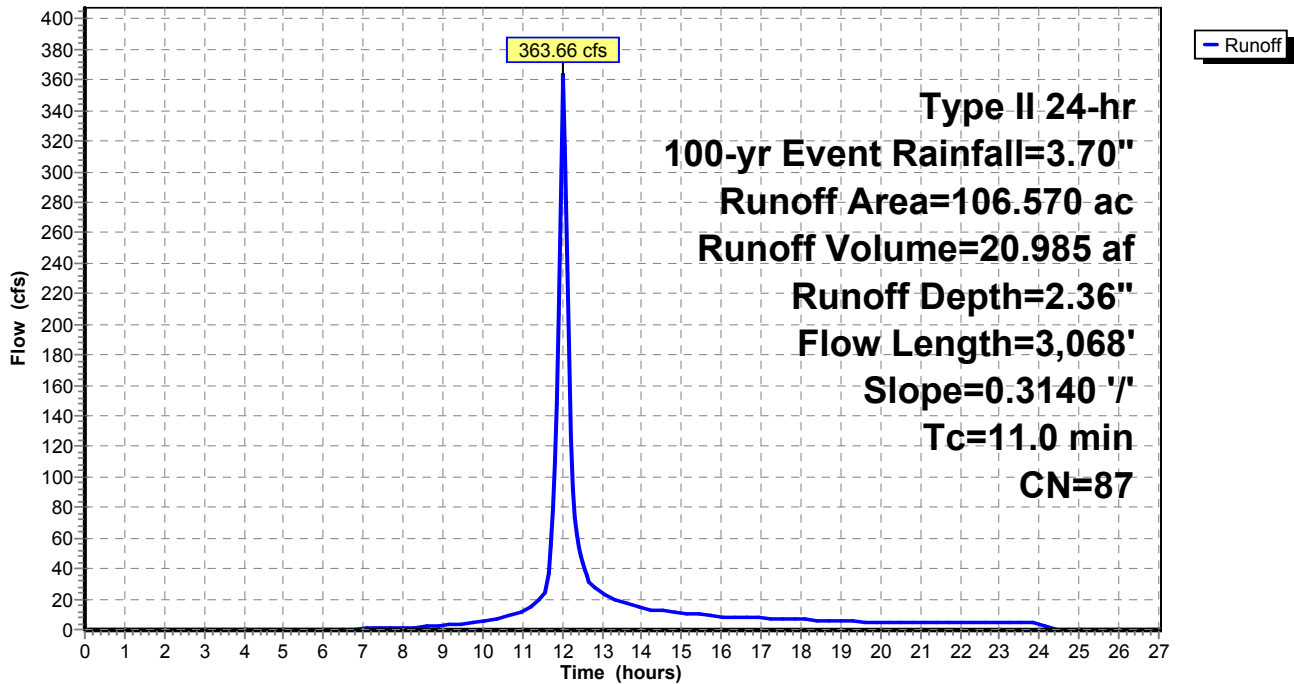
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 3: WS 3**

Runoff = 129.54 cfs @ 12.00 hrs, Volume= 6.890 af, Depth= 2.36"

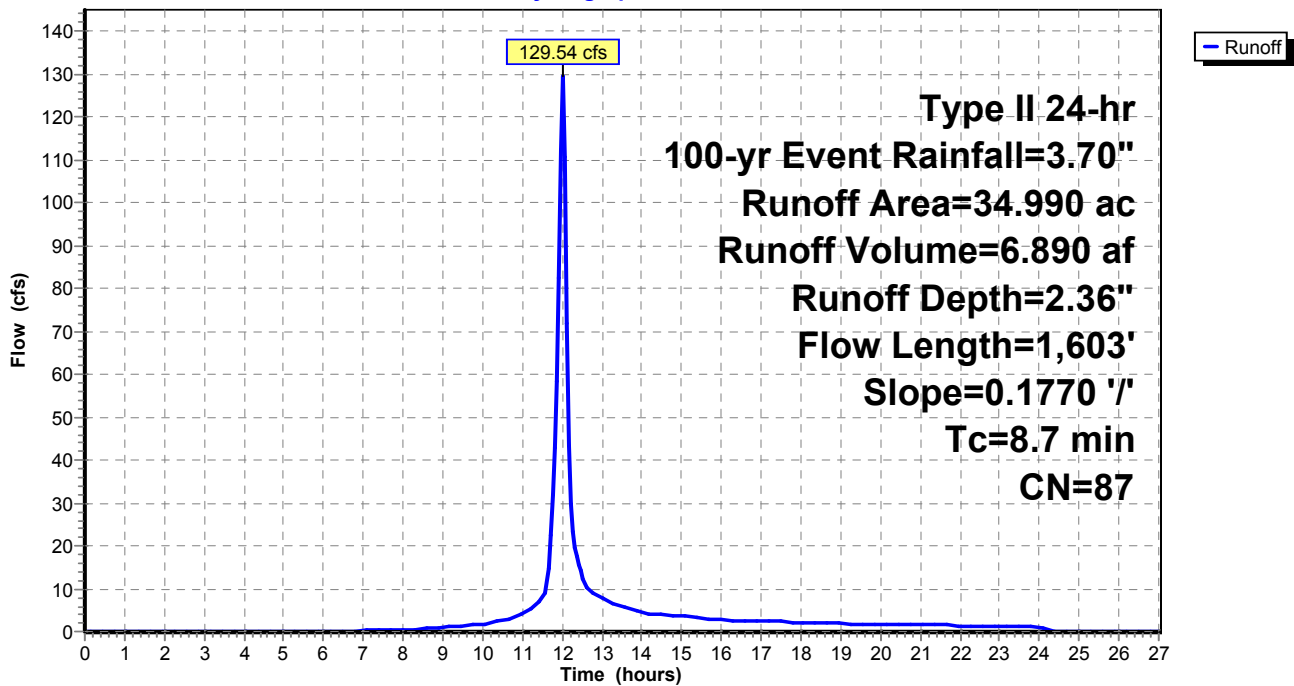
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 4: WS 4**

Runoff = 259.92 cfs @ 12.02 hrs, Volume= 14.773 af, Depth= 2.36"

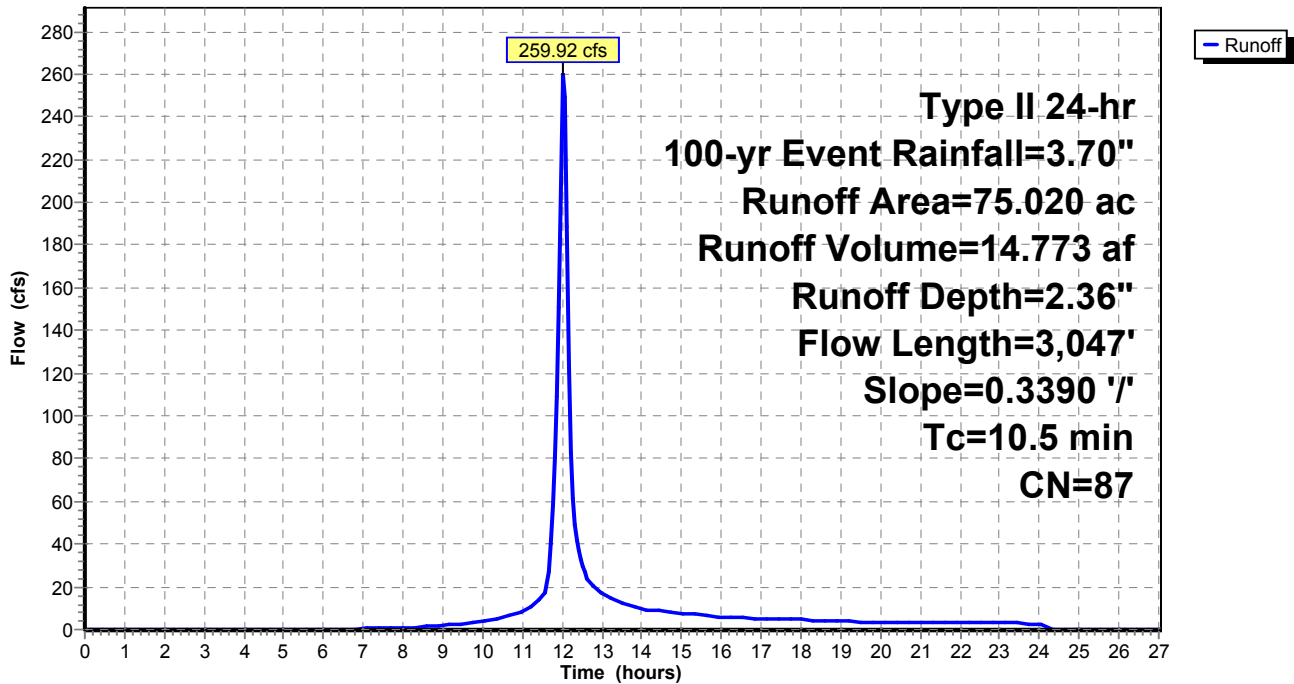
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 5: WS 5**

Runoff = 342.49 cfs @ 12.10 hrs, Volume= 24.455 af, Depth= 2.36"

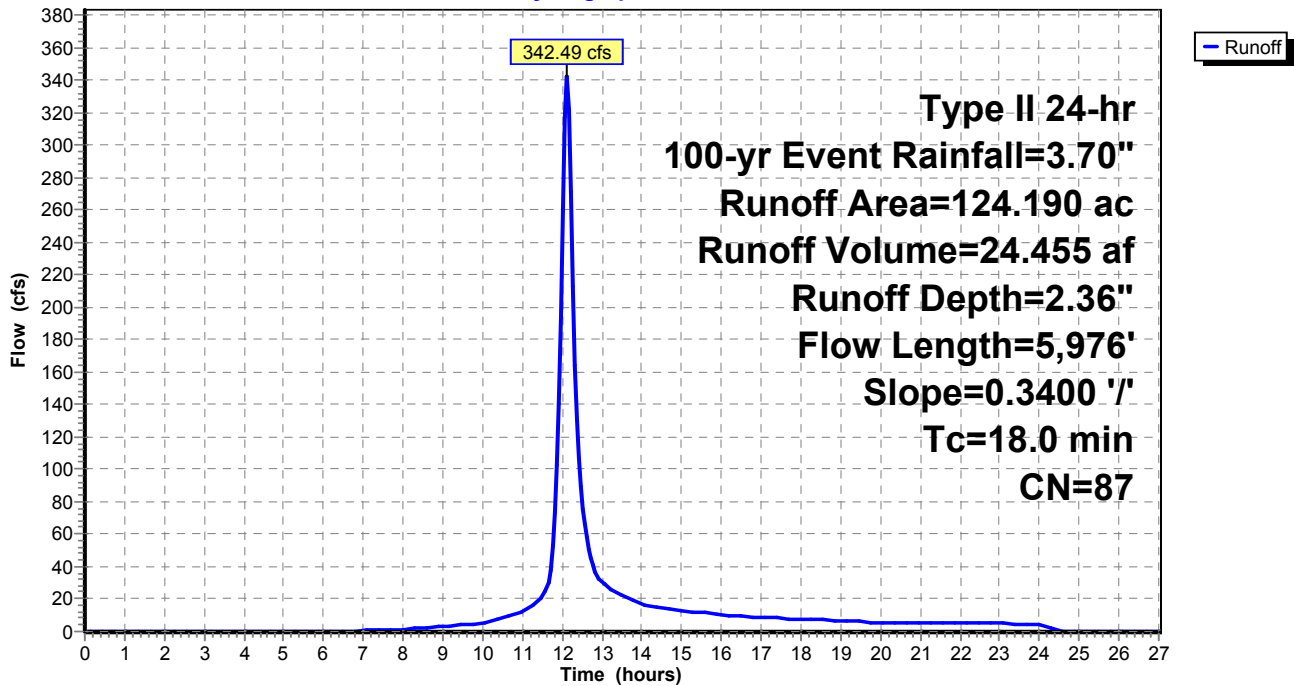
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 6: WS 6**

Runoff = 774.13 cfs @ 12.17 hrs, Volume= 65.223 af, Depth= 2.36"

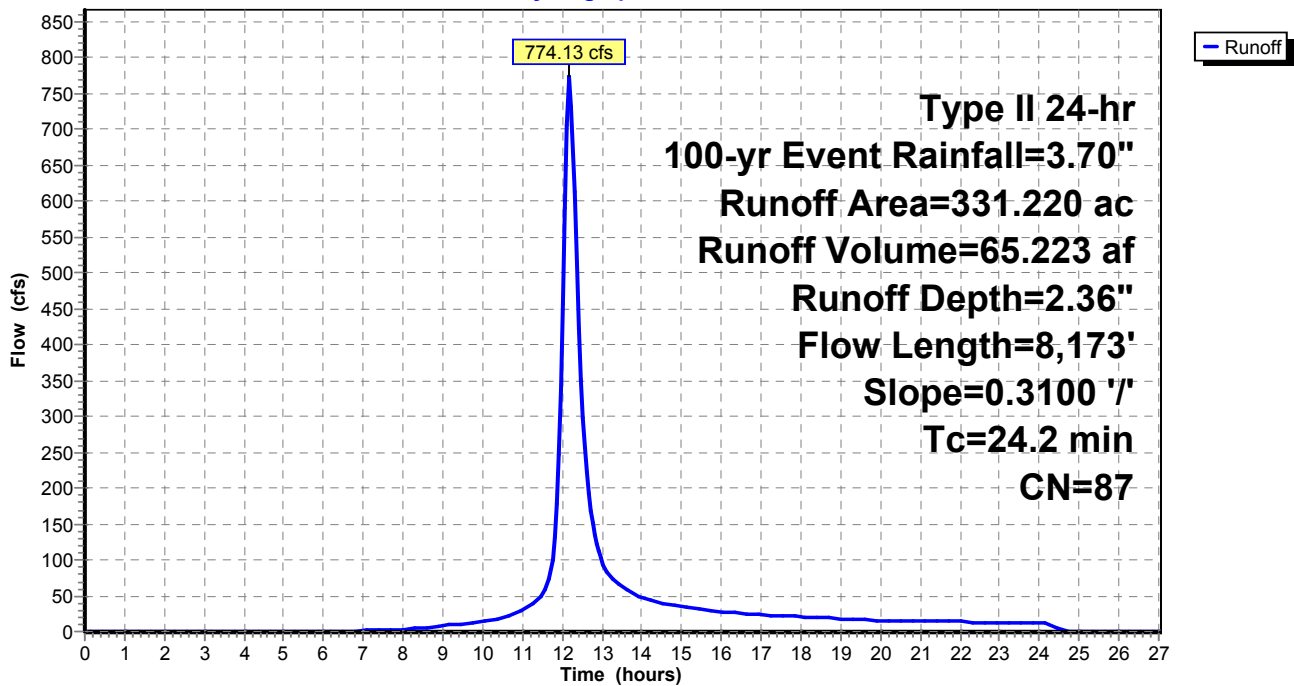
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 7: WS 7**

Runoff = 411.06 cfs @ 12.09 hrs, Volume= 28.449 af, Depth= 2.36"

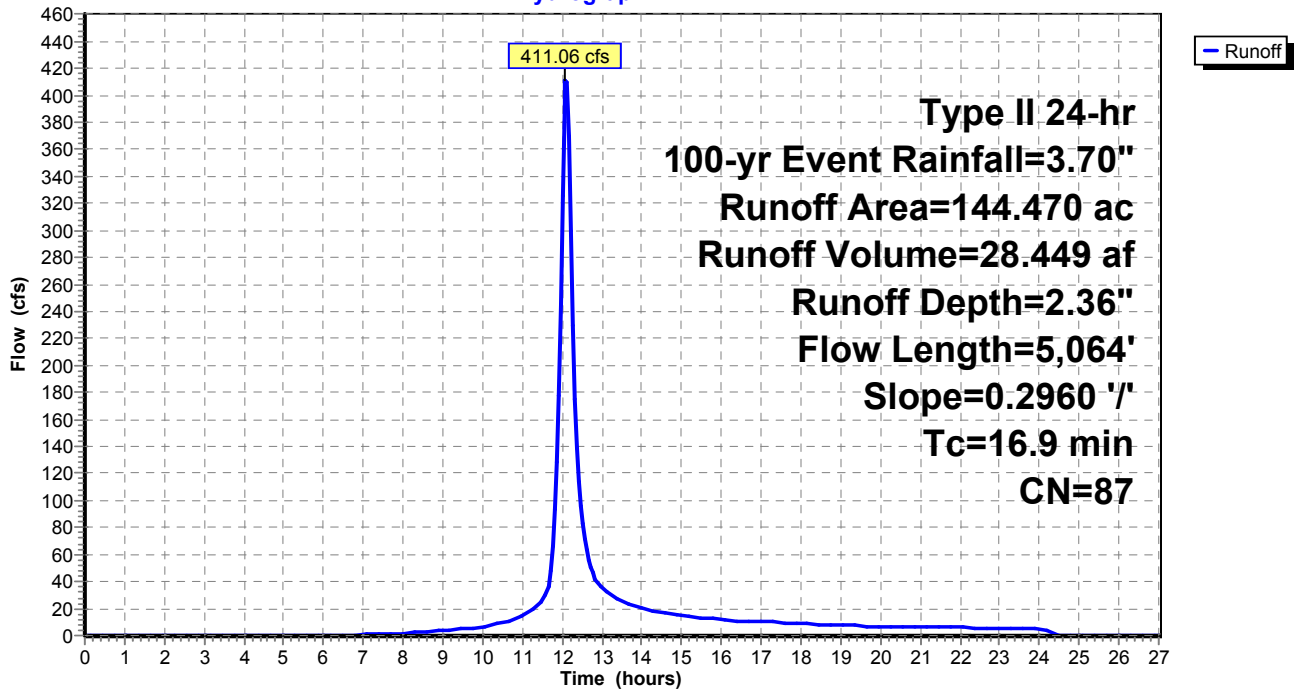
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph





**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 8: WS 8**

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

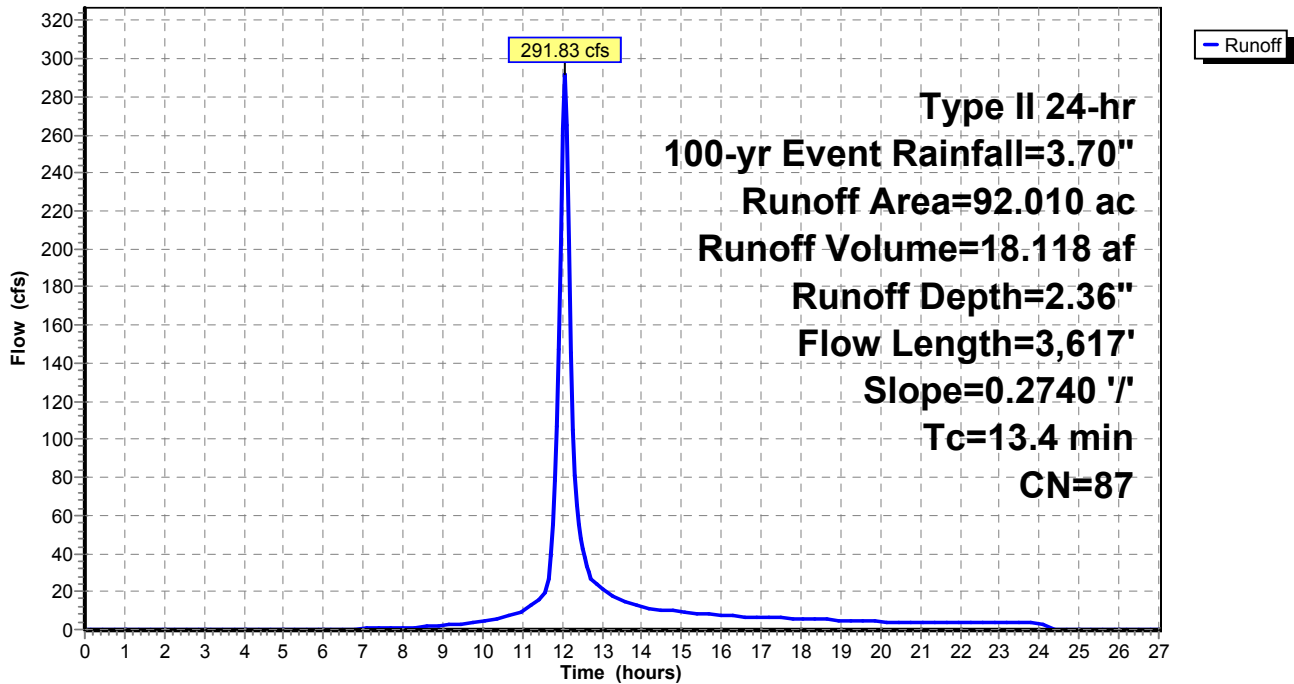
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 9: WS 9**

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

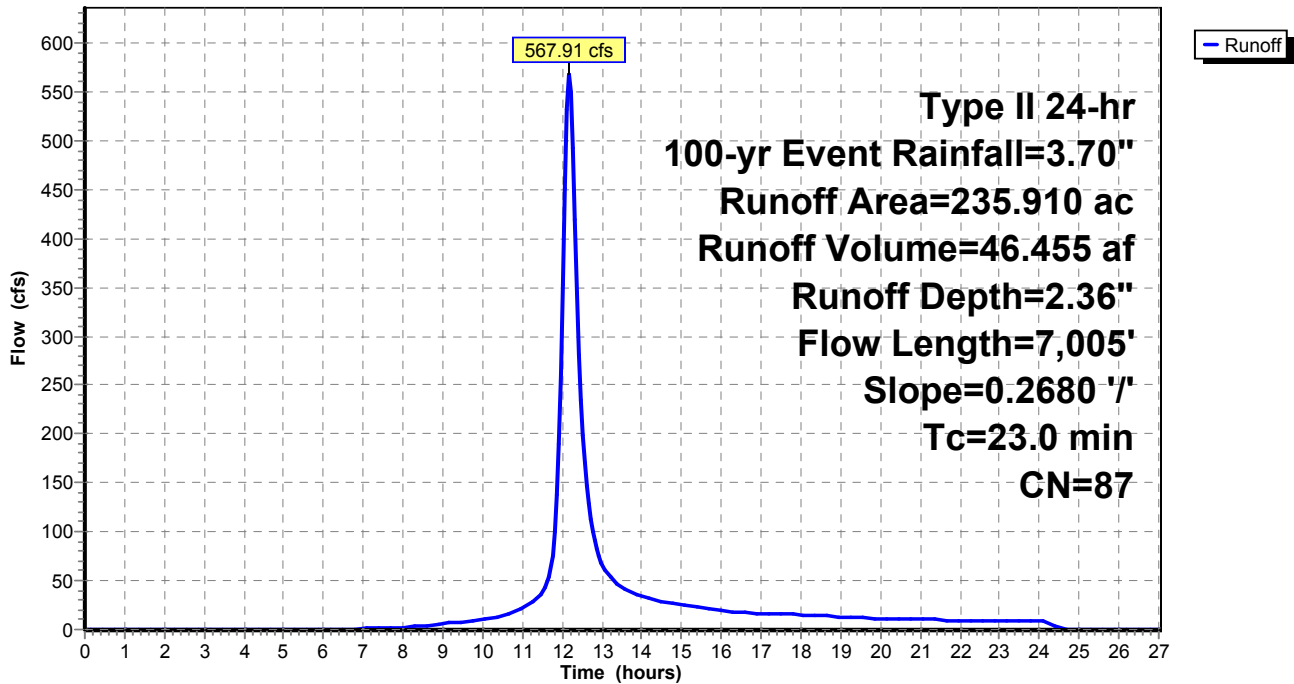
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 10: WS 10**

Runoff = 663.73 cfs @ 12.25 hrs, Volume= 65.063 af, Depth= 2.36"

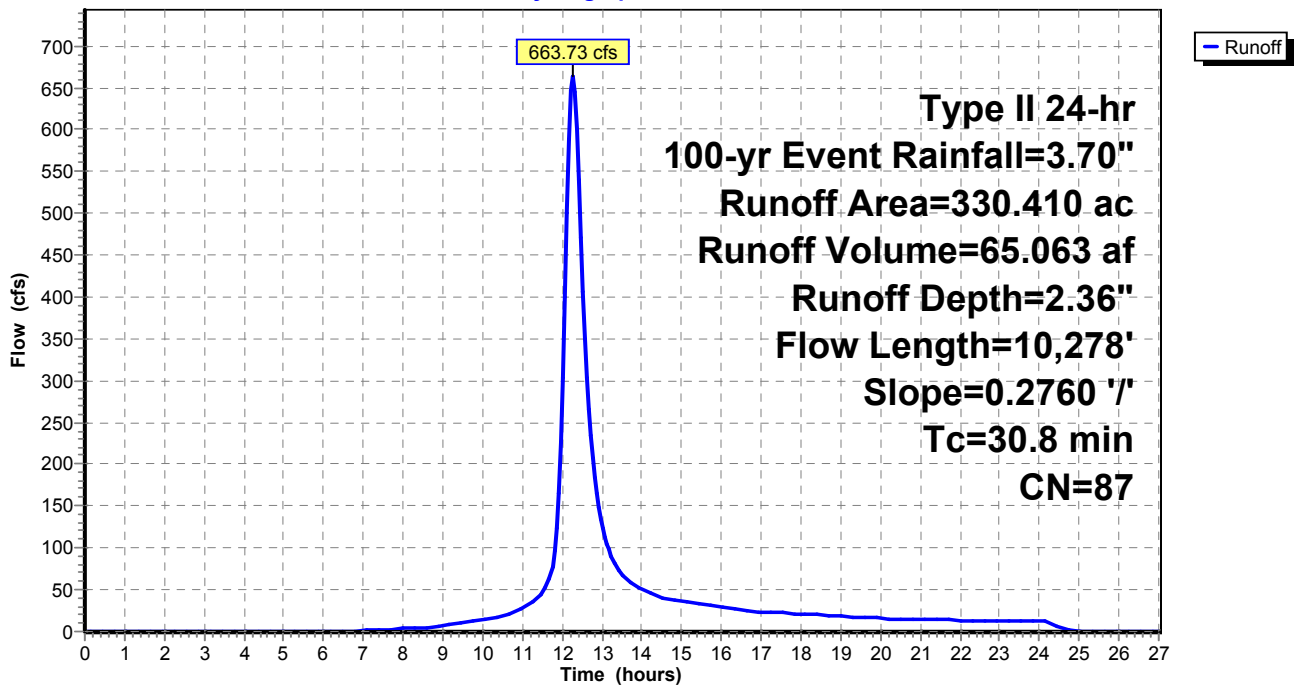
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 11: WS 11**

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

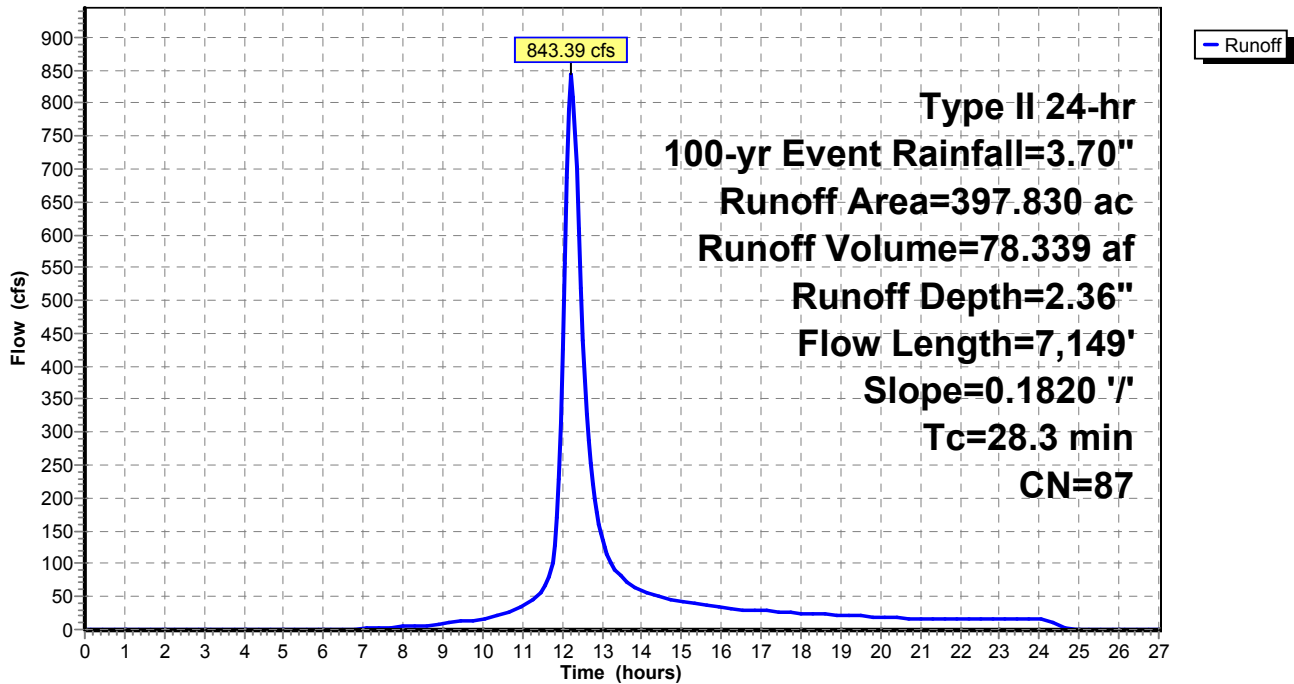
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 12: WS 12**

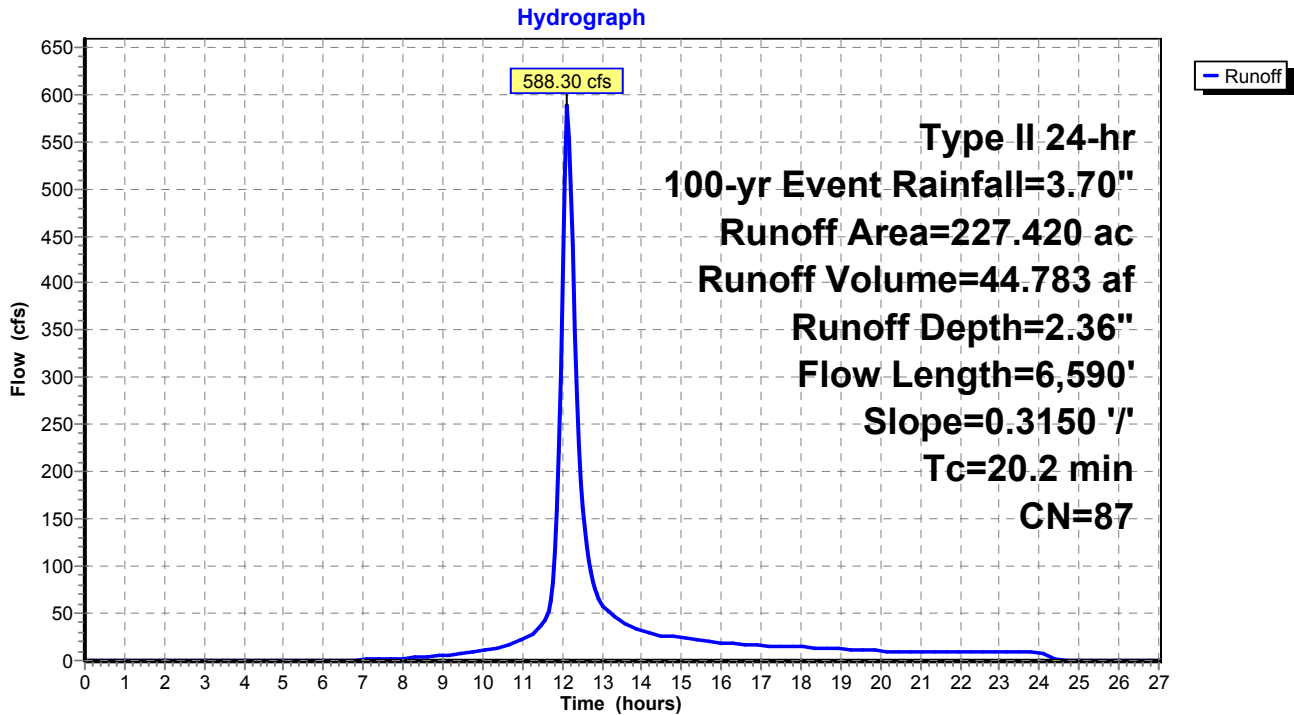
Runoff = 588.30 cfs @ 12.13 hrs, Volume= 44.783 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 13: WS 13**

Runoff = 649.87 cfs @ 12.17 hrs, Volume= 54.252 af, Depth= 2.36"

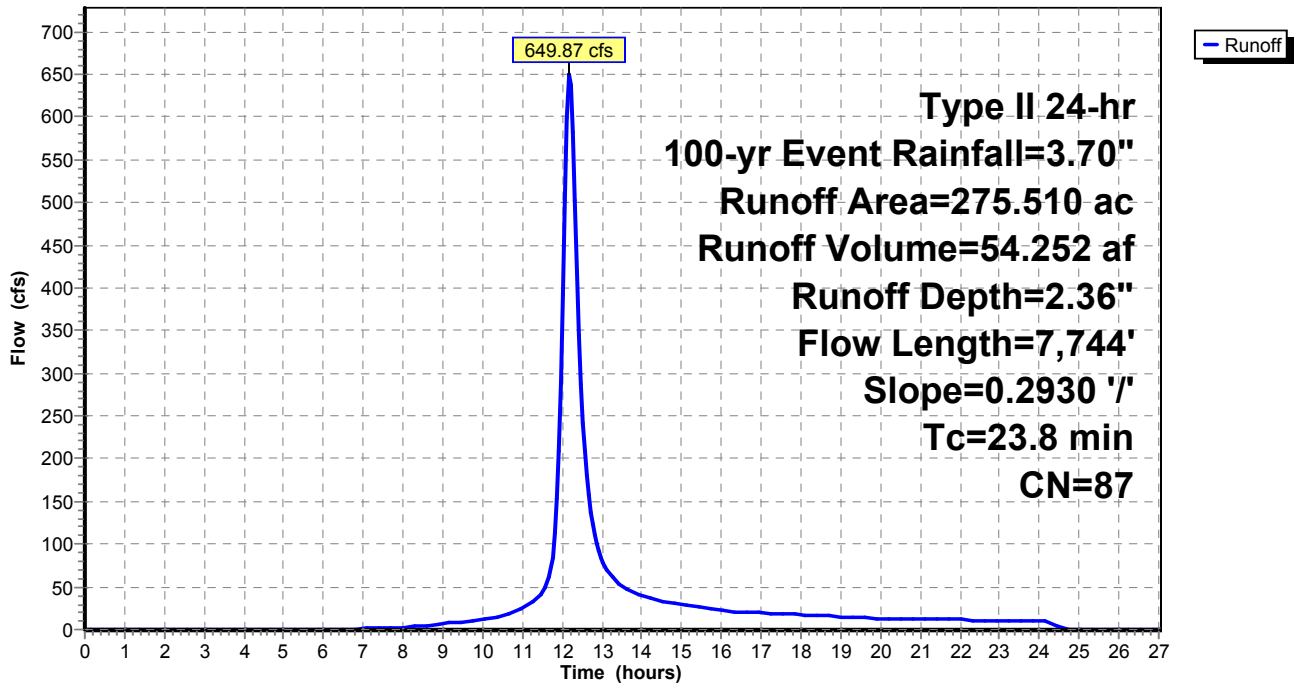
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 14: WS 14**

Runoff = 243.90 cfs @ 12.06 hrs, Volume= 15.747 af, Depth= 2.36"

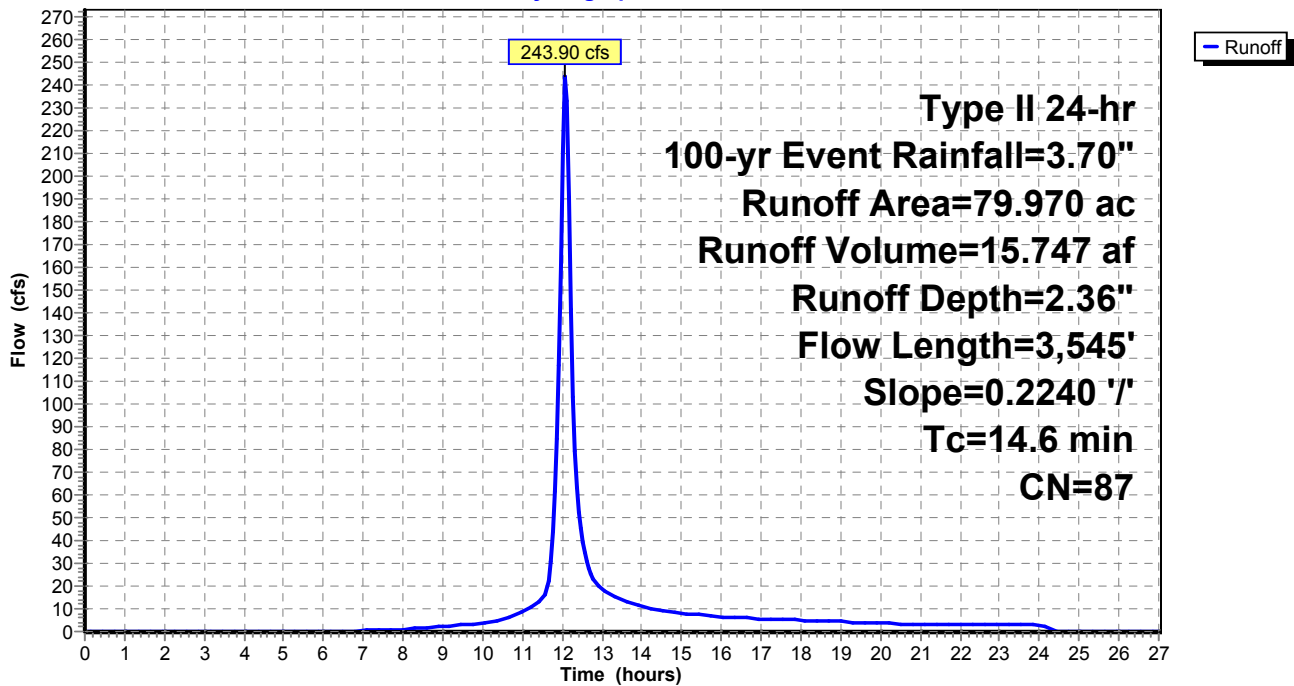
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



**Existing Watersheds (Post-Quintana)***Type II 24-hr 200-yr Event Rainfall=4.09"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment1: WS 1** Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.72"  
 Tc=5.0 min CN=87 Runoff=133.97 cfs 6.393 af

**Subcatchment2: WS 2** Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=415.35 cfs 24.150 af

**Subcatchment3: WS 3** Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=148.20 cfs 7.929 af

**Subcatchment4: WS 4** Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=297.56 cfs 17.000 af

**Subcatchment5: WS 5** Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=392.70 cfs 28.143 af

**Subcatchment6: WS 6** Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=888.50 cfs 75.058 af

**Subcatchment7: WS 7** Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=471.20 cfs 32.739 af

**Subcatchment8: WS 8** Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=334.29 cfs 20.851 af

**Subcatchment9: WS 9** Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=651.70 cfs 53.460 af

**Subcatchment10: WS 10** Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=762.30 cfs 74.875 af

**Subcatchment11: WS 11** Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=968.42 cfs 90.153 af

**Subcatchment12: WS 12** Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=674.80 cfs 51.536 af

**Subcatchment13: WS 13** Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=745.85 cfs 62.434 af

**Subcatchment14: WS 14** Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=2.72"  
 Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=279.49 cfs 18.122 af

**Total Runoff Area = 2,483.730 ac Runoff Volume = 562.843 af Average Runoff Depth = 2.72"**  
**90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac**



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

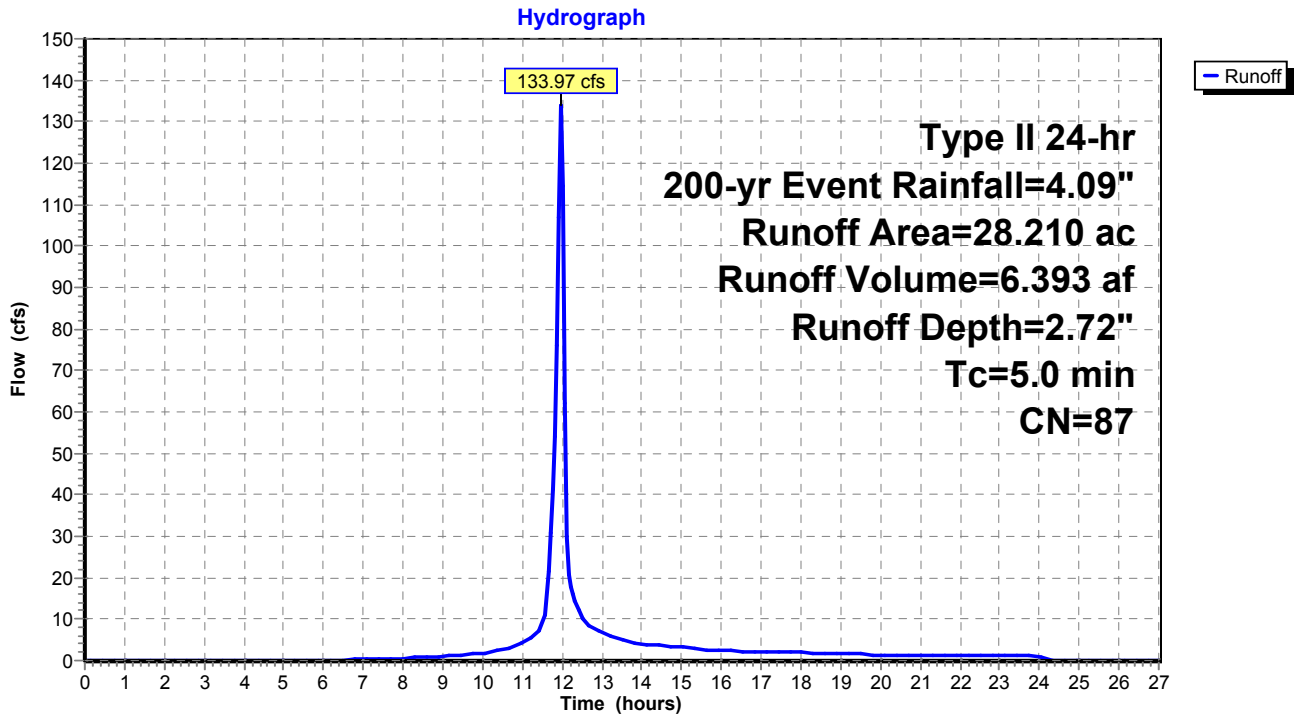
Runoff = 133.97 cfs @ 11.95 hrs, Volume= 6.393 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 2: WS 2**

Runoff = 415.35 cfs @ 12.02 hrs, Volume= 24.150 af, Depth= 2.72"

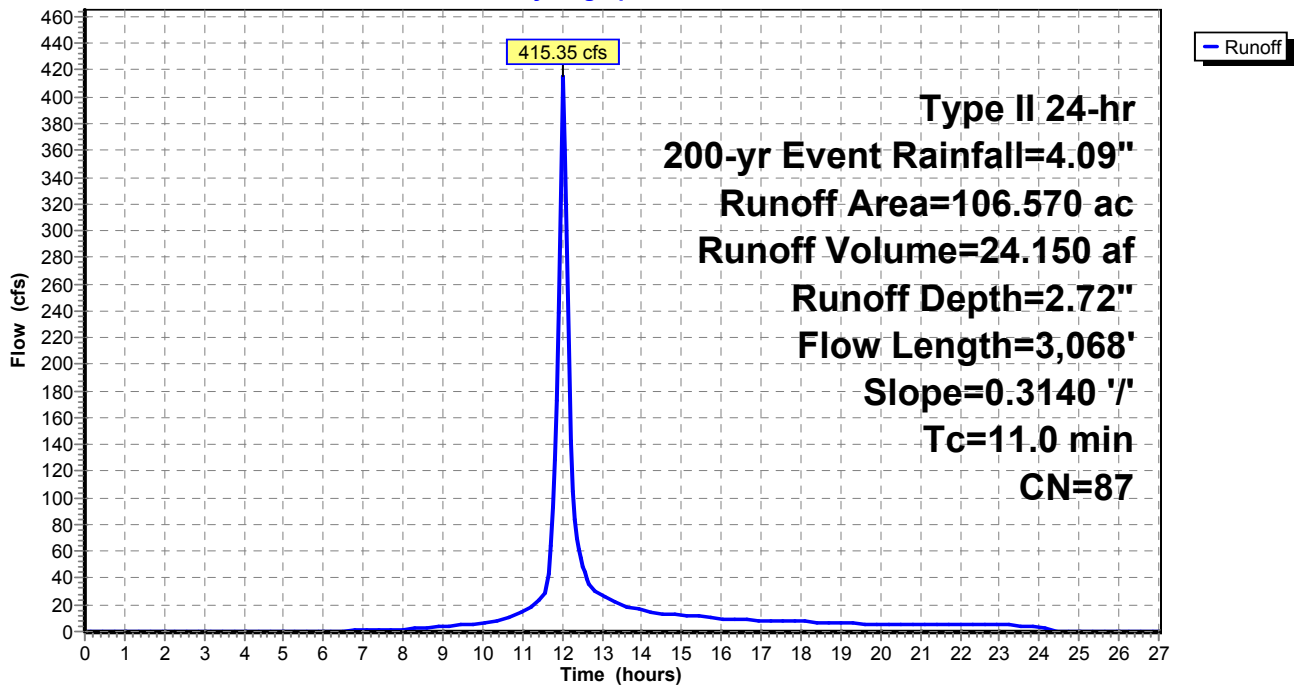
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 3: WS 3**

Runoff = 148.20 cfs @ 12.00 hrs, Volume= 7.929 af, Depth= 2.72"

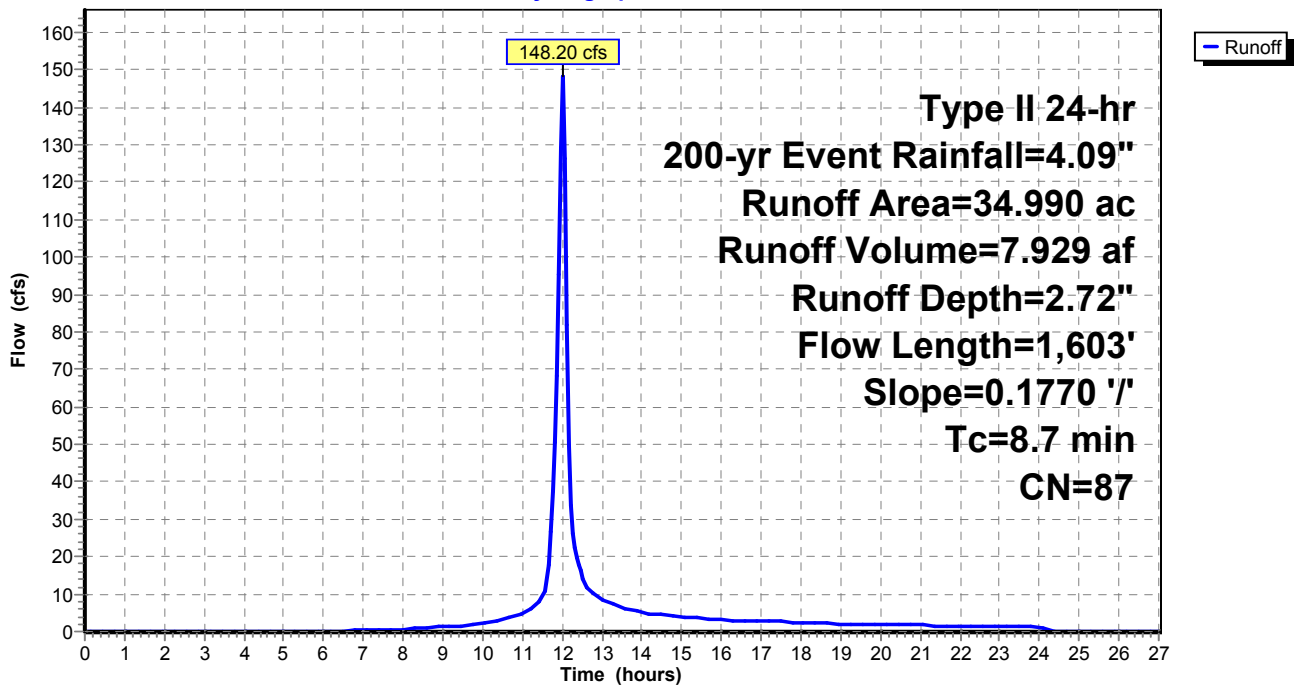
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 4: WS 4**

Runoff = 297.56 cfs @ 12.02 hrs, Volume= 17.000 af, Depth= 2.72"

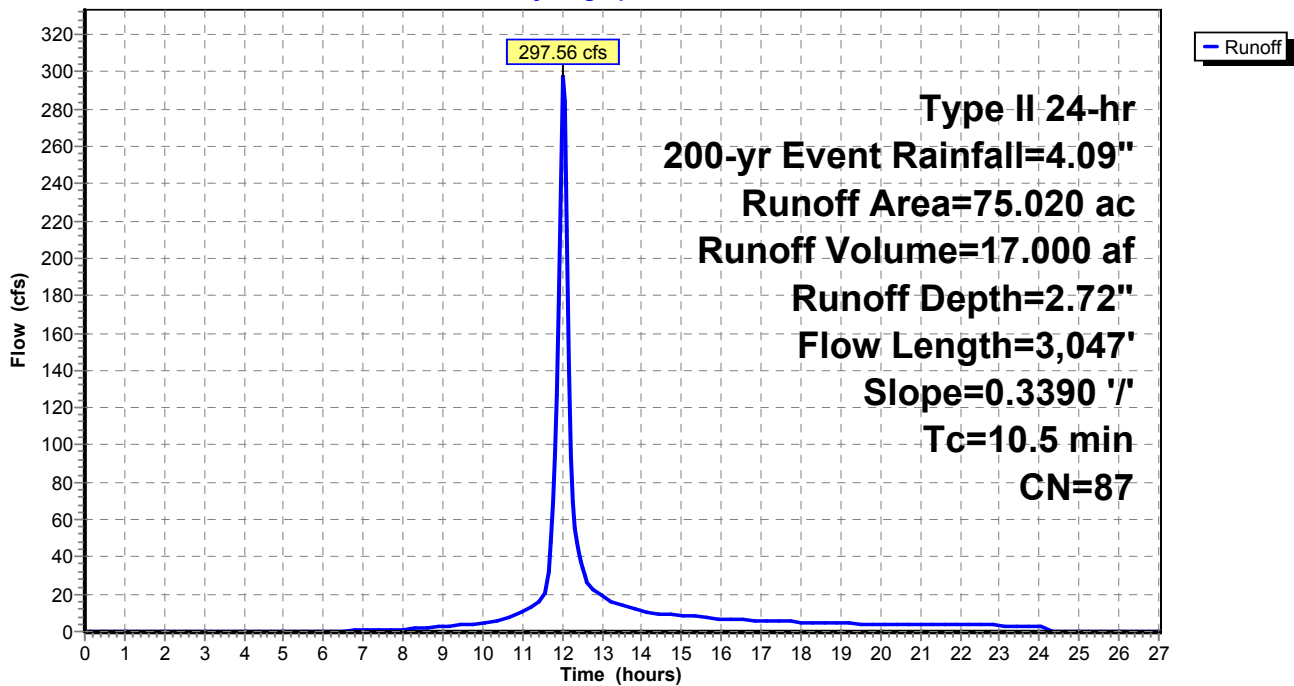
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 5: WS 5**

Runoff = 392.70 cfs @ 12.10 hrs, Volume= 28.143 af, Depth= 2.72"

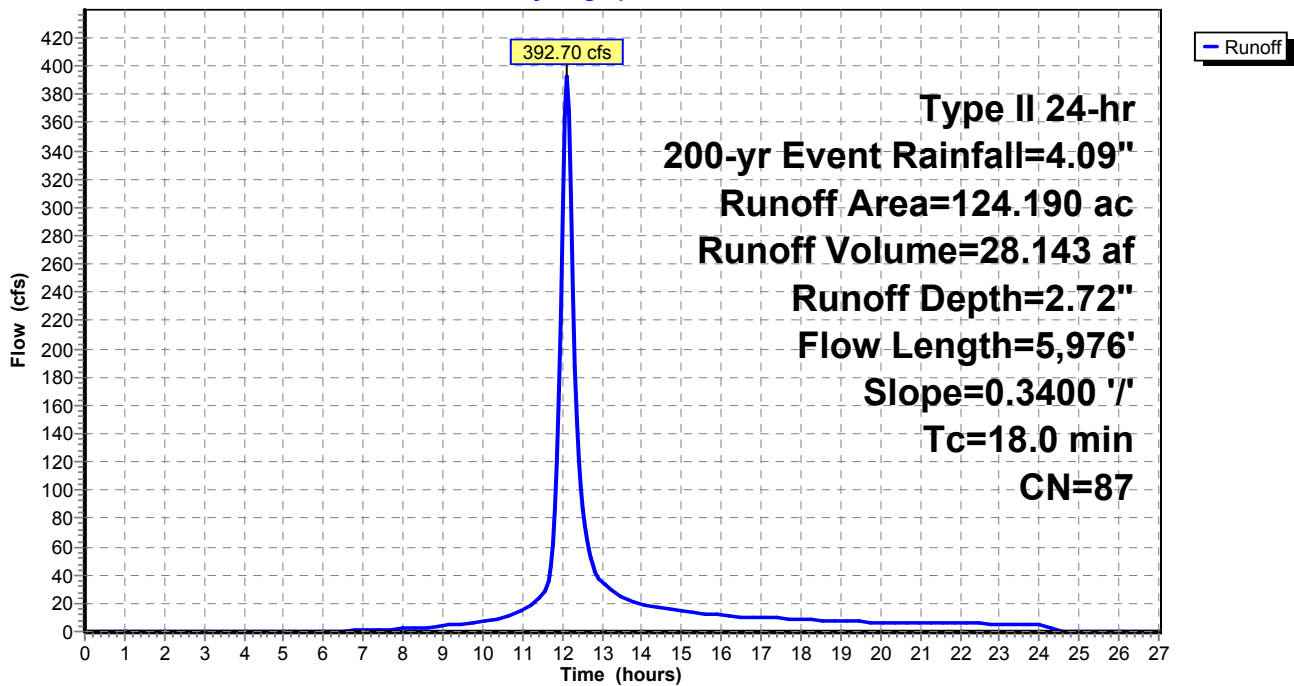
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 6: WS 6**

Runoff = 888.50 cfs @ 12.17 hrs, Volume= 75.058 af, Depth= 2.72"

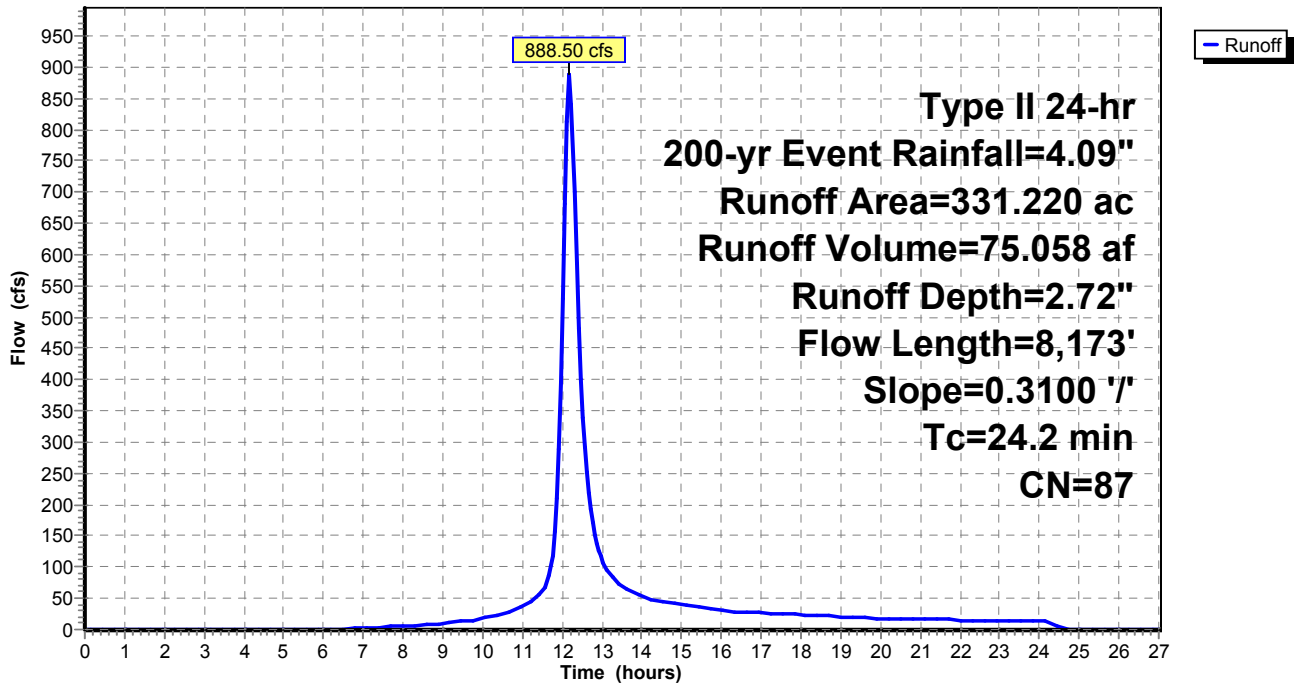
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 7: WS 7**

Runoff = 471.20 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

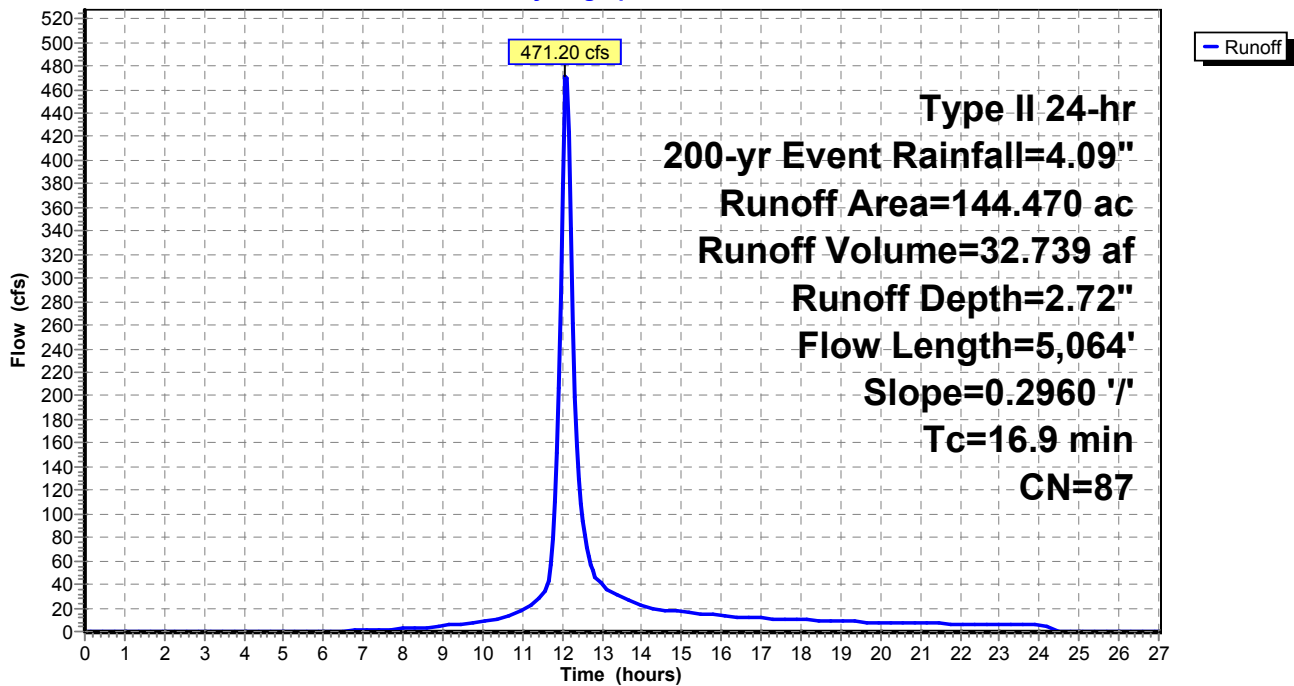
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 8: WS 8**

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

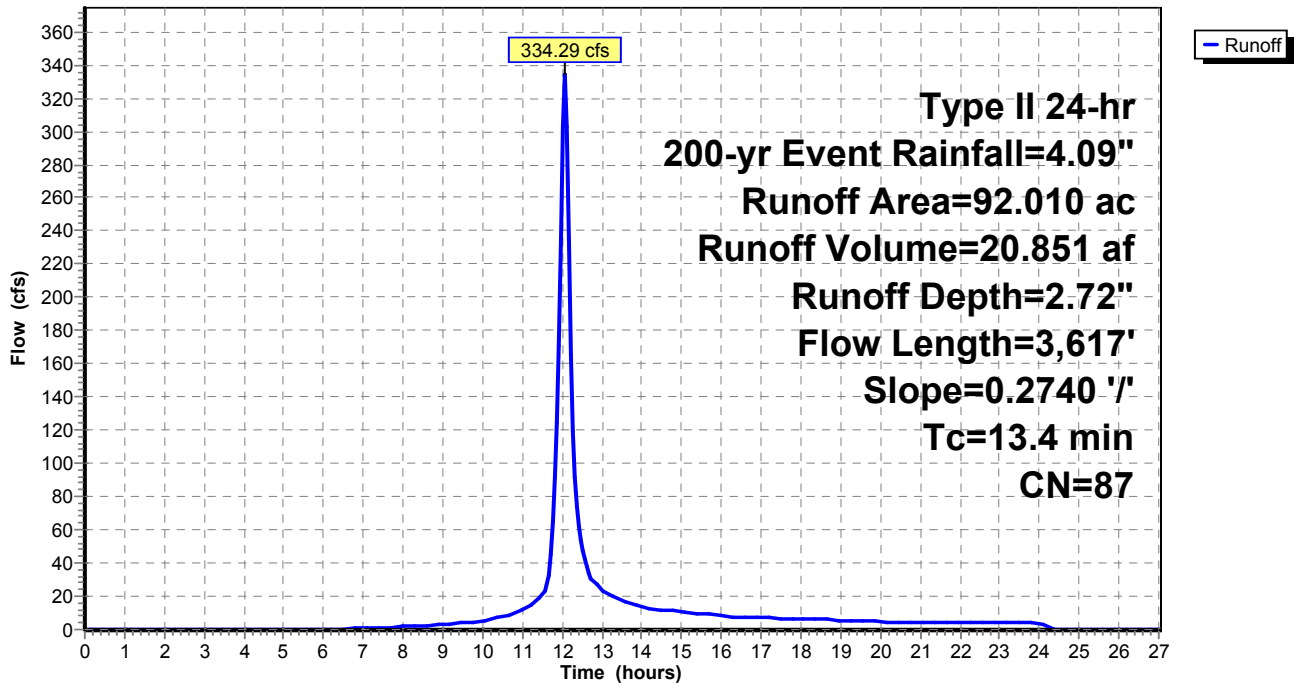
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph





**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 9: WS 9**

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

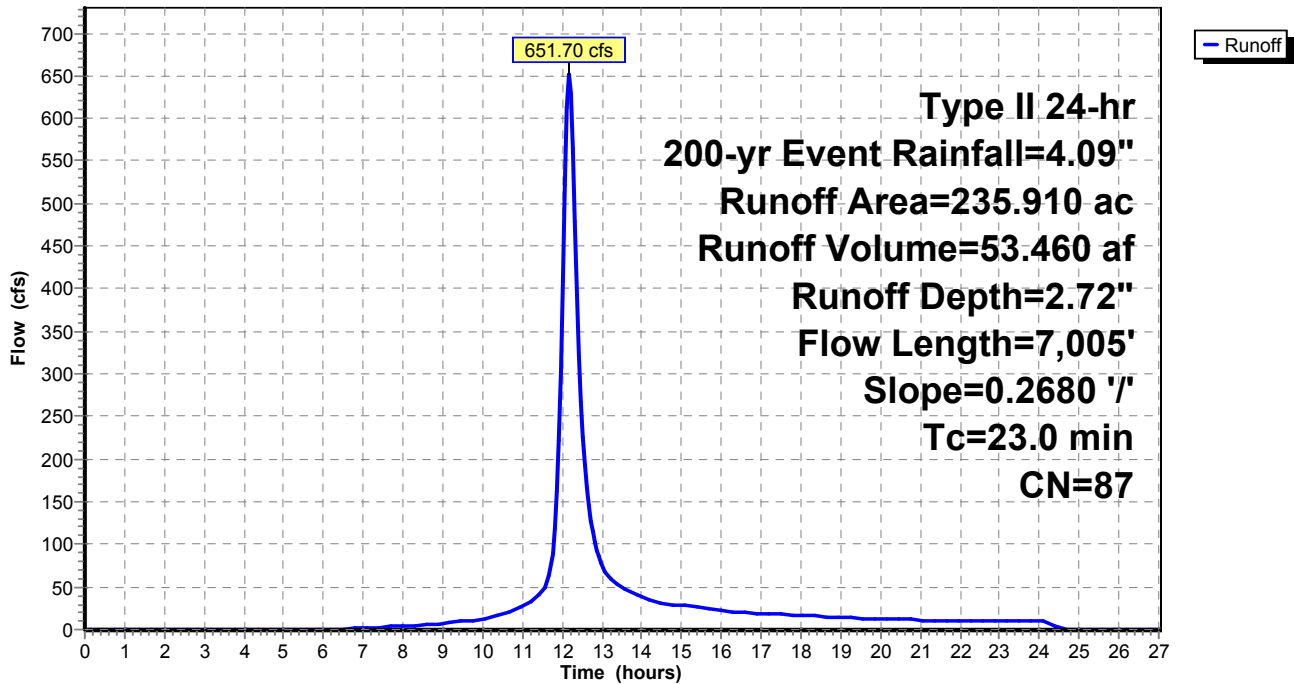
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 10: WS 10**

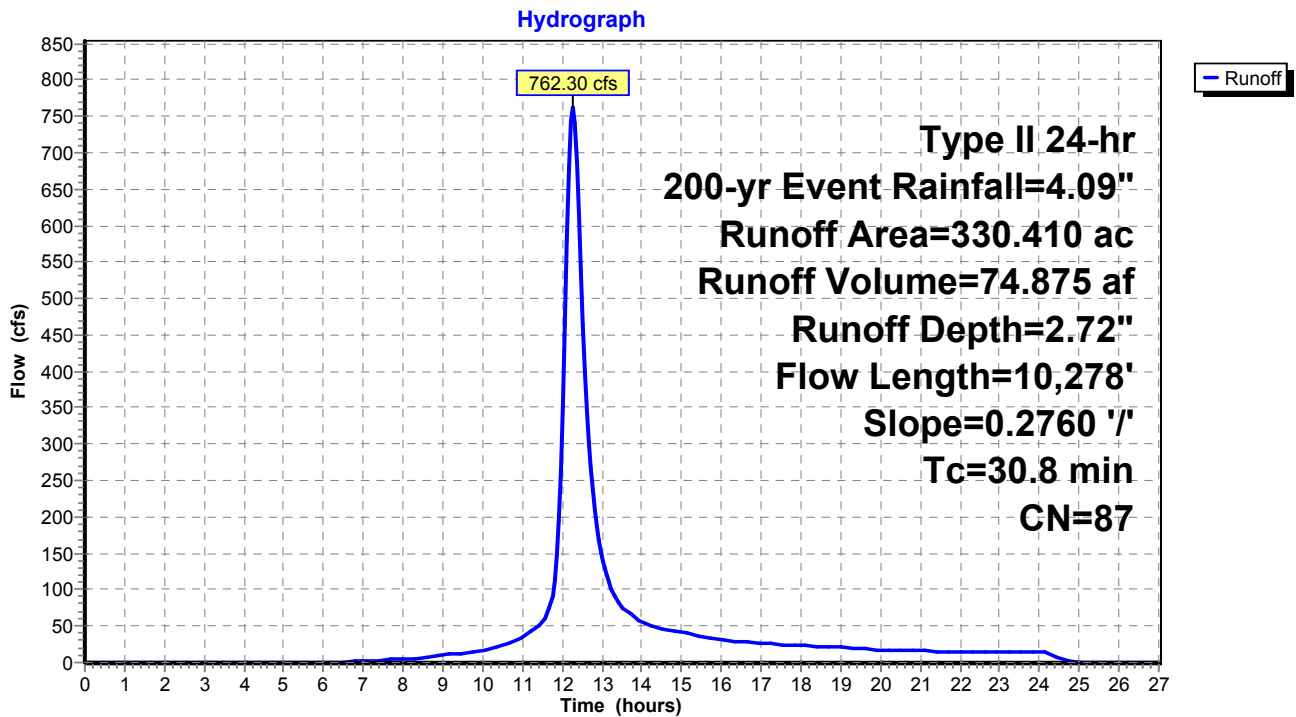
Runoff = 762.30 cfs @ 12.25 hrs, Volume= 74.875 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 11: WS 11**

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

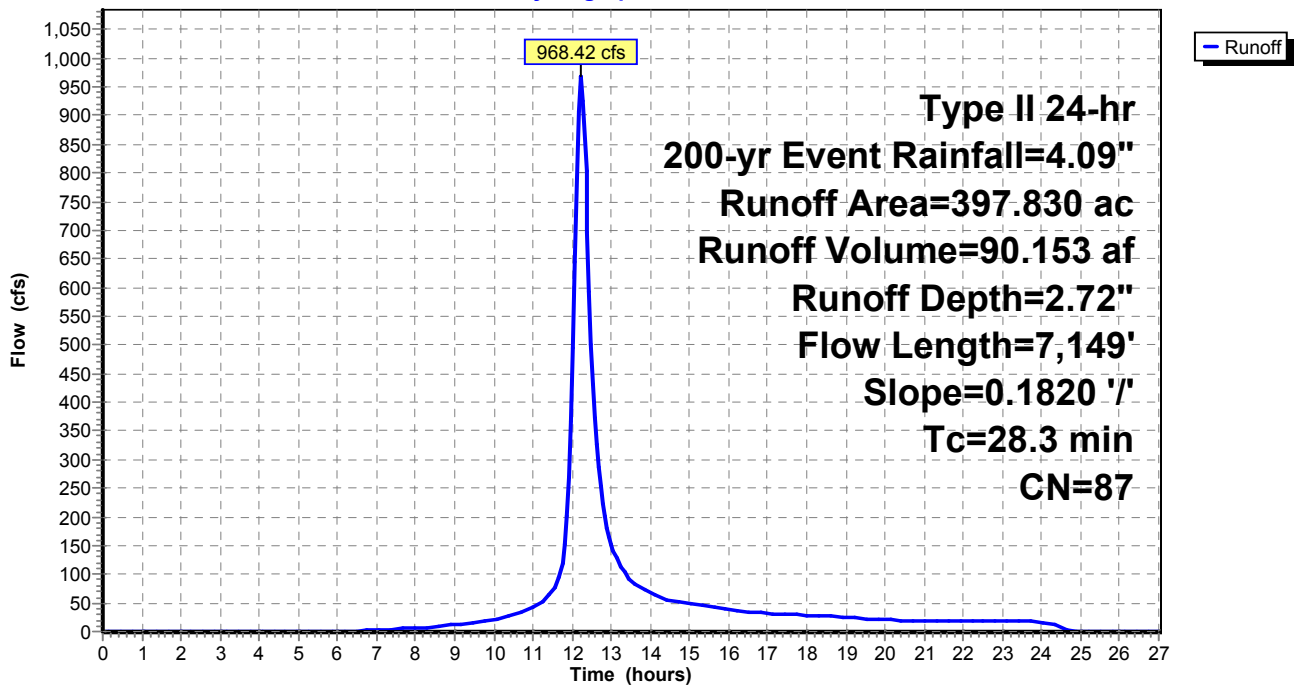
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 12: WS 12**

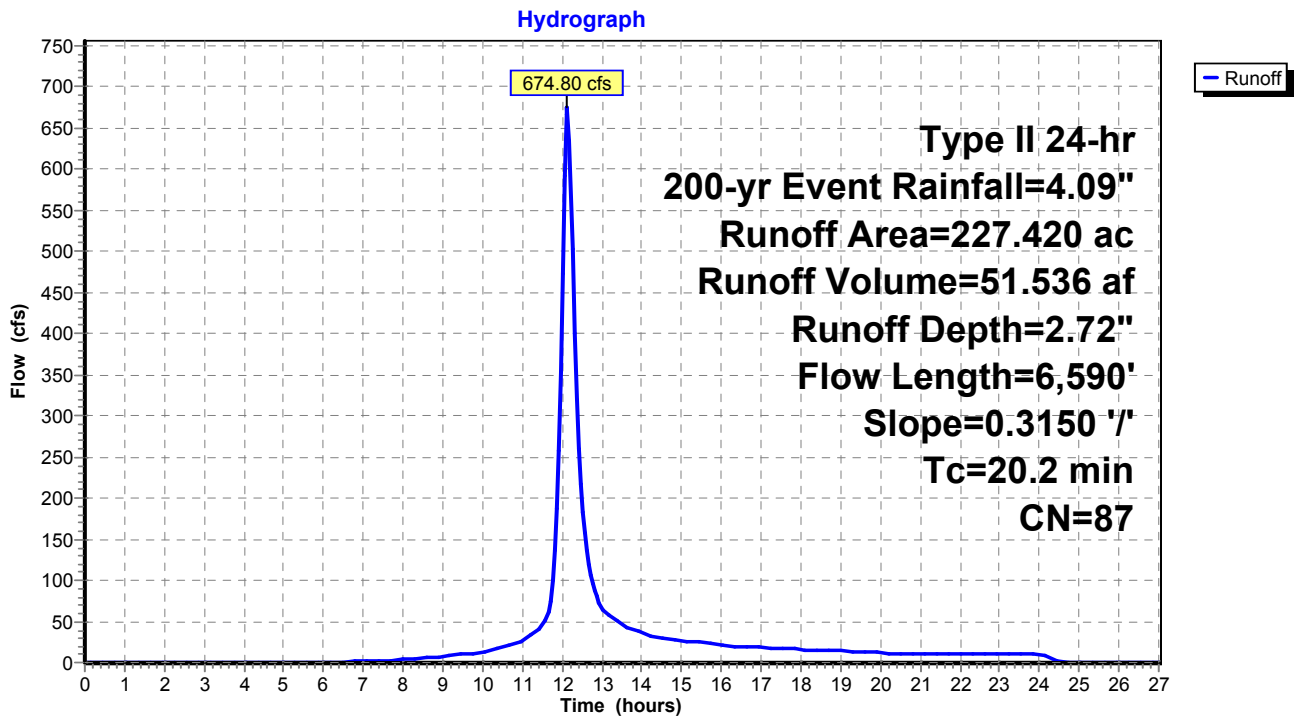
Runoff = 674.80 cfs @ 12.13 hrs, Volume= 51.536 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 13: WS 13**

Runoff = 745.85 cfs @ 12.17 hrs, Volume= 62.434 af, Depth= 2.72"

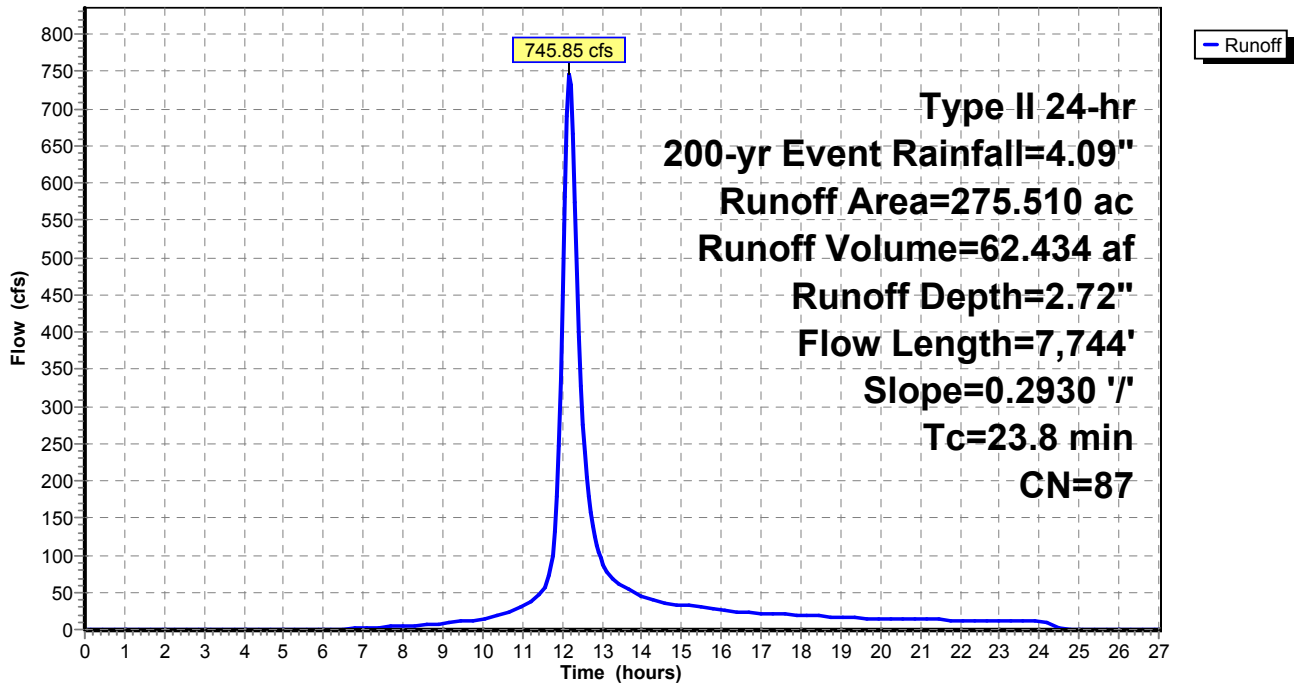
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 14: WS 14**

Runoff = 279.49 cfs @ 12.06 hrs, Volume= 18.122 af, Depth= 2.72"

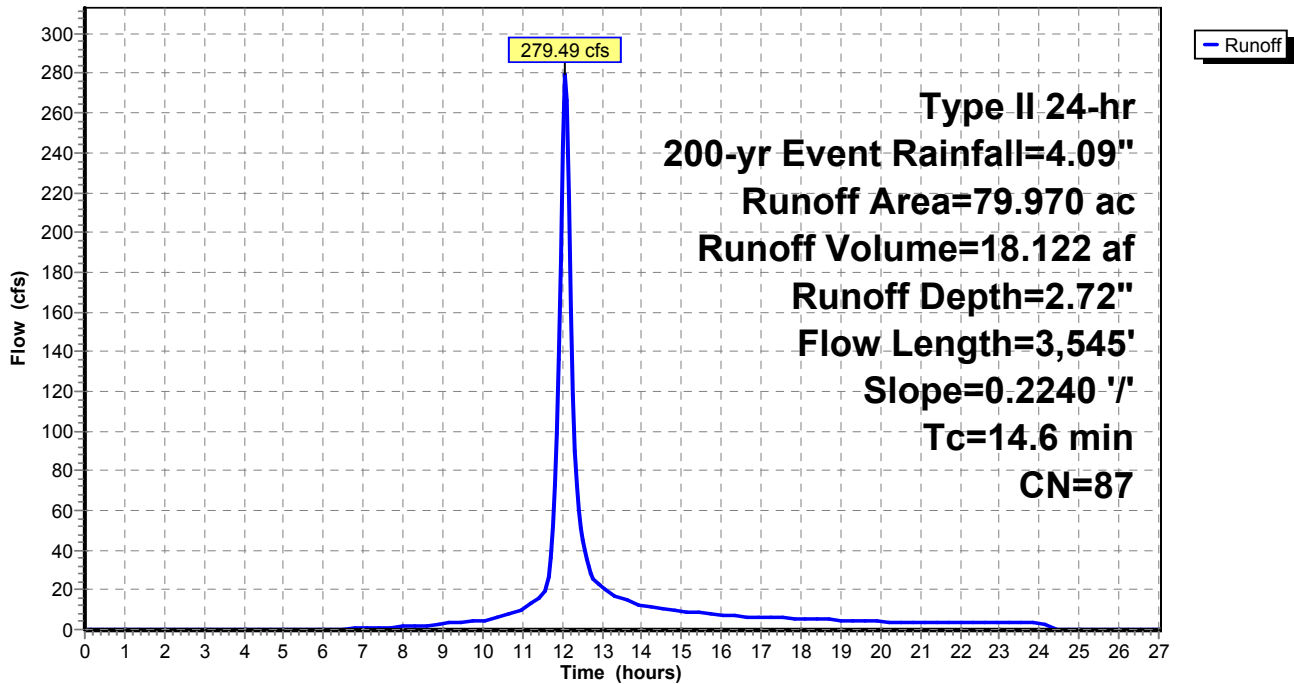
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



**Existing Watersheds (Post-Quintana)***Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment1: WS 1** Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=3.19"  
 Tc=5.0 min CN=87 Runoff=155.94 cfs 7.504 af

**Subcatchment2: WS 2** Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=484.31 cfs 28.349 af

**Subcatchment3: WS 3** Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=172.66 cfs 9.308 af

**Subcatchment4: WS 4** Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=346.92 cfs 19.956 af

**Subcatchment5: WS 5** Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=458.61 cfs 33.036 af

**Subcatchment6: WS 6** Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=1,038.74 cfs 88.109 af

**Subcatchment7: WS 7** Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=550.14 cfs 38.431 af

**Subcatchment8: WS 8** Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=389.99 cfs 24.476 af

**Subcatchment9: WS 9** Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=761.76 cfs 62.755 af

**Subcatchment10: WS 10** Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=891.85 cfs 87.894 af

**Subcatchment11: WS 11** Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=1,132.73 cfs 105.828 af

**Subcatchment12: WS 12** Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=789.72 cfs 60.497 af

**Subcatchment13: WS 13** Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=871.93 cfs 73.289 af

**Subcatchment14: WS 14** Runoff Area=79.970 ac 10.00% Impervious Runoff Depth=3.19"  
 Flow Length=3,545' Slope=0.2240 '/' Tc=14.6 min CN=87 Runoff=326.19 cfs 21.273 af

**Total Runoff Area = 2,483.730 ac Runoff Volume = 660.707 af Average Runoff Depth = 3.19"**  
**90.00% Pervious = 2,235.360 ac 10.00% Impervious = 248.370 ac**

**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 155.94 cfs @ 11.95 hrs, Volume= 7.504 af, Depth= 3.19"

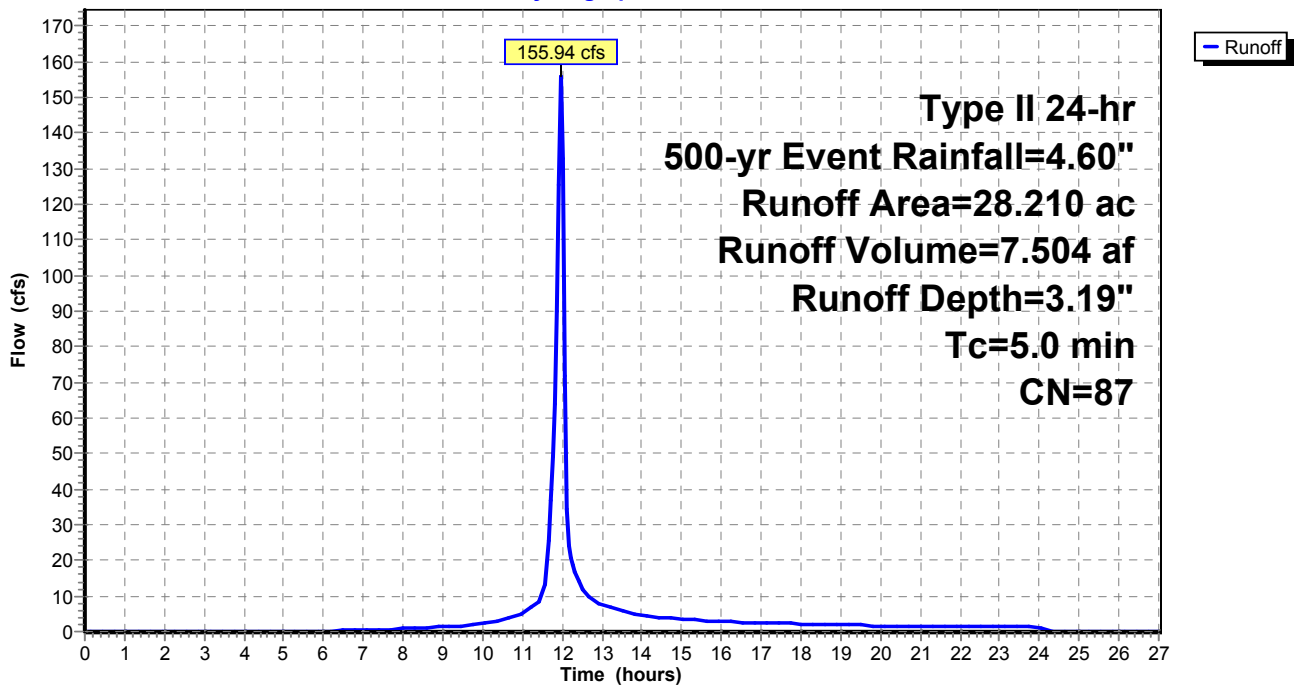
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**

Hydrograph





**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 2: WS 2**

Runoff = 484.31 cfs @ 12.02 hrs, Volume= 28.349 af, Depth= 3.19"

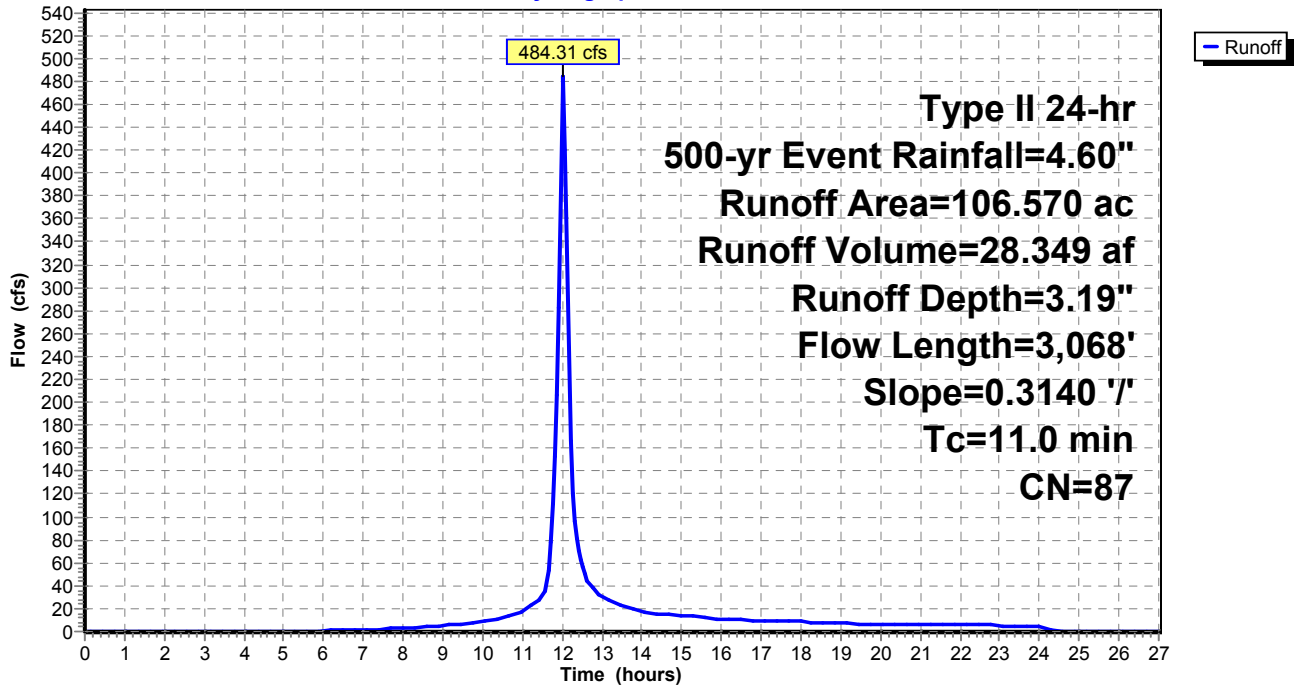
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 3: WS 3**

Runoff = 172.66 cfs @ 12.00 hrs, Volume= 9.308 af, Depth= 3.19"

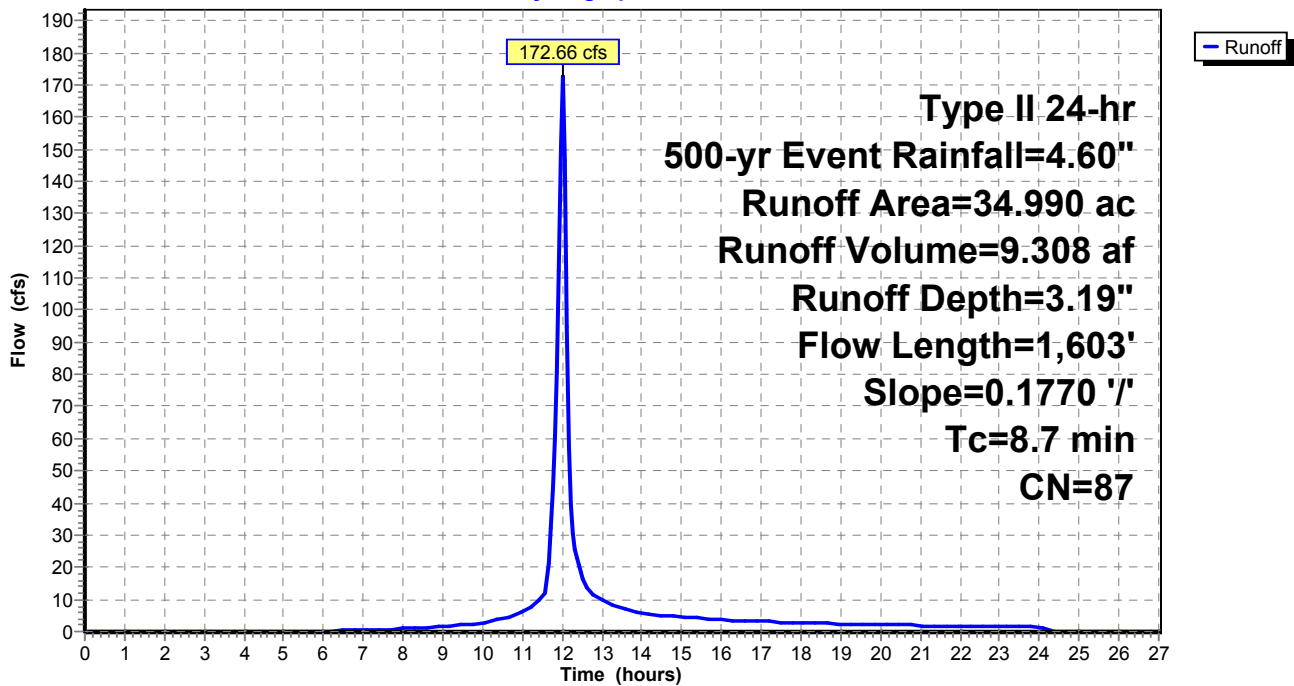
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 4: WS 4**

Runoff = 346.92 cfs @ 12.02 hrs, Volume= 19.956 af, Depth= 3.19"

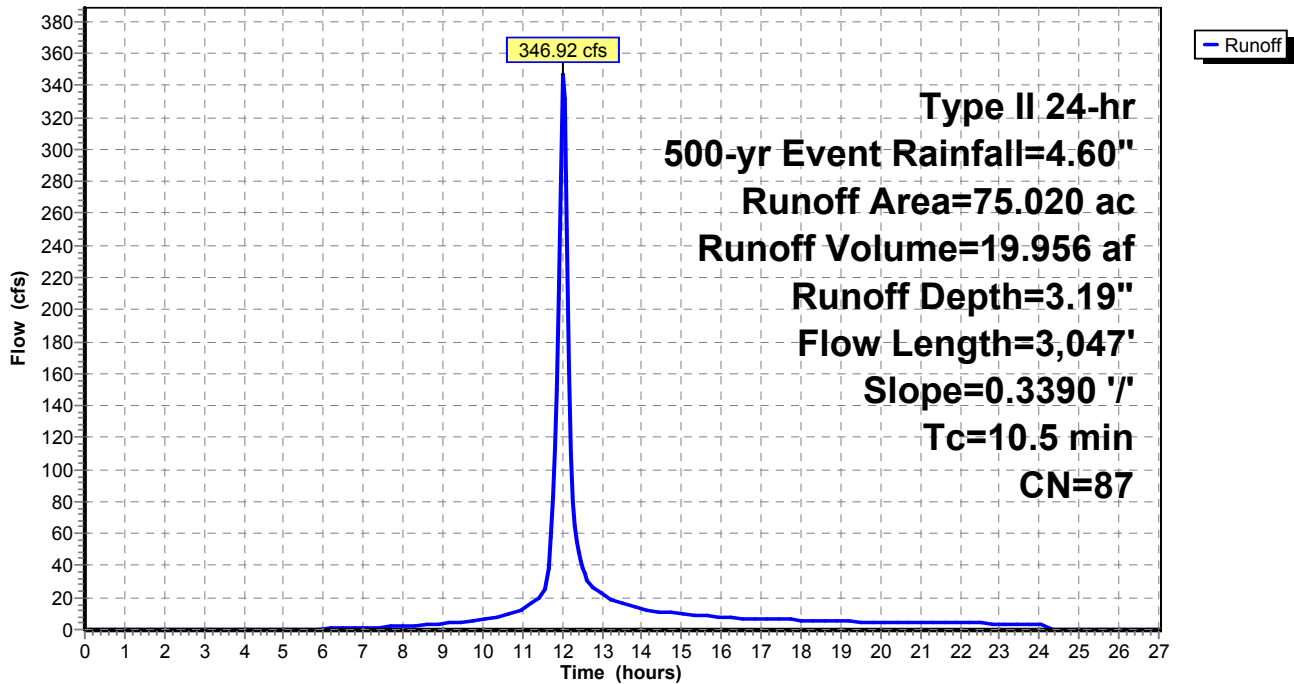
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 5: WS 5**

Runoff = 458.61 cfs @ 12.10 hrs, Volume= 33.036 af, Depth= 3.19"

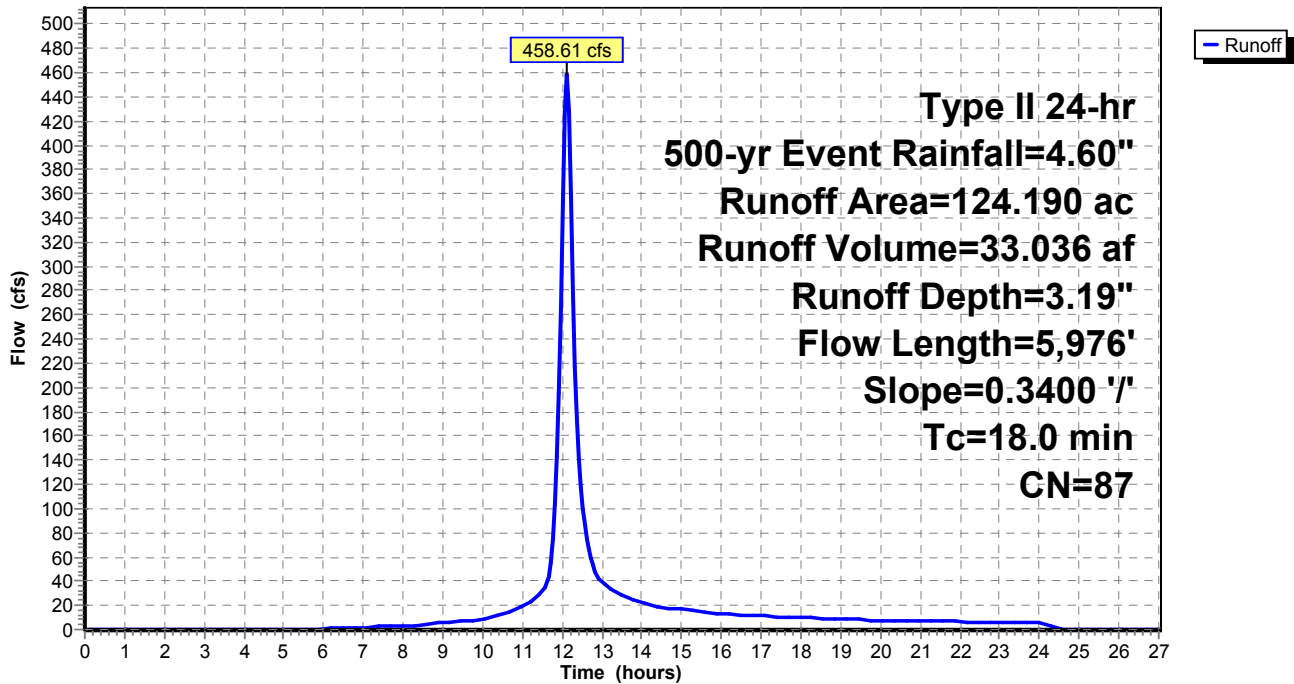
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 6: WS 6**

Runoff = 1,038.74 cfs @ 12.17 hrs, Volume= 88.109 af, Depth= 3.19"

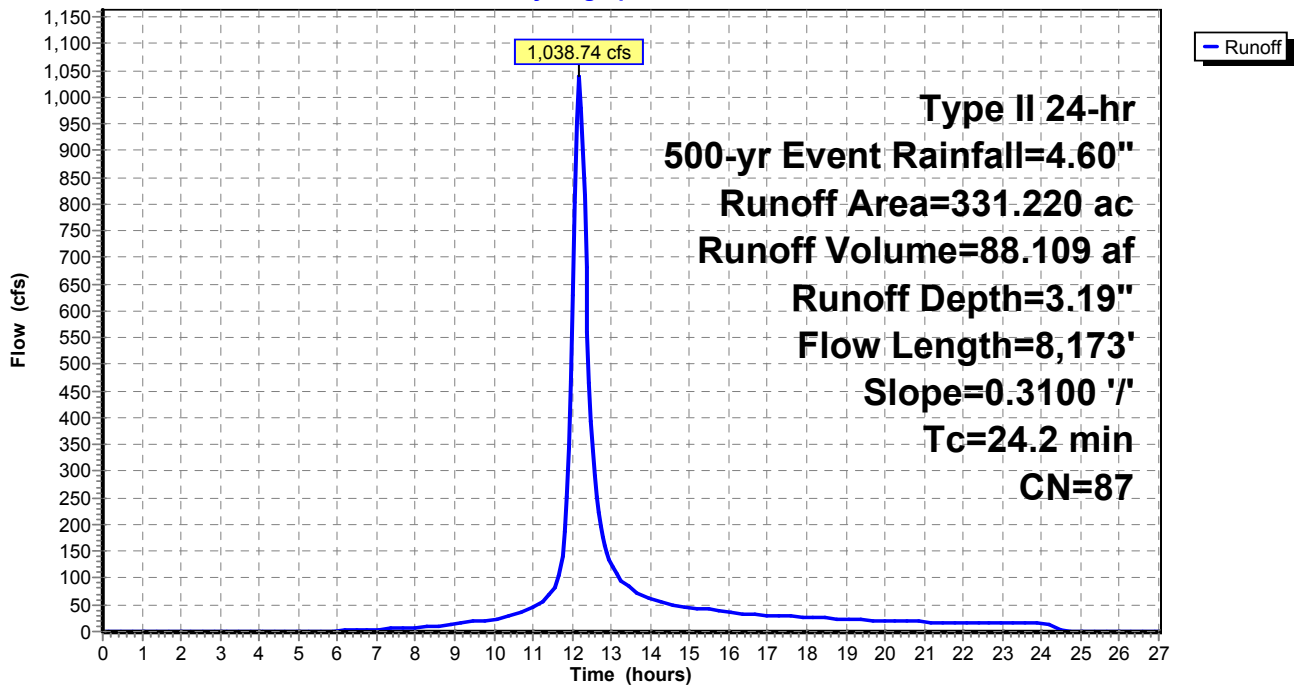
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 7: WS 7**

Runoff = 550.14 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

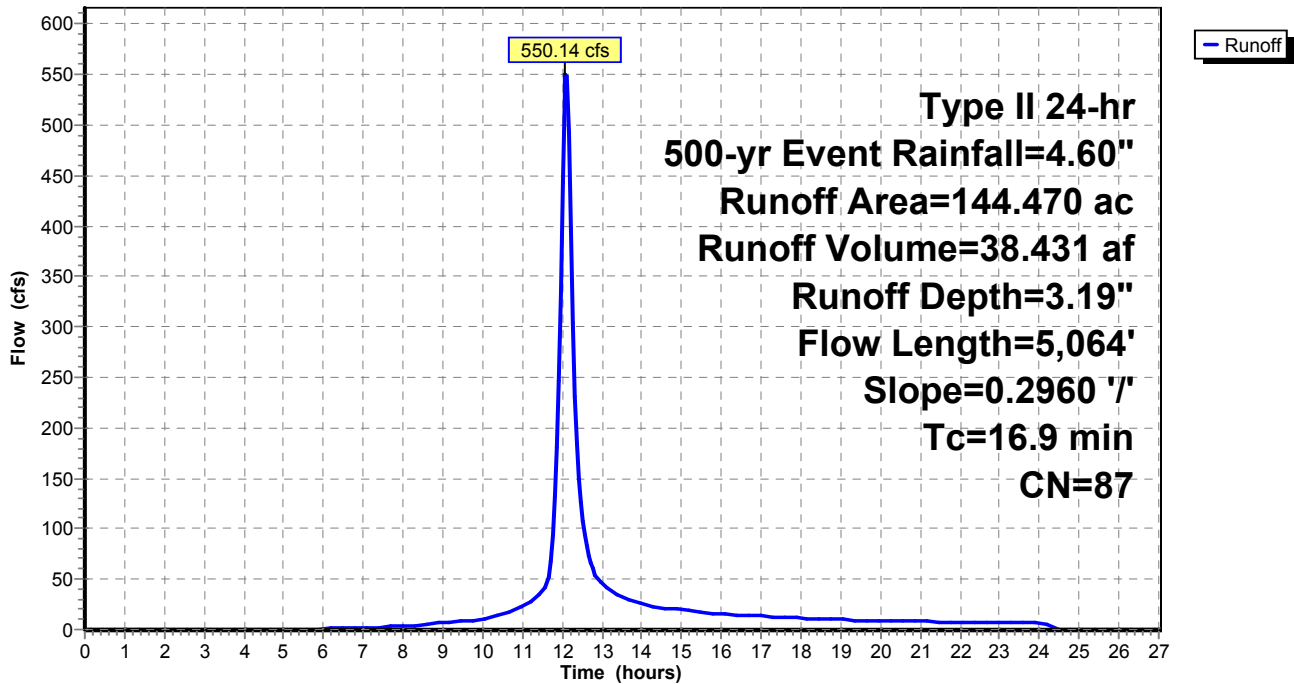
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 8: WS 8**

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

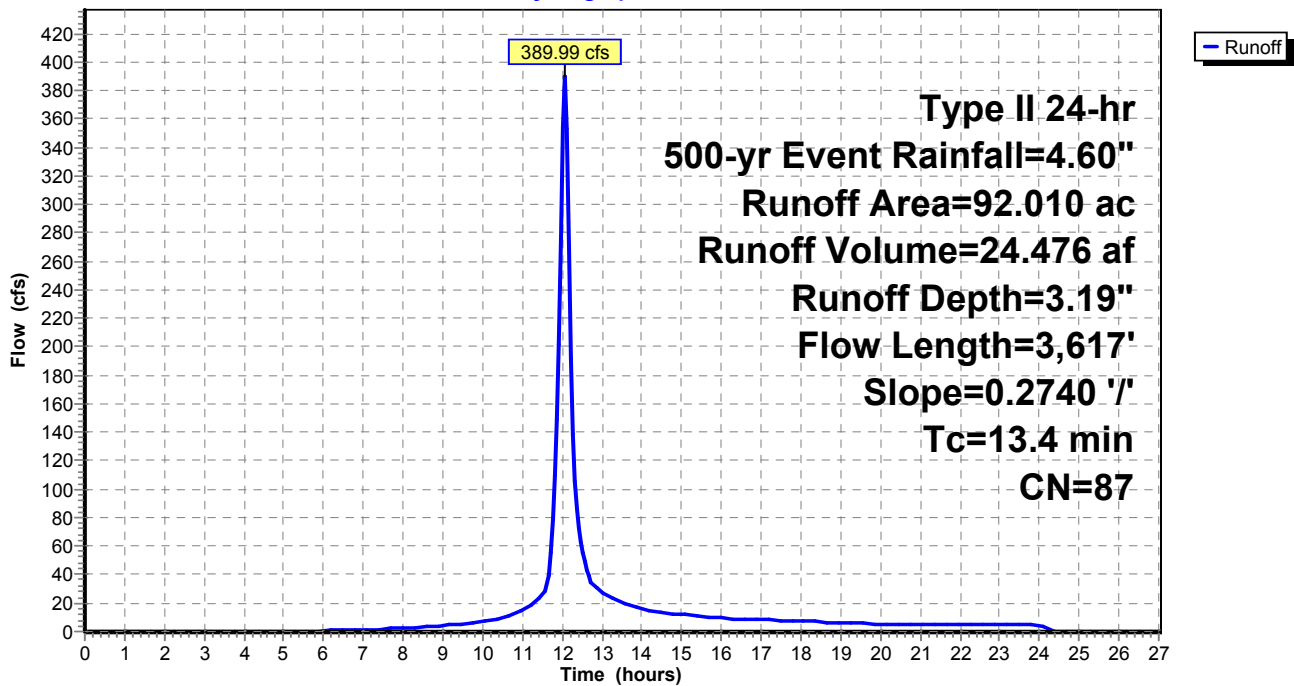
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 9: WS 9**

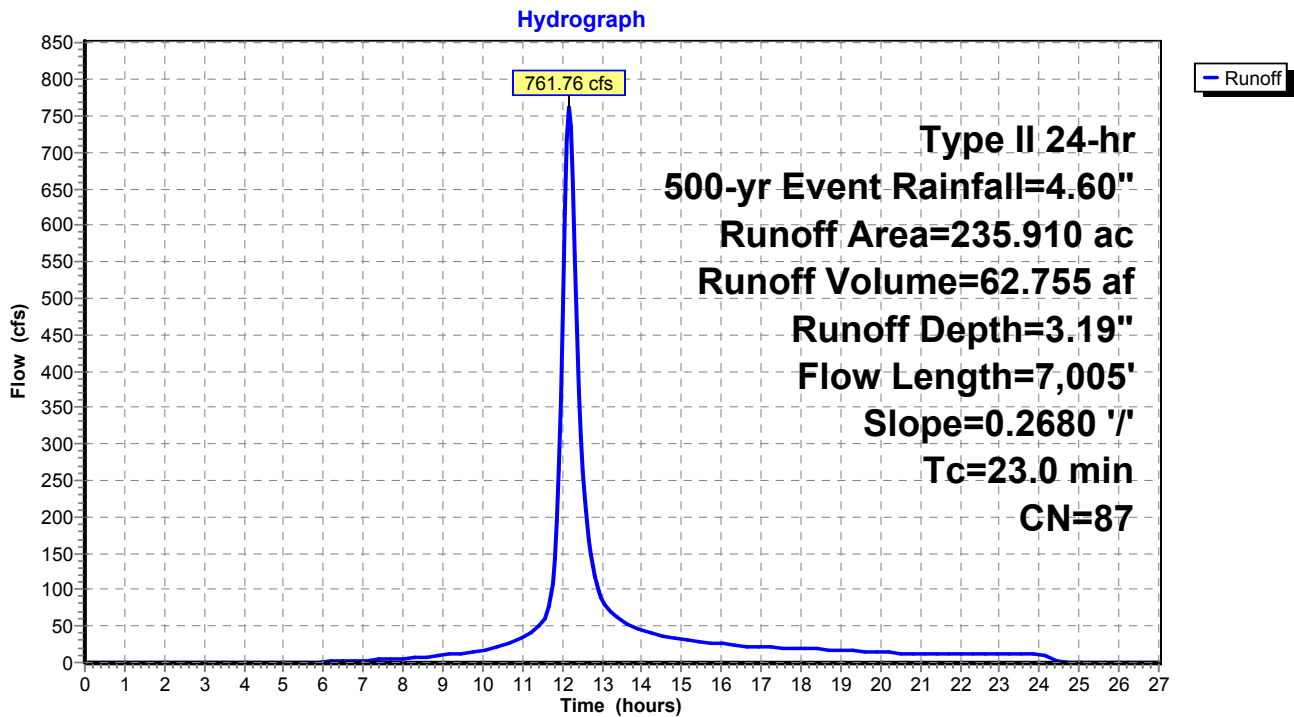
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**





**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 10: WS 10**

Runoff = 891.85 cfs @ 12.25 hrs, Volume= 87.894 af, Depth= 3.19"

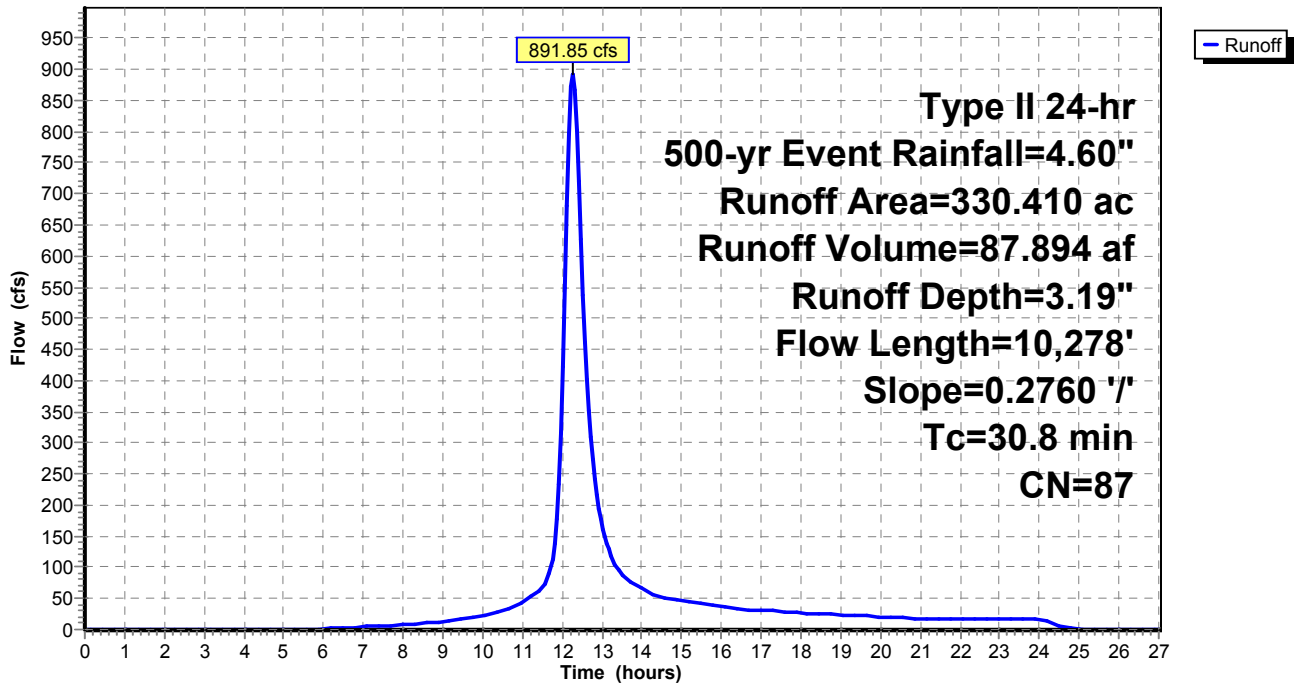
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 11: WS 11**

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

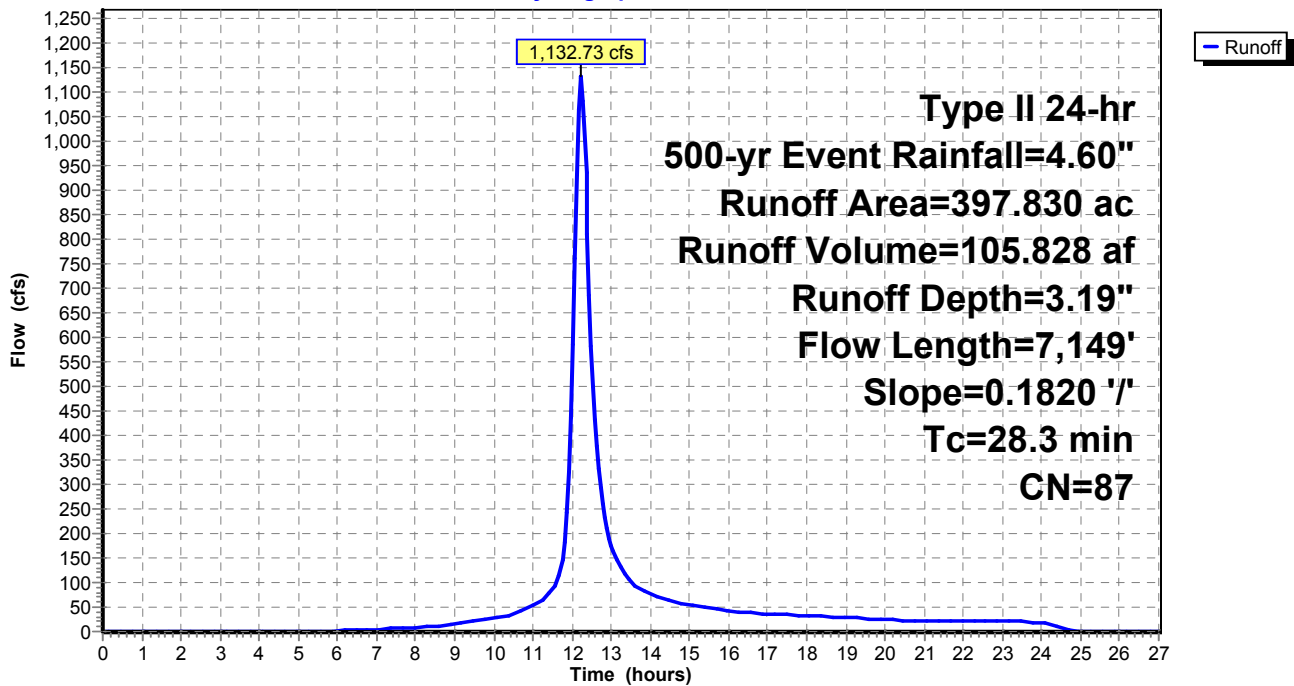
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 12: WS 12**

Runoff = 789.72 cfs @ 12.12 hrs, Volume= 60.497 af, Depth= 3.19"

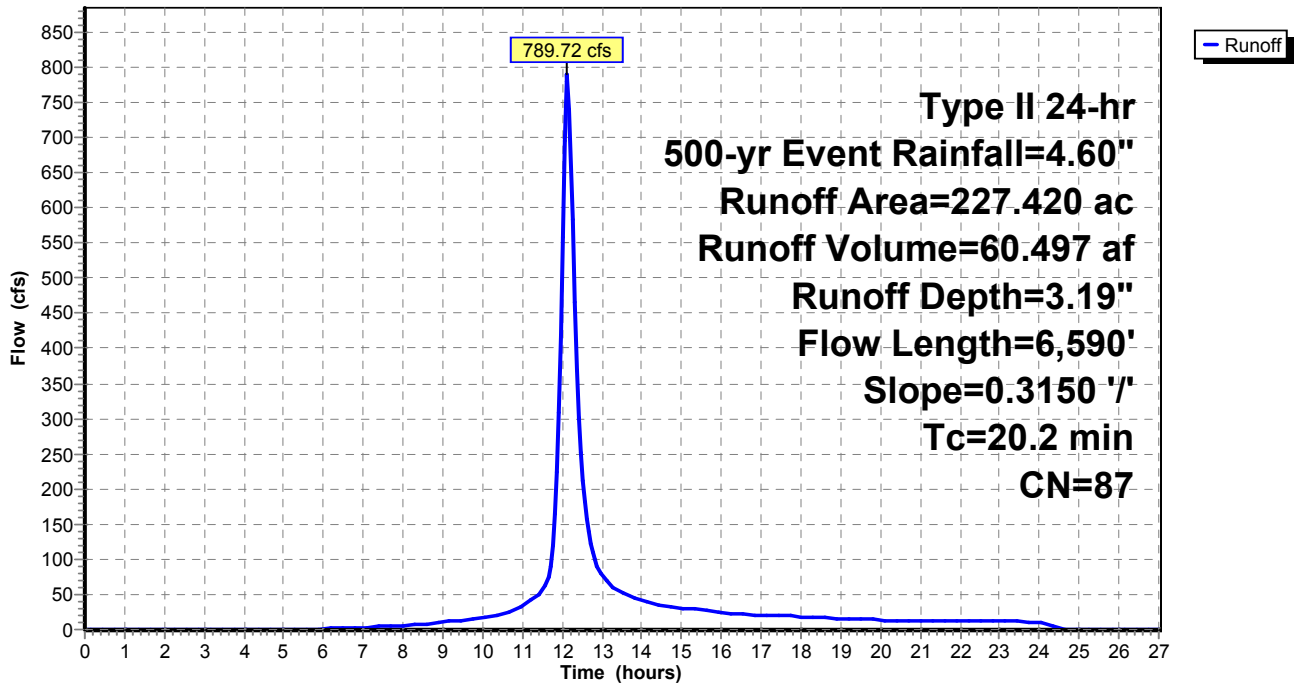
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 13: WS 13**

Runoff = 871.93 cfs @ 12.16 hrs, Volume= 73.289 af, Depth= 3.19"

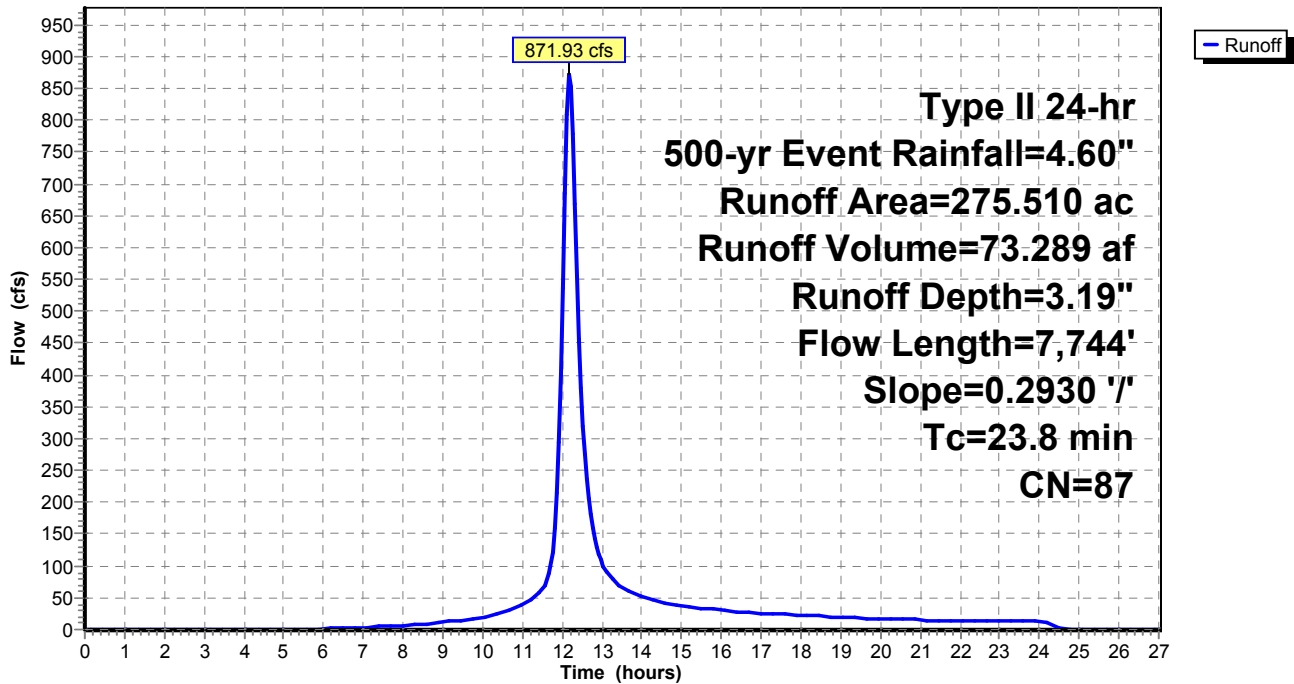
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Existing Watersheds (Post-Quintana)**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 14: WS 14**

Runoff = 326.19 cfs @ 12.06 hrs, Volume= 21.273 af, Depth= 3.19"

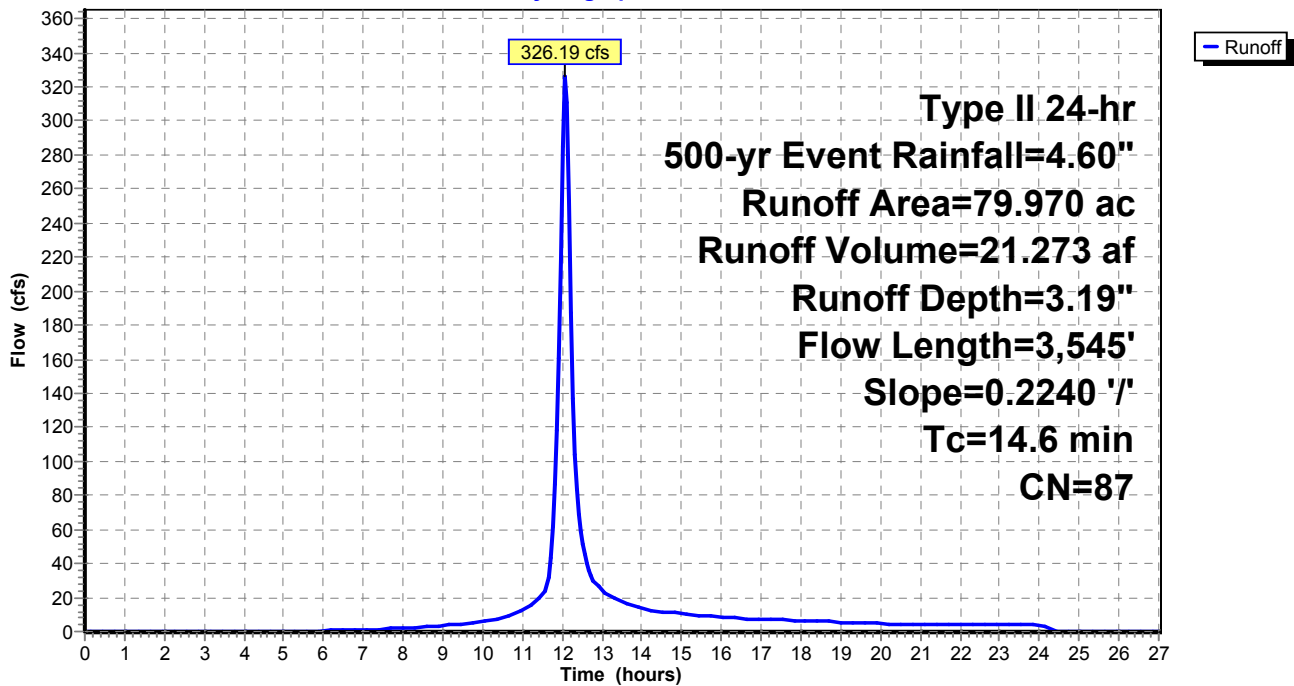
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
71.970	86	Desert shrub range, Fair, HSG D
* 8.000	98	Impervious, HSG D
79.970	87	Weighted Average
71.970		90.00% Pervious Area
8.000		10.00% Impervious Area

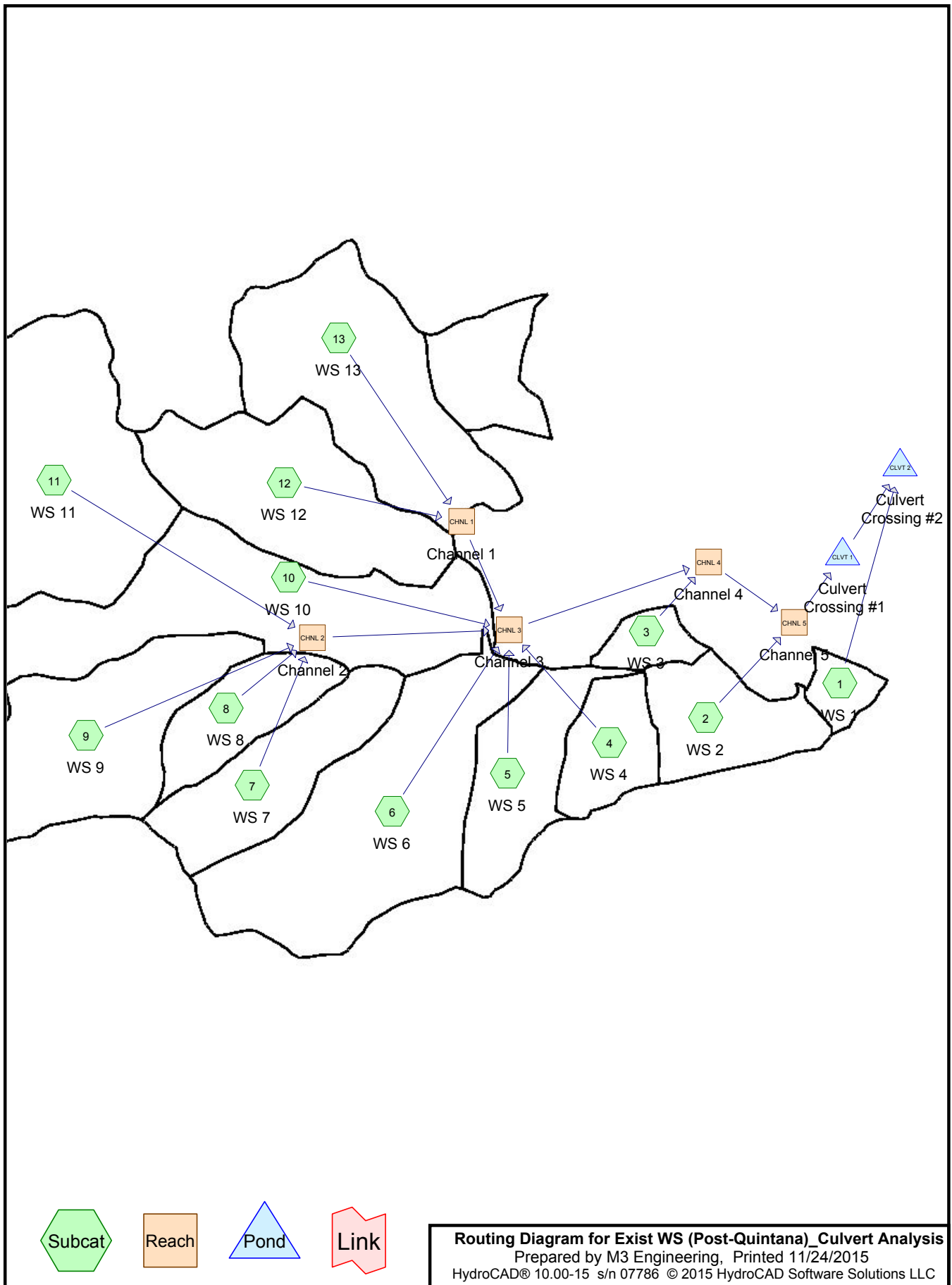
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6	3,545	0.2240	4.05		Lag/CN Method,

**Subcatchment 14: WS 14**

Hydrograph



## APPENDIX D



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
2,163.390	86	Desert shrub range, Fair, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13)
240.370	98	Impervious, HSG D (1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13)
<b>2,403.760</b>	<b>87</b>	<b>TOTAL AREA</b>



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2,403.760	HSG D	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
0.000	Other	
<b>2,403.760</b>		<b>TOTAL AREA</b>

**Exist WS (Post-Quintana)\_ Culvert Analysis**

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**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2,163.390	0.000	2,163.390	Desert shrub range, Fair	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
0.000	0.000	0.000	240.370	0.000	240.370	Impervious	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>2,403.760</b>	<b>0.000</b>	<b>2,403.760</b>	<b>TOTAL AREA</b>	

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.36" Tc=5.0 min CN=87 Runoff=117.20 cfs 5.555 af
<b>Subcatchment2: WS 2</b>	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=363.66 cfs 20.985 af
<b>Subcatchment3: WS 3</b>	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=129.54 cfs 6.890 af
<b>Subcatchment4: WS 4</b>	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=259.92 cfs 14.773 af
<b>Subcatchment5: WS 5</b>	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=342.49 cfs 24.455 af
<b>Subcatchment6: WS 6</b>	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=774.13 cfs 65.223 af
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=411.06 cfs 28.449 af
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=291.83 cfs 18.118 af
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=567.91 cfs 46.455 af
<b>Subcatchment10: WS 10</b>	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=663.73 cfs 65.063 af
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=843.39 cfs 78.339 af
<b>Subcatchment12: WS 12</b>	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=588.30 cfs 44.783 af
<b>Subcatchment13: WS 13</b>	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.36" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=649.87 cfs 54.252 af
<b>Reach CHNL 1: Channel 1</b>	Avg. Flow Depth=2.49' Max Vel=8.88 fps Inflow=1,228.49 cfs 99.035 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,184.09 cfs 99.033 af
<b>Reach CHNL 2: Channel 2</b>	Avg. Flow Depth=1.68' Max Vel=9.03 fps Inflow=1,933.74 cfs 171.361 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=1,565.27 cfs 170.940 af
<b>Reach CHNL 3: Channel 3</b>	Avg. Flow Depth=5.30' Max Vel=13.90 fps Inflow=3,580.90 cfs 439.487 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=3,520.63 cfs 439.082 af

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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**Reach CHNL 4: Channel 4** Avg. Flow Depth=3.72' Max Vel=20.81 fps Inflow=3,535.74 cfs 445.972 af  
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=3,523.29 cfs 445.885 af

**Reach CHNL 5: Channel 5** Avg. Flow Depth=6.13' Max Vel=16.97 fps Inflow=3,572.05 cfs 466.870 af  
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=3,552.39 cfs 466.654 af

**Pond CLVT 1: Culvert Crossing** Peak Elev=5,389.78' Storage=1,214,495 cf Inflow=3,552.39 cfs 466.654 af  
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,058.70 cfs 466.511 af

**Pond CLVT 2: Culvert Crossing** Peak Elev=5,386.54' Storage=5,175,763 cf Inflow=3,066.00 cfs 472.066 af  
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,545.71 cfs 472.015 af

**Total Runoff Area = 2,403.760 ac Runoff Volume = 473.340 af Average Runoff Depth = 2.36"**  
**90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac**

**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 117.20 cfs @ 11.95 hrs, Volume= 5.555 af, Depth= 2.36"

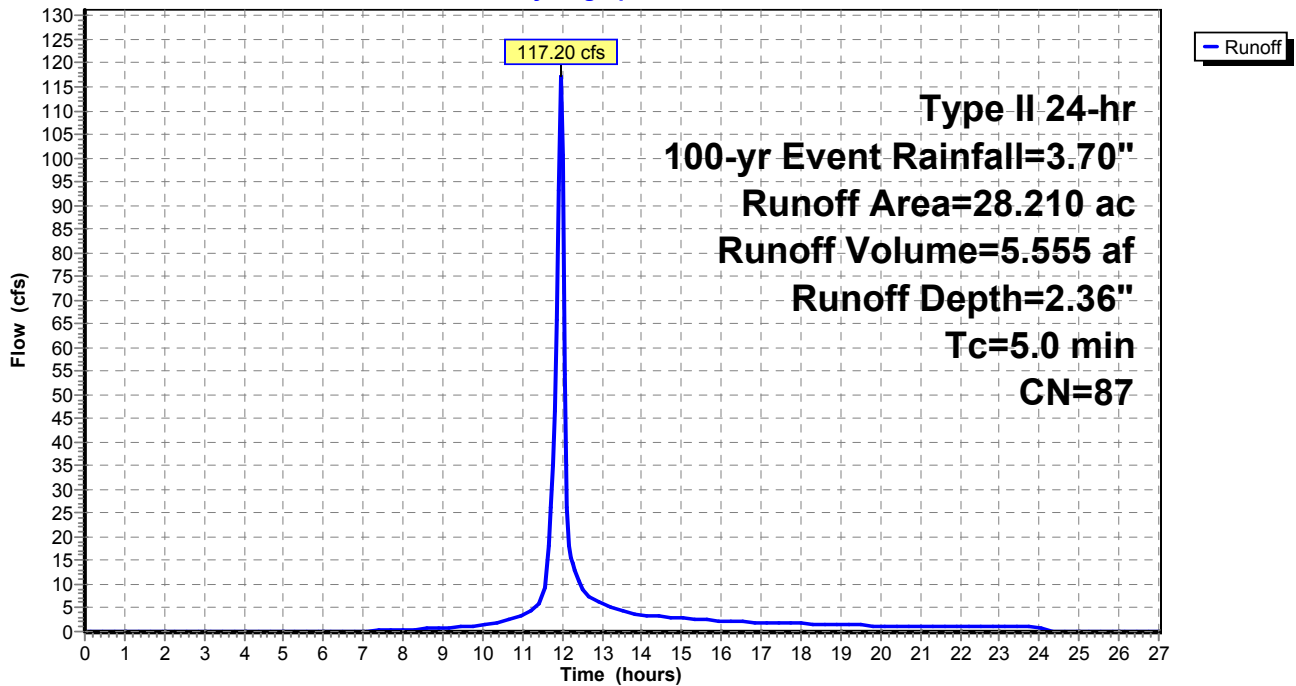
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 2: WS 2**

Runoff = 363.66 cfs @ 12.03 hrs, Volume= 20.985 af, Depth= 2.36"

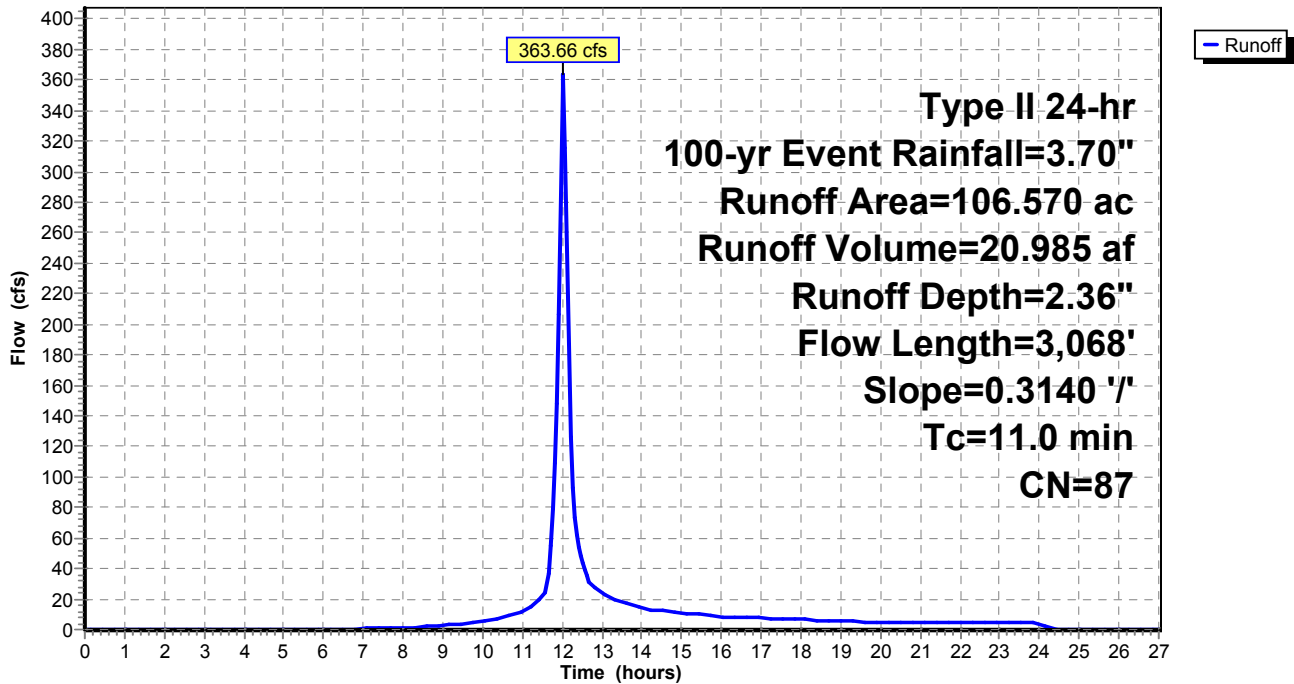
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 3: WS 3**

Runoff = 129.54 cfs @ 12.00 hrs, Volume= 6.890 af, Depth= 2.36"

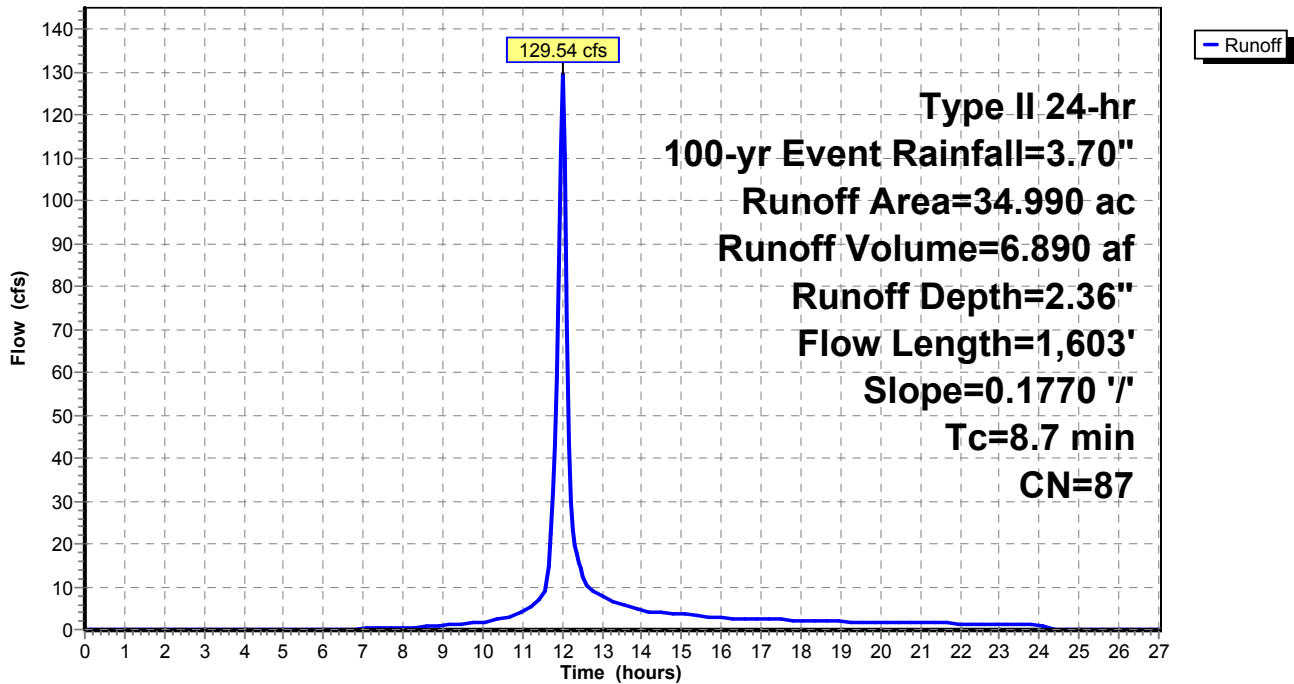
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 4: WS 4**

Runoff = 259.92 cfs @ 12.02 hrs, Volume= 14.773 af, Depth= 2.36"

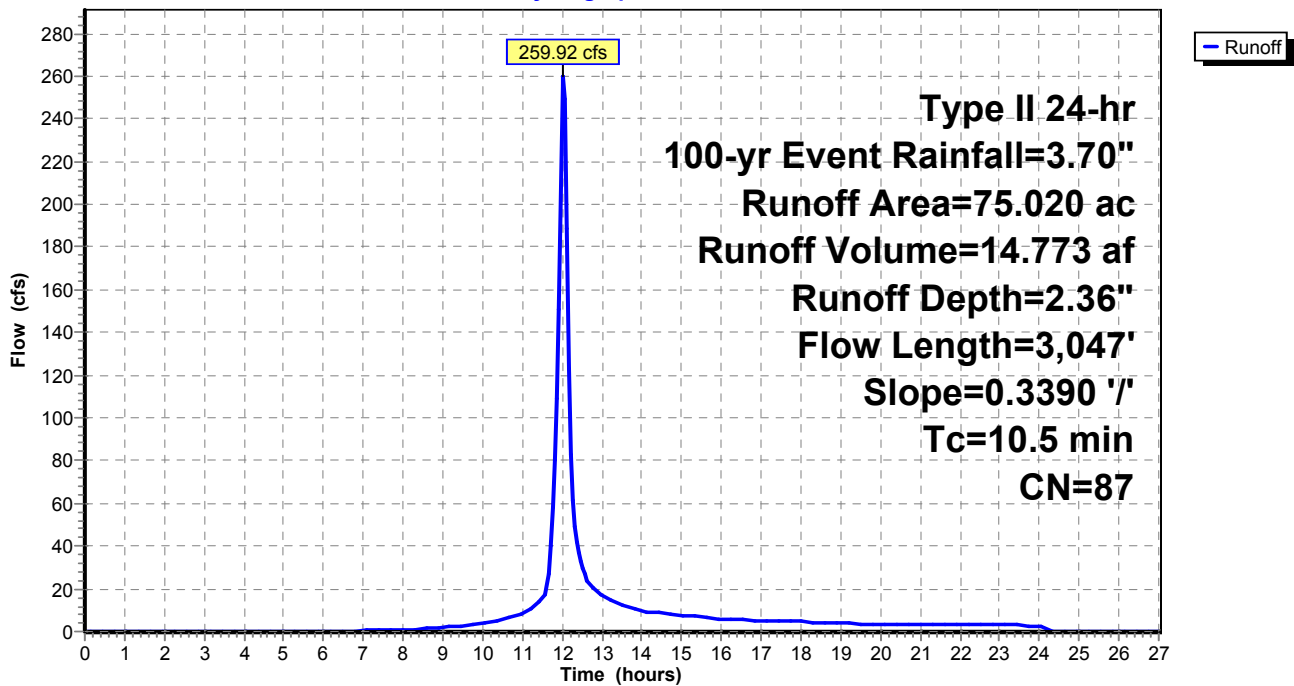
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph





**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 5: WS 5**

Runoff = 342.49 cfs @ 12.10 hrs, Volume= 24.455 af, Depth= 2.36"

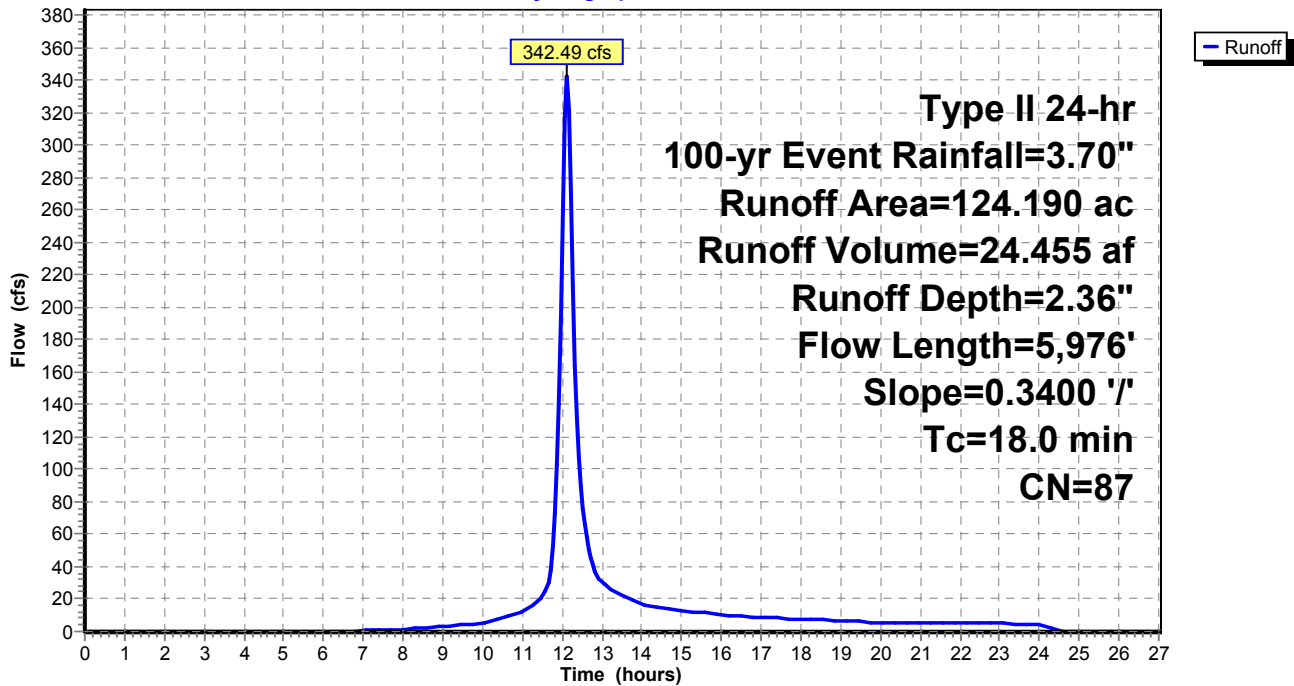
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 6: WS 6**

Runoff = 774.13 cfs @ 12.17 hrs, Volume= 65.223 af, Depth= 2.36"

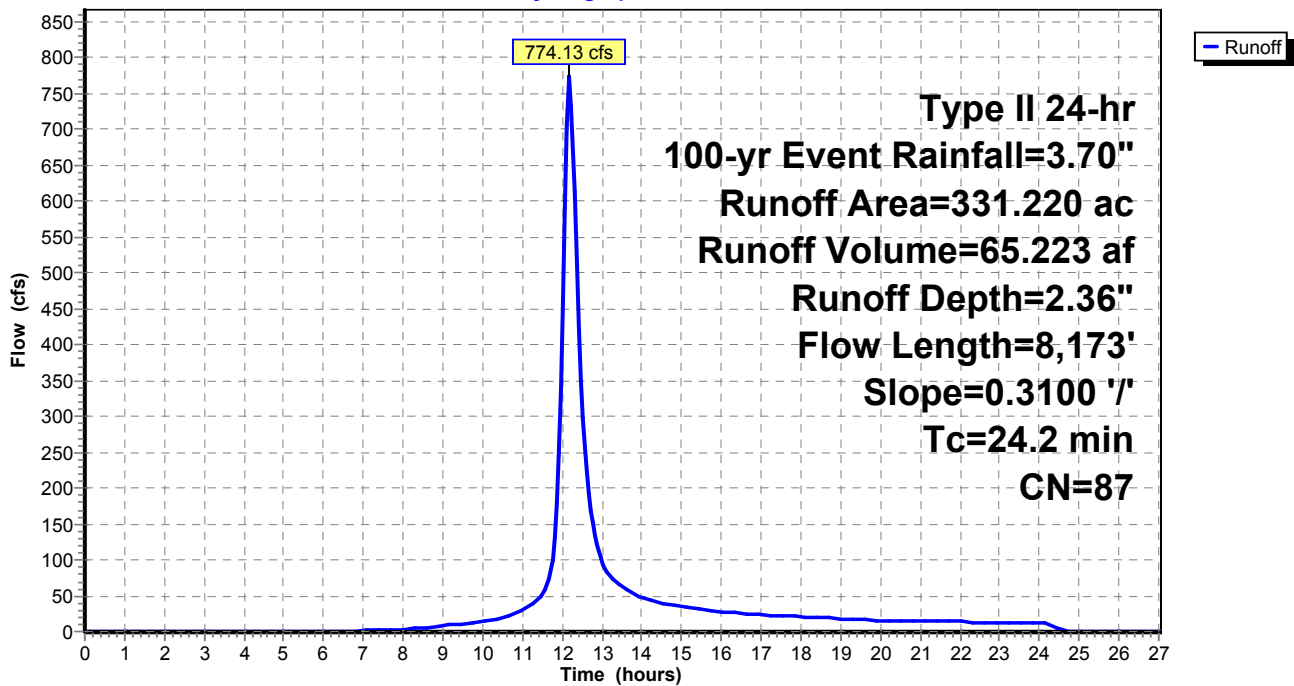
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 7: WS 7**

Runoff = 411.06 cfs @ 12.09 hrs, Volume= 28.449 af, Depth= 2.36"

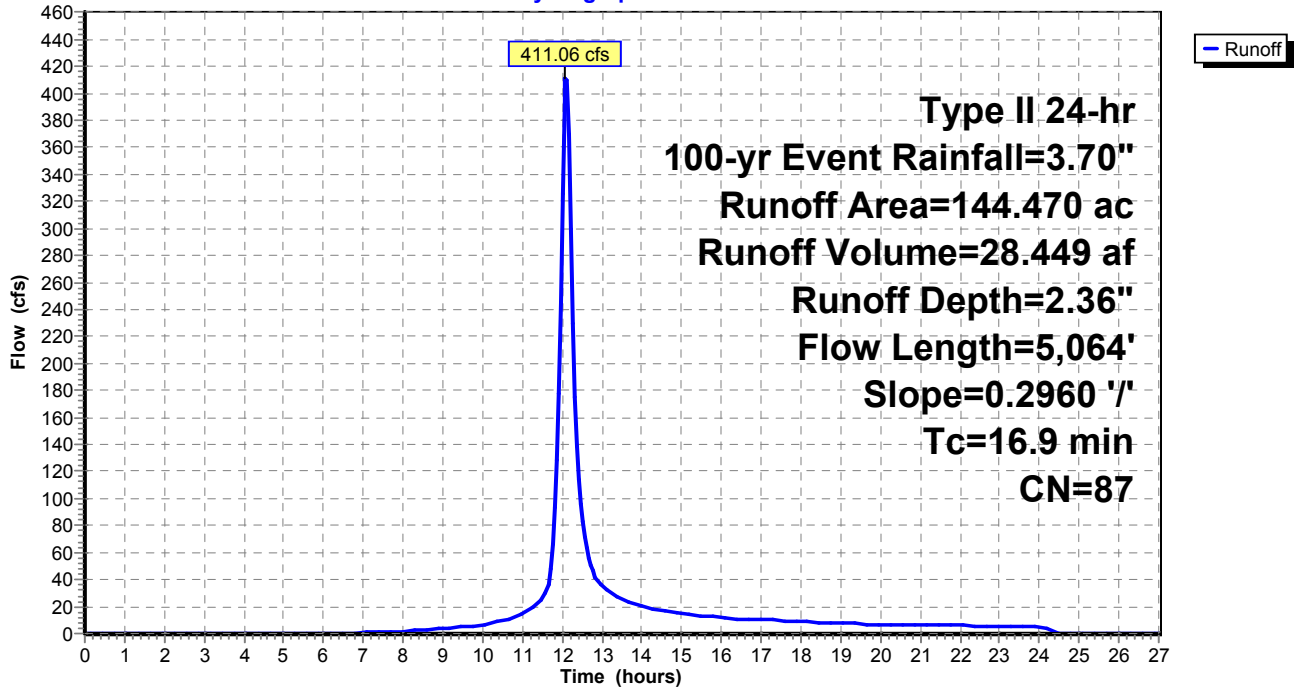
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



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**Summary for Subcatchment 8: WS 8**

Runoff = 291.83 cfs @ 12.05 hrs, Volume= 18.118 af, Depth= 2.36"

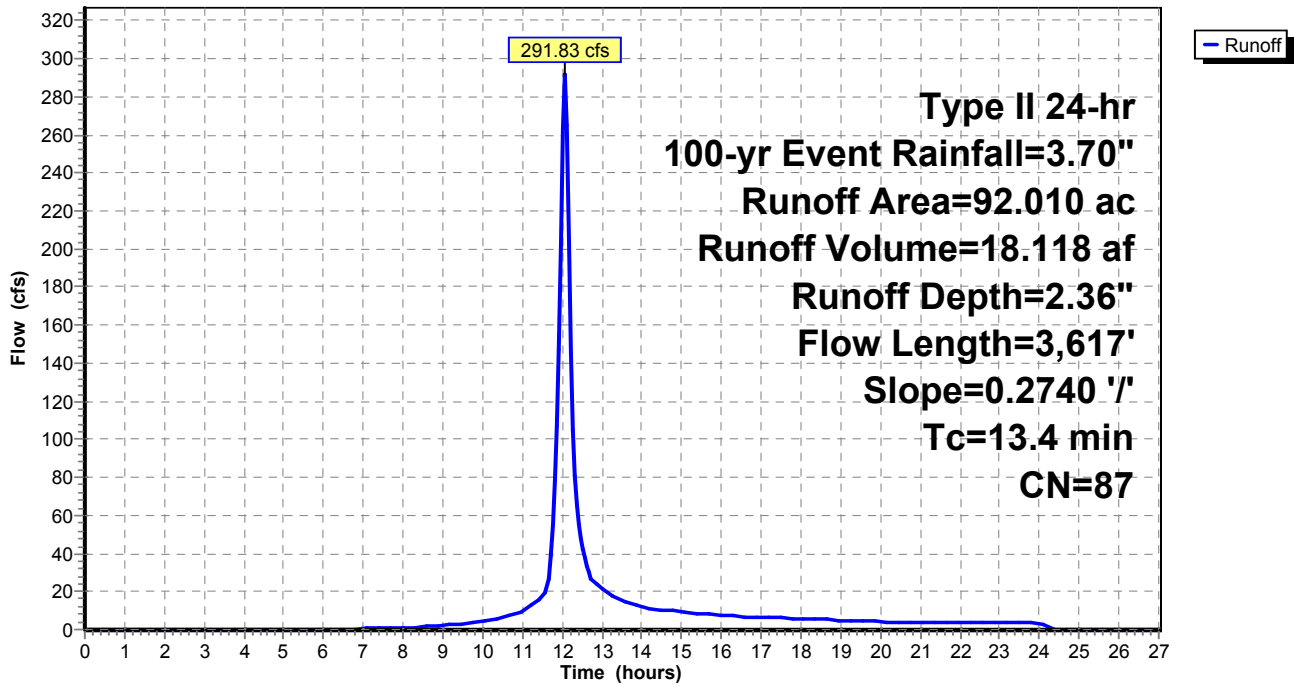
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



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**Summary for Subcatchment 9: WS 9**

Runoff = 567.91 cfs @ 12.16 hrs, Volume= 46.455 af, Depth= 2.36"

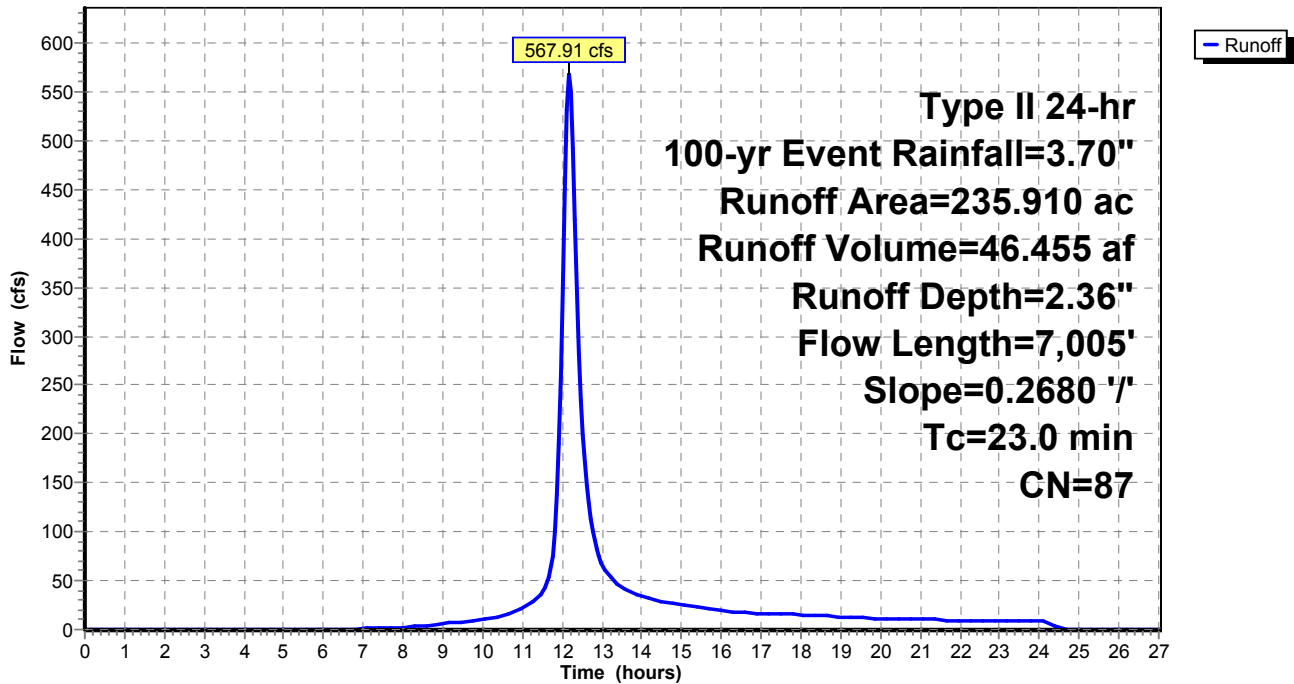
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph



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**Summary for Subcatchment 10: WS 10**

Runoff = 663.73 cfs @ 12.25 hrs, Volume= 65.063 af, Depth= 2.36"

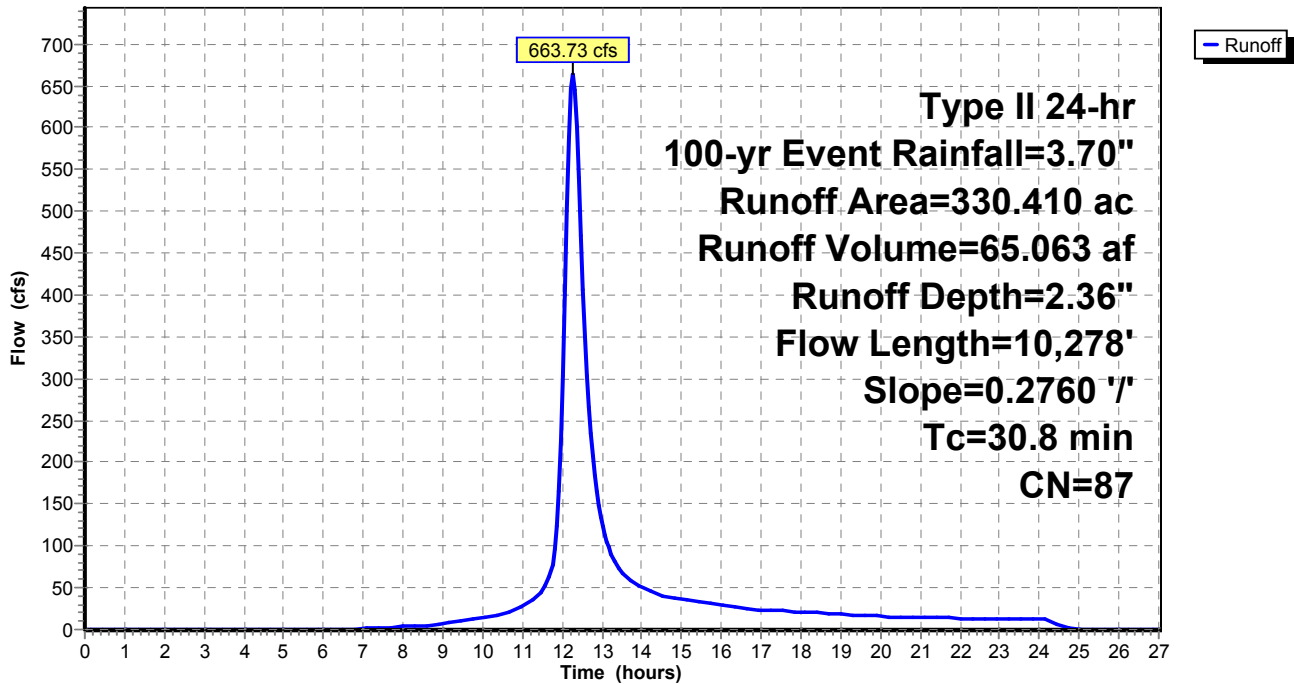
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 11: WS 11**

Runoff = 843.39 cfs @ 12.22 hrs, Volume= 78.339 af, Depth= 2.36"

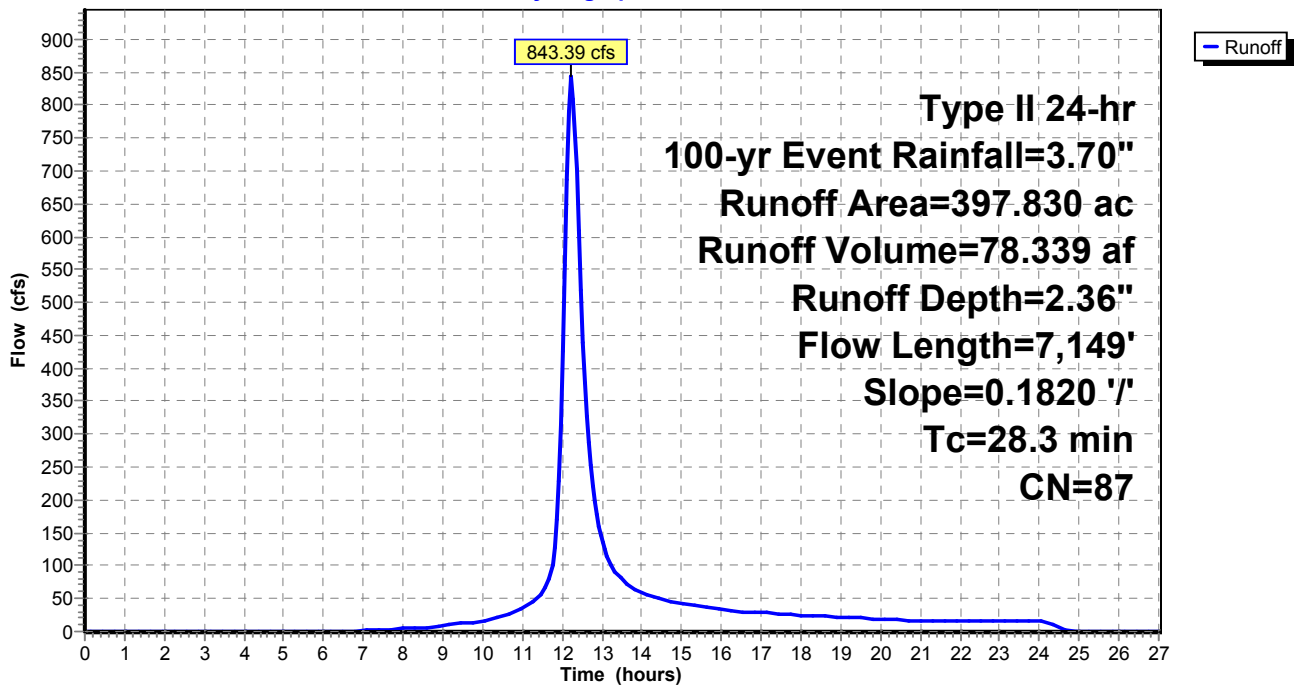
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Subcatchment 12: WS 12**

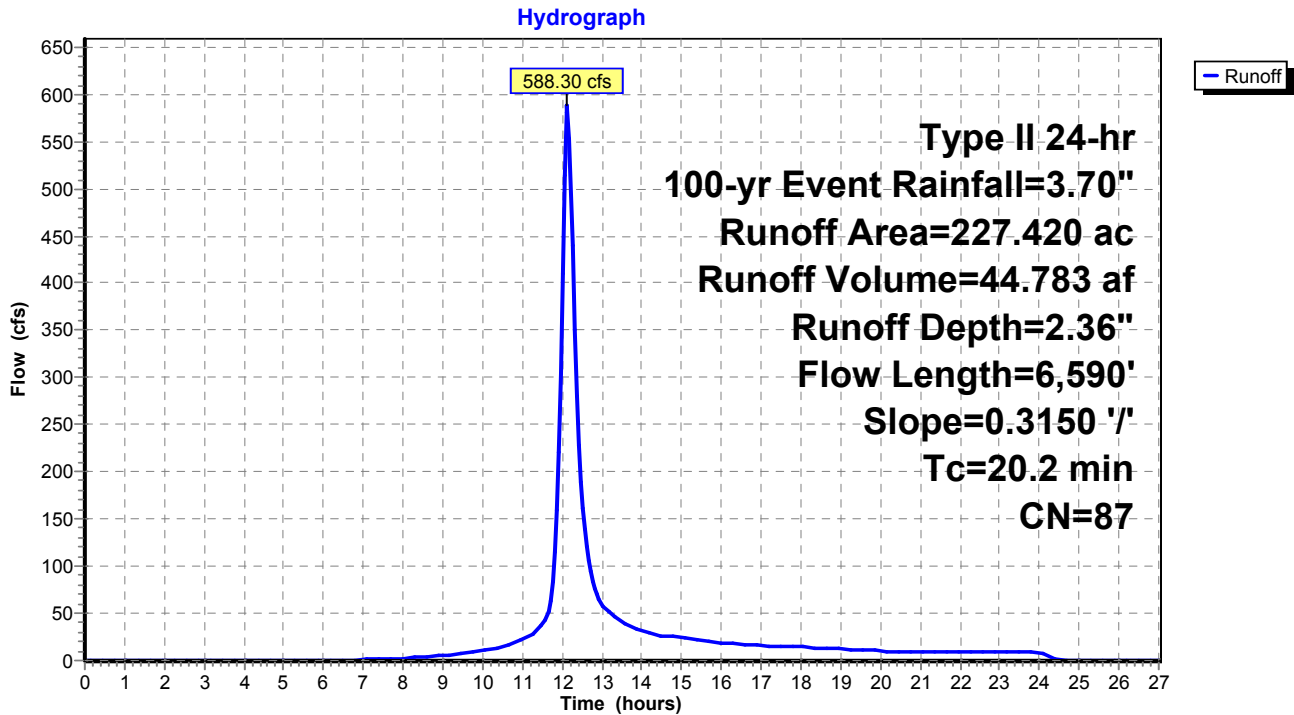
Runoff = 588.30 cfs @ 12.13 hrs, Volume= 44.783 af, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**





**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Subcatchment 13: WS 13**

Runoff = 649.87 cfs @ 12.17 hrs, Volume= 54.252 af, Depth= 2.36"

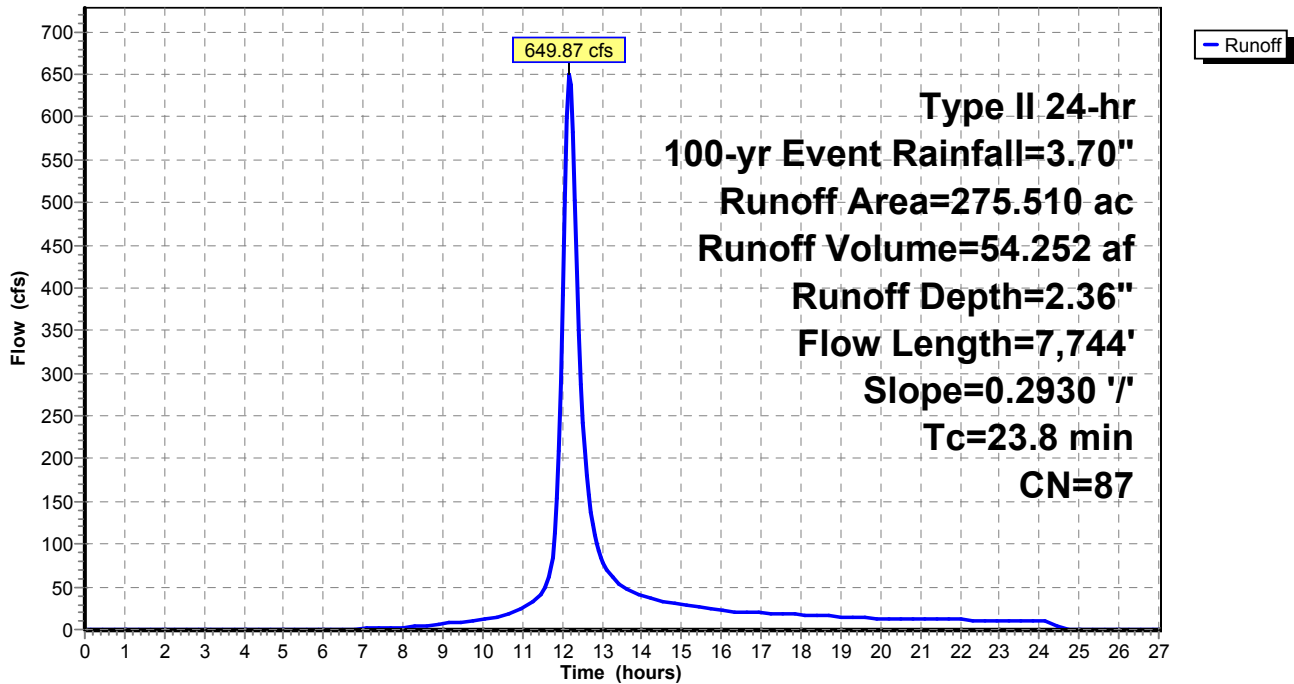
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 100-yr Event Rainfall=3.70"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

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**Summary for Reach CHNL 1: Channel 1**

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 2.36" for 100-yr Event event  
 Inflow = 1,228.49 cfs @ 12.15 hrs, Volume= 99.035 af  
 Outflow = 1,184.09 cfs @ 12.25 hrs, Volume= 99.033 af, Atten= 4%, Lag= 6.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 8.88 fps, Min. Travel Time= 3.6 min  
 Avg. Velocity = 2.21 fps, Avg. Travel Time= 14.4 min

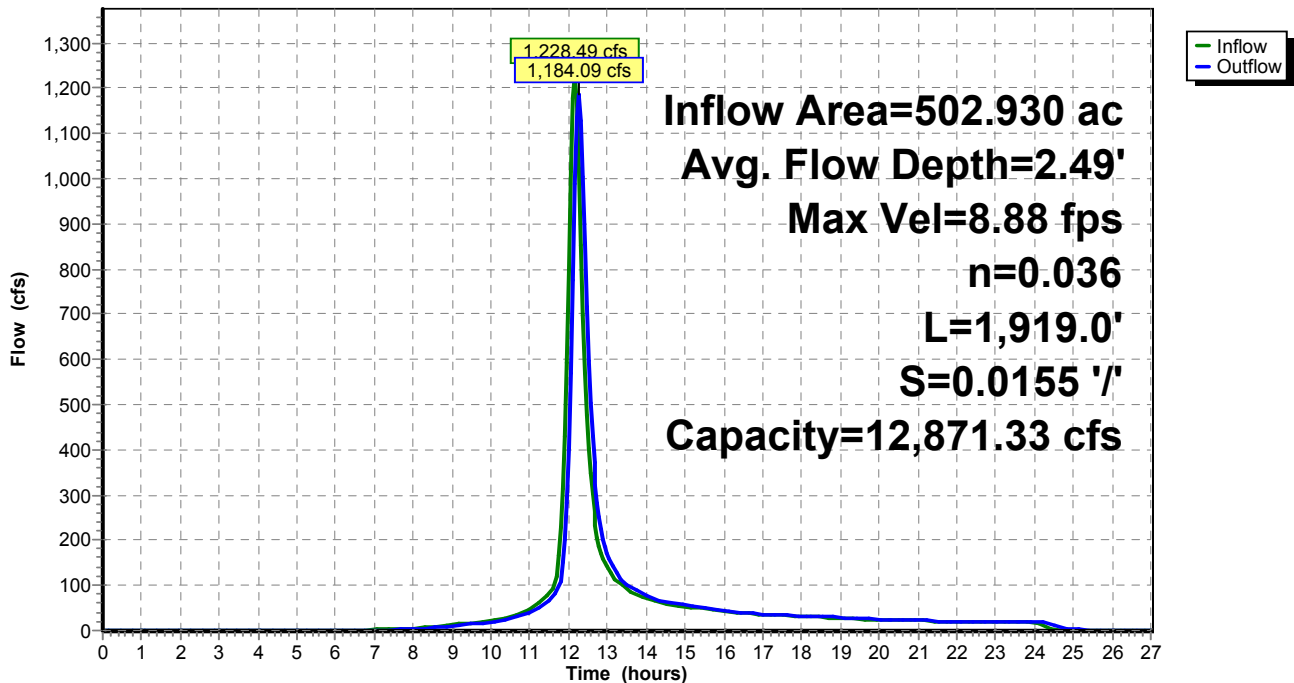
Peak Storage= 257,291 cf @ 12.19 hrs  
 Average Depth at Peak Storage= 2.49'  
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 80.00'  
 Length= 1,919.0' Slope= 0.0155 '/'  
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



**Reach CHNL 1: Channel 1**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Reach CHNL 2: Channel 2**

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 2.36" for 100-yr Event event  
 Inflow = 1,933.74 cfs @ 12.14 hrs, Volume= 171.361 af  
 Outflow = 1,565.27 cfs @ 12.49 hrs, Volume= 170.940 af, Atten= 19%, Lag= 21.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 9.03 fps, Min. Travel Time= 13.0 min  
 Avg. Velocity = 2.51 fps, Avg. Travel Time= 46.9 min

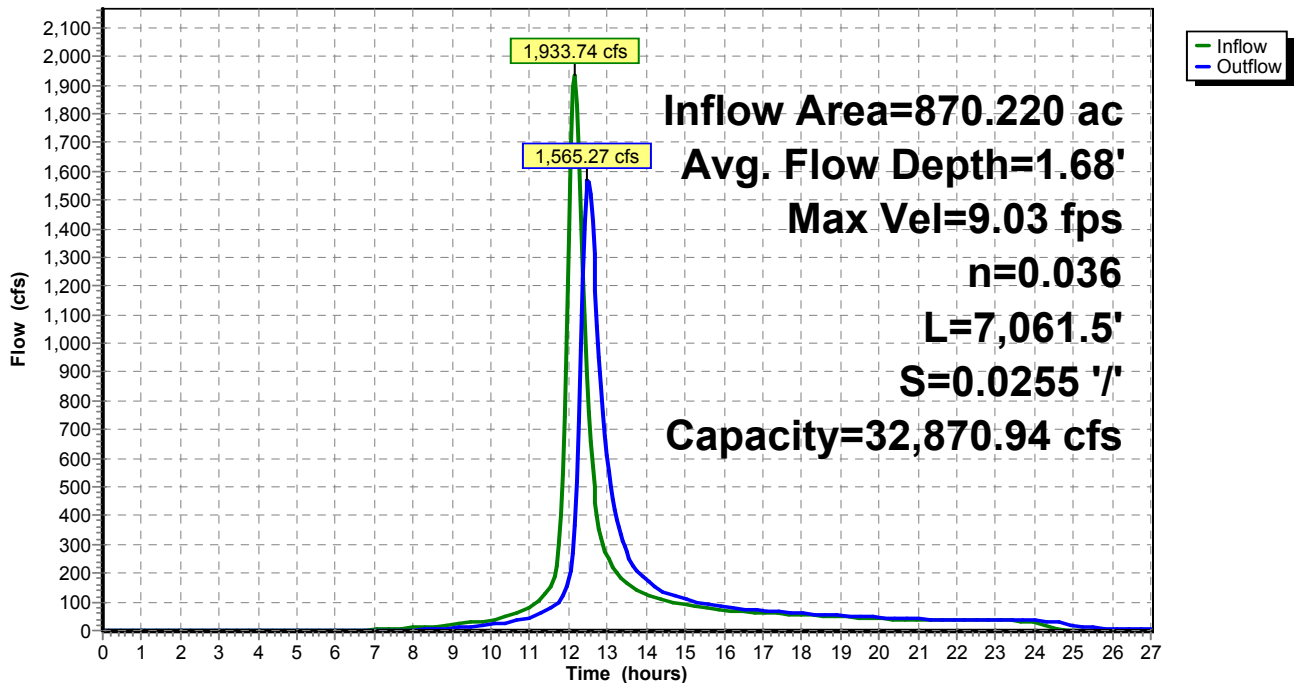
Peak Storage= 1,228,925 cf @ 12.27 hrs  
 Average Depth at Peak Storage= 1.68'  
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 2.5 2.0 ' / ' Top Width= 145.00'  
 Length= 7,061.5' Slope= 0.0255 ' / '  
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



**Reach CHNL 2: Channel 2**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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**Summary for Reach CHNL 3: Channel 3**

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 3.53' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 3.70' @ 12.40 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event  
 Inflow = 3,580.90 cfs @ 12.29 hrs, Volume= 439.487 af  
 Outflow = 3,520.63 cfs @ 12.42 hrs, Volume= 439.082 af, Atten= 2%, Lag= 7.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 13.90 fps, Min. Travel Time= 4.4 min  
 Avg. Velocity = 4.18 fps, Avg. Travel Time= 14.7 min

Peak Storage= 935,003 cf @ 12.35 hrs  
 Average Depth at Peak Storage= 5.30'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 3,680.5' Slope= 0.0162 '/'  
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



### Exist WS (Post-Quintana)\_Culvert Analysis

Type II 24-hr 100-yr Event Rainfall=3.70"

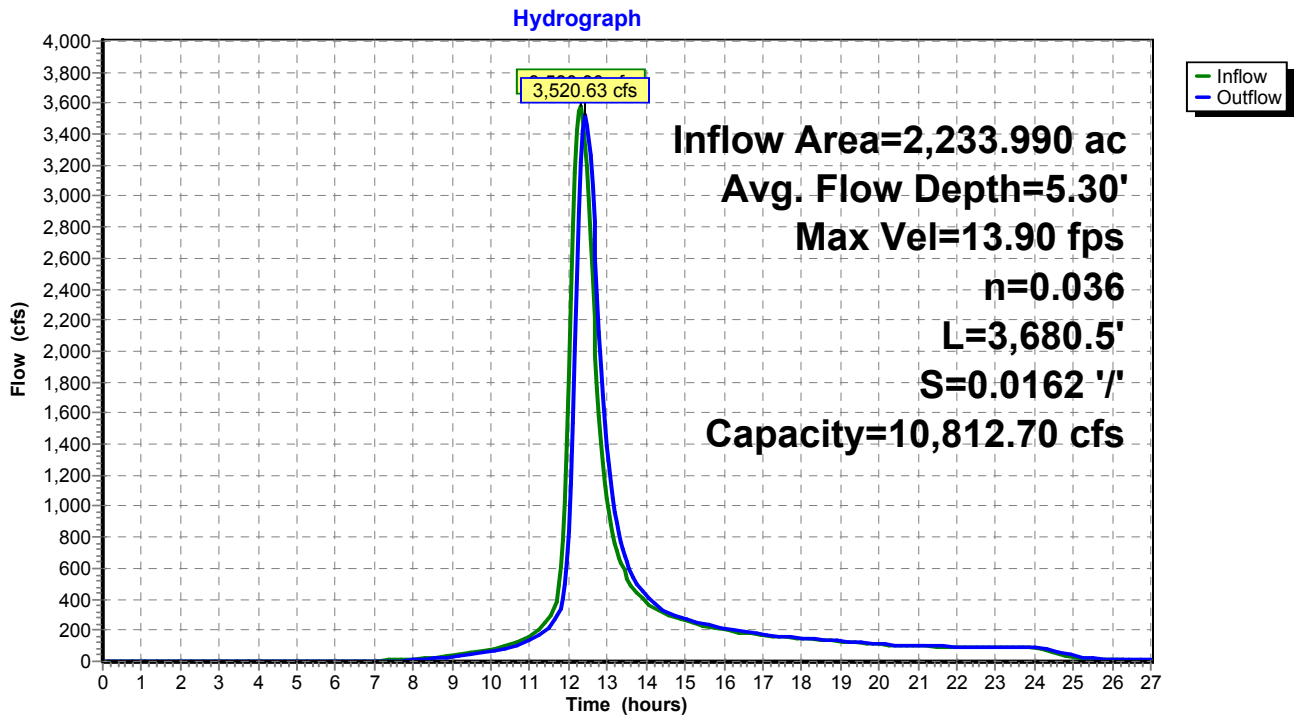
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### Reach CHNL 3: Channel 3



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Reach CHNL 4: Channel 4**

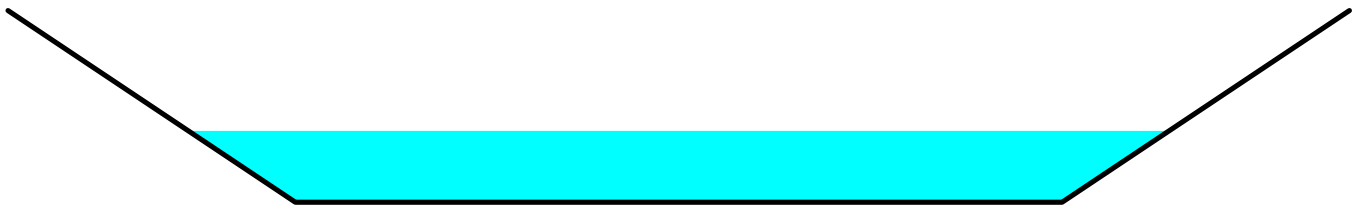
[61] Hint: Exceeded Reach CHNL 3 outlet invert by 3.72' @ 12.45 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event  
 Inflow = 3,535.74 cfs @ 12.42 hrs, Volume= 445.972 af  
 Outflow = 3,523.29 cfs @ 12.45 hrs, Volume= 445.885 af, Atten= 0%, Lag= 1.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 20.81 fps, Min. Travel Time= 0.9 min  
 Avg. Velocity = 6.18 fps, Avg. Travel Time= 3.0 min

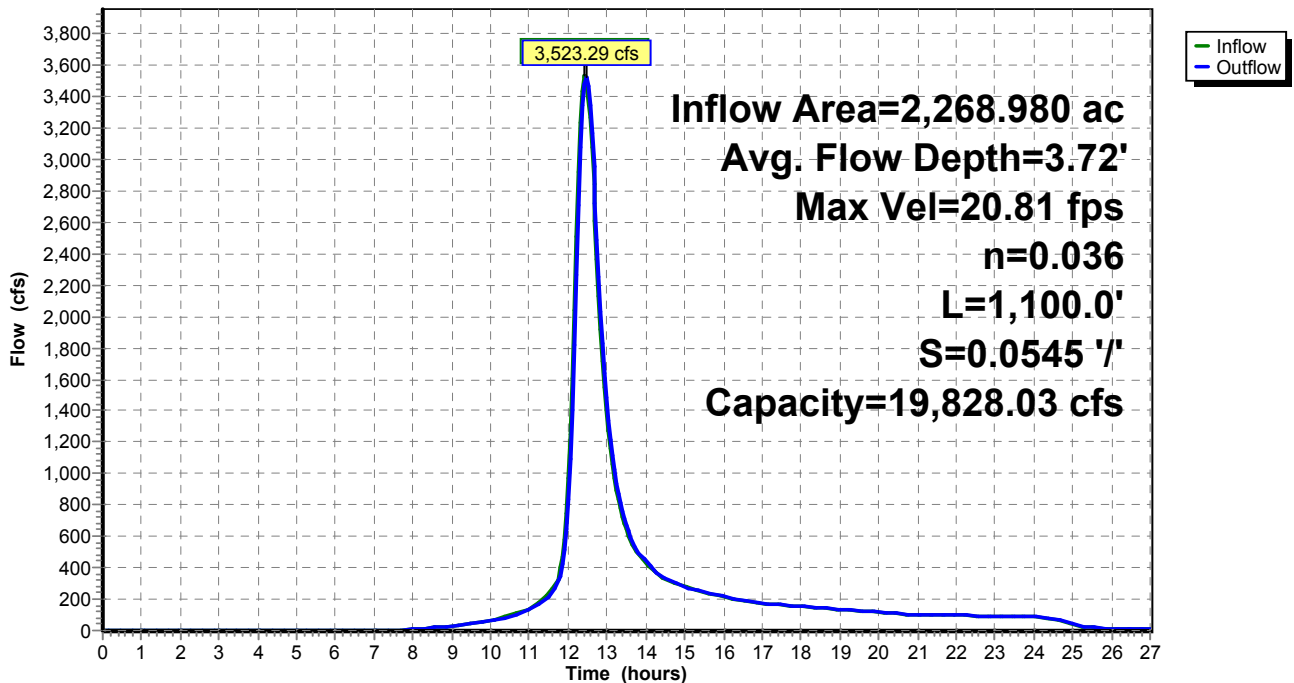
Peak Storage= 186,563 cf @ 12.43 hrs  
 Average Depth at Peak Storage= 3.72'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 1,100.0' Slope= 0.0545 '/'  
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



**Reach CHNL 4: Channel 4**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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**Summary for Reach CHNL 5: Channel 5**

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.45' @ 12.55 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event  
 Inflow = 3,572.05 cfs @ 12.44 hrs, Volume= 466.870 af  
 Outflow = 3,552.39 cfs @ 12.51 hrs, Volume= 466.654 af, Atten= 1%, Lag= 3.8 min

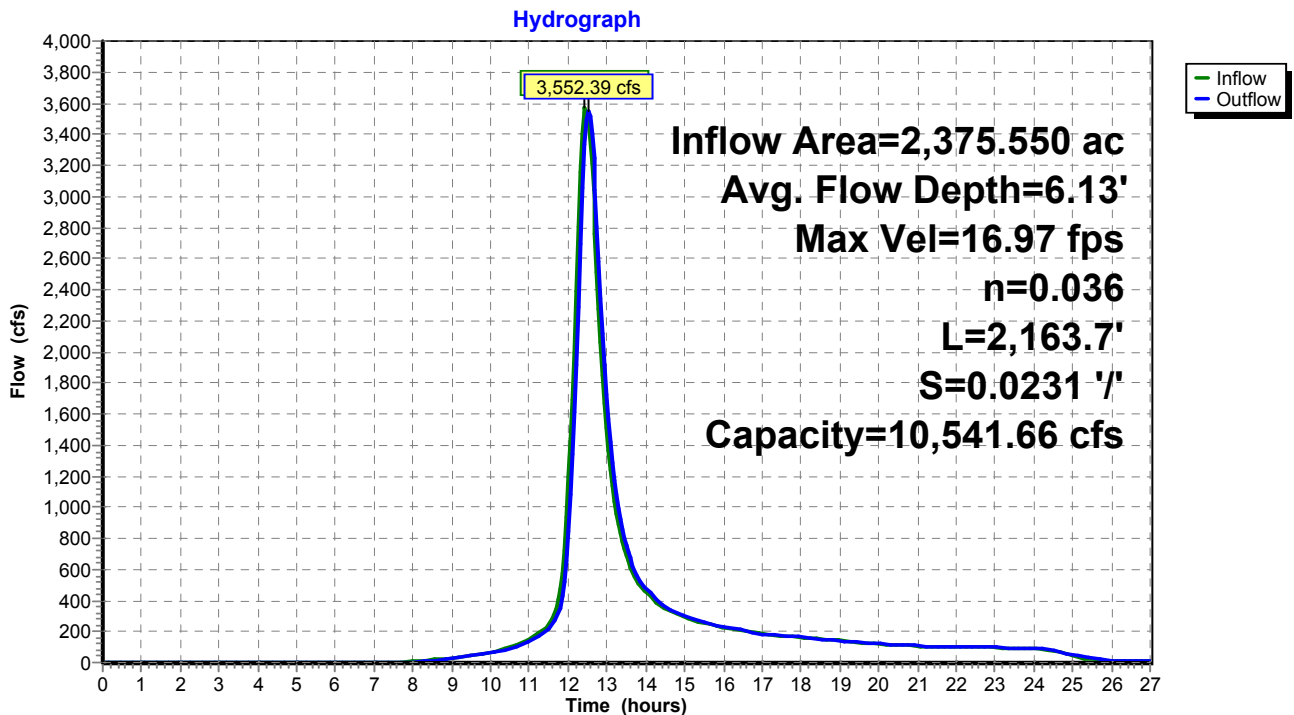
Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 16.97 fps, Min. Travel Time= 2.1 min  
 Avg. Velocity = 5.55 fps, Avg. Travel Time= 6.5 min

Peak Storage= 453,859 cf @ 12.47 hrs  
 Average Depth at Peak Storage= 6.13'  
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 58.00'  
 Length= 2,163.7' Slope= 0.0231 '/'  
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



**Reach CHNL 5: Channel 5**



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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**Summary for Pond CLVT 1: Culvert Crossing #1**

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 14.44' @ 12.75 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event  
 Inflow = 3,552.39 cfs @ 12.51 hrs, Volume= 466.654 af  
 Outflow = 3,058.70 cfs @ 12.70 hrs, Volume= 466.511 af, Atten= 14%, Lag= 11.4 min  
 Primary = 3,058.70 cfs @ 12.70 hrs, Volume= 466.511 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,389.78' @ 12.70 hrs Surf.Area= 0 sf Storage= 1,214,495 cf  
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 3.5 min calculated for 465.649 af (100% of inflow)  
 Center-of-Mass det. time= 3.3 min ( 865.7 - 862.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	<b>Culvert Crossing #1</b> Listed below



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

**Exist WS (Post-Quintana)\_ Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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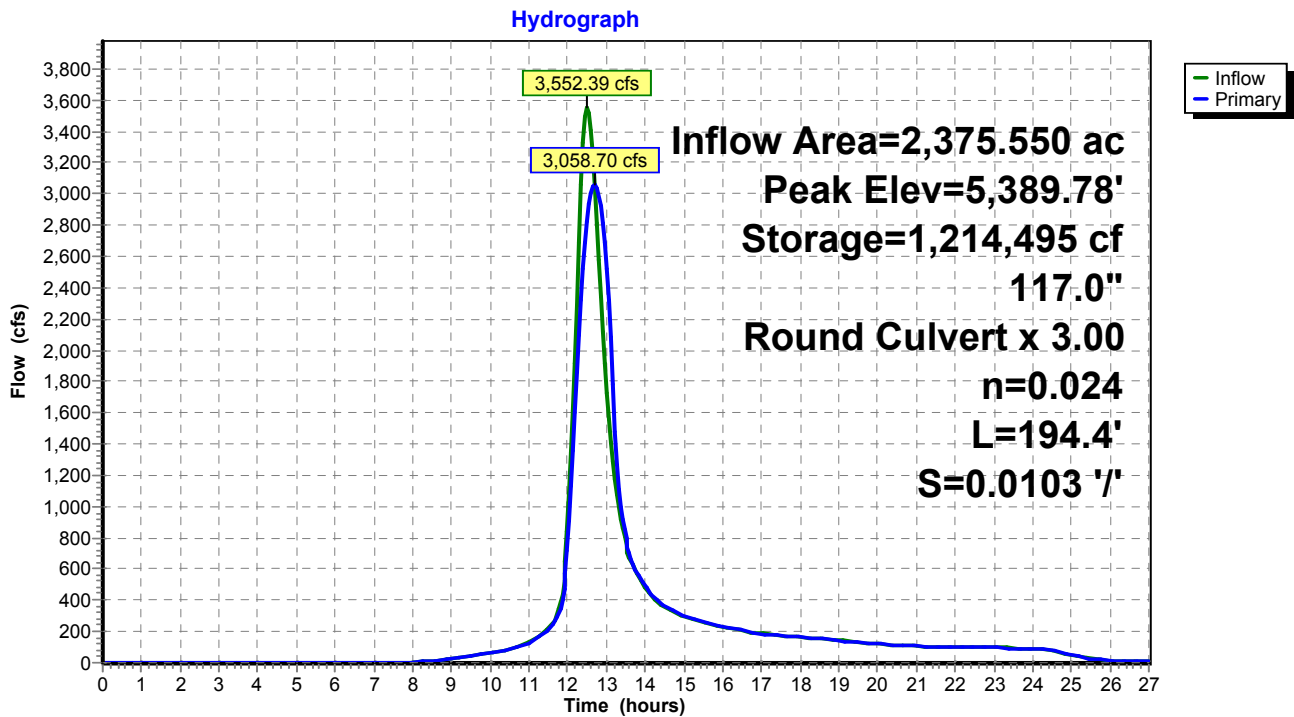
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	<b>117.0" Round Culvert X 3.00</b> L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=3,057.57 cfs @ 12.70 hrs HW=5,389.77' (Free Discharge)  
 ↳ **1=Culvert** (Inlet Controls 3,057.57 cfs @ 13.65 fps)

**Pond CLVT 1: Culvert Crossing #1**



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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**Summary for Pond CLVT 2: Culvert Crossing #2**

[81] Warning: Exceeded Pond CLVT 1 by 7.40' @ 13.35 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 2.36" for 100-yr Event event  
 Inflow = 3,066.00 cfs @ 12.70 hrs, Volume= 472.066 af  
 Outflow = 1,545.71 cfs @ 13.19 hrs, Volume= 472.015 af, Atten= 50%, Lag= 29.9 min  
 Primary = 1,545.71 cfs @ 13.19 hrs, Volume= 472.015 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,386.54' @ 13.19 hrs Surf.Area= 0 sf Storage= 5,175,763 cf  
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 26.2 min calculated for 471.143 af (100% of inflow)  
 Center-of-Mass det. time= 26.0 min ( 891.1 - 865.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	<b>Existing Pond</b> Listed below

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 100-yr Event Rainfall=3.70"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

**Exist WS (Post-Quintana)\_ Culvert Analysis**

Type II 24-hr 100-yr Event Rainfall=3.70"

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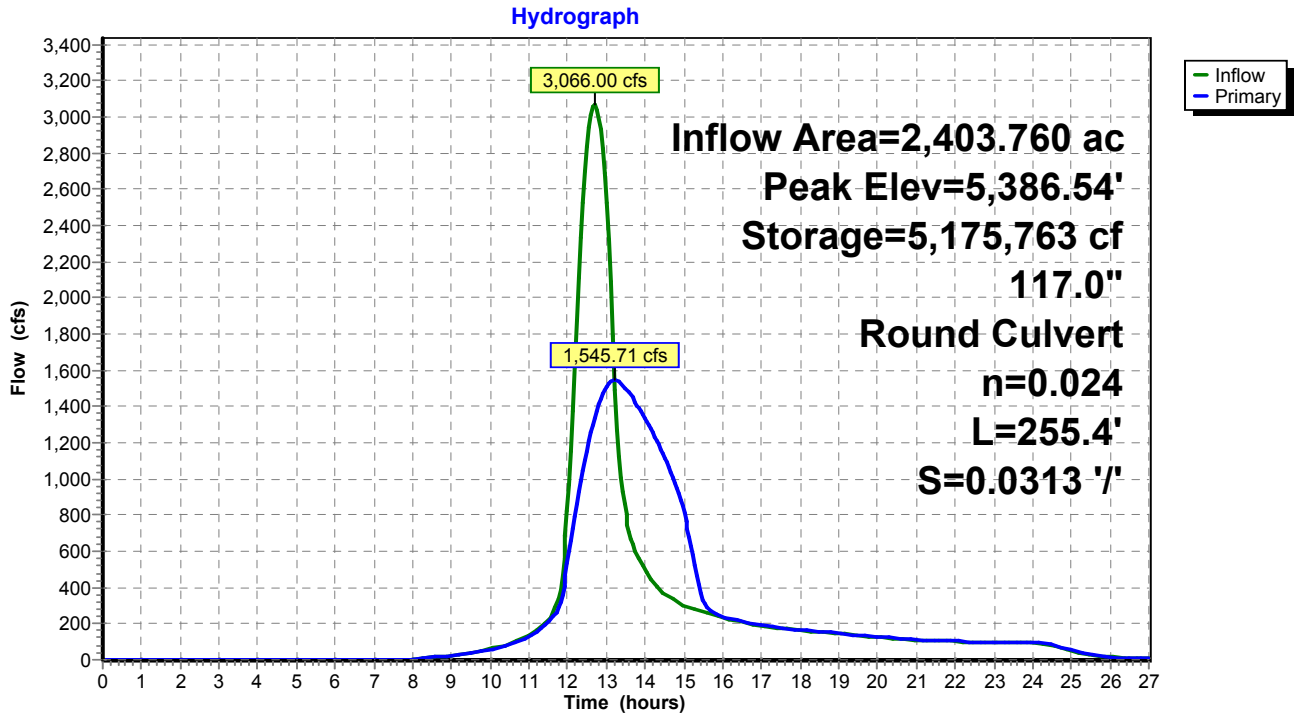
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5,402.00      9,814,540  
 5,403.00      10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	<b>117.0" Round Culvert</b> L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=1,545.45 cfs @ 13.19 hrs HW=5,386.53' (Free Discharge)  
 ←**1=Culvert** (Inlet Controls 1,545.45 cfs @ 20.70 fps)

**Pond CLVT 2: Culvert Crossing #2**



**Exist WS (Post-Quintana)\_Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=2.72" Tc=5.0 min CN=87 Runoff=133.97 cfs 6.393 af
<b>Subcatchment2: WS 2</b>	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=415.35 cfs 24.150 af
<b>Subcatchment3: WS 3</b>	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=148.20 cfs 7.929 af
<b>Subcatchment4: WS 4</b>	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=297.56 cfs 17.000 af
<b>Subcatchment5: WS 5</b>	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=392.70 cfs 28.143 af
<b>Subcatchment6: WS 6</b>	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=888.50 cfs 75.058 af
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=471.20 cfs 32.739 af
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=334.29 cfs 20.851 af
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=651.70 cfs 53.460 af
<b>Subcatchment10: WS 10</b>	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=762.30 cfs 74.875 af
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=968.42 cfs 90.153 af
<b>Subcatchment12: WS 12</b>	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=674.80 cfs 51.536 af
<b>Subcatchment13: WS 13</b>	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=2.72" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=745.85 cfs 62.434 af
<b>Reach CHNL 1: Channel 1</b>	Avg. Flow Depth=2.71' Max Vel=9.34 fps Inflow=1,409.59 cfs 113.970 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,364.28 cfs 113.968 af
<b>Reach CHNL 2: Channel 2</b>	Avg. Flow Depth=1.84' Max Vel=9.59 fps Inflow=2,220.40 cfs 197.202 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=1,835.68 cfs 196.763 af
<b>Reach CHNL 3: Channel 3</b>	Avg. Flow Depth=5.83' Max Vel=14.67 fps Inflow=4,230.35 cfs 505.807 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=4,159.08 cfs 505.394 af

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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**Reach CHNL 4: Channel 4** Avg. Flow Depth=4.10' Max Vel=22.02 fps Inflow=4,176.49 cfs 513.323 af  
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=4,160.94 cfs 513.234 af

**Reach CHNL 5: Channel 5** Avg. Flow Depth=6.72' Max Vel=17.83 fps Inflow=4,217.67 cfs 537.384 af  
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=4,191.95 cfs 537.163 af

**Pond CLVT 1: Culvert Crossing** Peak Elev=5,392.92' Storage=1,758,861 cf Inflow=4,191.95 cfs 537.163 af  
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,410.14 cfs 537.020 af

**Pond CLVT 2: Culvert Crossing** Peak Elev=5,391.04' Storage=6,444,314 cf Inflow=3,418.33 cfs 543.413 af  
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,658.81 cfs 543.361 af

**Total Runoff Area = 2,403.760 ac Runoff Volume = 544.720 af Average Runoff Depth = 2.72"**  
**90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac**

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

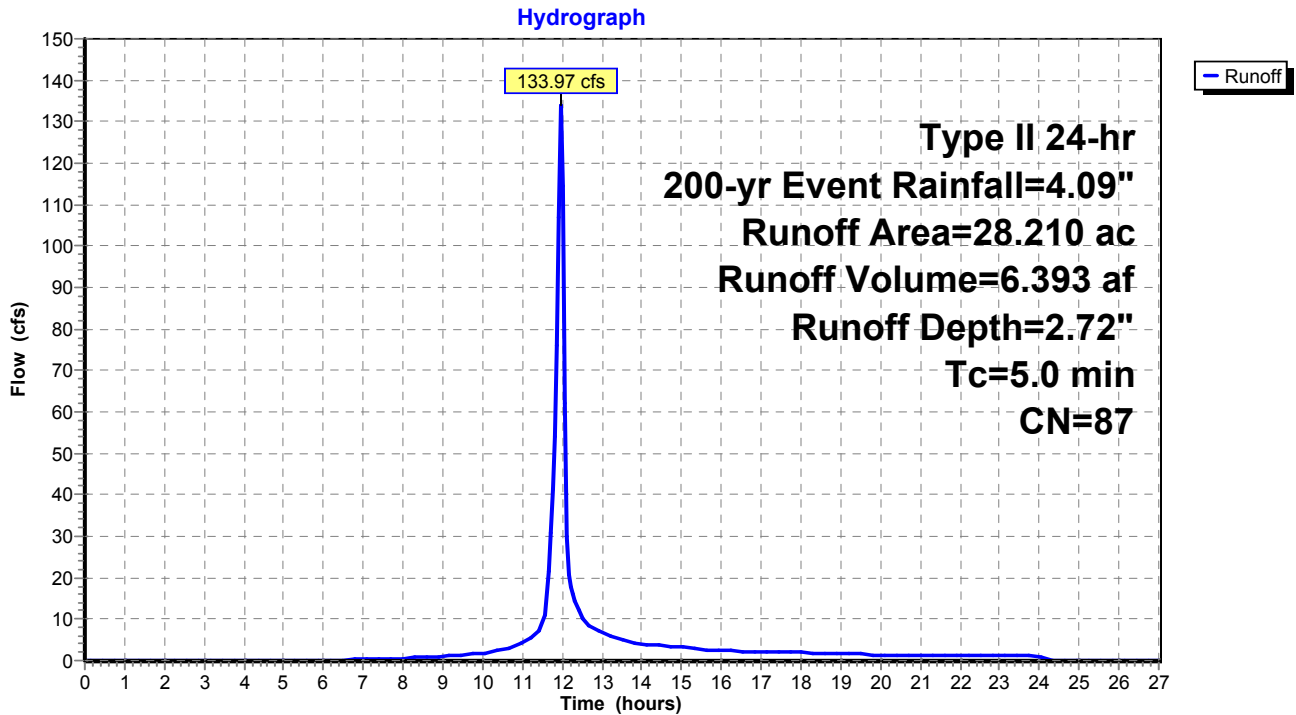
Runoff = 133.97 cfs @ 11.95 hrs, Volume= 6.393 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**





**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 2: WS 2**

Runoff = 415.35 cfs @ 12.02 hrs, Volume= 24.150 af, Depth= 2.72"

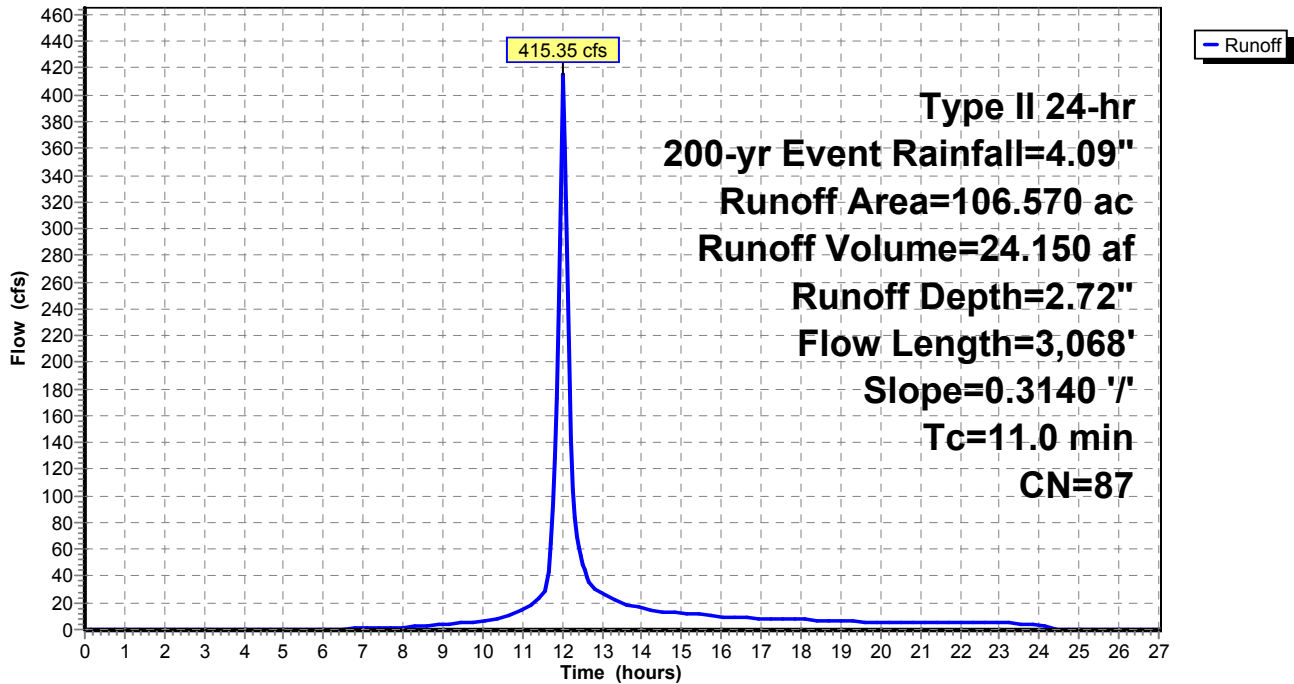
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 3: WS 3**

Runoff = 148.20 cfs @ 12.00 hrs, Volume= 7.929 af, Depth= 2.72"

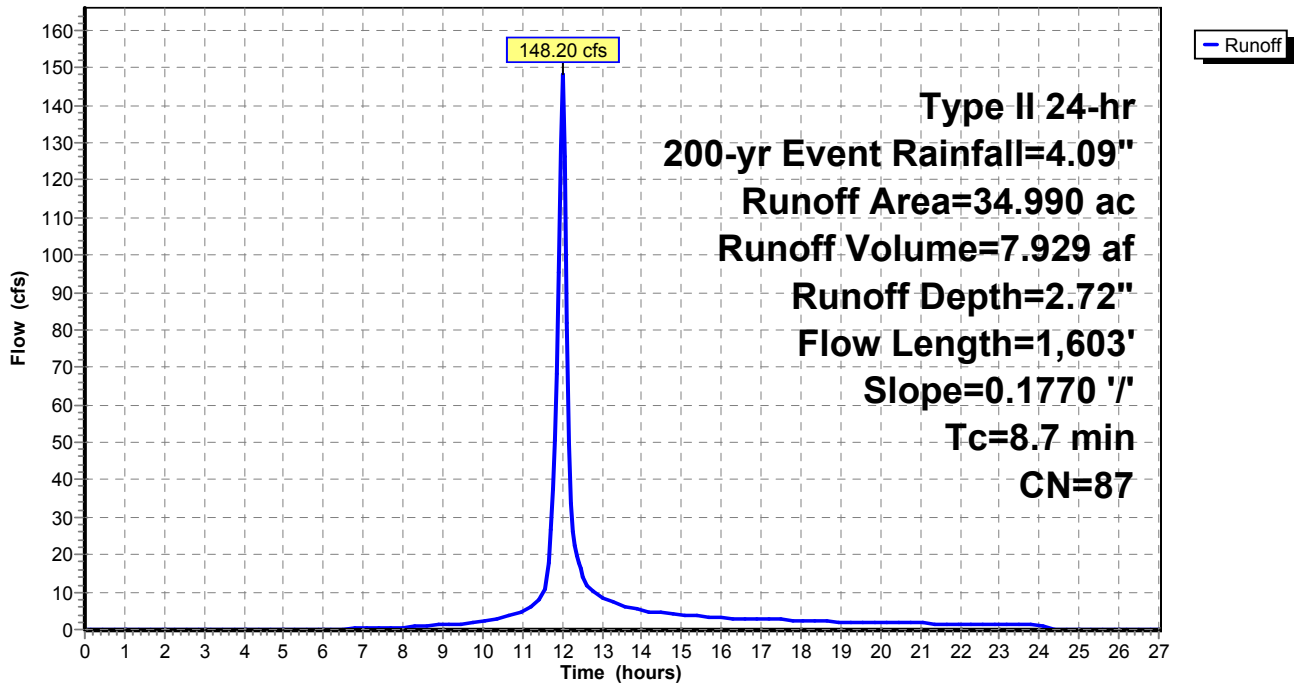
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 4: WS 4**

Runoff = 297.56 cfs @ 12.02 hrs, Volume= 17.000 af, Depth= 2.72"

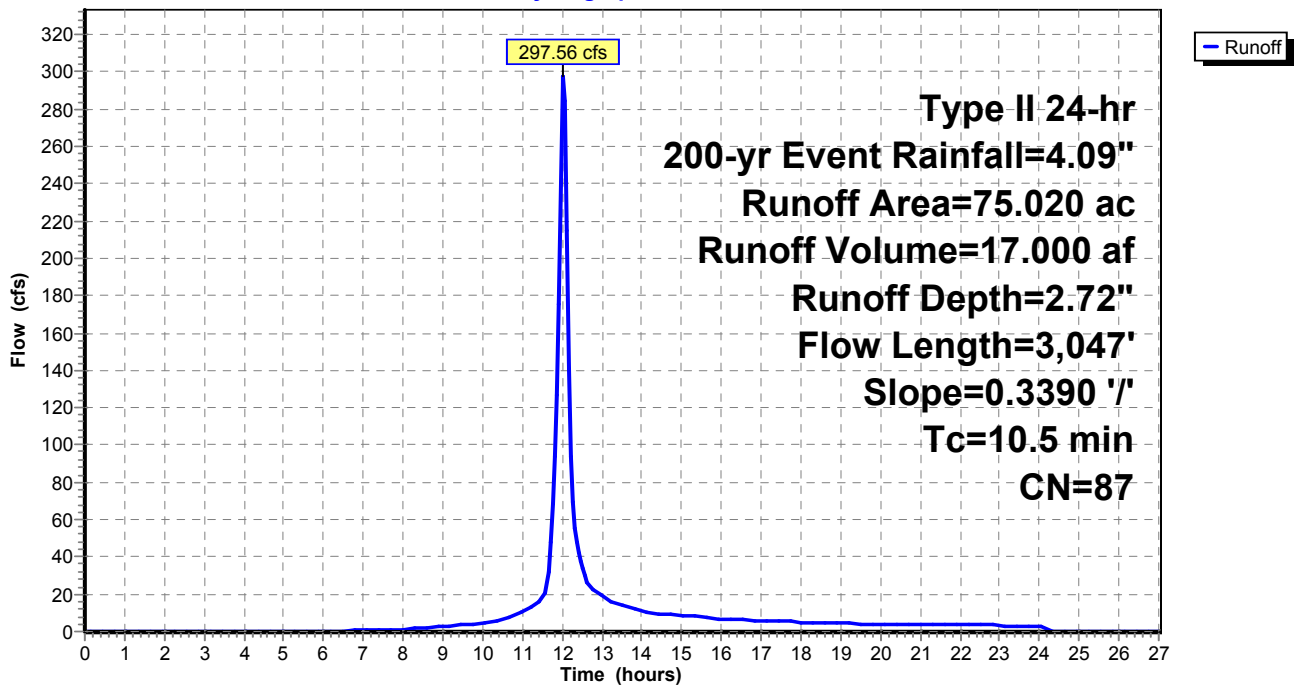
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 5: WS 5**

Runoff = 392.70 cfs @ 12.10 hrs, Volume= 28.143 af, Depth= 2.72"

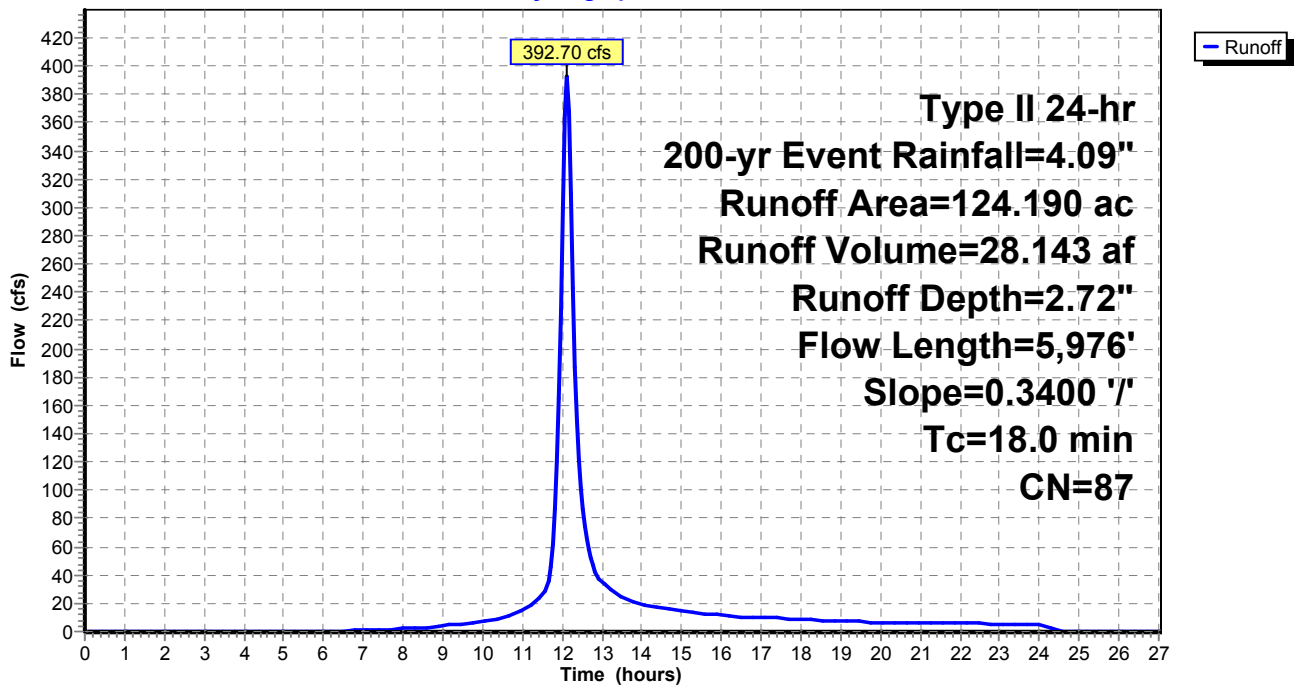
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 6: WS 6**

Runoff = 888.50 cfs @ 12.17 hrs, Volume= 75.058 af, Depth= 2.72"

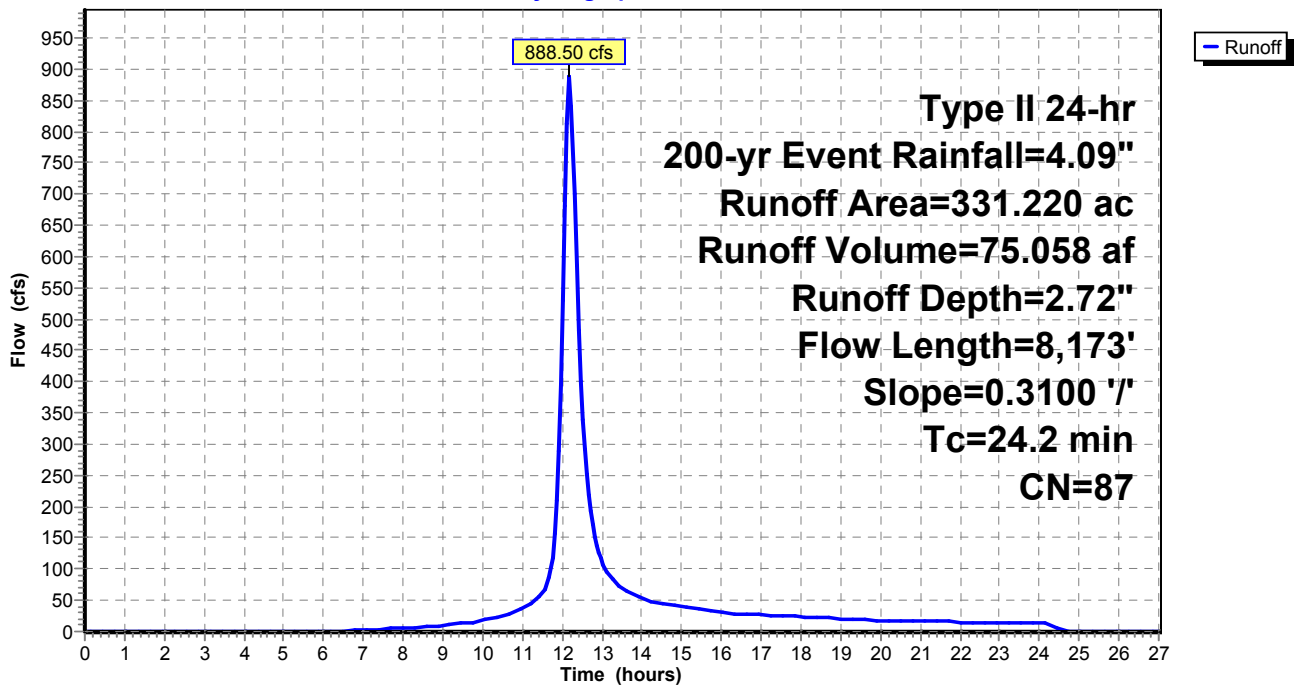
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 7: WS 7**

Runoff = 471.20 cfs @ 12.09 hrs, Volume= 32.739 af, Depth= 2.72"

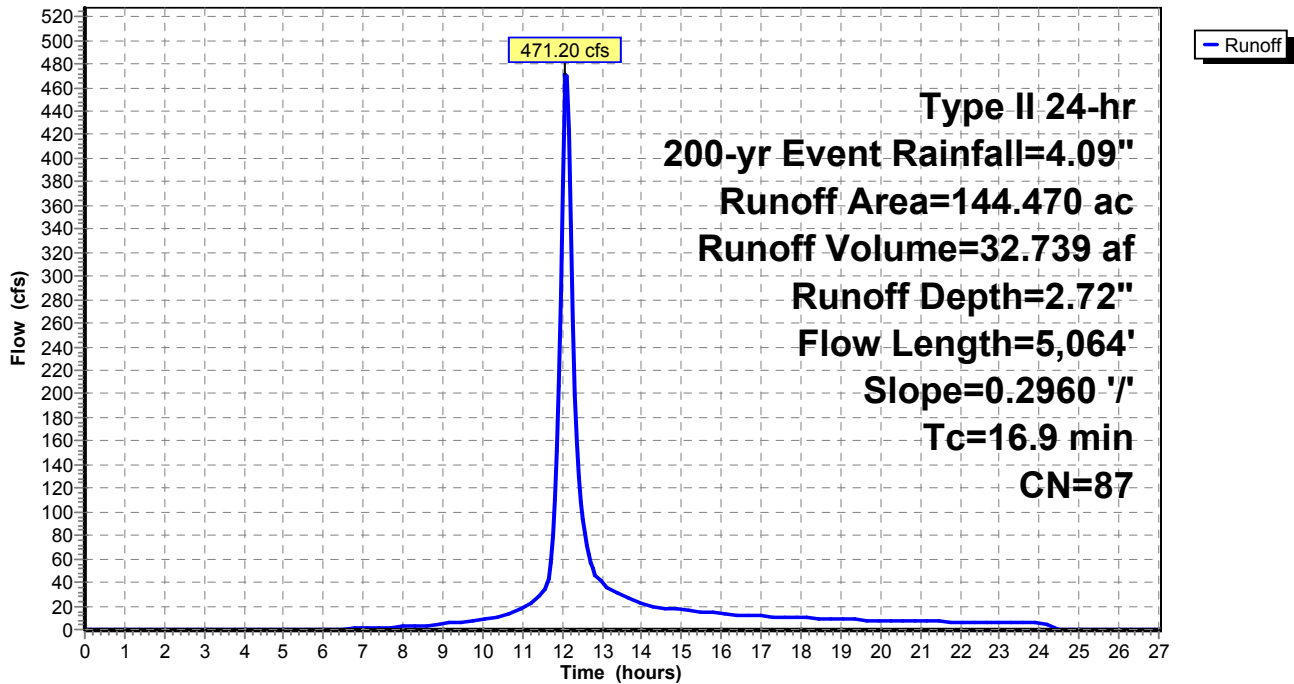
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 8: WS 8**

Runoff = 334.29 cfs @ 12.05 hrs, Volume= 20.851 af, Depth= 2.72"

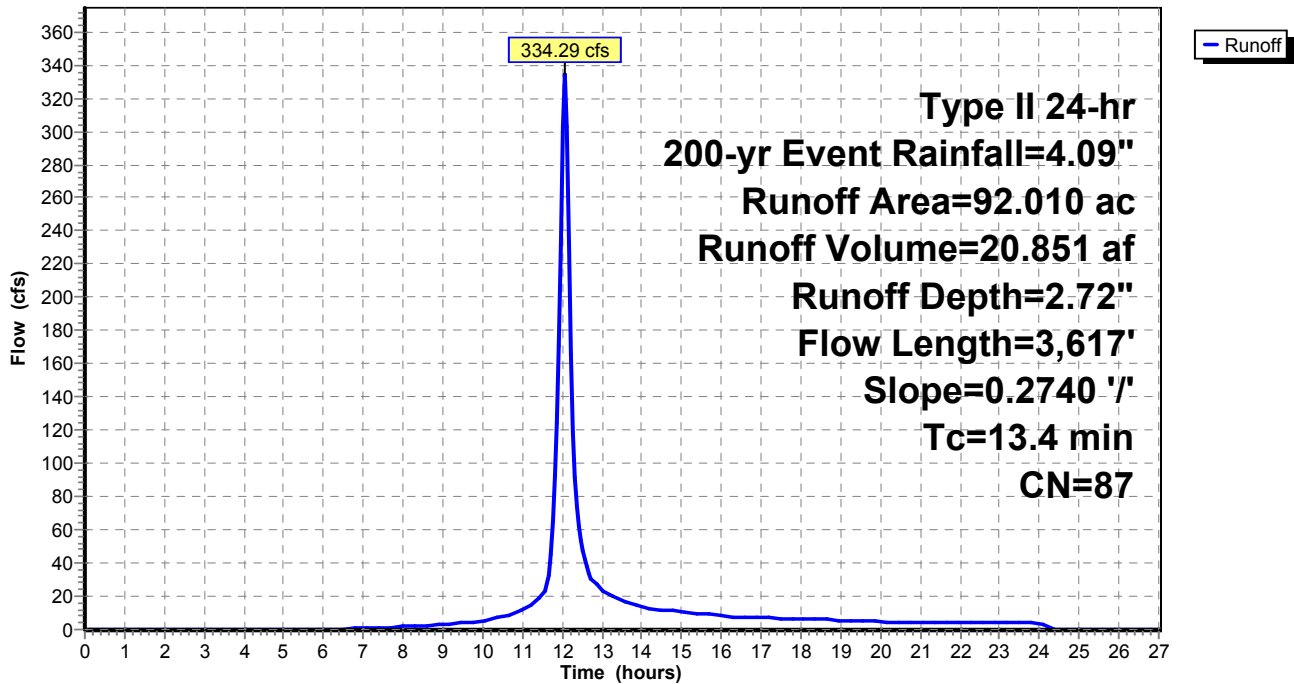
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 9: WS 9**

Runoff = 651.70 cfs @ 12.16 hrs, Volume= 53.460 af, Depth= 2.72"

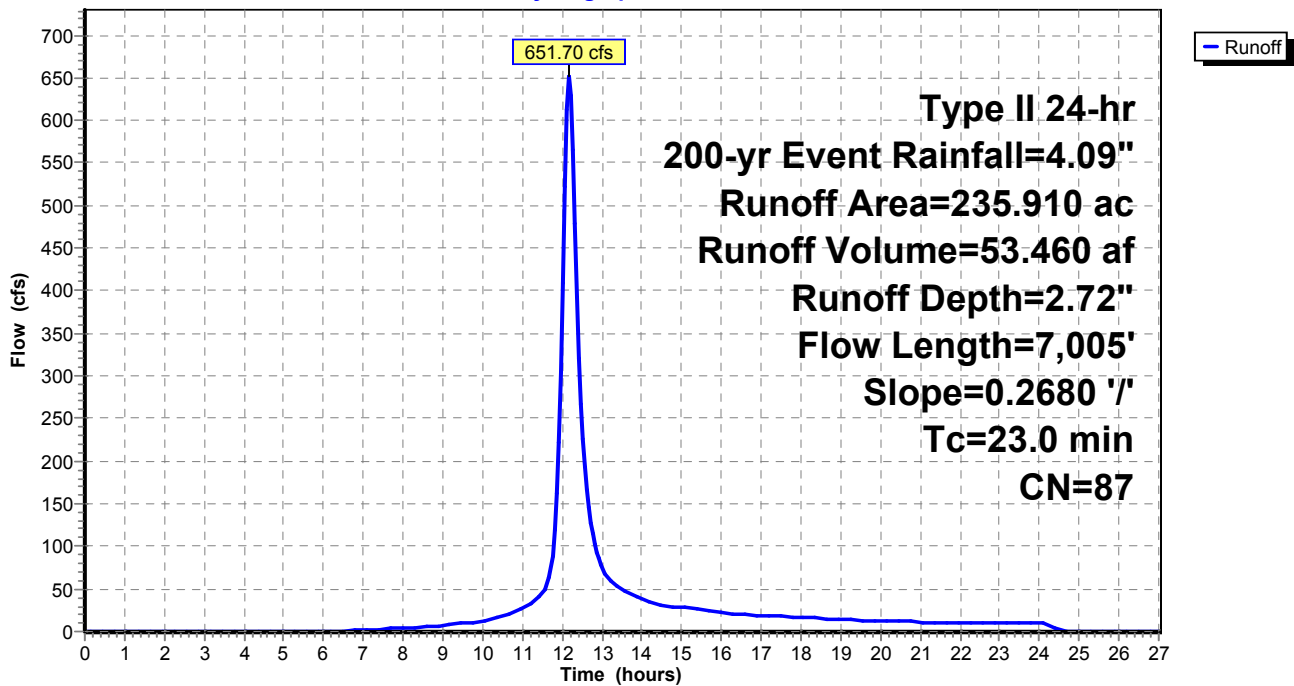
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**

Hydrograph





**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 10: WS 10**

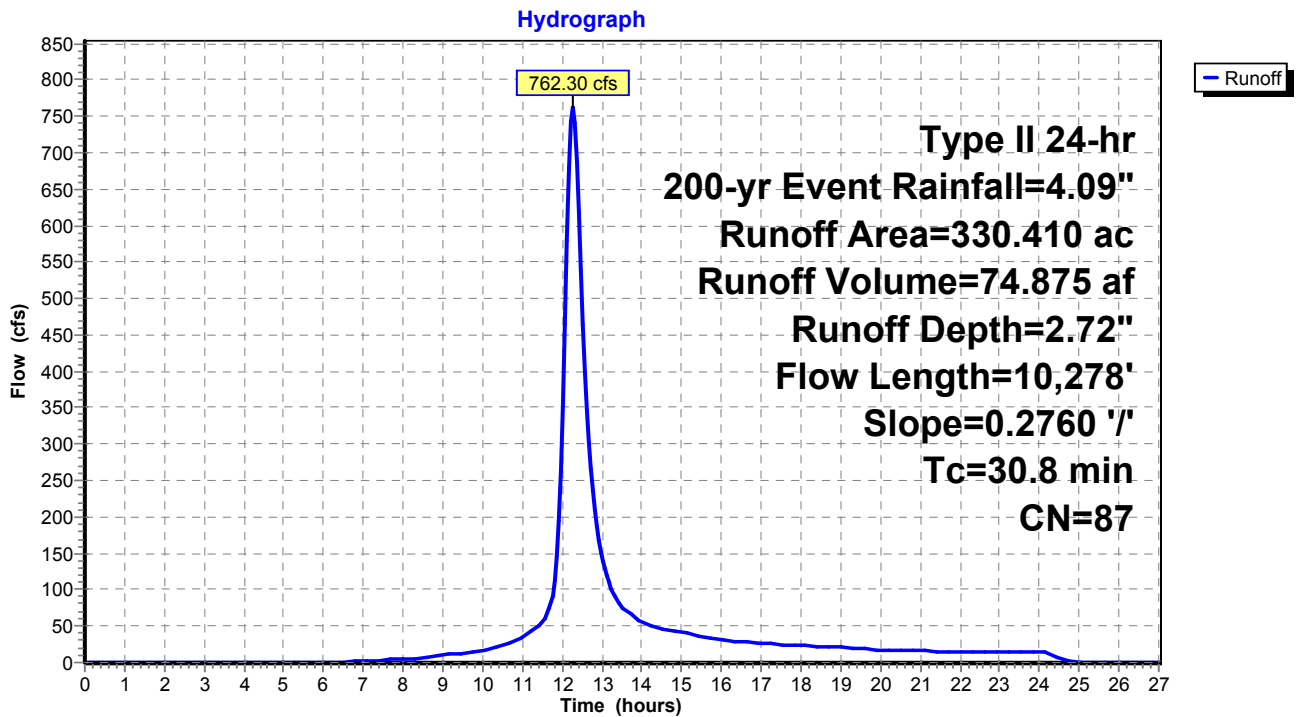
Runoff = 762.30 cfs @ 12.25 hrs, Volume= 74.875 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 11: WS 11**

Runoff = 968.42 cfs @ 12.22 hrs, Volume= 90.153 af, Depth= 2.72"

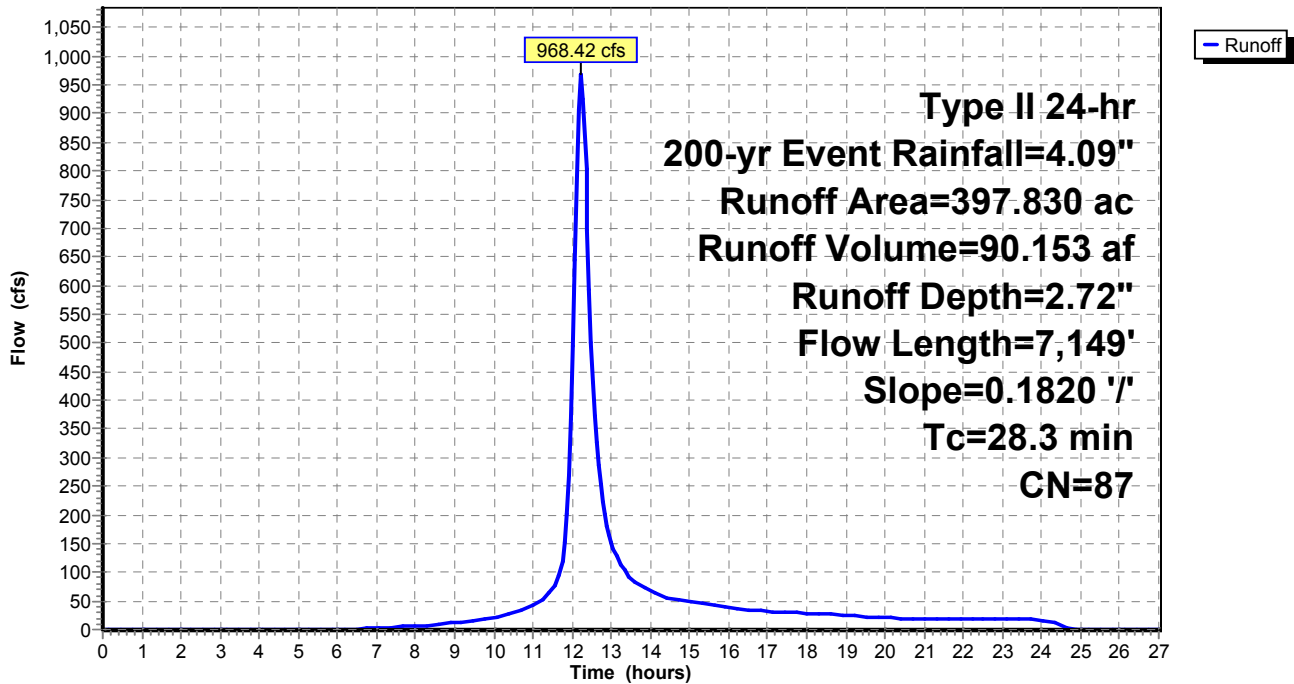
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 12: WS 12**

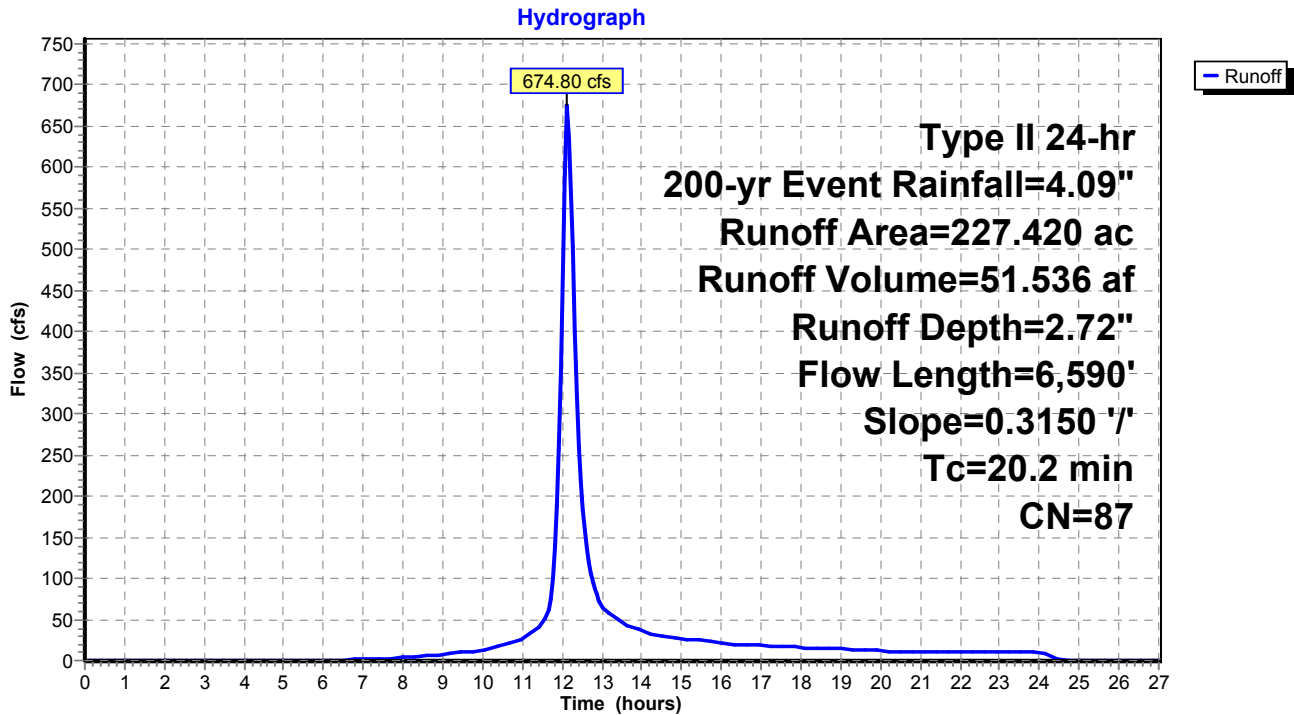
Runoff = 674.80 cfs @ 12.13 hrs, Volume= 51.536 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Subcatchment 13: WS 13**

Runoff = 745.85 cfs @ 12.17 hrs, Volume= 62.434 af, Depth= 2.72"

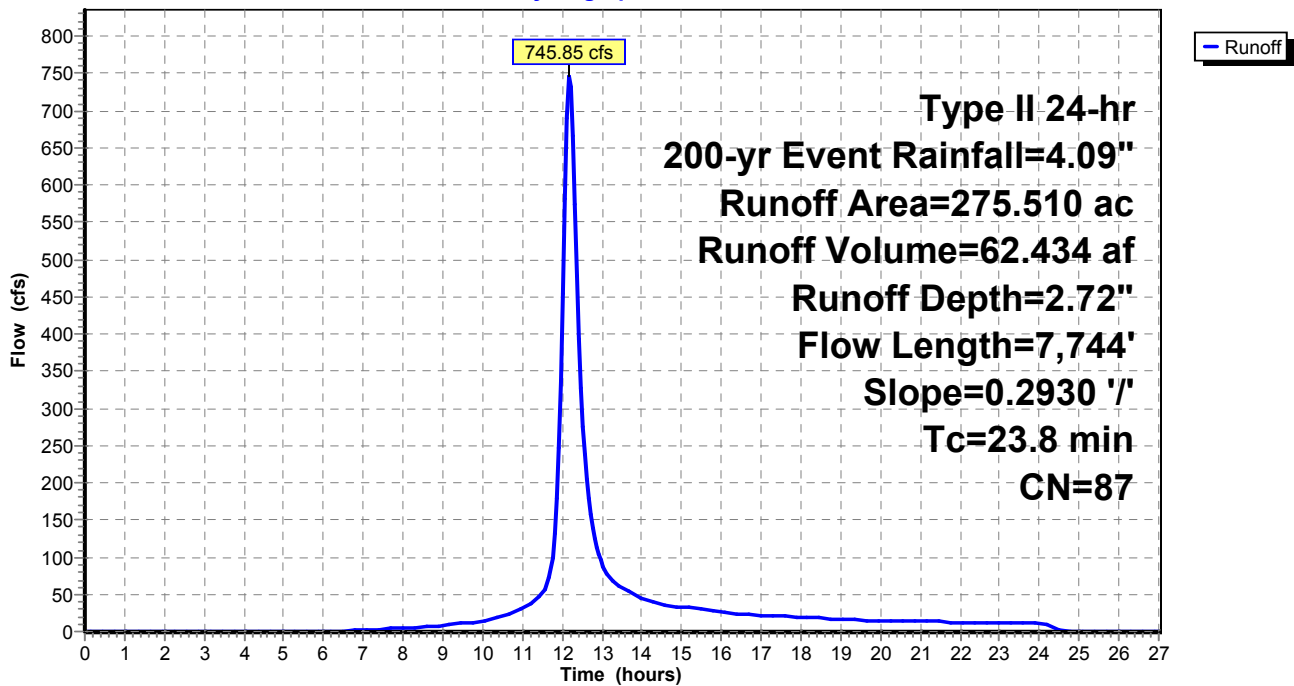
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 200-yr Event Rainfall=4.09"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Reach CHNL 1: Channel 1**

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 2.72" for 200-yr Event event  
 Inflow = 1,409.59 cfs @ 12.15 hrs, Volume= 113.970 af  
 Outflow = 1,364.28 cfs @ 12.24 hrs, Volume= 113.968 af, Atten= 3%, Lag= 5.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 9.34 fps, Min. Travel Time= 3.4 min  
 Avg. Velocity = 2.31 fps, Avg. Travel Time= 13.9 min

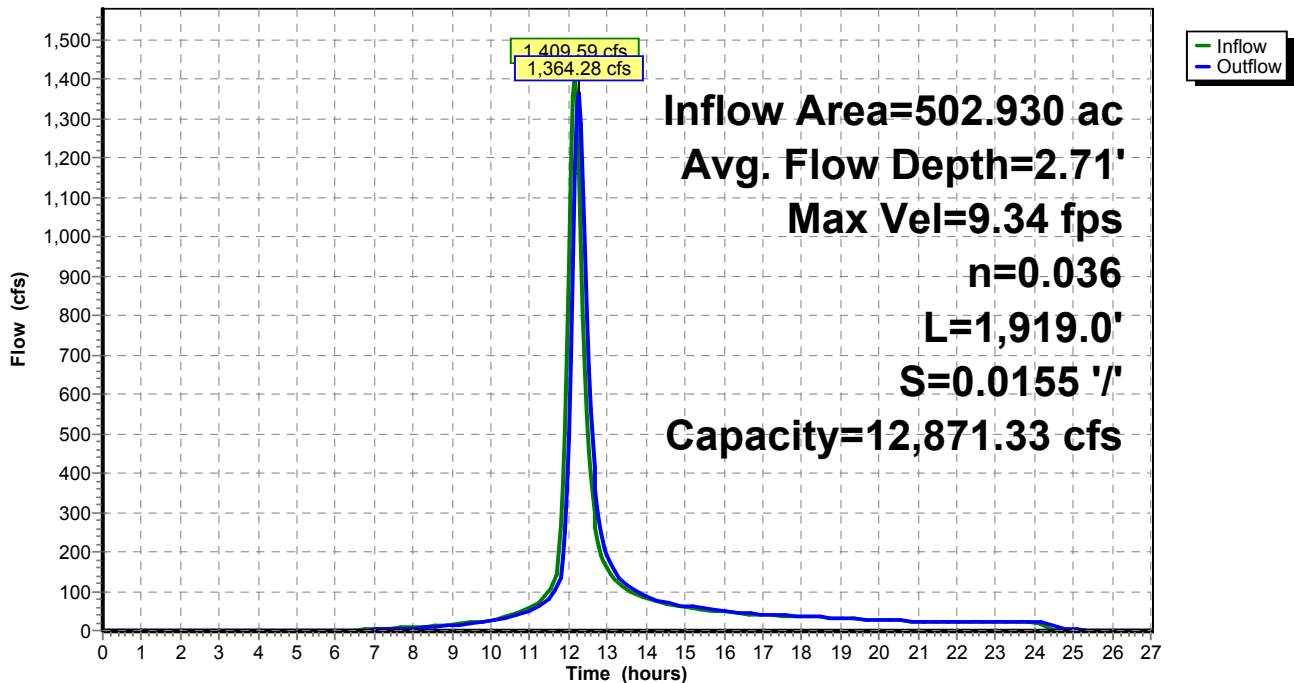
Peak Storage= 281,335 cf @ 12.18 hrs  
 Average Depth at Peak Storage= 2.71'  
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 ' / ' Top Width= 80.00'  
 Length= 1,919.0' Slope= 0.0155 ' / '  
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



**Reach CHNL 1: Channel 1**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Reach CHNL 2: Channel 2**

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 2.72" for 200-yr Event event  
 Inflow = 2,220.40 cfs @ 12.14 hrs, Volume= 197.202 af  
 Outflow = 1,835.68 cfs @ 12.47 hrs, Volume= 196.763 af, Atten= 17%, Lag= 20.0 min

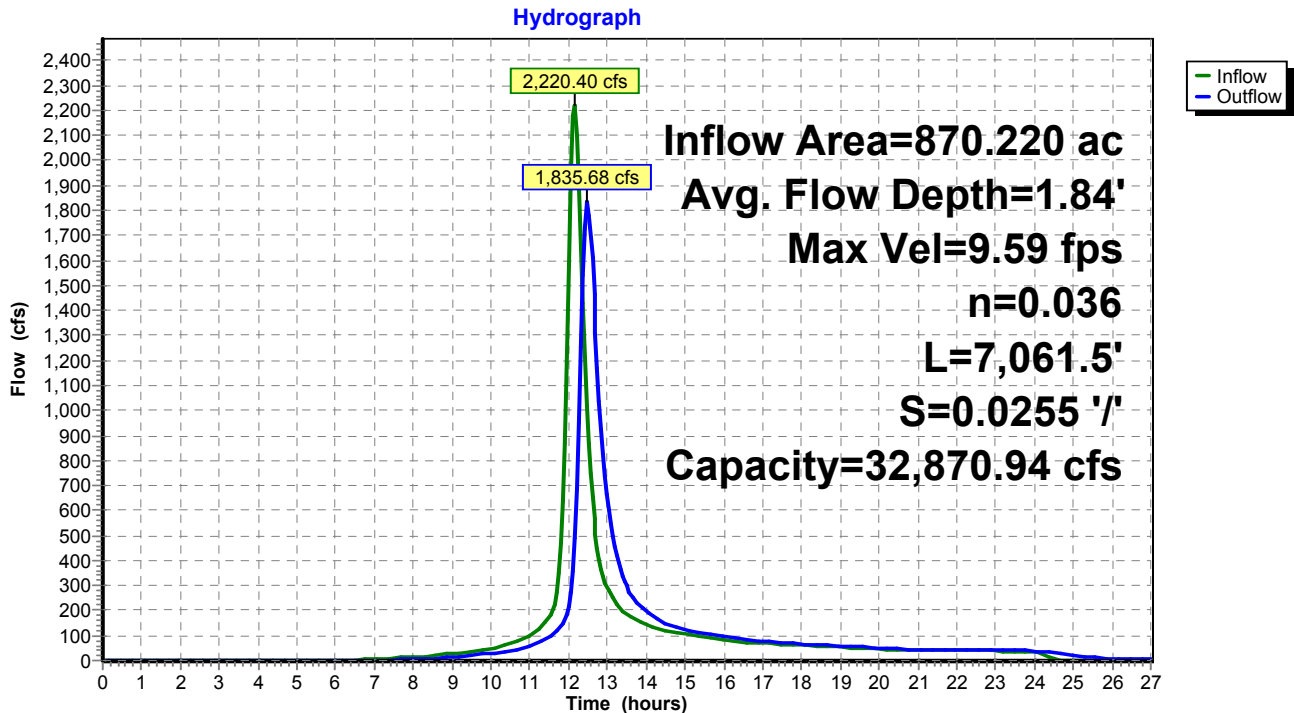
Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 9.59 fps, Min. Travel Time= 12.3 min  
 Avg. Velocity = 2.61 fps, Avg. Travel Time= 45.2 min

Peak Storage= 1,352,568 cf @ 12.27 hrs  
 Average Depth at Peak Storage= 1.84'  
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 2.5 2.0 ' / Top Width= 145.00'  
 Length= 7,061.5' Slope= 0.0255 ' /  
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



**Reach CHNL 2: Channel 2**



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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**Summary for Reach CHNL 3: Channel 3**

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 3.89' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 4.08' @ 12.40 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 2.72" for 200-yr Event event  
 Inflow = 4,230.35 cfs @ 12.29 hrs, Volume= 505.807 af  
 Outflow = 4,159.08 cfs @ 12.41 hrs, Volume= 505.394 af, Atten= 2%, Lag= 7.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 14.67 fps, Min. Travel Time= 4.2 min  
 Avg. Velocity = 4.37 fps, Avg. Travel Time= 14.1 min

Peak Storage= 1,046,926 cf @ 12.34 hrs  
 Average Depth at Peak Storage= 5.83'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 3,680.5' Slope= 0.0162 '/'  
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



### Exist WS (Post-Quintana)\_Culvert Analysis

Type II 24-hr 200-yr Event Rainfall=4.09"

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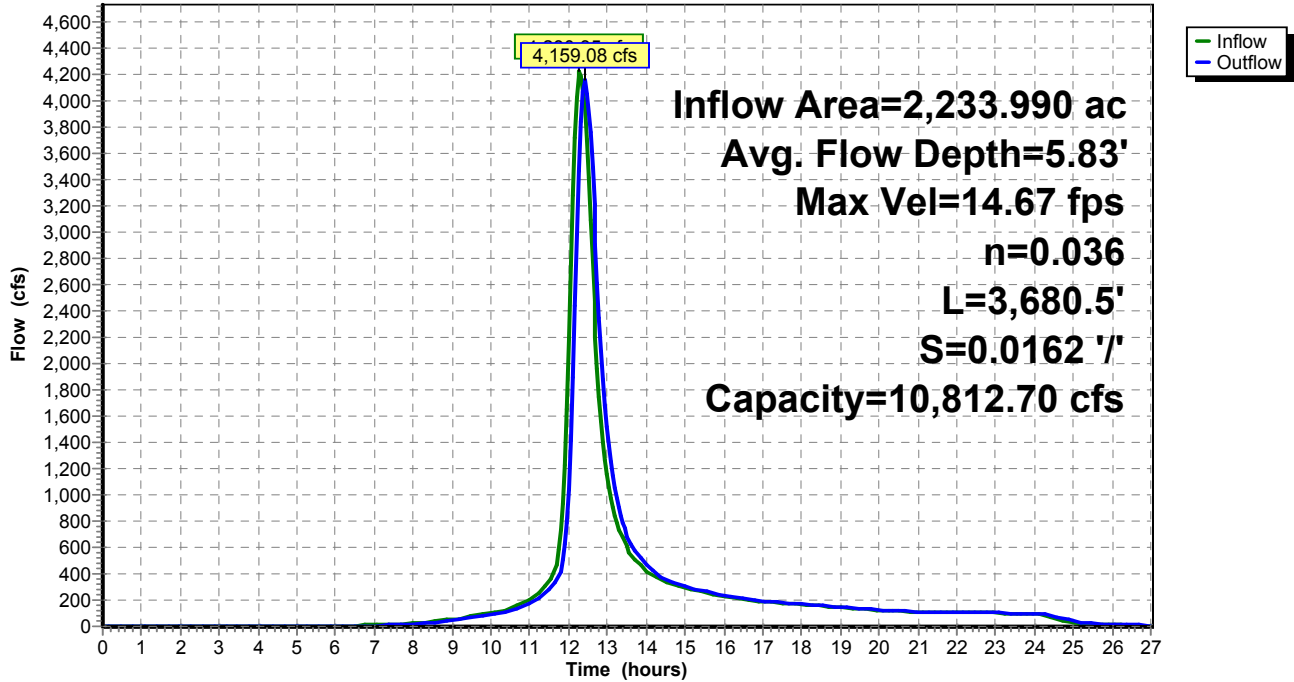
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### Reach CHNL 3: Channel 3

Hydrograph





**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Reach CHNL 4: Channel 4**

[61] Hint: Exceeded Reach CHNL 3 outlet invert by 4.10' @ 12.40 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event  
 Inflow = 4,176.49 cfs @ 12.41 hrs, Volume= 513.323 af  
 Outflow = 4,160.94 cfs @ 12.43 hrs, Volume= 513.234 af, Atten= 0%, Lag= 1.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 22.02 fps, Min. Travel Time= 0.8 min  
 Avg. Velocity = 6.44 fps, Avg. Travel Time= 2.8 min

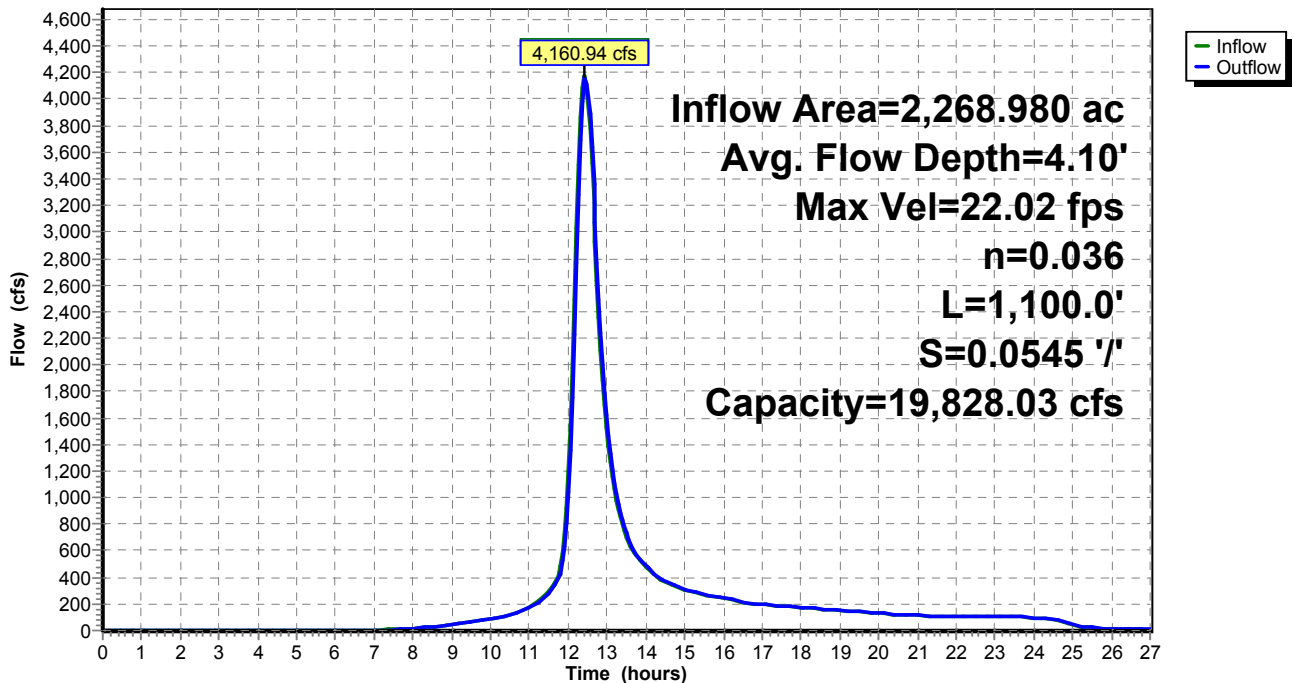
Peak Storage= 208,374 cf @ 12.42 hrs  
 Average Depth at Peak Storage= 4.10'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 1,100.0' Slope= 0.0545 '/'  
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



**Reach CHNL 4: Channel 4**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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**Summary for Reach CHNL 5: Channel 5**

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.66' @ 12.50 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event  
 Inflow = 4,217.67 cfs @ 12.43 hrs, Volume= 537.384 af  
 Outflow = 4,191.95 cfs @ 12.49 hrs, Volume= 537.163 af, Atten= 1%, Lag= 3.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 17.83 fps, Min. Travel Time= 2.0 min  
 Avg. Velocity = 5.77 fps, Avg. Travel Time= 6.2 min

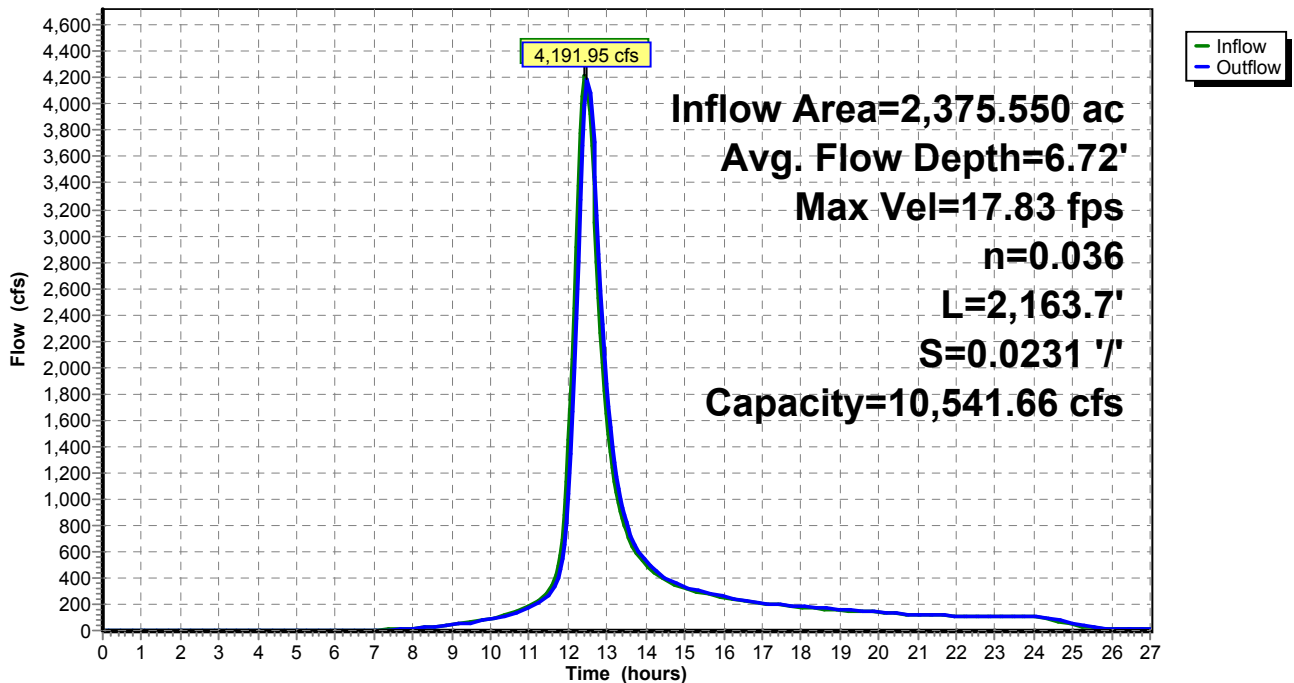
Peak Storage= 510,334 cf @ 12.46 hrs  
 Average Depth at Peak Storage= 6.72'  
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 58.00'  
 Length= 2,163.7' Slope= 0.0231 '/'  
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



**Reach CHNL 5: Channel 5**

**Hydrograph**



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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**Summary for Pond CLVT 1: Culvert Crossing #1**

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 17.28' @ 12.75 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event  
 Inflow = 4,191.95 cfs @ 12.49 hrs, Volume= 537.163 af  
 Outflow = 3,410.14 cfs @ 12.71 hrs, Volume= 537.020 af, Atten= 19%, Lag= 12.9 min  
 Primary = 3,410.14 cfs @ 12.71 hrs, Volume= 537.020 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,392.92' @ 12.71 hrs Surf.Area= 0 sf Storage= 1,758,861 cf  
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 4.1 min calculated for 537.020 af (100% of inflow)  
 Center-of-Mass det. time= 4.0 min ( 860.7 - 856.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	<b>Culvert Crossing #1</b> Listed below

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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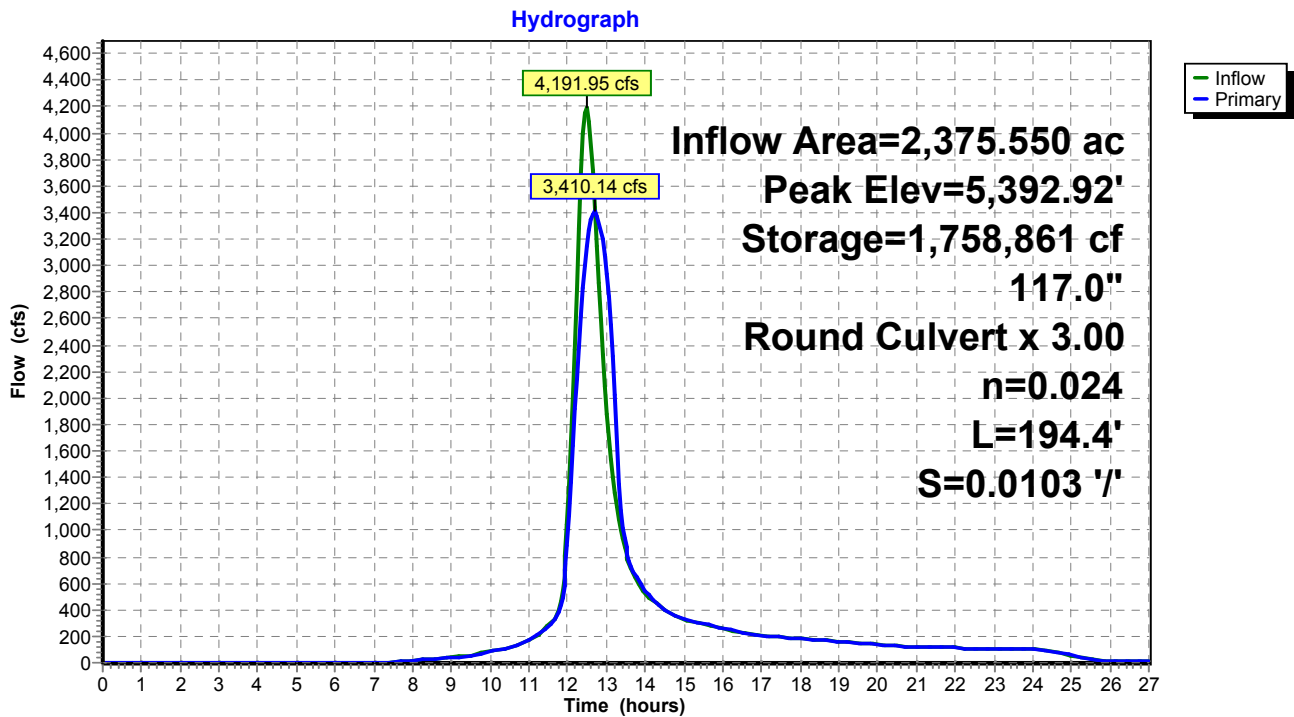
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	<b>117.0" Round Culvert X 3.00</b> L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=3,408.57 cfs @ 12.71 hrs HW=5,392.90' (Free Discharge)  
 ↳ **1=Culvert** (Inlet Controls 3,408.57 cfs @ 15.22 fps)

**Pond CLVT 1: Culvert Crossing #1**



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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**Summary for Pond CLVT 2: Culvert Crossing #2**

[81] Warning: Exceeded Pond CLVT 1 by 11.78' @ 13.40 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 2.71" for 200-yr Event event  
 Inflow = 3,418.33 cfs @ 12.71 hrs, Volume= 543.413 af  
 Outflow = 1,658.81 cfs @ 13.25 hrs, Volume= 543.361 af, Atten= 51%, Lag= 32.8 min  
 Primary = 1,658.81 cfs @ 13.25 hrs, Volume= 543.361 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,391.04' @ 13.25 hrs Surf.Area= 0 sf Storage= 6,444,314 cf  
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 30.6 min calculated for 543.361 af (100% of inflow)  
 Center-of-Mass det. time= 30.5 min ( 890.6 - 860.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	<b>Existing Pond</b> Listed below

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 200-yr Event Rainfall=4.09"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 200-yr Event Rainfall=4.09"

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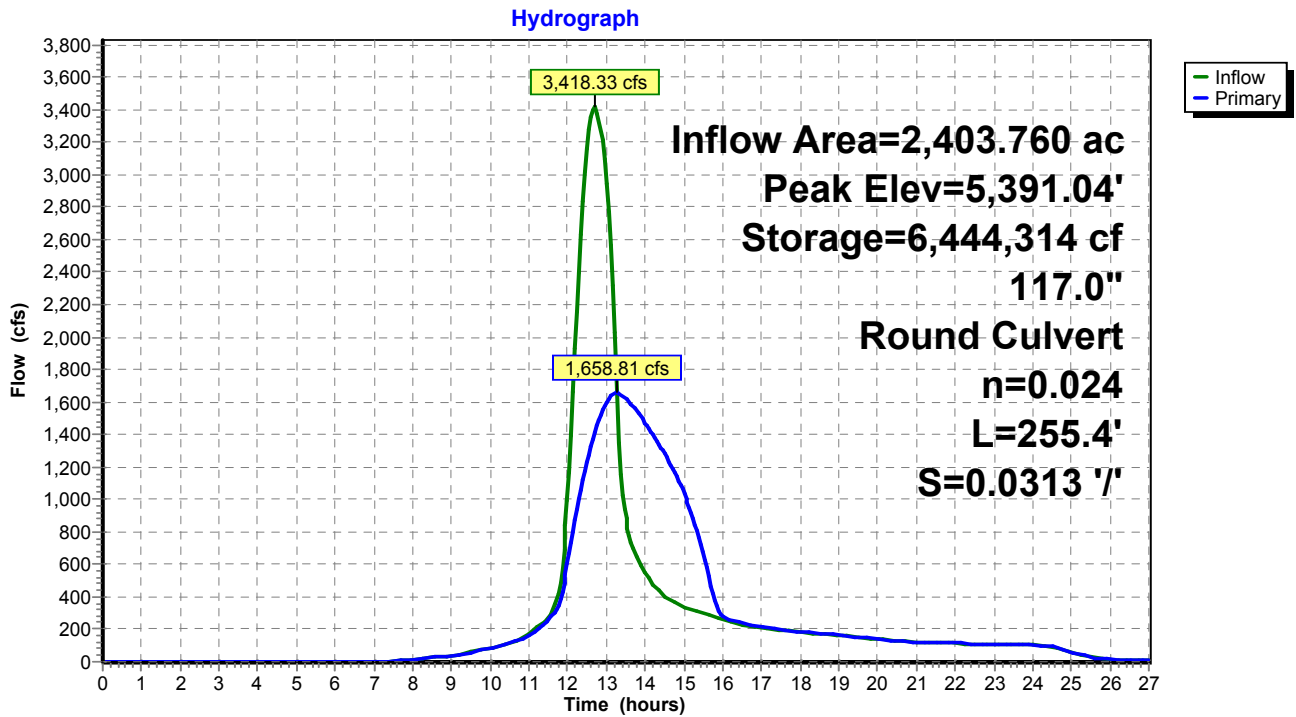
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5,402.00      9,814,540  
 5,403.00      10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	<b>117.0" Round Culvert</b> L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=1,658.65 cfs @ 13.25 hrs HW=5,391.03' (Free Discharge)  
 ←**1=Culvert** (Inlet Controls 1,658.65 cfs @ 22.22 fps)

**Pond CLVT 2: Culvert Crossing #2**





**Exist WS (Post-Quintana)\_Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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Time span=0.00-27.00 hrs, dt=0.05 hrs, 541 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment1: WS 1</b>	Runoff Area=28.210 ac 10.00% Impervious Runoff Depth=3.19" Tc=5.0 min CN=87 Runoff=155.94 cfs 7.504 af
<b>Subcatchment2: WS 2</b>	Runoff Area=106.570 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,068' Slope=0.3140 '/' Tc=11.0 min CN=87 Runoff=484.31 cfs 28.349 af
<b>Subcatchment3: WS 3</b>	Runoff Area=34.990 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=1,603' Slope=0.1770 '/' Tc=8.7 min CN=87 Runoff=172.66 cfs 9.308 af
<b>Subcatchment4: WS 4</b>	Runoff Area=75.020 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,047' Slope=0.3390 '/' Tc=10.5 min CN=87 Runoff=346.92 cfs 19.956 af
<b>Subcatchment5: WS 5</b>	Runoff Area=124.190 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=5,976' Slope=0.3400 '/' Tc=18.0 min CN=87 Runoff=458.61 cfs 33.036 af
<b>Subcatchment6: WS 6</b>	Runoff Area=331.220 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=8,173' Slope=0.3100 '/' Tc=24.2 min CN=87 Runoff=1,038.74 cfs 88.109 af
<b>Subcatchment7: WS 7</b>	Runoff Area=144.470 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=5,064' Slope=0.2960 '/' Tc=16.9 min CN=87 Runoff=550.14 cfs 38.431 af
<b>Subcatchment8: WS 8</b>	Runoff Area=92.010 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=3,617' Slope=0.2740 '/' Tc=13.4 min CN=87 Runoff=389.99 cfs 24.476 af
<b>Subcatchment9: WS 9</b>	Runoff Area=235.910 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,005' Slope=0.2680 '/' Tc=23.0 min CN=87 Runoff=761.76 cfs 62.755 af
<b>Subcatchment10: WS 10</b>	Runoff Area=330.410 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=10,278' Slope=0.2760 '/' Tc=30.8 min CN=87 Runoff=891.85 cfs 87.894 af
<b>Subcatchment11: WS 11</b>	Runoff Area=397.830 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,149' Slope=0.1820 '/' Tc=28.3 min CN=87 Runoff=1,132.73 cfs 105.828 af
<b>Subcatchment12: WS 12</b>	Runoff Area=227.420 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=6,590' Slope=0.3150 '/' Tc=20.2 min CN=87 Runoff=789.72 cfs 60.497 af
<b>Subcatchment13: WS 13</b>	Runoff Area=275.510 ac 10.00% Impervious Runoff Depth=3.19" Flow Length=7,744' Slope=0.2930 '/' Tc=23.8 min CN=87 Runoff=871.93 cfs 73.289 af
<b>Reach CHNL 1: Channel 1</b>	Avg. Flow Depth=2.98' Max Vel=9.88 fps Inflow=1,647.46 cfs 133.786 af n=0.036 L=1,919.0' S=0.0155 '/' Capacity=12,871.33 cfs Outflow=1,601.86 cfs 133.784 af
<b>Reach CHNL 2: Channel 2</b>	Avg. Flow Depth=2.04' Max Vel=10.26 fps Inflow=2,597.10 cfs 231.491 af n=0.036 L=7,061.5' S=0.0255 '/' Capacity=32,870.94 cfs Outflow=2,180.53 cfs 231.028 af
<b>Reach CHNL 3: Channel 3</b>	Avg. Flow Depth=6.49' Max Vel=15.55 fps Inflow=5,094.93 cfs 593.807 af n=0.036 L=3,680.5' S=0.0162 '/' Capacity=10,812.70 cfs Outflow=5,008.23 cfs 593.385 af

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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**Reach CHNL 4: Channel 4** Avg. Flow Depth=4.57' Max Vel=23.45 fps Inflow=5,028.84 cfs 602.692 af  
 n=0.036 L=1,100.0' S=0.0545 '/ Capacity=19,828.03 cfs Outflow=5,012.38 cfs 602.599 af

**Reach CHNL 5: Channel 5** Avg. Flow Depth=7.44' Max Vel=18.81 fps Inflow=5,080.00 cfs 630.949 af  
 n=0.036 L=2,163.7' S=0.0231 '/ Capacity=10,541.66 cfs Outflow=5,046.09 cfs 630.724 af

**Pond CLVT 1: Culvert Crossing** Peak Elev=5,397.29' Storage=2,570,184 cf Inflow=5,046.09 cfs 630.724 af  
 117.0" Round Culvert x 3.00 n=0.024 L=194.4' S=0.0103 '/ Outflow=3,847.35 cfs 630.580 af

**Pond CLVT 2: Culvert Crossing** Peak Elev=5,396.71' Storage=8,137,086 cf Inflow=3,856.73 cfs 638.084 af  
 117.0" Round Culvert n=0.024 L=255.4' S=0.0313 '/ Outflow=1,791.19 cfs 638.031 af

**Total Runoff Area = 2,403.760 ac Runoff Volume = 639.433 af Average Runoff Depth = 3.19"**  
**90.00% Pervious = 2,163.390 ac 10.00% Impervious = 240.370 ac**

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 1: WS 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 155.94 cfs @ 11.95 hrs, Volume= 7.504 af, Depth= 3.19"

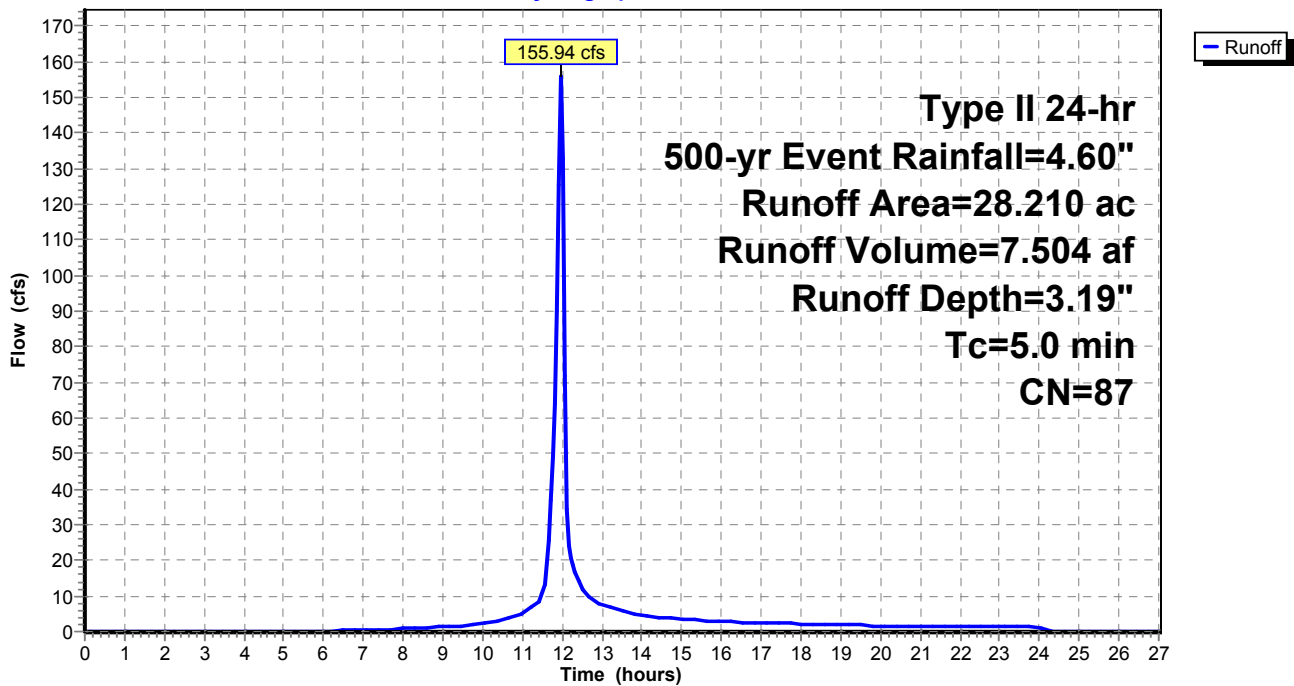
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
25.390	86	Desert shrub range, Fair, HSG D
* 2.820	98	Impervious, HSG D
28.210	87	Weighted Average
25.390		90.00% Pervious Area
2.820		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

**Subcatchment 1: WS 1**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 2: WS 2**

Runoff = 484.31 cfs @ 12.02 hrs, Volume= 28.349 af, Depth= 3.19"

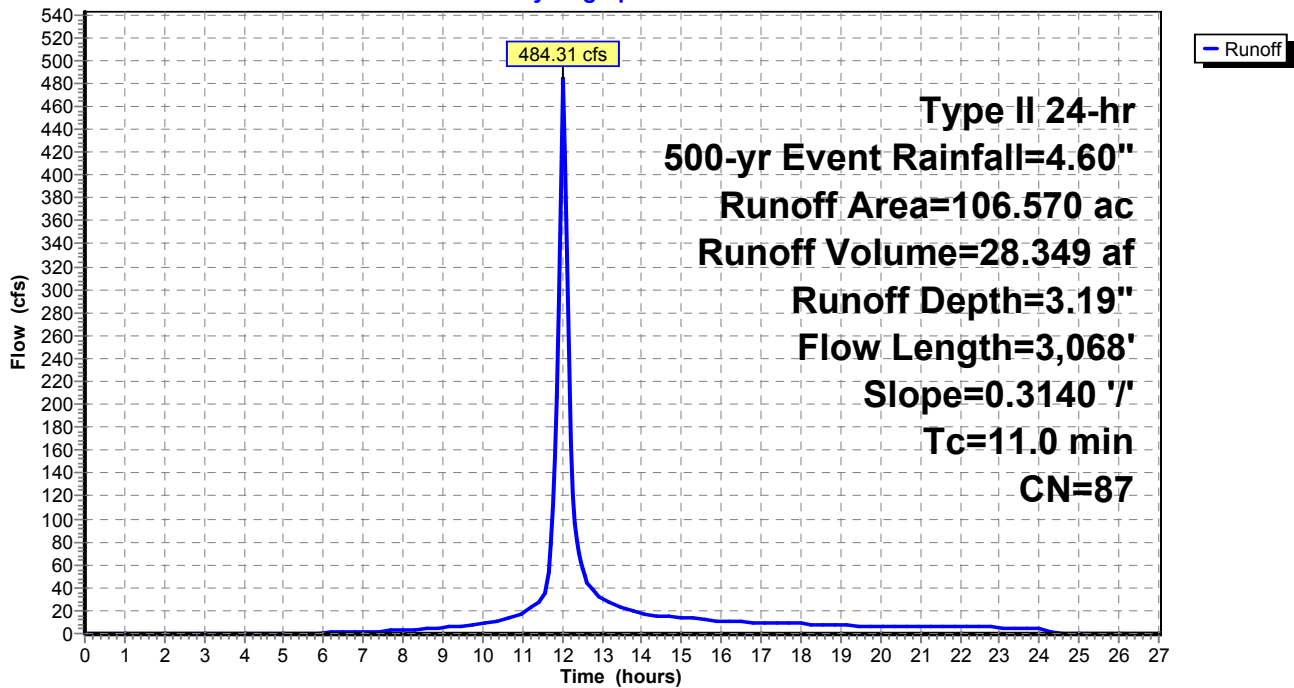
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
95.910	86	Desert shrub range, Fair, HSG D
* 10.660	98	Impervious, HSG D
106.570	87	Weighted Average
95.910		90.00% Pervious Area
10.660		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	3,068	0.3140	4.66		Lag/CN Method,

**Subcatchment 2: WS 2**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 3: WS 3**

Runoff = 172.66 cfs @ 12.00 hrs, Volume= 9.308 af, Depth= 3.19"

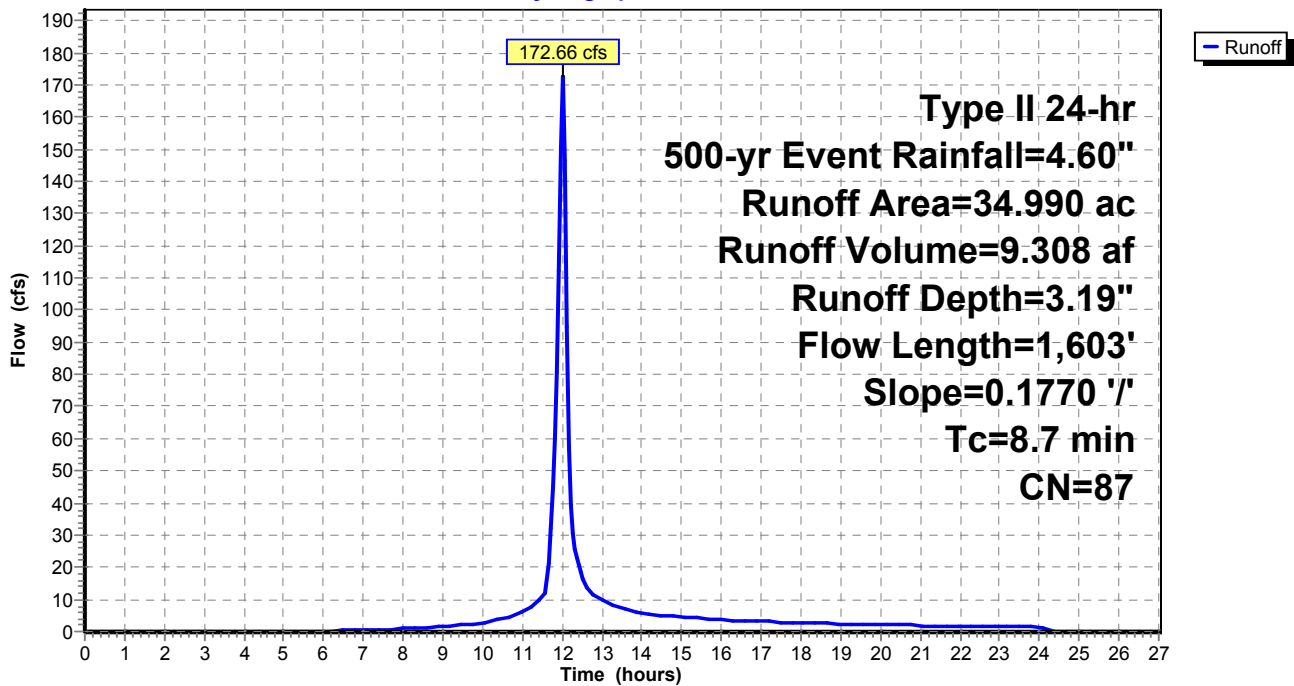
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
31.490	86	Desert shrub range, Fair, HSG D
* 3.500	98	Impervious, HSG D
34.990	87	Weighted Average
31.490		90.00% Pervious Area
3.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	1,603	0.1770	3.07		Lag/CN Method,

**Subcatchment 3: WS 3**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 4: WS 4**

Runoff = 346.92 cfs @ 12.02 hrs, Volume= 19.956 af, Depth= 3.19"

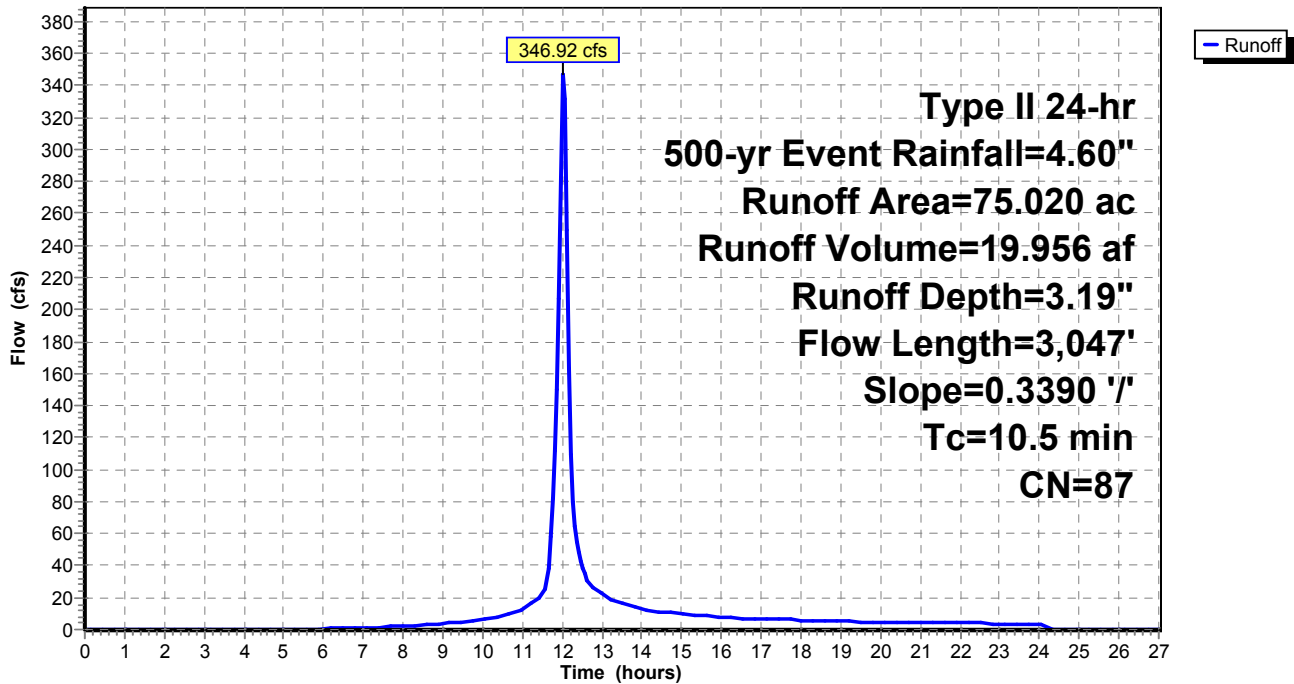
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
67.520	86	Desert shrub range, Fair, HSG D
* 7.500	98	Impervious, HSG D
75.020	87	Weighted Average
67.520		90.00% Pervious Area
7.500		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	3,047	0.3390	4.84		Lag/CN Method,

**Subcatchment 4: WS 4**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 5: WS 5**

Runoff = 458.61 cfs @ 12.10 hrs, Volume= 33.036 af, Depth= 3.19"

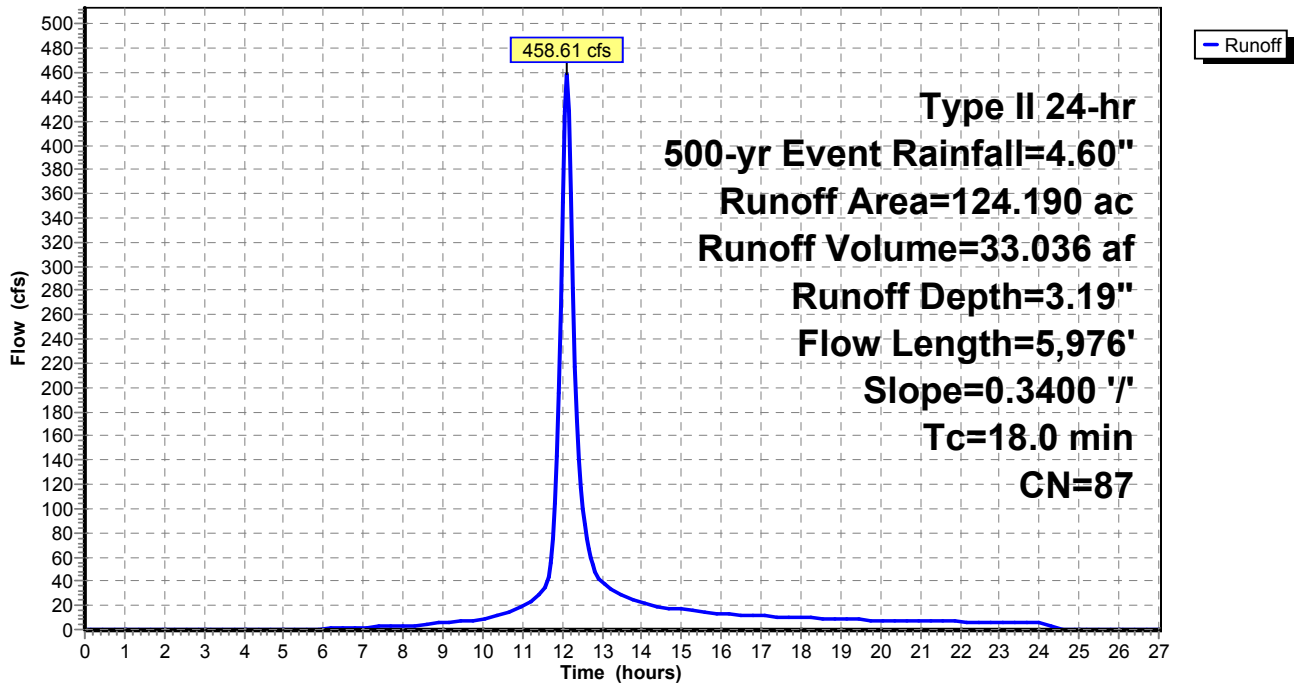
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
111.770	86	Desert shrub range, Fair, HSG D
* 12.420	98	Impervious, HSG D
124.190	87	Weighted Average
111.770		90.00% Pervious Area
12.420		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0	5,976	0.3400	5.54		Lag/CN Method,

**Subcatchment 5: WS 5**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 6: WS 6**

Runoff = 1,038.74 cfs @ 12.17 hrs, Volume= 88.109 af, Depth= 3.19"

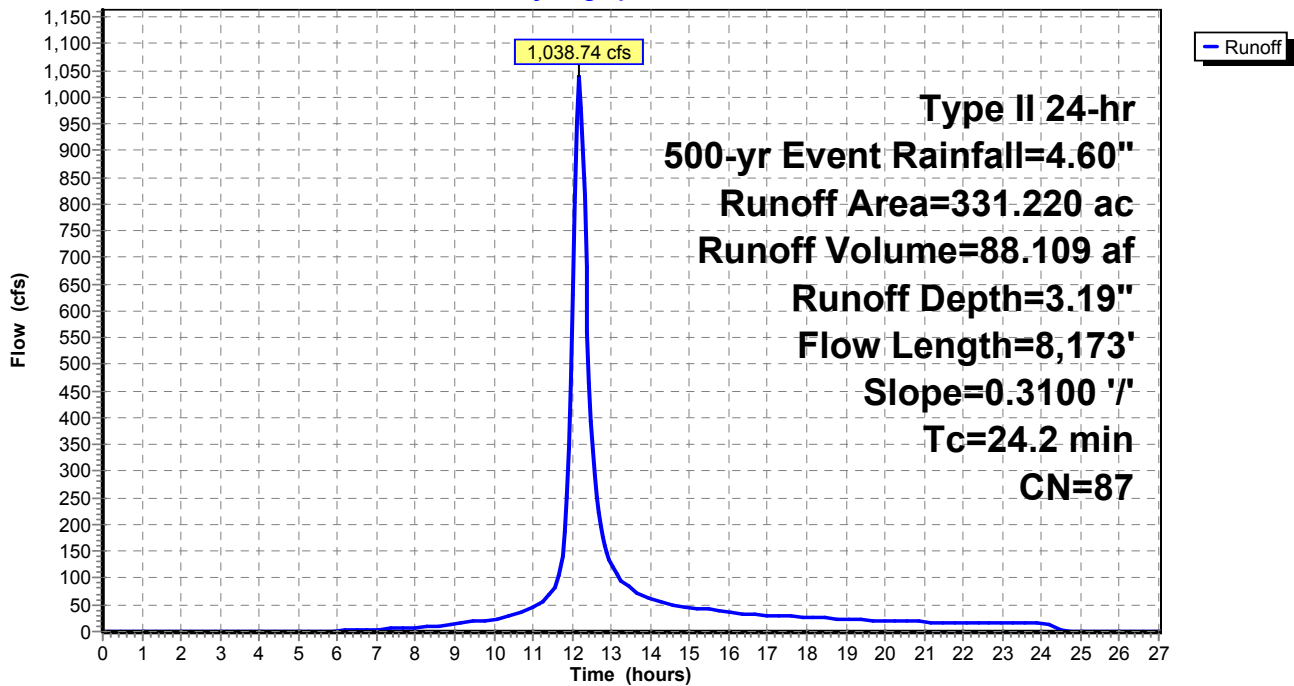
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
298.100	86	Desert shrub range, Fair, HSG D
* 33.120	98	Impervious, HSG D
331.220	87	Weighted Average
298.100		90.00% Pervious Area
33.120		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	8,173	0.3100	5.64		Lag/CN Method,

**Subcatchment 6: WS 6**

Hydrograph





**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 7: WS 7**

Runoff = 550.14 cfs @ 12.09 hrs, Volume= 38.431 af, Depth= 3.19"

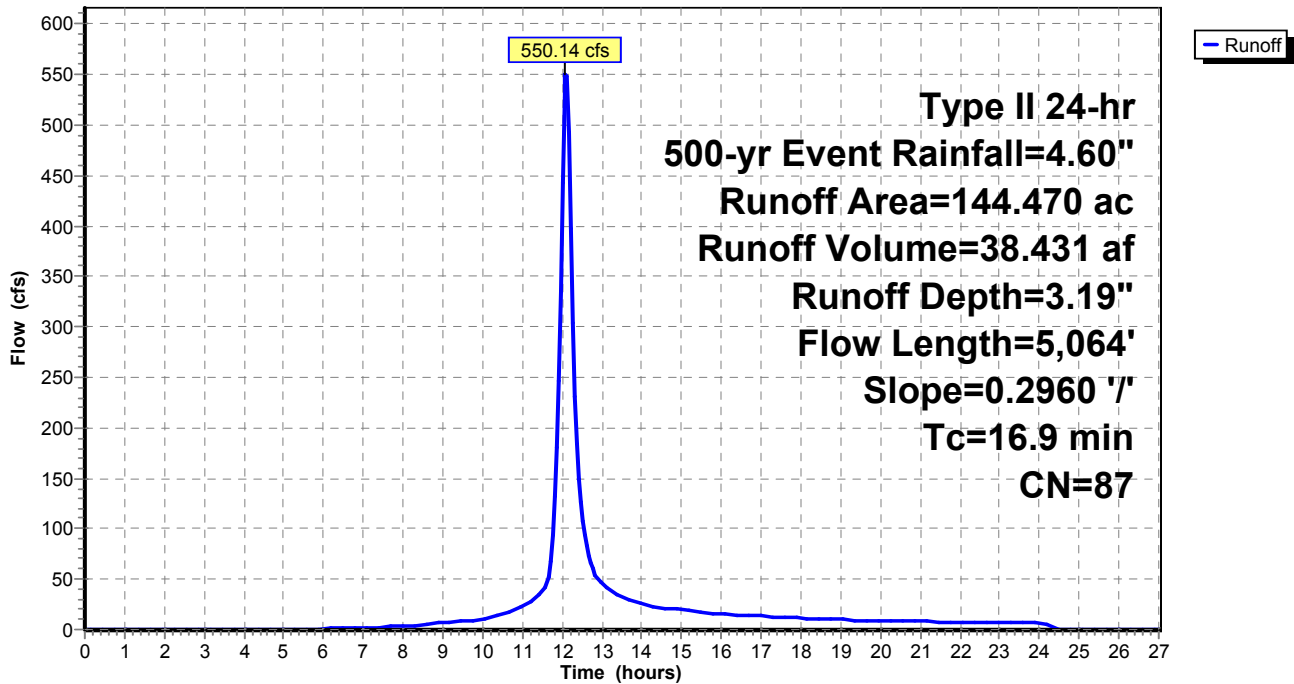
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
130.020	86	Desert shrub range, Fair, HSG D
* 14.450	98	Impervious, HSG D
144.470	87	Weighted Average
130.020		90.00% Pervious Area
14.450		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.9	5,064	0.2960	5.00		Lag/CN Method,

**Subcatchment 7: WS 7**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 8: WS 8**

Runoff = 389.99 cfs @ 12.05 hrs, Volume= 24.476 af, Depth= 3.19"

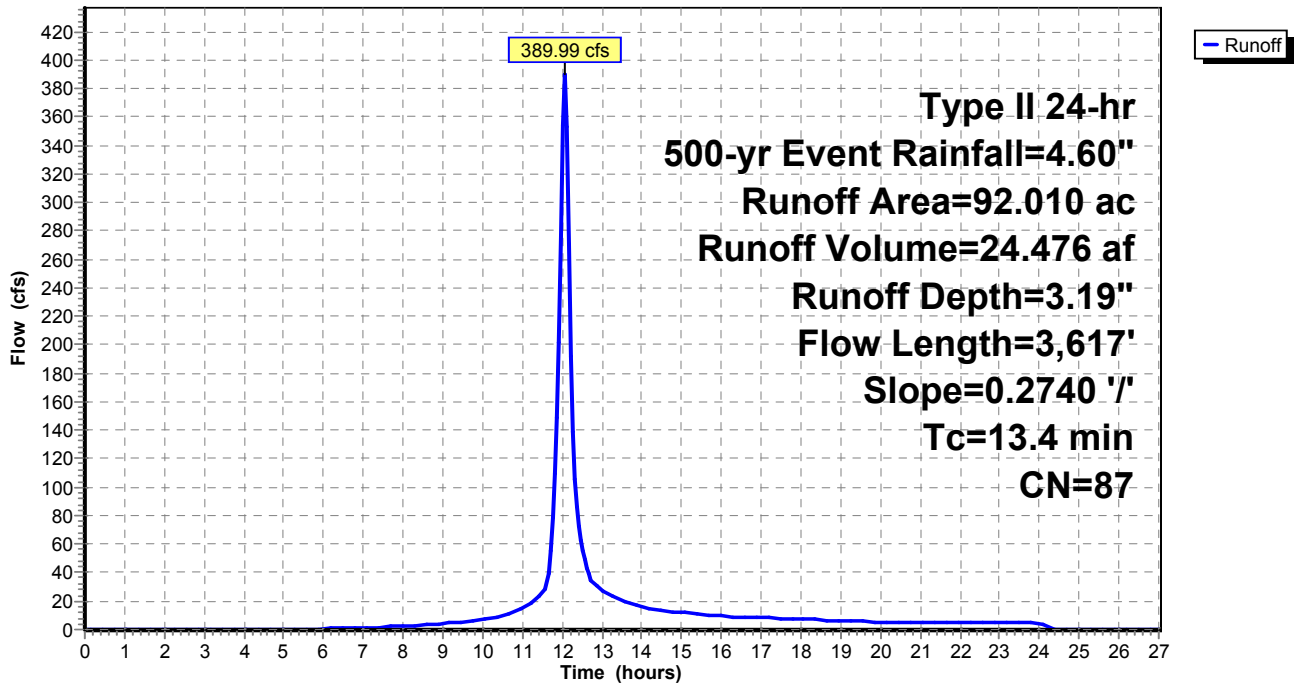
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
82.810	86	Desert shrub range, Fair, HSG D
* 9.200	98	Impervious, HSG D
92.010	87	Weighted Average
82.810		90.00% Pervious Area
9.200		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.4	3,617	0.2740	4.50		Lag/CN Method,

**Subcatchment 8: WS 8**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 9: WS 9**

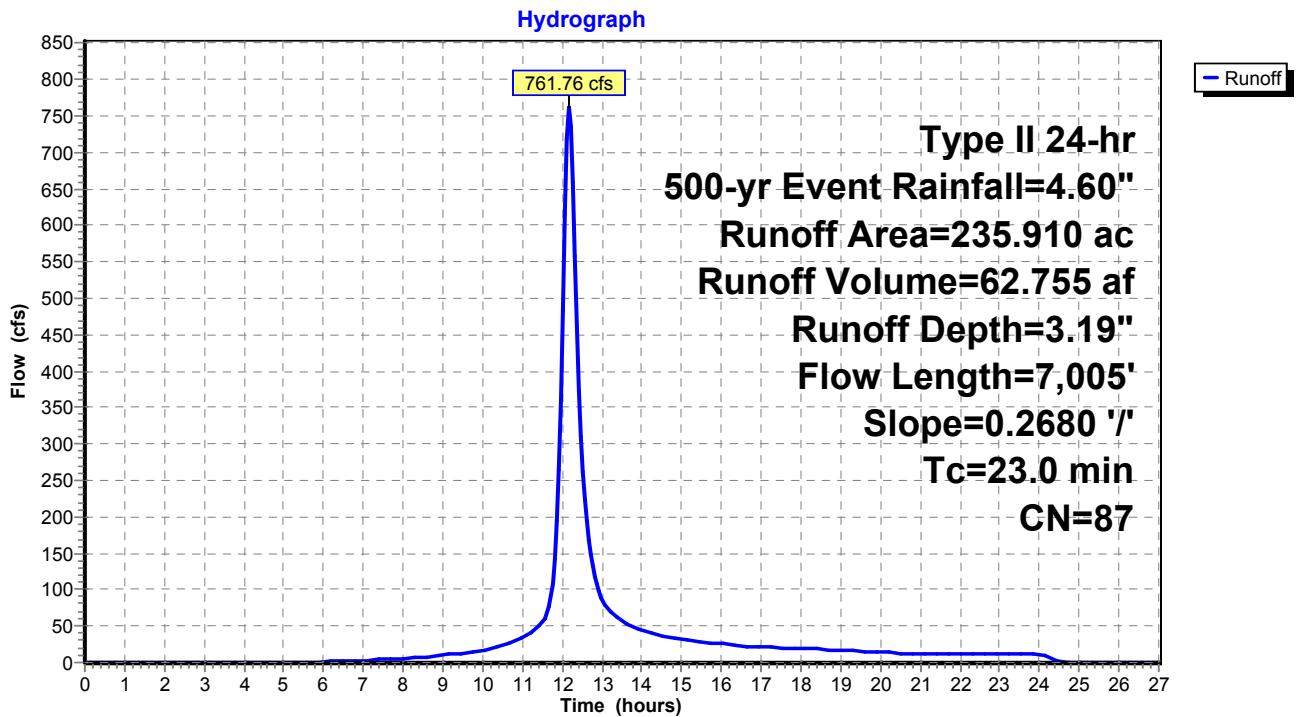
Runoff = 761.76 cfs @ 12.16 hrs, Volume= 62.755 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
212.320	86	Desert shrub range, Fair, HSG D
* 23.590	98	Impervious, HSG D
235.910	87	Weighted Average
212.320		90.00% Pervious Area
23.590		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0	7,005	0.2680	5.08		Lag/CN Method,

**Subcatchment 9: WS 9**



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 10: WS 10**

Runoff = 891.85 cfs @ 12.25 hrs, Volume= 87.894 af, Depth= 3.19"

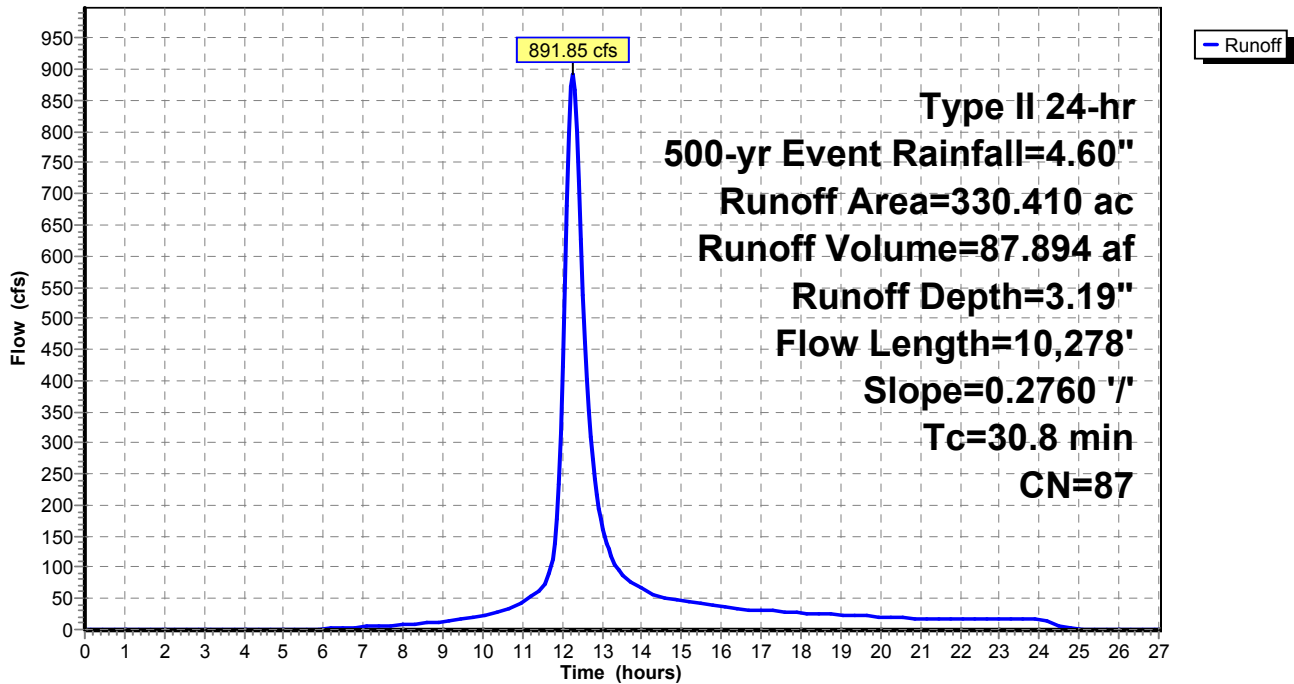
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
297.370	86	Desert shrub range, Fair, HSG D
* 33.040	98	Impervious, HSG D
330.410	87	Weighted Average
297.370		90.00% Pervious Area
33.040		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	10,278	0.2760	5.57		Lag/CN Method,

**Subcatchment 10: WS 10**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 11: WS 11**

Runoff = 1,132.73 cfs @ 12.22 hrs, Volume= 105.828 af, Depth= 3.19"

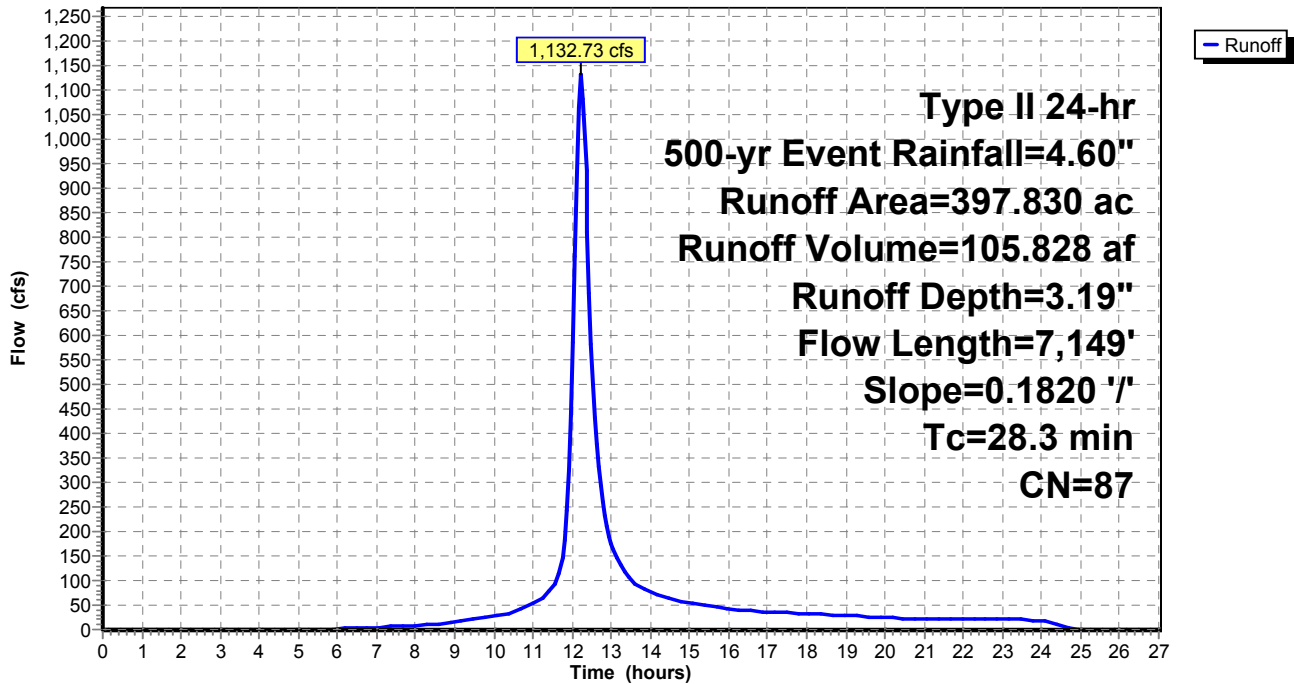
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
358.050	86	Desert shrub range, Fair, HSG D
* 39.780	98	Impervious, HSG D
397.830	87	Weighted Average
358.050		90.00% Pervious Area
39.780		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	7,149	0.1820	4.20		Lag/CN Method,

**Subcatchment 11: WS 11**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 12: WS 12**

Runoff = 789.72 cfs @ 12.12 hrs, Volume= 60.497 af, Depth= 3.19"

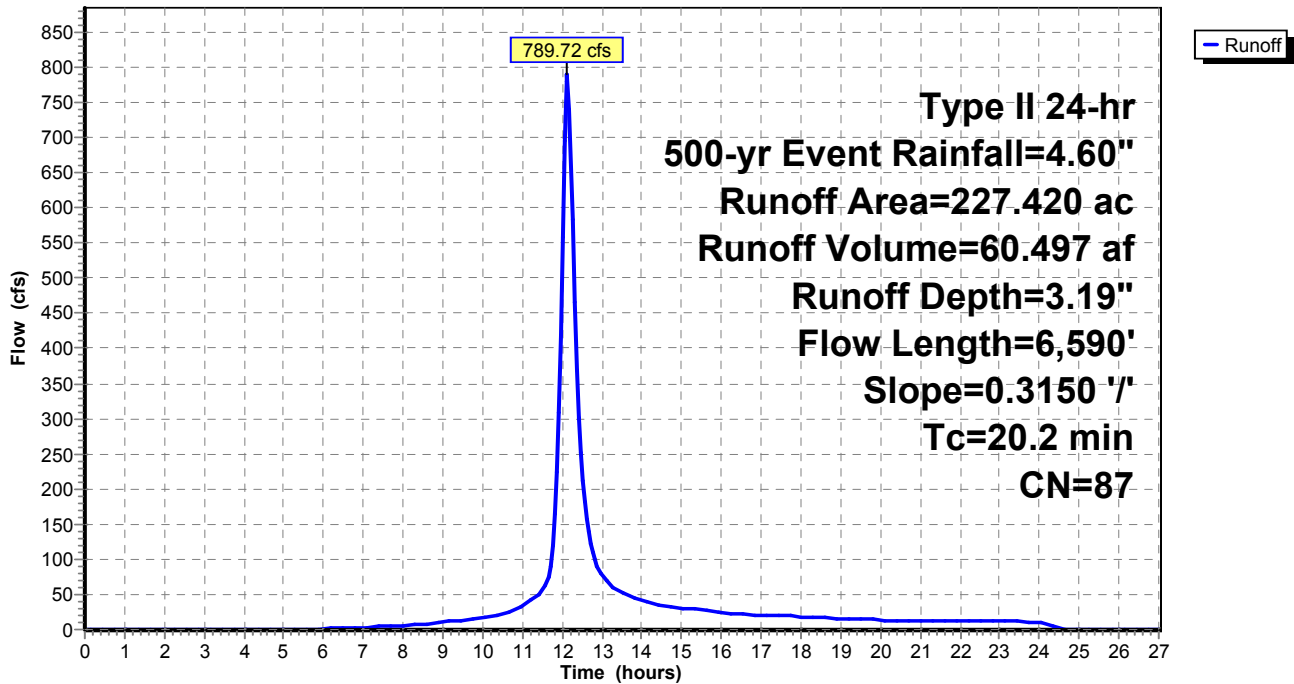
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
204.680	86	Desert shrub range, Fair, HSG D
* 22.740	98	Impervious, HSG D
227.420	87	Weighted Average
204.680		90.00% Pervious Area
22.740		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.2	6,590	0.3150	5.44		Lag/CN Method,

**Subcatchment 12: WS 12**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Subcatchment 13: WS 13**

Runoff = 871.93 cfs @ 12.16 hrs, Volume= 73.289 af, Depth= 3.19"

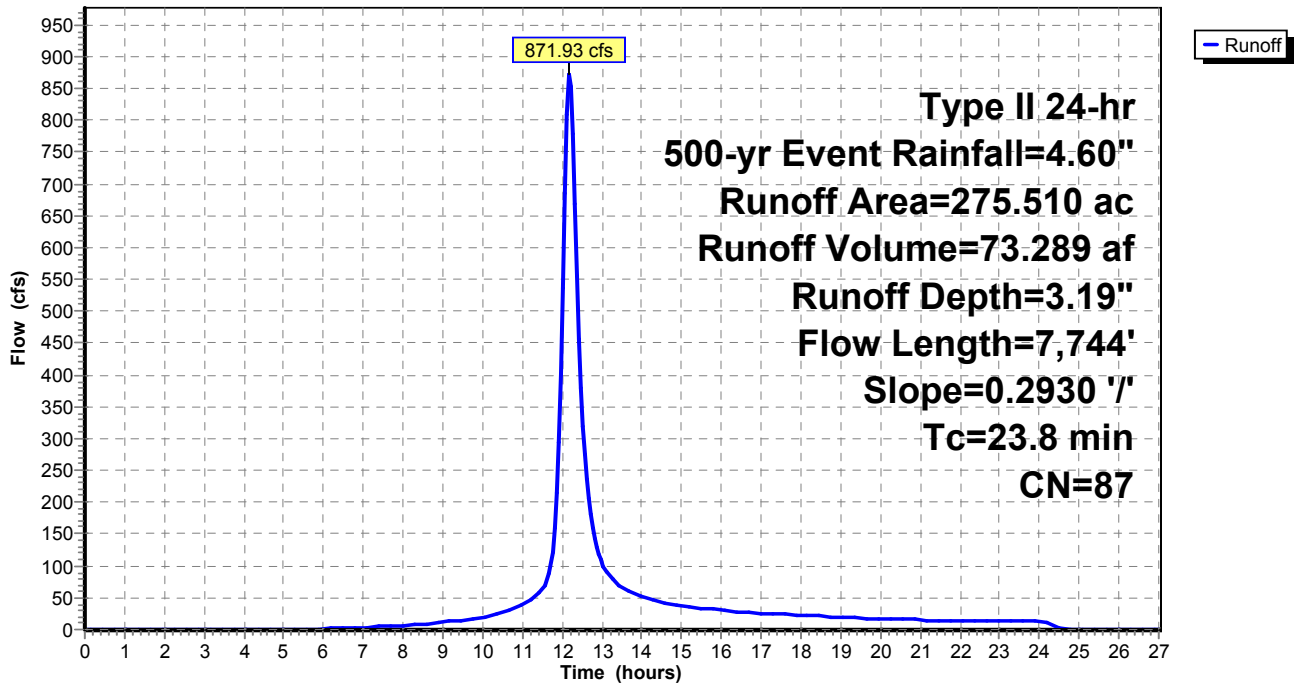
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Type II 24-hr 500-yr Event Rainfall=4.60"

Area (ac)	CN	Description
247.960	86	Desert shrub range, Fair, HSG D
* 27.550	98	Impervious, HSG D
275.510	87	Weighted Average
247.960		90.00% Pervious Area
27.550		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.8	7,744	0.2930	5.42		Lag/CN Method,

**Subcatchment 13: WS 13**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Reach CHNL 1: Channel 1**

Inflow Area = 502.930 ac, 10.00% Impervious, Inflow Depth = 3.19" for 500-yr Event event  
 Inflow = 1,647.46 cfs @ 12.14 hrs, Volume= 133.786 af  
 Outflow = 1,601.86 cfs @ 12.23 hrs, Volume= 133.784 af, Atten= 3%, Lag= 5.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 9.88 fps, Min. Travel Time= 3.2 min  
 Avg. Velocity = 2.42 fps, Avg. Travel Time= 13.2 min

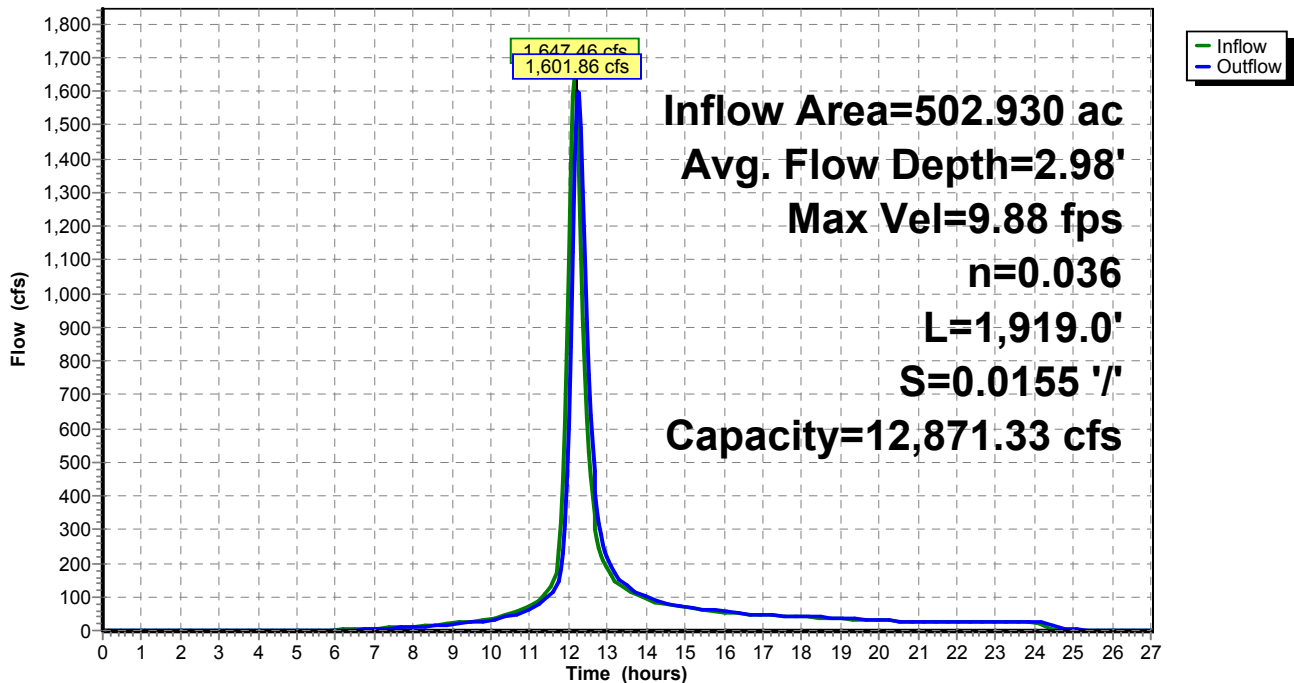
Peak Storage= 311,381 cf @ 12.18 hrs  
 Average Depth at Peak Storage= 2.98'  
 Bank-Full Depth= 10.00' Flow Area= 650.0 sf, Capacity= 12,871.33 cfs

50.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 ' / ' Top Width= 80.00'  
 Length= 1,919.0' Slope= 0.0155 ' / '  
 Inlet Invert= 5,569.50', Outlet Invert= 5,539.70'



**Reach CHNL 1: Channel 1**

**Hydrograph**





**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Reach CHNL 2: Channel 2**

Inflow Area = 870.220 ac, 10.00% Impervious, Inflow Depth = 3.19" for 500-yr Event event  
 Inflow = 2,597.10 cfs @ 12.14 hrs, Volume= 231.491 af  
 Outflow = 2,180.53 cfs @ 12.45 hrs, Volume= 231.028 af, Atten= 16%, Lag= 18.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 10.26 fps, Min. Travel Time= 11.5 min  
 Avg. Velocity = 2.72 fps, Avg. Travel Time= 43.2 min

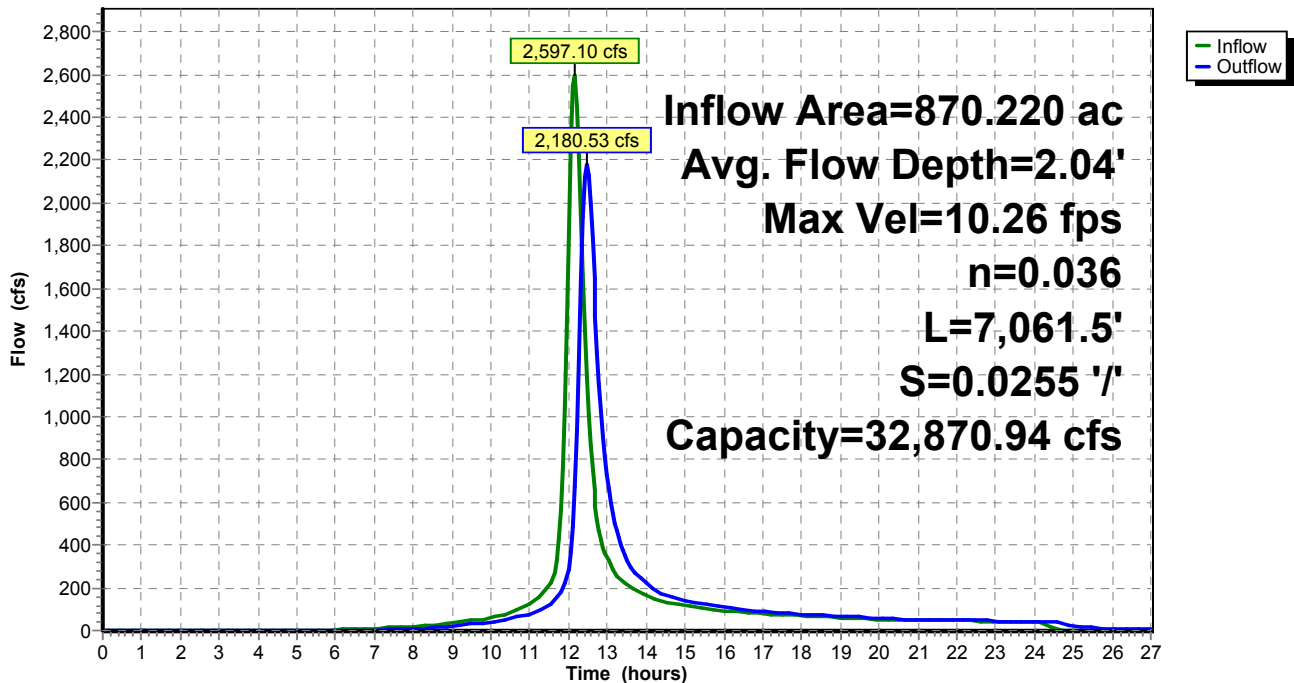
Peak Storage= 1,506,746 cf @ 12.26 hrs  
 Average Depth at Peak Storage= 2.04'  
 Bank-Full Depth= 10.00' Flow Area= 1,225.0 sf, Capacity= 32,870.94 cfs

100.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 2.5 2.0 ' / Top Width= 145.00'  
 Length= 7,061.5' Slope= 0.0255 ' /  
 Inlet Invert= 5,720.00', Outlet Invert= 5,539.70'



**Reach CHNL 2: Channel 2**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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**Summary for Reach CHNL 3: Channel 3**

[62] Hint: Exceeded Reach CHNL 1 OUTLET depth by 4.30' @ 12.45 hrs

[62] Hint: Exceeded Reach CHNL 2 OUTLET depth by 4.53' @ 12.35 hrs

Inflow Area = 2,233.990 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event  
 Inflow = 5,094.93 cfs @ 12.28 hrs, Volume= 593.807 af  
 Outflow = 5,008.23 cfs @ 12.40 hrs, Volume= 593.385 af, Atten= 2%, Lag= 6.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 15.55 fps, Min. Travel Time= 3.9 min  
 Avg. Velocity = 4.58 fps, Avg. Travel Time= 13.4 min

Peak Storage= 1,188,451 cf @ 12.33 hrs  
 Average Depth at Peak Storage= 6.49'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 10,812.70 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 3,680.5' Slope= 0.0162 '/'  
 Inlet Invert= 5,539.70', Outlet Invert= 5,480.00'



### Exist WS (Post-Quintana)\_Culvert Analysis

Type II 24-hr 500-yr Event Rainfall=4.60"

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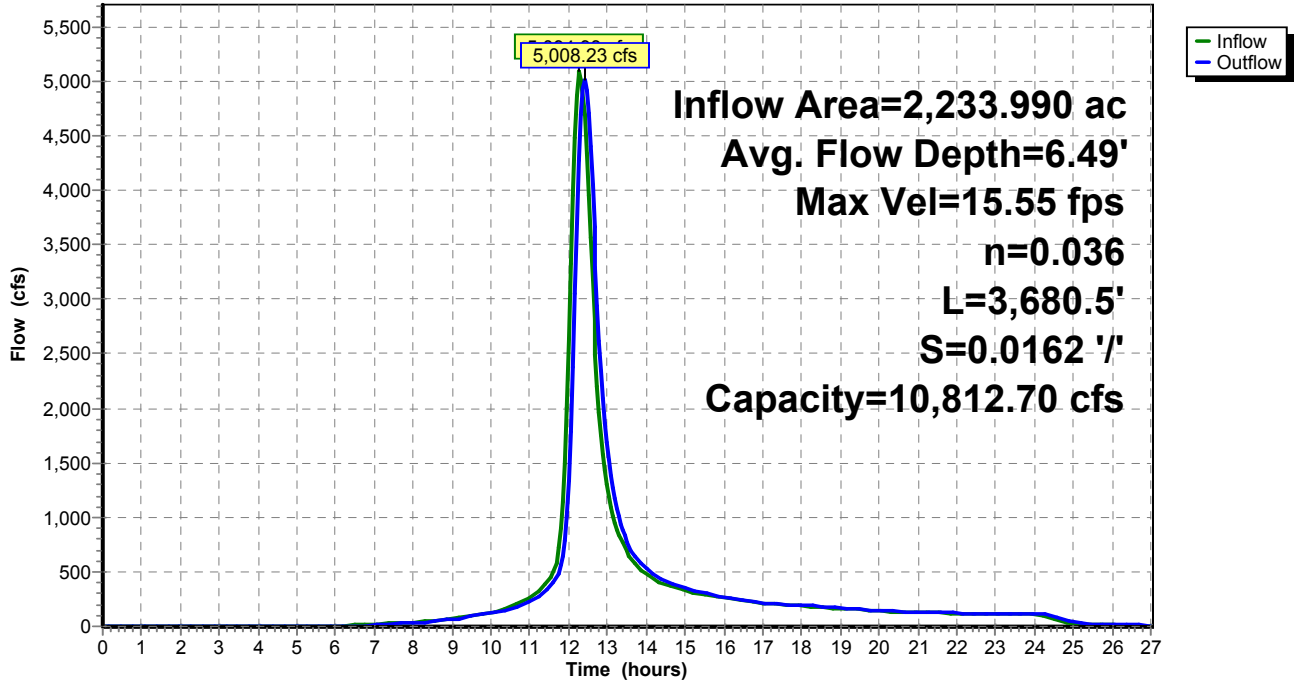
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### Reach CHNL 3: Channel 3

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Reach CHNL 4: Channel 4**

[61] Hint: Exceeded Reach CHNL 3 outlet invert by 4.57' @ 12.40 hrs

Inflow Area = 2,268.980 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event  
 Inflow = 5,028.84 cfs @ 12.40 hrs, Volume= 602.692 af  
 Outflow = 5,012.38 cfs @ 12.42 hrs, Volume= 602.599 af, Atten= 0%, Lag= 1.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 23.45 fps, Min. Travel Time= 0.8 min  
 Avg. Velocity = 6.76 fps, Avg. Travel Time= 2.7 min

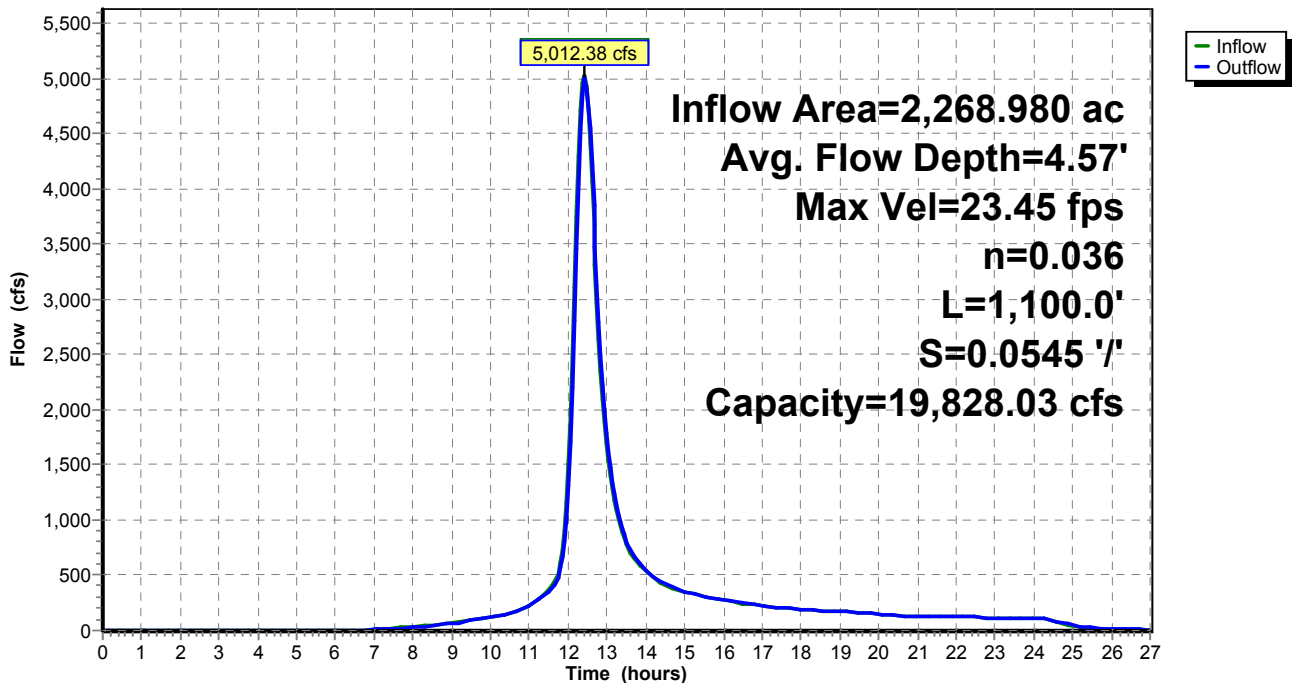
Peak Storage= 235,792 cf @ 12.41 hrs  
 Average Depth at Peak Storage= 4.57'  
 Bank-Full Depth= 10.00' Flow Area= 550.0 sf, Capacity= 19,828.03 cfs

40.00' x 10.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 '/' Top Width= 70.00'  
 Length= 1,100.0' Slope= 0.0545 '/'  
 Inlet Invert= 5,480.00', Outlet Invert= 5,420.00'



**Reach CHNL 4: Channel 4**

Hydrograph



**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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**Summary for Reach CHNL 5: Channel 5**

[62] Hint: Exceeded Reach CHNL 4 OUTLET depth by 2.92' @ 12.50 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event  
 Inflow = 5,080.00 cfs @ 12.42 hrs, Volume= 630.949 af  
 Outflow = 5,046.09 cfs @ 12.47 hrs, Volume= 630.724 af, Atten= 1%, Lag= 3.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 18.81 fps, Min. Travel Time= 1.9 min  
 Avg. Velocity = 6.05 fps, Avg. Travel Time= 6.0 min

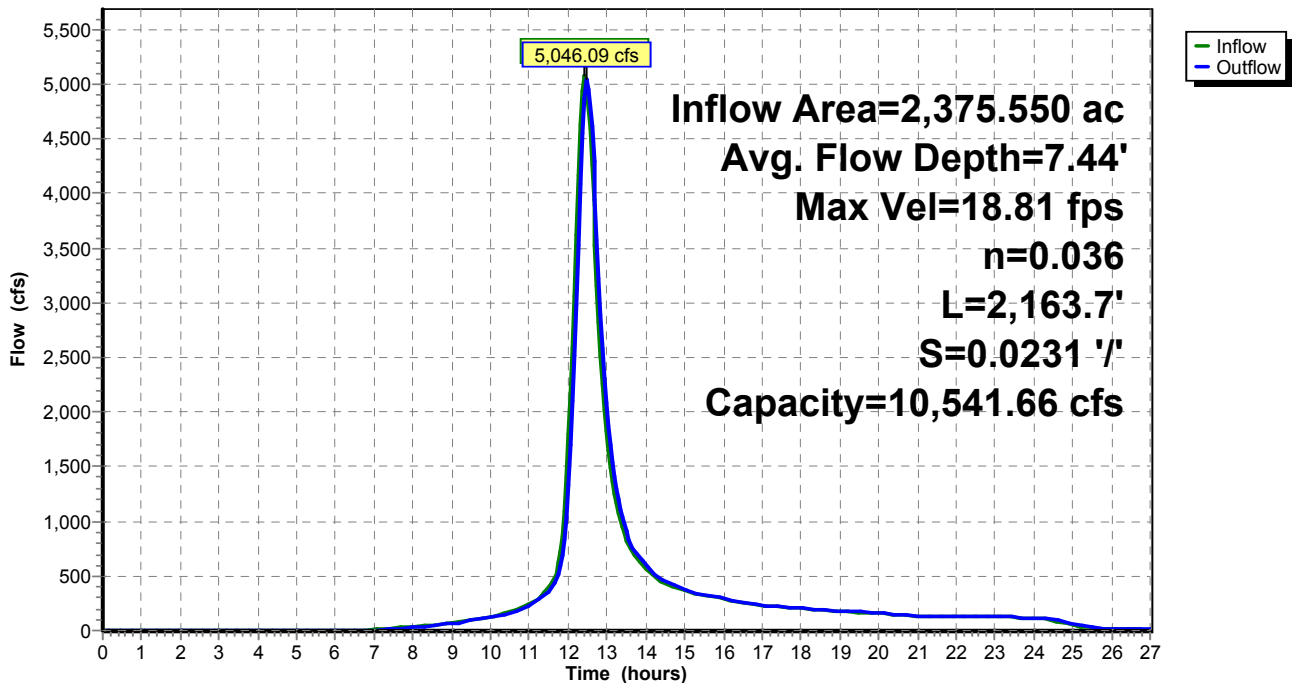
Peak Storage= 582,162 cf @ 12.44 hrs  
 Average Depth at Peak Storage= 7.44'  
 Bank-Full Depth= 11.00' Flow Area= 456.5 sf, Capacity= 10,541.66 cfs

25.00' x 11.00' deep channel, n= 0.036  
 Side Slope Z-value= 1.5 ' ' Top Width= 58.00'  
 Length= 2,163.7' Slope= 0.0231 ' '  
 Inlet Invert= 5,420.00', Outlet Invert= 5,370.00'



**Reach CHNL 5: Channel 5**

Hydrograph



**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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**Summary for Pond CLVT 1: Culvert Crossing #1**

[62] Hint: Exceeded Reach CHNL 5 OUTLET depth by 21.32' @ 12.80 hrs

Inflow Area = 2,375.550 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event  
 Inflow = 5,046.09 cfs @ 12.47 hrs, Volume= 630.724 af  
 Outflow = 3,847.35 cfs @ 12.71 hrs, Volume= 630.580 af, Atten= 24%, Lag= 14.4 min  
 Primary = 3,847.35 cfs @ 12.71 hrs, Volume= 630.580 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,397.29' @ 12.71 hrs Surf.Area= 0 sf Storage= 2,570,184 cf  
 Flood Elev= 5,419.00' Surf.Area= 0 sf Storage= 12,108,960 cf

Plug-Flow detention time= 5.1 min calculated for 629.414 af (100% of inflow)  
 Center-of-Mass det. time= 4.9 min ( 855.3 - 850.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,370.00'	12,108,960 cf	<b>Culvert Crossing #1</b> Listed below

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Elevation (feet)	Cum.Store (cubic-feet)
5,370.00	0
5,371.00	1,264
5,372.00	4,909
5,373.00	7,513
5,374.00	16,645
5,375.00	29,237
5,376.00	46,725
5,377.00	71,156
5,378.00	103,816
5,379.00	144,375
5,380.00	193,809
5,381.00	252,611
5,382.00	321,098
5,383.00	399,998
5,384.00	488,627
5,385.00	587,544
5,386.00	697,578
5,387.00	818,723
5,388.00	950,841
5,389.00	1,094,170
5,390.00	1,248,216
5,391.00	1,412,011
5,392.00	1,587,481
5,393.00	1,774,410
5,394.00	1,971,598
5,395.00	2,046,356
5,396.00	2,266,171
5,397.00	2,498,604
5,398.00	2,742,207
5,399.00	2,998,577
5,400.00	3,272,610
5,401.00	3,564,605
5,402.00	3,875,249
5,403.00	4,205,411
5,404.00	4,554,001
5,405.00	4,921,330
5,406.00	5,306,733
5,407.00	5,710,058
5,408.00	6,132,117
5,409.00	6,572,849
5,410.00	7,032,579
5,411.00	7,512,479
5,412.00	8,012,116
5,413.00	8,533,107
5,414.00	9,078,172
5,415.00	9,647,155
5,416.00	10,235,642
5,417.00	10,841,380
5,418.00	11,465,162
5,419.00	12,108,960

**Exist WS (Post-Quintana)\_ Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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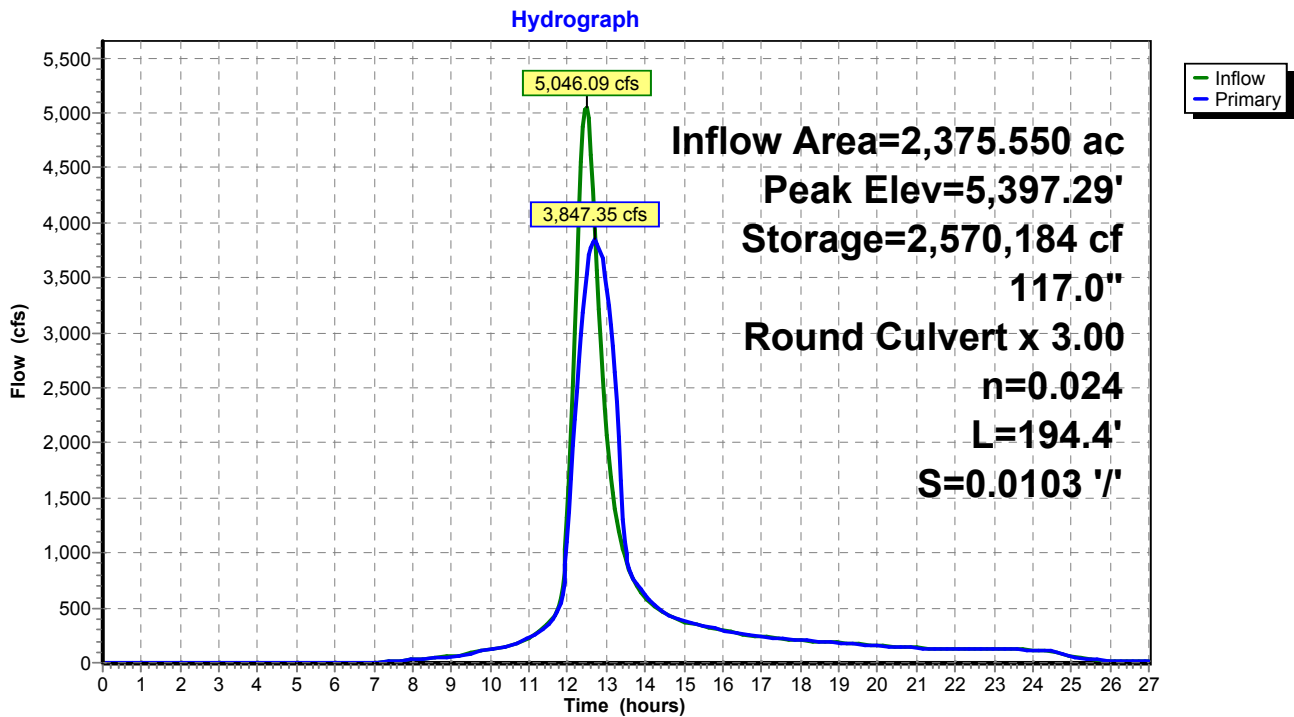
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Device	Routing	Invert	Outlet Devices
#1	Primary	5,372.00'	<b>117.0" Round Culvert X 3.00</b> L= 194.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,372.00' / 5,370.00' S= 0.0103 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=3,844.98 cfs @ 12.71 hrs HW=5,397.27' (Free Discharge)  
 ↳ **1=Culvert** (Inlet Controls 3,844.98 cfs @ 17.17 fps)

**Pond CLVT 1: Culvert Crossing #1**





**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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**Summary for Pond CLVT 2: Culvert Crossing #2**

[81] Warning: Exceeded Pond CLVT 1 by 17.33' @ 13.50 hrs

Inflow Area = 2,403.760 ac, 10.00% Impervious, Inflow Depth > 3.19" for 500-yr Event event  
 Inflow = 3,856.73 cfs @ 12.71 hrs, Volume= 638.084 af  
 Outflow = 1,791.19 cfs @ 13.33 hrs, Volume= 638.031 af, Atten= 54%, Lag= 37.4 min  
 Primary = 1,791.19 cfs @ 13.33 hrs, Volume= 638.031 af

Routing by Stor-Ind method, Time Span= 0.00-27.00 hrs, dt= 0.05 hrs  
 Peak Elev= 5,396.71' @ 13.33 hrs Surf.Area= 0 sf Storage= 8,137,086 cf  
 Flood Elev= 5,403.00' Surf.Area= 0 sf Storage= 10,142,539 cf

Plug-Flow detention time= 36.3 min calculated for 636.852 af (100% of inflow)  
 Center-of-Mass det. time= 36.2 min ( 890.9 - 854.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	5,352.00'	10,142,539 cf	<b>Existing Pond</b> Listed below

**Exist WS (Post-Quintana)\_ Culvert Analysis***Type II 24-hr 500-yr Event Rainfall=4.60"*

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Elevation (feet)	Cum.Store (cubic-feet)
5,352.00	0
5,353.00	2,681
5,354.00	7,382
5,355.00	15,397
5,356.00	28,286
5,357.00	47,224
5,358.00	75,773
5,359.00	117,133
5,360.00	170,345
5,361.00	235,331
5,362.00	312,331
5,363.00	399,765
5,364.00	497,494
5,365.00	605,663
5,366.00	724,610
5,367.00	855,928
5,368.00	1,000,315
5,369.00	1,157,122
5,370.00	1,325,832
5,371.00	1,505,243
5,372.00	1,693,878
5,373.00	1,890,808
5,374.00	2,095,234
5,375.00	2,306,959
5,376.00	2,525,755
5,377.00	2,751,367
5,378.00	2,983,426
5,379.00	3,221,478
5,380.00	3,465,168
5,381.00	3,714,247
5,382.00	3,968,434
5,383.00	4,227,291
5,384.00	4,490,405
5,385.00	4,757,518
5,386.00	5,028,418
5,387.00	5,302,894
5,388.00	5,580,732
5,389.00	5,861,867
5,390.00	6,146,219
5,391.00	6,433,776
5,392.00	6,724,543
5,393.00	7,018,534
5,394.00	7,315,762
5,395.00	7,616,273
5,396.00	7,920,106
5,397.00	8,227,290
5,398.00	8,537,855
5,399.00	8,851,826
5,400.00	9,169,229
5,401.00	9,490,117

**Exist WS (Post-Quintana)\_Culvert Analysis**

Type II 24-hr 500-yr Event Rainfall=4.60"

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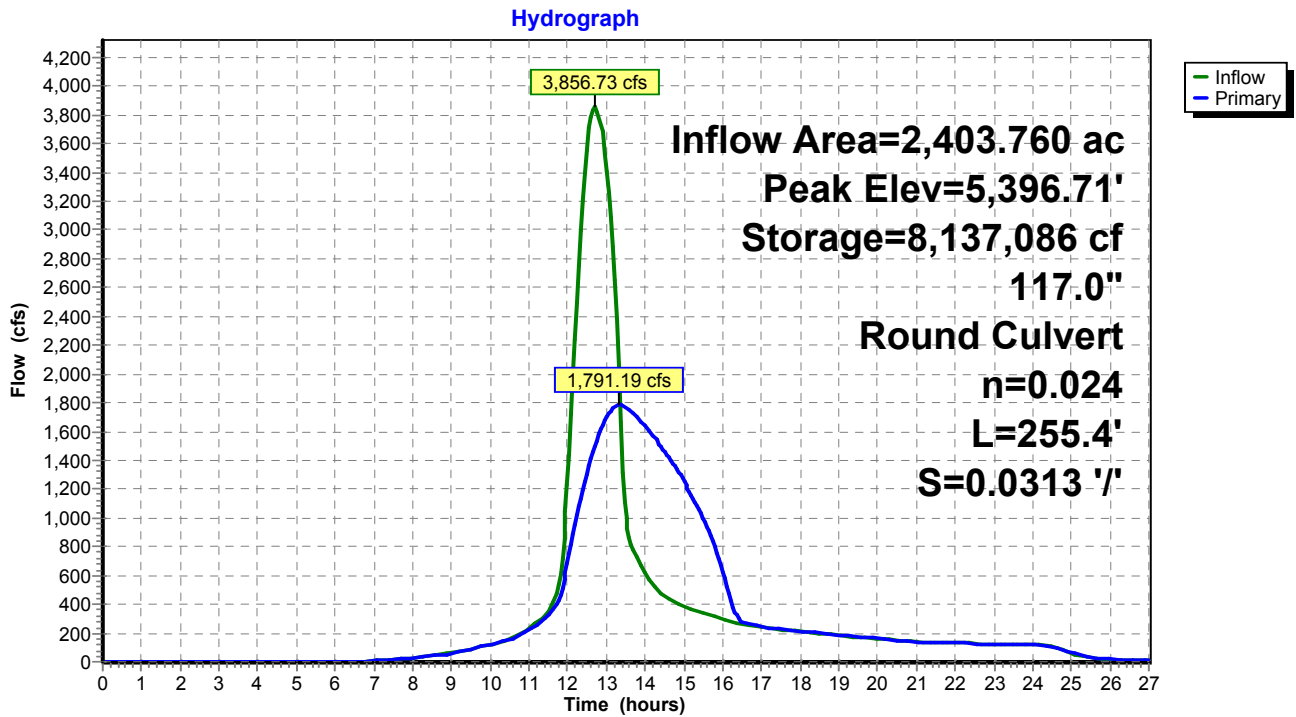
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5,402.00      9,814,540  
 5,403.00      10,142,539

Device	Routing	Invert	Outlet Devices
#1	Primary	5,352.00'	<b>117.0" Round Culvert</b> L= 255.4' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 5,352.00' / 5,344.00' S= 0.0313 '/ Cc= 0.900 n= 0.024, Flow Area= 74.66 sf

**Primary OutFlow** Max=1,790.68 cfs @ 13.33 hrs HW=5,396.68' (Free Discharge)  
 ←**1=Culvert** (Inlet Controls 1,790.68 cfs @ 23.98 fps)

**Pond CLVT 2: Culvert Crossing #2**



## APPENDIX E

# HY-8 Culvert Analysis Report

## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 3552.39 cfs

Maximum Flow: 3552.39 cfs

**Table 1 - Summary of Culvert Flows at Crossing: Crossing #1 (Q100)**

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.14	355.24	355.24	0.00	1
5379.56	710.48	710.48	0.00	1
5380.24	1065.72	1065.72	0.00	1
5381.15	1420.96	1420.96	0.00	1
5382.25	1776.19	1776.19	0.00	1
5383.46	2131.43	2131.43	0.00	1
5384.93	2486.67	2486.67	0.00	1
5386.88	2841.91	2841.91	0.00	1
5389.10	3197.15	3197.15	0.00	1
5391.64	3552.39	3552.39	0.00	1
5419.00	6067.40	6067.40	0.00	Overtopping

**Table 2 - Culvert Summary Table: Culvert 1**

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	355.24	355.24	5379.14	3.732	7.141	1-S1t	2.429	2.542	9.000	9.000	1.661
	710.48	710.48	5379.56	5.429	7.560	1-S1t	3.514	3.636	9.000	9.000	3.322
	1065.72	1065.72	5380.24	6.895	8.241	1-S1t	4.391	4.498	9.000	9.000	4.983
	1420.96	1420.96	5381.15	8.285	9.153	1-S1t	5.196	5.224	9.000	9.000	6.645
	1776.19	1776.19	5382.25	9.701	10.246	7-M1t	5.989	5.872	9.000	9.000	8.221
	2131.43	2131.43	5383.46	11.227	11.459	7-M1t	6.823	6.452	9.000	9.000	9.865
	2486.67	2486.67	5384.93	12.934	12.754	7-M1t	7.797	6.982	9.000	9.000	11.509
	2841.91	2841.91	5386.88	14.878	14.187	3-M2t	9.750	7.458	9.000	9.000	13.153
	3197.15	3197.15	5389.10	17.102	16.162	7-M2t	9.750	7.882	9.000	9.000	14.797
	3552.39	3552.39	5391.64	19.638	18.453	7-M2t	9.750	8.252	9.000	9.000	16.441

\*\*\*\*\*  
Straight Culvert  
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft  
Culvert Length: 194.41 ft, Culvert Slope: 0.0103  
\*\*\*\*\*

### Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 5372.00 ft

Outlet Station: 194.40 ft

Outlet Elevation: 5370.00 ft

Number of Barrels: 3

### Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 9.75 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in

Barrel Manning's n: 0.0240

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: NONE



**Table 3 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q100))**

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
355.24	5379.00	9.00
710.48	5379.00	9.00
1065.72	5379.00	9.00
1420.96	5379.00	9.00
1776.19	5379.00	9.00
2131.43	5379.00	9.00
2486.67	5379.00	9.00
2841.91	5379.00	9.00
3197.15	5379.00	9.00
3552.39	5379.00	9.00

**Tailwater Channel Data - Crossing #1 (Q100)**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

**Roadway Data for Crossing: Crossing #1 (Q100)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft

**Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 4191.95 cfs

Maximum Flow: 4191.95 cfs

**Table 4 - Summary of Culvert Flows at Crossing: Crossing #1 (Q200)**

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.20	419.19	419.19	0.00	1
5379.78	838.39	838.39	0.00	1
5380.71	1257.59	1257.59	0.00	1
5381.93	1676.78	1676.78	0.00	1
5383.33	2095.97	2095.97	0.00	1
5385.08	2515.17	2515.17	0.00	1
5387.43	2934.36	2934.36	0.00	1
5390.18	3353.56	3353.56	0.00	1
5393.38	3772.76	3772.76	0.00	1
5397.05	4191.95	4191.95	0.00	1
5419.00	6067.40	6067.40	0.00	Overtopping

**Table 5 - Culvert Summary Table: Culvert 1**

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	419.19	419.19	5379.20	4.077	7.197	1-S1t	2.671	2.769	9.000	9.000	1.960
	838.39	838.39	5379.78	5.973	7.777	1-S1t	3.833	3.962	9.000	9.000	3.920
	1257.59	1257.59	5380.71	7.648	8.708	1-S1t	4.827	4.903	9.000	9.000	5.881
	1676.78	1676.78	5381.93	9.297	9.926	7-M1t	5.767	5.696	9.000	9.000	7.761
	2095.97	2095.97	5383.33	11.067	11.334	7-M1t	6.738	6.397	9.000	9.000	9.701
	2515.17	2515.17	5385.08	13.080	12.862	7-M1t	7.879	7.022	9.000	9.000	11.641
	2934.36	2934.36	5387.43	15.428	14.613	3-M2t	9.750	7.573	9.000	9.000	13.581
	3353.56	3353.56	5390.18	18.179	17.154	7-M2t	9.750	8.052	9.000	9.000	15.521
	3772.76	3772.76	5393.38	21.377	19.962	7-M2t	9.750	8.453	9.000	9.000	17.461
	4191.95	4191.95	5397.05	25.048	23.060	7-M2t	9.750	8.776	9.000	9.000	19.402

\*\*\*\*\*  
Straight Culvert  
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft  
Culvert Length: 194.41 ft, Culvert Slope: 0.0103  
\*\*\*\*\*

**Site Data - Culvert 1**

Site Data Option: Culvert Invert Data  
Inlet Station: 0.00 ft  
Inlet Elevation: 5372.00 ft  
Outlet Station: 194.40 ft  
Outlet Elevation: 5370.00 ft  
Number of Barrels: 3

**Culvert Data Summary - Culvert 1**

Barrel Shape: Circular  
Barrel Diameter: 9.75 ft  
Barrel Material: Corrugated Steel  
Embedment: 0.00 in  
Barrel Manning's n: 0.0240  
Culvert Type: Straight  
Inlet Configuration: Thin Edge Projecting  
Inlet Depression: NONE

**Table 6 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q200))**

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
419.19	5379.00	9.00
838.39	5379.00	9.00
1257.59	5379.00	9.00
1676.78	5379.00	9.00
2095.97	5379.00	9.00
2515.17	5379.00	9.00
2934.36	5379.00	9.00
3353.56	5379.00	9.00
3772.76	5379.00	9.00
4191.95	5379.00	9.00

**Tailwater Channel Data - Crossing #1 (Q200)**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

**Roadway Data for Crossing: Crossing #1 (Q200)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft



**Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 5046.09 cfs

Maximum Flow: 5046.09 cfs

**Table 7 - Summary of Culvert Flows at Crossing: Crossing #1 (Q500)**

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
5379.00	0.00	0.00	0.00	1
5379.28	504.61	504.61	0.00	1
5380.12	1009.22	1009.22	0.00	1
5381.42	1513.83	1513.83	0.00	1
5383.06	2018.44	2018.44	0.00	1
5385.12	2523.05	2523.05	0.00	1
5388.00	3027.65	3027.65	0.00	1
5391.49	3532.26	3532.26	0.00	1
5395.63	4036.87	4036.87	0.00	1
5400.48	4541.48	4541.48	0.00	1
5406.01	5046.09	5046.09	0.00	1
5419.00	6067.41	6067.41	0.00	Overtopping

**Table 8 - Culvert Summary Table: Culvert 1**

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
	0.00	0.00	5379.00	0.000	7.000	0-NF	0.000	0.000	9.000	9.000	0.000
	504.61	504.61	5379.28	4.496	7.284	1-S1t	2.918	3.045	9.000	9.000	2.360
	1009.22	1009.22	5380.12	6.670	8.116	1-S1t	4.252	4.370	9.000	9.000	4.719
	1513.83	1513.83	5381.42	8.649	9.424	3-M1t	5.404	5.401	9.000	9.000	7.006
	2018.44	2018.44	5383.06	10.725	11.064	7-M1t	6.552	6.274	9.000	9.000	9.342
	2523.05	2523.05	5385.12	13.121	12.892	7-M1t	7.902	7.033	9.000	9.000	11.677
	3027.65	3027.65	5388.00	16.003	15.097	3-M2t	9.750	7.686	9.000	9.000	14.013
	3532.26	3532.26	5391.49	19.485	18.319	7-M2t	9.750	8.233	9.000	9.000	16.348
	4036.87	4036.87	5395.63	23.634	21.878	7-M2t	9.750	8.665	9.000	9.000	18.684
	4541.48	4541.48	5400.48	28.477	25.882	7-M2t	9.750	8.986	9.000	9.000	21.019
	5046.09	5046.09	5406.01	34.012	30.317	7-M2c	9.750	9.218	9.218	9.000	23.018

\*\*\*\*\*  
Straight Culvert  
Inlet Elevation (invert): 5372.00 ft, Outlet Elevation (invert): 5370.00 ft  
Culvert Length: 194.41 ft, Culvert Slope: 0.0103  
\*\*\*\*\*

### Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 5372.00 ft

Outlet Station: 194.40 ft

Outlet Elevation: 5370.00 ft

Number of Barrels: 3

### Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 9.75 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in

Barrel Manning's n: 0.0240

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: NONE

**Table 9 - Downstream Channel Rating Curve (Crossing: Crossing #1 (Q500))**

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	5379.00	9.00
504.61	5379.00	9.00
1009.22	5379.00	9.00
1513.83	5379.00	9.00
2018.44	5379.00	9.00
2523.05	5379.00	9.00
3027.65	5379.00	9.00
3532.26	5379.00	9.00
4036.87	5379.00	9.00
4541.48	5379.00	9.00
5046.09	5379.00	9.00

**Tailwater Channel Data - Crossing #1 (Q500)**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 5379.00 ft

**Roadway Data for Crossing: Crossing #1 (Q500)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 5419.00 ft

Roadway Surface: Paved

Roadway Top Width: 57.00 ft

# APPENDIX E

## Revision 1

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# WATER-QUALITY MONITORING PLAN FOR THE COPPER FLAT MINE DISCHARGE PERMIT PURSUANT TO 20.6.7.11.R AND 20.6.7.28 NMAC

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prepared by

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Albuquerque, New Mexico 87109

June 2016



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## **APPENDIX E**

### **Revision 1**

# **WATER-QUALITY MONITORING PLAN FOR THE COPPER FLAT MINE DISCHARGE PERMIT PURSUANT TO 20.6.7.11.R AND 20.6.7.28 NMAC**

by

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June 2016






**Certified Professional Geologist Seal**

This report documents the work of the following Certified Professional Geologist:

Steven T. Finch, Jr., CPG, PG

Mr. Finch, Principal Hydrogeologist-Geochemist, was the Project Manager for this work and was responsible for preparing this report.

  
\_\_\_\_\_  
Signature

*June 17, 2016*  
\_\_\_\_\_  
Date



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## WATER-QUALITY MONITORING PLAN FOR THE COPPER FLAT MINE DISCHARGE PERMIT, PURSUANT TO 20.6.7.11.R AND 20.6.7.28 NMAC

### 1.0 INTRODUCTION

John Shomaker & Associates, Inc. (JSAI) has prepared this Water-Quality Monitoring Plan and has identified certain existing monitoring wells at the site in combination with proposed monitoring wells to comprise a monitoring well network to monitor water quality at Copper Flat Mine to fulfill the requirements of 20.6.7.11.R and 20.6.7.28 NMAC. The monitoring well network is based on the requirements set forth in 20.6.7.28 NMAC and takes into account surface topography, hydrogeologic conditions, geologic controls, infrastructure, engineering design plans, depth to groundwater, safe working distance, and land ownership.

The proposed mine facilities and well network locations are shown on Figure 1. The units of the mine facility that will be monitored include the following:

1. open pit
2. Waste Rock Stockpiles (WRSP)
3. Tailings Storage Facility (TSF) including the Tailings Impoundment, Underground Collection Pond, and Surge Pond
4. Process Water Reservoir
5. Impact Stormwater Impoundments A, B and C

Figure 1 also identifies inset Figures 4, 5, and 6. These figures show in more detail, areas of the site and the attendant monitoring network for each area. A checklist of 20.6.7.28 NMAC water-quality monitoring requirements is presented as Table 1 with a cross reference to corresponding report section. The monitoring plan is organized in a manner such that each component of the plan is identified together with the relevant regulatory requirement as outlined in Table 1.

The groundwater monitoring network will utilize certain existing monitoring wells and proposed new monitoring wells. The open pit surface drainage area, shown on Figure 1, is based on the depicted land surface contours. The land surface contours are based on current topographic features with some modifications that will take place at the beginning of mining (e.g., haul road improvements).

The well network locations, existing and proposed, were selected based on the groundwater-flow direction established during Baseline Data collection and Stage 1 Abatement data collection. Other than the area around the existing open pit, which is a hydraulic sink, the groundwater-flow direction is west-to-east. Figure 2 shows the direction of groundwater flow below the site.

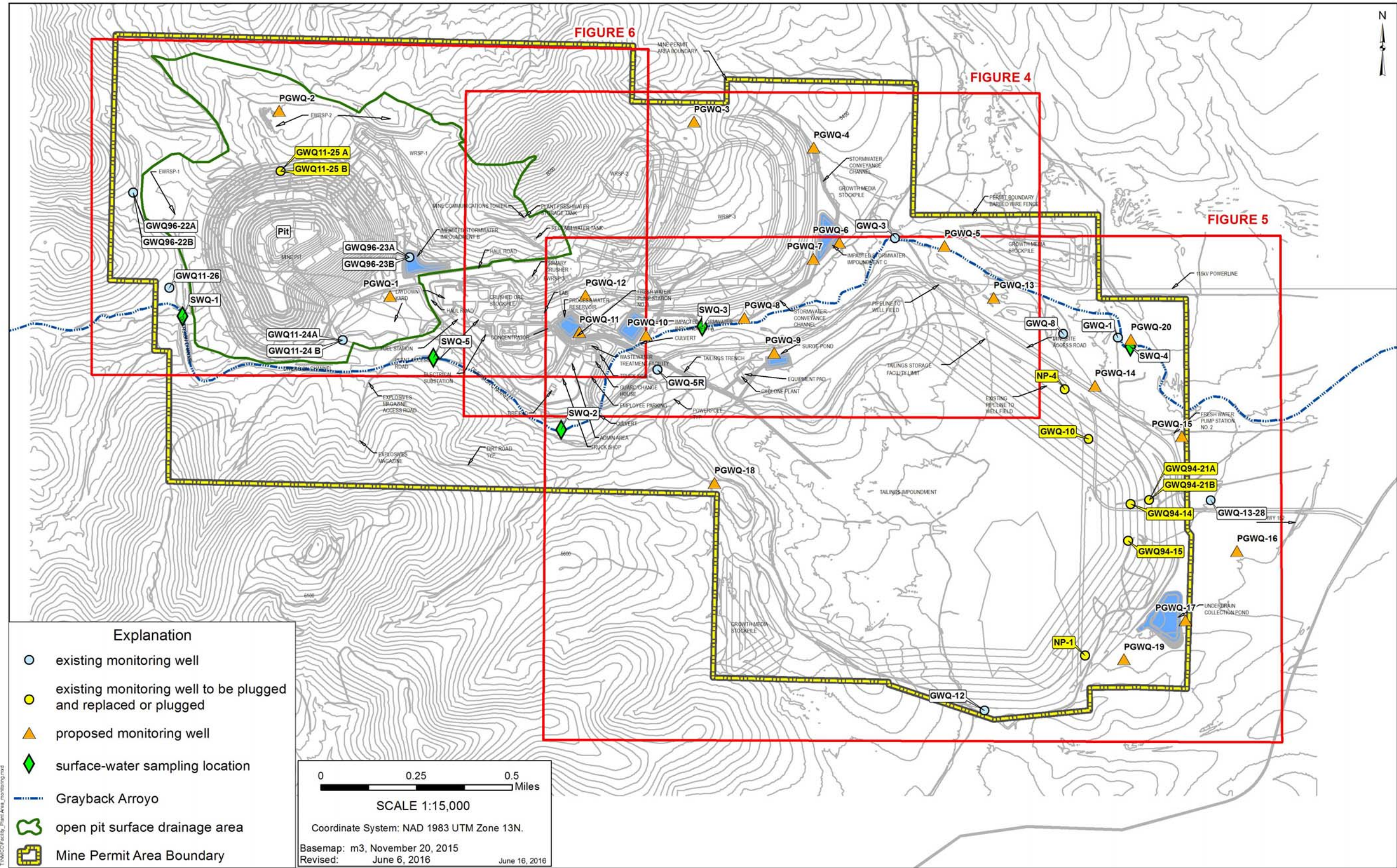


Figure 1. Proposed facility layout and proposed monitoring locations.

**Table 1. Checklist for water-quality monitoring requirements in 20.6.7.28 NMAC**

NMAC	requirement	report section
20.6.7.28.A.(1)	scaled map required by 20.6.7.11.J NMAC monitoring well location proposals *	Figure 1
20.6.7.28.A.(2)	map showing groundwater flow direction to determine monitor well locations	Figure 2
20.6.7.28.B.(1)	use of existing wells	2.1
20.6.7.28.B.(2)	groundwater monitoring for stockpiles and tailings impoundments	2.2
20.6.7.28.B.(3)	groundwater monitoring for process water and impacted stormwater impoundments	2.3
20.6.7.28.B.(4)	groundwater monitoring for open pit	2.4
20.6.7.28.B.(5)	groundwater monitoring up-gradient of each potential source	2.5
20.6.7.28.B.(6)	groundwater monitoring up-gradient of copper mine facility	2.6
20.6.7.28.C	monitoring well ID tags	3.0
20.6.7.28.D.(1) – (13)	monitoring well construction and completion	3.0
20.6.7.28.E	NMOSE permit requirements	3.0
20.6.7.28.F.(1) – (5)	groundwater sample collection procedures	4.0
20.6.7.28.G	groundwater sampling existing mine facilities	not applicable
20.6.7.28.H	groundwater sampling reduction of sampling analytes	not applicable
20.6.7.28.I	groundwater sampling for new monitoring wells	4.0
20.6.7.28.J	monitoring well survey and groundwater flow determination	5.0
20.6.7.28.K	monitoring well completion report	5.0
20.6.7.28.L	groundwater elevation contour maps	5.0
20.6.7.28.M	perennial stream sampling and reporting	not applicable
20.6.7.28.N	process water, tailings slurry, impacted storm water, seep, and spring sampling and reporting	6.0

\* Figures 4 through 6 provide detailed insets of Figure 1.  
 NMOSE – New Mexico Office of the State Engineer

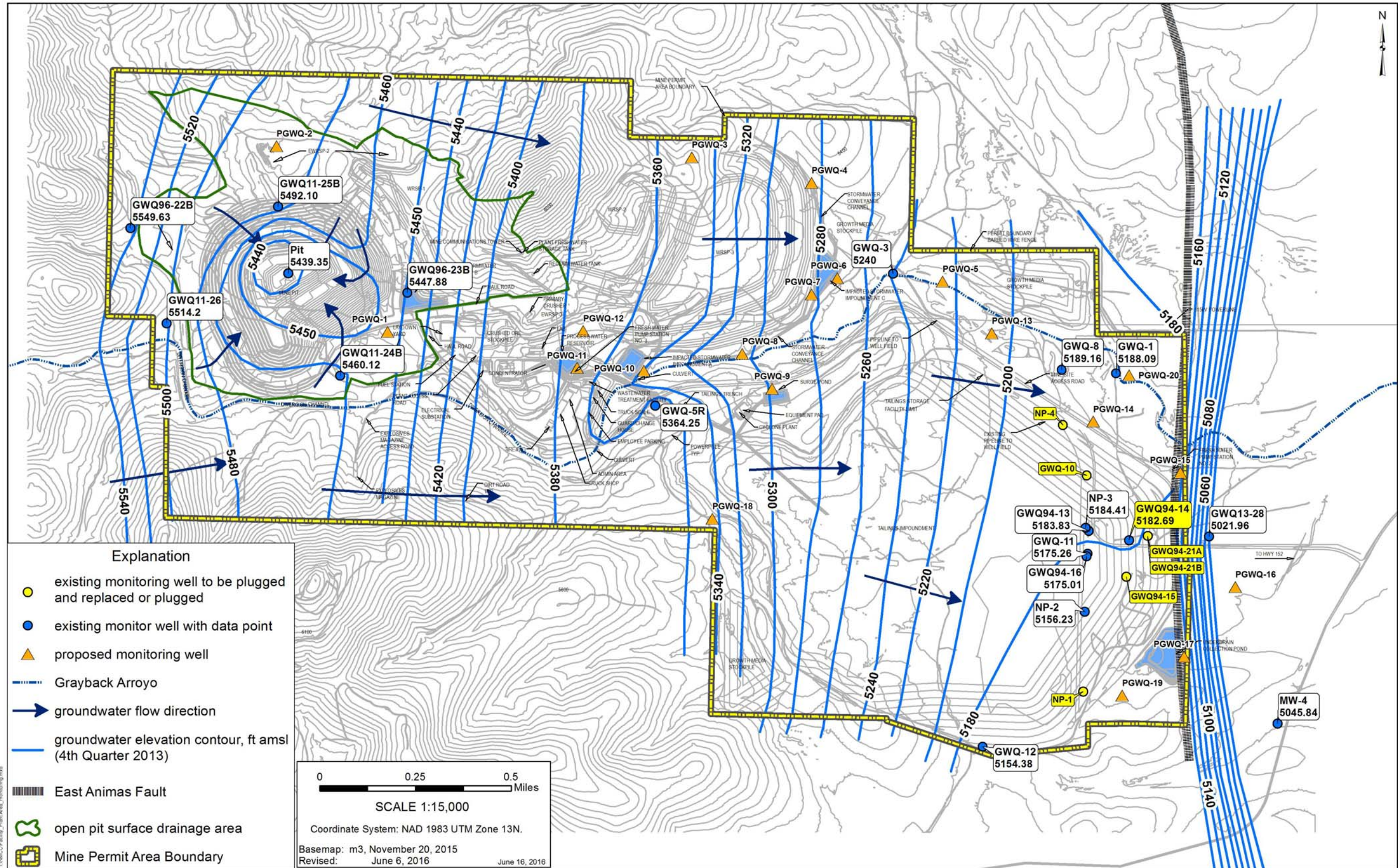


Figure 2. Groundwater flow direction.

## 2.0 MONITORING WELL NETWORK (20.6.7.28.A.(1) and (2))

The purpose of the monitoring well network is to monitor groundwater quality, potential discharges, and hydraulic-gradient as close as practicable up-gradient, around the perimeter and down-gradient of the mine pit, Waste Rock Stockpiles, Tailings Impoundment, Process Water Reservoir, and Impacted Stormwater Impoundments at Copper Flat as required by 20.6.7.28.B. The monitoring wells will be located in a manner to provide the best opportunity to detect exceedances or a trend toward exceedance of applicable standards as early as possible. The locations of the wells making up the network are identified in Figure 1, per 20.6.7.28.A.(1). Groundwater elevation contours and flow direction through the facility used to determine the well locations (pursuant to 20.6.7.28.A.(2)) can be referenced from Figure 2. A more detailed explanation of the rationale used for the locations chosen for each well and the manner in which the new proposed monitoring wells will be completed is provided in the sections below.

A summary of all existing wells within the mine boundary that have been used for Baseline Data collection and Stage 1 abatement are summarized in Table 2 and shown on Figure 3.

The monitoring wells proposed for the monitoring network are presented in Table 3. The existing wells to be utilized in the network are identified in Table 3 by their current well name, i.e., “GWQ-##.” The proposed wells are identified as “PGWQ-##.” As indicated in Table 3, some of the existing monitor wells may go dry over time as a result of pit dewatering. As such the new proposed monitoring wells are designed to accommodate expected water-level declines and to provide groundwater monitoring for at least a 5-year period. As indicated previously and discussed with each relevant section of the monitoring plan later herein, Figures 4 through 6 are topographic maps showing a more detailed view of the units to be monitored and the proposed monitoring network.



**Table 2. Summary of existing monitoring well data for the Copper Flat Mine, Sierra County, New Mexico**

well name	well type	facility area	year drilled	casing diameter (inches)	total depth (ft bgl)	screen interval (ft bgl)	measuring-point elevation (ft amsl)	geologic unit	proposed use
GWQ96-22A	monitoring	pit	1996	2	244	174 to 244	5,596.17	andesite	monitor network
GWQ96-22B	monitoring	pit	1996	2	380	340 to 380	5,595.95	andesite	monitor network
GWQ96-23A	monitoring	pit	1996	2	101	50 to 100	5,489.84	quartz monzonite	monitor network
GWQ96-23B	monitoring	pit	1996	2	251	150 to 250	5,489.70	quartz monzonite	monitor network
GWQ11-24A	monitoring	pit	2011	2	90	60 to 90	5,517.37	quartz monzonite	monitor network
GWQ11-24B	monitoring	pit	2011	2	250	230 to 250	5,517.26	quartz monzonite	monitor network
GWQ11-25A	monitoring	pit	2011	2	100	70 to 100	5,533.60	quartz monzonite	monitor network
GWQ11-25B	monitoring	pit	2011	2	242	222 to 242	5,533.41	quartz monzonite	monitor network
GWQ11-26	monitoring	pit	2011	4	43	23 to 43	5,539.75	alluvium	monitor network
open pit	monitoring	pit	1982	-	-	-	5,430.00	quartz monzonite	monitor network
GWQ-1	supply	waste rock/mill site	1972	14/12	391	100 to 391	5,195.59	Santa Fe Group	monitor network
GWQ-3	supply	waste rock/mill site	1932	40 x 43	33	10 to 33	5,252.60	alluvium/andesite	monitor network
GWQ-5R	monitoring	waste rock/mill site	2011	4	120	80 to 120	5,412.80	andesite	monitor network
GWQ-6N	supply	Tailings Storage Facility (TSF)	na	8	85	na	5,395.36	andesite	water supply
GWQ-6S	supply	Tailings Storage Facility (TSF)	na	62	30	rocklined	5,382.77	andesite	water supply
GWQ-7	supply	Tailings Storage Facility (TSF)	1932	14	500	74 to 500	5,181.60	Santa Fe Group	water supply
GWQ-8	supply	waste rock/mill site	1931	8	148	81 to 148	5,216.94	Santa Fe Group	monitor network
GWQ-9	supply	Tailings Storage Facility (TSF)	1972	14	767	na	5,208.13	Santa Fe Group	water supply
GWQ-10	monitoring	Tailings Storage Facility (TSF)	1981	3	125	95 to 120	5,213.28	Santa Fe Group	monitor network
GWQ-11	monitoring	Tailings Storage Facility (TSF)	1981	3	70	40 to 65	5,196.44	alluvium/Santa Fe Group	plug and abandon
GWQ-12	monitoring	Tailings Storage Facility (TSF)	1981	3	110	80 to 105	5,237.28	Santa Fe Group	monitor network
GWQ94-13	monitoring	Tailings Storage Facility (TSF)	1994	5	106	74 to 104.5	5,200.47	Santa Fe Group	plug and abandon
GWQ94-14	monitoring	Tailings Storage Facility (TSF)	1994	5	159	127.5 to 157.5	5,192.69	Santa Fe Group	monitor network
GWQ94-15	monitoring	Tailings Storage Facility (TSF)	1994	4	142	112 to 142	5,183.21	Santa Fe Group	monitor network
GWQ94-16	monitoring	Tailings Storage Facility (TSF)	1994	5	46	25 to 45	5,197.41	alluvium	plug and abandon
GWQ94-17	monitoring	Tailings Storage Facility (TSF)	1994	4	150	120 to 150	5,198.14	Santa Fe Group	plug and abandon
GWQ94-18	monitoring	Tailings Storage Facility (TSF)	1994	4	51	10 to 50	5,194.83	alluvium	plug and abandon
GWQ94-19	monitoring	Tailings Storage Facility (TSF)	1994	4	53	10 to 50	5,203.36	alluvium	plug and abandon
GWQ94-20	monitoring	Tailings Storage Facility (TSF)	1994	4	338	288 to 338	5,203.94	Santa Fe Group	plug and abandon
GWQ94-21A	monitoring	Tailings Storage Facility (TSF)	1994	2	263	213 to 263	5,192.71	Santa Fe Group	monitor network
GWQ94-21B	monitoring	Tailings Storage Facility (TSF)	1994	2	315	285 to 315	5,192.22	Santa Fe Group	monitor network
GWQ13-28	monitoring	Tailings Storage Facility (TSF)	2013	4	198	150 to 190	5,178.16	Santa Fe Group	monitor network
IW-1	monitoring	Tailings Storage Facility (TSF)	1982	4	49	na	5,198.99	alluvium	plug and abandon
IW-2	monitoring	Tailings Storage Facility (TSF)	1982	4	46	na	5,208.01	alluvium	plug and abandon
IW-3	monitoring	Tailings Storage Facility (TSF)	1982	4	45	na	5,213.17	alluvium	plug and abandon
NP-1	monitoring	Tailings Storage Facility (TSF)	1981	2	110	90 to 105	5,188.75	Santa Fe Group	monitor network
NP-2	monitoring	Tailings Storage Facility (TSF)	1981	2	110	90 to 105	5,192.54	Santa Fe Group	plug and abandon
NP-3	monitoring	Tailings Storage Facility (TSF)	1981	2	90	70 to 85	5,199.73	Santa Fe Group	plug and abandon
NP-4	monitoring	Tailings Storage Facility (TSF)	1981	2	100	80 to 95	5,225.91	Santa Fe Group	monitor network
NP-5	monitoring	Tailings Storage Facility (TSF)	1981	2	90	70 to 85	5,199.21	Santa Fe Group	plug and abandon
MW-4	supply	off site	1975	6	1,500	123 to 1,500	5,146.12	Santa Fe Group	water supply

ft bgl – feet below ground level  
 ft amsl – feet above mean sea level

ft bmp – feet below measuring point  
 na – not available

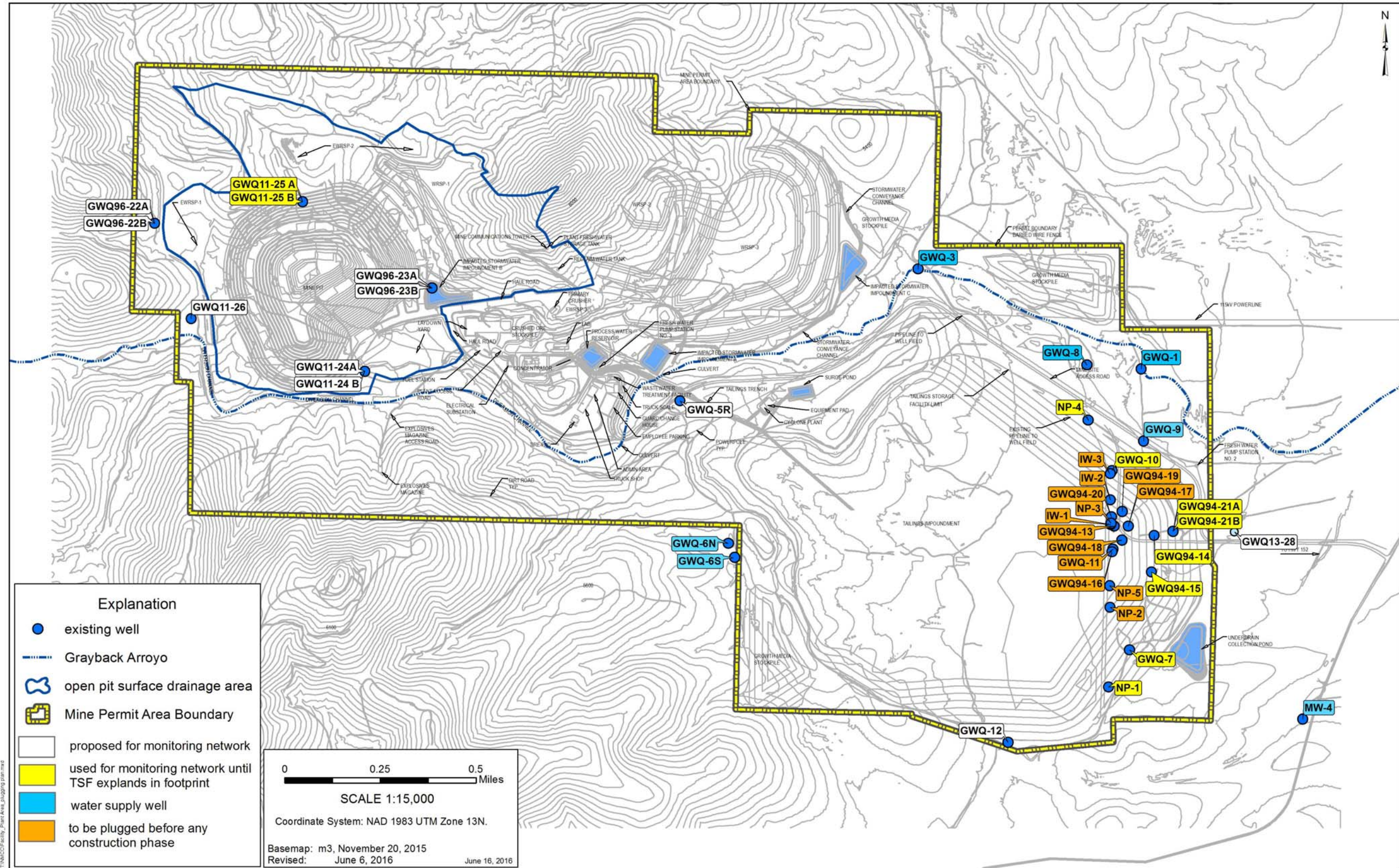


Figure 3. Map showing locations of existing wells colored coded for proposed use.

**Table 3. Proposed water-quality well network for the Copper Flat Mine, Sierra County, New Mexico**

well name <sup>4</sup>	unit(s) monitored	relation to unit(s) monitored	total depth <sup>5</sup> (ft)	screen interval <sup>5</sup> (ft)	screen length <sup>5</sup> (ft)	estimated maximum drawdown after first five years of mining(ft)	geologic unit	current depth to water (ft)
GWQ96-22A	entire site	up-gradient	244	174 to 244	70	100	andesite	55
GWQ96-22B	entire site	up-gradient	380	340 to 380	40	100	andesite	55
GWQ11-26 <sup>2</sup>	entire site	up-gradient	43	23 to 43	20	150	alluvium	39
GWQ96-23(A) <sup>3</sup>	open pit	perimeter	251	150 to 250	100	200	quartz monzonite	41
GWQ96-23(B) <sup>3</sup>	open pit	perimeter						
GWQ11-24A <sup>1</sup>	open pit	perimeter	90	60 to 90	30	200	quartz monzonite	50
GWQ11-24B <sup>2</sup>	open pit	perimeter	250	230 to 250	20	200	quartz monzonite	57
GWQ-5R <sup>2</sup>	Surge Pond	up-gradient	120	80 to 120	40	< 10	andesite	99
GWQ-1	TSF	off-gradient	391	100 to 391	291	< 10	Santa Fe Group	5
GWQ-8	TSF	off-gradient	148	81 to 148	67	< 10	Santa Fe Group	7
GWQ-10	TSF	off-gradient	125	95 to 120	25	< 10	Santa Fe Group	23
GWQ-12	TSF	off-gradient	110	80 to 105	25	< 10	Santa Fe Group	83
NP-1	TSF	down-gradient	110	90 to 105	15	< 10	Santa Fe Group	30
NP-4	TSF	off-gradient	100	80 to 95	15	< 10	Santa Fe Group	34
GWQ94-14	TSF	down-gradient	159	127.5 to 157.5	30	< 10	Santa Fe Group	7
GWQ94-15	TSF	down-gradient	142	112 to 142	30	< 10	Santa Fe Group	5
GWQ94-21A	TSF	down-gradient	263	213 to 263	40	< 10	Santa Fe Group	8
GWQ94-21B	TSF	down-gradient	315	285 to 315	30	< 10	Santa Fe Group	8
GWQ13-28	TSF	down-gradient	190	150 to 190	40	< 10	Santa Fe Group	156
PGWQ-1	open pit	perimeter	250	150 to 250	100	200	quartz monzonite	40
PGWQ-2	open pit	perimeter	375	275 to 375	100	150	quartz monzonite	115
PGWQ-3	WRSP-2 and -3	up-gradient	150	130 to 150	20	< 10	andesite	130
PGWQ-4	WRSP-2 and -3	down-gradient	105	85 to 105	20	< 10	andesite	85
PGWQ-5	WRSP-2 and -3	down-gradient	35	15 to 35	20	< 10	alluvium/Santa Fe Group	15
PGWQ-6	WRSP-2 and -3	down-gradient	55	35 to 55	20	< 10	Santa Fe Group/andesite	40
	Impacted Stormwater Impoundment C	down-gradient						
PGWQ-7	WRSP-2 and -3	down-gradient	40	20 to 40	20	< 10	Santa Fe Group/andesite	20
	Impacted Stormwater Impoundment C	up-gradient						
PGWQ-8	WRSP-2 and -3	down-gradient	45	25 to 45	20	< 10	alluvium/andesite	25
PGWQ-9	Surge Pond	down-gradient	100	80 to 100	20	< 10	alluvium/andesite	80
PGWQ-10	Impacted Stormwater Impoundment A	down-gradient	50	30 to 50	20	< 10	alluvium/andesite	20
PGWQ-11	Process Water Reservoir	down-gradient	120	100 to 120	20	10	alluvium/andesite	100
PGWQ-12	Impacted Stormwater Impoundment A and Process Water Reservoir	up-gradient	120	100 to 120	20	20	alluvium/andesite	100
PGWQ-13	WRSP-2 and -3	down-gradient	35	15 to 35	20	< 10	alluvium/Santa Fe Group	15
	TSF	perimeter						
PGWQ-14	TSF	perimeter	35	15 to 35	20	< 10	alluvium/Santa Fe Group	20
PGWQ-15	TSF	down-gradient	50	30 to 50	20	< 10	alluvium/Santa Fe Group	30
PGWQ-16	TSF	down-gradient	180	160 to 180	20	< 10	Santa Fe Group	160
PGWQ-17	Underdrain Collection Pond	down-gradient	75	55 to 75	20	< 10	alluvium/Santa Fe Group	55
PGWQ-18	TSF	up-gradient	120	100 to 120	20	< 10	andesite	100
PGWQ-19	TSF	down-gradient	75	55 to 75	20	< 10	Santa Fe Group	50
PGWQ-20	WRSP-2 and -3	down-gradient	30	20 to 30	10	< 10	alluvium	10

<sup>1</sup> will likely go dry as a result of pit dewatering within first 5 years of mining

<sup>2</sup> may go dry towards the end of mining as a result of pit dewatering

<sup>3</sup> will likely go dry as a result of pit dewatering, but not within first 5 years of mining

<sup>4</sup> "GWQ" designation is for existing wells and "PGWQ" is for new proposed wells

<sup>5</sup> estimated for proposed new wells based on groundwater model (JSAI, 2014) results after 5 years of mining

TSF - tailings storage facility  
WRSP – waste rock stockpile

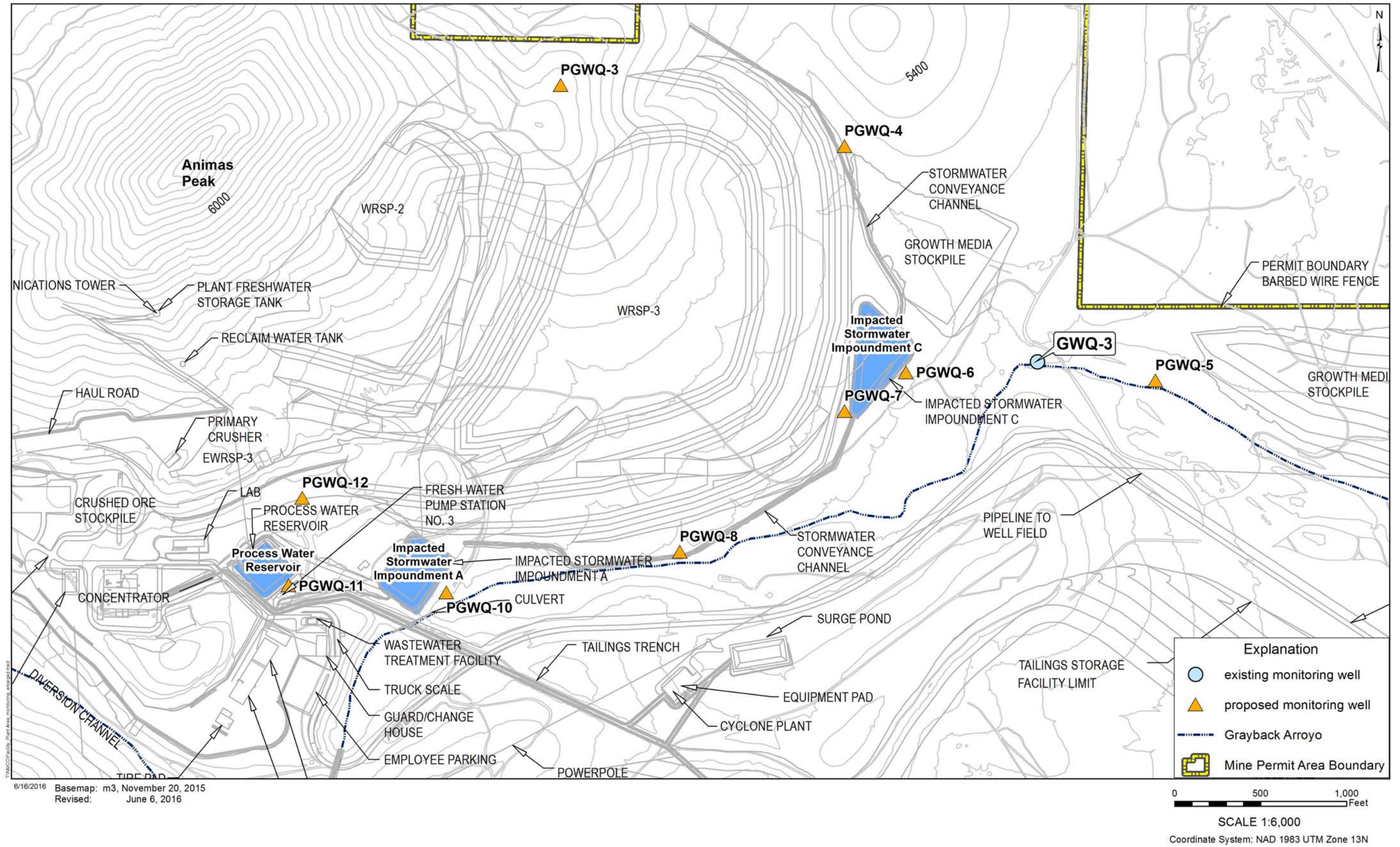


Figure 4. Monitoring wells location detail for WRSP-2 and -3, and Impact Stormwater Impoundments A and C.

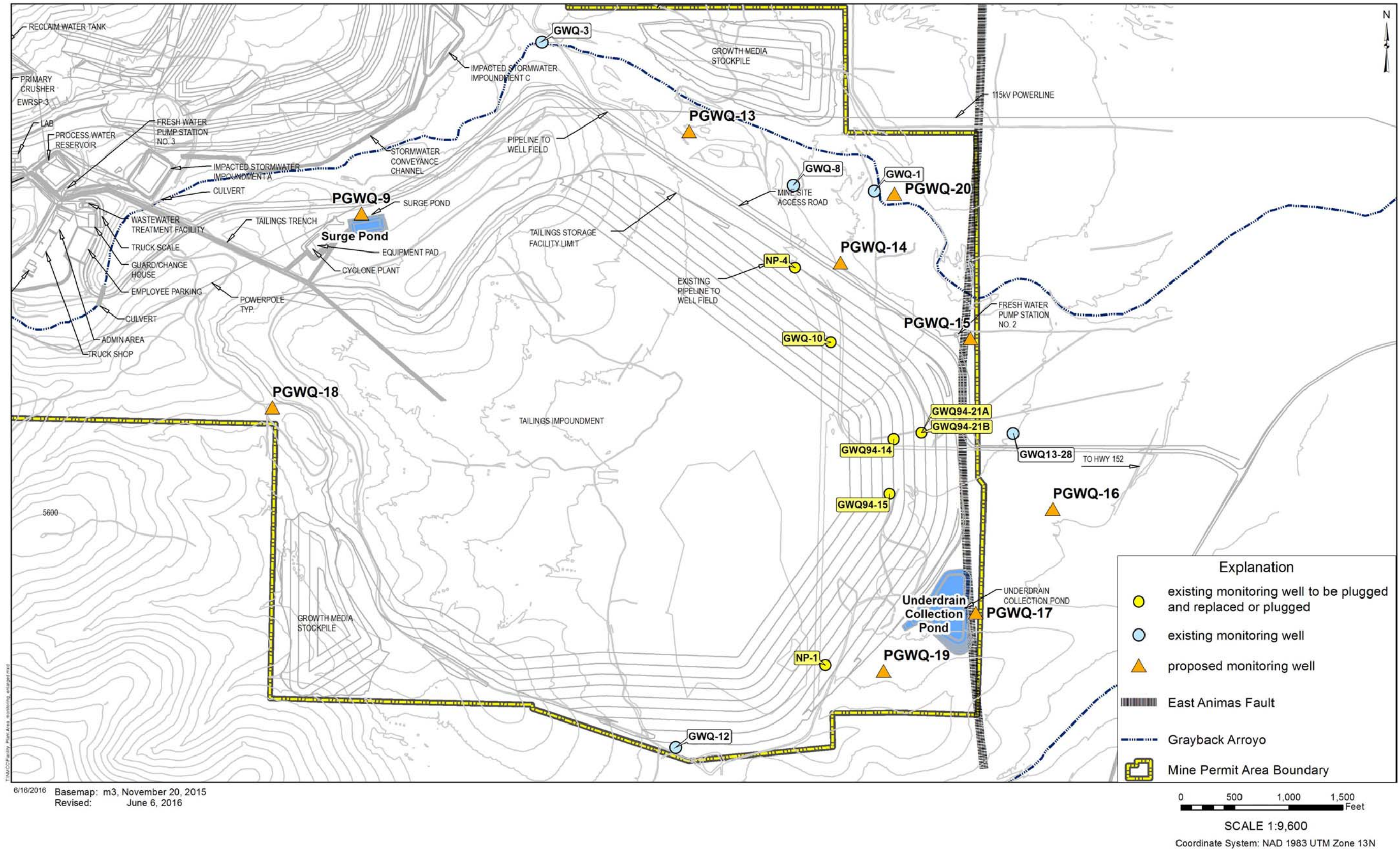


Figure 5. Monitoring wells location detail for Tailings Storage Facility, Underdrain Collection Pond, and Surge Pond.

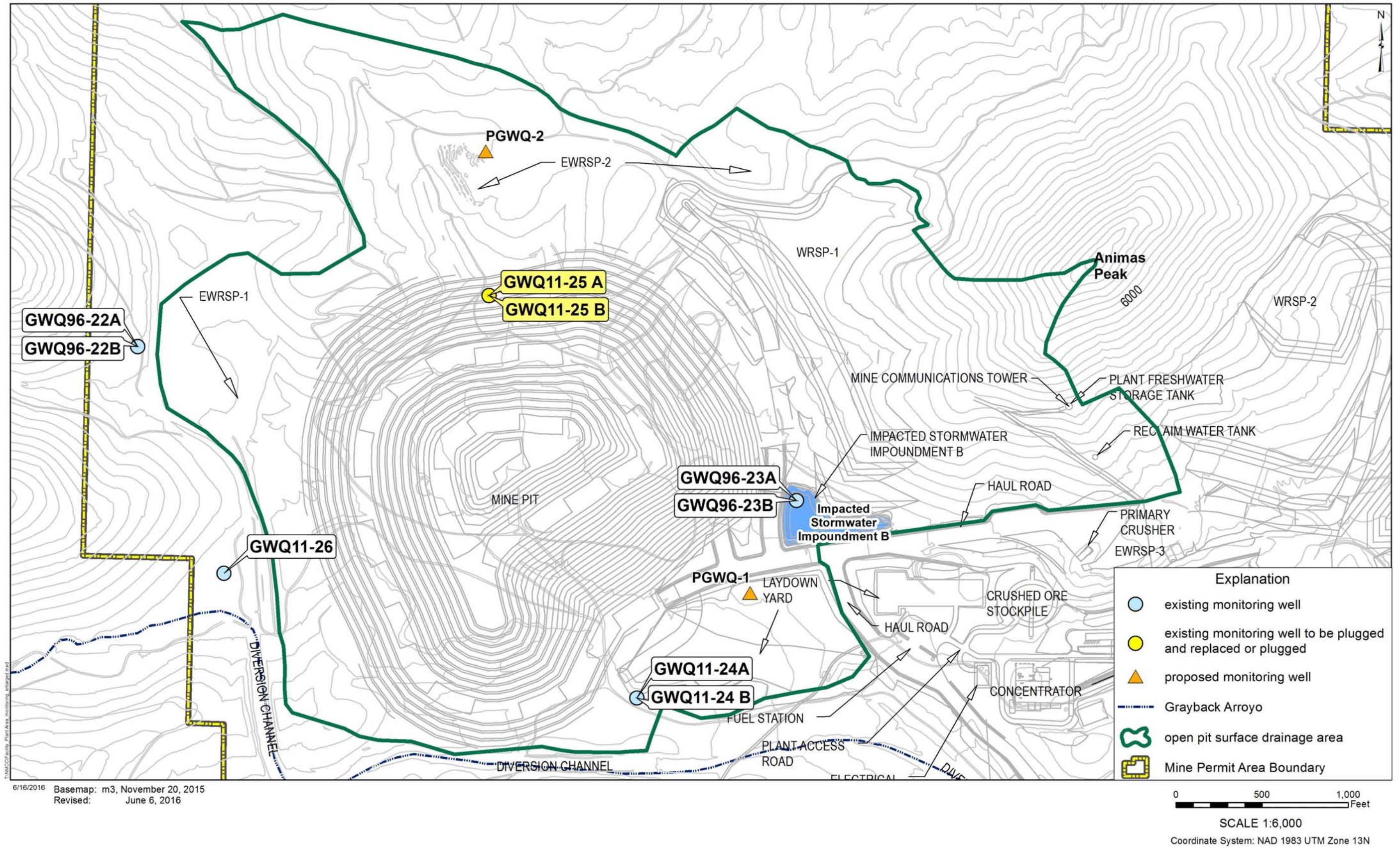


Figure 6. Monitoring wells location detail for mine pit and WRSP-1.

## **2.1 Existing Monitoring Wells (20.6.7.28.B.(1))**

An existing monitoring well network has been used previously for Baseline Data assessment and Stage 1 Abatement plan monitoring (Table 2). The water-level data presented on Figure 2 were collected from the Stage 1 Abatement monitoring (JSAI, 2014b).

The Stage 1 Abatement monitoring network was previously approved by the New Mexico Environment Department (NMED). As noted above, JSAI has selected a subset of the existing well network to be included in this proposed Water-Quality Monitoring Plan as identified in Table 3. Subsection 20.6.7.B.(1) NMAC provides that an existing monitoring well can be an approved location for groundwater monitoring provided the monitoring well location was previously approved by the NMED. The wells selected by JSAI for inclusion in the network provide the best opportunity, in combination with the proposed wells, to provide data required by 20.6.7.28 NMAC. Some of the existing monitoring wells used in the previous studies will not be available for the long-term as they will be plugged and abandoned, so proposed mine facilities can be built. Others, such as GWQ-1, GWQ-3, and GWQ-8 may not be of the quality of construction or may have other operational issues making them less desirable for use in this proposed monitoring network.

The proposed use of the existing wells for the monitoring network is summarized in Table 2. Many of the existing monitoring wells are located within the footprint of the Phase 1 construction of the Tailings Storage Facility (TSF) and will be plugged and abandoned prior to construction. Some are water supply wells that will be maintained for mine site supply. Several of the existing monitoring wells will be used as part of the monitoring network described in the following sections.

## **2.2 Waste Rock Stockpiles and Tailings Impoundments (20.6.7.28.B.(2))**

This section describes the proposed monitoring network for the Waste Rock Stockpiles (WRSP) and the TSF located outside of the open pit surface drainage area, pursuant to 20.6.7.28.B.(2).

### **2.2.1 Waste Rock Stockpile 1**

The WRSP-1 (surface topography and under topography) is within the open pit surface drainage area, which does not require monitoring.

### 2.2.2 Waste Rock Stockpiles 2 and 3

Waste Rock Stockpiles 2 and 3 (WRSP-2 and -3), shown on Figure 4, are located outside the open pit surface drainage area. Because of their close proximity to each other, the Monitoring Plan will consider them as one WRSP regarding groundwater monitoring. The monitoring network in this area will monitor groundwater quality and potential discharges around and down-gradient of WRSP-2 and -3 with four proposed monitoring wells (PGWQ-4, PGWQ-7, PGWQ-8, and PGWQ-12) located around the perimeter of the WRSPs, close to the toe of the final configuration, as shown on Figure 4. Groundwater quality and hydraulic-gradient will also be defined by these four perimeter wells.

These perimeter wells are located to monitor groundwater below WRSP-2 and -3. For example, PGWQ-4 is located in the drainage channel along the northeast side of WRSP-2 and -3, and PGWQ-7 and PGWQ-8 will be located directly down-gradient of WRSP-2 and -3 in the channel of the largest drainages underlying the proposed stockpiles. Proposed monitoring well PGWQ-12 will be off-gradient of WRSP-2 and -3. Proposed monitoring well PGWQ-3 will be up-gradient of WRSP-2 and -3.

In addition to the four proposed perimeter wells, proposed monitoring wells PGWQ-5, PGWQ-13, and PGWQ-20 will monitor groundwater quality farther down-gradient of WRSP-2 and -3 (see Fig. 1). PGWQ-5 will replace GWQ-3, which is also shown on Figures 1 and 4 but is not part of this proposed Monitoring Plan. GWQ-3 is an existing well that has provided data for other purposes. The well is considered questionable for use in a long-term monitoring program as it is truly a historic cistern box. The immediate area around GWQ-3 is classified as a cultural property. Therefore, the proposed location of PGWQ-5 may vary slightly in the field based on access and cultural resource considerations.

The proposed monitoring wells will be installed at least 180 days before emplacement of waste rock to allow sampling prior to discharge, pursuant to 20.6.7.28.B.(2)(a).

### 2.2.3 Tailings Storage Facility (TSF)

The proposed groundwater monitoring network for the TSF is shown on Figure 5. Existing monitoring wells will be used to the maximum extent possible. Existing monitoring wells NP-1, NP-4, GWQ-10, GWQ-12, GWQ94-14, GWQ94-15, GWQ94-21(A and B), and GWQ13-28 will be used as the initial monitoring network for monitoring groundwater quality



and potential discharges around and down-gradient of the TSF. As the TSF expands, monitoring wells in the expanded footprint will be plugged and abandoned and replaced by proposed monitoring wells. The replacement program is as follows:

- PGWQ-14 will replace NP-4
- PGWQ-15 will replace GWQ-10
- PGWQ-16 will replace GWQ94-14, GWQ94-15, and GWQ94-21(A and B)
- PGWQ-19 will replace NP-1

The proposed monitoring network will monitor groundwater quality and potential discharges around and down-gradient of the TSF with three perimeter wells (NP-4, GWQ-10, GWQ-12), and six down-gradient wells (NP-1, GWQ94-14, GWQ94-15, GWQ94-21(A and B), and GWQ13-28). Down-gradient well GWQ13-28 is located on the east and down-dropped side of the East Animas Fault identified in JSAI (2014b) (see Fig. 4). Existing supply wells GWQ-1 and GWQ-8 will be monitored for water levels to help define the direction of groundwater flow and hydraulic-gradient along Grayback Arroyo on the north side of the TSF. Groundwater quality and hydraulic-gradient will be defined by the proposed off-gradient and down-gradient monitoring wells.

All of the proposed monitoring wells will be installed at least 180 days before discharge of tailings to allow sampling prior to discharge, pursuant to 20.6.7.28.B.(2)(a).

### **2.3 Process Water and Impacted Stormwater Impoundments (20.6.7.28.B.(3))**

Copper Flat will construct a number of impoundments and ponds to manage water as shown on Figure 1. There will be three impacted storm-water impoundments (Impoundments A, B, and C) constructed to control and manage storm-water runoff from the WRSPs. There will also be an underdrain collection pond to collect water from below the Tailings Impoundment and runoff from the exterior surface of the dam. There will be a Surge Pond location coincident with the cyclone plant at the TSF to manage potential process water upset conditions. There will be a Process Water Reservoir that will receive recycled water from the Tailings Impoundment, the Underdrain Pond, the Impacted Stormwater Impoundments, as well as fresh water from the facility well field.

With the exception of Impacted Stormwater Impoundment B, all of these ponds and impoundments containing process water or impacted water will have down-gradient monitoring

wells located with 75 ft of the impoundment, pursuant to 20.6.7.28.B.(3). Down-gradient monitoring is shown on the figures identified as follows:

- PGWQ-11 down-gradient of the Process Water Reservoir (Fig. 4)
- PGWQ-10 down-gradient of Impacted Stormwater Impoundment A (Fig. 4)
- PGWQ-6 down-gradient of Impacted Stormwater Impoundment C (Fig. 4)
- PGWQ-9 down-gradient of the Surge Pond (Fig. 5)
- PGWQ-17 down-gradient of the Underdrain Collection Pond (Fig. 5)

Impacted Stormwater Impoundment B will collect storm-water runoff from WRSP-1. WRSP-1 and the Impacted Stormwater Impoundment B are located within the open pit surface drainage area as shown in Figures 1 and 6. As such, no down-gradient monitoring is required for these.

The proposed location for PGWQ-9 is down-gradient from the proposed Surge Pond with respect to land surface. While the location of PGWQ-9 appears to be off-gradient with respect to regional groundwater flow direction, it is important to locate the monitoring well down-gradient with respect to local groundwater gradient and to be able to monitor potential releases to groundwater as close to the pond as possible. Therefore, the proposed location of PGWQ-9 is in the best possible location for detecting potential discharges from the Surge Pond.

All of the proposed monitoring wells will be installed at least 180 days before discharging to the impoundments to allow sampling prior to discharge, pursuant to 20.6.7.28.B.(3)(a).

#### **2.4 Open Pit (20.6.7.28.B.(4))**

The proposed groundwater monitoring network will include a sufficient number of monitoring wells around the perimeter of the planned open pit to monitor groundwater quality and hydraulic-gradient around the open pit. The proposed open pit monitoring network includes three existing monitoring wells located up-gradient of the open pit (GWQ96-22A and B), and GWQ11-26), and six monitoring wells (four existing and two new) for the open pit perimeter (GWQ96-23(A and B); GWQ11-24(A and B), PGWQ-1, and PGWQ-2). The locations of these wells are shown on Figure 6. The location for PGWQ-2 is based on the closest location to the well being replaced (GWQ11-25(A and B)) that will not be in the way of other planned mine facilities. The other objective to monitoring the open pit is to maintain perimeter wells on the east, west, north, south, and southeast sides.

Proposed monitoring well PGWQ-1 will help define the hydraulic containment on the southeast corner of the open pit area. Proposed monitoring well PGWQ-2 will replace GWQ11-25(A and B), which will be destroyed during excavation of the open pit. The proposed location for PGWQ-2 will be off-gradient of the open pit as shown on Figure 6. GWQ96-23(A and B) will be maintained, but it is possible that the existing wells may be replaced so Impacted Stormwater Impoundment B can be constructed. If it is determined that GWQ96-23(A and B) need to be replaced, a monitoring well identical to PGWQ-1 will be drilled adjacent to the north side of Impacted Stormwater Impoundment B.

## **2.5 Up-Gradient of Each Potential Source (20.6.7.28.B.(5))**

As specified in 20.6.7.28.B.(5), a minimum of one monitoring well shall be located up-gradient of each new unit to establish up-gradient groundwater quality conditions not likely to be affected by each contamination source that is being monitored. Using the groundwater elevation contours from Figure 2, the following monitoring is proposed:

- GWQ96-22(A and B) shown on Figures 1 and 6 will provide up-gradient monitoring for the open pit.
- PGWQ-3 shown on Figures 1 and 4 will provide up-gradient monitoring for the WRSP-2 and -3.
- Proposed groundwater monitoring well PGWQ-12 shown on Figures 1 and 4 will establish up-gradient groundwater quality conditions for the Process Water Reservoir and Impacted Stormwater Impoundment A.
- PGWQ-7 shown on Figures 1 and 4 will provide up-gradient monitoring of Impacted Stormwater Impoundment C below the WRSP-3.
- Monitoring well GWQ-5R shown on Figures 1 and 5 will provide up-gradient monitoring for the surge pond.
- PGWQ-18 shown on Figures 1 and 5 will establish up-gradient groundwater quality conditions for the Tailings Storage Facility and the Underdrain Collection Pond.

All of the proposed monitoring wells will be installed at least 180 days before discharge of tailings or other contaminants to allow sampling prior to discharge, pursuant to 20.6.7.28.B.(5)(a).

## **2.6 Up-Gradient of Copper Mine Facilities (20.6.7.28.B.(6))**

NMAC 20.6.7.28.B.(6) requires up-gradient groundwater monitoring for a copper mine facility. Three existing monitoring wells located up-gradient of Copper Flat Mine will establish up-gradient groundwater quality conditions that are not affected by any potential contamination sources at Copper Flat Mine (GWQ96-22(A and B), GWQ11-26, and PGWQ-18). As shown on Figure 2, these wells are up-hydraulic-gradient of all proposed facilities.

All of the proposed monitoring wells will be installed at least 180 days before discharge of tailings or other contaminants to allow sampling prior to discharge, pursuant to 20.6.7.28.B.(6).

## **3.0 MONITORING WELL CONSTRUCTION AND COMPLETION 20.6.7.28.C, D & E**

Monitoring well construction and completion requirements are specified in 20.6.7.28.D NMAC. The requirements as contained in Subsections 20.6.7.28.C, D, and E are well understood by New Mexico Copper Corporation (NMCC) and its drilling contractors. NMCC will ensure that its drilling contractors adhere to the construction and completion and well screening requirements set forth in paragraphs (1) through (13) of 20.6.6.28.D NMAC. Drilling specifications detailing these requirements will be prepared to solicit bids from qualified well drillers licensed in New Mexico. The drilling specifications will define drilling methods, well construction materials, and wellhead completion details that conform to the following:

- Monitoring well identification tags in 20.6.7.28.C NMAC
- Monitoring well construction and completion in 20.6.7.28.D NMAC.
- Monitoring well permits from the Office of the State Engineer in 20.6.7.28.E NMAC

## **3.1 Proposed Monitoring Well Depth and Screen Interval**

Significant drawdown will occur around the open pit as a result of the first 5 years of open pit mining and dewatering. Model-predicted drawdown representative of the first 5 years of mining is illustrated on Figure 7. Proposed depths and screen intervals have been adjusted for monitoring wells within the 20-ft model-predicted drawdown contour. Proposed monitoring wells outside of the area of drawdown will be constructed with 20-ft screens spanning the water table as specified in 20.6.7.28.D.(7)(a) NMAC. Proposed well depths and screen intervals can be referenced from Table 3.

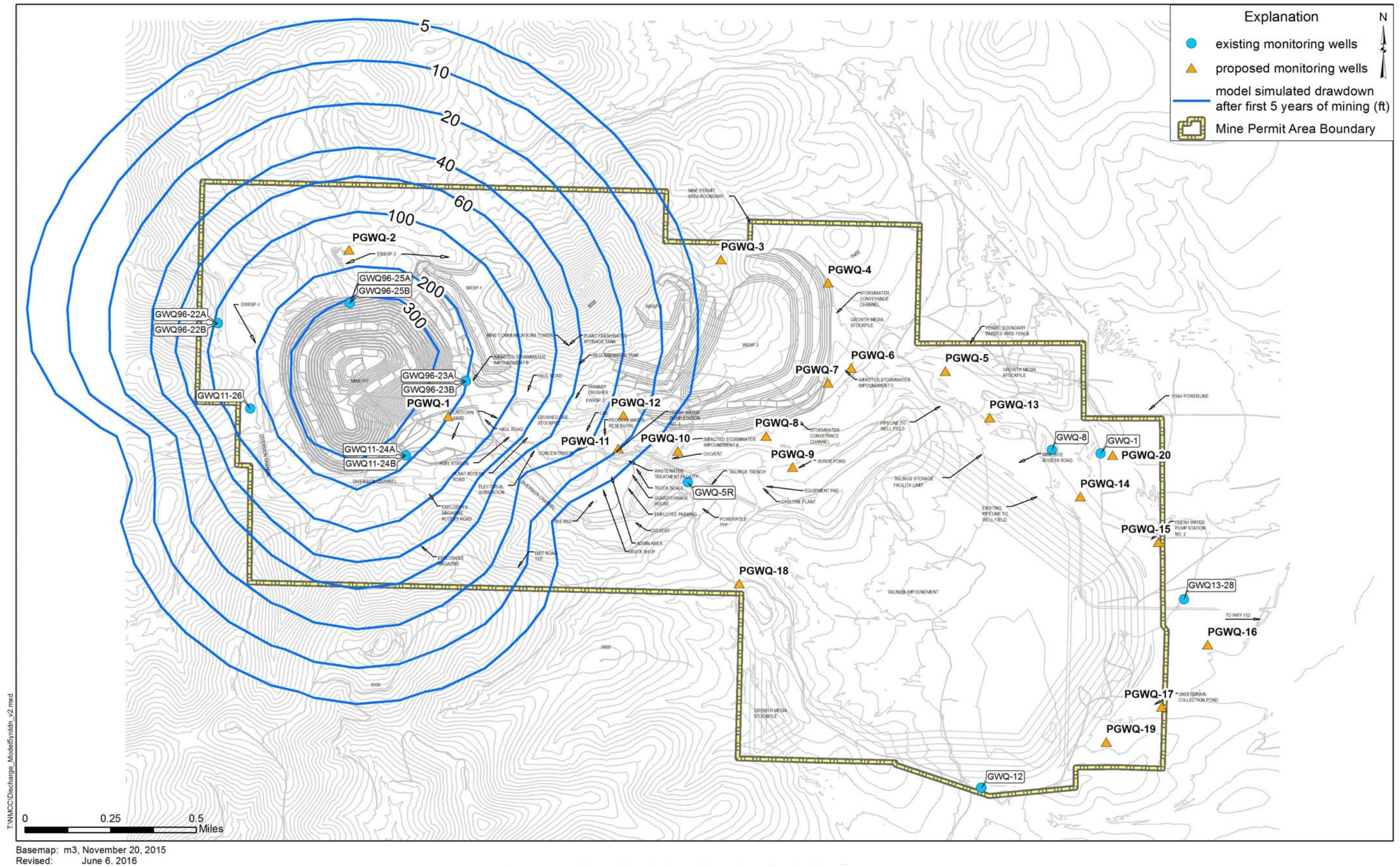


Figure 7. Model-predicted mine pit drawdown for the first 5 years.

Except for open pit perimeter monitoring wells, proposed monitoring wells will be located to detect a discharge to groundwater at the earliest possible occurrence. Depths and screen intervals for proposed monitoring wells are estimated to the nearest 5-ft increment (see Table 3).

Due to excessive model-predicted drawdown around the open pit, proposed perimeter monitoring wells (PGWQ-1 and PGWQ-2) will have a 100-ft screen length. It is anticipated that the water level will draw down to the top of the screen in these proposed open pit perimeter monitoring wells during the first few years of mining and open pit dewatering

#### **4.0 GROUNDWATER SAMPLING 20.6.7.28.E THROUGH I**

Existing monitoring wells were previously sampled as a result of Baseline Data assessment (INTERA, 2012) and Stage 1 Abatement (JSAI, 2014). However, more than half of the groundwater monitoring wells proposed for this Water-Quality Monitoring Plan will be new, and thus, have no background data. As required in 20.6.7.28. I. NMAC, a permittee shall submit a proposal for quarterly groundwater monitoring from each newly installed monitoring required pursuant to this section. Proposed analytes for quarterly monitoring are summarized in Table 4. The proposed analyte list is based on field parameters required in 20.6.7.28.F NMAC, analytes in 20.6.2.3103 NMAC applicable to expected discharges at Copper Flat Mine, and analytes required in 20.6.7.28.I NMAC.

Radioactive and hydrocarbon analytes will be analyzed from each well in the proposed monitoring network during the first year of the quarterly sampling event to establish background conditions. Subsequent sampling events will only include the field parameters and inorganic analytes listed in Table 4, unless background sampling or other potential discharge justifies sampling for the full list in Table 4.

Groundwater sampling procedures will be in accordance with 20.6.7.28.F NMAC, and include the following:

- Measurement of depth to water as specified in 20.6.7.28.F.(1) NMAC
- Proper purging of monitoring well prior to sampling as specified in 20.6.7.28.F.(2) NMAC
- Collection of field parameters following purging as specified in 20.6.7.28.F.(3) NMAC
- Collection of samples for laboratory analysis as specified in 20.6.7.28.F.(5) NMAC

**Table 4. Proposed analytes for Water-Quality Monitoring Plan**

analyte	unit	discharge standard	comment
pH	standard units	between 6 and 9	field measurement
specific conductance	µS/cm	none	field measurement
temperature	Celsius	none	field measurement
pH	standard units	between 6 and 9	lab measurement
alkalinity (HCO <sub>3</sub> ; CO <sub>3</sub> )	mg/L as CaCO <sub>3</sub>	none	inorganic
total dissolved solids (TDS)	mg/L	1,000	inorganic
chloride	mg/L	250	inorganic
fluoride	mg/L	1.6	inorganic
nitrate (NO <sub>3</sub> as N)	mg/L	10.0	inorganic
sulfate	mg/L	600	inorganic
arsenic	mg/L	0.1	inorganic
barium	mg/L	1.0	inorganic
cadmium	mg/L	0.01	inorganic
chromium	mg/L	0.05	inorganic
copper	mg/L	1.0	inorganic
iron	mg/L	1.0	inorganic
lead	mg/L	0.05	inorganic
manganese	mg/L	0.2	inorganic
total mercury	mg/L	0.002	inorganic
selenium	mg/L	0.05	inorganic
silver	mg/L	0.05	inorganic
uranium	mg/L	0.03	inorganic
zinc	mg/L	10.0	inorganic
radium 226 and 228	pCi/L	30.0	1 <sup>st</sup> quarter only
benzene	mg/L	0.01	1 <sup>st</sup> quarter only
ethylbenzene	mg/L	0.75	1 <sup>st</sup> quarter only
toluene	mg/L	0.75	1 <sup>st</sup> quarter only
total xylenes	mg/L	0.62	1 <sup>st</sup> quarter only
PCB's	mg/L	0.001	1 <sup>st</sup> quarter only
carbon tetrachloride	mg/L	0.01	1 <sup>st</sup> quarter only
1,2-dichloroethane (EDC)	mg/L	0.01	1 <sup>st</sup> quarter only
1,1-dichloroethane	mg/L	0.005	1 <sup>st</sup> quarter only
1,1,2,2-tetrachloroethylene	mg/L	0.02	1 <sup>st</sup> quarter only
methylene chloride	mg/L	0.1	1 <sup>st</sup> quarter only
chloroform	mg/L	0.1	1 <sup>st</sup> quarter only
1,1-dichloroethane	mg/L	0.025	1 <sup>st</sup> quarter only
ethylene dibromide (EDB)	mg/L	0.0001	1 <sup>st</sup> quarter only
1,1,1-trichloroethane	mg/L	0.06	1 <sup>st</sup> quarter only
1,1,2-trichloroethane	mg/L	0.01	1 <sup>st</sup> quarter only
1,1,2,2-tetrachloroethane	mg/L	0.01	1 <sup>st</sup> quarter only
vinyl chloride	mg/L	0.001	1 <sup>st</sup> quarter only
PAHs	mg/L	0.03	1 <sup>st</sup> quarter only
benzo-a-pyrene	mg/L	0.0007	1 <sup>st</sup> quarter only

µS/cm - microSiemens per centimeter  
 mg/L - milligrams per liter

pCi/L picocuries per liter

## 5.0 REPORTING

There are several reporting requirements specified in 20.6.7.28 NMAC. The following is a summary of proposed reporting requirements:

- Monitoring well completion reports submitted 60 days after completion of newly installed monitoring well. The reports shall contain construction and lithologic logs, survey results, wellhead completion detail, map showing location, depth to water measurement(s), water-level elevation contour map, and results from groundwater samples. Other details can be referenced from 20.6.7.28 subsections J and K NMAC.
- Semi-annual groundwater elevation contour maps, as detailed in 20.6.7.28.L NMAC, will also include the extent of the existing open pit surface drainage area.

## 6.0 SURFACE-WATER SAMPLING

There are no perennial streams of the state within proposed Copper Flat Mine permit area that would require routine monitoring defined in NMAC 20.6.7.28.M; however, Grayback Arroyo is an ephemeral stream that drains the Copper Flat Mine site as shown in Figure 1.

A surface-water sampling network along Grayback Arroyo was established for Stage 1 Abatement data collection, and will be used as part of the monitoring network for the proposed Copper Flat Mine in fulfillment of 20.6.7.28.N NMAC. An additional storm-water sampling location (SWQ-5) is proposed in Grayback Arroyo between SWQ-1 and SWQ-2. The locations where auto-samplers (SWQ-1 through SWQ-4) have been installed to collect storm-water runoff samples are shown on Figure 1. Locations SWQ-1 through SWQ-3 are the same locations sampled from previous studies dating back to the 1970s, and subsequently have historical water-quality data. An auto-sampler was installed at SWQ-4 during 2014.

SWQ-1 will provide background storm water quality data, and SWQ-2 through SWQ-5 will provide down-gradient water-quality data (see Fig. 1). Collected storm-water samples will be analyzed for the list of inorganic analytes in Table 4.



In addition, 20.6.7.28.N NMAC requires the quarterly monitoring of process water, tailings slurry, impacted storm water, seeps, and springs at a copper mine facility. There are no seeps or springs at the site to be included in the sampling program, and the open pit will be dewatered during mining operations. The following surface waters at the site are proposed for quarterly sampling, to meet the requirements of 20.6.7.28.N NMAC:

1. Impacted Stormwater Impoundment A
2. Impacted Stormwater Impoundment B
3. Impacted Stormwater Impoundment C
4. Process Water Reservoir
5. Surge Pond
6. Underdrain Collection Pond
7. Mine Pit Water
8. Seeps or springs identified outside of the open pit surface drainage area

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- [JSAI] John Shomaker & Associates, Inc., 2014b, Results from first year of Stage 1 abatement investigation at the Copper Flat Mine site near Hillsboro, New Mexico: consultant's report prepared by John Shomaker & Associates, Inc. for New Mexico Copper Corporation, May 2014.



# Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico

Report Prepared for

**THEMAC Resources Group Ltd.**



Report Prepared by



SRK Consulting (U.S.), Inc.  
SRK Project Number 191000.03  
December 2017

# **Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico**

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## Executive Summary

SRK Consulting (SRK) has undertaken a predictive geochemical modeling exercise to assess future pit lake chemistry associated with the Copper Flat Project, New Mexico and to compare this to existing pit lake water quality. This work has been undertaken on behalf of New Mexico Copper Corporation (NMCC – a subsidiary of THEMAC Resources Group Ltd. [THEMAC]) to demonstrate compliance with New Mexico Mining Act regulations “Performance and Reclamation Standards for New Mining Operations” at 19.10.6.603 NMAC, applicable to the future pit water body, specifically that:

- The operations must be planned and conducted to minimize change in the hydrologic balance in both the permit and potentially affected areas; and
- Reclamation must result in a hydrologic balance similar to pre-mining conditions.

The work also forms part of the geochemical characterization study to assess the Acid Rock Drainage and Metal Leaching (ARDML) potential of the Project.

The Copper Flat Project is a porphyry copper-molybdenum deposit located on the western margin of the Rio Grande Rift. The deposit also contains minor, but potentially recoverable, gold and silver mineralization. The deposit is hosted by a quartz monzonite stock that intrudes a sequence of andesitic volcanic rocks.

Preliminary pit lake predictions for the Project were presented in the SRK December 2014 report entitled *‘Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico’*, which was presented to Regulatory authorities to generate discussion and input. A number of modifications and refinements have been made to the pit lake models since this report was submitted, including:

- Incorporation of the Feasibility Study geologic block model;
- Incorporation of the current open pit design, which is detailed in the 2017 Mine Operation and Reclamation Plan (2017 MORP pit);
- Refinement of the pit wall composition to include delineation of material types by primary lithology, oxidation and mineralized versus weakly-mineralized material;
- Refinement of humidity cell test (HCT) inputs to include separate source terms for major and trace elements, reflecting the different processes that control their release;
- Refinement of mineral equilibrium phases based on predicted chemistry;
- Refinement of the water balance to use a reduced annual evaporation rate of 50 inches and to include a separate runoff term for reclaimed areas in the pit and the open pit watershed;
- Revisions to the groundwater chemistry inputs; and
- Incorporation of pit management and reclamation measures; including rapid fill of the pit and reclamation of the pit haul road and other areas within the pit and the pit watershed.

The objective of the report is to provide an analysis that demonstrates that future pit lake water quality results in a water body with similar chemistry to that of pre-mining conditions upon implementation of the reclamation actions proposed by NMCC in its MORP and Reclamation Plan, including rapid-fill of the open pit after closure of the mine.

Geochemical predictions were developed for three scenarios, including: (i) a calibration model for the existing pit lake; (ii) a natural fill model for the future unreclaimed pit; and (iii) a rapid fill model for the future reclaimed pit. Rapid fill has been proposed as the water quality component of NMCC's reclamation strategy for the future pit lake. It will include filling the pit with 2,202 acre-feet of good quality water from the production water supply wells during the first six months of groundwater recovery and pit infilling.

This report describes the approach taken for the revised pit lake predictive modeling effort, details the assumptions made, and presents the results of the revised pit lake geochemical predictions.

## **Model Calibration**

The results of the existing pit lake model show good calibration of constituents, demonstrating water quality can be predicted with a good degree of accuracy for the future pit lake. The baseline water quality data utilized in the calibration model are data for existing water quality chemistry in the pit lake between 2010 and 2013. This is a subset of the entire baseline data generated between 1998 and July 2017. The full data set was utilized in comparing existing water quality chemistry to projected future water quality of the pit lake in discussed in Sections 5 and 6.

## **Unreclaimed Fill Scenario**

In the unreclaimed pit scenario, allowing the pit to fill naturally will result in the pit walls and benches being exposed over a much longer period of time, i.e., approximately 150 years, before the pit lake reaches hydrologic equilibrium. In this scenario, the proposed future Copper Flat open pit is expected to be seasonally stratified but otherwise well-mixed, oxygenated and not acidic. Waters are predicted to be moderately alkaline (pH 7.9 – 8.2), primarily due to the buffering capacity of the inflowing groundwater. During the early stages of pit infilling (i.e., the first six months post-closure), removal/flushing of soluble salts from the pit walls is likely to result in a spike in boron, lead, mercury, manganese, molybdenum, nickel, selenium, vanadium, zinc and sulfate in the early pit lake. The effects of this initial flush will be dissipated by inflowing groundwater and precipitation, and pit lake chemistry will then evolve over time, with some parameters increasing in concentration as a result of evaporation effects. This is similar to the trends observed in the existing pit lake where elemental concentrations have increased since the start of pit infilling in response to evapoconcentration.

A comparison of predicted pit lake water chemistry for the unreclaimed fill scenario to chemistry measured in the existing pit lake between 1989 and 2017 demonstrates that the concentrations of the majority of constituents are comparable to existing concentrations, and therefore water quality of the future pit lake is expected to be similar to existing pit lake water quality.

## **Reclaimed Fill Scenario**

Rapidly refilling the pit with water from the water supply wells during the first six months post-closure will result in a better initial water quality within the pit lake due to the good quality of the water that will be used. The long-term result is that the effects of evapoconcentration are not as pronounced as the pit lake reaches hydrogeologic equilibrium, and predicted concentrations of many major ions and trace elements will remain lower than in the unreclaimed fill scenario. This is particularly the case for constituents such as boron, sulfate and chloride, which are strongly influenced by evaporation effects and are predicted to be much lower in concentration for the rapid fill scenario compared to the natural fill scenario. In addition, the rapid fill will also quickly submerge walls and benches within six months and thus limit the exposure of sulfide minerals to oxygen, which will reduce trace element release into the pit lake. By contrast, the unreclaimed fill scenario allows the pit to fill naturally and results in the pit walls and benches being exposed over a much longer period of time, i.e., approximately 150 years, before the pit lake reaches hydrologic equilibrium. A comparison of predicted pit lake chemistry for the reclaimed pit rapid fill scenario to chemistry measured in the existing pit lake between 1989 and 2017 demonstrates that concentrations of the majority of

predicted constituent concentrations are comparable to existing concentrations and therefore, water quality of the future pit lake is expected to be similar to existing pit lake water quality.

## **Conclusions**

Based on the model results presented herein, the changes to the hydrologic balance of the future pit water body that will form post-mining will be nil or minimal and the water quality will be very similar to that of the existing pit lake. The existing pit lake at Copper Flat is an artificial water body created as a result of mineral extraction with little or limited ability to sustain aquatic life (Aquatic Consultants, Inc. 2014). The post-mining water body is anticipated to be similar to the existing pit lake and is not expected to be conducive to providing aquatic habitat or supporting fish life.

This geochemical modeling report demonstrates that the mine pit reclamation proposed for the Copper Flat mine that is outlined in Section 3.1.8 of this report meets the water quality similarity requirements of 19.10.6.603 NMAC.



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Appendix H: PHREEQC Input Files (electronic)

Appendix I: PHREEQC Output File (electronic)

Appendix J: Aquatic Consultants Inc. Biological Assessment of the Existing Copper Flat Pit Lake

# 1 Introduction

## 1.1 Purpose and Scope

SRK Consulting, Inc. (SRK) has undertaken a geochemical modeling assessment on behalf of New Mexico Copper Corporation (NMCC – a subsidiary of THEMAC Resources Group Ltd. [THEMAC]) to predict future pit lake chemistry associated with the Copper Flat Project (the Project), New Mexico. The purpose of the assessment is to evaluate the future environmental impacts of the Project as required by the New Mexico Mining Act and State environmental regulations. The work forms part of the geochemical characterization study to assess the Acid Rock Drainage and Metal Leaching (ARDML) potential of the Project.

Preliminary pit lake model results were presented in the December 18, 2014 report entitled *'Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico'* (SRK, 2014a). The purpose of this preliminary report was to outline the methodology for the pit lake modeling in order to seek feedback from the agencies, and to present the initial results of the pit lake modeling. Since this preliminary report was submitted, a number of modifications and refinements have been made to the pit lake models, including:

- Incorporation of the Feasibility Study geologic block model;
- Incorporation of the current open pit design, which is detailed in the 2017 Mine Operation and Reclamation Plan (2017 MORP pit);
- Refinement of the pit wall composition to include delineation of material types by primary lithology, oxidation and mineralized versus non-mineralized material;
- Refinement of humidity cell test (HCT) inputs to include separate source terms for major and trace elements, reflecting the different processes that control their release;
- Refinement of mineral equilibrium phases based on predicted chemistry;
- Refinement of the water balance to use a reduced annual evaporation rate of 50 inches and to include a separate runoff term for reclaimed areas in the pit and the open pit watershed;
- Revisions to the groundwater chemistry inputs; and
- Incorporation of pit reclamation measures, including rapid fill of the pit and reclamation of the pit haul road and other areas within the pit and the pit watershed.

This final report describes the approach taken for the revised pit lake predictive modeling effort, details the assumptions made, and presents the final results of the revised pit lake geochemical predictions.

Applicable standards to the post-mining Copper Flat pit lake are contained in the New Mexico Mining and Minerals Division (MMD) regulations administered under the Mining Act. Specifically, the performance and reclamation standards require that reclamation must result in a hydrologic balance similar to pre-mining conditions. With respect to water quality in the pit lake, post mining water quality must be similar to baseline pre-mining water quality in the pit lake. The model results presented herein have been compared to pre-mining baseline water quality of the existing pit lake.

## 1.2 Background

The Copper Flat Project is a porphyry copper/molybdenum deposit located in the Hillsboro Mining District in South Central New Mexico, in Sierra County located approximately 150 miles south of Albuquerque, New Mexico and approximately 20 miles southwest of Truth or Consequences, New Mexico (straight-line distances). Access from Truth or Consequences is by 24 miles of paved highway and 3 miles of all-weather gravel road. The Copper Flat Project location is shown in Figure 1-1.

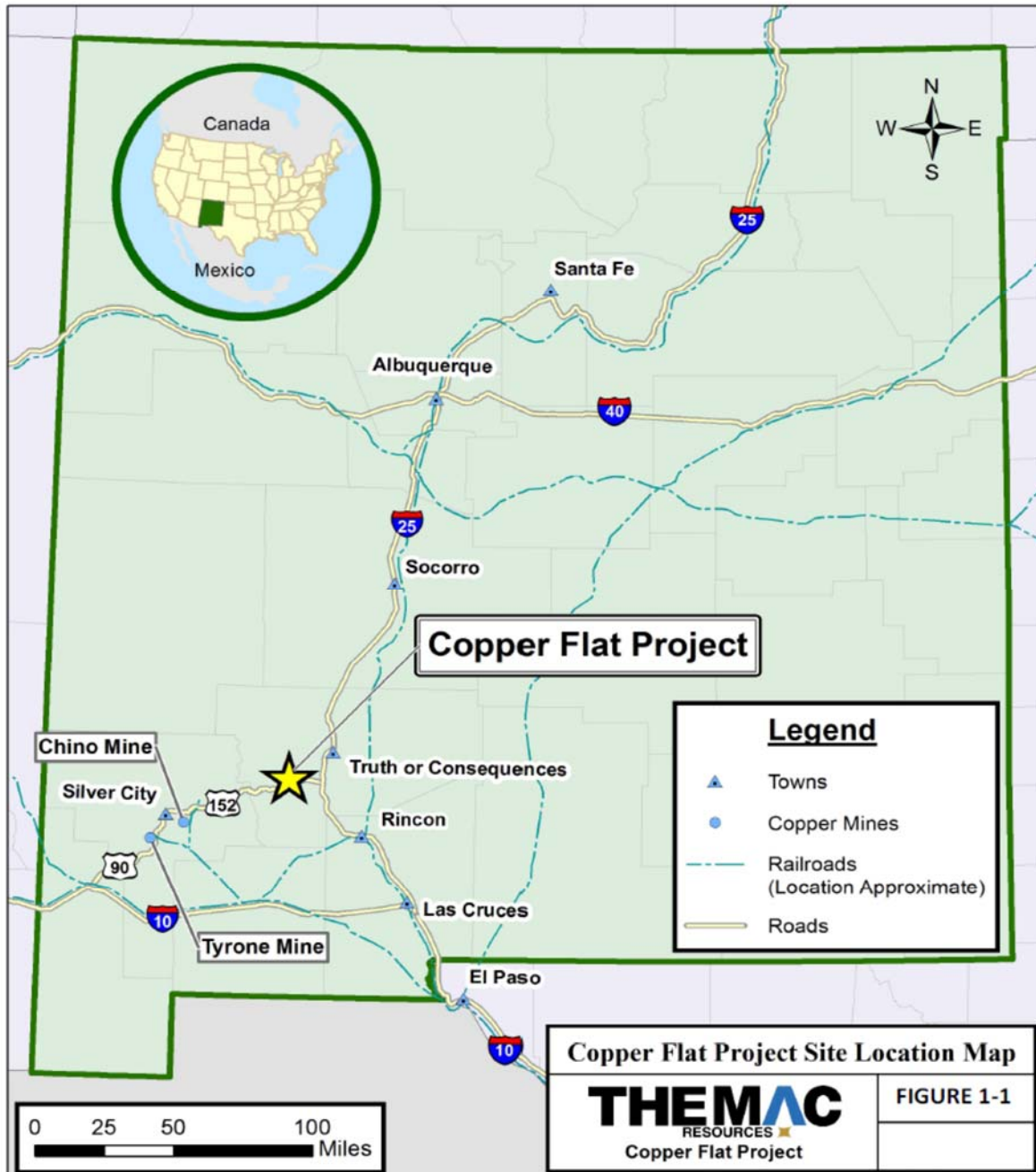


Figure 1-1: Project Location

### 1.2.1 Climate

The regional climate is high desert, and is generally hot with a July average of 76°F (record maximum 107°F), and January average of 39°F (record minimum 1°F). The area is generally dry with about 13 inches of average annual precipitation, which occurs mostly as rainfall during July to September.

Winters are cold and dry. Snowfall is possible from October through April, but more typically occurring between December and February. The average annual total is 8 inches of snowfall. Prevailing wind direction is predominantly from the west, and secondarily from the north, and averages 10 to 15 miles per hour. Wind speeds in excess of 50 mph may occur as major storms pass through the area.

### 1.2.2 Prior Mining Operations

Mining activities in the Hillsboro Mining District began in the late-1800s. Gold was mined from shafts and adits at Copper Flat and from placer workings developed along drainages to the east and southwest of Black and Animas Peaks. Gold mining was further developed during the early 1900s and continued until World War II. Today, small scale placer mining continues. Copper exploration began in the 1950s and continued to the early 1980s, when Quintana Minerals Corporation defined 60 Mt of reserves sufficient to operate for a 11-year mine life at an extraction rate of 15,000 tons of ore per day (tpd). Operations included the development of the open pit, waste rock stockpiles, TSF and other mine disturbances observed today, but mining stopped after three months due to low metal prices. Mine buildings and equipment were dismantled in 1985; however structural foundations, power lines, water wells, and in-ground infrastructure were left in-place for a future restart. During the 1990s, plans to reopen the mine were considered. Existing surface disturbances and facilities in the Project area include the following:

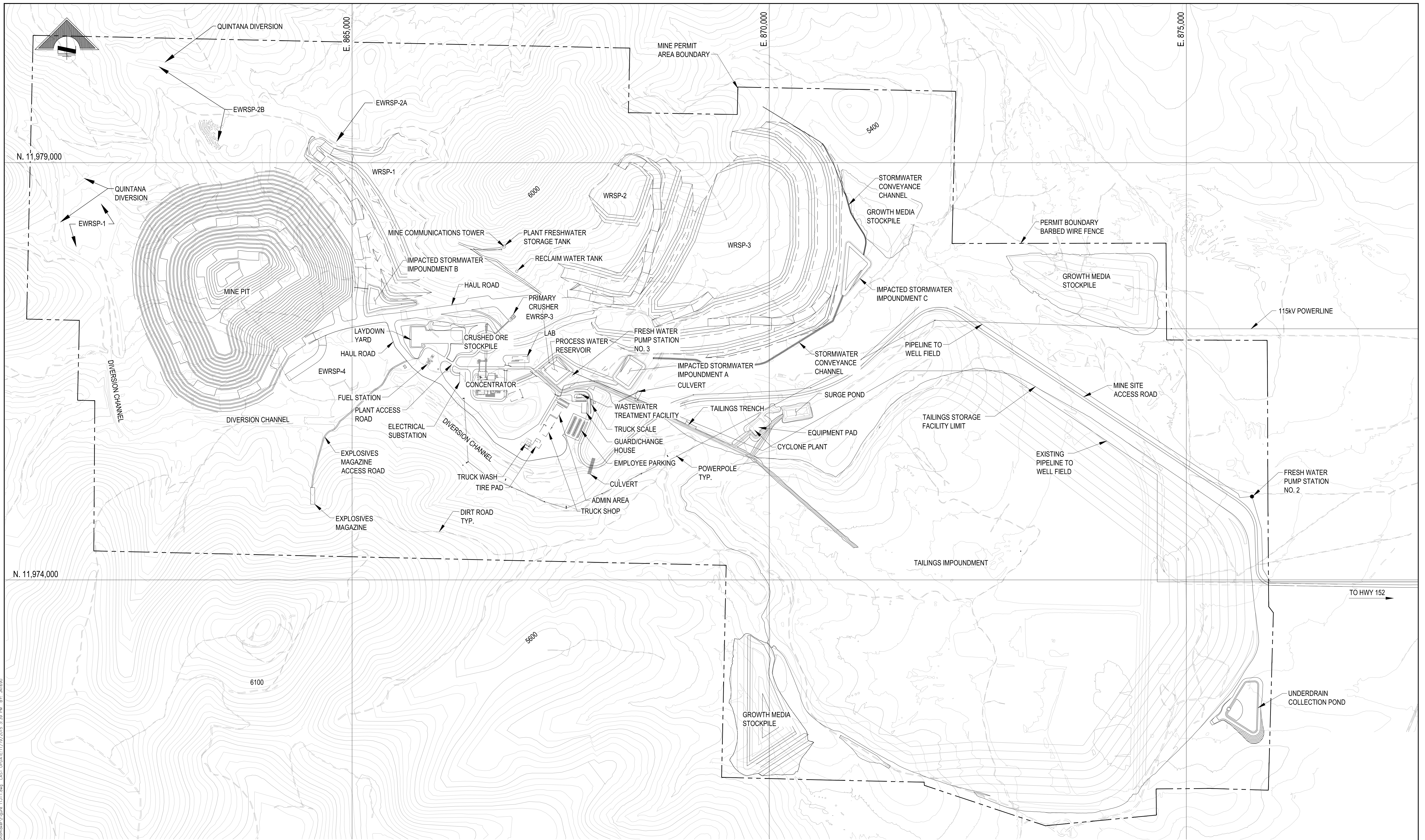
- A pit with a small pit lake;
- Waste rock stockpiles (WRSP);
- A 115-kilovolt power line from the Caballo Substation to the mine site;
- Production wellfield and 20-inch pipeline from the wellfield to the mine site;
- A diversion channel collecting stormwater from west and south of the pit and diverting unimpacted flows down Grayback wash;
- A diversion channel collecting stormwater from north of the pit and diverting unimpacted flows to the east;
- Existing concrete foundations and structures including:
  - Primary crusher structure and stacking conveyor tunnel
  - Coarse ore reclaim tunnel
  - Concentrator building foundation
  - Truck shop foundation
  - Administration building foundation
  - Concentrate storage foundation
  - Mine office and change house foundation.
- Site grading and roads; and
- A tailings storage facility (TSF) containing approximately 1.4 Mt of tailings from the Quintana mining operation.



### 1.2.3 Mine Plan

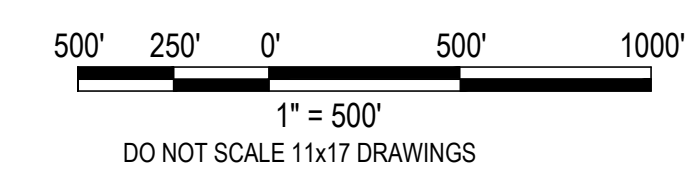
The proposed Project consists of an open pit mine, flotation mill, tailings storage facility, waste rock stockpiles and ancillary facilities. During the mine life, the proposed Project is expected to produce approximately 113 million tons of copper ore and 45 million tons of waste rock. Ore extraction will take place by conventional truck and loader methods using 25-foot high benches. Backfilling of the pit will not take place during or after mining.

Beneficiation will be achieved through the use of a conventional concentrator using standard crushing, grinding and flotation technologies. The operation is designed to recover copper, molybdenum, gold, and silver into separate copper and molybdenum concentrates. The nominal ore throughput rate is 30,000 tpd and an operational life of 11 to 12 years is currently projected. The proposed layout of the mine facilities is shown in Figure 1-2. The current pit configuration is modified from the pit design developed for the Copper Flat Feasibility Study (FS) published in November 2013 (M3, 2013) and matches the pit design presented in the 2017 MORP (THEMAC, 2017a).



EWRSP = EXISTING WASTE ROCK STOCKPILE  
 WRSP = WASTE ROCK STOCKPILE

**SITE PLAN**  
 SCALE: 1:500



**PRELIMINARY**  
 FOR AGENCY REVIEW



REFERENCES				REFERENCES				REVISIONS				REVISIONS			
DWG. NO.	TITLE	DWG. NO.	TITLE	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT	NO.	DESCRIPTION	BY	APP'D	DATE	CLIENT

SCALE:	1" = 500'	DATE:	
DESIGNED BY:	SAM	DATE:	DEC12
DRAWN BY:	SAM	DATE:	DEC12
CHECKED BY:	TDL	DATE:	JAN13
PROJECT MGR:	RKZ		
CLIENT APPR:			

ARCHITECTURE  
 ENGINEERING  
 CONSTRUCTION MANAGEMENT  
 www.m3eng.com

**COPPER FLAT PROJECT**

**SITE GENERAL CIVIL PROJECT AREA PROPOSED SITE PLAN**

JOB NO. M3 PN-120085  
 DWG. NO. **FIGURE 2-2**  
 REV. NO. P18 DATE 16 NOV 15

File: P:\2012\200805\Civil\0441544-2\_Dwg\Site\_Plan\0441544-2.dwg, LAST\_UPDATE: 11/19/2015, 3:39 PM, BY: 401690

## 1.2.4 Geology and Mineralization

The following description of geology and mineralization is from the Copper Flat Feasibility Study (FS) published in November 2013 (M3, 2013). The Copper Flat Project is a porphyry copper-molybdenum deposit located on the western margin of the Rio Grande Rift. The deposit also contains recoverable, gold and silver. The deposit is hosted by a small quartz monzonite stock having a porphyritic texture that intrudes a sequence of andesitic volcanic rocks of similar age covering an area approximately 4 miles in diameter.

### Regional Geology

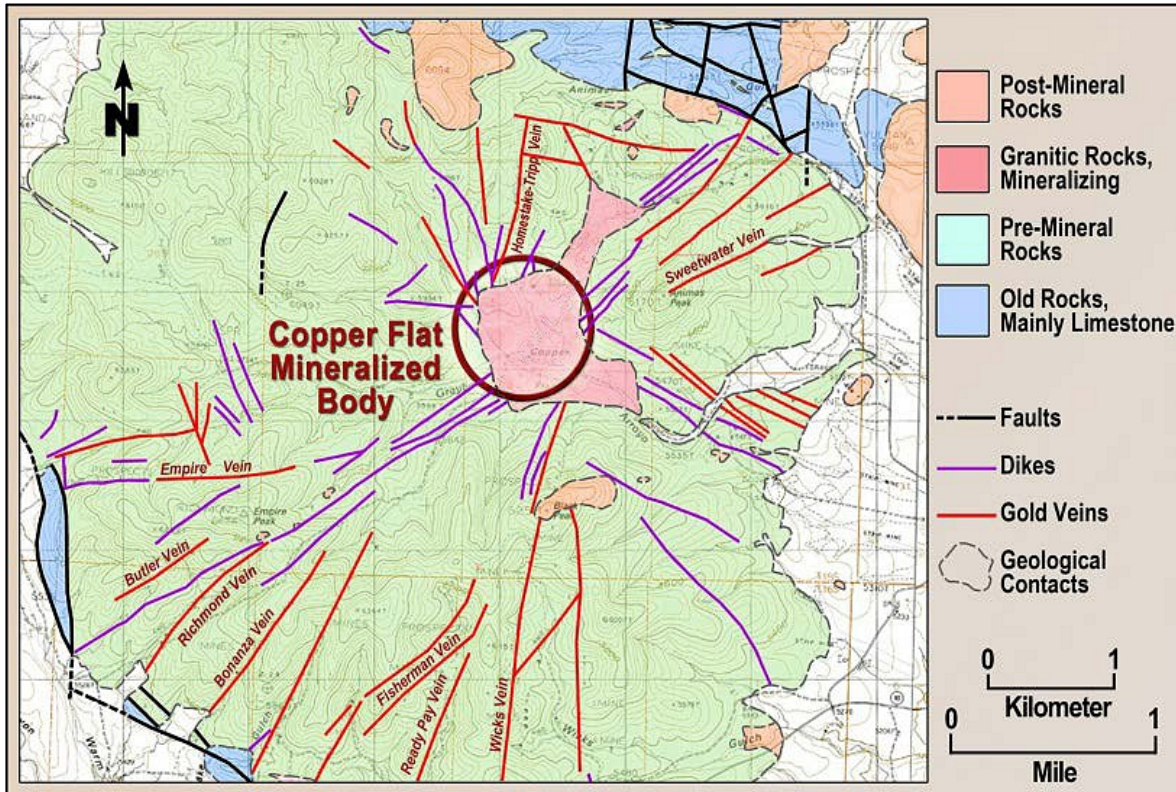
The Copper Flat Project lies within the Mexican Highlands portion of the Basin and Range Physiographic Province. The Project is located in the Hillsboro Mining District in the Las Animas Hills, which are part of the Animas Uplift, a horst on the western edge of the Rio Grande valley. The Animas Uplift is separated from the Rio Grande by nearly 20 miles of Santa Fe Group alluvial sediments, referred to as the Palomas Basin of the Rio Grande valley. To the west of the Animas Uplift is the Warm Springs valley, a graben that parallels the Rio Grande valley. Further west, the Black Mountains form the backbone of the Continental Divide, rising to about 9,000 feet above sea level. The regional geology is discussed in more detail in the *Baseline Data Report for the Copper Flat Mine* (BDR) (INTERA, 2012). The focus of this report is on the local and Copper Flat ore body geology.

Basement rocks in the area consist of Precambrian granite and Paleozoic and Mesozoic sandstones, shales, limestones, and evaporites. Sedimentary units that crop out within the Animas Uplift include the Ordovician Montoya Limestone, the Silurian Fusselman Dolomite, and the Devonian Percha Shale. The Cretaceous-age Laramide orogeny, which was characterized by the intrusion of magma associated with the subduction of the Farallon plate beneath the North American plate, affected this region between 75 and 50 million years ago (Ma). Volcanic activity during the late Cretaceous and Tertiary periods resulted in localized flows, dikes, and intrusive bodies, some of which were associated with the development of the nearby Tertiary Emory and Good Sight-Cedar Hills calderas. Later basaltic flows resulted from the tectonic activity associated with the formation of the Rio Grande rift. Tertiary and Quaternary alluvial sediments of the Santa Fe Group and more recent valley fill overlie the older Paleozoic and Mesozoic units in the area.

### Local Geology

The district geology described below is modified from McLemore et al. (2000) and Raugust (2003). The predominant geologic feature of the Hillsboro Mining District is the Cretaceous Copper Flat stratovolcano, a circular body of Cretaceous andesite that is 4 miles in diameter (Figure 1-3). The Hillsboro Mining District comprises the Las Animas Hills, a low range formed by the Animas Hills horst at the western edge of the Rio Grande Rift. Faults that bound the Animas Hills horst are related to the tectonic activity of the Miocene-age Rio Grande Rift (Dunn, 1982). Due to the difference in ages and in spite of its close proximity, there is no known connection between the Rio Grande rift and the Copper Flat volcanic/intrusive complex. The Copper Flat volcanic/intrusive complex has been interpreted as an eroded stratovolcano based on the presence of agglomerate and flow band textures in some of the andesite (Richards, 2003).

The Copper Flat Quartz Monzonite (CFQM) intrudes the core of the volcanic complex. The CFQM stock has a surface expression of approximately 0.4 mi<sup>2</sup> and has been dated by the argon-argon (<sup>40</sup>Ar/<sup>39</sup>Ar) techniques to be 74.93 ±0.66 million years old (McLemore et al., 2000). The surrounding andesite has also been dated using argon-argon techniques to be 75.4 ±3.5 million years old (McLemore et al., 2000).



**Figure 1-3: Geology of the Copper Flat Mine (Dunn, 1982)**

**Geology of the Copper Flat Orebody**

The Copper Flat andesite is generally fine-grained with phenocrysts of plagioclase (andesine) and amphibole in a groundmass of plagioclase and potassium feldspar and rare quartz. Some agglomerates or flow breccias are locally present, but the andesite is generally massive. Magnetite is commonly associated with the mafic phenocrysts, and accessory apatite is commonly found.

Although the depth of erosion is uncertain, the center of the stratovolcano was eroded to form a topographic low. To the east of the site, this andesite body is in fault contact with Santa Fe Group sediments, which are at least 2,000 feet thick in the immediate Copper Flat area and thickening to the east. Near-vertical faults characterize the contacts on the remaining perimeter of the andesite body; these faults juxtapose the andesite with Paleozoic sedimentary rocks. Historical drill holes indicate the andesite is locally more than 3,000 feet thick. This feature, combined with the concentric fault pattern, indicate that the local geology represents a deeply eroded Cretaceous-age volcanic complex. A detailed geologic map of the Copper Flat orebody is provided in Figure 1-4 and a south-north geologic cross section through the Copper Flat orebody is provided in Figure 1-5.

Copper Flat Quartz Monzonite (CFQM) intrudes the core of the volcanic complex. Sulfide mineralization is present as veinlets and disseminations in the CFQM, but is most strongly developed in and adjacent to the west end of a steeply dipping breccia pipe that is centrally located within the CFQM stock and elongated in the northwest-southeast direction (Figure 1-5).

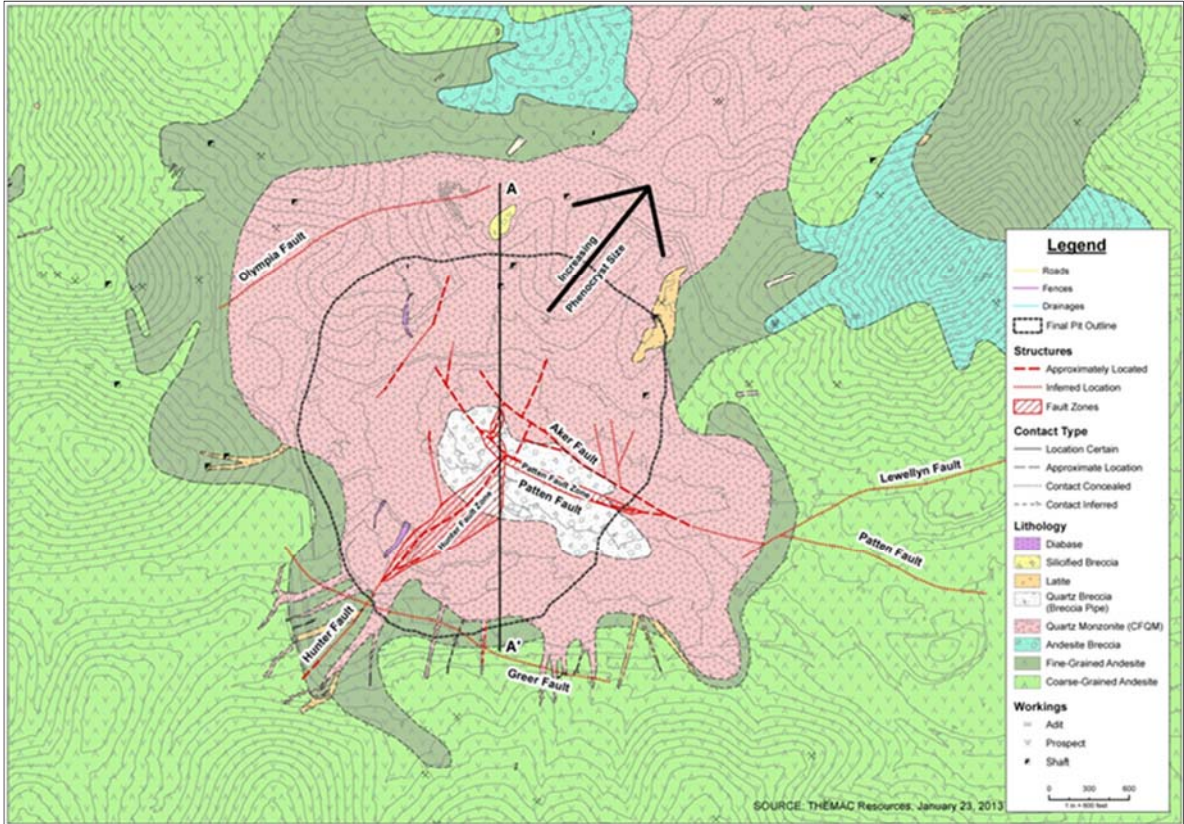


Figure 1-4: Detailed Geologic Map of the Copper Flat Orebody (M3, 2013)

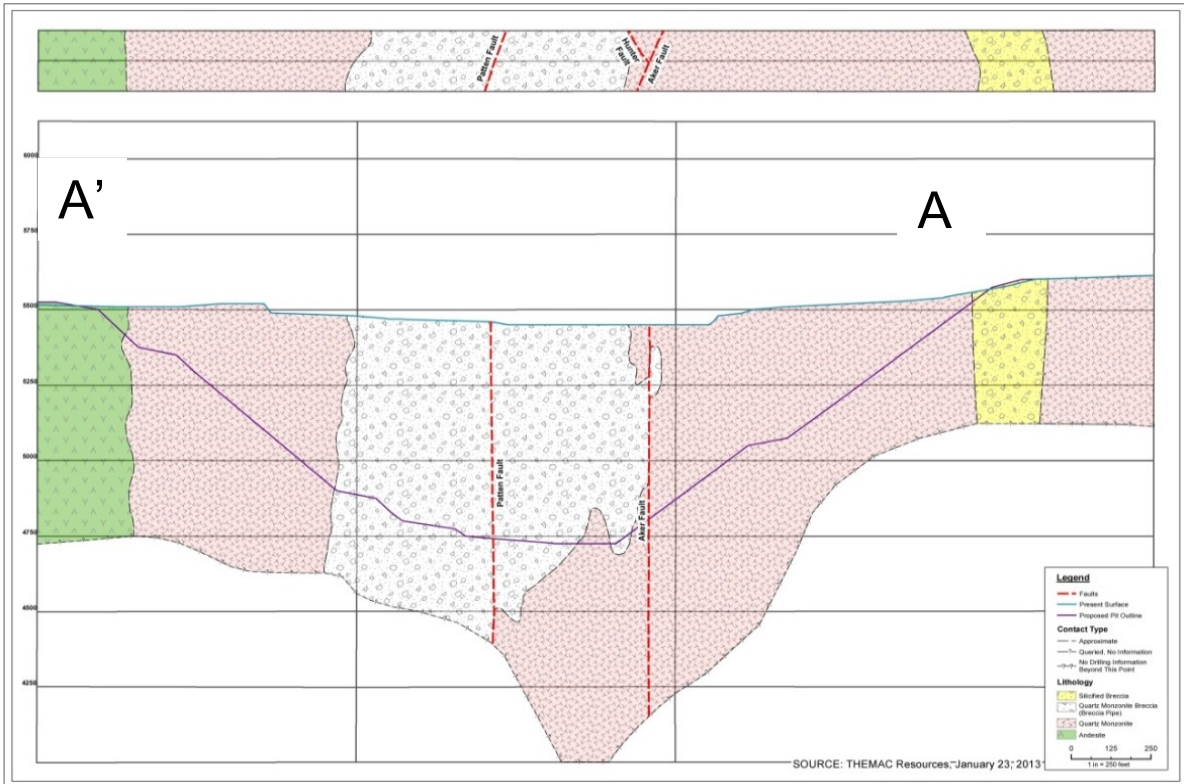


Figure 1-5: Geologic Cross Section through the Copper Flat Orebody (M3, 2013)

## Lithology

The CFQM intruded into the center of the andesite sequence at the intersection of two principal structures that trend respectively N50°W and N20°E. The CFQM is an irregular-shaped stock underlying a surface area of approximately 0.40 square miles and has been dated to approximately 75 Ma. In the few exposures in which the CFQM is in contact with the andesite, the andesite shows no obvious signs of contact metamorphism. The CFQM is a medium- to coarse-grained, holocrystalline porphyry composed primarily of potassium feldspar, plagioclase, hornblende, and biotite; trace amounts of magnetite, apatite, zircon, and rutile are also present, along with localized mineralized zones containing pyrite, chalcopyrite, and molybdenite. About 15 percent of the monzonite is quartz, which occurs both as small phenocrysts and as part of the groundmass; however, quartz is absent in some parts of the stock.

Numerous dikes, some of which are more than a mile in length and mostly of latite composition, radiate from and cut the CFQM stock. Most of the dikes trend to the northeast or northwest and represent late stage differentiation of the CFQM stock. Diabase has been mapped in contact with the CFQM at Copper Flat. Immediately south of the quartz monzonite, the andesite is coarse-grained, perhaps indicating a shallow intrusive phase. An irregular mass of andesite breccia along the northwestern contact of the quartz monzonite contains potassium feldspar phenocrysts and andesitic rock fragments in a matrix of sericite with minor quartz. This may represent a pyroclastic unit. Magnetite, chlorite, epidote, and accessory apatite are also present in the andesite breccia.

## Structure

Three principal structural zones are present at Copper Flat, the most prominent of which is a northeast-striking fault that trends N 20°-40°E that includes the Hunter and parallel faults or the Hunter fault zone. In addition, west-northwest striking zones of structural weakness (N50°-70°W) are marked by the Patten and Greer faults, and east-northeast striking zones are marked by the Olympia and Lewellyn faults. All faults have a near-vertical dip; the Hunter fault system dips 80°W, the Patten dips approximately 70°S-80°S, and both the Olympia and Lewellyn fault systems dip between 80°S and 90°S. These three major fault zones appear to have been established prior to the emplacement of the CFQM and controlled subsequent igneous events and in the case of the Patten and Hunter controlled mineralization.

As previously stated, the CFQM emplacement is largely controlled by the three structural zones. The southern contact parallels and is cut by the Greer fault, although the contact is cut by the fault, and the southeastern and northwestern contacts are roughly parallel to the Olympia and Lewellyn faults, respectively. The CFQM stock is principally elongated along the Patten fault, as well as along the Hunter fault zone.

Although latite dikes strike in all the three principal fracture directions, most of the dikes strike northeast. The northeast trending fault zones contain a high proportion of wet gouge, often with no recognizable rock fragments. Reportedly in underground exposures the material comprising the Hunter fault zone has the same consistency as wet concrete and has been observed to flow in underground headings. Based on recent drilling the Patten fault consists of a mixture of breccia and gouge. However, the material in the east-northeast fault zones contains only highly broken rock and minor gouge. The width of individual structures in all three systems varies along strike from less than a foot to nearly 25 feet in the Patten fault east of the Project. Despite intense brecciation, the total displacement along the faults does not appear to exceed a few tens of feet. At the western edge of the CFQM intrusion, a younger porphyritic dike was emplaced in a fault that offsets an early latite dike, indicating that fault movement occurred during the time that dikes were being emplaced.

Post-dike movement is evident in all the three principal fault zones, and both the Hunter and Patten fault systems show signs of definite post-mineral movement. Fault movement has smeared sulfide deposits and offset the breccia pipe as well as the zones within the breccia pipe. Post-mineral movement along faults has resulted in wide, strongly brecciated fault zones. Some of the post-mineral dikes have been emplaced within these fault zones.

NMCC has mapped the pit area and diversion cuts in detail at 1 inch equals 40 feet (1:480) and has examined the pre- and post-mineral stress orientations in the andesite and CFQM. Findings indicate no significant difference in the stress fields before and after mineralization. During NMCC's mapping efforts, the Greer and Olympia previously mapped fault locations could not be verified; therefore, these faults were labeled as inferred.

## **Mineralization**

The CFQM hosts mineralization dominated by pyrite and chalcopyrite with subsidiary molybdenite, minor bornite and recoverable amounts of gold and silver. The mineralization is focused along intersecting northeast- and northwest-trending faults, and these intersections may have originally controlled emplacement of the CFQM.

Although copper occurs almost exclusively as chalcopyrite locally accompanied by trace amounts of bornite, minor amounts of chalcocite and copper oxide minerals are locally present near the surface and along fractures. The supergene enrichment typical of many porphyry copper deposits in the Southwest is virtually non-existent at Copper Flat. During the early mining days, a 20 to 50-foot leached oxide zone existed over the ore body, but this material was stripped during the mining activities that occurred in the early 1980s. Most of the remaining ore is unoxidized and consists primarily of chalcopyrite and pyrite with some molybdenite and locally traces of bornite, galena and sphalerite. Recently completed mineralogical studies indicate that fine grained disseminated chalcopyrite is often inter grown with pyrite and occurs interstitial to silicate minerals. Deposition of chalcopyrite and molybdenite (76.2 Ma) occurred within the same mineralizing event as the pyrite.

Sulfide mineralization is present as veinlets and disseminations in the CFQM, but is most strongly developed in and adjacent to the west end of a steeply dipping breccia pipe, that is centrally located within the CFQM stock and elongated in the northwest-southeast direction roughly along, but south of the Patten fault. The sulfide mineralization first formed in narrow veinlets and as disseminations in the quartz monzonite with weakly developed sericitic alteration. This stage of mineralization was followed by the formation of the breccia pipe with the introduction of coarse "clotty" pyrite and chalcopyrite along with veinlet controlled molybdenite and milky quartz, and the development of strong potassic alteration.

The breccia pipe, which can best be described as a crackle breccia, consists largely of subangular fragments of mineralized CFQM, with locally abundant mineralized latite where dikes exposed in the CFQM projected into the brecciated zone that range in size from an inch to several inches in diameter. Andesite occurs only as mixed fragments partially in contact with intrusive CFQM and appears to represent the brecciation of relatively unaltered andesite xenoliths in the CFQM. The matrix contains varying proportions of quartz, biotite (phlogopite), potassium feldspar, pyrite, and chalcopyrite, with magnetite, molybdenite, fluorite, anhydrite, and calcite locally common. Apatite is a common accessory mineral. Breccia fragments are rimmed with either biotite or potassium feldspar, and the quartz and sulfide minerals have generally formed in the center of the matrix.

Two types of breccia within the quartz monzonite breccia pipe have been identified as distinguishable units based on the dominant mineral filling the matrix between clasts. Recent drilling has shown that the two breccia types, biotite breccia and feldspar breccia, grade into one another as well as with the CFQM. Interestingly, from a recovery perspective, metallurgical testing has shown that the mineralization behaves virtually the same irrespective of the lithology.

The total sulfide content ranges from 1 percent (by volume) in the eastern part of the breccia pipe and the surrounding CFQM to 5 percent in the CFQM to the south, north, and west. Sulfide content is highly variable within the breccia, with portions in the western part of the breccia containing as much as 20 percent sulfide minerals. The strongest copper mineralization is concentrated in the western half of the breccia pipe and in the adjoining stockwork veined CFQM in the vicinity of the intersection of the Patten fault and the Hunter fault zone. Sulfide mineralization is concentrated in the CFQM and breccia pipe, and drops significantly at the andesite contact. Minor pyrite mineralization extends into the andesite along the pre-mineral dikes and in quartz-pyrite-bearing structures, some of which were historically prospected for gold.

Molybdenite occurs in some steeply dipping quartz veins or as thin coatings on fractures. Minor sphalerite and galena are present in both carbonate and quartz veinlets in the CFQM stock. Preliminary 2011 evaluations of the mineralization at Copper Flat indicate that copper mineralization concentrates and trends along the N50°W structural influences, whereas the molybdenum, gold and silver appear to favor a N10°-20°E trend.

### 1.2.5 Hydrology

Hydrological information pertaining to the Copper Flat Project has been summarized from the Baseline Data Report (INTERA, 2012) and is provided herein to provide a context for the pit lake modeling. The mine permit area is located in the Lower Rio Grande watershed, which includes approximately 5,000 square miles in Catron, Socorro, Sierra, and Doña Ana Counties and is dominated by the Rio Grande and its tributaries as well as the two large reservoirs of Elephant Butte and Caballo. Numerous tributaries drain into the Rio Grande from the west, but none contribute perennial flow to the Rio Grande. The mine permit area is drained by ephemeral streams (arroyos) within the Greenhorn Arroyo Drainage Basin. The Greenhorn Arroyo Drainage Basin is composed of Greenhorn Arroyo, Grayback Arroyo, and Hunkidori Gulch. The Grayback Arroyo passes through the permitted mine area and is diverted around the existing mine pit. Drainages within this watershed are ephemeral, flowing in response to heavy or sustained precipitation events. Water quality data for the Grayback Arroyo are summarized in Table 1-1.

**Table 1-1: Summary of Hydrochemical Information in the Grayback Arroyo (INTERA, 2012)**

Details	pH (s.u.)	Chloride (mg/L)	Sulfate (mg/L)	TDS (mg/L)
Min	7.42	0.71	11	78
Max	7.92	130	2,900	4,500

Surface waters in the Grayback Arroyo are typically characterized by higher major ion and trace element concentrations, with sulfate concentrations up to 2,900 mg/L and TDS up to 4,500 mg/L.

### 1.2.6 Hydrogeology

Hydrogeological information pertaining to the Copper Flat Project has been summarized from the Baseline Data Report (INTERA, 2012) and is provided herein. This report identifies three aquifers within the Copper Flat Project area (Figure 1-6) including:

1. Crystalline bedrock aquifer;
2. Santa Fe Group aquifer; and
3. Quaternary alluvial aquifer.

Details of these aquifers are provided below.



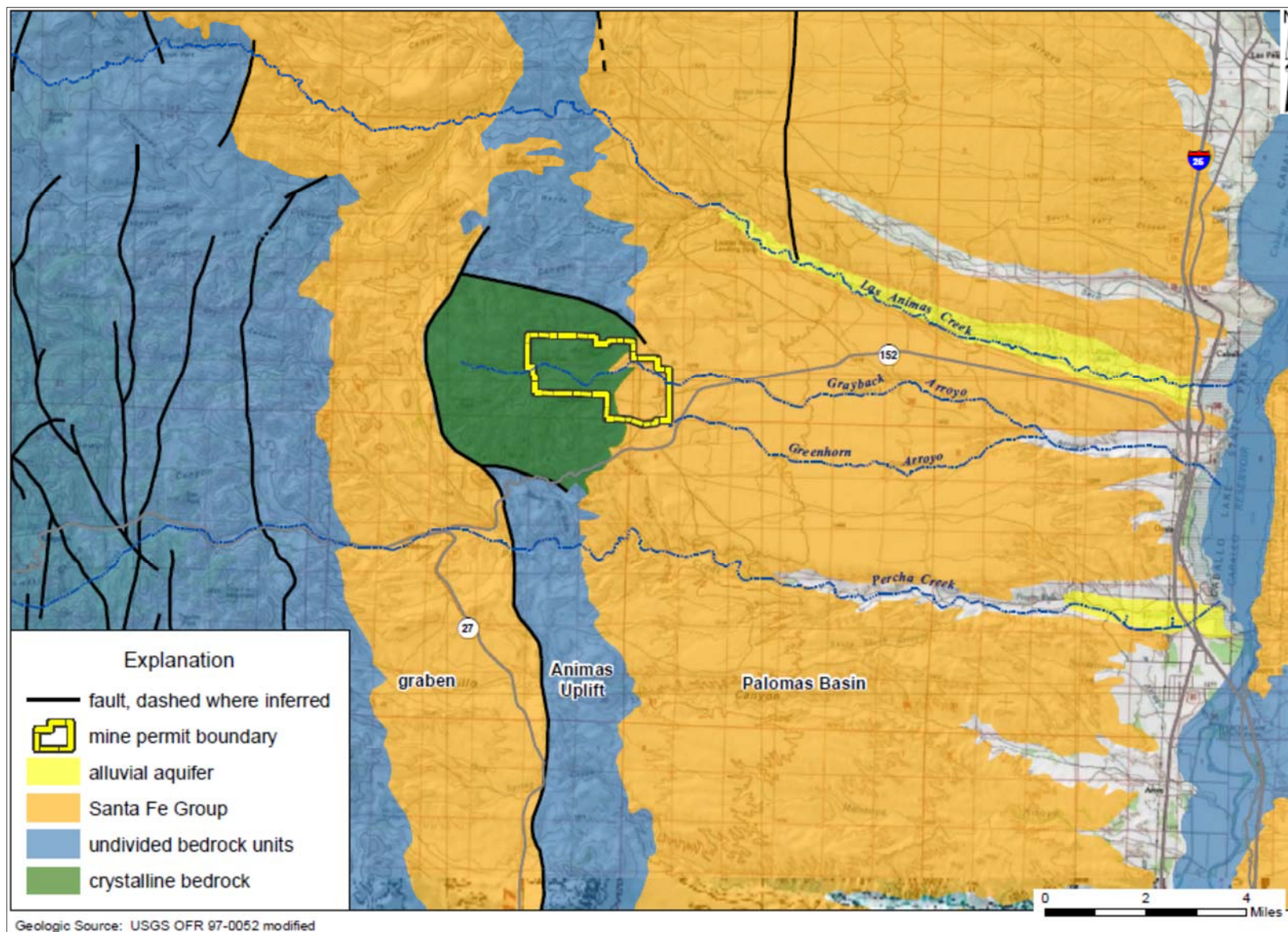
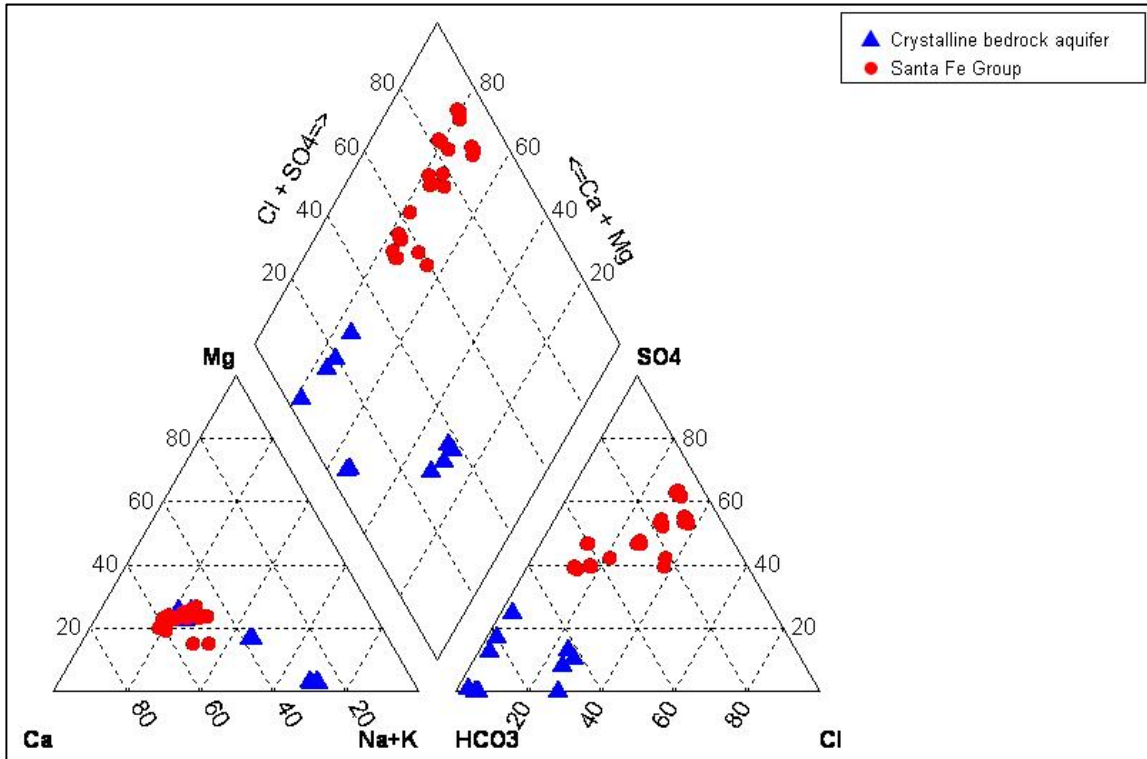


Figure 1-6: Map Showing Location of Crystalline Bedrock, Santa Fe Group Sediments and Alluvial Aquifer Zones (INTERA, 2012)

- 1. Crystalline Bedrock Aquifer:** Groundwater is present within the crystalline volcanic rocks (quartz monzonite and andesite) that constitute much of the western portion of the mine permit area. Though the rocks themselves have practically no inter-granular permeability, faulting and jointing of the monzonite have created locally permeable zones through which water can move. Groundwater flow is generally from west to east, with the exception of the area surrounding the pit lake, which behaves as an evaporative sink. The permeability of the andesite is extremely low (<0.003 feet/day), whereas the permeability of the monzonite rocks averages 0.1 feet/day due to localized secondary porosity from fracturing. Groundwater in the Crystalline Bedrock Aquifer is characterized by moderately alkaline pH (~8 s.u.) and can generally be classed as sodium / calcium plus bicarbonate (Na / Ca + HCO<sub>3</sub>) type waters based on their major ion signature (Figure 1-7).
- 2. Santa Fe Group Aquifer:** Overlying and adjacent to the crystalline bedrock aquifer is the Santa Fe Group Aquifer system, which receives recharge from precipitation. The aquifer is located approximately 1 mile downgradient of the existing pit lake, and the low hydraulic conductivity of the andesite limits cross formational flow. The sediments of the Santa Fe Group are stratified, contain a wide variety of grain sizes, and, in general, dip to the east. The direction of groundwater flow is from west to east and the groundwater elevation contours indicate groundwater flows from the andesite to the alluvium and Santa Fe Group sediments. Groundwater in the Santa Fe Group Aquifer is characterized by circum-neutral to moderately alkaline pH (7 – 8 s.u.) and can generally be grouped into the calcium plus bicarbonate (Ca + HCO<sub>3</sub>) or calcium plus sulfate (Ca + SO<sub>4</sub>) hydrochemical facies based on major ion chemistry (Figure 1-7). The sulfate signature of some of the groundwater samples is associated with wells within the Santa Fe Group Aquifer near the existing TSF, which are known to be influenced by a sulfate plume from the historic tailings.
- 3. Quaternary Alluvial Aquifer:** This aquifer is comprised of channel and floodplain gravels, sands and silts and represents the uppermost aquifer in the vicinity of the Copper Flat Project. The alluvial aquifer is typically recharged by infiltration of rainfall.



**Figure 1-7: Piper Plot of Major Ion Chemistry of Groundwater in the Mine Permit Area (analyses from 2010 and 2011 only)**

### 1.2.7 Existing Pit Lake

Beginning in the late 1980s, a pit lake formed in the existing pit. This lake represents an artificial water body that has formed in a man-made void. The surface area of the pit lake was approximately 13.8 acres at its maximum extent, but the lake has subsequently reduced in size as a result of evaporation and limited precipitation (i.e., drought conditions). A recent evaluation by John Shomaker and Associates (JSAI, who have been assisting THEMAC with site management of water resources) indicates that the pit lake currently covers an area of approximately 5.2 acres and contains approximately 70 acre-feet of water (NMCC estimate, 2015). Bathymetric measurements carried out as part of the INTERA (2012) baseline data collection program indicate that the depth of the existing pit lake varies between 10 and 35 feet. Water levels are typically highest in the winter month of January and lowest in the summer month of July. The analytical results do not indicate the presence of a chemocline or any chemical stratification in the lake. However, the temperature profiles for the winter and summer sampling showed a greater than 1°C per meter change, indicating the presence of a seasonal thermocline. The pit currently represents a hydraulic sink, with evaporation from the lake surface exceeding groundwater inflow, precipitation and surface runoff (M3, 2012).

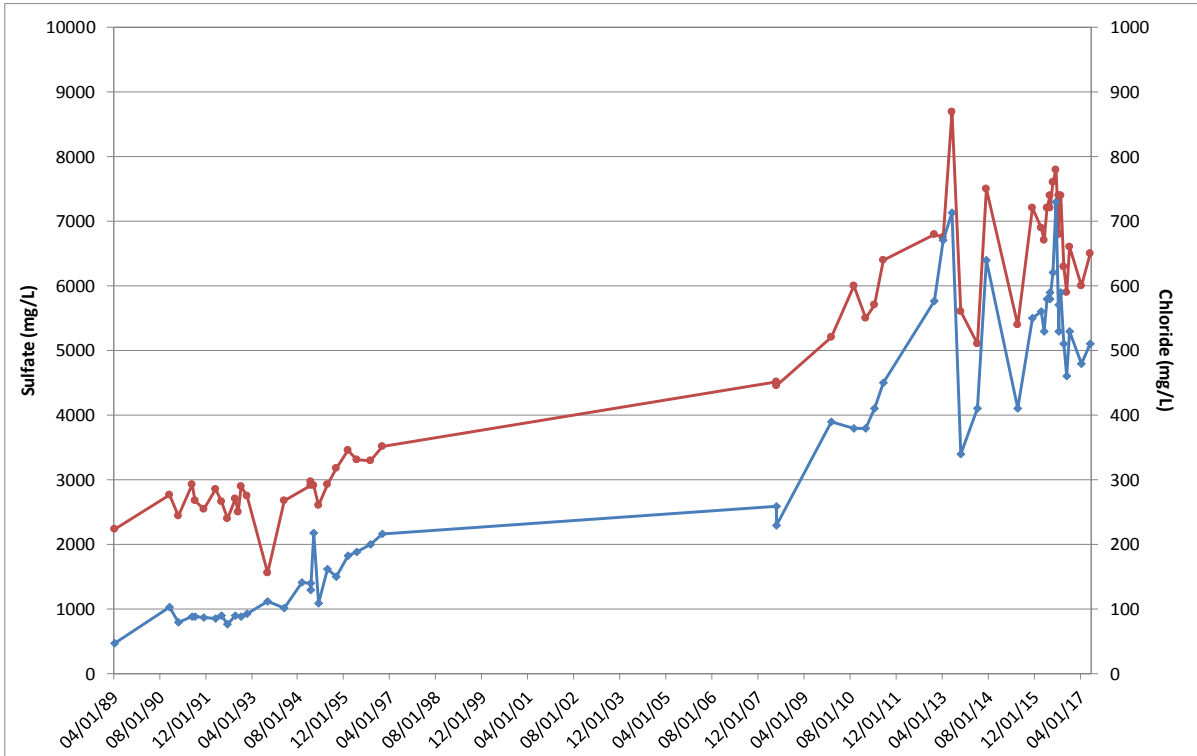
Monitoring of the existing pit lake water quality has taken place periodically between 1989 and present, with a total of 57 samples being collected for analysis. Monitoring took place on at least an annual basis between 1989 and 1997, with 26 samples collected during this period. The monitoring program was then re-established in 2010 as part of the INTERA (2012) baseline data collection program, which included collection of samples from the deepest part of the pit lake in September 2010, January 2011, April 2011 and July 2011. JSAI collected four quarters of additional data in 2013 as part of the Stage 1 abatement investigation (JSAI, 2014a). Monitoring of pit lake water quality is ongoing, with NMCC collecting three samples in 2014, two samples in 2015, 13 samples in 2016 and two samples to date in 2017.

The results of the existing pit lake monitoring are summarized in Table 1-2 and time-series plots of key parameters are provided in Appendix A. This demonstrates that the pH of the pit lake waters has been variable over the period of record, ranging from a minimum of pH 3.6 to a maximum of pH 8.3. In general, the pit lake waters are circum-neutral (average of pH 6.5); any periodic decreases in pH (for example between March and October 1992, June 2008 and June 2015 [Figure 1-10]) are associated with periodic Acid Wall Seep (AWS) events. Concentrations of sulfate, chloride, TDS, manganese, magnesium, cobalt, fluoride, sodium and potassium have increased between 1989 and 2017 (Appendix A). In particular, evapoconcentration effects have increased the concentrations of sulfate and chloride (Figure 1-8), resulting in supersaturation of pit lake waters and subsequent precipitation of salts (primarily gypsum) around the rim of the existing pit lake. These precipitated solids form a thick crust on the pit walls (Figure 1-11).

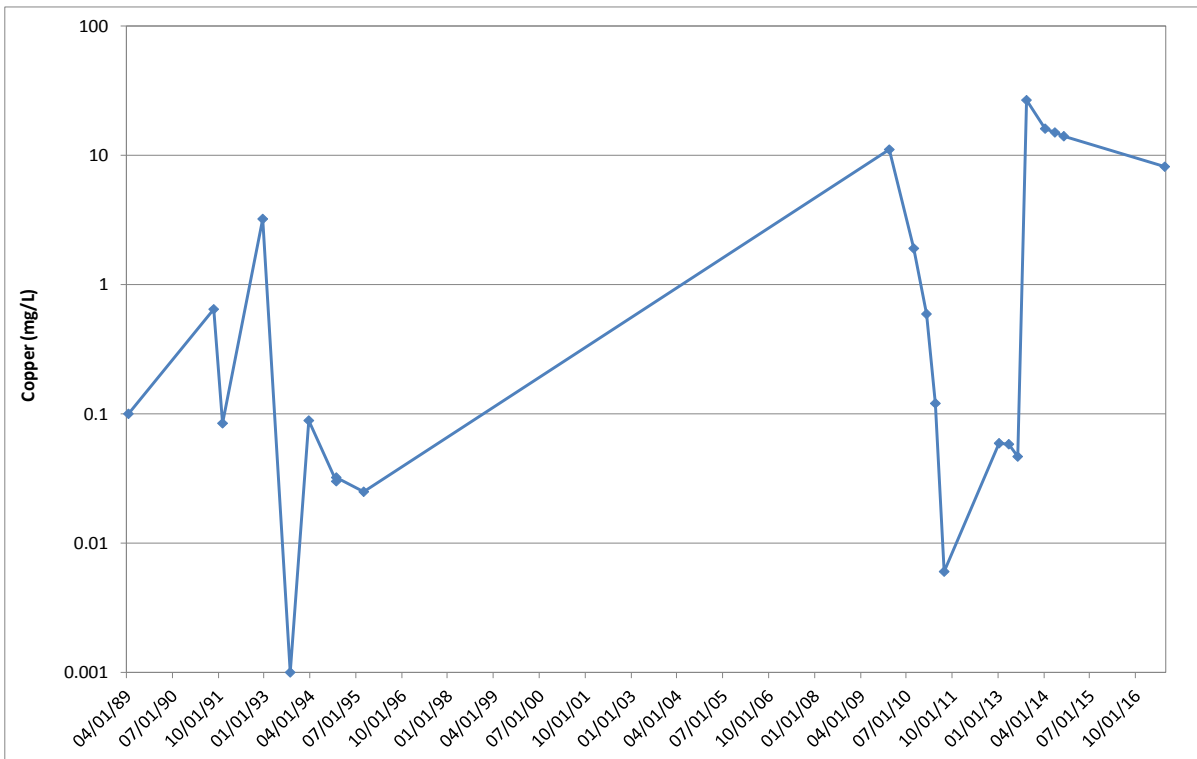
Copper concentrations in the open pit are influenced by AWS events (Figure 1-9). The elevated copper concentrations observed in 2010 are naturally mitigated to below analytical detection limits by 2011. This demonstrates that pit lake chemistry is temporally variable, with copper concentrations varying from below analytical detection limits up to a maximum of 26.5 mg/L.

Temperature and dissolved oxygen profiles for the existing pit lake (INTERA, 2012, Aquatic Consultants, 2014) show the pit water is not significantly stratified. The water stays well oxygenated for the entire depth for each season (6 to 8 mg/L dissolved oxygen). Thermal stratification requires a 1°C change in temperature per meter (Wetzel, 2001), which can occur in the summer months as the upper water column heats up and the lower water column remains cool, and well oxygenated. Figure 1-12 also shows that there is no depth-dependent variation in key chemical constituents (pH, TDS, copper, iron, zinc, manganese). This supports the assumption that the current pit lake is not stratified and that no chemocline exists.

A biological assessment of the pit lake was performed by Aquatic Consultants, Inc. (Aquatic Consultants, 2014, Appendix J) as part of the baseline data gathering effort to determine if aquatic life was present in the existing pit lake. While some algae were identified in the waters, no zooplankton, macroinvertebrates and no fish species were recovered during sampling, indicating the pit lake does not provide a suitable aquatic habitat. The biological assessment in conjunction with the other information provided in this section demonstrates that the existing pit lake is an artificial water body created as a result of mineral extraction with little or limited ability to sustain aquatic life and should not be equated to conditions that may be encountered in natural lakes..



**Figure 1-8: Plot of Sulfate and Chloride Concentrations in Existing Pit Lake**



**Figure 1-9: Plot of Copper Concentrations in Existing Pit Lake**

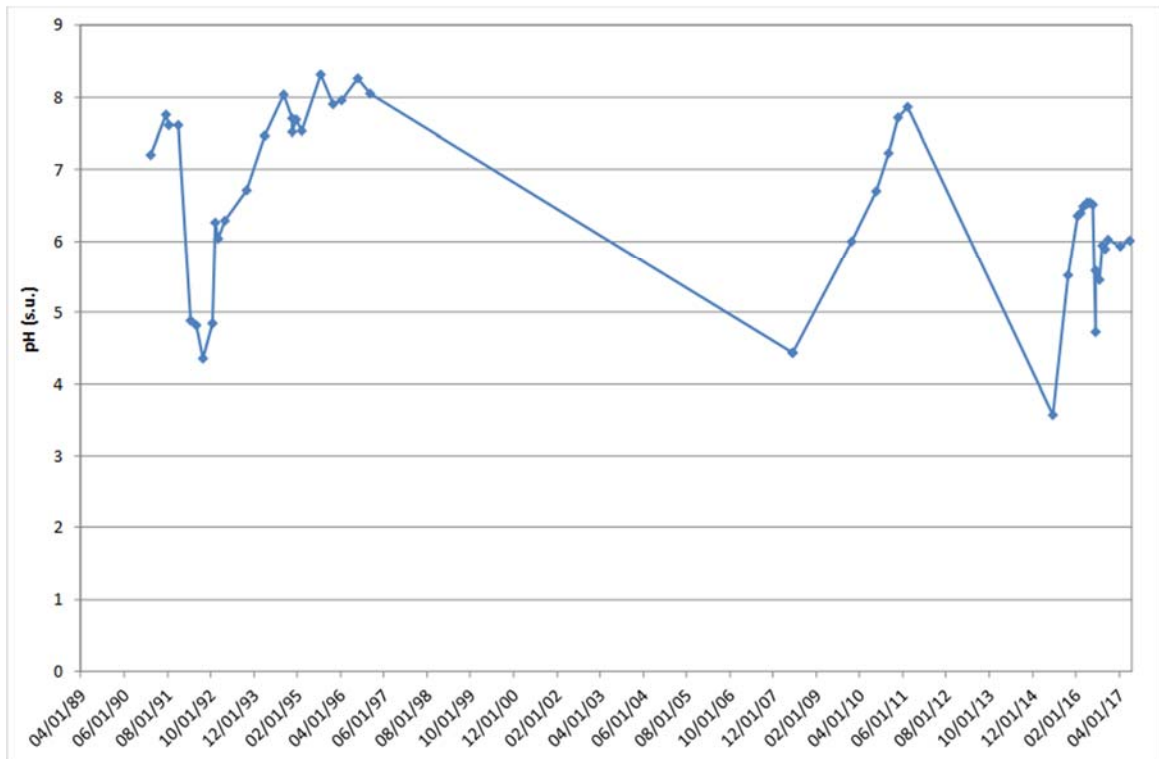


Figure 1-10: Plot of pH in Existing Pit Lake



Figure 1-11: Precipitated Salts around Rim of Existing Pit Lake

**Table 1-2: Existing Pit Lake Chemistry (1989 – 2017)**

Parameter	Units	n	Average	Minimum	Maximum
pH	s.u.	47	6.5	3.6	8.3
TDS	mg/L	56	7,538	2,711	14,800
Bicarbonate	mg/L	37	40.4	<3	122
Sulfate	mg/L	55	4,803	1,566	8,690
Chloride	mg/L	55	332	47.3	730
Fluoride	mg/L	33	19.2	4.8	34
Calcium	mg/L	37	550	455	684
Magnesium	mg/L	37	698	43	1,120
Sodium	mg/L	37	888	165	1,400
Potassium	mg/L	37	32.1	11	60.6
Aluminum	mg/L	33	10.4	<0.02	82.6
Antimony	mg/L	7	<0.001*		
Arsenic	mg/L	10	0.004	<0.001	0.006
Boron	mg/L	9	0.14	<0.1	0.2
Cadmium	mg/L	35	0.05	<0.005	0.1
Chromium	mg/L	11	0.03	<0.006	0.1
Cobalt	mg/L	32	0.29	<0.05	0.49
Copper	mg/L	22	4.44	0.001	26.5
Iron	mg/L	11	0.2	<0.02	1.3
Lead	mg/L	11	0.02	<0.005	0.1
Manganese	mg/L	35	34.8	0.02	59
Mercury	mg/L	10	0.0005	<0.0002	0.001
Molybdenum	mg/L	9	0.04	0.015	0.1
Nickel	mg/L	9	0.06	0.039	0.1
Selenium	mg/L	34	0.028	<0.001	0.25
Silver	mg/L	12	0.026	<0.005	0.1
Thallium	mg/L	8	0.0045	<0.001	0.005
Uranium	mg/L	4	0.11	0.11	0.12
Vanadium	mg/L	4	0.1	<0.05	0.25
Zinc	mg/L	33	5.4	0.01	9
Total Dissolved Solids	mg/L	56	7,538	2,711	14,800

*n* Number of samples

\* Indicates parameter was uniformly below analytical detection limits in pit lake water over monitoring period, but detection limit was variable. Concentration shown in table represents lower limit of analytical detection.

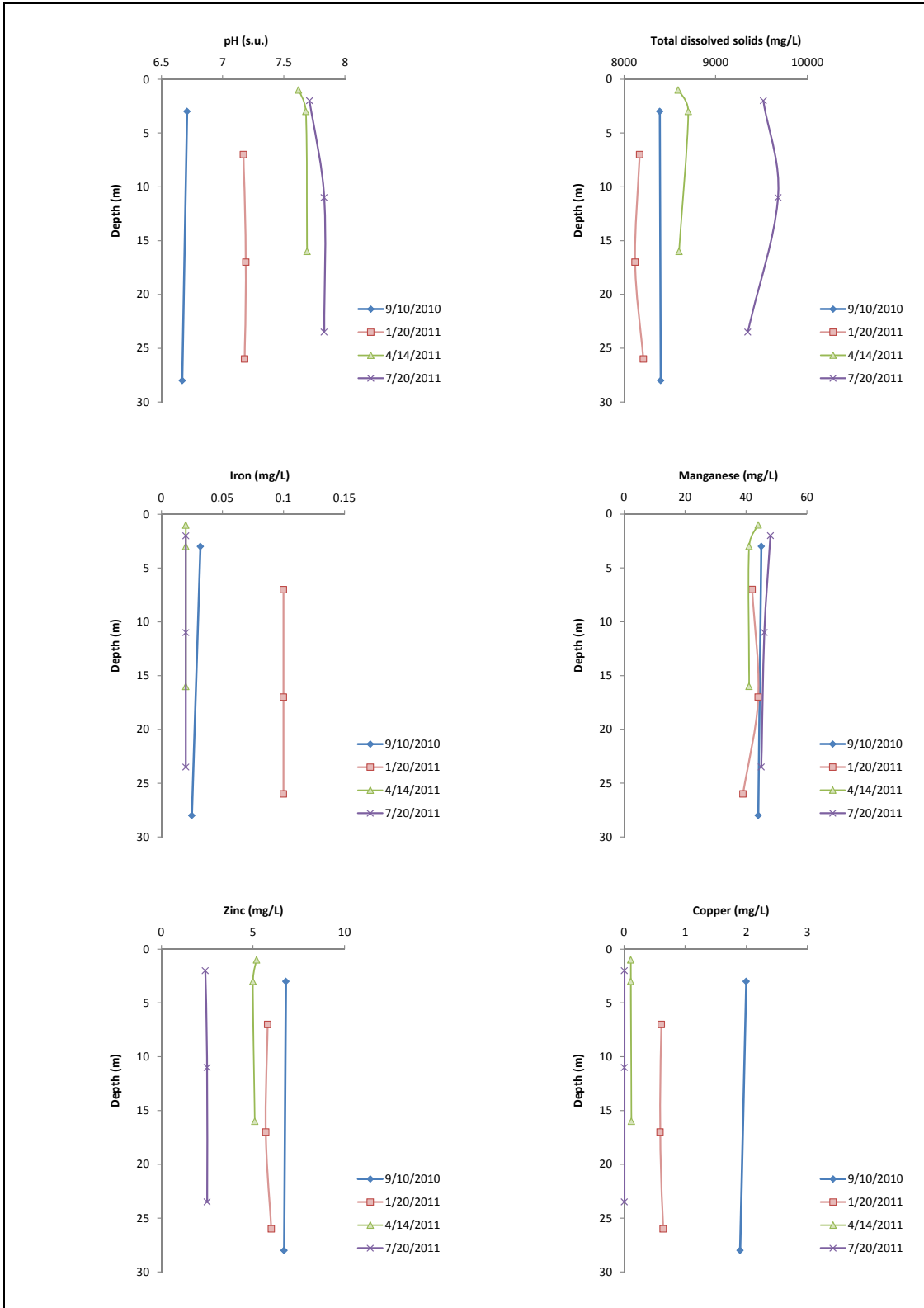


Figure 1-12: Depth Profiles of Key Constituents in Existing Pit Lake



## 2 Geochemical Characterization Testwork Summary

SRK has conducted a geochemical characterization program for the Copper Flat Project, which has included the testing of 91 waste rock samples, 41 samples representative of low grade ore and 11 samples of tailings material to investigate the potential for ARDML generation. The results of this program are presented in the *Geochemical Characterization Report for the Copper Flat Project, New Mexico* (SRK, 2012) and the main findings are summarized below.

Waste rock and ore sample intervals were selected from both exploration core holes drilled within the proposed pit boundaries in 2009, 2010 and 2011 and from the surface of existing WRSPs and pit walls on site. Samples were selected to represent the range of waste rock and ore material types that will be encountered during future mining. Tailings samples were collected from the metallurgical program and from the existing (historic) TSF on site. The static test methods used for the geochemical characterization program include multi-element analysis using four-acid digest and ICP-MS analysis, modified Sobek Acid Base Accounting (ABA), Net Acid Generation (NAG) test and the ASTM E2242-13 Meteoric Water Mobility Procedure (MWMP; ASTM, 2013). These static tests were selected to address total acid generation or neutralization potential of the samples and concentration of constituents in leachates derived from the material. However, these static tests do not consider the temporal variations that may occur in leachate chemistry as a result of long-term changes in oxidation, dissolution and desorption reaction rates. To address these factors, kinetic testing was also carried out as part of the geochemical characterization program and includes 32 humidity cell tests (HCTs) conducted on samples of waste rock, ore and tailings according to the ASTM D-5744-96 methodology (ASTM, 1996).

The results of the characterization program demonstrate that the acid generating potential of the Copper Flat waste rock is generally low and is largely dependent on the sulfide mineral content, with sulfide concentrations varying from less than analytical detection limits to a maximum of 2.52 wt%. The static testwork results indicate that the transitional waste material (i.e. mixed sulfide/oxide) is likely to be potentially acid forming based on a generally higher sulfide mineral content and the presence of secondary oxide minerals that formed as a result of supergene weathering. In contrast, the diabase, andesite and tailings are likely to be non-acid forming materials. The main material type for the Project consists of sulfide (i.e., non-oxidized) Quartz Monzonite and Breccia, which typically exhibited either non-acid forming characteristics or a low potential for acid generation. This is related to the encapsulation of sulfide minerals in a quartz matrix or occasionally in potassium feldspar. In addition, the sulfide minerals in the Copper Flat deposit are crystalline and often coarse grained and as such have slow weathering reaction kinetics. It is likely that the Copper Flat materials will offer limited silicate buffering (neutralizing) capacity; although this is unlikely to be high magnitude, it may modify/buffer pH in the near neutral range.

The Copper Flat waste rock and ore materials were found to be enriched in copper, sulfur and selenium in whole rock chemistry, which relates to the primary mineralization (predominantly chalcopyrite -  $\text{CuFeS}_2$ ). Silver, arsenic, cadmium, molybdenum, lead, thallium, uranium, tungsten, and zinc were also found to be enriched in one or more material types, with the greatest levels of enrichment occurring in the sulfide and transitional ore material types. Many of these elements are typically associated with copper porphyry deposits, which explain their enrichment in the Copper Flat materials (and more specifically in the ore grade samples). The diabase and andesite material types typically showed much lower levels of elemental enrichment, which is likely related to the lack of primary mineralization in these lithological units.

MWMP tests were conducted on a total of 49 waste rock and tailings samples to provide an indication of elemental mobility and metal(loid) release from the Copper Flat materials during meteoric rinsing. Metal mobility and release was also assessed from the results of the HCT program, the results of which are summarized in Appendix B. In general, metal leaching from the Copper Flat materials was found to be low and the majority of leachates generated during the MWMP and HCT

test programs could be classed as near-neutral, low-metal waters. However, several of the grab samples of transitional material collected from historic waste rock stockpiles produced acidic leachates and showed the potential for higher metal release than observed for the unoxidized sulfide materials. The higher release of acidity and metals from the transitional material likely represents the flushing of soluble acidic sulfate salts from the material surface that were produced by the prolonged weathering (over geological time) of the material.

## **3 Pit Lake Modeling**

### **3.1 Summary of Modifications to Pit Lake Models since submittal of SRK (2014a) Preliminary Report**

A number of modifications and refinements have been made to the Copper Flat pit lake models since the preliminary Pit Lake Geochemical Modeling Report was submitted in December 2014 (SRK, 2014a). These are detailed in Sections 3.1.1 to 3.1.8 below and are summarized in Table 3-1 at the end of this section.

#### **3.1.1 Incorporation of Current Geologic Block Model**

The revised models presented herein use the FS geologic block model to calculate the exposed surface areas of each lithology in the final pit walls. The FS block model represents the most up-to-date geological classification for the Project. Using the FS geologic block model results in minor changes to the relative proportions of each lithology that will be exposed in the final pit walls. In addition, the FS block model groups the biotite breccia and quartz feldspar breccia units together.

#### **3.1.2 Incorporation of Current Pit Design**

The revised models presented herein use the current pit design. The current pit design was developed along with the FS block model during the feasibility study and then modified to limit the future pit water body to private property with an expanded bench at the 4900 elevation in the NW corner of the pit (Figure 3-1). The current open pit design is detailed in the 2017 Mine Operation and Reclamation Plan (THEMAC, 2017a).

#### **3.1.3 Refinement of Pit Wall Composition**

The revised models include differentiation of the pit walls into mineralized and weakly to non-mineralized material, using a copper grade of 0.164% to differentiate between the two. This differentiation was used in addition to the lithology and oxidation classifications that were used in the original pit lake models (SRK, 2014a). The rationale for this refinement was based on a more in-depth review of the humidity cell chemistry data (see Appendix B), which showed that the release of certain parameters is greater from the mineralized material compared to weakly or non-mineralized material. As such, the source terms for these materials were defined separately. The redefinition and refinement of materials types within the pit walls provides a more representative calibration of existing pit lake conditions as described in Section 4 below.

#### **3.1.4 Refinement of HCT Inputs**

The revised models use different HCT inputs for trace elements and major ions to represent the different geochemical processes that control their release. An average of all weeks of humidity cell data were used for major ions (calcium, magnesium, sodium, potassium, aluminum, iron, manganese, chloride, sulfate, fluoride, bicarbonate) and an average of steady-state humidity cell data (i.e. minus the first 20 weeks of testing) were used for trace elements (silver, arsenic, boron, barium, cadmium, cobalt, chromium, copper, mercury, molybdenum, nickel, lead, antimony, selenium, uranium, vanadium and zinc). The main driver for this change in the input of HCT data was based around the improved calibration to existing conditions obtained by using the different sources of data. The results indicate that soluble salts are important in the input of major elements to the existing lake and, as such, all weeks of humidity cell data are needed for a valid prediction. By contrast, the release of trace elements is predominantly associated with longer term weathering processes, possibly sulfide oxidation and as a result the initial HCT flush concentrations were not included in the source term chemistry. Consequently, a closer calibration between predicted and

observed chemistry in the existing pit lake is achieved using this ‘mixed’ approach to humidity cell chemistry as described in Section 4.

### 3.1.5 Refinement of Mineral Equilibrium Phases

Minor modifications have been made to the mineral equilibrium phases specified in the PHREEQC model input file. This refinement was based on mineral phases that were observed to be close to saturation in the preliminary outputs to the refined model.

### 3.1.6 Refinement of Water Balance

Since submission of the December 2014 preliminary pit lake modeling report, JSAI has refined the pit lake water balance for the future pit lake to reflect an evaporation rate of 50 inches per year, compared to the 64 inch evaporation rate used previously. This refinement was based on the relationship between maximum ET ( $ET_0$ ), meteorological parameters including temperature, relative humidity and wind speed, and geographical parameters including altitude, latitude and time of year. Further details are provided in Appendix C.

In addition to the revised evaporation rate, the water balance and geochemical models were revised to reflect post-reclamation conditions for the proposed open pit and surface drainage area as presented in the 2017 MORP (THEMAC, 2017a) and summarized herein. The revised geochemical model includes separate source terms for reclaimed and unreclaimed areas of the pit and receiving watershed. Stormwater sourced from reclaimed pit areas is expected to have a chemistry similar to background surface water quality from SWQ-1.

Further details of how runoff coefficients were defined are provided in Appendix G.

### 3.1.7 Revisions to Groundwater Chemistry Inputs

JSAI developed a revised groundwater input chemistry from the available historic data. JSAI used the water quality database, well construction data and groundwater flow model results to determine the most representative groundwater flow chemistry to the existing and future open pits. Further details on how the groundwater chemistry inputs were refined are provided in Appendix D.

### 3.1.8 Incorporation of Pit Reclamation Measures

NMCC has developed a Mine Reclamation Plan for the Copper Flat Project (THEMAC, 2017a, THEMAC, 2017b, Golder, 2017). Pit reclamation aspects included in the MORP are:

- Reclamation of the pit haul road;
- Reclamation of the expanded section of the 4900 catch bench;
- Reclamation of benches at the crest of the pit; and
- Rapid fill of the open pit with fresh water from the production water supply wells after mining to create a pit lake with water surface at the 4987 feet elevation.

These reclamation measures are described in the following sections.

#### ***Pit Haul Road and Pit Bottom***

The open pit will be mined in benches over a 12 year period to create a terraced pit wall (Figure 3-1). Access into the open pit during mining will be via a 90 foot wide haul road constructed in the pit wall as mining advances. After mining, the haul road from pit crest to pit bottom will be covered with a suitable reclamation material. In addition, several benches at the bottom of the pit will also be covered in a similar manner before pit flooding occurs (Figure 3-2). The section of haul road above the final pit lake water surface will be prepared for revegetation as described in the MORP (JSAI, 2017a).

The reclaimed haul road will be used to convey stormwater to the bottom of the pit in a controlled manner. A system of surface water conveyance channels will be constructed around the pit crest to intercept and direct stormwater to the bottom of the pit through an engineered stormwater channel that is constructed in the alignment of the pit haul road.

**Expanded 4900 Catch Bench**

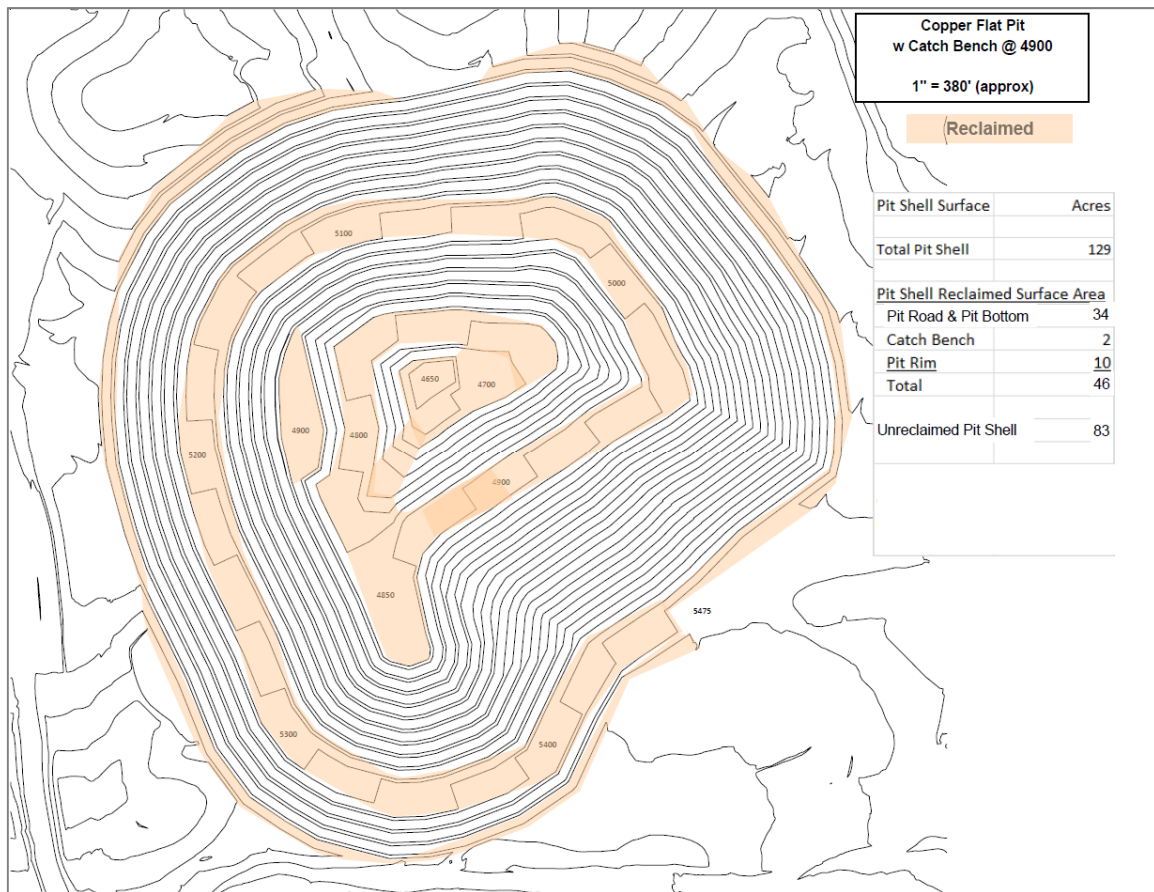
The 4900 elevation catch bench will be expanded to approximately 2 acres in size in the northwest corner of the pit (Figure 3-1). The surface of this catch bench will remain above water after rapid-fill is complete and the pit lake is established. The catch bench surface will be ripped and a growth media cover placed. The covered area will be revegetated.

**Pit Crest**

The upper benches of the pit shell will be laid back at an approximate 2:1 slope angle at the end of the mine operations to accommodate revegetation. The reclaimed benches will be blended into the surrounding reclaimed pit perimeter area described in the MORP. Revegetation will be accomplished by ripping the area and a growth media cover placed and re-contoured to blend with reclamation of the pit perimeter area and revegetated as described in the MORP.

**Rapid Fill**

After mining, the pit will be filled with fresh water coming from the mine freshwater production wells to rapidly create a pit lake (rapid fill). The rapid fill will begin immediately after mining and will be completed in approximately six months. The rapid fill requires pumping 2,200 acre-feet into the pit and will fill the pit to the 4894 ft elevation (JSAI 2017b).



**Figure 3-1: 2017 MORP Pit Showing Expanded 4900 Catch Bench and Pit Surfaces Scheduled for Cover**

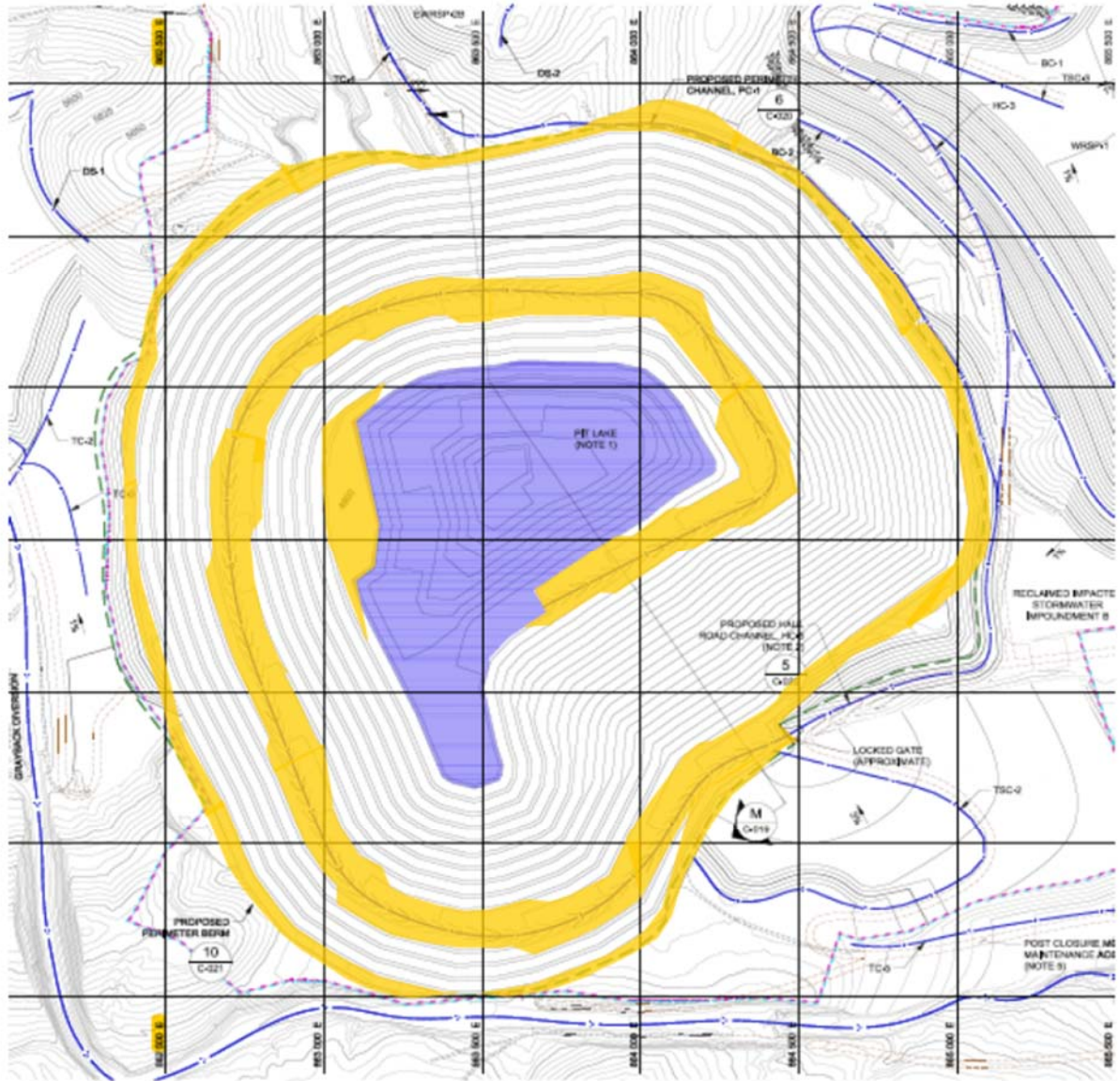


Figure 3-2: 2017 MORP Pit Showing Reclaimed Pit with Pit Lake

**Table 3-1: Summary of Modifications to Pit Lake Models since Submittal of Preliminary SRK (2014a) Report**

Component	Changed from (SRK, 2014a)	Changed to (current)
Geologic block model	PFS block model	FS block model
Pit shell	PFS pit shell	2017 MORP Pit
Pit wall composition	Delineated based on lithology and oxidation only	Delineated based on lithology, oxidation and mineralized versus weakly/non-mineralized
Source terms/HCT inputs	An average of all weeks of HCT data were used to develop source terms for each material type	Separate source terms were developed for major ions and trace elements. <ul style="list-style-type: none"> <li>• <b>Major ions</b> (Ca, Mg, Na, K, Al, Fe, Mn, Cl, SO<sub>4</sub>, F, HCO<sub>3</sub>): used an average of all weeks of HCT data</li> <li>• <b>Trace elements</b> (Ag, As, B, Ba, Cd, Co, Cr, Cu, Hg, Mo, Ni, Pb, Sb, Se, U, V, Zn): used steady-state HCT chemistry (i.e., minus the first 20 weeks of testing).</li> </ul>
Mineral equilibrium phases	Alunite, Ag <sub>2</sub> Se, albite, anhydrite, azurite, barite, boehmite, brochantite, brucite, calcite, chrysotile, Cr <sub>2</sub> O <sub>3</sub> , diaspore, epsomite, ferrihydrite, fluoride, gypsum, gibbsite, gummite, kaolinite, magnesite, malachite, mirabilite, otavite, pyromorphite, rhodochrosite, rutherfordine, schoepite, sepiolite, SiO <sub>2</sub> ; tenorite, U <sub>3</sub> O <sub>8</sub> , UO <sub>3</sub> , UO <sub>2</sub> (OH) <sub>2</sub>	Minor modifications were made to the equilibrium phases based on the predicted geochemical conditions. <ul style="list-style-type: none"> <li>• <b>Phases added:</b> CaMoO<sub>4</sub>, CaSeO<sub>3</sub>·2H<sub>2</sub>O, CdMoO<sub>4</sub>, Cr<sub>2</sub>O<sub>3</sub>, CuMoO<sub>4</sub>, Cu<sub>2</sub>Se, Mg<sub>3</sub>(PO<sub>4</sub>)<sub>2</sub>, MnSeO<sub>3</sub>, NiMoO<sub>4</sub>, Ni(OH)<sub>2</sub>, Ni<sub>3</sub>(AsO<sub>4</sub>)<sub>2</sub>·8H<sub>2</sub>O, PbMoO<sub>4</sub>, SbO<sub>2</sub>, ZnMoO<sub>4</sub>.</li> <li>• <b>Phases removed:</b> boehmite, diaspore, gibbsite, magnesite, malachite, pyromorphite, rhodochrosite, tenorite.</li> </ul>
Water balance	Evaporation rate of 64 inches.	Evaporation rate of 50 inches. Separate water balance terms were also developed for run-off from reclaimed surfaces in the pit and pit catchment.
Groundwater chemistry	Average of data for wells GWQ96-22A, GWQ96-22B, GWQ96-23A, GWQ96-22B, GWQ11-24B and GWQ11-25B.	Average of data for wells GWQ96-22A, GWQ96-22B, GWQ96-23A, GWQ96-22B and GWQ11-24B. Different groundwater inputs were also developed for the current and future pits according to the relative contribution of flow from the Quartz Monzonite and Andesite units.
Pit reclamation	None	Haul road will be reclaimed and revegetated, pit shell crest and expanded 4900 catch bench will be revegetated. Pit void will be rapidly filled with water from water supply wells.

## 3.2 General Pit Lake Modeling Approach

The results of the geochemical characterization testwork have been coupled with site-specific hydrologic, hydrogeologic and mine plan information to develop geochemical predictions of pit lake water quality for the Copper Flat Project. Geochemical predictions have been developed for three scenarios, including:

- (i) Calibration model for the existing pit lake;
- (ii) Natural fill model for the future unreclaimed pit; and
- (iii) Rapid fill model for the future reclaimed pit.

The conceptual models, inputs and assumptions for each of these model scenarios are presented in Sections 4, 5 and 6. The general approach to the modeling is provided in Sections 3.4 to 3.10 below.

Water chemistry predictions were made using the USGS code PHREEQC (Parkhurst and Appelo, 2010), which has been rigorously tested and is the industry standard for pit lake, waste rock dump and tailings facility geochemical predictions. The approach used herein is consistent with the industry-standard approach for modeling pit lake chemistry. Comparable approaches are reported in Tempel et al. (2000), Eary (1998) and Castendyk and Webster-Brown (2007).

The PHREEQC software uses thermodynamic equilibrium chemistry and solubility calculations to determine the residual concentration of mixing of solutions, allowing for mineral precipitation and attenuation of solutes through sorption reactions with specified mineral surface area. Furthermore, dissolution and oxidation can also be factored into the model to account for reaction with solid mineral phases which can be declared in the model in finite quantities. The resulting model output predicts not only the concentration of modeled elements but also the speciation of the aqueous solutes and the potential saturation indices of minerals of constituent components. This allows a geochemist to interpret trends in water quality data and to predict the resulting chemistry of the mixing reactions. These results are then compared to environmental and ecological risk water quality criteria to determine if a potential impact will result from the mineral-solute reactions. If appropriate, these data can also inform the development of mitigation strategies.

Data used as inputs to the models were derived from the following sources:

- Geological and mine planning information from the Baseline Data Report (INTERA, 2012), Feasibility Study (M3, 2013), the FS geologic block model, and the 2017 MORP (THEMAC, 2017a);
- Hydrologic and hydrogeologic information from the JSAI pit lake water balances developed for the three model scenarios;
- Geochemical data from laboratory humidity cell tests performed on representative mineralized and non-mineralized materials and then scaled to field conditions. These data were utilized to provide source term data for chemical leaching of exposed rock in the pit walls;
- Precipitation chemistry data from long-term monitoring at the Gila Cliff Dwellings National Monument meteorological station, New Mexico (NADP, 2012);
- Groundwater chemistry data from the groundwater monitoring program; and
- Published thermodynamic data provided with USGS PHREEQC and updated with additional sorption data for arsenic and manganese species.

These data were used to develop representative conceptual hydrogeochemical models for the three model scenarios.

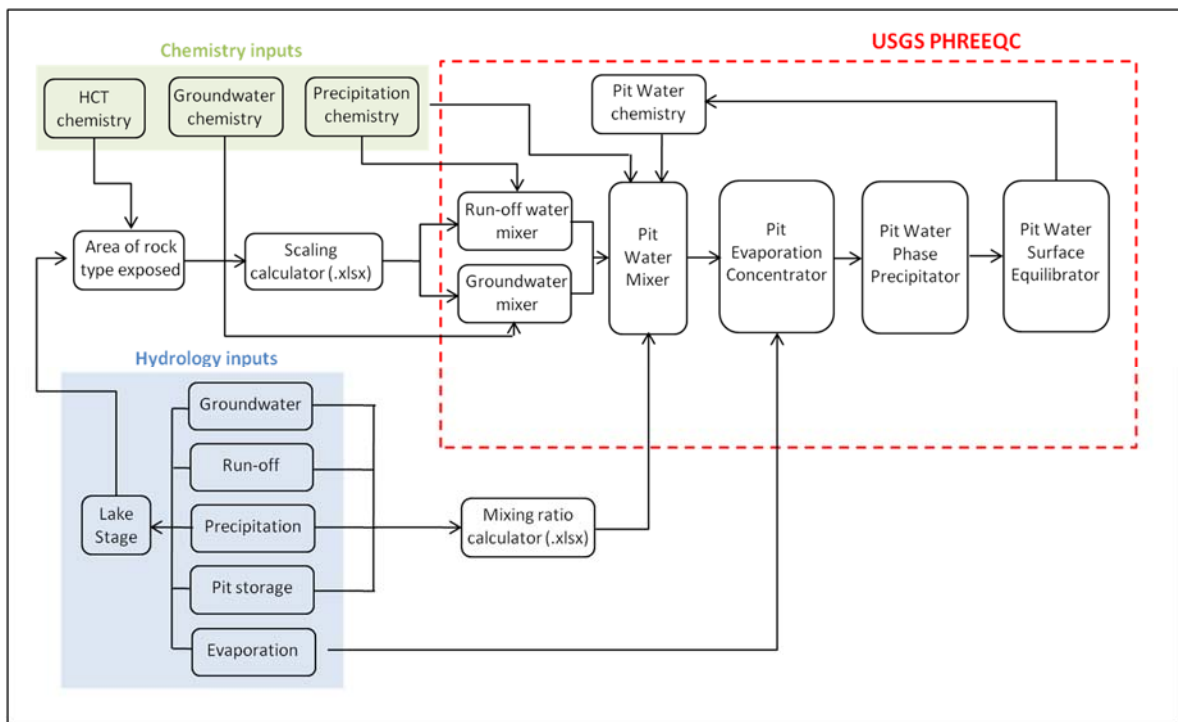


### 3.3 Model Logic and Coding

The conceptual models developed for the Copper Flat pit lake were translated into numerical models using a geochemical thermodynamic equilibrium code and several limiting and simplifying assumptions. The Copper Flat models used a modified version of the minteq.v4 thermodynamic database supplied with the v3.3.12.12704 version of PHREEQC (released May 10<sup>th</sup> 2017). This database is widely used for geochemical modeling and was selected for this study because it includes the full range of elements for consideration in this water quality prediction as well as key sorption reactions for iron oxyhydroxides. The database was modified to include sorption data for arsenic and manganese species.

The PHREEQC model consists of several components including the input data file, the thermodynamic database, the executable code and the output file. The input file consists of a series of logic statements and commands that define each of the components of the system and explains how these components interact. The input file is read by the executable code and commands are executed in a stepwise manner. Influent component waters were speciated and mixed to generate a series of intermediate waters, solid phases, and adsorbed phases. Selected outputs are specified and parceled out to various output files for analysis of results.

A logic flow diagram for the structure of the input code is provided in Figure 3-3 and discussed below. The PHREEQC input code is provided in Appendix H.



**Figure 3-3: Copper Flat Pit Lake Model Execution Mechanics**

The steps in the modeling process include the following items:

1. Define run-off water input specific to each exposed rock type. The run-off solution chemistries are comprised of scaled kinetic test cell leachate concentrations for each material type. These leachates are scaled to the water:rock ratio from the cell to the field based on the estimated presence of fractures in the wallrock and the thickness of the reaction rind.
2. Define the run-off solution mixing ratios. Mixing ratios are based on the amount of each material type that is sub-aerially exposed in the pit high wall at each time step.
3. Define the groundwater input. Groundwater chemistry is based on a mass addition function that combines the existing mass found within the groundwater with the mass of solute (per unit surface area and rock mass) released in the kinetic tests for specific material types exposed in the final pit walls. This is scaled to the water:rock ratio from the cell to the field, based on the estimated thickness of the reaction rind within the fractured wallrock.
4. Define groundwater solution mixing ratios based on the exposed surface area for each material type within the pit wall below the pit lake surface (i.e., within the submerged pit wallrock). As with the run-off mixing ratio, this ratio is dependent on the pit lake elevation and changes at each simulated time step.
5. Define precipitation water chemistry based on representative chemical analyses of rainwater.
6. Perform a master mixing calculation where run-off waters, groundwater, atmospheric precipitation and existing pit lake waters are mixed in ratios defined by the site-wide water balance for each time step.
7. Evapoconcentration. The resulting pit water is concentrated by a factor equivalent to the calculated evapoconcentration determined by the site-wide water balance for each determined time step. A fixed percentage of water is removed as a reverse titration of water. At the end of each titration, the volume of water is readjusted to one liter.
8. Equilibrate and precipitate. Once mixed, the model is equilibrated with atmospheric gases and select mineral phases are allowed to precipitate at the calculated pH, with pE fixed at a subatmospheric value equal to 12 minus pH. This represents a transitional equilibrium between mixed pit lake water and the atmosphere and is the most likely scenario based on the conceptual model.
9. Calculate sorption. After mineral precipitation, trace elements were allowed to adsorb onto iron oxyhydroxides (i.e., ferrihydrite). The total mass of ferrihydrite is equivalent to the mass predicted to be generated during the previous reaction step. This assumption is conservative in that it does not account for sorption to other minerals such as aluminum oxide or clay, or to iron oxides present in the pit wallrock.
10. Save chemistry for the next time step. At the end of each time step, the predicted pit water chemistry is exported to a spreadsheet for analysis.
11. The model was terminated after sufficient iterations to simulate water quality over a 100-year filling period.

### 3.4 Mineral and Gas Phase Equilibration

For the purpose of the Copper Flat geochemical models, it was assumed that any run-off, groundwater and precipitation entering the pit would mix evenly and completely. Under these circumstances the solutes in these waters will react with each other and may form chemical precipitates if the concentrations and geochemical conditions (Eh, pH, pCO<sub>2</sub>, pO<sub>2</sub>, and ionic strength) allow super saturation to occur. The geochemical models required the specification of a number of equilibrium phases that were allowed to precipitate if they become oversaturated. The suite of minerals chosen was based on the geology and mineralization of the deposit, an understanding of the types of minerals commonly observed in waste rock leachates and an assessment of mineral phases that were close to saturation based on the initial model iterations.

The relative saturation of all minerals was calculated by comparing the calculated concentration of dissolved ionic pairs with their theoretical thermodynamic limit. Where these values were equal, the saturation index was zero and the solution was said to be at equilibrium with that mineral. At equilibrium, any amount of the mineral that dissolves will precipitate to maintain the relative solute: mineral balance. The target saturation index was set to zero and the minerals that were allowed to form in the geochemical model are given in Table 3-2. These precipitates will sink to the bottom of the pit lake and be removed from future chemical interactions as a sediment layer accumulates on the pit bottom. The precipitated mineral phases are unlikely to re-dissolve unless the pH or redox conditions of the pit lake change substantially. As such, the model assumes that precipitated mineral phases are removed from the system and that subsequent re-dissolution of these phases does not occur. Sulfide mineral reactions are already accounted for in the model because HCT data were used as inputs. The HCT test provides an estimate of long-term accelerated rates of elemental release as a result of oxidation reactions, including sulfide mineral oxidation. Kinetic data for sulfide mineral phases are also limited, with data generally being limited to silicate mineral phases. Further, in evaluating long term changes to water chemistry it is reasonable to assume thermodynamic equilibrium will be attained by the system and as such the approach taken in this study is valid.

**Table 3-2: Equilibrium Phases Included in the Pit Lake Geochemical Model**

Equilibrium phase*	Ideal formula	Rationale for inclusion in PHREEQC model
Alunite	$KAl_3(SO_4)_2(OH)_6$	Mineral observed at Copper Flat (SRK, 1996; 1997)
Anhydrite	$CaSO_4$	Close to saturation in initial model runs.
Barite	$BaSO_4$	Primary control on barium at neutral to alkaline pH (Eary, 1999). Mineral observed in Copper Flat mineralogical study (SRK, 2014b)
$Ba_3(AsO_4)_2$	$Ba_3(AsO_4)_2$	Close to saturation in initial model runs.
Brochantite	$Cu_4^{2+}(SO_4)(OH)_6$	Primary control on copper at neutral to alkaline pH (Eary, 1999). Mineral observed at Copper Flat (SRK, 1996; 1997).
Brucite	$Mg(OH)_2$	Close to saturation in initial model runs.
Calcite	$CaCO_3$	Primary control on alkalinity at neutral to alkaline pH (Eary, 1999). Mineral observed at Copper Flat (SRK, 1996; 1997)
$CaMoO_4$	$CaMoO_4$	Close to saturation in initial model runs.
$CaSeO_3 \cdot 2H_2O$	$CaSeO_3 \cdot 2H_2O$	Close to saturation in initial model runs.
Carnotite	$K_2(UO_2)_2(VO_4)_2 \cdot H_2O$	Close to saturation in initial model runs.
Chrysotile	$Mg_3Si_2O_5(OH)_4$	Close to saturation in initial model runs.
$Cd(BO_2)_2$	$Cd(BO_2)_2$	Close to saturation in initial model runs.
$Cr_2O_3$	$Cr_2O_3$	Close to saturation in initial model runs.
$CuMoO_4$	$CuMoO_4$	Close to saturation in initial model runs.
$Cu_2Se$ (alpha)	$Cu_2Se$ (alpha)	Close to saturation in initial model runs.
Epsomite	$MgSO_4 \cdot 7H_2O$	Close to saturation in initial model runs.
Ferrihydrite	$5Fe_2O_3 \cdot 9H_2O$	Major control on iron. Thermodynamic properties well defined (Dzombak and Morel, 1990).
Fluorite	$CaF_2$	Primary control on fluoride (Eary, 1999). Mineral observed in Copper Flat mineralogical study (SRK, 2014b)
Gummite	$UO_3$	Close to saturation in initial model runs.
Gypsum	$CaSO_4 \cdot 2H_2O$	Primary control on sulfate (Eary, 1999). Observed in significant quantities around existing pit lake (SRK, 1996; 1997; 2014b).
HgSe	$HgSe$	Close to saturation in initial model runs.
$Mg_3(PO_4)_2$	$Mg_3(PO_4)_2$	Close to saturation in initial model runs
Mirabilite	$NaSO_4 \cdot 10H_2O$	Mineral observed at Copper Flat (SRK, 1996; 1997)
$MnSeO_3$	$MnSeO_3$	Close to saturation in initial model runs.
$NiCO_3$	$NiCO_3$	Primary control on nickel at neutral to alkaline pH
$NiMoO_4$	$NiMoO_4$	Close to saturation in initial model runs.
$Ni(OH)_2$	$Ni(OH)_2$	Close to saturation in initial model runs.
$Ni_3(AsO_4)_2 \cdot 8H_2O$	$Ni_3(AsO_4)_2 \cdot 8H_2O$	Close to saturation in initial model runs.
Otavite	$CdCO_3$	Primary control on cadmium at neutral to alkaline pH (Eary, 1999)
$PbMoO_4$	$PbMoO_4$	Close to saturation in initial model runs.
Rutherfordine	$UO_2CO_3$	Close to saturation in initial model runs.
$SbO_2$	$SbO_2$	Close to saturation in initial model runs.
Schoepite	$UO_2(OH)_2 \cdot H_2O$	Close to saturation in initial model runs.
Sepiolite	$Mg_4Si_6O_{15}(OH)_2 \cdot 6H_2O$	Close to saturation in initial model runs.
$SiO_2$ (am-ppt)	$SiO_2$	Close to saturation in initial model runs.
Tyuyamunite	$Ca(UO_2)_2V_2O_8(5-8)H_2O$	Close to saturation in initial model runs.
$U_3O_8$	$U_3O_8$	Close to saturation in initial model runs.
$UO_3$	$UO_3$	Close to saturation in initial model runs.
$UO_2(OH)_2$ (beta)	$UO_2(OH)_2$ (beta)	Close to saturation in initial model runs.
$ZnMoO_4$	$ZnMoO_4$	Close to saturation in initial model runs.

### 3.5 Adsorption

In solution, trace element concentrations are mostly controlled by adsorption onto common mineral phases or are removed from solution through a process of co-precipitation. The Copper Flat pit lake models assumed that trace metals may be removed from solution via sorption onto freshly generated mineral precipitates such as iron oxides. Sorption is likely to represent an important metal removal mechanism at circum-neutral to moderately alkaline pH, with many metal ions sorbing more effectively under these pH conditions. Ferrihydrite ( $5\text{Fe}_2\text{O}_3 \cdot 9\text{H}_2\text{O}$ ) was selected as a sorption surface because it is a common sorption substrate in oxygenated natural waters and because the trace element sorption thermodynamic properties of these reactions are well defined by numerous empirical studies. Adsorption of soluble phases to hydrous ferric oxides (HFO) is highly pH dependent as is the solubility of HFO itself. Below a pH of around 4.5, only minimal sorption of most dissolved metal species is observed (Stumm and Morgan, 1996). The mass of ferrihydrite used in the models was assumed to be identical to the mass of the mineral phase ferrihydrite precipitated in the previous model reaction step and is controlled by the chemistry of the system. The model assumes that the ferrihydrite is characterized by both strong (HFO\_s) and weak (HFO\_w) surface adsorption sites. In order to be consistent with the properties of ferrihydrite published by Dzombak and Morel (1990) the geochemical models assumed a surface site density of 0.2 moles of weak sites and 0.005 moles of strong sites per mole of ferrihydrite. Because the future pit lake predictions start from time zero (i.e., cessation of mining), there will be no prior pit lake in the void at that point. Any HFO/ferrihydrite will therefore originate from the precipitation of oversaturated mineral phases that develop upon solution mixing.

As with mineral phase precipitation, the adsorbed mass of trace elements removed through this mechanism is assumed in the conceptual model to be permanently removed from the system following incorporation and co-precipitation with the HFO phase. In the case of a major shift in pH or redox conditions, it is possible that material adsorbed to the HFO surface may be released. However, based on the HCT results available to date, a major shift in pH conditions is not likely.

### 3.6 Evapoconcentration

The Copper Flat pit lake is an evaporative sink, both in its current state and under future post-operational conditions (JSAI, 2017b). There will be no outflow to groundwater and the only mechanism of water loss will be through direct evaporation from the pit lake surface. As such, solutes within the pit lake will evapoconcentrate and the only mechanism for removing solutes is the formation and settling of chemical precipitates and the adsorption of trace elements onto these particulates.

### 3.7 Treatment of Analytical Reporting Limits

The Copper Flat pit lake models incorporate groundwater and humidity cell data that have been collected over extended periods of time, including both detectable elemental concentrations and constituent concentrations that may be below analytical reporting limits (ARL). The treatment of analytical reporting limits within the geochemical model has important implications for the model results, particularly where the data are scaled to address the difference in solid:liquid ratio between the laboratory-scale test and field conditions.

When analysis of the humidity cell leachates identified certain elements to be below the ARL, the reporting limit was adjusted to 10% of the reported limit for the purpose of calculating the average release rate for the model input. Where a constituent was consistently below the ARL throughout the course of the humidity cell testwork, the constituent was excluded from the model input for that material type to limit overstating constituent concentrations that may arise as an artifact of the modeling exercise from the scaling of humidity cell data to field conditions or from equilibration of groundwater source data that are below ARLs.

Nitrate was excluded from the geochemical predictions due to the lack of mineralogical controls in PHREEQC code. The exemption of nitrate is supported by the data as this parameter is consistently below the ARL in both the humidity cell effluent leachates and the groundwater surrounding the pit. Nitrate is also below the ARL in the existing pit lake, supporting the assumption that this parameter is unlikely to be a problem during future operations.

### 3.8 Model Assumptions and Limitations

The pit water quality predictions presented herein are considered the best representation of likely future water quality associated with the Copper Flat pit lake. However, it is recognized that there are a number of assumptions and limitations associated with the predictive calculations including:

- The models have been developed using site-specific geochemical, hydrochemical, geological, hydrogeological and mine plan information. Therefore, changes in operational decisions may result in a change in the future pit lake water quality at Copper Flat.
- The models assume that groundwater and surface water input chemistry can be simulated using laboratory kinetic (humidity cell) leachate chemistries, which are appropriately scaled to field conditions. The reactive surface area, ratio of water-to-rock and flushing rates in laboratory tests are different from actual field conditions. Grain size is smaller in the kinetic and static test cells and the resulting surface area for reactivity is greater than field conditions. The laboratory test cells are operated at a higher water-to-rock ratio than would be expected in the field and are flushed more frequently, so that mineral-water reaction rates are enhanced. Because the future Copper Flat pit does not yet exist, field scale parameters cannot be measured, so scaling relies on published estimates of future groundwater flux and fracture density. These estimates and assumptions are supported by the geochemical model for the existing pit (Section 4), which shows good calibration to current conditions.
- Modeling was limited to predicting water quality within the pit lake for a 100-year time period. This length of time is not intended to imply that the pit lake geochemistry or hydrogeology for the natural fill scenario will achieve steady-state, hydrogeochemical equilibrium at 100-years.
- The models rely on an external database of thermodynamic constants for mineral phase precipitates and sorbed surface complexes. These thermodynamic constants are valid at 25°C and 1 atmosphere of pressure. The models do not consider the effects associated with the formation and precipitation of mineral species other than those specified. Due to kinetic constraints, a portion of the potentially oversaturated mineral phases will not actually precipitate. A select suite of minerals is therefore specified that are allowed to precipitate, based on relevance for the environment in question, site-specific knowledge, experience in evaluating kinetic constraints and relevance of key phases for given styles of mineralization, and literature review (Eary, 1999). The nature of the thermodynamic databases means that the constants for all major elements and a large number of trace elements are well understood and have been rigorously tested and verified. However, constants for certain parameters (for example vanadium, boron and nitrate) are not as well understood. As such, the mineralogical controls on these elements in PHREEQC are poorly defined, which may affect their precipitation (i.e., removal) from solution in the predictive calculations.
- The models assume atmospheric equilibrium with oxygen and carbon dioxide gas, with pH + pE equal to 12 (based on calculations by Baas-Becking et al., 1960 to define stability limits of natural waters).
- The models are limited to thermodynamic equilibrium reactions and do not simulate the effects of reaction kinetics and rates.
- The models are limited to inorganic reactions and do not take into account the complexities associated with biologically mediated reactions.

None of these limitations affect the ability to use model as intended, which is to assess potential future pit lake chemistry and evaluate the future environmental impacts of the Project.

### **3.9 Analysis of Model Input Variability**

The various parameters that have been used as data inputs for the pit lake geochemical model have been assessed to determine their relative significance in influencing the model results. For the purpose of this exercise, each parameter has been assigned a qualitative value based on the degree to which it influences the final predicted solution chemistry:

- “Minor” represents less than 1% control on the final model output;
- “Moderate” represents between 1% and 10% control on the final model output; and
- “Significant” represents between 10% and 50% control on the final model output.

The results of this exercise are displayed in Table 3-3.

**Table 3-3: Analysis of Pit Lake Model Input Variability**

Category	Parameter	Assumptions / data used in model	Source	Control on final model results*
<b>Hydrogeologic information</b>	Pit lake water balances	Water balances provided by JSAI for the three model scenarios, including water elevation and surface area, groundwater inflows, direct precipitation, run-off and evaporation data.	JSAI, 2017	Significant. The water balances define the mixing ratios for the PHREEQC input solutions.
<b>Chemical inputs</b>	Groundwater chemistry	Baseline groundwater chemistry data from the ongoing monitoring program: average of data for wells GWQ96-22A, GWQ96-22B, GWQ96-23A, GWQ96-22B and GWQ11-24B.	INTERA, 2012; JSAI, 2017a	Significant during the early years post-closure when groundwater is likely to represent the dominant solution input to the pit lake.
	Precipitation chemistry	Averaged precipitation chemistry from Gila Cliff Dwelling National Monument Meteorological Station (1985-2011)	NADP, 2012	Minor. The precipitation chemistry represents a near-pure solution chemistry. In the absence of site-specific data, published precipitation chemistry from this meteorological station in New Mexico is the best representation of precipitation chemistry in the area.
	HCT chemistry	Averaged HCT chemistry from the HCT programs.	SRK 2012; 2014b	Significant. The solutions generated by the HCT programs represent the main chemical inputs for the pit wall source terms.
	Water Supply well chemistry (rapid fill model only)	Groundwater quality data from water supply wells PW-1 and PW-3	JSAI, 2017c	Significant. The water supply well chemistry represents the largest solution contributor to the pit lake during the first six months of filling.
<b>Geological information</b>	Pit wall surface area and lithologic composition	Pit wall surface areas were calculated for each simulated time step using the geologic block model and 2017 MORP pit	SRK/ NMCC	Significant. The lithological composition of the pit wall defines the mixing ratios for the PHREEQC input solutions.
<b>Geochemical model assumptions</b>	Mass of pit wall rock available for reaction	Mass of future pit wall available for reaction was calculated assuming an oxidized rind of 0.04 feet thickness and a fractured zone of 1 feet thickness (with 10% fractures).	SRK/ NMCC	Moderate. The values were assigned based on communication with NMCC regarding future blasting practices for the Project and are considered a conservative estimate and are consistent with industry practice.
	Equilibrium/mineral phases	The equilibrium/mineral phases listed in Table 3-2 were used as input to the models	SRK	Moderate. Mineral precipitation will influence final solution chemistry. Equilibrium phases were selected based on knowledge of site-specific geologic and mineralogic conditions and were then verified and refined by calibrating with the existing pit lake chemistry.

\* Minor: <1%  
Moderate: 1 - 10%  
Significant: 10 - 50%



### 3.10 Comparative Guidelines

The standards that apply to the post-mining Copper Flat pit water body are contained in the regulations MMD administers under the Mining Act; specifically “Performance and Reclamation Standards for New Mining Operations” at 19.10.6.603 NMAC. These MMD standards require that the pit water body comply to the following performance standard:

- Operations must be planned and conducted to minimize change in the hydrologic balance in both the permit and potentially affected areas; and
- Reclamation must result in a hydrologic balance similar to pre-mining conditions.

MMD must determine that the NMCC mine operating and reclamation plan complies with these standards before a mining permit can be issued. The mine plan must take into account the site-specific characteristics of the mining operation and the site in meeting the standards and requirements. The MMD regulations require that the permit area be reclaimed to a self-sustaining ecosystem appropriate for the life zone of the surrounding area following closure unless conflicting with the approved post-mining land use. Specifically, NMAC 19.10.6.603.C.(4), Hydrologic Balance states that the performance and reclamation standards identified in this subsection require that, if not in conflict with the approved post-mining land use, reclamation must result in a hydrologic balance similar to pre-mining conditions. Section 19.10.6.602.D.(13)(g)(v) of the regulations identifies the environmental baseline information required to establish pre-mining conditions and outlines the hydrologic and water quality data requirements for baseline data.

There are several site-specific factors to consider regarding the Copper Flat Project in determining what standards apply. First, the existing pit water body is and the future pit water body will be fully confined to private land. The two-acre catch bench at the 4900 ft amsl elevation of the pit ensures that the future pit lake remains on private property. The pit is and will be a hydraulic evaporative sink in the future, and, as such, is not a flow-through system (INTERA, 2012; JSAI, 2017b). As a result of being confined to private land and remaining a hydrologic sink, the current and future pit water body will not be a water of the state and the surface water standards the NMED Surface Water Quality Bureau (SWQB) administers will not apply to the pit water. Because the pit is and will be a hydraulic evaporative sink in the future, NMED Groundwater Quality Bureau (GWQB) standards are also not applicable to the future pit water body.

Therefore, the applicable standard for the future pit water body as provided by the MMD regulations will be “similarity”, NMCC must demonstrate that post-mining hydrologic conditions, i.e., the post-mining hydrologic balance is similar to the pre-mining hydrologic conditions. The MMD regulations do not contain a definition of “hydrologic balance. Nonetheless, Section 19.10.6.602.D.(13)(g)(v) requires that a determination be made of the probable hydrologic consequences of the operation and reclamation, including water quality. These two regulatory requirements are interpreted to require the NMCC demonstrate that that the water quality of the future pit lake be similar to that of the pre-mining pit water quality and, thus, allow NMCC to demonstrate that the water quality hydrologic consequence is nil.

This report provides the required demonstration as to the similarity of the future pit lake water quality to present pit lake water quality. In this report, the pit lake predictive model results are compared to existing pit lake water quality to demonstrate that the anticipated post-mining water quality of the future pit is similar to pre-mining pit water body quality present at Copper Flat today. In addition, the existing pit water body has been previously studied by Aquatic Consultants, Inc. (Aquatic Consultants, 2014) and it has been determined that the environment within the existing water body does not reflect a natural lake environment. There are no fish in the existing pit water body and water quality reflects the mineralized nature of the surrounding pit walls. When mining is complete, the pit water body will re-form; the NMCC reclamation and closure plan is designed to leave the future pit water body in a condition similar to its current condition.

## 4 Existing Pit Calibration Model

Numerical predictions have been undertaken to model the current (i.e., existing) pit lake chemistry in order to calibrate and verify the future pit lake geochemical predictions. A water balance for the existing pit was provided to SRK by JSAI and this was coupled with the results of the HCT testwork and data relating to the existing pit wall geology to carry out numerical simulations of water quality in the existing pit lake.

### 4.1 Conceptual Model

A conceptual model for the existing pit lake at Copper Flat is provided in Figure 4-1. The inputs to the model are discussed in Sections 4.2 to 4.5 below.

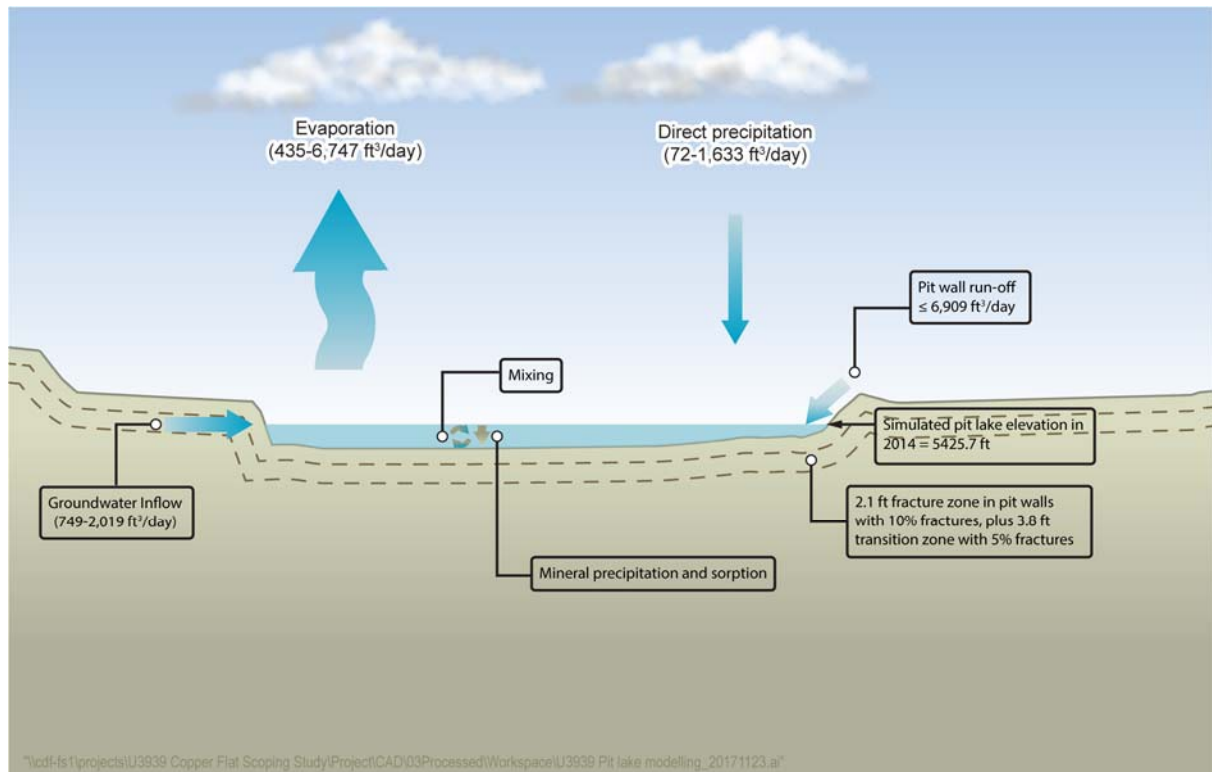


Figure 4-1: Existing Pit Conceptual Model

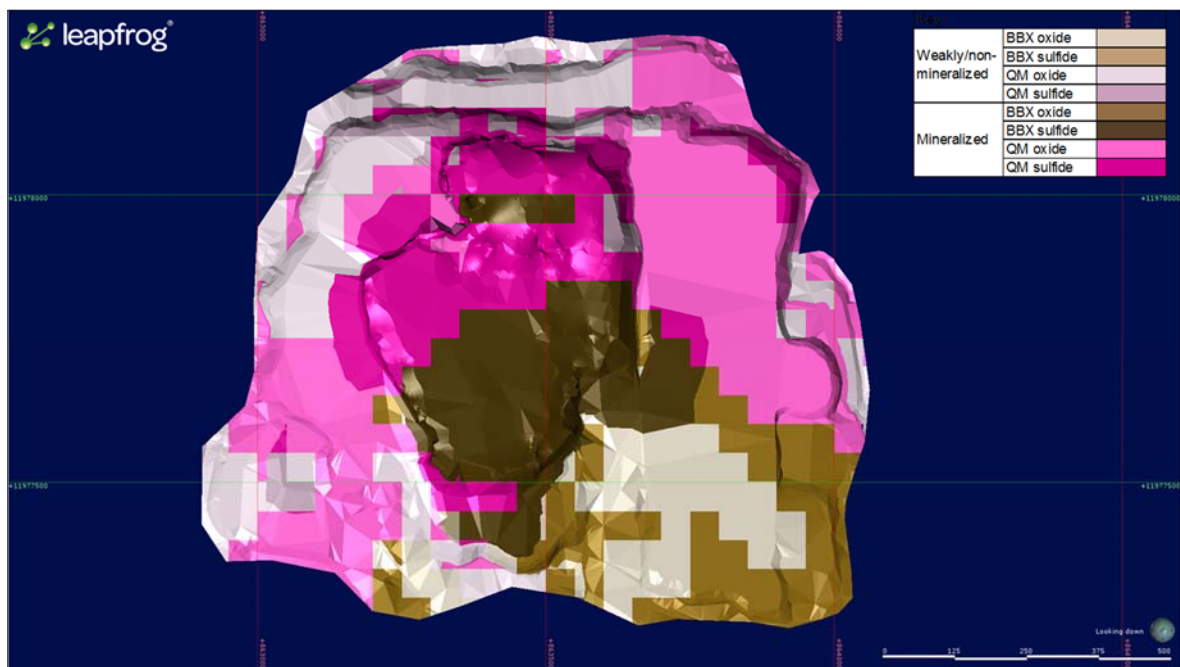
### 4.2 Pit Wall Surface Areas

The proportional surface areas of the main material types that are exposed in the existing pit walls have been calculated from the FS geologic block model. Material types have been delineated based on primary lithology, oxidation (redox) and mineralization (i.e., mineralized versus weakly/non-mineralized).

The three-dimensional surface areas used as input to the existing pit model are provided in Table 4-1 and are illustrated in Figure 4-2. This demonstrates that mineralized, oxidized quartz monzonite represents the dominant material type exposed in the existing pit walls.

**Table 4-1: Pit Wall Surface Areas used in Existing Pit (Calibration) Model**

Mineralization	Rock Type	Redox	Three-dimensional surface area	
			Square feet	%
Weakly/non-mineralized	Biotite Breccia	Oxide	88,213	8.5
		Sulfide (non-ox.)	5,073	0.5
	Quartz Monzonite	Oxide	171,155	16.5
		Sulfide (non-ox.)	27,011	2.6
Mineralized	Biotite Breccia	Oxide	118,474	11.4
		Sulfide (non-ox.)	153,348	14.8
	Quartz Monzonite	Oxide	291,547	28.1
		Sulfide (non-ox.)	184,085	17.1
<b>Total</b>			<b>1,038,906</b>	<b>100%</b>



**Figure 4-2: Material Types Exposed in Existing Pit (Calibration) Model**

### 4.3 Calculation of Pit Wall Rock Available for Leaching

During Quintana’s operations, the existing pit at Copper Flat did not reach its final configuration and the pit walls were not prepared using pre-split drilling and smooth wall blasting. Therefore, the existing pit wall has significantly deeper fracturing than predicted for the future final pit wall from the proposed operation. The literature demonstrates that open pit wall blast damage for granite, granodiorite and quartz monzonite rocks extends 2 to 4 ft in depth when assessing effects from production type blasting (e.g., Carroll and Scott, 1966; Siskind and Fumanti, 1974; Kelsall et al., 1984) (Appendix F).

For the existing pit lake scenario, an estimate of the reactive rind thickness is provided by results from a U.S. Bureau of Mines experimental study on fracturing produced in the vicinity of large-diameter blast holes in Lithonia granite (Siskind and Fumanti, 1974). From this study, a fractured zone (‘fracture zone’) was identified that extends approximately 2 feet into the pit wall and a second zone (‘transition zone’) characterized by a lesser degree of fracturing extends from approximately 2 to 4 feet (Figure 4-3). Oxygen infiltration extends no further than the predicted depth of fracturing of 2

feet, and that the percent of the rim rock mass fractured during mining will range from 10% within the fracture zone to 5% within the transition zone. This estimate of fracturing is supported by Atchison (1968). An oxidized rind of 0.04 feet thickness has also been assumed in the pit walls. This scenario is considered a conservative input of pit wall fracturing based on the information provided in Appendix F.

Using these assumptions for the fracture zone, transition zone and oxidized rind, the reactive mass ( $R_m$ ) of each material type in the pit wall was calculated as:

$$R_m = (S \times F_{FZ} \times L_{FZ} \times D) + (S \times F_{TZ} \times L_{TZ} \times D) + (S \times L_{OR} \times D)$$

Where:

$S$  is the three-dimensional pit wall surface area of the given material type in square meters (defined by the geological block model; see Table 4-1);

$F_{FZ}$  is the fracture density in the fracture zone (10%);

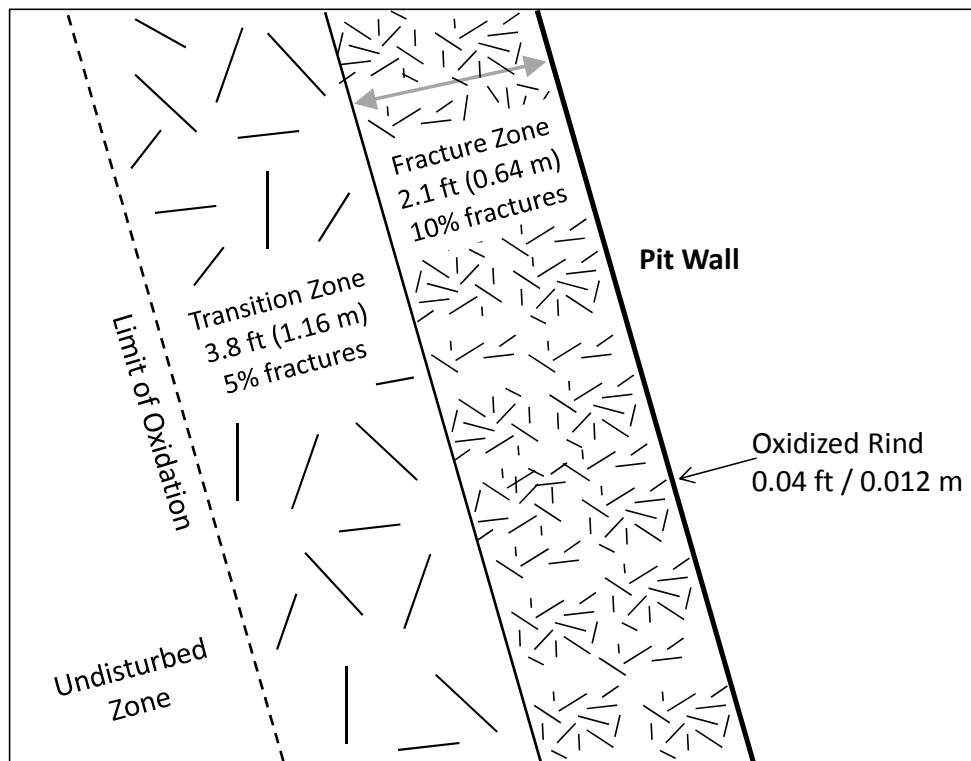
$L_{FZ}$  is the thickness of the fracture zone in meters (0.64m);

$F_{TZ}$  is the fracture density in the transition zone (5%);

$L_{TZ}$  is the thickness of the transition zone in meters (1.16m);

$L_{OR}$  is the thickness of the oxidized rind in meters (0.012m);

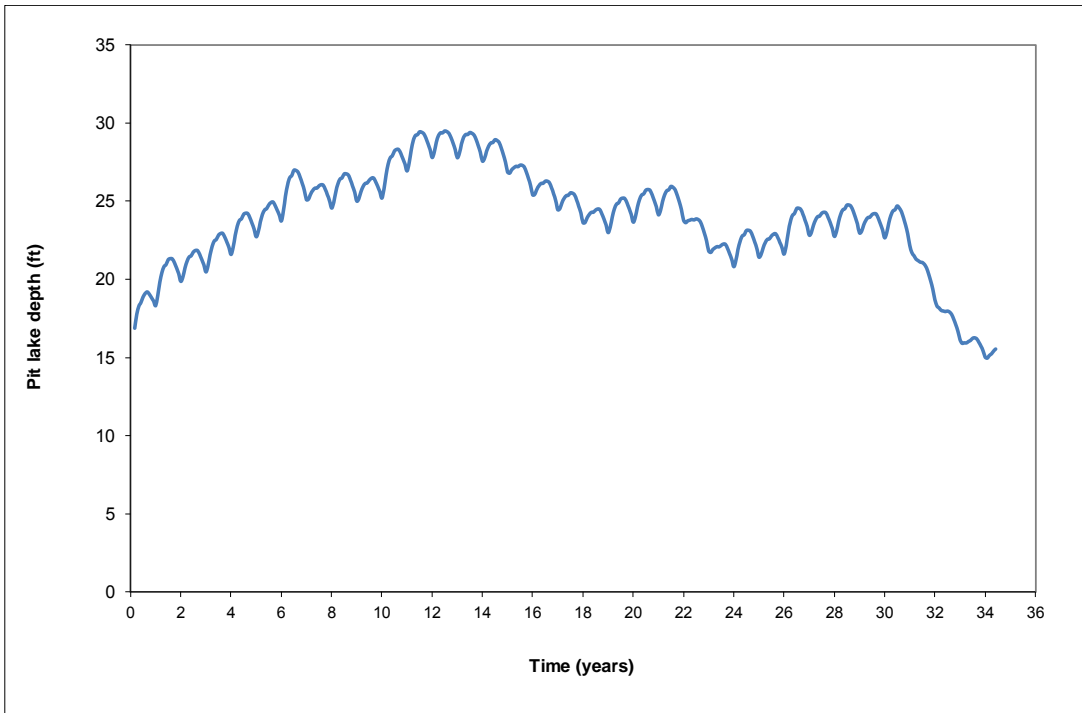
$D$  is the rock density in  $\text{kg/m}^3$  ( $2700 \text{ kg/m}^3$ , Young and Olhoeft, 1976).



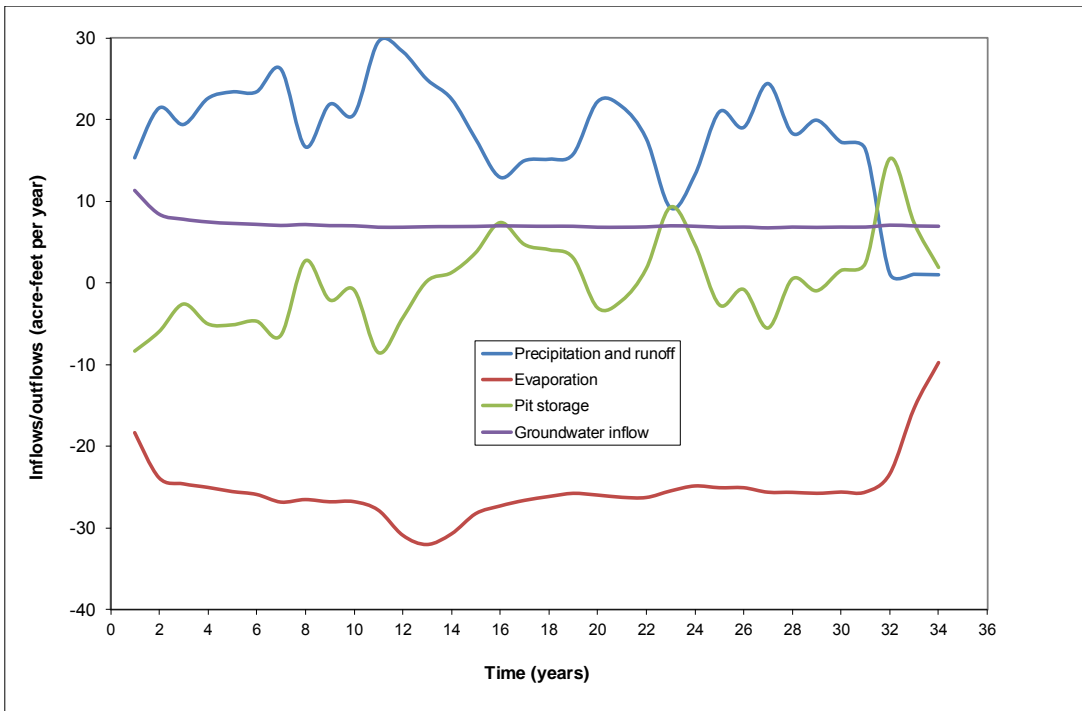
**Figure 4-3: Existing Pit Wall Conceptual Model**

## 4.4 Water Balance

A pit lake water balance for the existing pit lake was provided to SRK by JSAI. The water balance data used in the existing pit lake predictions are summarized in Figure 4-4 and Figure 4-5 below. Figure 4-4 shows the simulated pit lake elevation with time and Figure 4-5 shows the simulated inflows and outflows to the existing pit.



**Figure 4-4: Simulated Water Level for the Existing Pit Lake**



**Figure 4-5: Existing Pit Lake Inflows/Outflows**

## 4.5 Solution Inputs

### 4.5.1 Precipitation Chemistry

The primary wall rock lixiviant for the pit high walls in both the existing pit and the future pit is assumed to be rainwater (i.e. meteoric precipitation). Representative precipitation chemistry data were obtained from monthly monitoring carried out between 1985 and 2011 at the Gila Cliff Dwellings National Monument meteorological station, Catron County, New Mexico (NADP, 2012) (Figure 4-6). In the absence of any site-specific precipitation chemistry, this is considered the most representative precipitation chemistry available for use in both the existing and future pit lake models.

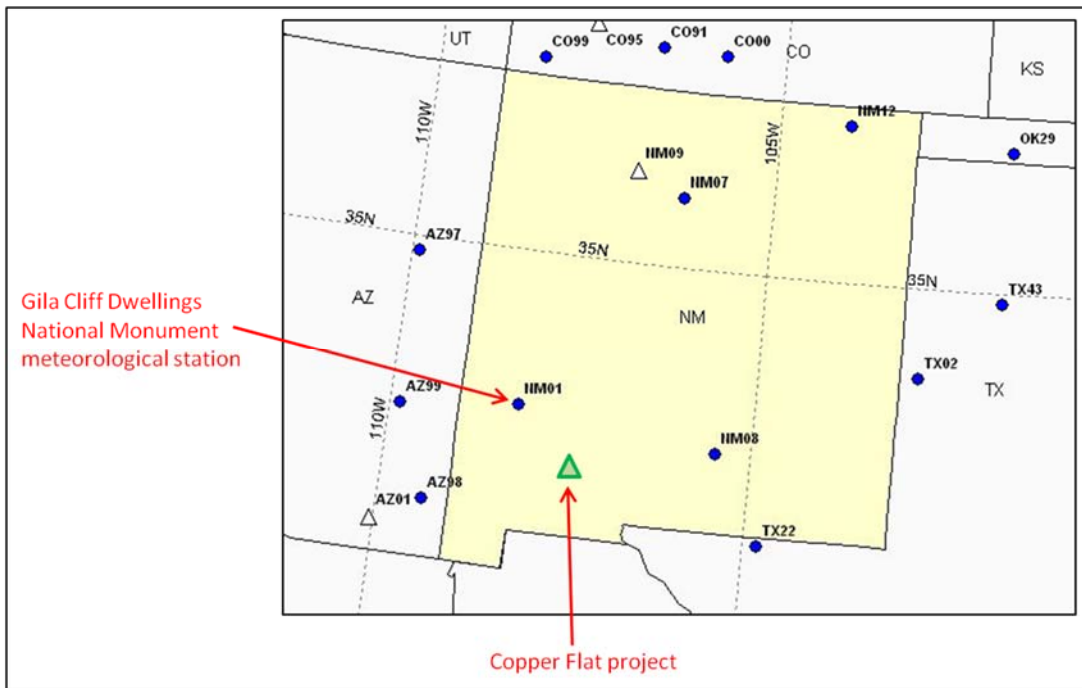


Figure 4-6: Location of Gila Cliff Dwellings National Monument Meteorological Station

### 4.5.2 Groundwater Chemistry

Representative groundwater chemistry data for the existing pit lake model were obtained from the historical data compiled by JSAI and NMCC. There are four sets of piezometers surrounding the existing pit that have been sampled, with two piezometer sets representing groundwater in the andesite (GWQ96-22[A,B] and GWQ96-23[A,B]), and two in the quartz monzonite (GWQ11-24[A,B] and GWQ11-25[A,B]). GWQ96-23(A,B) is located at the transition between andesite and quartz monzonite; however the water quality is similar to GWQ96-22(A,B) and indicative of andesite.

The results from wells GWQ96-22(A,B), GWQ96-23(A,B), GWQ11-24(B) and GWQ11-25(B) were averaged and used as input to the existing pit lake geochemical model (Table 4-2). Wells GWQ11-24A and GWQ11-25A were not used in the model input as they may have been affected by oxidation of sulfides in fractures during well development and may not be representative of groundwater reporting to the open pit. Furthermore, GWQ11-25A represents a localized and isolated fracture system recharged by oxygenated meteoric water that is not connected to the open pit (JSAI, 2017a). For these reasons, data from GWQ11-24(A) and GWQ11-25(A) were not considered as part of the groundwater inflow to the existing pit.

Further information on how the groundwater chemistry data were derived is provided in the JSAI technical memorandum in Appendix D.

### 4.5.3 Wall Rock Chemistry

Source term solutions for material types exposed in the existing pit walls at Copper Flat were developed from the results of site-specific HCT testing conducted as part of the SRK (2012) geochemical characterization program that were scaled to field conditions. The application of a scaling factor is necessary because laboratory tests are operated at a higher water-to-rock ratio than would be expected in the field, meaning that mineral-water reaction rates are enhanced in the laboratory. The scaling factor is based on site-specific information relating to the pit water balance, geological model, pit wall fracturing and wall rock density.

The reactive mass ( $R_m$ ) of pit wall rock available for chemical weathering reactions in both the unsaturated high wall and the submerged pit wall was calculated using the methodology outlined in Section 4.3. The reactive mass for each material type was coupled with the pit water balance to determine the changes in run-off and groundwater chemistry as any water that interacts with the pit walls migrates through the reactive fracture zones. This is demonstrated by the equation below:

$$C_i = \frac{r_i \cdot R_m}{Q}$$

Where:

$C_i$  represents the predicted concentration (in mg/L) of element  $i$ ;

$r_i$  represents the average release rate of element  $i$  in mg/kg/week in the humidity cell tests;

$R_m$  indicates the pit wall reactive mass in kg; and

$Q$  represents either the rate of groundwater inflow into the pit or the rate of pit wall run-off in L/week.

The modified chemistry of the precipitation from these pit rim reactions was then used as the source term contribution to the pit. Separate source terms were developed for each of the material types exposed in the current pit walls (see Table 4-1).

Different HCT inputs were used for trace elements and major ions to represent the different geochemical processes that control their release. Soluble salts are important in the input of major elements to the existing lake and, as such, all weeks of humidity cell data are needed for a valid prediction. By contrast, the release of trace elements is predominantly associated with longer term weathering processes, possibly sulfide oxidation and as a result the initial HCT flush information does not contribute sufficiently. As such, an average of all weeks of humidity cell data were used for major ions (calcium, magnesium, sodium, potassium, aluminum, iron, manganese, chloride, sulfate, fluoride, bicarbonate) and an average of steady-state humidity cell data (i.e., minus the first 20 weeks of testing) were used for trace elements (silver, arsenic, boron, barium, cadmium, cobalt, chromium, copper, mercury, molybdenum, nickel, lead, antimony, selenium, uranium, vanadium and zinc).

The solutions used as inputs to the geochemical model are provided in Table 4-2.

**Table 4-2: Groundwater, Wall Rock and Precipitation Chemistry used as Input to the Existing Pit Model**

Parameter		Units	Precipitation chemistry	Groundwater chemistry	Wall rock chemistry							
					Mineralized				Weakly/non-mineralized			
					Biotite breccia oxide	Biotite breccia sulfide	Quartz Monzonite oxide	Quartz Monzonite sulfide	Biotite breccia oxide	Biotite breccia sulfide	Quartz Monzonite oxide	Quartz Monzonite sulfide
			<i>Gila Cliff Dwellings National Monument meteorological station</i>	<i>Average of wells GWQ96-22(A,B), GWQ96-23(A,B), GWQ11-24(B) and GWQ11-25(B).</i>	<i>Average of HCT SRK 0854</i>	<i>Average of HCTs 604767, 604787, 604811, 604854, 604862, 604867 and 605033</i>	<i>Average of HCT SRK 0867</i>	<i>Average of HCTs 604652, 604606, 604653, 604656 and 604669</i>	<i>Average of HCT SRK 0872</i>	<i>Average of HCTs 604811, 604854, 604862, 604867 and 605033</i>	<i>Average of HCT 604569</i>	<i>Average of HCTs 604673 and 605153</i>
pH	pH	s.u	4.93	6.91	5.22	7.86	6.9	7.95	6.51	7.91	7.85	5.74
HCO <sub>3</sub>	Bicarbonate	mg/L		316	0.47	45	9.27	38.2	6.4	54.9	22.6	12.3
Ag	Silver	mg/L		0.009	-	-	-	-	-	-	-	-
Al	Aluminium	mg/L		0.12	0.39	0.005	0.07	0.008	0.08	0.006	0.03	0.04
As	Arsenic	mg/L		0.0023	0.0011	0.00034	-	-	0.00095	0.00025	0.00025	-
B	Boron	mg/L		0.136	-	0.005	0.0047	0.0049	-	0.0049	0.005	0.005
Ba	Barium	mg/L		0.089	0.012	0.0091	0.0075	0.012	0.01	0.0062	0.0005	0.035
Ca	Calcium	mg/L	0.21	336	14.1	24.1	25.9	19.5	27.8	28	9.05	6.32
Cd	Cadmium	mg/L		0.001	0.0013	-	0.00005	-	0.00008	-	0.00005	0.00034
Co	Cobalt	mg/L		0.01	0.0009	-	0.0005	-	0.0005	-	-	-
Cr	Chromium	mg/L		0.0066	-	-	-	-	-	-	0.00025	-
Cu	Copper	mg/L		0.0037	18.2	0.0085	0.0056	-	0.0034	0.013	0.0025	0.38
F	Fluoride	mg/L		4.6	0.25	1.09	0.56	0.81	0.33	1.2	0.74	0.43
Fe	Iron	mg/L		1.48	0.7	0.001	0.1	0.001	0.1	0.001	0.001	0.004
Hg	Mercury	mg/L		0.000002	-	-	-	-	-	-	-	0.00002
K	Potassium	mg/L	0.03	4.39	1.42	3.75	1.08	3.84	0.48	4.43	2.5	1.84
Mg	Magnesium	mg/L	0.02	57.8	1.44	3.97	2.24	3.51	1.16	4	2.54	0.98
Mn	Manganese	mg/L		2.47	0.32	0.07	0.47	0.13	0.18	0.04	0.04	0.02
Mo	Molybdenum	mg/L		0.0119	-	0.0052	0.0051	0.0074	0.079	0.0056	0.0005	0.002
Na	Sodium	mg/L	0.08	115	0.61	2.41	0.93	3.46	0.45	2.6	3.23	1.69
Ni	Nickel	mg/L		0.0125	0.0005	-	0.0005	-	0.0005	-	0.0005	-
Pb	Lead	mg/L		0.0025	0.0034	-	-	0.00012	0.00012	-	0.00012	0.0016
Sb	Antimony	mg/L		0.0009	-	-	0.003	-	0.00051	-	-	-
Se	Selenium	mg/L		0.0022	0.00023	0.00031	0.00024	0.00032	0.00024	0.00035	0.00025	0.00025
U	Uranium	mg/L		0.0015	0.0013	0.0033	0.0005	0.0012	0.0013	0.0017	0.0005	0.0046
V	Vanadium	mg/L		0.0009	0.0005	0.001	0.0005	0.0005	0.0005	0.0015	0.0005	0.0005
Zn	Zinc	mg/L		0.08	0.088	0.0027	0.0016	0.0046	0.0013	0.0014	0.0023	0.015
SO <sub>4</sub>	Sulfate	mg/L	0.86	954	99.6	44.5	72.3	38.7	74.4	47.3	21.6	14.9
Cl	Chloride	mg/L	0.12	34	0.69	1.3	0.74	2.17	0.6	1.34	1.07	0.71

- Indicates parameter was uniformly below ARLs in the HCT effluent leachates and was excluded from the PHREEQC model input for the specified material type



## 4.6 Results

The results of the existing pit calculations are shown in Table 4-3. This shows predicated pit lake chemistry in 2014 (i.e., the final point in the simulated water balance). The predicted chemistry has been compared to average measured chemistry in the existing pit lake between 2010 and 2013 and also the range of chemistry observed during this time period. The PHREEQC model only predicts chemistry at a fixed point in time and does not account for seasonal or longer-term variations in chemistry that may occur. As such, comparison of predicted pit lake chemistry to the range of measured chemistry is likely a more reliable indicator of the accuracy of the model in predicting future chemical conditions.

The model results show good calibration for pH, bicarbonate, calcium, aluminum, cobalt, chromium, copper, mercury, manganese, sodium, nickel, selenium, uranium, zinc and TDS. Predicted concentrations of these constituents are within the range of chemistry measured in the existing pit lake between 2010 and 2013. This demonstrates that they can be predicted with a good degree of accuracy for the future pit lake. In comparison, a few constituents are either positively or negatively-biased in the pit lake calibration model.

Boron, potassium, molybdenum and antimony are slightly overestimated by the PHREEQC model. This likely relates to a combination of factors, including: evapoconcentration effects within the PHREEQC model and a lack of appropriate mineralogical controls in the thermodynamic code. This means the mechanisms that are responsible for removal of these constituents from solution in the existing pit lake (e.g., adsorption on clays or precipitation of mineralogical phases that are not included in the minteq database) are not accounted for in the geochemical model, resulting in concentrations of these constituents being artificially increased over time.

By contrast, concentrations of arsenic, barium, cadmium, fluoride and iron are slightly underestimated by the PHREEQC model. For iron, this underestimate likely relates to the fact that PHREEQC reports only truly dissolved phases. It is possible that iron in the existing pit lake may exist in the form of fine-grained colloids that pass through a 0.45 µm filter, which explains the high measured concentrations of iron in the existing pit lake. This has implications for arsenic concentrations due to the strong affinity of arsenic for Fe-oxyhydroxides (Bowell, 1994). The model predicts that arsenic concentrations will primarily be controlled by adsorption onto Fe-oxyhydroxides; however the calculations assume thermodynamic equilibrium and it may be that speciation of arsenic in the lake is more complex than predicted and adsorption may be limited as a result.

For fluoride and barium, the lower concentrations predicted by the model may relate to an over-estimate of fluorite and barite precipitation. Although both of these minerals have been observed around the existing pit lake at Copper Flat and are likely to form based on the predicted chemistry, the model may overestimate the mass of these minerals that will precipitate, resulting in lower predicted concentrations.

Despite these minor differences in predicted and measured concentrations for a small number of parameters, the existing pit lake model shows that the majority of parameters can be predicted with a good degree of accuracy for the future pit lake.

**Table 4-3: Existing Pit (Calibration) Model Results**

Parameter		Units	Average measured chemistry in existing pit lake (2010 - 2013)	Range of measured chemistry in existing pit lake (2010 - 2013)	PHREEQC predicted chemistry for existing pit lake
pH	pH	s.u.	7.30	6.0 – 7.9	7.94
pe	pe	s.u.	-	-	4.84
Alk	Alkalinity as CaCO <sub>3</sub>	mg/L	-	-	65.1
HCO <sub>3</sub>	Bicarbonate	mg/L	49.7	<20 – 123	37.8
Ag	Silver	mg/L	<0.005	<0.005	0.002
Al	Aluminium	mg/L	4.58	<0.02 – 82.6	0.02
As	Arsenic	mg/L	0.003	<0.001 – 0.0077	0.0001
B	Boron	mg/L	0.17	0.13 – 0.19	0.85
Ba	Barium	mg/L	0.012	<0.01 – 0.014	0.003
Ca	Calcium	mg/L	567	453 – 670	461
Cd	Cadmium	mg/L	0.055	0.038 – 0.064	0.007
Co	Cobalt	mg/L	0.29	0.049 – 0.49	0.06
Cr	Chromium	mg/L	<0.006	<0.006	0.0001
Cu	Copper	mg/L	2.21	<0.006 – 26.5	0.03
F	Fluoride	mg/L	18.4	15 – 29.8	4.74
Fe	Iron	mg/L	0.12	<0.02 – 1.3	0.0001
Hg	Mercury	mg/L	<0.0002	<0.0002	0.0002
K	Potassium	mg/L	33	24 – 49	397
Mg	Magnesium	mg/L	720	570 – 1120	524
Mn	Manganese	mg/L	41	28 - 48	38.7
Mo	Molybdenum	mg/L	0.02	<0.015 – 0.025	0.29
Na	Sodium	mg/L	871	604 – 1400	923
Ni	Nickel	mg/L	0.058	0.039 – 0.069	0.06
Pb	Lead	mg/L	0.011	<0.005 – 0.026	0.001
Sb	Antimony	mg/L	<0.001	<0.001	0.13
Se	Selenium	mg/L	0.027	0.013 – 0.059	0.03
U	Uranium	mg/L	0.12	0.11 – 0.12	0.10
V	Vanadium	mg/L	<0.05	<0.05	0.006
Zn	Zinc	mg/L	4.29	0.78 – 7.36	2.05
SO <sub>4</sub>	Sulfate	mg/L	6,128	5,200 – 8,690	5,304
Cl	Chloride	mg/L	451	340 – 714	224
TDS	Total Dissolved Solids	mg/L	9,188	7,770 – 14,800	7,918

## 5 Unreclaimed Pit Model with Natural Fill

### 5.1 Conceptual Model

The unreclaimed model assumes that dewatering will occur during mining operations and limited water will pond within the pit itself. At the end of open pit mining operations, dewatering will cease and a pit lake will ultimately form by natural refill as a result of inflow of groundwater into the pit, direct precipitation onto the pit lake, run-off from the pit walls and runoff from the open pit surface drainage area. Predictions of future pit lake chemistry for this scenario were made at selected time intervals (beginning when the pit lake starts to fill after mining and dewatering operations cease). Water quality predictions were made for the time periods of 0.5, 1, 2, 5, 10, 25, 50, 75, and 100 years after the start of pit lake formation. These predictions were based on mass load mixing of waters from different sources and allowing the resulting mix to establish thermodynamic equilibrium under imposed conditions by dissolving or precipitating specified solids, with attenuation of trace elements through sorption reactions.

A conceptual geochemical model was developed for the unreclaimed pit model from a review of background and site-specific data in addition to experience with similar projects. The conceptual model is provided in Figure 5-1 and the inputs to the model are discussed in Sections 5.2 to 5.5, below.

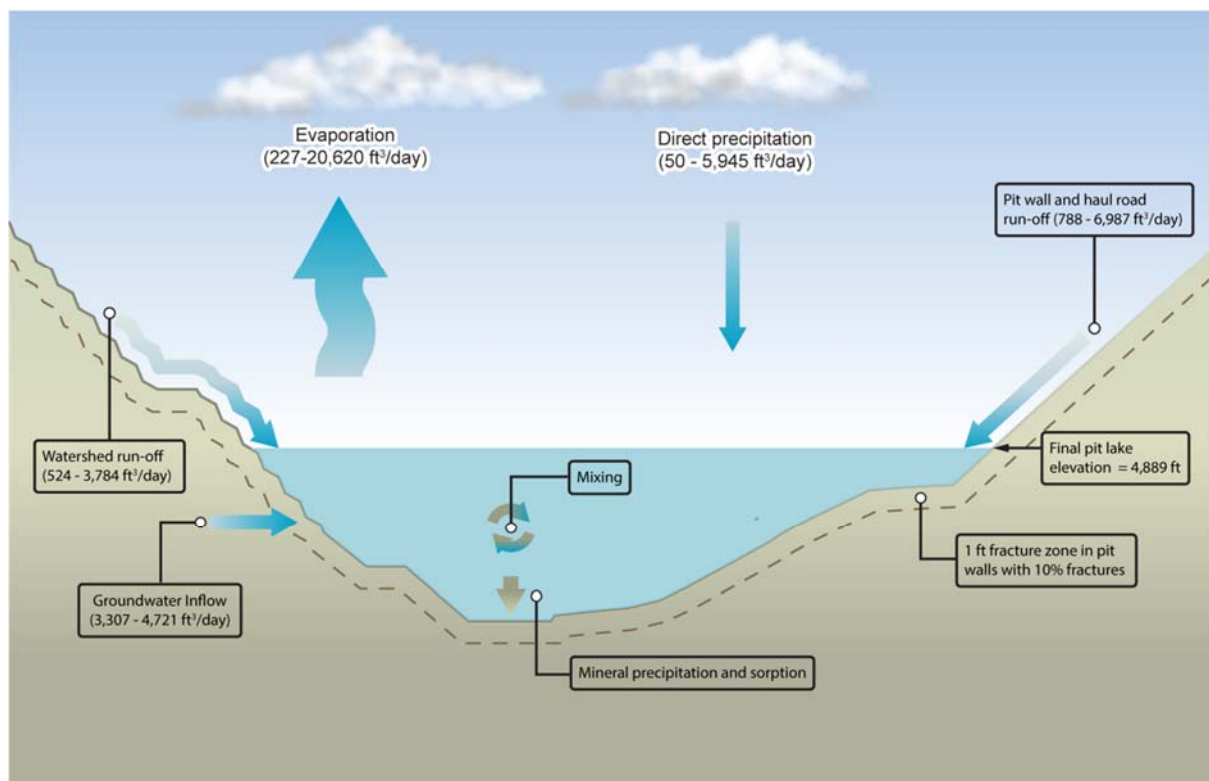


Figure 5-1: Conceptual Model for Unreclaimed Pit with Natural Fill

## 5.2 Pit Wall Surface Areas

The proportional surface areas of the main material types that will be exposed in the final walls of the unreclaimed pit have been calculated from the FS geologic block model and pit shell with expanded 4900 catch bench. The block model was used to calculate the three-dimensional surface area of each material type that will be exposed in the pit wall both above and below the water level as pit filling progresses. Three-dimensional surface areas were calculated for each of the modeled time steps (i.e., for 0.5, 1, 2, 5, 10, 25, 50, 75 and 100 years after the start of pit lake formation). Material types were delineated based on primary lithology, oxidation (redox) and mineralization (i.e., mineralized versus weakly/non-mineralized).

The three-dimensional surface areas of each material type in the unreclaimed pit at the end of mine life are provided in Table 5-1 and are illustrated in Figure 5-2. This demonstrates that unoxidized Quartz Monzonite will represent the dominant material type that will be exposed in the final walls of the unreclaimed pit.

**Table 5-1: Three-dimensional Surface Areas of Pit Wall Rock Material Types for Final Unreclaimed Pit**

Mineralization	Rock Type	Redox	Three-dimensional surface area	
			Square feet	%
Weakly/non-mineralized	Andesite	Oxide	4,150	0.05%
		Sulfide (non-ox.)	171,177	2.2%
	Biotite Breccia	Oxide	13,856	0.2%
		Sulfide (non-ox.)	340,496	4.4%
	Quartz Monzonite	Oxide	12,826	0.2%
		Sulfide (non-ox.)	2,823,022	36.3%
	Coarse Crystalline Porphyry	Oxide	8,874	0.1%
		Sulfide (non-ox.)	705,534	9.1%
Mineralized	Biotite Breccia	Sulfide (non-ox.)	813,861	10.5%
	Quartz Monzonite	Oxide	1,768	0.02%
		Sulfide (non-ox.)	2,543,813	32.7%
	Coarse Crystalline Porphyry	Oxide	77	0.001%
Sulfide (non-ox.)		335,045	4.3%	
<b>Total</b>			<b>7,774,501</b>	<b>100%</b>

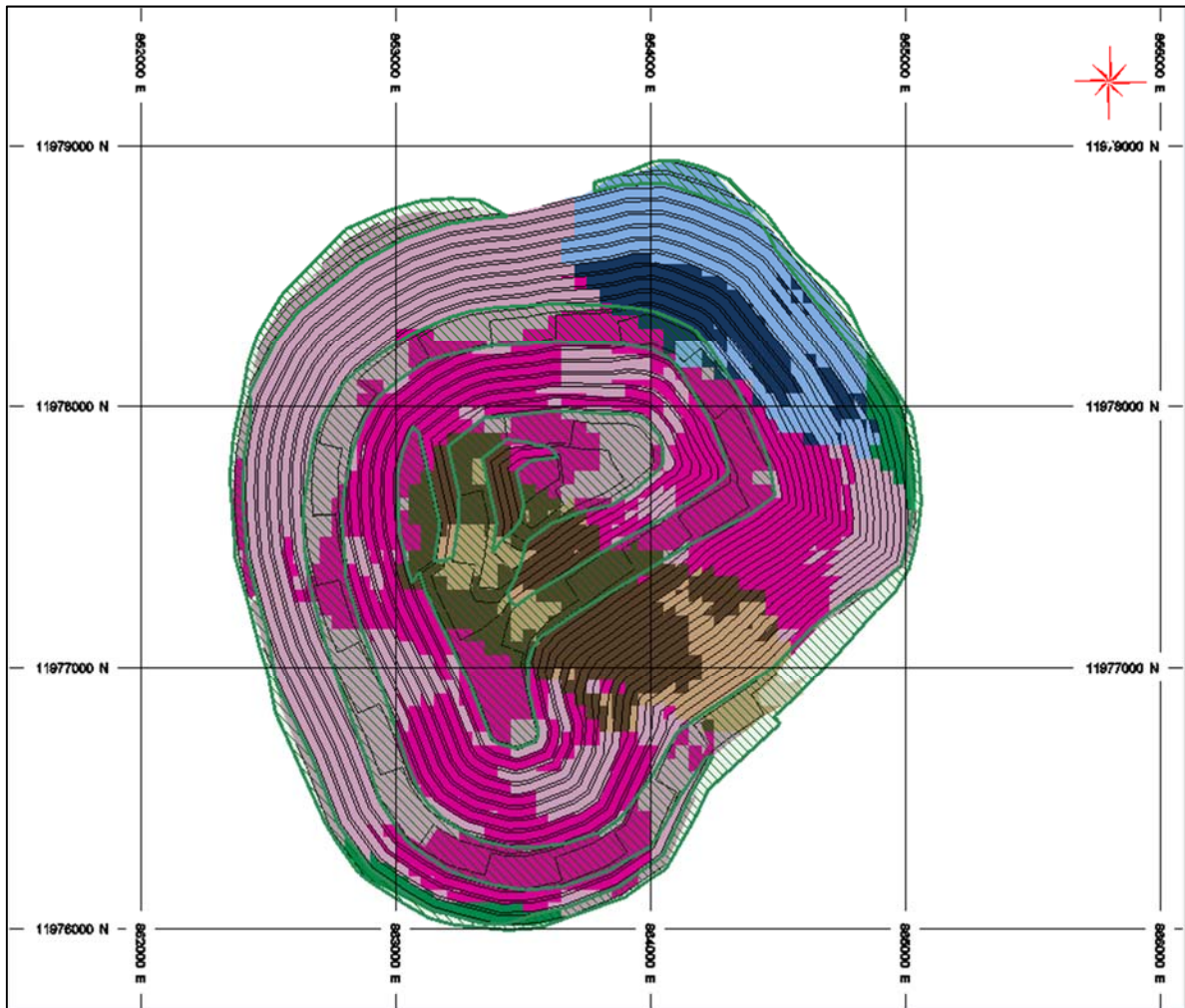


Figure 5-2: Exposed Material Types in Final Walls of Unreclaimed Pit

### 5.3 Calculation of Pit Wall Rock Available for Leaching

During the period of dewatering the pit walls will be exposed to oxygenated conditions and will weather to form secondary minerals, including soluble salts. As the pit wall resaturates during rebound of the groundwater table, soluble salts and other weathering products will dissolve into the ambient groundwater that drains into the pit. In addition, dissolution of these soluble salts by run-off waters in the unsaturated high wall of the pit may occur. In order that laboratory leach data can be used to determine the mass release of solutes under field leaching conditions, it was necessary to determine the total reactive mass ( $R_m$ ) of material available for leaching in the pit walls based on the exposed surface areas of each lithology in both the unsaturated high wall and in the submerged pit walls. The reactive mass will be dependent on the density of the pit wall rocks, the density of any fractures produced by blasting, and the depth to which this fracturing penetrates in the pit walls.

A number of studies have evaluated the density and thickness of pit wall fracturing caused by blasting (e.g., Carroll and Scott, 1966; Siskind and Fumanti, 1974; Kelsall et al., 1984; Molebatsi et al., 2009). A detailed summary of this research is presented in Appendix F. This demonstrates that the depth of pit wall fracturing is found to be variable between 1 and 16 feet.

An estimate of the reactive mass in the future pit high wall at Copper Flat was made based on the review of the published information on pit wall fracturing (Appendix F) and from site-specific information provided by NMCC. Future blasting practices at Copper Flat will include pre-split drilling and smooth wall blasting to protect final pit walls, which is considered best practice for geotechnical stability and will effectively reduce fracturing within the final pit walls. Kelsall et al. (1984) studied blasting effects in granite and basalt wall rock and found that blasting enhances permeability by approximately 10 times near the blast face. However, the extent of blast effects is generally limited to <1m (<3.3ft), and as little as 0.3m (1ft) when using low-charge blast methods. Given that the future blasting techniques at Copper Flat will include protective measures such as smooth wall blasting at the final pit wall and that the pit wall composition (i.e., quartz monzonite) will be similar to the granitic material studied in Kelsall et al. (1984), a 1 foot thickness of reactive rock in the pit walls has been assumed for the purpose of the future pit lake model. It is assumed that fracturing in this zone will average 10% (Siskind and Fumanti, 1974; Kelsall et al., 1984). This assumption (i.e., 10% fractures) is considered conservative because the rock comprising the proposed pit shell has low fracture permeability and the limited natural fractures are mineralized (quartz and calcite are common minerals in fractures).

In addition to the fracture zone described above, mineralogy work carried out by SRK on humidity cell tests for previous projects indicates particles generally show water infiltration and products of reactivity up to 0.04 feet into the individual rock fragments. Therefore an oxidized rind of 0.04 feet (0.012 m) thickness has also been assumed on the surface of the pit walls (Figure 5-3).

Using these assumptions for the fracture zone and oxidized rind, the reactive mass ( $R_m$ ) of each material type in the pit wall was calculated as:

$$R_m = (S \times F_{FZ} \times L_{FZ} \times D) + (S \times L_{OR} \times D)$$

Where:

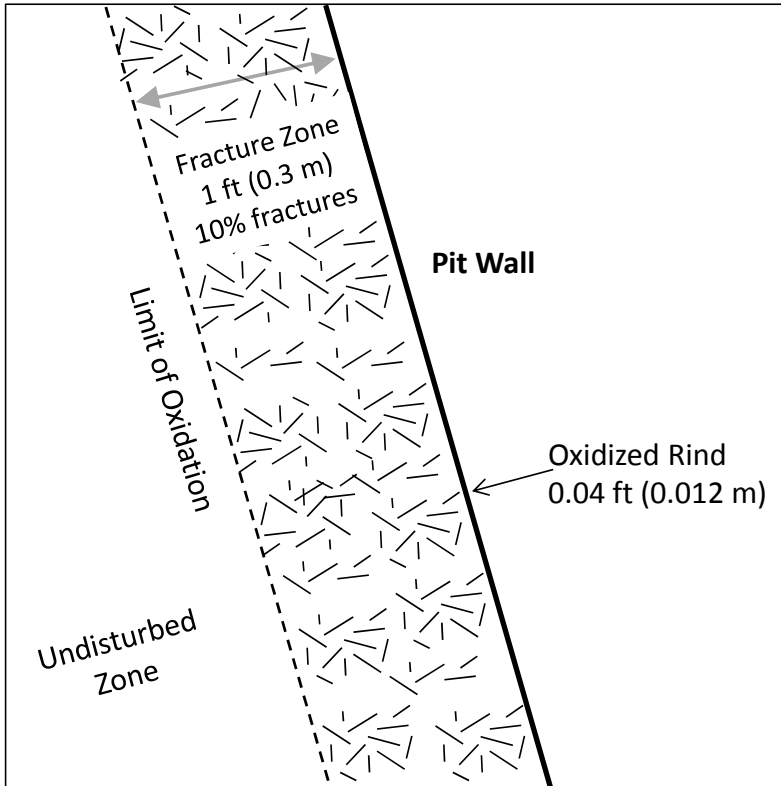
$S$  is the three-dimensional pit wall surface area of a given material type in square meters (defined by the geological block model; see Table 3-1);

$F_{FZ}$  is the fracture density in the fracture zone (10%);

$L_{FZ}$  is the thickness of the fracture zone in meters (0.3m);

$L_{OR}$  is the thickness of the oxidized rind in meters (0.012m);

$D$  is the rock density in kg/m<sup>3</sup> (2700 kg/m<sup>3</sup>, Young and Olhoeft, 1976).



**Figure 5-3: Future Pit Wall Conceptual Model**

## 5.4 Water Balance

A pit lake water balance for the unreclaimed pit model was developed by JSAI; details of the groundwater flow model are presented in JSAI (2014b). The post-mining pit water levels and water balance for this scenario were simulated assuming the 2017 MORP pit geometry with expanded 4900 catch bench and watershed shown in Figure 3-1. The model assumes that upon cessation of mining, pumping will cease in and around the pit, allowing the pit to naturally refill over a number of years.

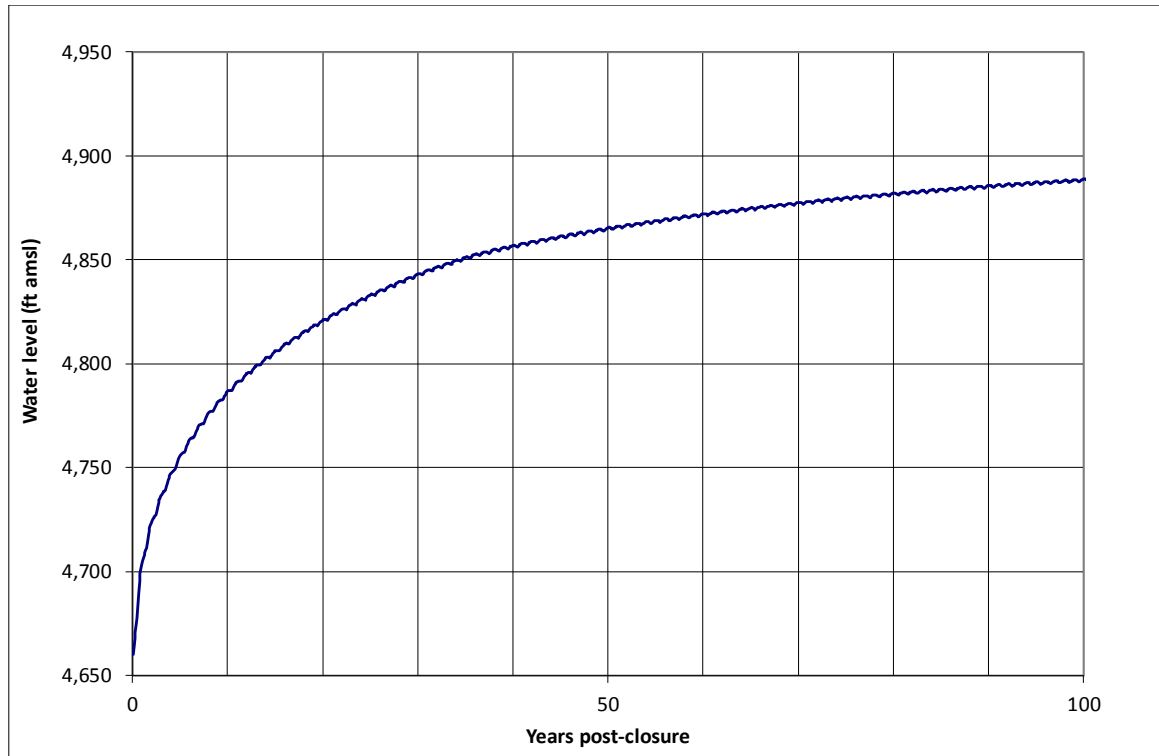
The water balance for the unreclaimed pit natural fill model is based on the following inputs/assumptions from JSAI (JSAI, 2014b; JSAI, 2015a; JSAI, 2017b):

- The primary solution inputs to the pit are assumed to be groundwater inflow, direct precipitation onto the high walls of the pit and run-off from the pit walls, haul road and receiving watershed;
- Evaporation will represent the dominant solution loss;
- The annual average precipitation rate is 12.5 inches per year; and
- The pit lake evaporation rate is 50 inches per year (JSAI, 2015a).

The JSAI water balance projects that the final pit lake elevation for the unreclaimed pit model will be 4,897 ft. The resulting lake will cover an area of approximately 20.7 acres with a depth of approximately 247 ft. The final pit water balance will be approximately 93 acre-feet per year, comprising 57 acre-feet of precipitation and run-off and 36 acre-feet per year of groundwater inflow.

The future pit will be a hydrologic sink, capturing groundwater flowing from all directions (INTERA, 2012; JSAI, 2017b). Surface water from within the footprint of the pit and runoff from the open pit surface drainage area will also be captured. Even with the surface water inflows, the pit will be a hydraulic sink with evaporation rates greatly exceeding precipitation and groundwater inflows on an annual basis (JSAI, 2017b). It is expected that the water levels of the lake will fluctuate seasonally by a few feet depending on precipitation and evaporation rates; rising during periods of lower evaporation (winter months) and decreasing during summer months.

The pit lake filling curve for the unreclaimed pit model is shown in Figure 4-4 and the various inputs/outputs to the pit are shown in Figure 5-5.



**Figure 5-4: Pit Lake Elevation Curve for Unreclaimed Pit Model (source: JSAI)**



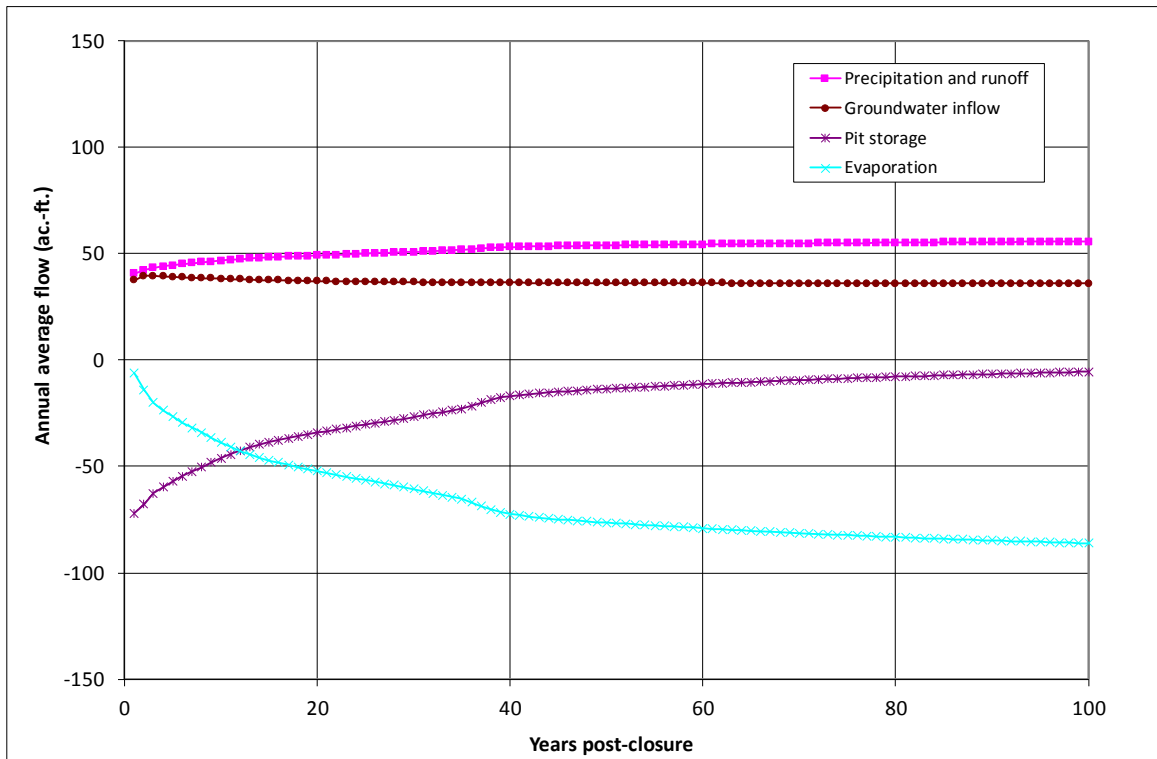


Figure 5-5: Pit Lake Flux for Unreclaimed Pit Model (source: JSAI)

## 5.5 Solution Inputs

### 5.5.1 Precipitation Chemistry

As with the existing pit model, the primary wall rock lixiviant for the future pit high walls is assumed to be precipitation. Representative precipitation chemistry data were obtained from monthly monitoring carried out between 1985 and 2011 at the Gila Cliff Dwellings National Monument meteorological station, Catron County, New Mexico (NADP, 2012) (Figure 4-6, Table 5-2).

### 5.5.2 Groundwater Chemistry

Representative groundwater chemistry data for the future pit lake model were obtained from the historical data compiled by JSAI and NMCC. Based on the current mine plan, a large proportion of the quartz monzonite is removed by mining and the remaining quartz monzonite is dewatered. Groundwater reporting to the future pit is therefore likely to be representative of the andesite rock. Based on this assumption, data from wells GWQ96-22(A), GWQ96-22(B), GWQ96-23(A) and GWQ96-23(B) were used as input to the future pit lake geochemical model.

Further information on how the groundwater chemistry data were derived is provided in the JSAI technical memorandum in Appendix D. The groundwater chemistry used as input to the unreclaimed pit model is presented in Table 5-2.

### 5.5.3 Wall Rock Chemistry

As with the existing pit model, source term solutions for the future pit lake were developed from the results of site-specific HCT testing conducted as part of the SRK (2012) geochemical characterization program and scaled to field conditions. The HCT testwork results were used to develop separate source terms for each material type that will be exposed in the final pit wall (see Table 5-1). The method used to scale the laboratory HCT data to field conditions was identical to that described in Section 4.5.3 and was based on site-specific information relating to the pit water balance, geological model, pit wall fracturing and wall rock density.

As with the existing pit lake model, different HCT inputs were used for trace elements and major ions to represent the different geochemical processes that control their release. An average of all weeks of humidity cell data were used for major ions (calcium, magnesium, sodium, potassium, aluminum, iron, manganese, chloride, sulfate, fluoride, bicarbonate) and an average of steady-state humidity cell data (i.e., minus the first 20 weeks of testing) were used for trace elements (silver, arsenic, boron, barium, cadmium, cobalt, chromium, copper, mercury, molybdenum, nickel, lead, antimony, selenium, uranium, vanadium and zinc).

The solutions used as inputs to the geochemical model are provided in Table 5-2.

**Table 5-2: Groundwater, Wall Rock, Haul Road and Precipitation Chemistry used as Input to the Unreclaimed Pit Model**

Parameter	Units	Precipitation chemistry	Groundwater chemistry	Haul road and watershed run-off chemistry	Wall Rock Chemistry													
					Mineralized					Weakly/non-mineralized								
					Biotite breccia sulfide	Quartz Monzonite oxide	Quartz Monzonite sulfide	Coarse Crystalline Porphyry oxide	Coarse Crystalline Porphyry sulfide	Andesite oxide	Andesite sulfide	Biotite breccia oxide	Biotite breccia sulfide	Quartz Monzonite oxide	Quartz Monzonite sulfide	Coarse Crystalline Porphyry oxide	Coarse Crystalline Porphyry sulfide	
		<i>Gila Cliff Dwellings National Monument meteorological station</i>	<i>Average of wells GWQ96-22(A,B) and GWQ96-23(A,B)</i>	<i>Average of SWQ-1</i>	<i>Average of HCTs 604767, 604787, 604811, 604854, 604862, 604867 and 605033</i>	<i>Average of HCT SRK 0867</i>	<i>Average of HCTs 604652, 604606, 604653, 604656 and 604669</i>	<i>Average of HCT CF-11-02 (0-27)</i>	<i>Average of HCT CF-11-02 (367-408)</i>	<i>Average of HCTs SRK 0864 and SRK 0866</i>	<i>Average of HCTs SRK 0864 and SRK 0866</i>	<i>Average of HCTs SRK 0872 and SRK 0854</i>	<i>Average of HCTs 604811, 604854, 604862, 604867 and 605033</i>	<i>Average of HCTs SRK 0858 604569</i>	<i>Average of HCTs 604673 and 605153</i>	<i>Average of HCT CF-11-02 (0-27)</i>	<i>Average of HCT CF-11-02 (367-408)</i>	
pH	pH	s.u	4.93	7.85	8.3	7.86	6.90	7.95	7.92	7.74	7.32	7.32	5.50	7.91	2.99	5.74	7.92	7.74
HCO <sub>3</sub>	Bicarbonate	mg/L		408	430	45.0	9.27	38.2	30.1	19.9	10.6	10.6	3.44	54.9	N/A	12.2	30.1	19.9
Ag	Silver	mg/L		0.009		-	-	-	-	-	-	-	-	-	-	-	-	-
Al	Aluminum	mg/L		0.029		0.0046	0.070	0.0078	0.019	0.050	0.0090	0.0090	0.237	0.0059	2.96	0.037	0.019	0.050
As	Arsenic	mg/L		0.0023		0.00034	-	-	-	-	-	-	0.0010	0.00025	0.00036	-	-	-
B	Boron	mg/L		0.136	0.02	0.0050	0.0047	0.0049	0.0049	0.0048	-	-	-	0.0049	0.018	0.0050	0.0049	0.0048
Ba	Barium	mg/L		0.089		0.0091	0.0075	0.012	0.00049	0.0028	0.0033	0.0033	0.011	0.0062	0.0021	0.035	0.00049	0.0028
Ca	Calcium	mg/L	0.21	85.8	109	24.1	25.9	19.5	9.95	7.36	8.36	8.36	20.9	28.0	9.59	6.32	9.95	7.36
Cd	Cadmium	mg/L		0.0008		-	4.72E-05	-	-	-	-	-	0.00068	-	0.0014	0.00034	-	-
Co	Cobalt	mg/L		0.008		-	0.00047	-	-	-	-	-	0.00070	-	0.015	-	-	-
Cr	Chromium	mg/L		0.0066		-	-	-	-	-	-	-	-	0.0056	-	-	-	-
Cu	Copper	mg/L		0.0061		0.0085	0.0056	-	-	0.0049	-	-	9.11	0.013	2.41	0.384	-	0.0049
F	Fluoride	mg/L		2.1	0.3	1.09	0.558	0.807	0.820	0.548	0.425	0.425	0.289	1.20	1.98	0.432	0.820	0.548
Fe	Iron	mg/L		1.48		0.00069	0.099	0.00087	0.0025	0.0022	0.0014	0.0014	0.400	0.00074	6.75	0.0039	0.0025	0.0022
Hg	Mercury	mg/L		0.000002		-	-	4.91E-06	9.97E-06	4.83E-06	-	-	-	-	-	1.62E-05	9.97E-06	4.83E-06
K	Potassium	mg/L	0.03	2.96	1.80	3.75	1.08	3.84	2.18	1.70	0.974	0.974	0.950	4.43	1.66	1.84	2.18	1.70
Mg	Magnesium	mg/L	0.02	19.3	36.0	3.97	2.24	3.51	1.74	0.570	1.27	1.27	1.30	4.00	1.64	0.978	1.74	0.570
Mn	Manganese	mg/L		0.66		0.072	0.468	0.130	0.019	0.0094	0.0095	0.0095	0.248	0.043	0.125	0.018	0.019	0.0094
Mo	Molybdenum	mg/L		0.012		0.0052	0.0051	0.0074	0.00049	0.00048	0.00046	0.00046	0.040	0.0056	0.0018	0.0020	0.00049	0.00048
Na	Sodium	mg/L	0.08	119	107	2.41	0.932	3.46	2.31	2.04	1.71	1.71	0.530	2.60	1.98	1.69	2.31	2.04
Ni	Nickel	mg/L		0.0125		-	0.00047	-	-	-	-	-	0.00047	-	0.0018	-	-	-
Pb	Lead	mg/L		0.0025		-	-	-	0.00012	0.00012	0.00012	-	0.0018	-	0.0019	0.0016	0.00012	0.00012
Sb	Antimony	mg/L		0.0009		0.00012	0.0030	0.00012	-	-	-	-	0.00040	0.00012	-	-	-	-
Se	Selenium	mg/L		0.0015		0.00031	0.00024	0.00032	0.00024	0.00024	0.00023	0.00023	0.00024	0.00035	0.00023	0.00025	0.00024	0.00024
U	Uranium	mg/L		0.0015		0.0033	0.00047	0.0012	0.0024	0.0024	-	-	0.0013	0.0017	0.0051	0.0046	0.0024	0.0024
V	Vanadium	mg/L		0.0009		0.0010	0.00047	0.00049	0.00049	-	0.00046	0.00046	0.00047	0.0015	0.0018	0.00050	0.00049	-
Zn	Zinc	mg/L		0.03		0.0027	0.0016	0.0046	-	-	-	-	0.045	0.0014	0.017	0.015	-	-
SO <sub>4</sub>	Sulfate	mg/L	0.86	84	261	44.5	72.3	38.7	12.1	7.66	20.3	20.3	87.0	47.3	89.1	14.9	12.1	7.66
Cl	Chloride	mg/L	0.12	49	30	1.30	0.739	2.17	0.999	1.37	0.708	0.708	0.647	1.34	1.26	0.711	0.999	1.37

- Indicates parameter was uniformly below ARLs in the HCT effluent leachates and was excluded from the PHREEQC model input for the specified material type

## 5.6 Potential for Future Pit Lake Stratification

The existing Copper Flat pit lake contained approximately 70 acre feet of water in 2014 (NMCC estimate). The water surface measures 5.2 acres with an average diameter of 537 feet (Figure 8-8 in INTERRA, 2012). The average depth is approximately 13 feet deep and the maximum depth is 35 feet (INTERRA, 2012), which results in a relative depth (RD) of 7%. Samples taken from various depths of the existing pit lake demonstrate that the pit lake is homogeneous and no stratification exists (SRK, 1996, INTERA, 2012, Aquatic Consultants Inc., 2014). Baseline data from the existing pit water body provides evidence that a thermocline develops in the summer and mixing occurs in the winter (INTERA, 2012). A chemocline does not appear to develop, and the water body remains oxygenated (DO = 6 to 9 mg/L) throughout the full water column year-round with similar chemistry throughout the lake (see JSAI, 2014c, Appendix F). Based on elevation and latitude, the Copper Flat open pit water body is classified as a warm monomitic type lake (Wetzel, 2001). A warm monomitic lake mixes freely once a year in the winter assuming the temperature is above 4°C. However, wind effects and water body geometry can have an effect on the magnitude and frequency of mixing (Castendyk, 2009).

Mine pit lakes can develop vertical density stratification that may be seasonal or permanent. The density of water is a function of both its temperature and its salinity or total dissolved solids (TDS) content. Freshwater is most dense at a temperature of about 4°C. At a given temperature, water density increases with increasing TDS. As TDS increases, the temperature of the maximum density of water also decreases (Atkins et al., 1997; Parshley and Bowell, 2003).

Long-term (multi-year) or permanent density stratification can occur if a lake has a significant vertical variation in TDS due to large differences in the TDS of various source waters to the lake and/or to processes in the lake that increase the TDS. This in turn affects the density of the deeper water. For example, if a lake contains enough organic matter to deplete oxygen in the hypolimnion, then during the summer, ferric hydroxide that precipitates at the surface will sink, become reduced, and dissolve in the basal anoxic water, raising the TDS content and the density of the bottom water.

Water in the hypolimnion will generally become anoxic and will continuously dissolve any ferric hydroxide precipitates falling into it from above. This process further increases the TDS of the hypolimnion and strengthens the density gradient between it and the overlying layer, perpetuating the stratification. Sulfidization in the hypolimnion will lead to natural attenuation of metals and metalloids as well as sulfur. Few studies reporting site-specific limnological data have been published to date (Atkins et al., 1997; Parshley and Bowell, 2003). For Copper Flat, the presence of solute material that will modify pit lake chemistry (i.e., sulfide minerals and gypsum) will likely prevent permanent chemical stratification or layering of the lake. This was validated in the 1990s from depth sampling of the pit lake at Copper Flat (SRK, 1996), and in 2010 and 2011 from baseline data collection (INTERA, 2012). The results from this study demonstrated that the current pit lake is homogeneous and no stratification exists. Temperature and dissolved oxygen profiles for the existing pit lake (INTERA, 2012, Aquatic Consultants Inc., 2014) show the pit water is not significantly stratified. The water stays well oxygenated for the entire depth for each season (6 to 8 mg/L). Thermal stratification requires a 1°C change in temperature per meter (Wetzel, 2001), which can occur in the summer months as the upper water column heats up and the lower water column remains cool, and well oxygenated.

When established, the future Copper Flat pit lake will contain approximately 2,300 acre feet of water. The water surface is projected to measure 22 acres with an average diameter of 1,105 feet. The average depth will be approximately 105 feet and the maximum depth will be 247 feet, which results in a relative depth (RD) of 22% (JSAI Pit Water Balance, 2017).

The 23% RD for the future Copper Flat pit lake is greater than the average value of 2% for natural lakes and suggests the lake may stratify. Such stratification may result in oxidizing conditions in the upper portions of the lake and more chemically reducing (oxygen-deprived) conditions at depth. However, this stratification is likely to be temporary and influenced by seasonal changes. A prerequisite for permanent stratification is that precipitation plus runoff is greater than evaporation during the summer months when the water body is potentially undergoing temporary thermal stratification (Jewell, 2009). This is not the case at Copper Flat, where annual evaporation from the pit lake (100 acre-feet per year) will greatly exceed precipitation plus run-off (63 acre-feet per year). As such, permanent stratification is unlikely for the current and future Copper Flat pit lake. Consequently, in keeping with many pit lakes in arid regions there is a lower potential for stratification than a single relative depth metric would imply (Jewell, 2009).

Jewell (2009) evaluated six permanently stratified and eight seasonally stratified open pit lakes, and concludes that permanently stratified lakes have vertical density contrast greater than  $0.0005 \text{ g/cm}^3$  and a Wedderburn number greater than 1. The Wedderburn number considers thermocline depth, maximum lake length, water density, and wind speed. Jewell (2009) failed to note that most permanently-stratified open pit lakes receive AWS inputs and have resulting acidic water at the surface. A summary table of existing open pit water bodies and their characteristics is presented in Table 5-3.

The future Copper Flat open pit lake is expected to be well mixed, oxygenated, and not acidic, although seasonal stratification may occur. Relative depth does not appear to govern the conditions for creating a permanently stratified open pit water body; however acidic water and higher latitude are key conditions for creating permanent stratification. In addition, another related control is the total dissolved solids or salinity which will also exert control over the density or buoyancy of the mine pit lake. At Copper Flat, direct surface water inputs to the existing lake over time are unlikely to be significant and therefore the potential for turnover is less.

Stratification within the pit lake has implications for redox conditions, mineral solubility and sorption reactions. The pit lake model results presented herein assume the pit lake will be fully mixed. A number of studies on deep mine pit lakes, including Summer Camp Pit in Nevada (Parshley and Howell, 2003) and unpublished reports on Lone Tree Mines, Yerrington mine and the Robinson Mining District, also in Nevada, have demonstrated the tendency for incomplete seasonal overturn.

Based on observations of the current Copper Flat pit lake, the development of a metal-rich brine in the hypolimnion of the future pit lake is unlikely. The conditions for this are summarized in Castendyk (2009). Rather, the future pit lake is expected to be mixed and well oxygenated because: (i) the existing and future pit lake can be classified as monomictic with frequent or continuous periods of circulation with no ice cover in the winter; and (ii) the existing and future pit lake can also be characterized as oligotrophic, i.e., having little to no nutrient input and organic production, with dissolved oxygen content regulated largely by physical processes.

While stratification of an open pit water body has implications for water chemistry at depth, particularly in terms of redox changes, the near surface waters of the future Copper Flat pit lake are expected to remain oxidizing. These near surface waters are considered the most critical from a perspective of potential ecological risks associated with the lake, reduced water quality that may develop at depth is less important since the proposed Copper Flat pit will remain a terminal sink post closure.

**Table 5-3: Summary of open pit water bodies and stratification characteristics (JSAI, 2014c)**

Open pit	Location	Effective length (ft)	Maximum depth (ft)	Relative depth (%)	Thermocline depth (ft)	Acidic
<b>Permanently stratified</b>						
Brenda	British Columbia	2,296	492	21	39	No
Spenceville	California	253	50	20	13	Yes
Berkeley	Montana	5,900	426	7	23	Yes
<b>Seasonally stratified and well mixed</b>						
Humbolt	Nevada	944	137	15	8	No
Blackhawk	Utah	492	na	na	33	No
Blowout	Utah	656	230	35	39	No
Colosseum	California	482	157	33	na	No
Cunningham	New Mexico	407	90	22	20	No
Copper Flat (existing)**	New Mexico	537	35	7	20	No*
Copper Flat (proposed)***	New Mexico	1105	247	22	TBD	No
Yerington	Nevada	5,412	400	13	49	No

\* Predominantly circum-neutral with the development of occasional temporary acidity

\*\* Updated from JSAI (2014c) to reflect Baseline Data Report (INTERA, 2012)

\*\*\* Updated from JSAI (2014c) to reflect current pit water balance and mine plan

TBD – to be determined

## 5.7 Results

The predicted pit lake chemistry for the unreclaimed pit model is summarized in Table 5-4 and illustrated in Figure 5-6 to Figure 5-19 for selected parameters. These show predicted pit lake chemistry at each of the modeled time steps (i.e., 0.5, 1, 2, 5, 10, 25, 50, 75 and 100 years post-closure). In each case, the predicted pit lake chemistry is compared to the chemistry measured in the existing pit lake between 1989 and 2017. The full PHREEQC output file is provided in Appendix I, which shows precipitating and dissolving mineral species at each time step as part of the mass transfer calculations.

Pit lake waters for the unreclaimed pit are predicted to be moderately alkaline (pH 7.9– 8.2) with a magnesium plus sulfate (Mg + SO<sub>4</sub>) major ion signature. During the early stages of pit infilling (i.e., the first six months post-closure), the prediction is that an early flush will occur in boron, lead, mercury, manganese, molybdenum, nickel, selenium, vanadium, zinc and sulfate. This initial flush occurs due to dissolution of soluble sulfate salts that will have developed on the pit walls during the life of mine. This initial flush is only observed for the natural fill model, but the effects are dissipated in the rapid fill model and no initial flush is observed.

Inflowing groundwater and direct precipitation on the pit lake surface will then provide some dilution and the effects of this initial flush will be dissipated. Following this initial flush, pit lake waters are predicted to evolve over time, with increasing concentrations of chloride, sulfate, TDS and trace elements owing to the effects of evapoconcentration. This is similar to the trends observed in the existing pit lake, where elemental concentrations (particularly boron, cadmium, fluoride, magnesium, manganese, sodium and sulfate) have increased over time. The macrochemistry (Ng-Na-SO<sub>4</sub>) changes are reflected in the Piper plot in Figure 5-19, which shows a progressive change in pit lake major ion chemistry post-closure, with waters becoming increasingly dominated by sulfate and magnesium over time. However, pH remains moderately alkaline throughout pit infilling.

Pit lake chemistry is likely to be dominated by groundwater chemistry plus evapoconcentration effects. Over time, the groundwater contribution will decrease slightly as the pit lake is established. Both adsorption and secondary mineral precipitation are likely to be the major controls on trace element chemistry.

Pit lake waters for the unreclaimed pit are predicted to be 'near-neutral, low-metal' waters for years zero (i.e., end of mine life) to year 50, based on pH values between 7.9 and 8.2 and total Ficklin metal concentrations<sup>1</sup> less than 1 mg/L (Figure 5-18). The effects of evapoconcentration are predicted to result in increasing metal concentrations, with pit lake waters being classed as 'near-neutral, high metal' from year 75 onwards (Figure 5-18).

A comparison of predicted pit lake chemistry to chemistry measured in the existing pit lake between 1989 and 2017 demonstrates that concentrations of the majority of constituents are either comparable to or less than existing concentrations. In particular, predicted concentrations of arsenic, cadmium, copper, cobalt, chromium, fluoride, lead, manganese, nickel, zinc and sulfate in the future unmitigated pit are lower than those observed in the existing pit lake at Copper Flat. This relates to a number of factors, including:

- The future pit walls will be prepared using pre-split drilling and smooth wall blasting, which will reduce the depth of fracturing and oxidation, and consequently reduce solute loading to the pit lake;
- The future pit walls will contain less mineralized material than the existing Copper Flat pit, which will also reduce solute loading to the pit lake;
- The future pit walls will contain less transitional material than the existing Copper Flat pit, that is the source of the AWS events; and
- The dominant groundwater flow into the future pit will originate from the Andesite, which is typically characterized by lower constituent concentrations than the Quartz Monzonite groundwater (JSAI, 2017a).

The only constituents that are predicted to be higher in the future pit lake compared to the existing pit lake are boron, molybdenum, potassium and antimony. From the calibration model (Section 3.10) these constituents are known to be over-predicted by PHREEQC, and therefore the predicted concentrations of boron, molybdenum, potassium and antimony presented in Table 5-4 are likely to be an overestimate.

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<sup>1</sup> Ficklin metals are the base metals copper, cobalt, cadmium, lead, nickel and zinc (Ficklin et al., 1992)

**Table 5-4: Unreclaimed Pit Model Results**

Parameter		Units	Measured Chemistry in Existing Pit (1989 - 2017)			Predicted Future Chemistry (Years Post-Closure)								
			Average	Minimum	Maximum	0.5	1	2	5	10	25	50	75	100
pH	pH	s.u.	6.5	3.6	8.3	8.2	8.1	8.1	8.0	8.0	8.0	7.9	7.9	7.9
HCO <sub>3</sub>	Bicarbonate	mg/L	40.4	<3	122	54.8	45.4	42.6	40.6	39.3	37.3	35.2	33.8	34.6
Al	Aluminium	mg/L	10.4	<0.02	82.6	0.06	0.10	0.10	0.11	0.12	0.13	0.16	0.18	0.16
As	Arsenic	mg/L	0.004	<0.001	0.006	1.28E-05	1.07E-05	1.27E-05	1.58E-05	1.96E-05	3.07E-05	4.46E-05	5.86E-05	7.27E-05
B	Boron	mg/L	0.14	<0.1	0.2	0.44	0.30	0.31	0.34	0.38	0.49	0.67	0.85	1.04
Ca	Calcium	mg/L	550	455	684	99.8	127	150	177	202	262	360	460	489
Cd	Cadmium	mg/L	0.05	<0.005	0.1	0.0012	0.0014	0.0015	0.0017	0.0019	0.0025	0.0033	0.003	0.003
Co	Cobalt	mg/L	0.29	<0.05	0.49	0.008	0.005	0.006	0.006	0.007	0.009	0.01	0.02	0.02
Cr	Chromium	mg/L	0.03	<0.006	0.1	4.68E-05	5.14E-05	5.43E-05	5.73E-05	5.98E-05	6.53E-05	7.32E-05	8.05E-05	8.24E-05
Cu	Copper	mg/L	4.44	0.001	26.5	0.012	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
F	Fluoride	mg/L	19.2	4.8	34	3.30	3.02	3.34	3.83	4.25	4.11	4.00	3.95	4.16
Fe	Iron	mg/L	0.2	<0.02	1.3	4.64E-05	4.88E-05	5.03E-05	5.18E-05	5.30E-05	5.55E-05	5.88E-05	6.17E-05	6.20E-05
Hg	Mercury	mg/L	0.0005	<0.0002	0.001	0.0006	0.0004	0.0004	0.0004	0.0005	0.0006	0.0008	0.0011	0.0013
K	Potassium	mg/L	32.1	11.0	60.6	192	131	135	148	166	212	290	372	453
Mg	Magnesium	mg/L	698	43	1,120	171	121	125	136	152	194	266	341	416
Mn	Manganese	mg/L	34.8	0.02	59.0	4.66	3.19	3.30	3.62	4.04	5.15	7.04	9.02	11.00
Mo	Molybdenum	mg/L	0.04	0.015	0.1	0.26	0.18	0.19	0.20	0.23	0.28	0.36	0.45	0.53
Na	Sodium	mg/L	888	165	1,400	278	202	210	230	257	326	445	570	694
Ni	Nickel	mg/L	0.06	0.039	0.1	0.009	0.007	0.007	0.008	0.009	0.011	0.015	0.019	0.022
Pb	Lead	mg/L	0.02	<0.005	0.1	0.0005	0.0005	0.0007	0.0008	0.0008	0.0006	0.001	0.0005	0.0005
Sb	Antimony	mg/L	<0.001*			0.005	0.003	0.003	0.004	0.004	0.005	0.007	0.009	0.011
Se	Selenium	mg/L	0.028	<0.001	0.25	0.015	0.011	0.011	0.012	0.013	0.016	0.022	0.028	0.034
U	Uranium	mg/L	0.11	0.11	0.12	0.073	0.051	0.049	0.05	0.06	0.08	0.11	0.15	0.18
V	Vanadium	mg/L	0.1	<0.05	0.25	0.0004	0.0004	0.0003	0.0003	0.0004	0.0005	0.0007	0.0009	0.001
Zn	Zinc	mg/L	5.4	0.01	9	0.52	0.36	0.37	0.40	0.45	0.58	0.79	1.01	1.23
SO <sub>4</sub>	Sulfate	mg/L	4,803	1,566	8,690	1,505	1,196	1,284	1,441	1,626	2,096	2,887	3,708	4,353
Cl	Chloride	mg/L	332	47.3	730	135	95.6	99.1	109	121	154	210	269	328
TDS	Total Dissolved Solids	mg/L	7,538	2,711	14,800	2,446	1,926	2,053	2,291	2,573	3,293	4,507	5,770	6,786

\* Indicates parameter was uniformly below analytical detection limits in pit lake water over monitoring period, but detection limit was variable. Concentration shown in table represents lower limit of analytical detection.



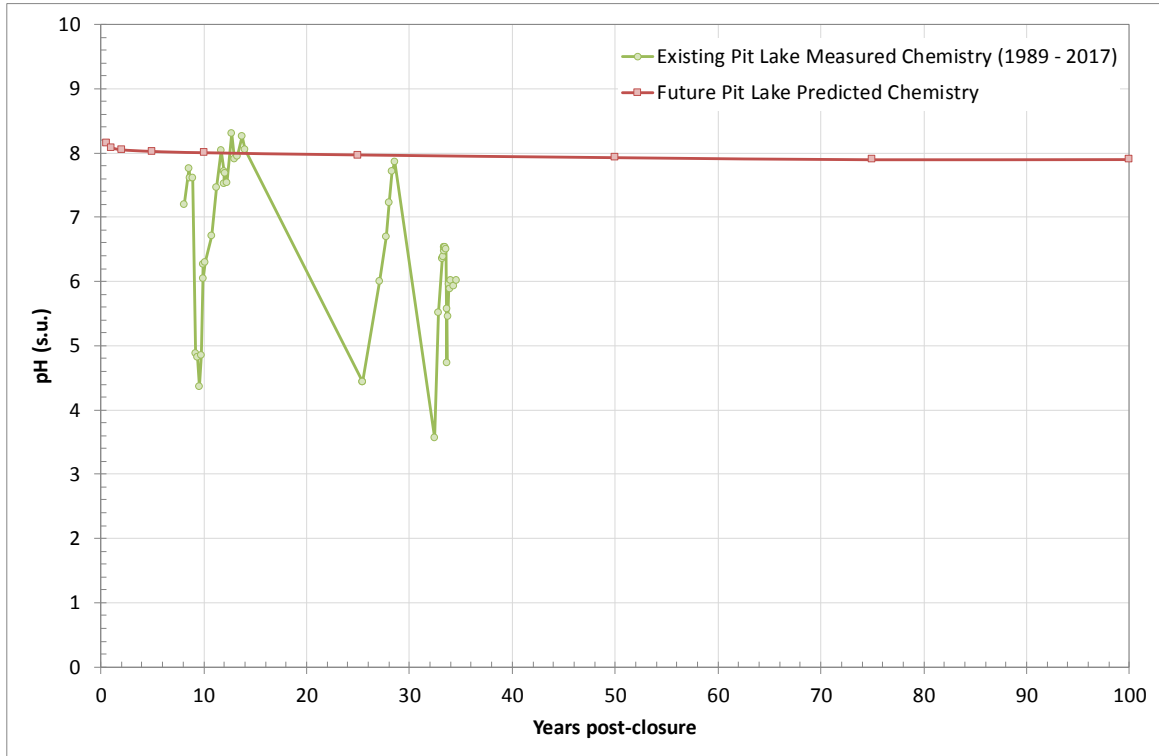


Figure 5-6: Time-series Plot of Predicted pH for the Unreclaimed Pit Model

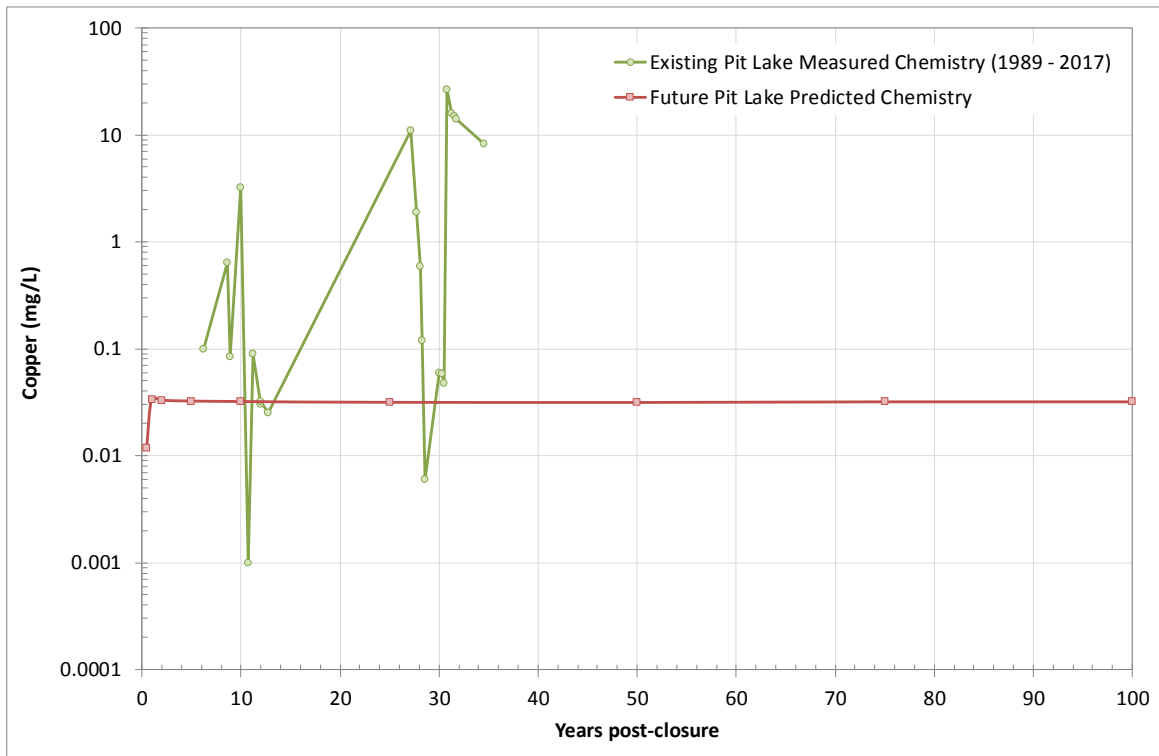
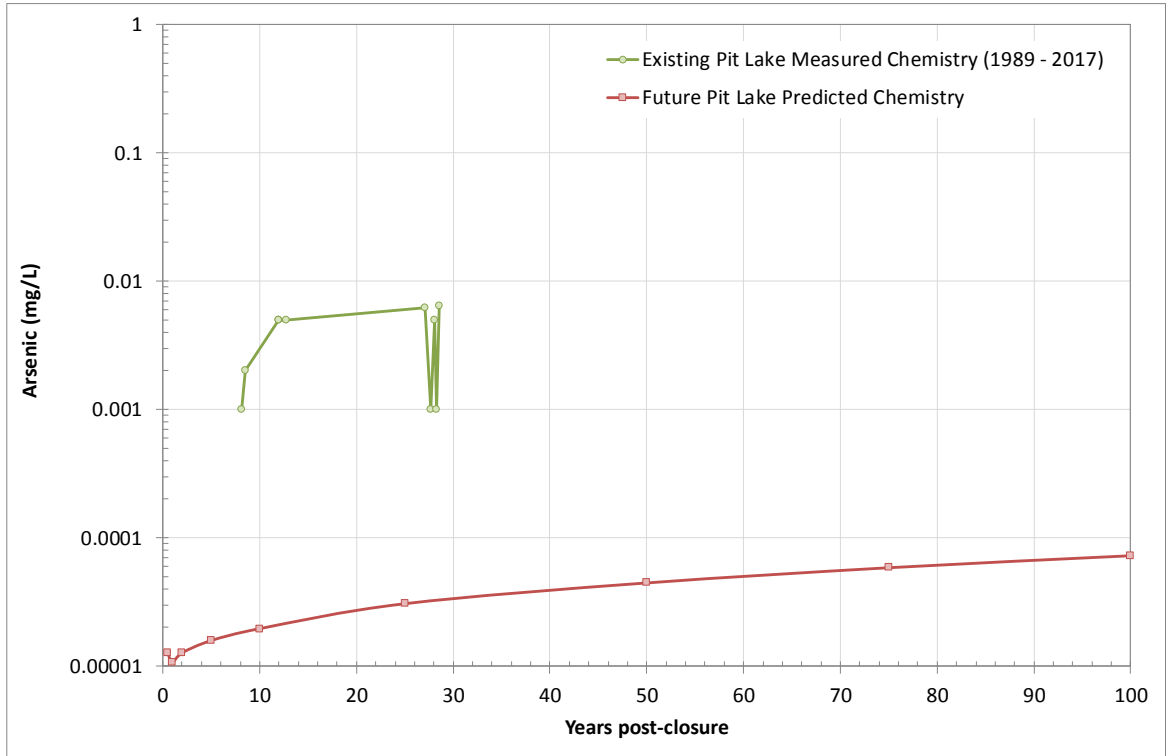
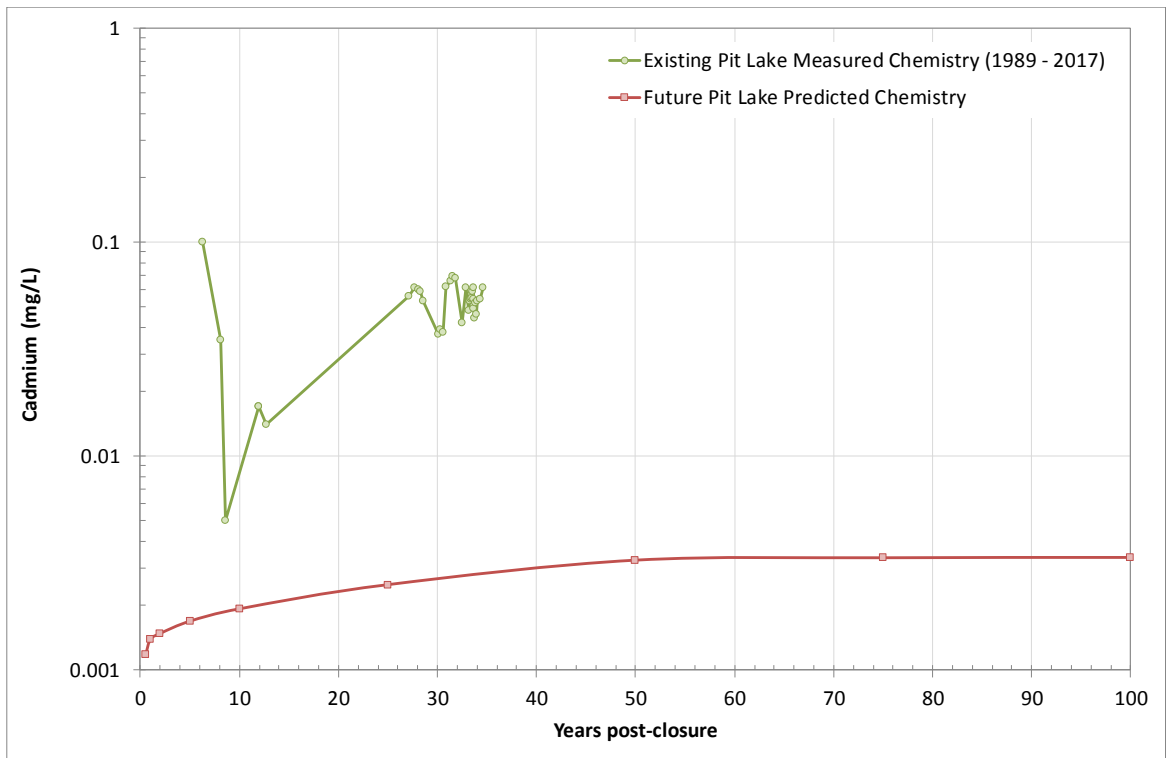


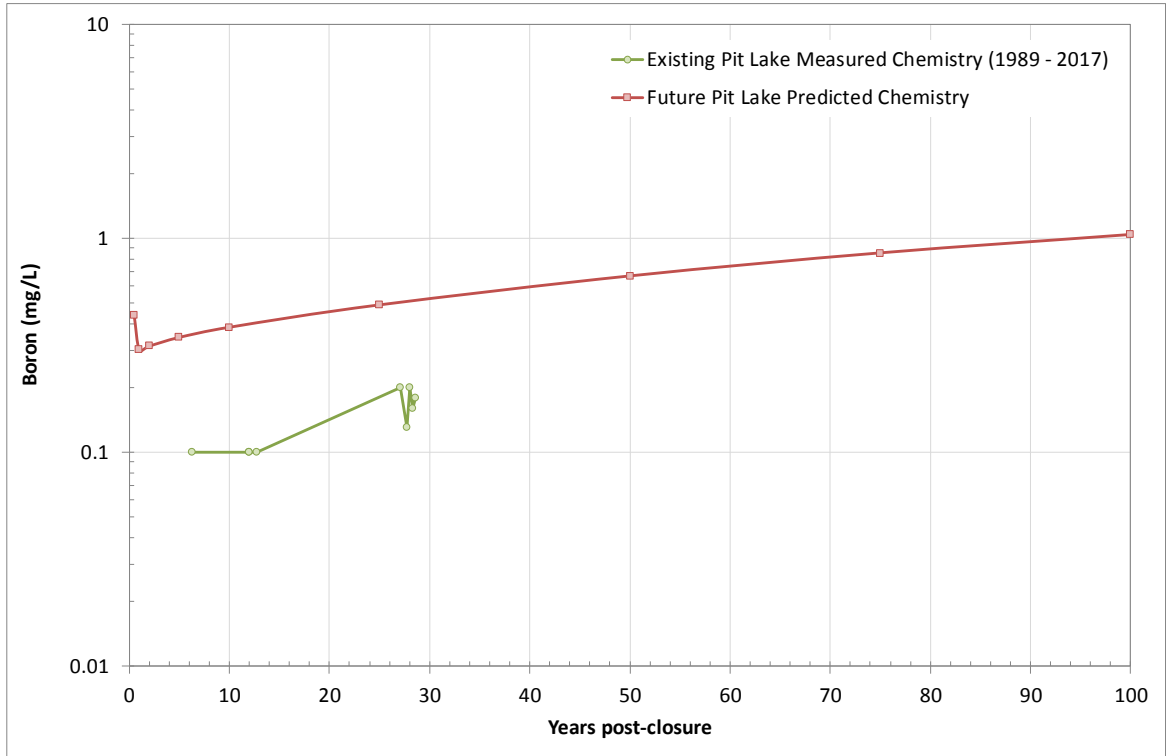
Figure 5-7: Time-series Plot of Predicted Copper for the Unreclaimed Pit Model



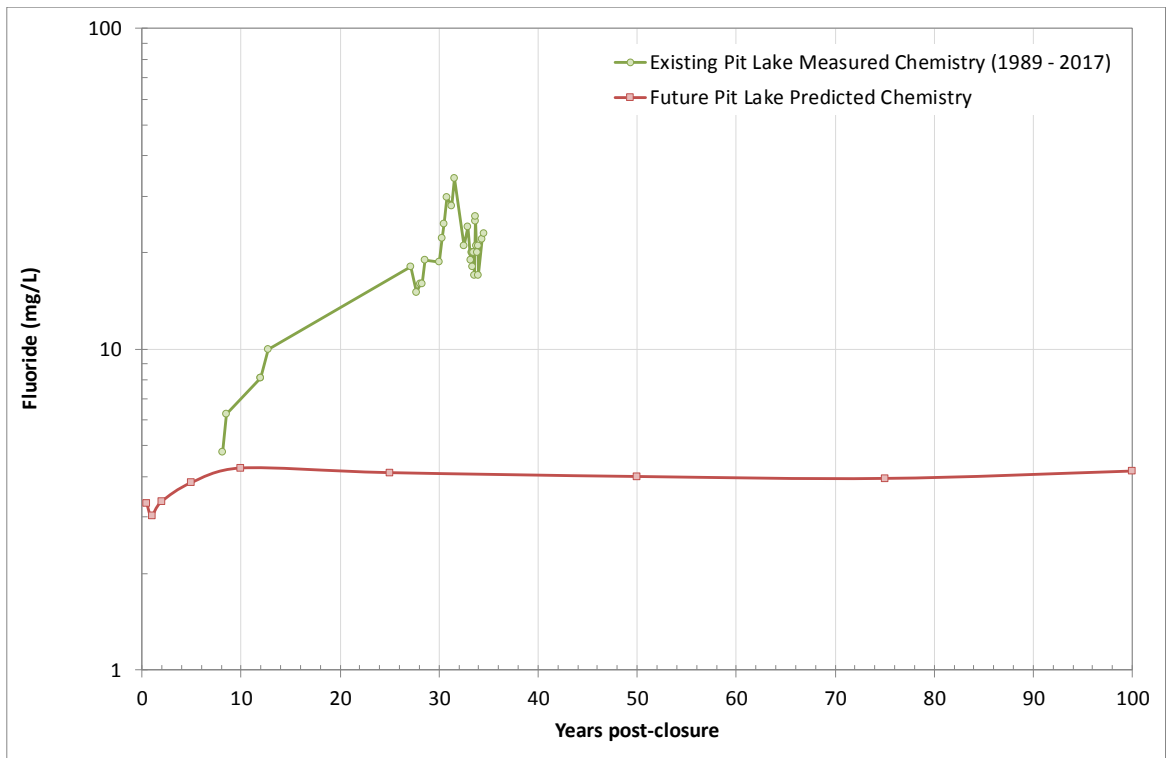
**Figure 5-8: Time-series Plot of Predicted Arsenic for the Unreclaimed Pit Model**



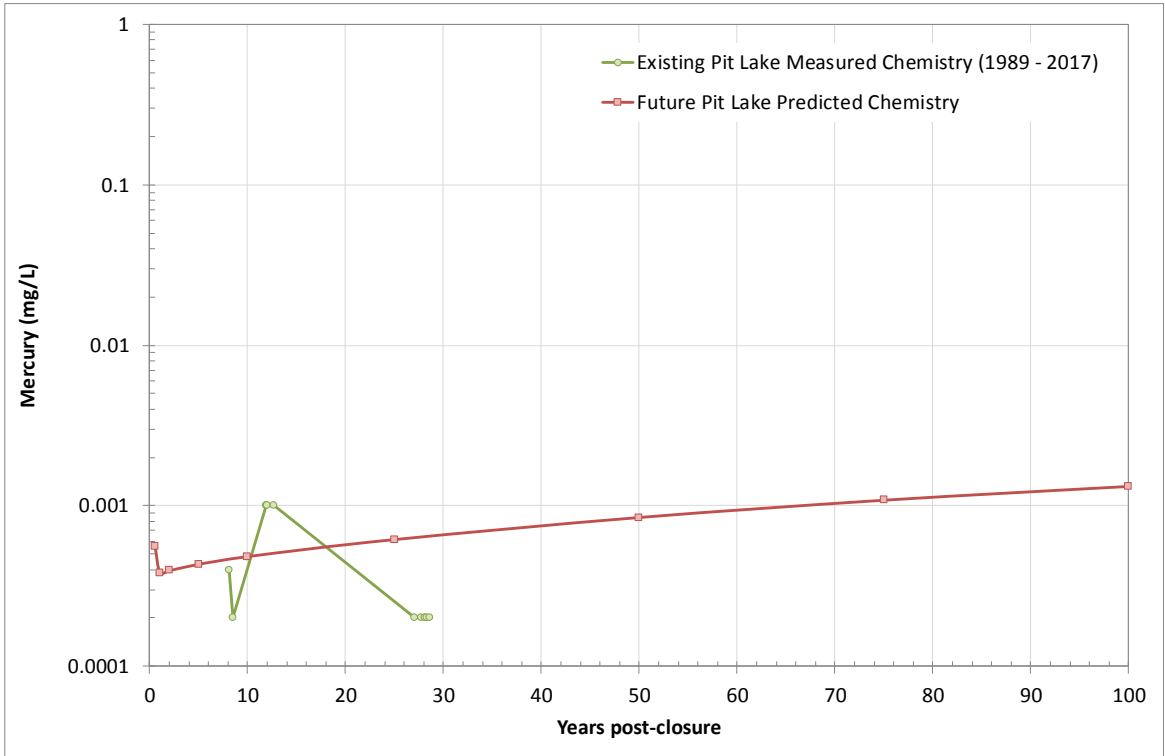
**Figure 5-9: Time-series Plot of Predicted Cadmium for the Unreclaimed Pit Model**



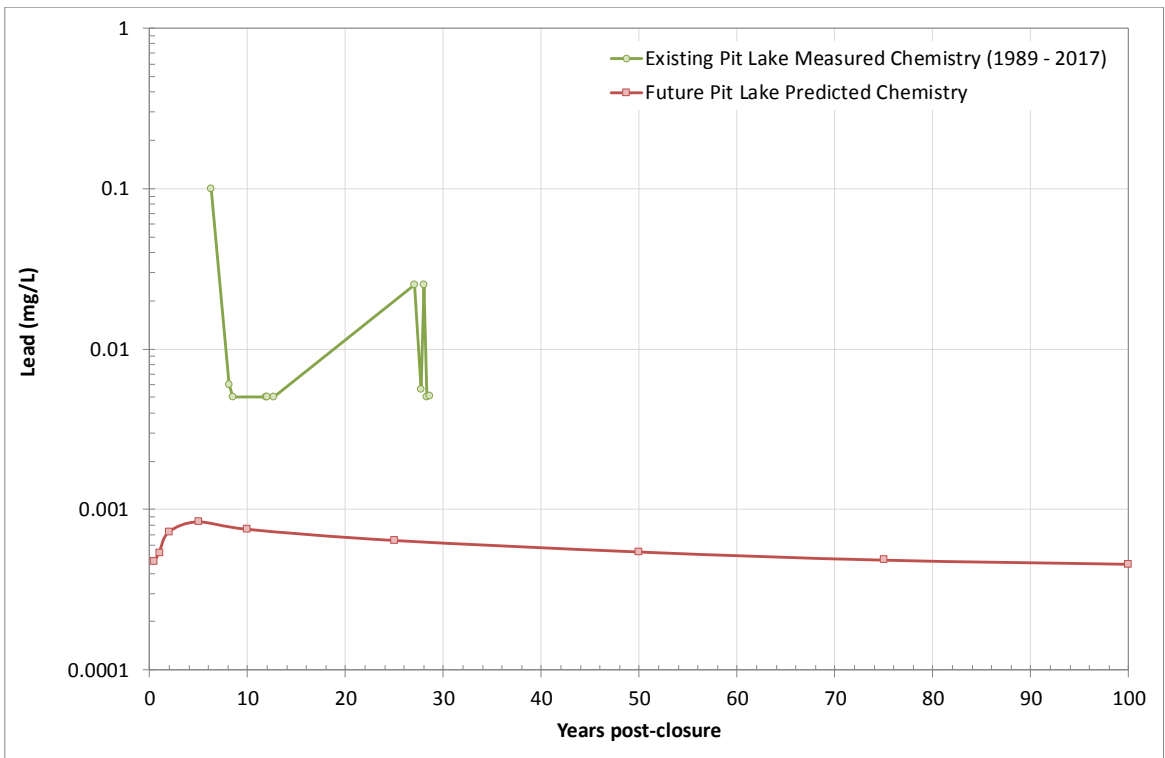
**Figure 5-10: Time-series Plot of Predicted Boron for the Unreclaimed Pit Model**



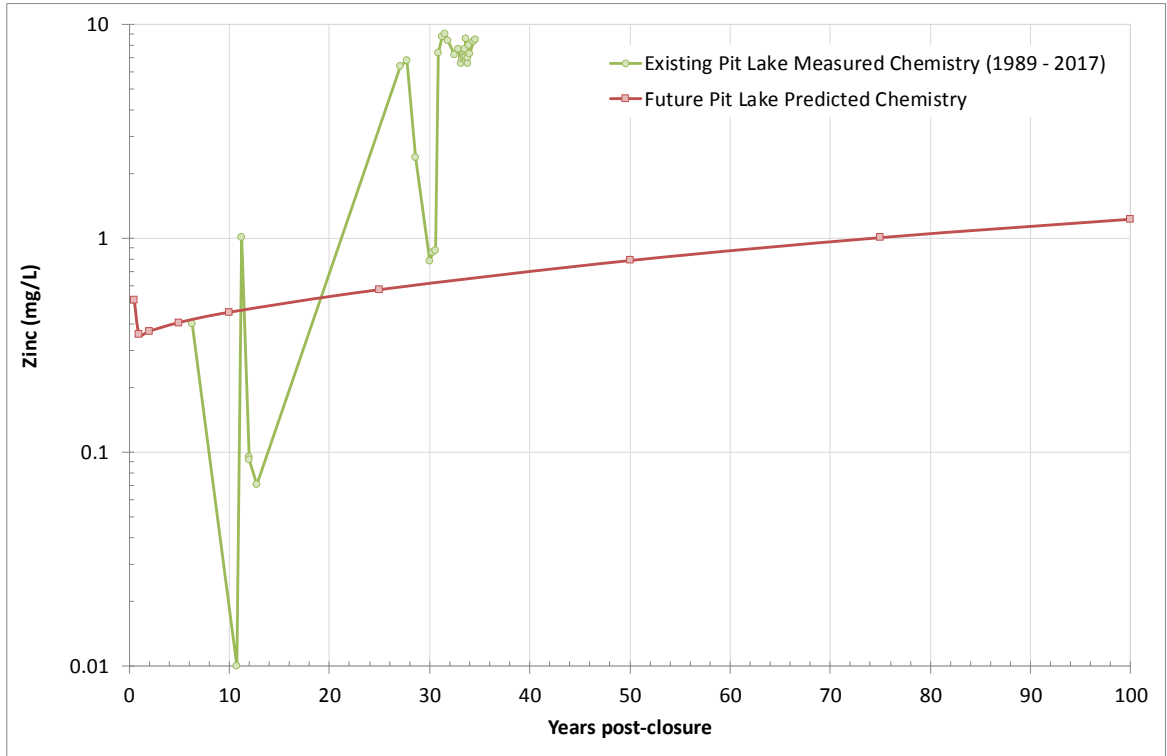
**Figure 5-11: Time-series Plot of Predicted Fluoride for the Unreclaimed Pit Model**



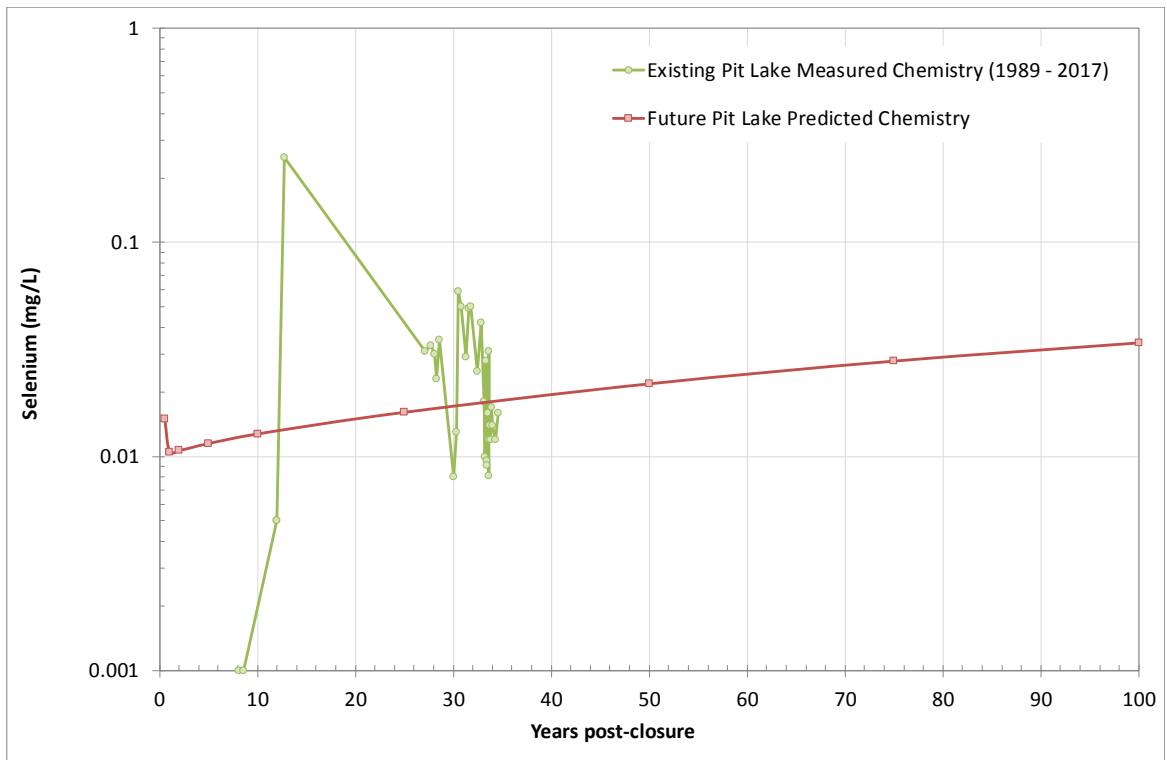
**Figure 5-12: Time-series Plot of Predicted Mercury for the Unreclaimed Pit Model**



**Figure 5-13: Time-series Plot of Predicted Lead for the Unreclaimed Pit Model**



**Figure 5-14: Time-series Plot of Predicted Zinc for the Unreclaimed Pit Model**



**Figure 5-15: Time-series Plot of Predicted Selenium for the Unreclaimed Pit Model**

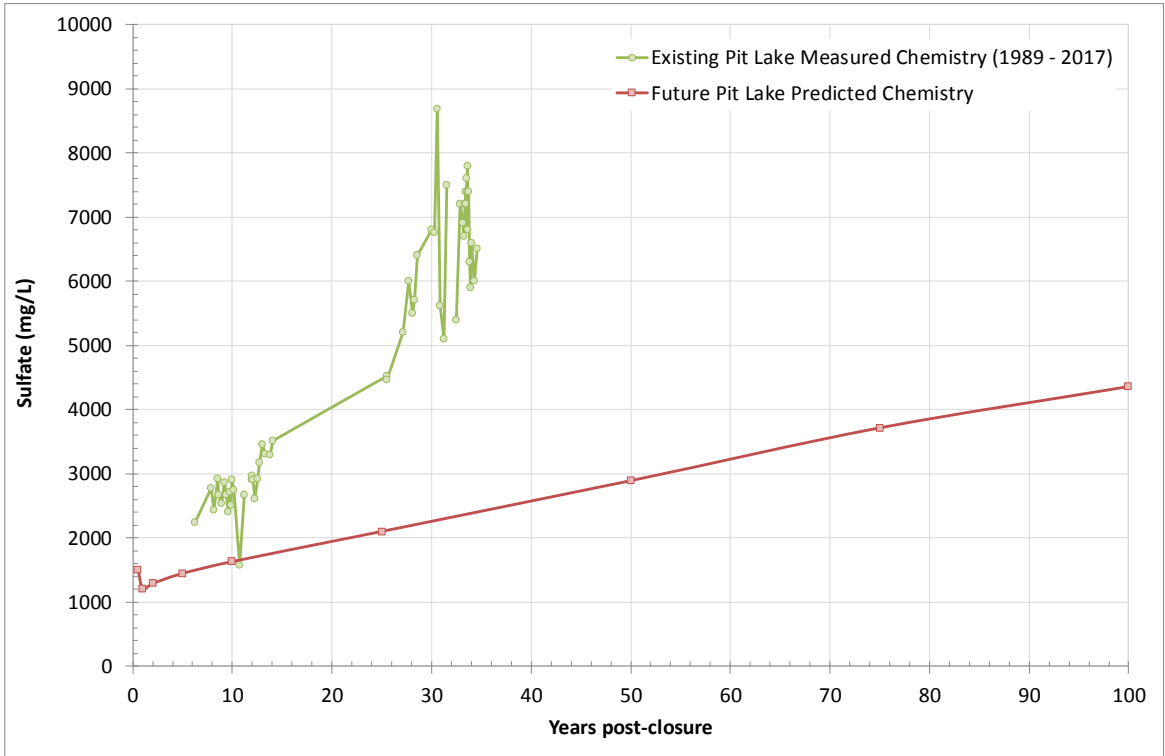


Figure 5-16: Time-series Plot of Predicted Sulfate for the Unreclaimed Pit Model

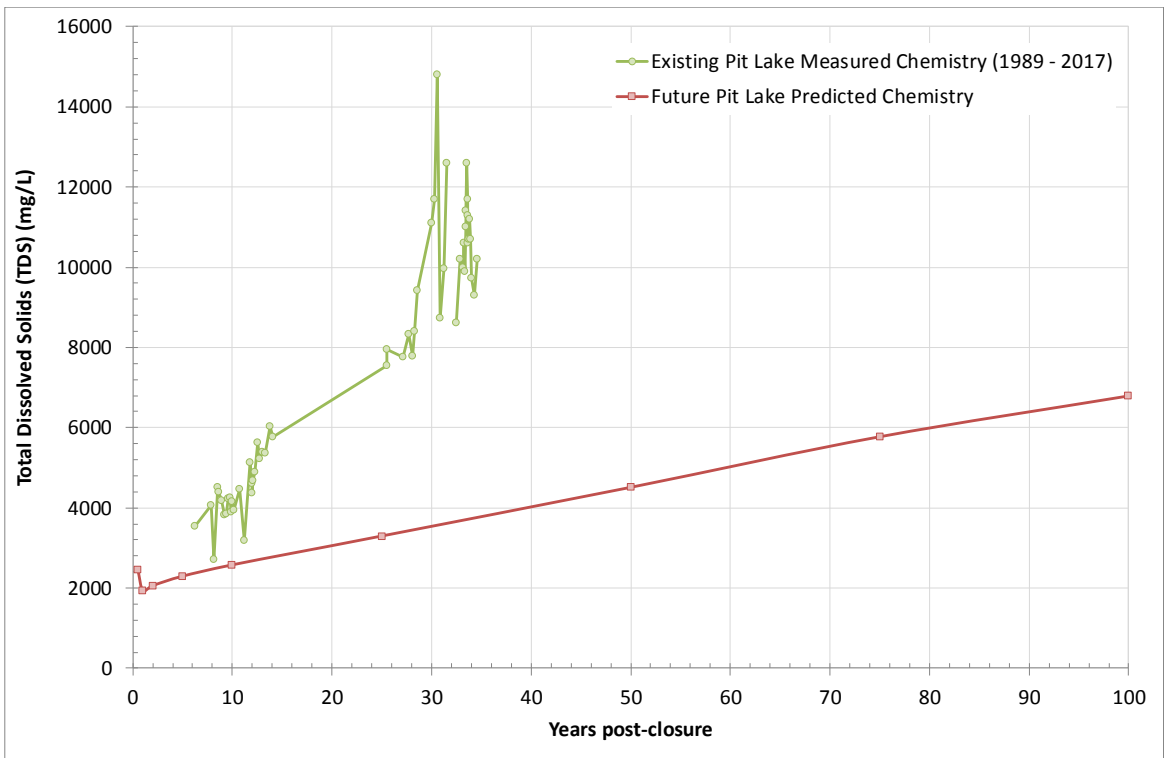


Figure 5-17: Time-series Plot of Predicted TDS for the Unreclaimed Pit Model

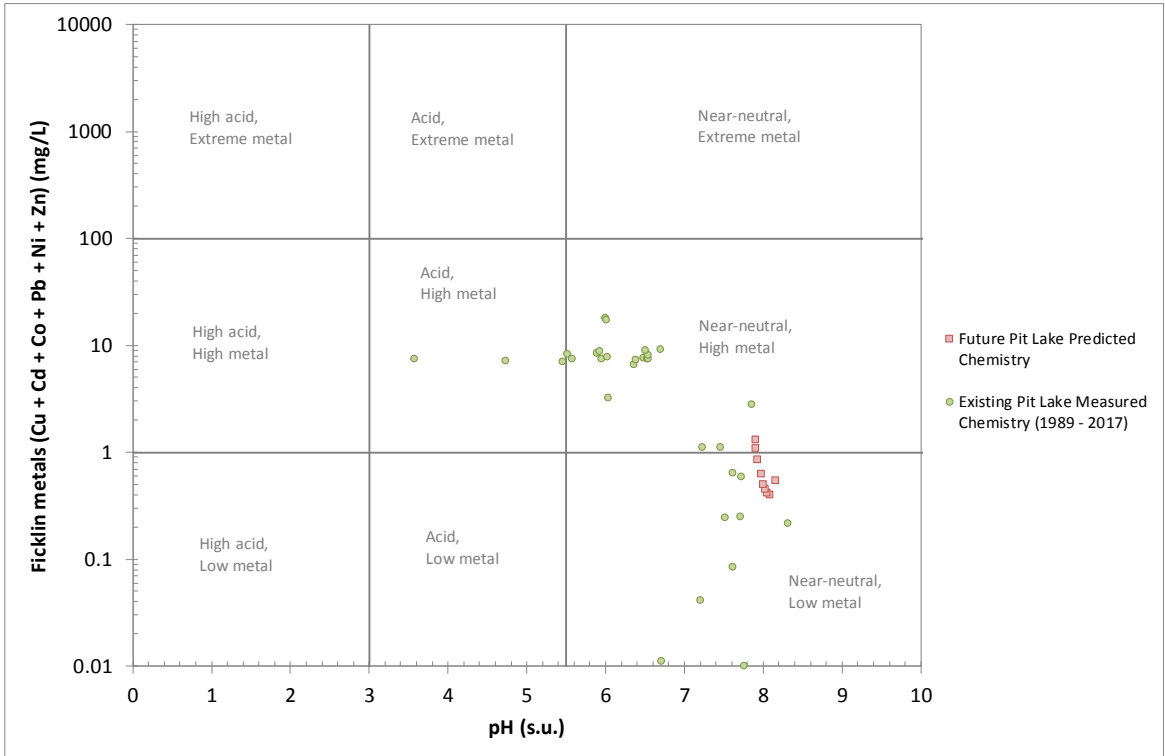


Figure 5-18: Ficklin Plot for the Unreclaimed Pit Model

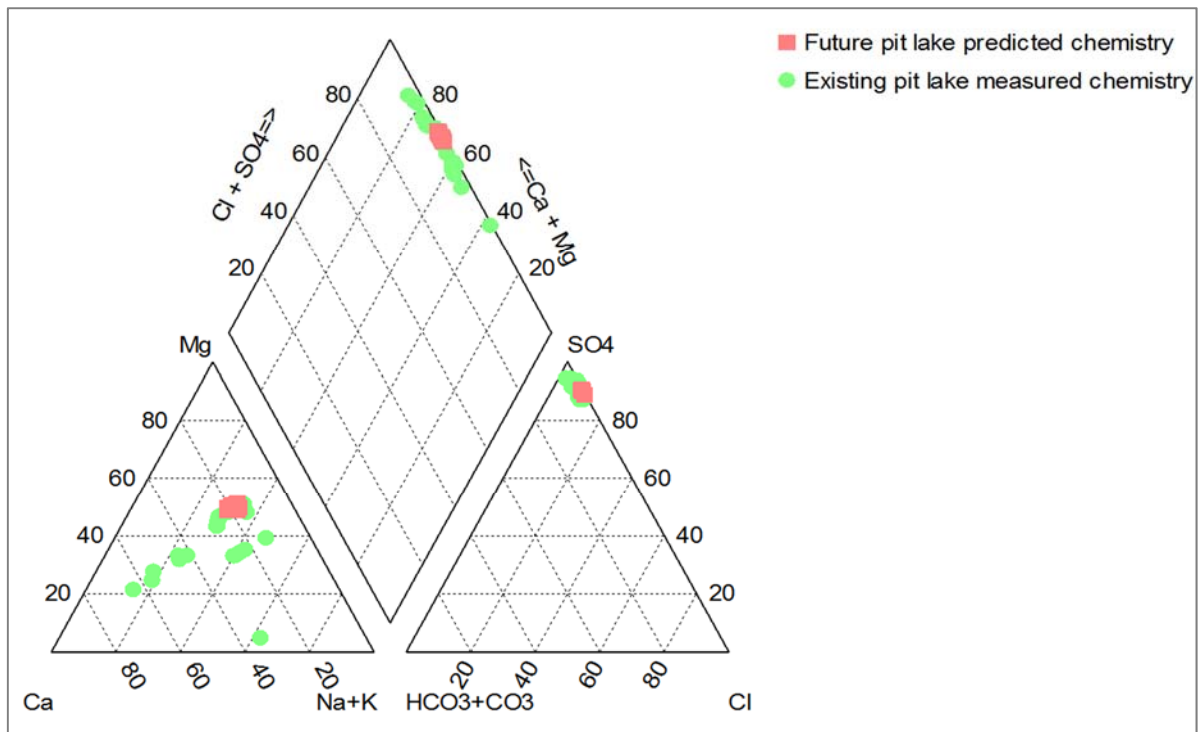


Figure 5-19: Piper Plot of Predicted Major Ion Chemistry for the Unreclaimed Pit Model

## 6 Reclaimed Pit Model with Rapid Fill

### 6.1 Conceptual Model

Rapid fill has been proposed as a reclamation strategy for the future pit and will dilute solutes derived from water-rock interaction. Rapid fill will quickly submerge walls and benches to limit the exposure of sulfide minerals to oxygen, and will reduce the effects of evapoconcentration over time. To assess the effects of initial rapid fill on predicted pit lake chemistry for the future pit, an alternative model has been run. This alternative fills the pit with 2,200 acre-feet from the water supply wells during the six months of pit filling. Rapid fill stops when the 4,897 ft water elevation is achieved. Additional reclamation activities for this scenario includes reclamation of the haul road, the expanded section of the 4900 catch bench and the pit shell crest (see Section 3.1.8).

Water quality predictions for this scenario were made for time periods of 0.5, 1, 2, 5, 10, 25, 50, 75, and, 100 years after the start of pit lake formation. A conceptual model for the reclaimed pit rapid fill scenario is presented in Figure 6-1 and inputs to the model are discussed in Sections 6.2 to 6.5.

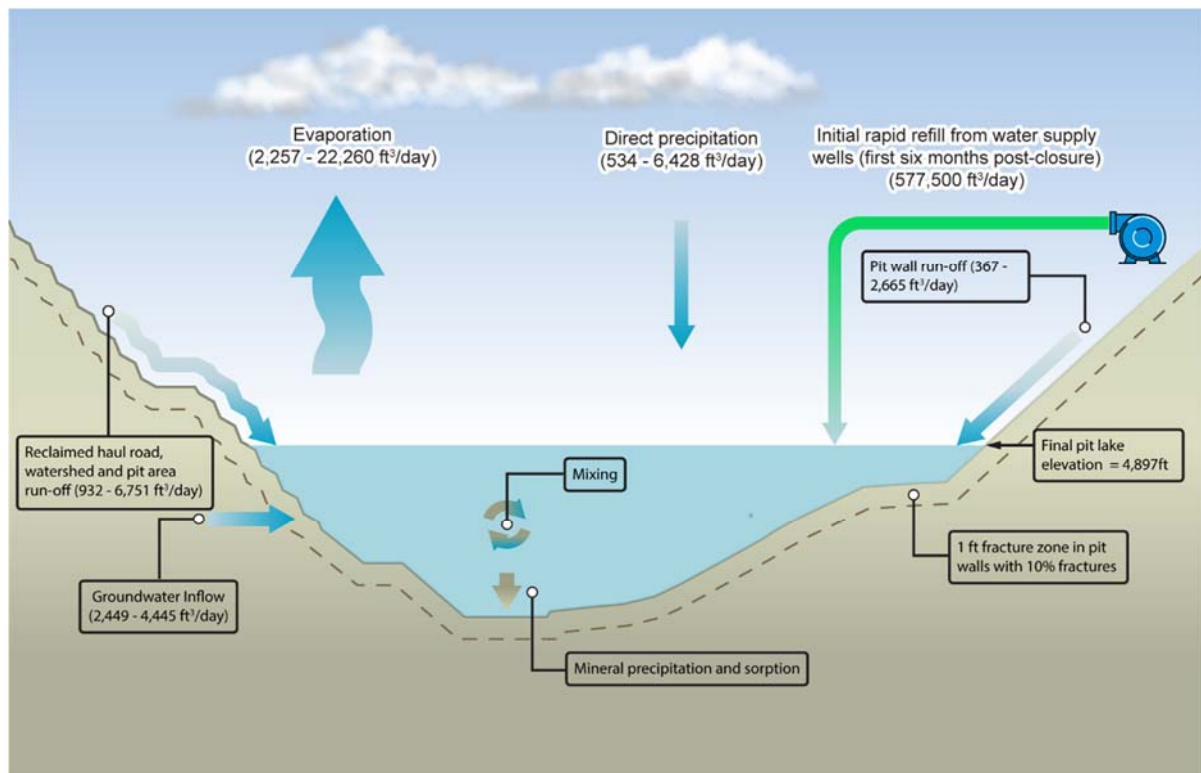


Figure 6-1: Conceptual Model for Reclaimed Pit with Rapid Fill



## 6.2 Pit Wall Surface Areas

The proportional surface areas of the main material types that will be exposed in the final walls of the reclaimed pit have been calculated from the FS geologic block model and the 2017 MORP pit. The block model was used to calculate the three-dimensional surface area of each material type that will be exposed in the pit wall both above and below the water level as pit filling progresses. Three-dimensional surface areas were calculated for each of the modeled time steps (i.e., for 0.5, 1, 2, 5, 10, 25, 50, 75 and 100 years after the start of pit lake formation). Material types were delineated based on primary lithology, oxidation (redox) and mineralization (i.e., mineralized versus weakly/non-mineralized). Areas proposed for cover and reclamation are excluded from the exposed surface areas.

The three-dimensional surface areas of each material type in the reclaimed pit at the end of mine life are provided in Table 6-1 and are illustrated in Figure 6-2. This demonstrates that unoxidized Quartz Monzonite will represent the dominant material type that will be exposed in the final walls of the reclaimed pit.

**Table 6-1: Three-dimensional Surface Areas of Pit Wall Rock Material Types for Final Reclaimed Pit**

Mineralization	Rock Type	Redox	Three-dimensional surface area	
			Square feet	%
Weakly/non-mineralized	Andesite	Oxide	41	0.001%
		Sulfide (non-ox.)	118,926	1.5%
	Biotite Breccia	Oxide	434	0.01%
		Sulfide (non-ox.)	206,789	2.7%
	Quartz Monzonite	Oxide	236	0.003%
		Sulfide (non-ox.)	2,052,482	26.4%
	Coarse Crystalline Porphyry	Oxide	790	0.01%
		Sulfide (non-ox.)	596,808	7.7%
Mineralized	Biotite Breccia	Sulfide (non-ox.)	526,770	6.8%
	Quartz Monzonite	Oxide	0	0%
		Sulfide (non-ox.)	1,777,718	22.9%
	Coarse Crystalline Porphyry	Oxide	0	0%
		Sulfide (non-ox.)	302,134	3.9%
Reclaimed area			2,191,373	28.2%
<b>Total</b>			<b>7,774,501</b>	<b>100%</b>

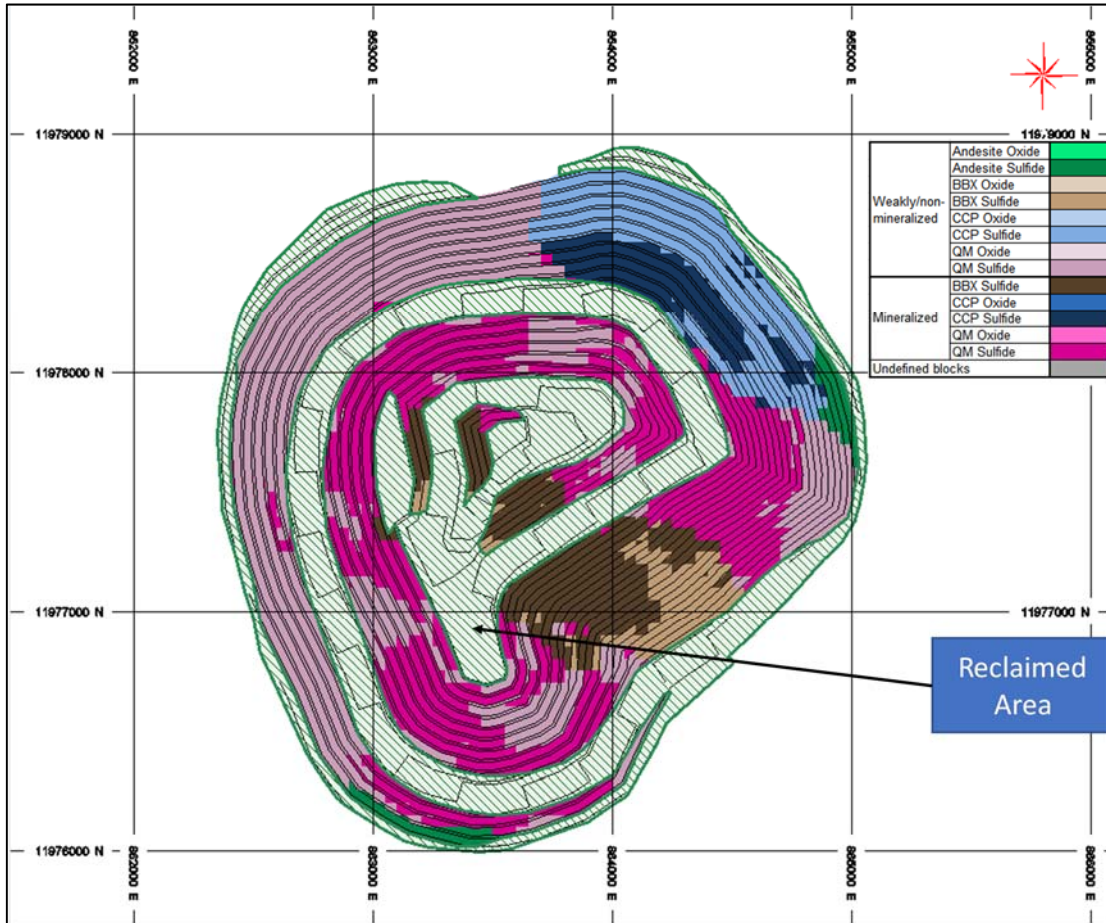


Figure 6-2: Exposed Material Types in Final Walls of the Reclaimed Pit

### 6.3 Calculation of Pit Wall Rock Available for Leaching

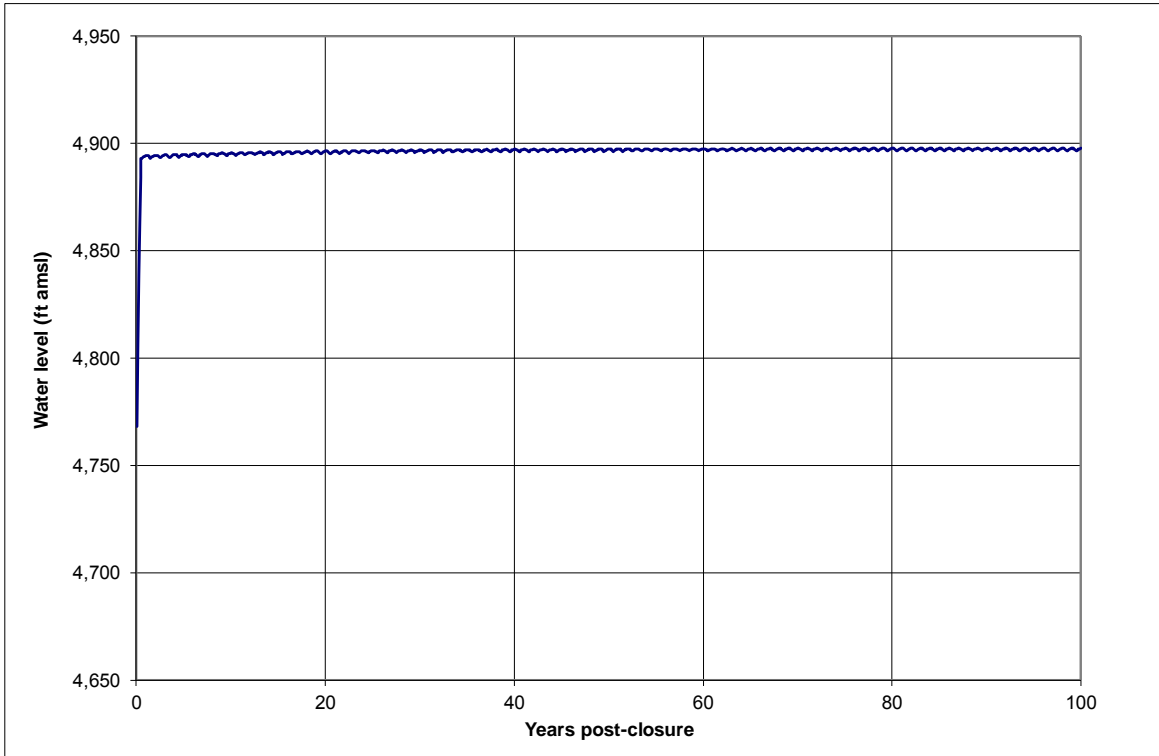
The blasting techniques that will be used for the reclaimed pit will be identical to those for the unreclaimed pit model. As such, a 1 foot thickness of reactive rock in the pit walls has also been assumed for the reclaimed pit model (Siskind and Fumanti, 1974; Kelsall et al., 1984). The method used to calculate the mass of pit wall available for leaching was identical to that used for the unreclaimed pit model (Section 5.3).

### 6.4 Water Balance

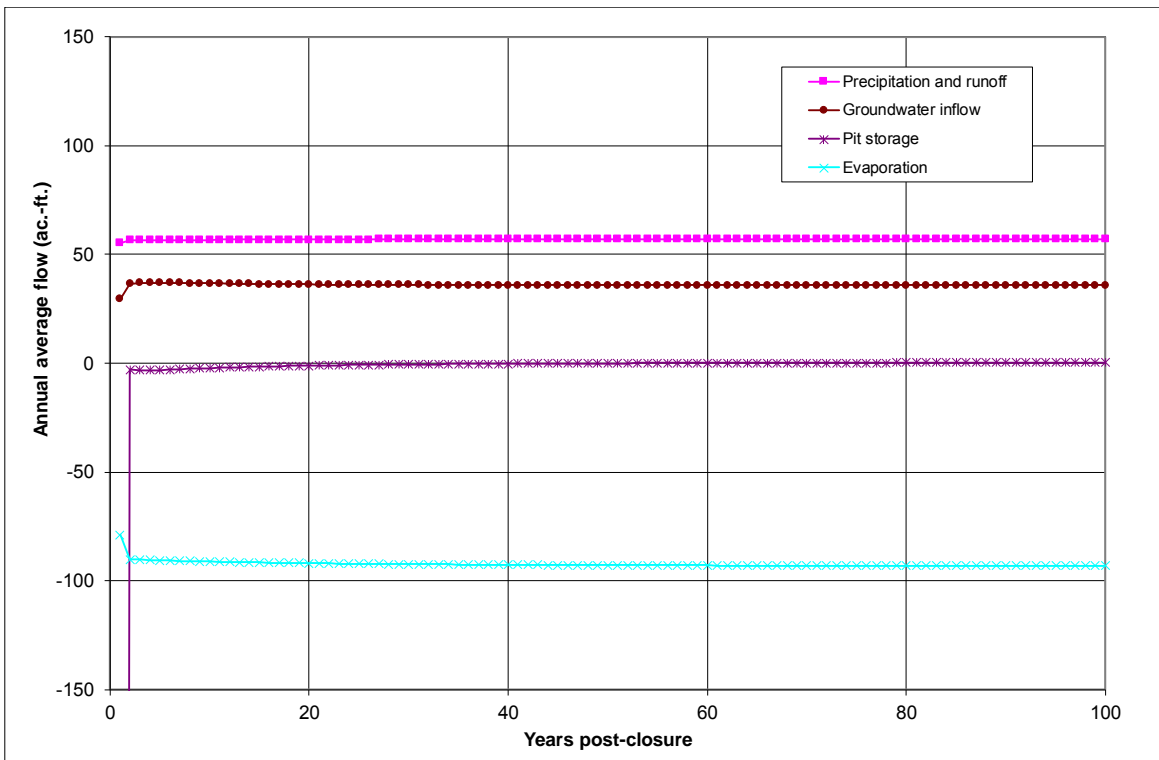
A pit lake water balance for the reclaimed pit model with rapid fill has been developed by JSAI and is based on the following inputs/assumptions (JSAI, 2017):

- The pit will be filled with 2,200 acre-feet from the water supply wells during the six months of pit infilling;
- Rapid fill stops when the 4,897 ft water elevation is achieved;
- Evaporation will represent the dominant solution loss; and
- The pit lake evaporation rate is 50 inches per year.

As with the unreclaimed pit model, the pit lake for the reclaimed pit model will also be a hydrologic sink. The pit lake filling curve is shown in Figure 6-3 and the various inputs/outputs to the pit are shown in Figure 6-4.



**Figure 6-3: Pit Lake Elevation Curve for Reclaimed Pit Model with Rapid Fill**



**Figure 6-4: Pit Lake Flux for Reclaimed Pit Model with Rapid Fill**

## **6.5 Solution Inputs**

### **6.5.1 Precipitation Chemistry**

As with the existing pit model (Section 4) and unreclaimed pit model (Section 5), the primary wall rock lixiviant for the pit high walls in the reclaimed pit model is assumed to be precipitation. Representative precipitation chemistry data were obtained from monthly monitoring carried out between 1985 and 2011 at the Gila Cliff Dwellings National Monument meteorological station, Catron County, New Mexico (NADP, 2012).

### **6.5.2 Groundwater Chemistry**

Following the initial rapid fill with water from the supply wells, groundwater will continue to enter the pit. The groundwater chemistry used for the reclaimed pit model was identical to that used for the unreclaimed pit model (Section 5.5.2, Table 5-2).

### **6.5.3 Wall Rock Chemistry**

The pit shell and exposed wall rocks for the reclaimed pit model will be identical to those in the unreclaimed model. As such, the same wall rock source terms were used in the model (Section 5.5.3, Table 5-2).

### **6.5.4 Water Supply Well Chemistry**

Water used to rapidly fill the pit is represented by hydrochemical data from water supply wells PW-1 and PW-3 (Table 6-2; JSAI, 2017c; Appendix E).

**Table 6-2: Water Supply Well Chemistry for PW-1 and PW-3 used to Represent Rapid Fill Water Quality in the Reclaimed Pit Model**

Parameter		Units	Average Chemistry for PW-1 and PW-3
pH	pH	s.u.	8.03
HCO <sub>3</sub>	Bicarbonate	mg/L	135
Ag	Silver	mg/L	<0.005*
Al	Aluminum	mg/L	<0.02*
As	Arsenic	mg/L	0.005
B	Boron	mg/L	0.08
Ba	Barium	mg/L	0.009
Be	Beryllium	mg/L	<0.002*
Ca	Calcium	mg/L	28
Cd	Cadmium	mg/L	<0.002*
Cl	Chloride	mg/L	41
Co	Cobalt	mg/L	<0.006*
Cu	Copper	mg/L	<0.006*
Cr	Chromium	mg/L	0.006
F	Fluoride	mg/L	1.45
Fe	Iron	mg/L	0.053
Hg	Mercury	mg/L	<0.0002*
K	Potassium	mg/L	3.35
Mg	Magnesium	mg/L	2.05
Mn	Manganese	mg/L	0.0025
Mo	Molybdenum	mg/L	<0.008*
Na	Sodium	mg/L	69.5
Ni	Nickel	mg/L	<0.01*
Pb	Lead	mg/L	<0.005*
SO <sub>4</sub>	Sulfate	mg/L	27
Se	Selenium	mg/L	<0.001*
Si	Silica	mg/L	19
U	Uranium	mg/L	0.0023
V	Vanadium	mg/L	<0.05*
Tl	Thallium	mg/L	<0.001
Zn	Zinc	mg/L	0.023

\* Parameters below analytical detection limits were not included in the input to the PHREEQC model

### 6.5.5 Reclaimed Surface Chemistry

At closure, several areas of the pit will be reclaimed. Water quality associated with run-off from these areas is therefore likely to have a different chemical composition from the rest of the pit walls. As such, the water balance provided by JSAI includes a separate input to the water balance for the reclaimed areas and receiving watershed. Conveyed stormwater is expected to have a chemistry similar to background surface water quality from SWQ-1 (Table 6-3; JSAI, 2015b).

**Table 6-3: Water Supply Well Chemistry for SWQ-1 used to Represent reclaimed pit Run-off Water Quality in the Reclaimed Pit Model**

Parameter		Units	Average Chemistry for SWQ-1
pH	pH	s.u.	8.3
HCO <sub>3</sub>	Bicarbonate	mg/L	430
Al	Aluminum	mg/L	<0.1*
As	Arsenic	mg/L	<0.005*
B	Boron	mg/L	0.02
Ba	Barium	mg/L	<0.5*
Ca	Calcium	mg/L	109
Cd	Cadmium	mg/L	<0.002*
Cl	Chloride	mg/L	30
Co	Cobalt	mg/L	<0.05*
Cu	Copper	mg/L	<0.01*
Cr	Chromium	mg/L	<0.02*
F	Fluoride	mg/L	0.3
Fe	Iron	mg/L	<0.05*
Hg	Mercury	mg/L	<0.001*
K	Potassium	mg/L	1.8
Mg	Magnesium	mg/L	36
Mn	Manganese	mg/L	<0.02*
Mo	Molybdenum	mg/L	<0.02*
Na	Sodium	mg/L	107
Pb	Lead	mg/L	<0.02*
Se	Selenium	mg/L	<0.005*
SO <sub>4</sub>	Sulfate	mg/L	261
Zn	Zinc	mg/L	<0.01*

\* Parameters below analytical detection limits were not included in the input to the PHREEQC model

## 6.6 Results

The predicted pit lake chemistry for the reclaimed pit model is summarized in Table 6-4 and illustrated in Figure 6-5 to Figure 6-18 for selected parameters. These show predicted pit lake chemistry at each of the modeled time steps (i.e., 0.5, 1, 2, 5, 10, 25, 50, 75 and 100 years post-closure) compared to water quality in the existing pit lake. The full PHREEQC output file is provided in Appendix I, which shows precipitating and dissolving mineral species at each time step as part of the mass transfer calculations.

As with the unreclaimed pit model, pit lake waters for the reclaimed pit model are predicted to be moderately alkaline (pH 8.1 – 8.4) with a predominantly sodium + chloride/sulfate (Na + SO<sub>4</sub>/Cl) major ion signature (Figure 6-18). Rapidly filling the pit with the water supply wells during the first six months post-closure results in a more dilute initial water chemistry with a sodium-chloride (Na+Cl) signature. The result is that the effects of evapoconcentration are not as pronounced as the pit lake reaches hydrogeologic equilibrium, and predicted concentrations of many major ions and trace elements at 100 years remain lower than if natural fill were used. This is particularly the case for constituents such as boron, sulfate and chloride, which are strongly influenced by evaporation effects and are predicted to be much lower in concentration for the rapid fill scenario compared to the natural fill scenario. The rapid fill will also quickly submerge walls and benches to limit the exposure of sulfide minerals to oxygen, which will reduce trace element release into the pit lake.

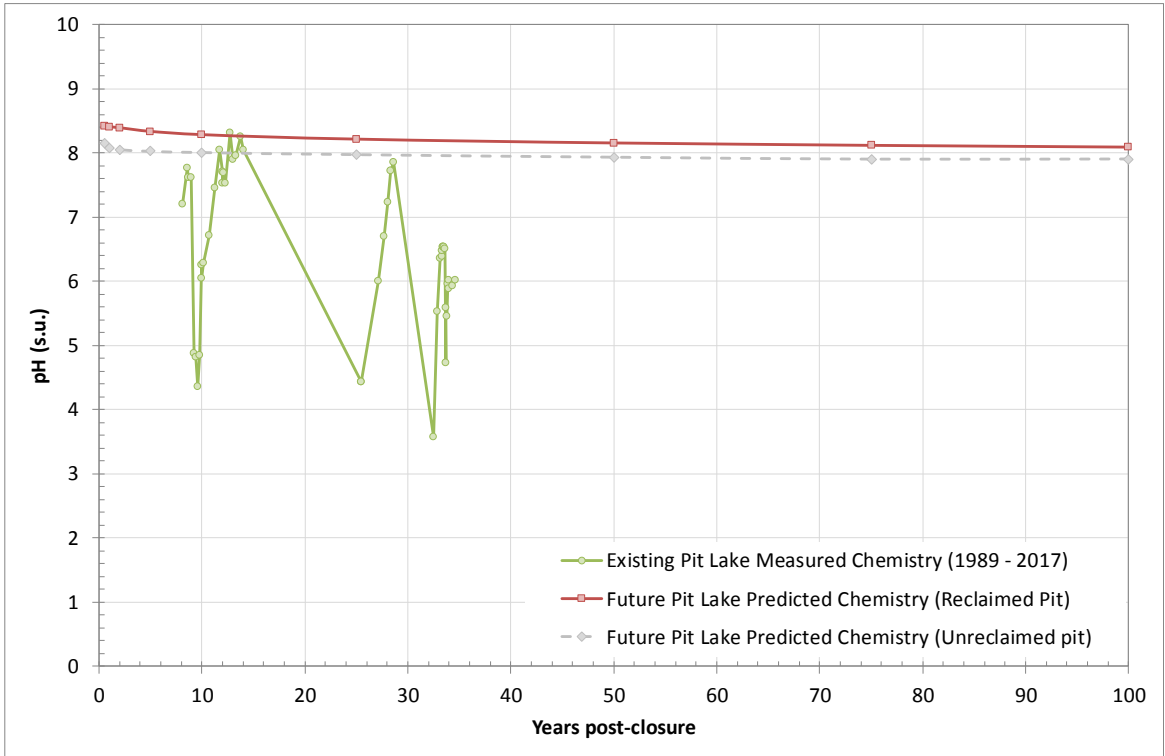
As with the unreclaimed model, concentrations of the majority of constituents are either comparable to or less than concentrations in the existing pit lake at Copper Flat. Pit lake waters for the reclaimed pit model are predicted to be 'near-neutral, low-metal' waters based on pH values between 8.1 and 8.4 and total Ficklin metal concentrations less than 1 mg/L (Figure 6-17).

**Table 6-4: Reclaimed Pit Model Results**

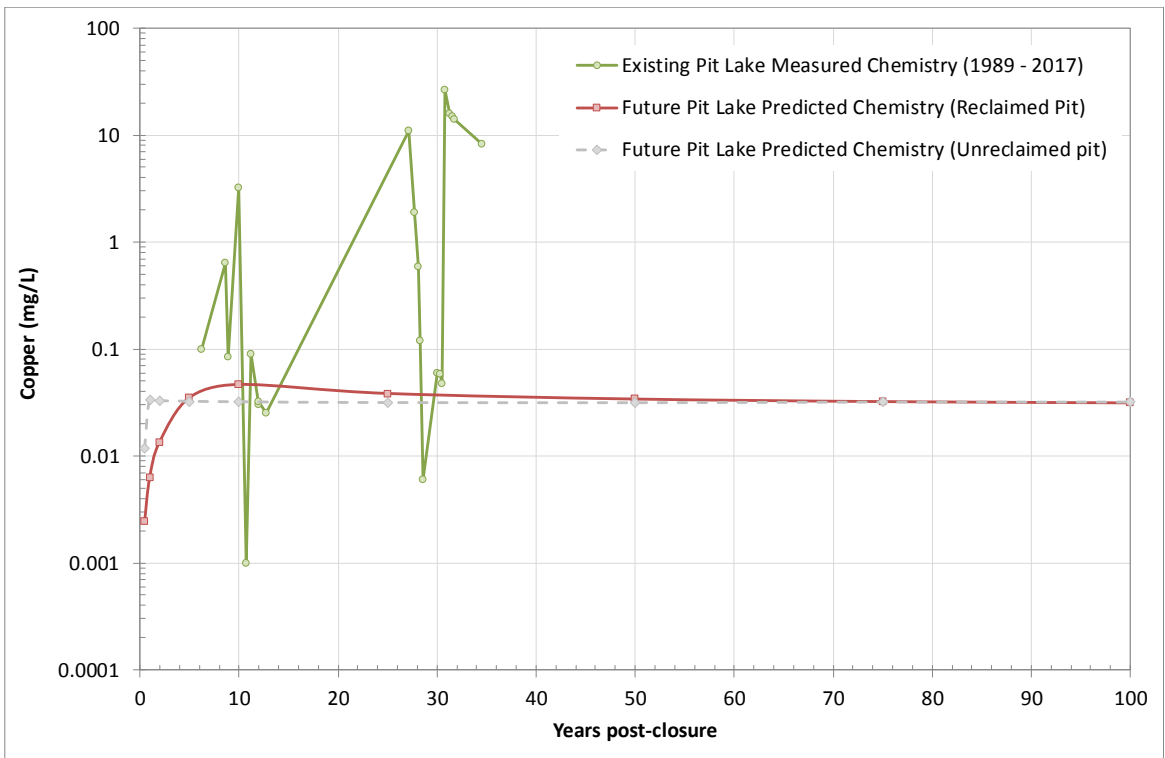
Parameter		Units	Measured Chemistry in Existing Pit (1989 - 2017)			Predicted Future Chemistry (Years Post-Closure)								
			Average	Minimum	Maximum	0.5	1	2	5	10	25	50	75	100
pH	pH	s.u.	6.5	3.6	8.3	8.4	8.4	8.4	8.3	8.3	8.2	8.2	8.1	8.1
HCO <sub>3</sub>	Bicarbonate	mg/L	40.4	<3	122	85.3	83.6	81.3	72.6	65.9	58.9	54.0	51.5	49.7
Al	Aluminium	mg/L	10.4	<0.02	82.6	0.0004	0.001	0.003	0.008	0.02	0.04	0.07	0.11	0.15
As	Arsenic	mg/L	0.004	<0.001	0.006	5.22E-04	4.61E-04	3.63E-04	1.35E-04	2.75E-05	7.44E-06	1.19E-05	2.32E-05	3.81E-05
B	Boron	mg/L	0.14	<0.1	0.2	0.08	0.09	0.09	0.12	0.16	0.27	0.46	0.64	0.83
Ca	Calcium	mg/L	550	455	684	12.7	13.7	15.4	22.5	32.9	58.9	99.5	138	177
Cd	Cadmium	mg/L	0.05	<0.005	0.1	0.00001	0.00003	0.0001	0.0002	0.0003	0.0008	0.0016	0.002	0.003
Co	Cobalt	mg/L	0.29	<0.05	0.49	0.00005	0.0001	0.0002	0.0006	0.001	0.003	0.006	0.010	0.013
Cr	Chromium	mg/L	0.03	<0.006	0.1	3.07E-05	3.10E-05	3.16E-05	3.38E-05	3.64E-05	4.12E-05	4.70E-05	5.16E-05	5.57E-05
Cu	Copper	mg/L	4.44	0.001	26.5	0.002	0.01	0.01	0.03	0.05	0.04	0.03	0.03	0.03
F	Fluoride	mg/L	19.2	4.8	34	1.49	1.52	1.61	1.88	2.31	3.58	5.68	5.81	5.80
Fe	Iron	mg/L	0.2	<0.02	1.3	3.93E-05	3.94E-05	3.96E-05	4.05E-05	4.16E-05	4.39E-05	4.64E-05	4.84E-05	5.00E-05
Hg	Mercury	mg/L	0.0005	<0.0002	0.001	0.000005	0.00001	0.00002	0.0000	0.0001	0.0002	0.0004	0.0007	0.0009
K	Potassium	mg/L	32.1	11.0	60.6	4.91	6.39	9.43	18.5	33.4	77.7	151	224	298
Mg	Magnesium	mg/L	698	43	1,120	3.47	5.06	7.65	13.7	26.1	68.3	141	214	288
Mn	Manganese	mg/L	34.8	0.02	59.0	0.04	0.08	0.15	0.37	0.74	1.83	3.64	5.44	7.24
Mo	Molybdenum	mg/L	0.04	0.015	0.1	0.002	0.004	0.01	0.019	0.04	0.09	0.19	0.28	0.37
Na	Sodium	mg/L	888	165	1,400	72.6	74.9	80.6	97.6	126	209	348	486	625
Ni	Nickel	mg/L	0.06	0.039	0.1	0.0001	0.0002	0.000	0.001	0.002	0.005	0.010	0.015	0.020
Pb	Lead	mg/L	0.02	<0.005	0.1	0.00001	0.00003	0.0001	0.0002	0.0004	0.001	0.001	0.001	0.001
Sb	Antimony	mg/L		<0.001*		0.00004	0.0001	0.0001	0.0004	0.001	0.002	0.004	0.005	0.007
Se	Selenium	mg/L	0.028	<0.001	0.25	0.0001	0.0003	0.001	0.001	0.003	0.007	0.013	0.019	0.026
U	Uranium	mg/L	0.11	0.11	0.12	0.002	0.002	0.003	0.01	0.01	0.02	0.04	0.06	0.07
V	Vanadium	mg/L	0.1	<0.05	0.25	2.84E-06	9.11E-06	2.00E-05	4.92E-05	9.63E-05	2.32E-04	4.57E-04	6.81E-04	9.06E-04
Zn	Zinc	mg/L	5.4	0.01	9	0.03	0.03	0.04	0.06	0.11	0.23	0.44	0.65	0.85
SO <sub>4</sub>	Sulfate	mg/L	4,803	1,566	8,690	39.3	55.3	83.6	168	307	719	1,402	2,085	2,769
Cl	Chloride	mg/L	332	47.3	730	66.5	67.1	69.5	76.9	89.0	125	186	247	309
TDS	Total Dissolved Solids	mg/L	7,538	2,711	14,800	286	308	349	472	684	1,324	2,393	3,460	4,530

\* Indicates parameter was uniformly below analytical detection limits in pit lake water over monitoring period, but detection limit was variable. Concentration shown in table represents lower limit of analytical detection.

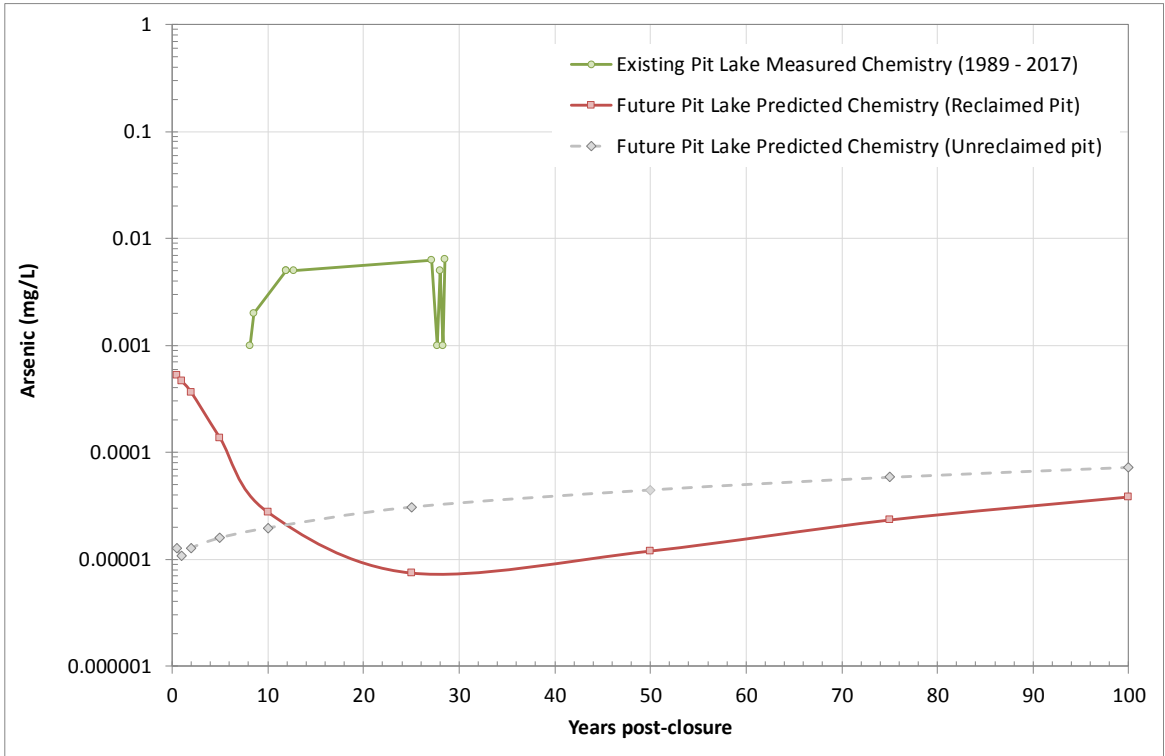




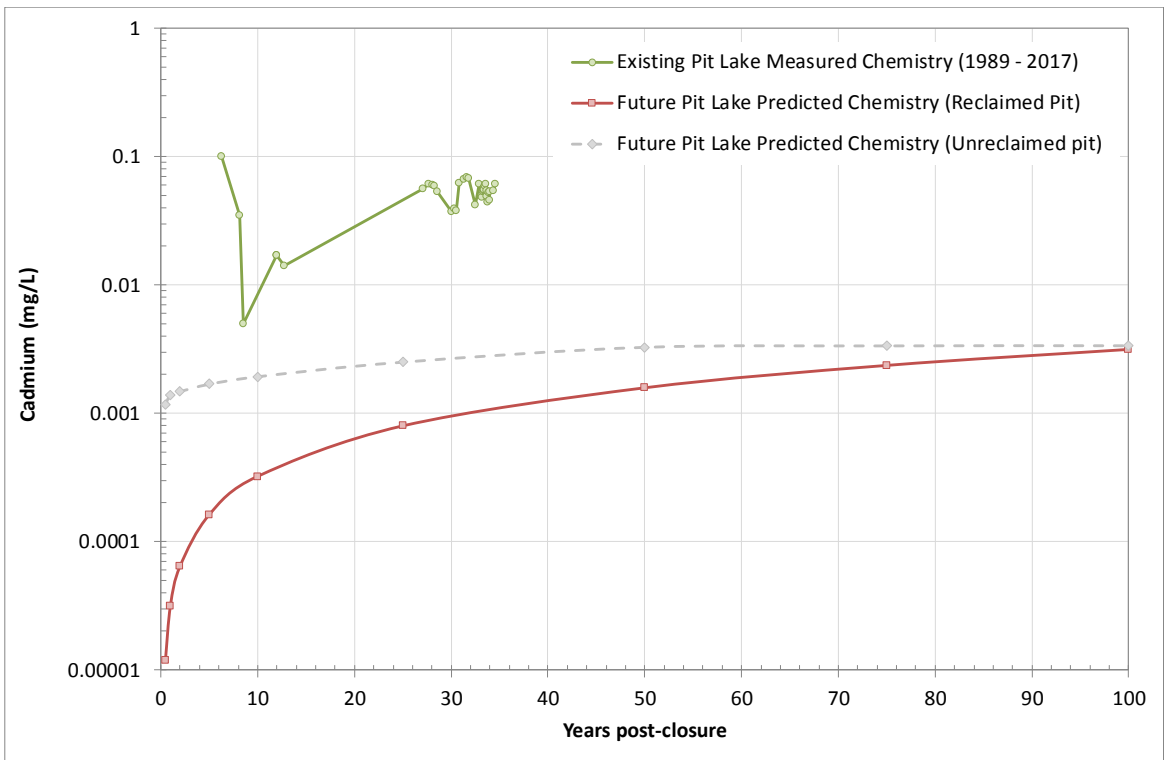
**Figure 6-5: Time-series Plot of Predicted pH for the Reclaimed Pit Model**



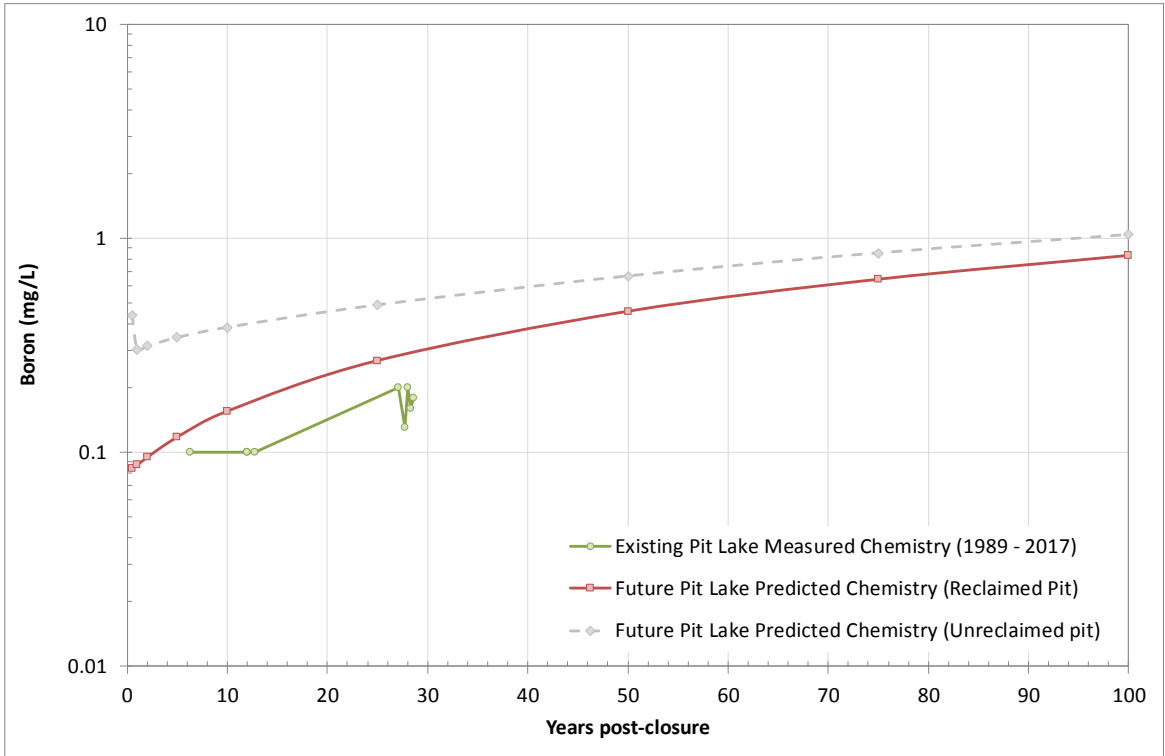
**Figure 6-6: Time-series Plot of Predicted Copper for the Reclaimed Pit Model**



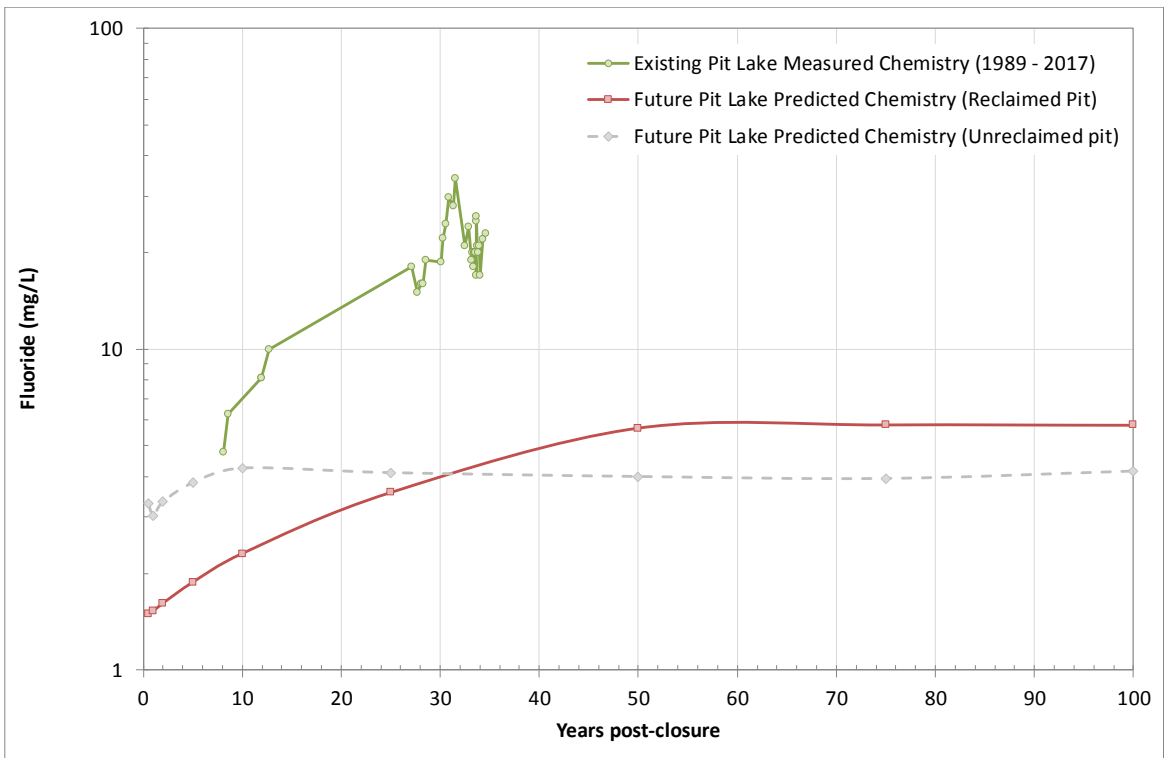
**Figure 6-7: Time-series Plot of Predicted Arsenic for the Reclaimed Pit Model**



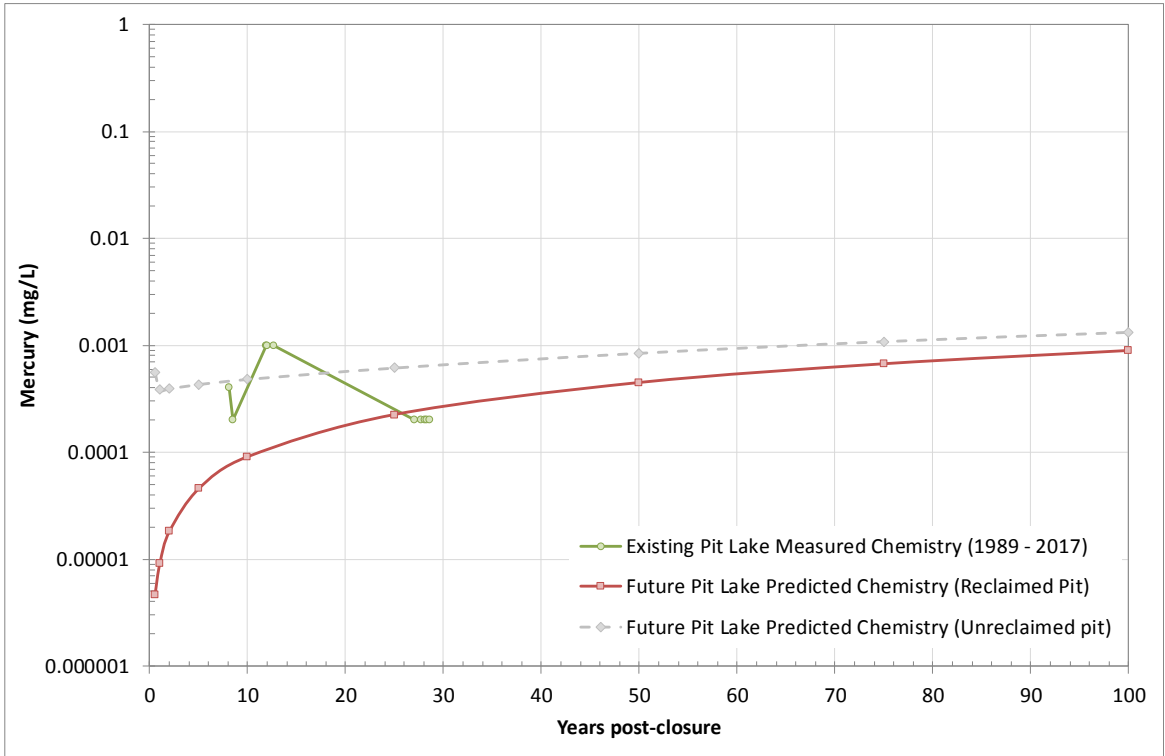
**Figure 6-8: Time-series Plot of Predicted Cadmium for the Reclaimed Pit Model**



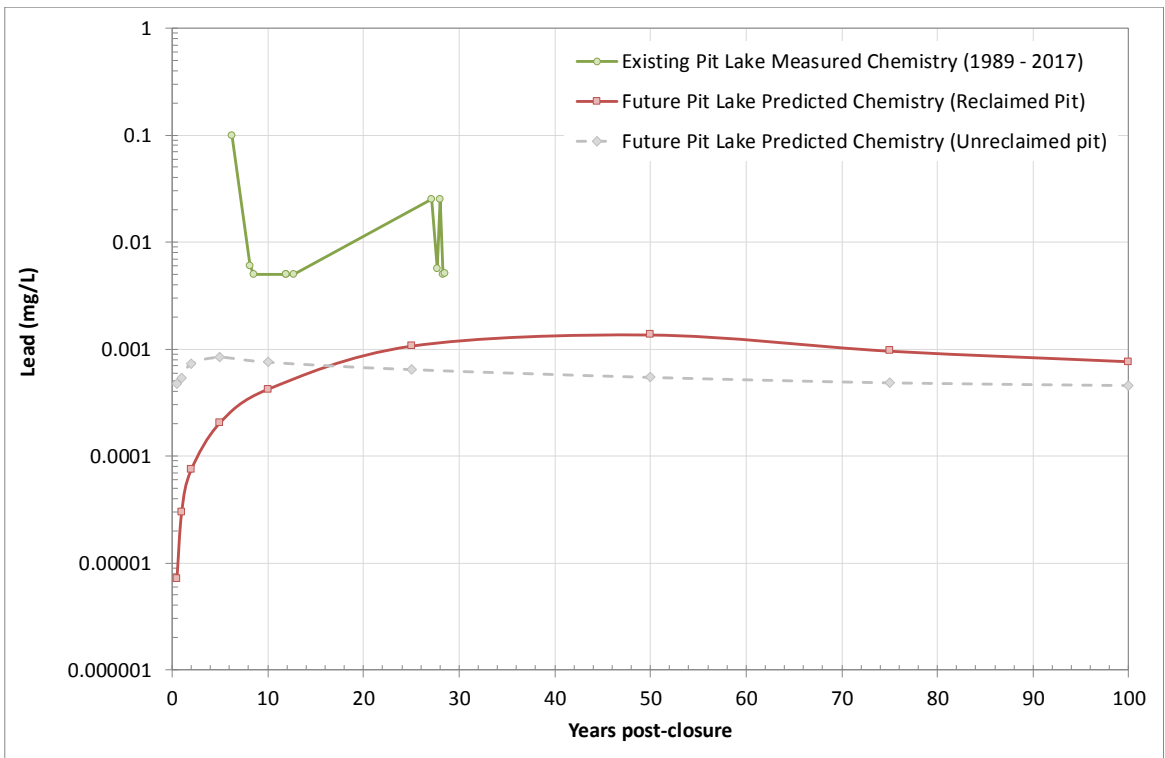
**Figure 6-9: Time-series Plot of Predicted Boron for the Reclaimed Pit Model**



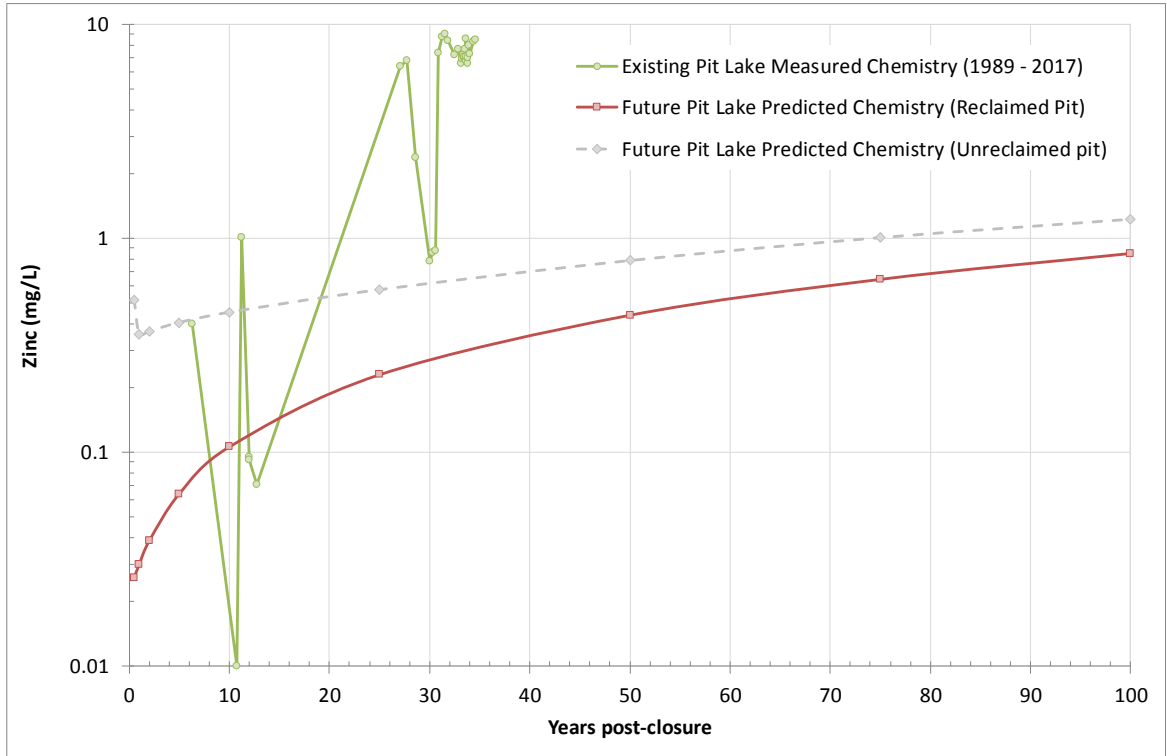
**Figure 6-10: Time-series Plot of Predicted Fluoride for the Reclaimed Pit Model**



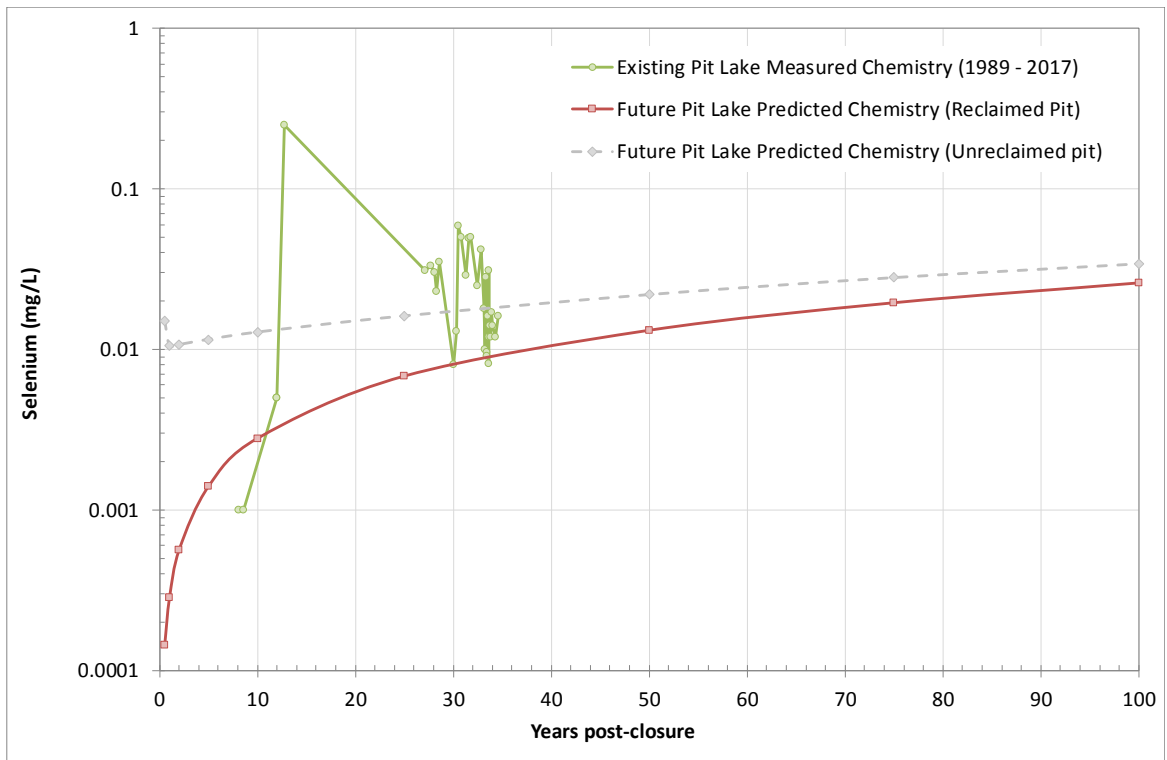
**Figure 6-11: Time-series Plot of Predicted Mercury for the Reclaimed Pit Model**



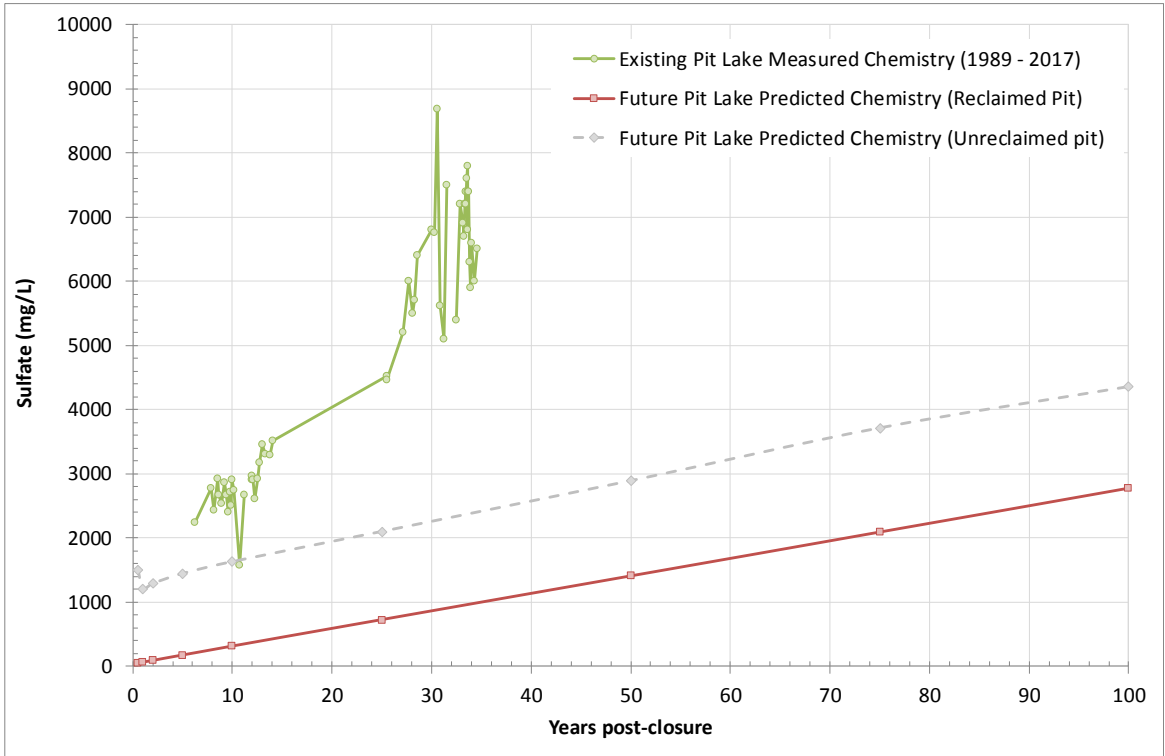
**Figure 6-12: Time-series Plot of Predicted Lead for the Reclaimed Pit Model**



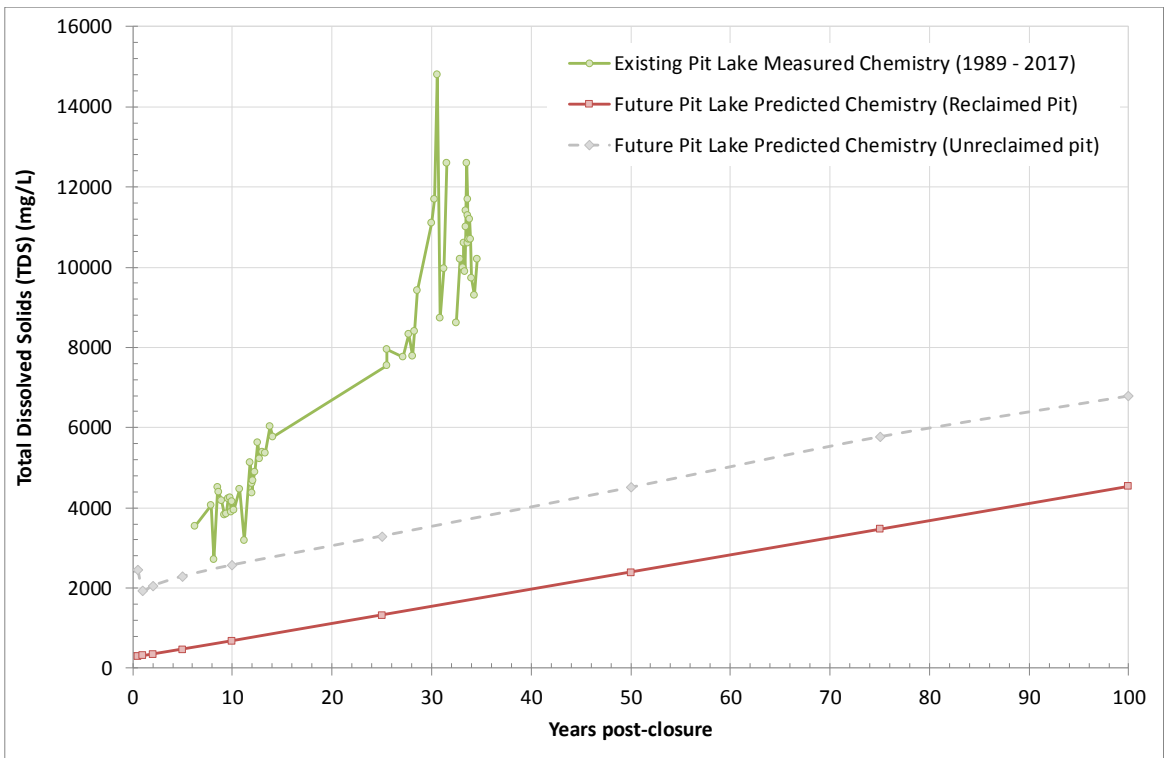
**Figure 6-13: Time-series Plot of Predicted Zinc for the Reclaimed Pit Model**



**Figure 6-14: Time-series Plot of Predicted Selenium for the Reclaimed Pit Model**



**Figure 6-15: Time-series Plot of Predicted Sulfate for the for the Reclaimed Pit Model**



**Figure 6-16: Time-series Plot of Predicted TDS for the for the Reclaimed Pit Model**

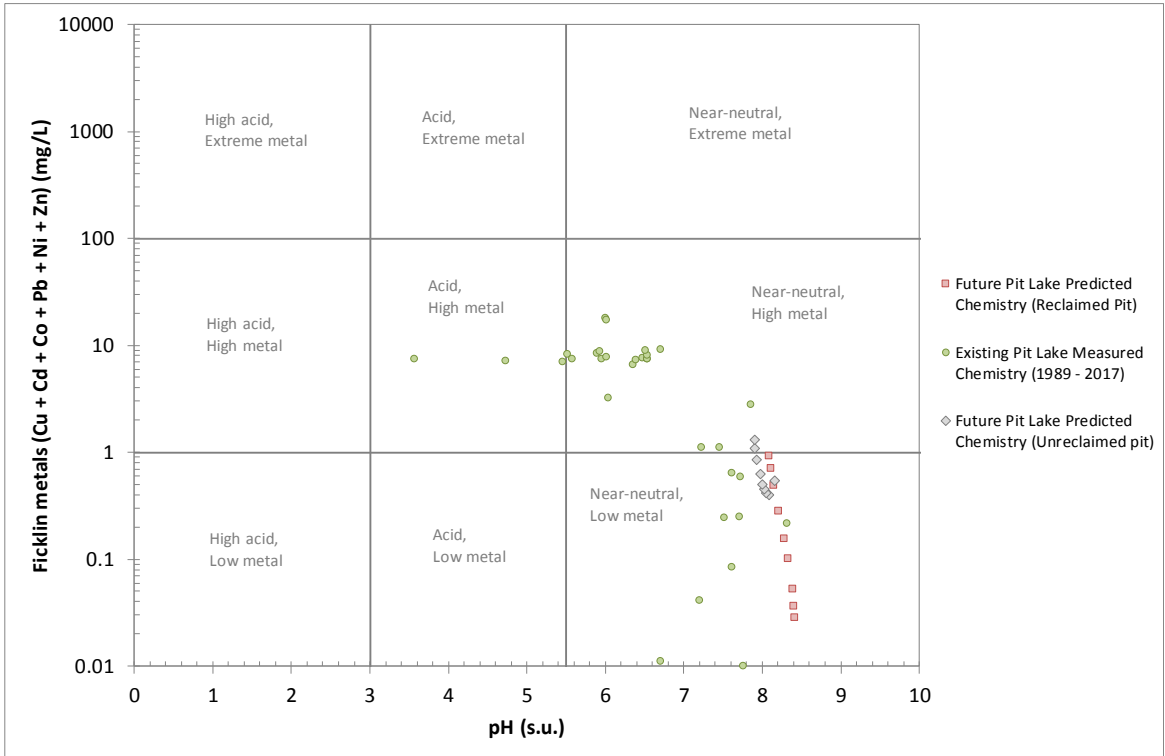


Figure 6-17: Ficklin Plot for the Reclaimed Pit Model

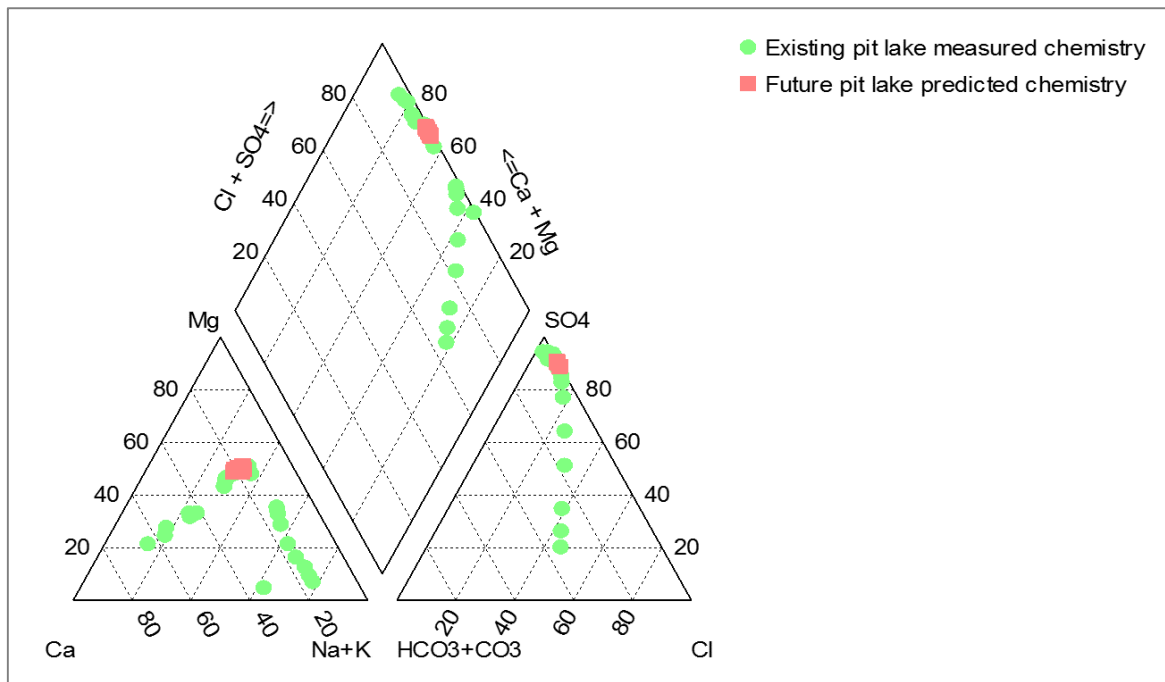


Figure 6-18: Piper Plot of Predicted Major Ion Chemistry for the Reclaimed Pit Model

## 7 Summary and Conclusions

SRK has undertaken a predictive geochemical modeling exercise to assess potential future pit lake chemistry associated with the Copper Flat Project in New Mexico and to compare this to the chemistry of the existing pit lake. The objective of this model and report is to provide the analysis that demonstrates that future pit lake water quality results in a hydrologic balance similar to that of pre-mining conditions upon implementation of the reclamation actions proposed by NMCC in its MORP and Reclamation Plan, including rapid fill of the open pit after closure of the mine.

Geochemical predictions were developed for three scenarios, including: (i) a calibration model for the existing pit lake; (ii) a natural fill model for the future unreclaimed pit; and (iii) a rapid fill model for the future reclaimed pit. Rapid fill has been proposed as the water component of NMCC's reclamation strategy for the future pit lake. It will include filling the pit with 2,202 acre-feet of good quality water from the production water supply wells during the first six months of groundwater recovery and pit infilling.

### 7.1 Model Calibration

The results of the existing pit lake model show good calibration for pH, bicarbonate, calcium, aluminum, cobalt, chromium, copper, mercury, manganese, sodium, nickel, selenium, uranium, zinc and TDS, demonstrating these constituents can be predicted with a good degree of accuracy for the future pit lake. The baseline water quality data utilized in the calibration model are data for existing water quality chemistry in the pit lake between 2010 and 2013, as discussed in Section 4. Model calibration was performed as part of the preliminary pit lake model results presented in the December 2014 report (SRK, 2014a). This is a subset of the entire baseline data generated between 1998 and July 2017. The full data set was utilized in comparing existing water quality chemistry to projected future water quality of the pit lake, as discussed in Sections 5 and 6.

### 7.2 Unreclaimed Fill Scenario

For the unreclaimed fill scenario, allowing the pit to fill naturally will result in the pit walls and benches being exposed over a much longer period of time, i.e., approximately 150 years, before the pit lake reaches hydrologic equilibrium. In the unreclaimed fill scenario, the proposed future Copper Flat open pit is expected to be seasonally stratified but otherwise well-mixed, oxygenated and not acidic. Waters are predicted to be moderately alkaline (pH 7.9 – 8.2), primarily due to the buffering capacity of the inflowing groundwater. During the early stages of pit infilling (i.e., the first six months post-closure), removal/flushing of soluble salts from the pit walls is likely to result in a flush in boron, lead, mercury, manganese, molybdenum, nickel, selenium, vanadium, zinc and sulfate in the early pit lake. The effects of this initial flush will be dissipated by inflowing groundwater and precipitation, and pit lake chemistry will then evolve over time, with some parameters increasing in concentration as a result of evapoconcentration effects. This is similar to the trends observed in the existing pit lake where elemental concentrations have increased since the start of pit infilling. However, the mineralized material to be mined and the future pit walls will be prepared using pre-split drilling and smooth wall blasting. This will reduce the depth of fracturing and oxidation and consequently reduce solute loading to the future pit lake.

A comparison of predicted pit lake water chemistry for the unreclaimed fill scenario to chemistry measured in the existing pit lake between 1989 and 2017 demonstrates that the predicted concentrations of the majority of constituents are comparable to existing concentrations.



### 7.3 Reclaimed Fill Scenario

Rapidly filling the pit with water from the production supply wells during the first six months post-closure will result in a better initial water quality within the pit lake due to the good quality of the water that will be used. The long-term result is that the effects of evapoconcentration will not be as pronounced as the pit lake reaches hydrogeologic equilibrium. Predicted concentrations of many major ions and trace elements remain lower in the reclaimed fill scenario. This is the case for constituents such as boron, sulfate and chloride, which are strongly influenced by evaporation effects and are predicted to be much lower in concentration for the reclaimed pit rapid fill scenario compared to the unreclaimed pit natural fill scenario. In addition, the rapid fill will also quickly submerge walls and benches to limit the exposure of sulfide minerals to oxygen, which will reduce trace element release into the pit lake. By contrast, the unreclaimed fill scenario allows the pit to fill naturally and results in the pit walls and benches being exposed over a much longer period of time, i.e., approximately 150 years, before the pit lake reaches hydrologic equilibrium. As is the case in the unreclaimed fill scenario, the mineralized material to be mined and the future pit walls will be prepared using pre-split drilling and smooth wall blasting, which will also reduce the depth of fracturing and oxidation and consequently reduce solute loading to the pit lake.

A comparison of predicted pit lake chemistry for the reclaimed pit rapid fill scenario to chemistry measured in the existing pit lake between 1989 and 2017 demonstrates that concentrations of the majority of predicted constituent concentrations are either comparable to or less than concentrations in the existing pit lake.

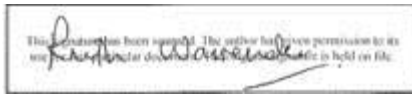
### 7.4 Conclusions

Standards applicable to the post-mining Copper Flat pit lake are contained in the New Mexico Mining and Minerals Division (MMD) regulations administered under the Mining Act. Specifically, the performance and reclamation standards require that reclamation must result in a hydrologic balance similar to pre-mining conditions. With respect to water quality in the pit lake, post mining water quality must be similar to baseline pre-mining water quality in the pit lake. The predictive geochemical model results presented herein have been compared to pre-mining baseline water quality of the existing pit lake, which has been in existence for more than 35 years.

Based on the model results presented herein, the changes to the hydrologic balance of the future pit water body that will form post-mining will be nil or minimal, and the water quality will be very similar to that of the existing pit lake. As noted above, the existing pit lake at Copper Flat is an artificial water body created as a result of mineral extraction that has little or limited ability to sustain aquatic life (Aquatic Consultants, Inc. 2014). The post-mining water body is anticipated to be similar to the existing pit lake and is not expected to be conducive to providing aquatic habitat or supporting fish life.

This report demonstrates that implementation of either the unreclaimed fill or reclaimed fill scenario will provide compliance with water quality requirements discussed in Section 3.10 above. However, the reclaimed fill scenario leads to improved water quality during the modeled period. In addition, the overall performance and reclamation standards and requirements of the Mining Act regulations set forth additional standards, beyond those which are the subject of analysis in this report. In this regard, NMCC has committed to the reclamation plan as described in the MORP, including the pit reclamation measures outlined in Section 3.1.8 of this report.

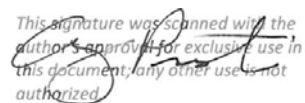
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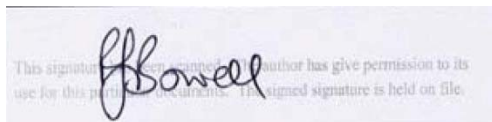


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## **Appendix A – Time-Series Plots of Existing Pit Lake Chemistry**

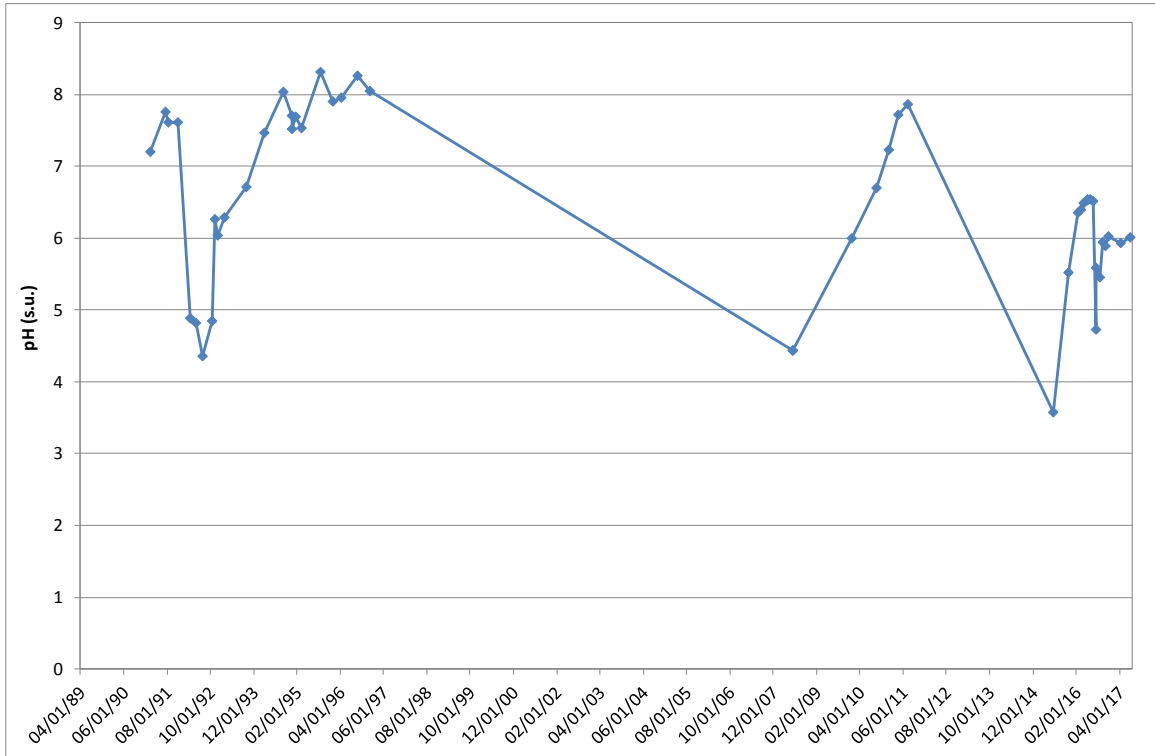


Figure A-1: pH Trends in Existing Pit Lake

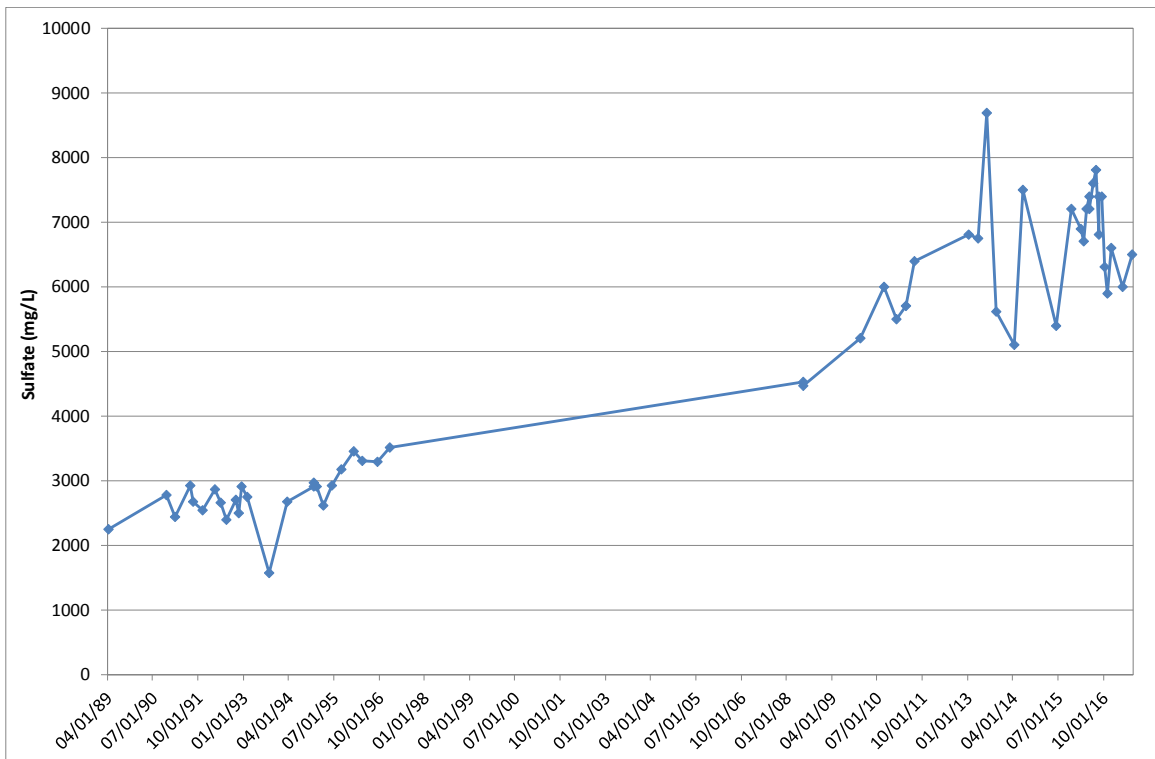
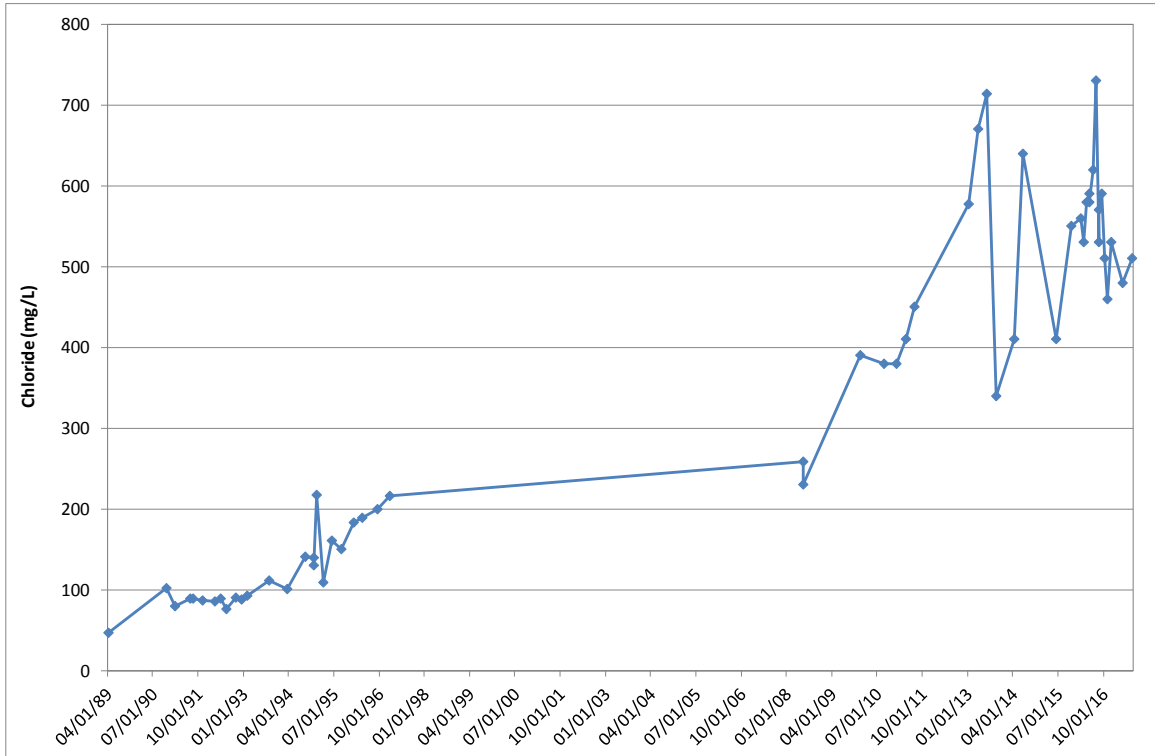
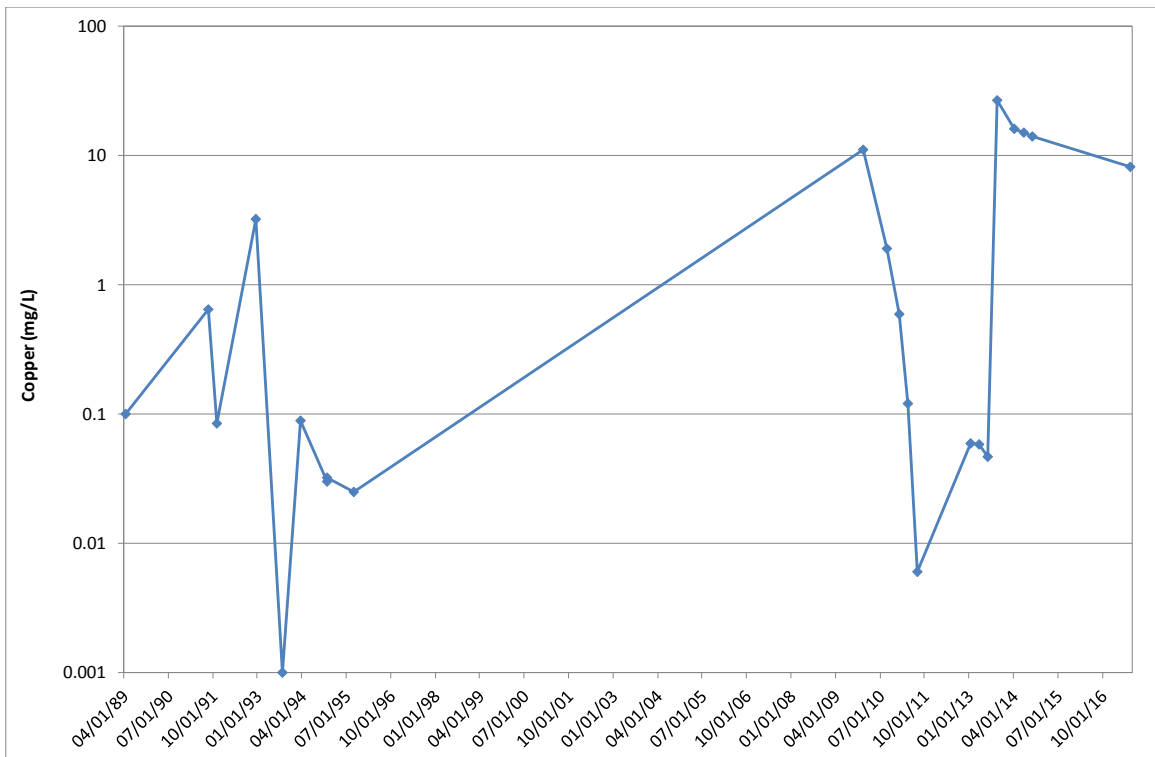


Figure A-2: Sulfate Trends in Existing Pit Lake





**Figure A-3: Chloride Trends in Existing Pit Lake**



**Figure A-4: Copper Trends in Existing Pit Lake**

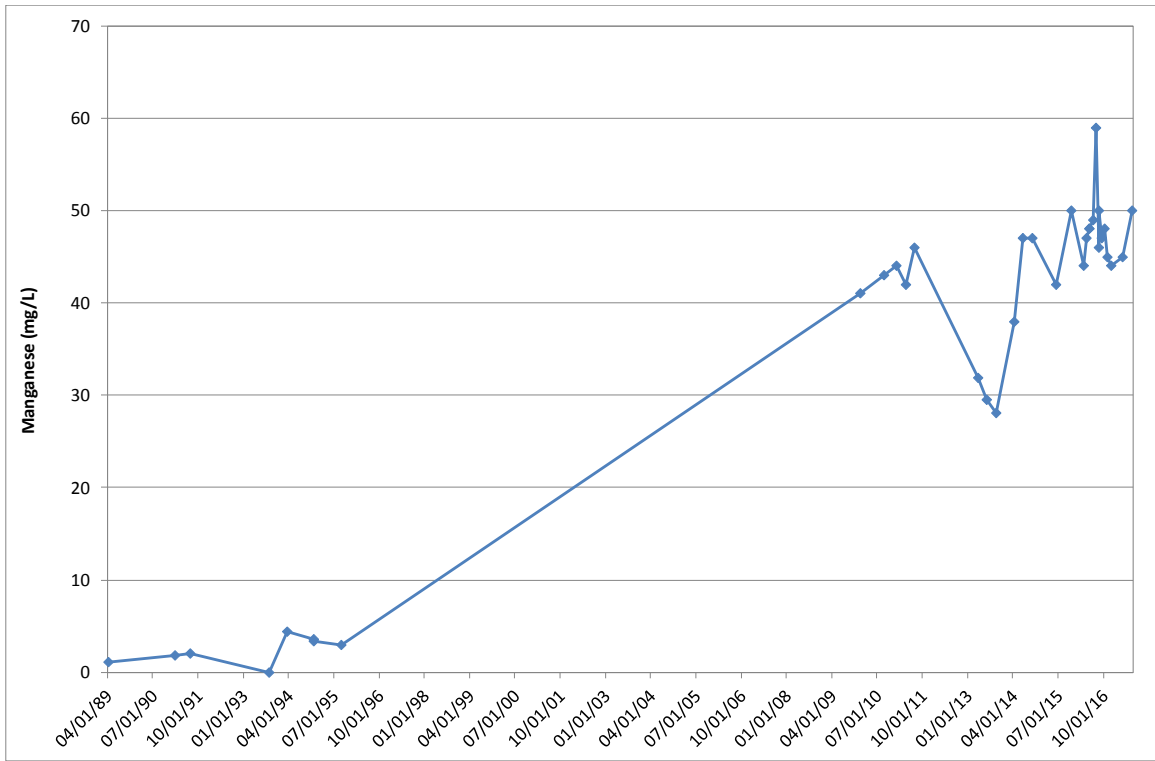


Figure A-5: Manganese Trends in Existing Pit Lake

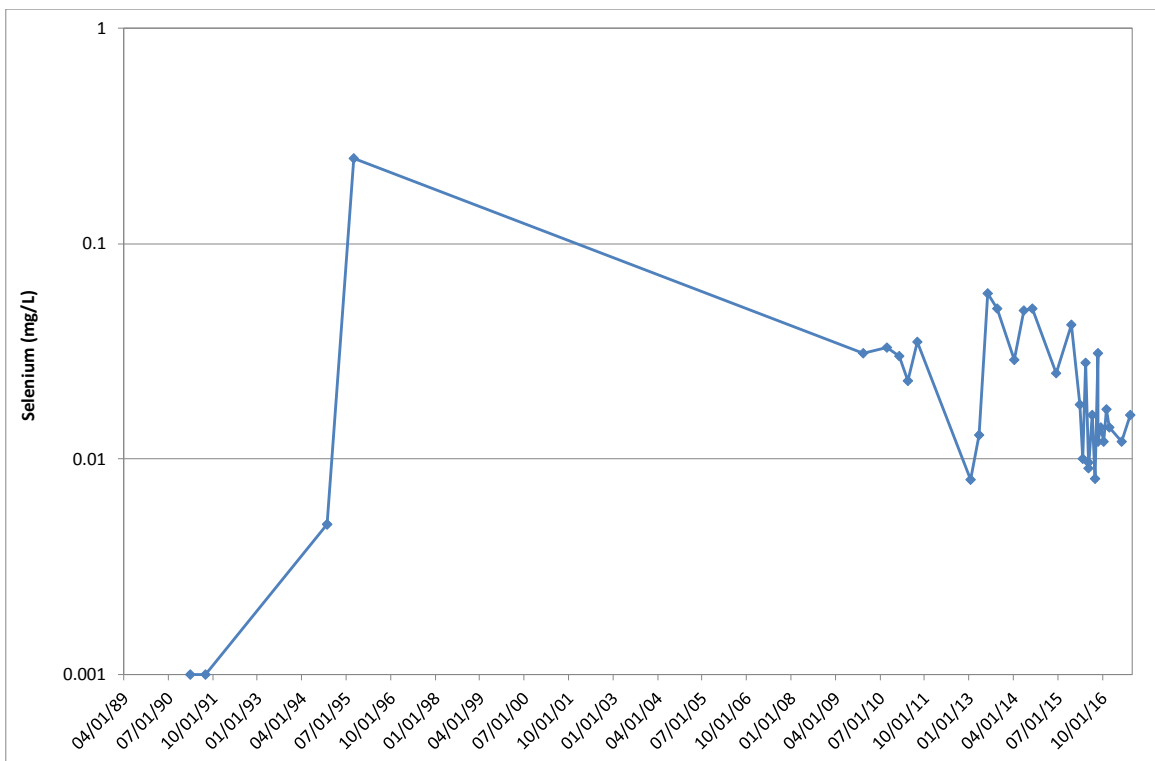
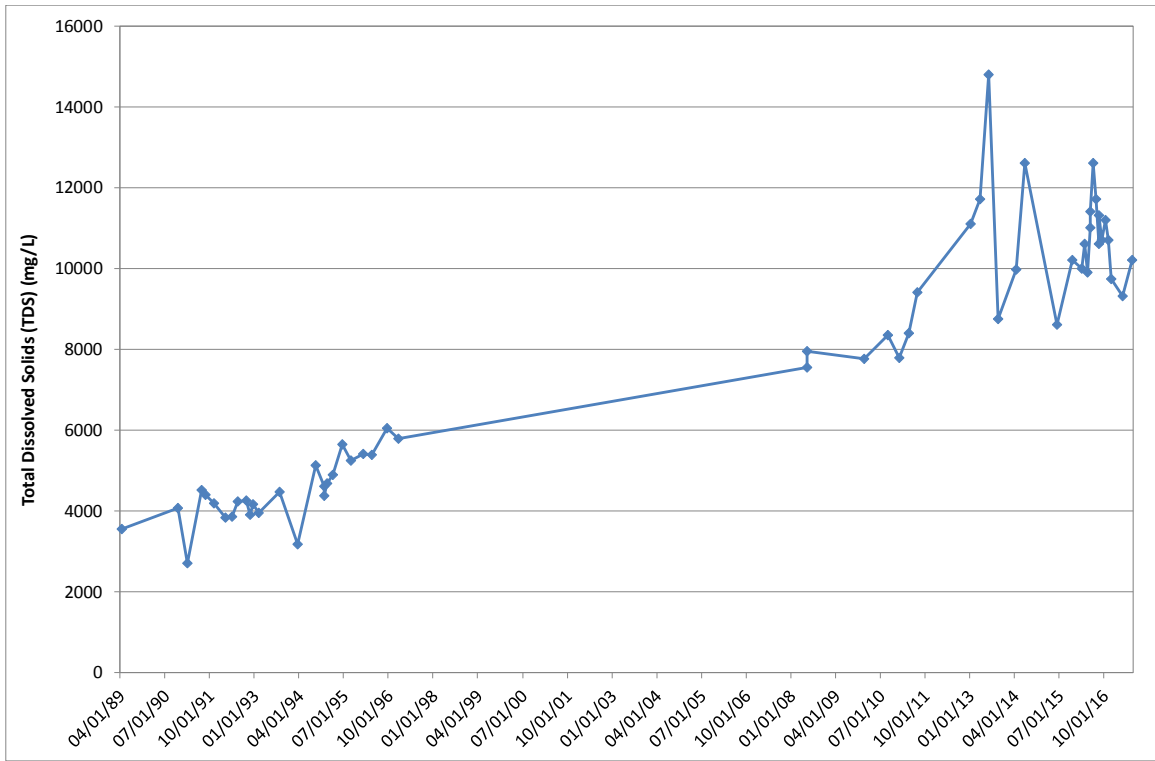
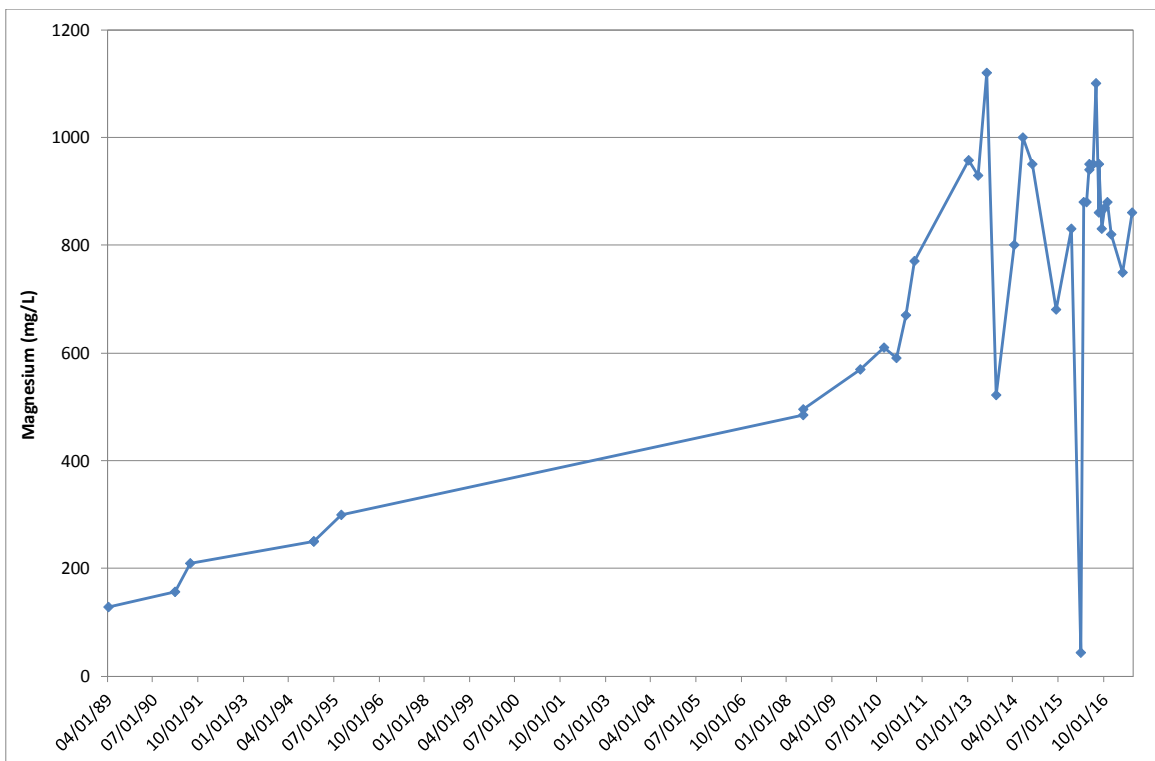


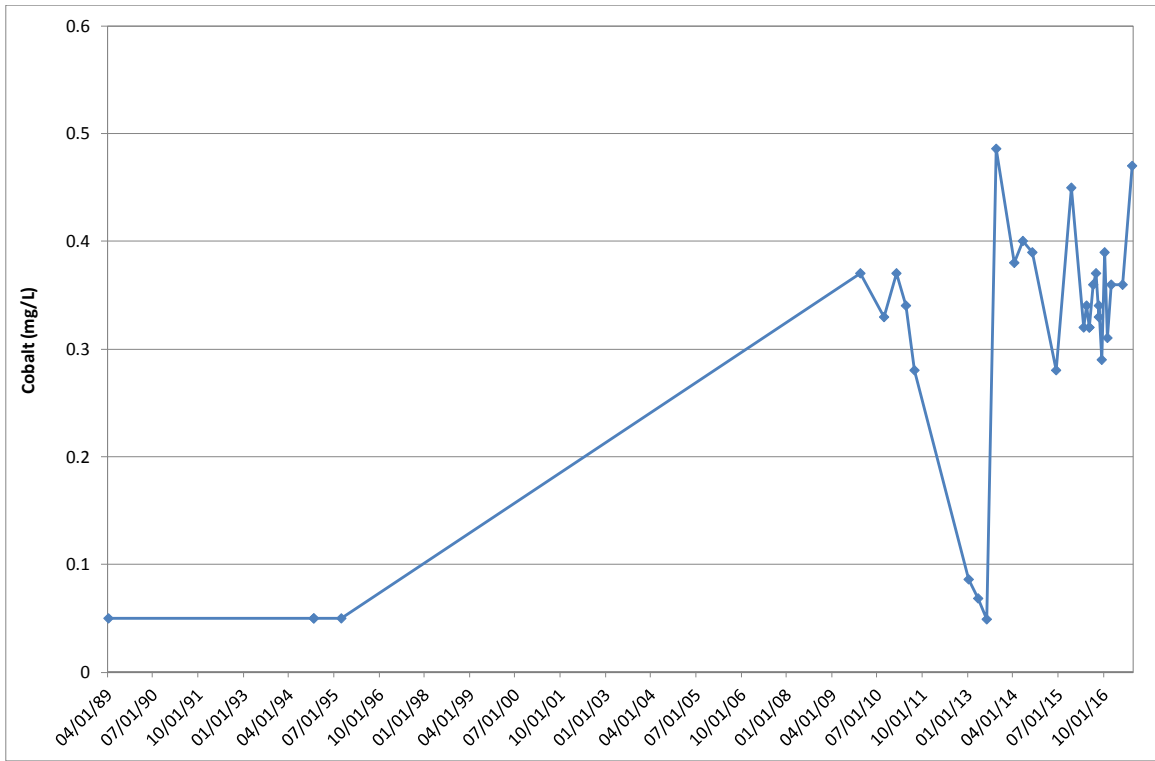
Figure A-6: Selenium Trends in Existing Pit Lake



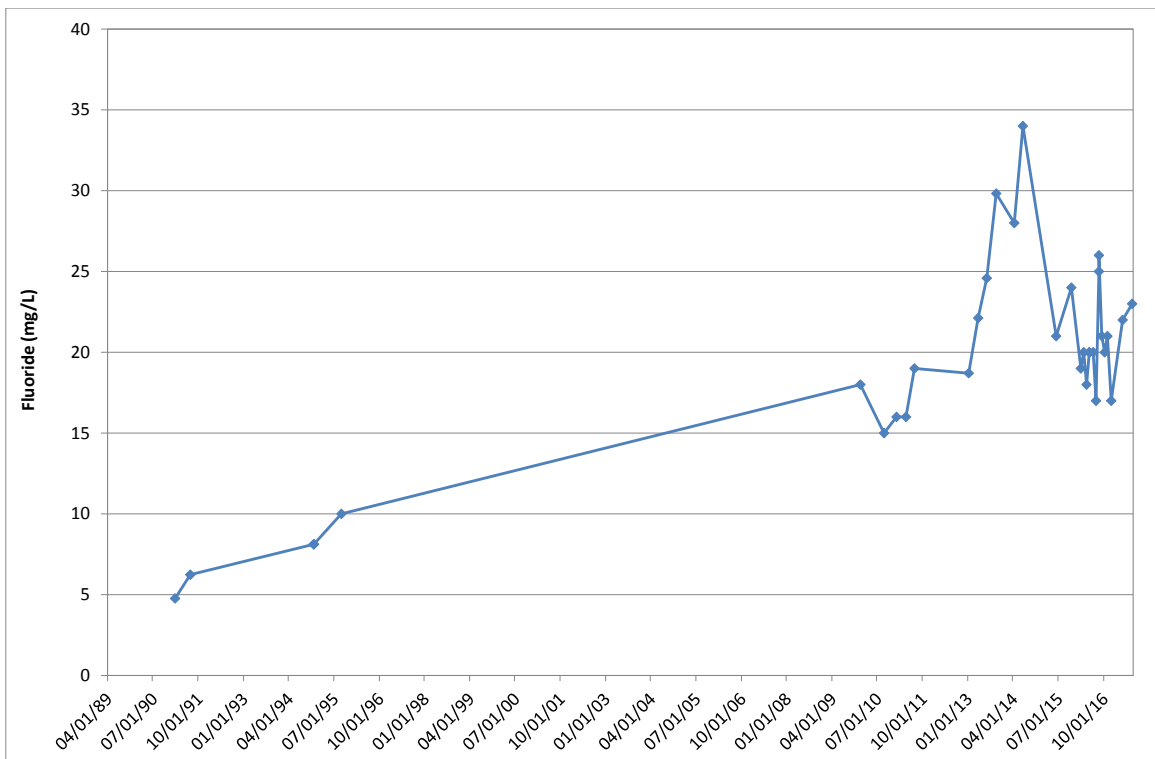
**Figure A-7: TDS Trends in Existing Pit Lake**



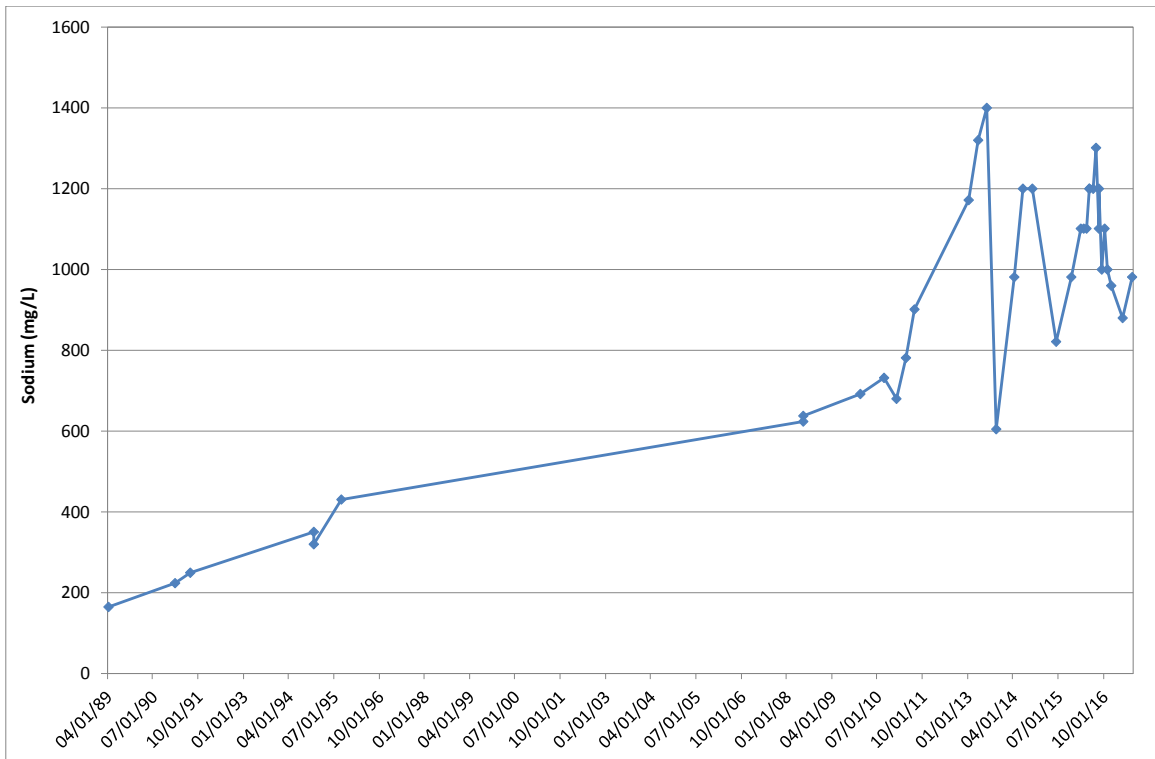
**Figure A-8: Magnesium Trends in Existing Pit Lake**



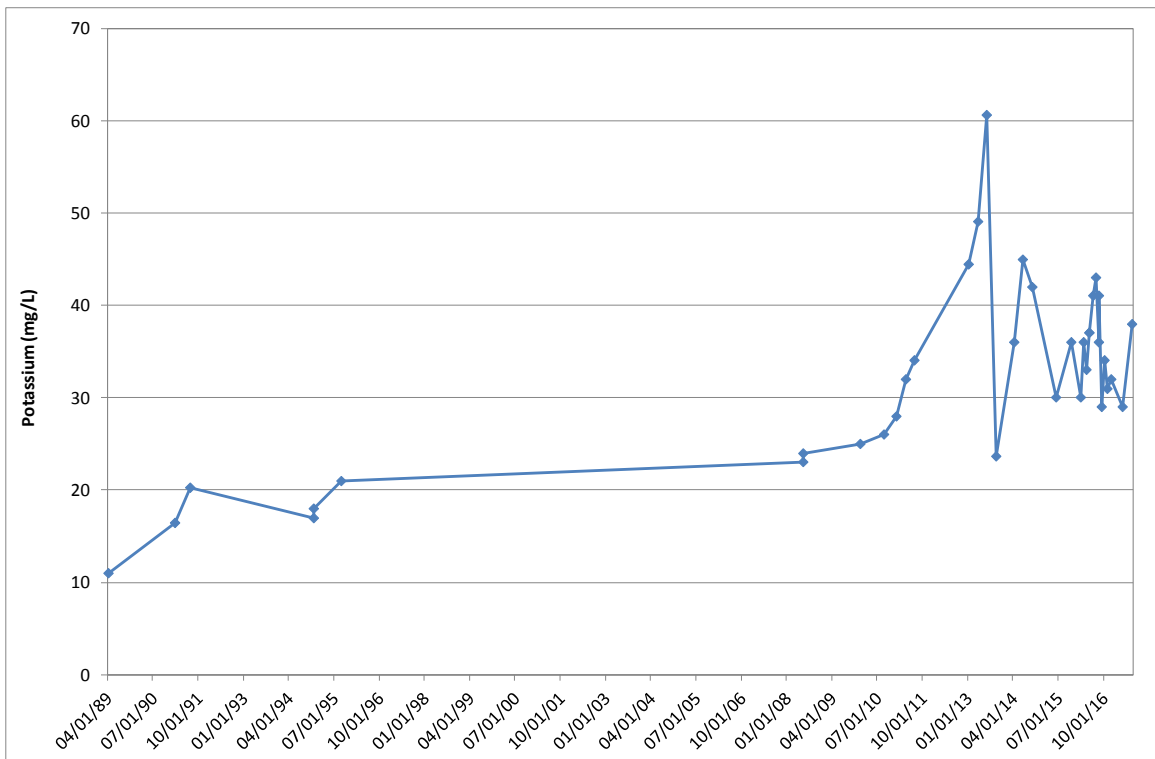
**Figure A-9: Cobalt Trends in Existing Pit Lake**



**Figure A-10: Fluoride Trends in Existing Pit Lake**



**Figure A-11: Sodium Trends in Existing Pit Lake**



**Figure A-12: Potassium Trends in Existing Pit Lake**

## **Appendix B – Humidity Cell Elemental Release Rate Graphs**

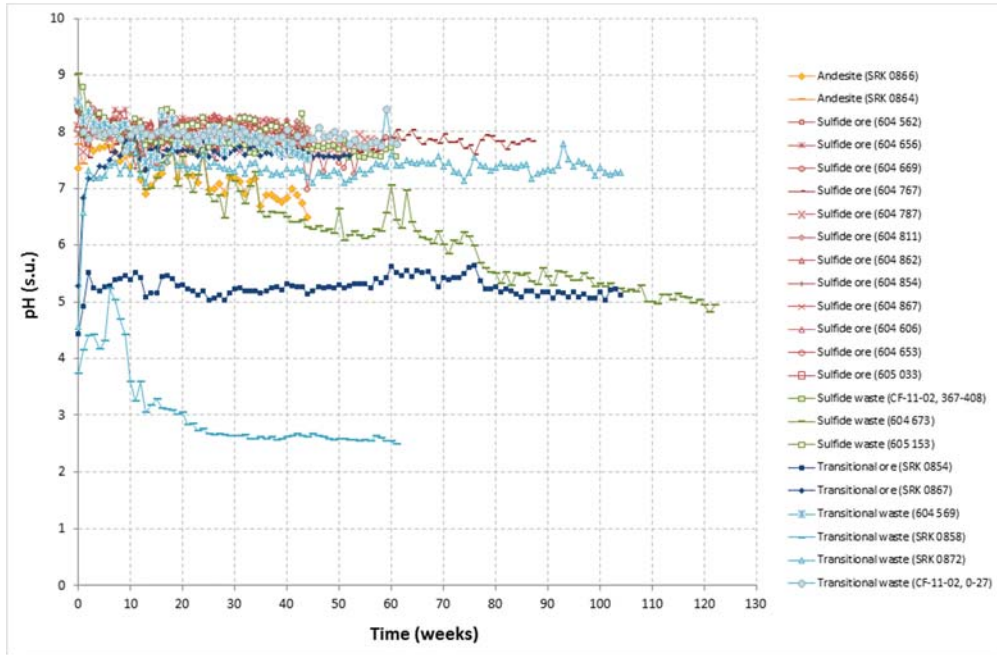


Figure B-1: Humidity Cell Effluent pH

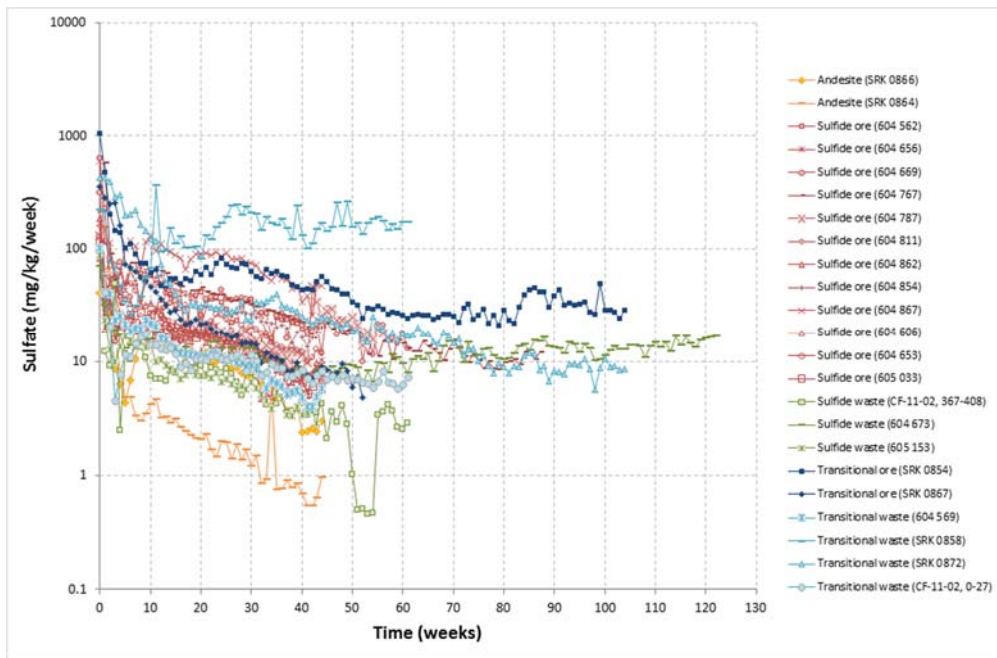


Figure B-2: Humidity Cell Effluent Sulfate

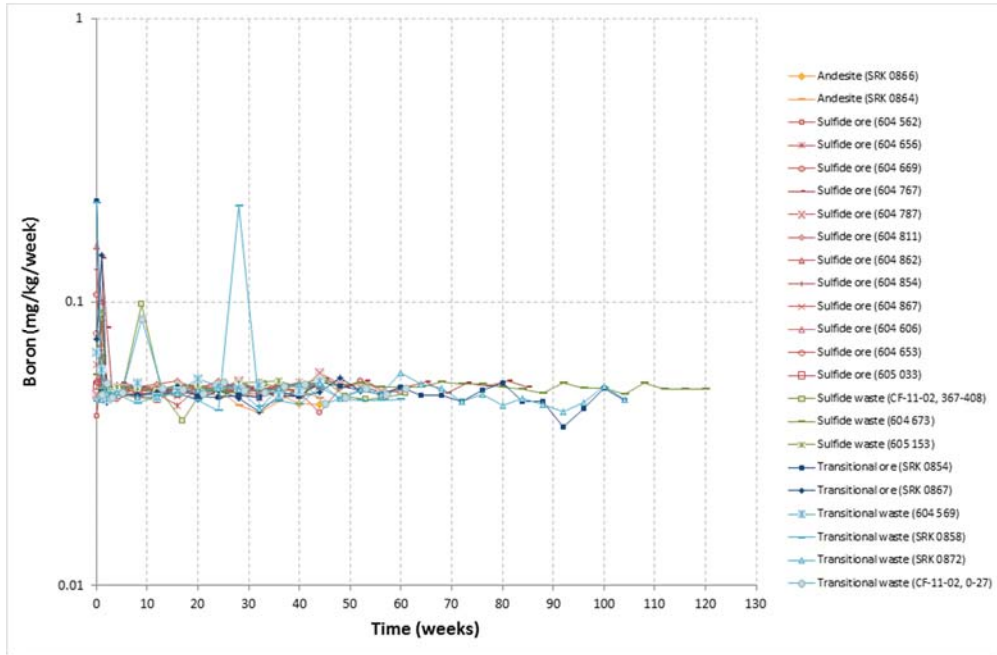


Figure B-3: Humidity Cell Effluent Boron

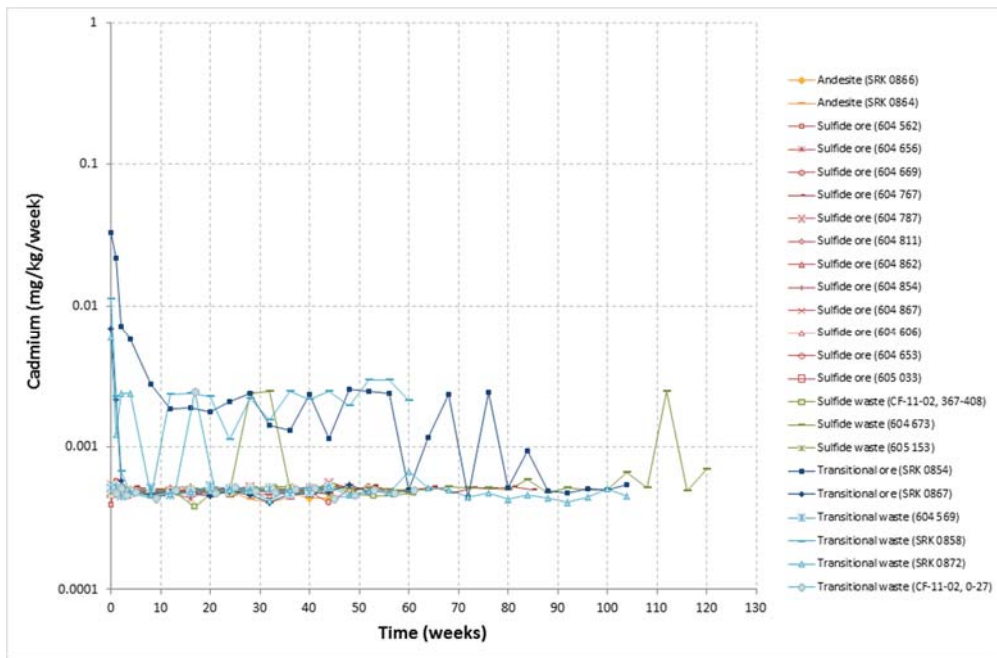


Figure B-4: Humidity Cell Effluent Cadmium



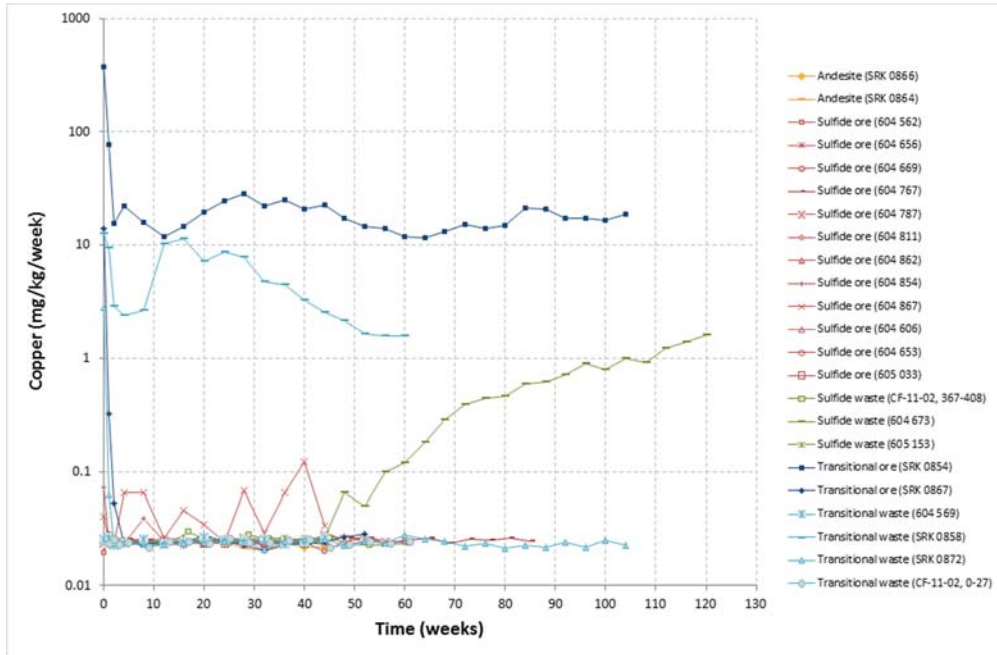


Figure B-5: Humidity Cell Effluent Copper

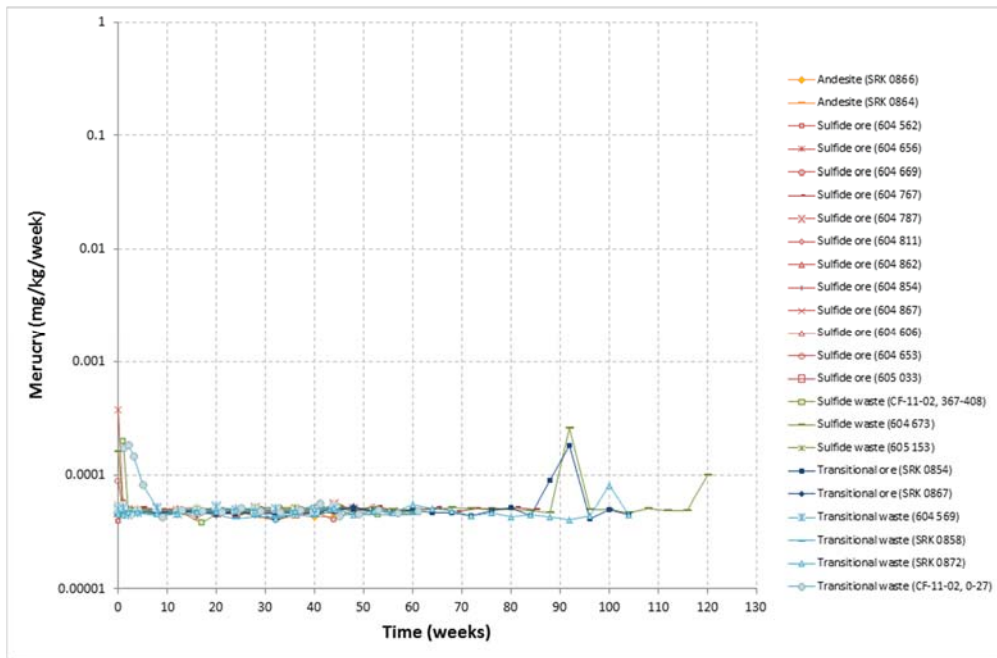


Figure B-6: Humidity Cell Effluent Mercury

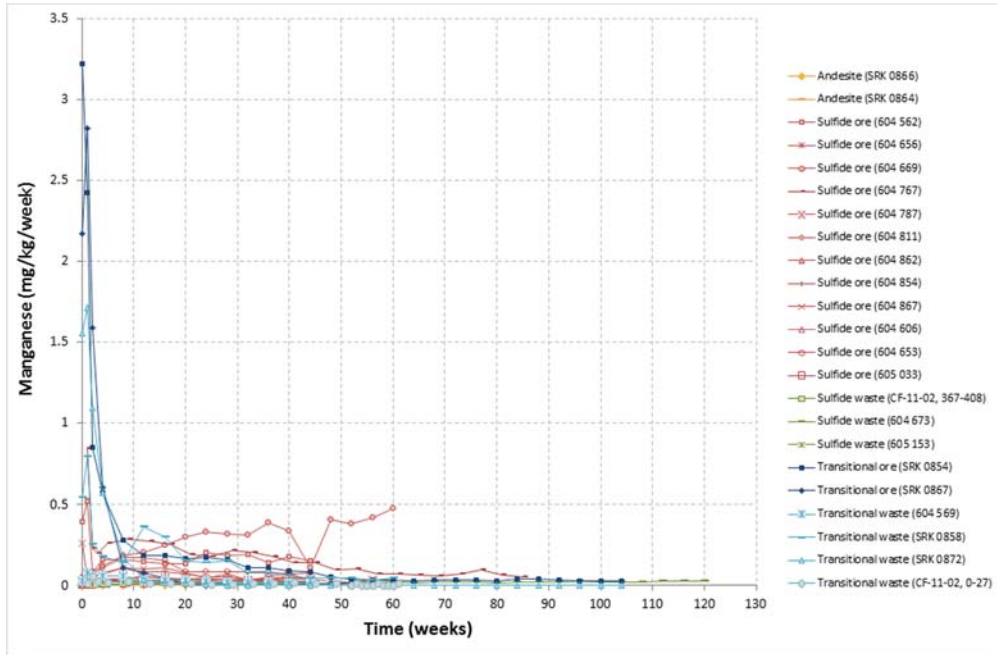


Figure B-7: Humidity Cell Effluent Manganese

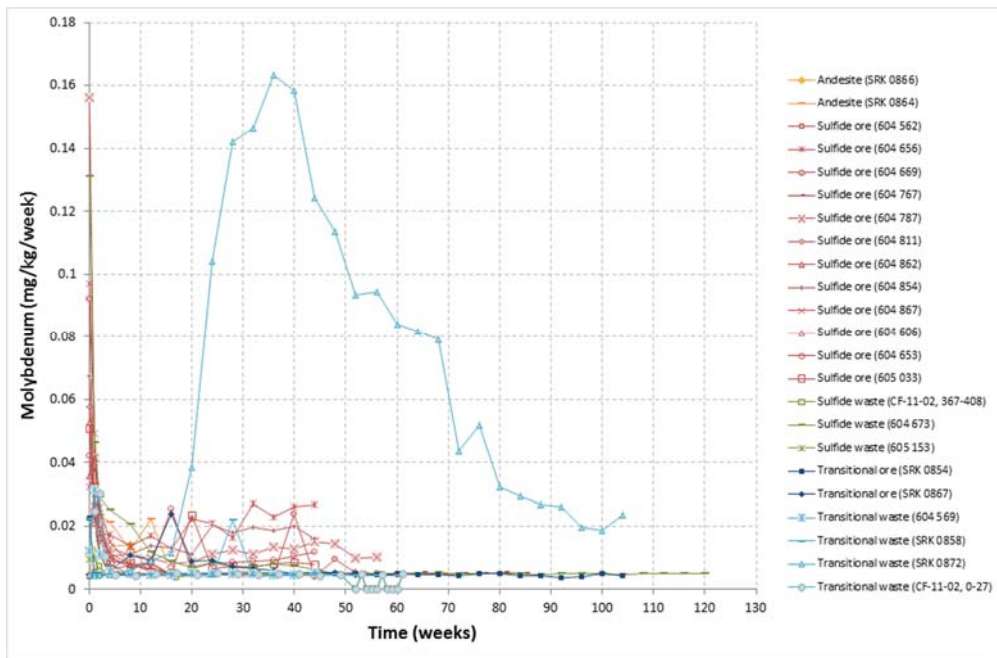


Figure B-8: Humidity Cell Effluent Molybdenum

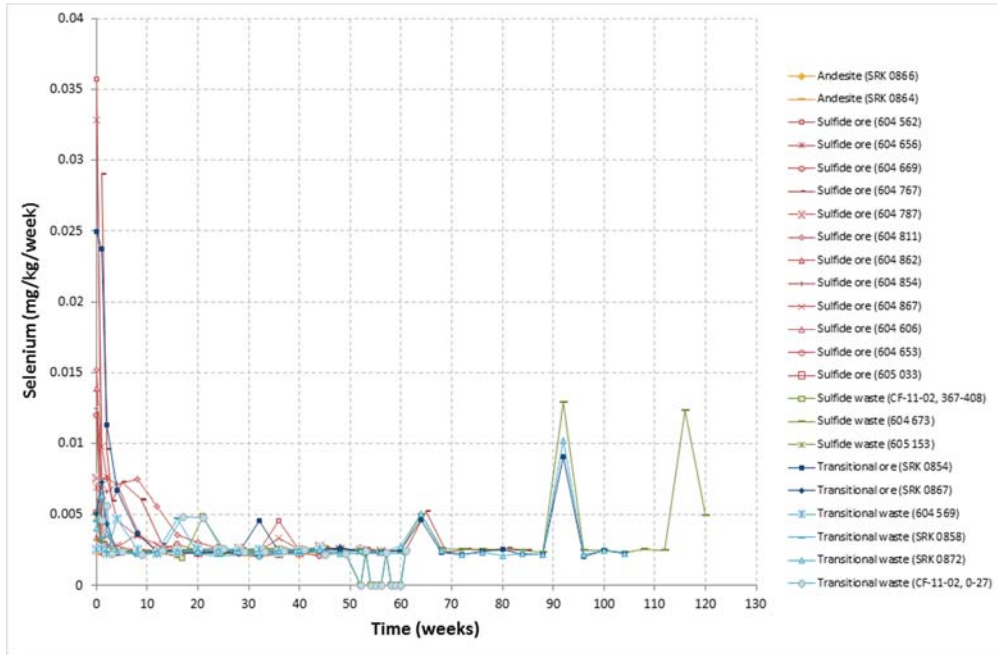


Figure B-9: Humidity Cell Effluent Selenium

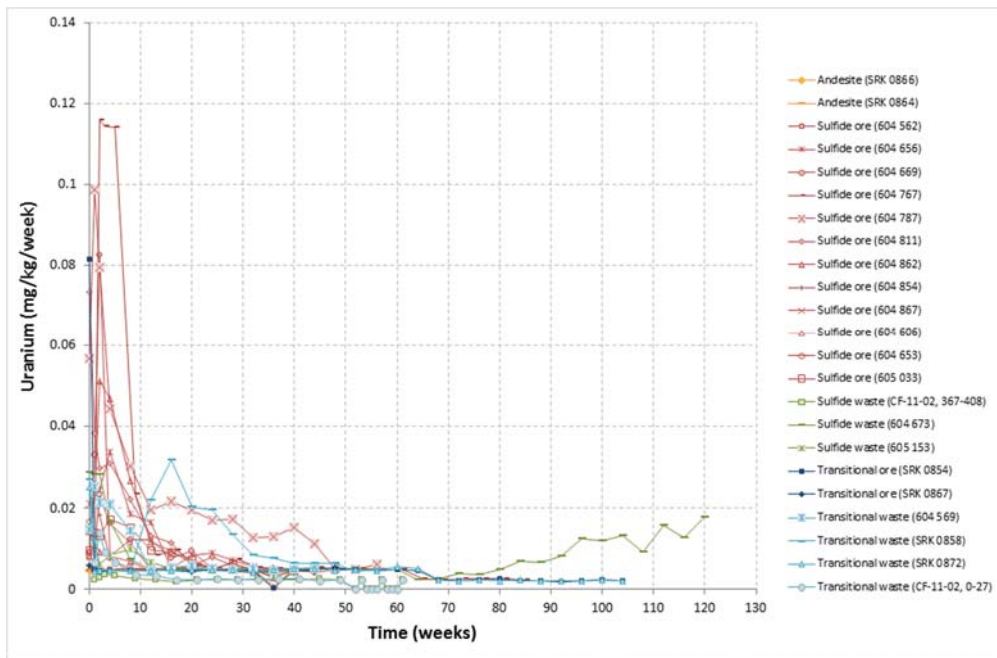


Figure B-10: Humidity Cell Effluent Uranium

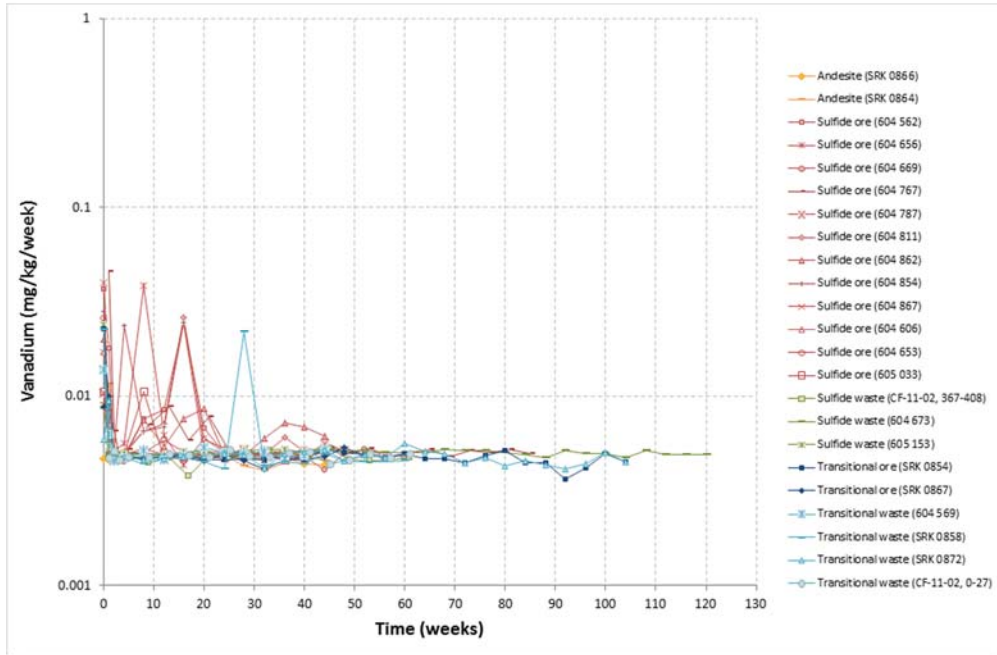


Figure B-11: Humidity Cell Effluent Vanadium

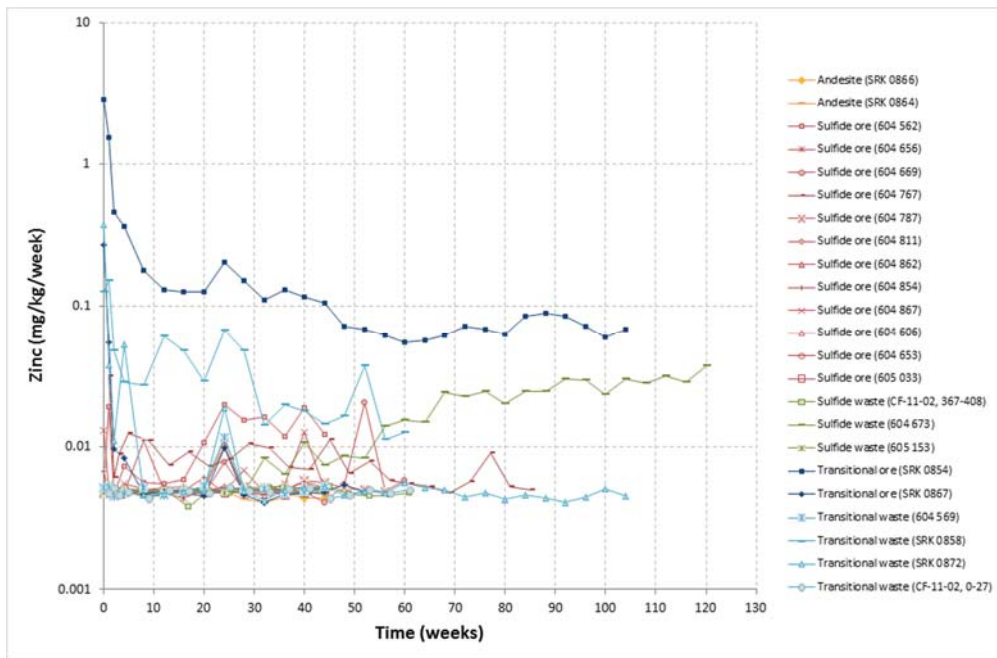


Figure B-12: Humidity Cell Effluent Zinc

## **Appendix C – JSAI Evaporation Rate Technical Memorandum**



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## TECHNICAL MEMORANDUM

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To: Steve Raugust, JS Raugust Consulting  
Jeff Smith, New Mexico Copper Corporation

From: Steven T. Finch, Jr., Principal Hydrogeologist-Geochemist, JSAI  
Annie McCoy, Senior Hydrogeologist, JSAI

Date: September 1, 2015

Subject: **Estimated evaporation rate for future Copper Flat open pit**

---

As discussed in the Copper Flat Project groundwater-flow model report (JSAI, 2014), potential evapotranspiration (ET), or the maximum evaporation and plant transpiration that can occur given full availability of water, is a function of geographical and climatic conditions, and is commonly estimated using the Penman-Monteith equations (Monteith, 1965). These relate maximum ET ( $ET_0$ ) to meteorological parameters including temperature, relative humidity and wind speed, and to geographical parameters (altitude, latitude, and time of year). Annual  $ET_0$  computed from results at Hillsboro meteorological station is about 60 in./yr, which compares well to previous estimates (SRK, 1997) of 65 in./yr of potential evaporation, and 64.6 in./yr estimated as 74 percent (an accepted conversion factor for the region (NOAA, 1982) between pan evaporation and evaporation from a normal open water surface) of Copper Flat pan evaporation. Actual evaporation or ET is less, depending on sun and wind exposure, ground conditions, and availability of water.

If  $ET_0$  is estimated to be 60 to 65 in./yr at the rim of the ultimate Copper Flat open pit (where the prior land surface intersects the open pit),  $ET_0$  will be somewhat less at the bottom of the ultimate open pit due to the fact that the bottom of the pit will have less exposure to sun and wind compared to the rim.

To estimate  $ET_0$  for the bottom of the ultimate Copper Flat open pit, the duration of sunlight at analogous established open pits was evaluated using the “sunlight across the landscape” tool in Google Earth, for the date April 29, 2015. April is a month with close-to-average duration of sunlight (as are the months of March, September, and October; Dunne and Leopold, 1978). Table 1 presents a summary of hours of sunlight for analogous pits ranging in depth from 300 to 1,400 ft.

**Table 1. Summary of hours of sunlight for selected open pits  
 in New Mexico and California, April 29, 2015**

<b>pit</b>	<b>rim elevation, ft amsl</b>	<b>bottom elevation, ft amsl</b>	<b>sunlight at rim, hours</b>	<b>sunlight at bottom, hours</b>	<b>bottom / rim sunlight ratio</b>
Cobre pit, SW NM	6,800	6,300	6:30 to 19:30 = 13 hours	9:30 to 18:30 = 9 hours	0.69
Santa Rita pit, SW NM	6,600	5,200	7:00 to 19:50 = 12.5 hours	9:30 to 16:30 = 7 hours	0.56
Tyrone main pit, SW NM	6,200	4,900	6:30 to 19:30 = 13 hours	8:30 to 17:30 = 9 hours	0.69
CHMRP pit, N. NM	7,100	6,800	7:30 to 19:30 = 12 hours	8:45 to 16:00 = 7.25 hours	0.60
Colosseum pit, S. CA	5,800	5,400	8:00 to 19:00 = 11 hours	9:00 to 16:00 = 7 hours	0.64
<b>average</b>			<b>12.3 hours</b>	<b>7.85 hours</b>	<b>0.64</b>

CHMRP – Cunningham Hill Mine Reclamation Project  
 ft amsl – feet above mean sea level  
 SW – southwest  
 N. – north  
 S. – south

Pan evaporation data were collected at the Cunningham Hill Mine Reclamation Project (CHMRP), near the rim of the open pit in June 2000, and at the bottom of the pit between April and July 2011 (JSAI, 2011). Pan evaporation was higher at the rim, despite higher summer precipitation in 2001 compared to 2011. The pan evaporation data were interpreted to represent an average evaporation rate of about 60 in./yr at the rim, and 54 in./yr at the bottom.

CHMRP evaporation data were used for an upper bound of 90 percent, in terms of percentage of evaporation at the rim that represents actual evaporation at the bottom of the pit, and the average sunlight ratio presented in Table 1 was used for a lower bound of 64 percent. For the ultimate Copper Flat open pit, actual evaporation at the bottom of the pit was assumed to be 50 in./yr, which is 77 to 83 percent of ETo values 60 to 65 in./yr estimated at the rim.

The estimate of 50 in./yr evaporation for the ultimate Copper Flat open pit is also in close agreement with the estimate of open water evaporation of 53 in./yr for the North Mine Area (Santa Rita pit) at Chino Mine in southwestern New Mexico (Golder, 2005).

STF:AMM

Enc: References

## References

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- [Golder] Golder Associates, Inc., 2005, *Report on North Mine Area groundwater flow model: Chino Mine, New Mexico: consultant's report prepared for Chino Mines Company*, January 13, 2005, 64 p. plus tables, figures, and appendices.
- [JSAI] John Shomaker & Associates, Inc., 2011, *Update and recalibration of groundwater-flow and solute-transport model for predicting potential effects from the Cunningham Hill Mine Open Pit, Santa Fe County, New Mexico: consultant's report prepared for LAC Minerals (USA) LLC*, June 27, 2011, 29 p. plus figures and appendices.
- [JSAI] John Shomaker & Associates, Inc., 2014, *Model of groundwater flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico: consultant's report prepared for New Mexico Copper Corporation*, August 15, 2014, 89 p. plus figures and appendices.
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## **Appendix D – JSAI Groundwater Chemistry Technical Memorandum**



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## TECHNICAL MEMORANDUM

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To: Jeff Smith, New Mexico Copper Corporation [jsmith@themacresourcesgroup.com](mailto:jsmith@themacresourcesgroup.com)

From: Steve Finch, Principal Hydrogeologist-Geochemist

Date: September 26, 2017

Subject: Copper Flat open pit area groundwater chemistry data and application to SRK geochemistry model

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John Shomaker & Associates, Inc. (JSAI) has evaluated the water quality data regarding Copper Flat open pit influent groundwater chemistry in order to assist SRK with completion of the open pit geochemistry model. All historical data and the Stage 1 abatement data were compiled and reported in JSAI (2014). JSAI used the water quality database, well construction data, and groundwater flow model results to determine the most representative groundwater flow chemistry to the existing and future open pits.

Groundwater quality data for the open pit area come from wells GWQ96-22(A,B), GWQ96-23(A,B), GWQ11-24(A,B), and GWQ11-25(A,B). Monitoring wells GWQ96-22(A,B) and GWQ96(A,B) represents groundwater in the andesite, where monitoring wells GWQ11-24(A) represents groundwater in the quartz monzonite ore body, and GWQ11-24(B) and GWQ11-25(B) represent parts of the quartz monzonite with lower grade of the ore body. Piezometers GWQ11-24(A) and GWQ11-25(A) may have been affected by oxidation of sulfides in fractures during well development, and not representative of groundwater reporting to the open pit. Further analysis of GWQ11-25(A) provided evidence that it represents a localized and isolated fracture system recharged by oxygenated meteoric water that is not connected to the open pit (JSAI, 2014).

### **Existing Open Pit Influent Groundwater Chemistry**

Table 1 is a summary of groundwater chemistry potentially influencing the existing open pit. Individual samples with values less than detection limits were assigned a value of one-half the detection limit. Results for selenium, mercury, and vanadium were evaluated for the lowest possible detection limit. Not all of the constituents analyzed in the baseline data report were analyzed as part of the Stage 1 abatement investigation, so results for GWQ11-24(A,B) and GWQ11-25(A,B) are limited by the Stage 1 constituent list (see Table 1).

### **Future Post-Mining Open Pit Influent Groundwater Chemistry**

Based on the mining plan, a good portion of the quartz monzonite is removed by mining and the remaining quartz monzonite is dewatered. The groundwater flow model simulates localized dewatering rates and volumes (JSAI, 2014a). Groundwater representative of the andesite rocks reports to the future pit, and all of the groundwater in the quartz monzonite surrounding the future pit is dewatered during mining and replaced with groundwater from the surrounding andesite (JSAI, 2014a). The calculated volume of groundwater in the quartz monzonite is removed and flushed three times by inflow of groundwater representative of andesite. A volume of 500 acre feet is calculated to be dewatered during mining of the proposed open pit of which 165 ac-ft represents groundwater stored in quartz monzonite.

A summary of groundwater chemistry potentially influencing the future open pit during post mining conditions is listed in Table 1. Groundwater chemistry representative of the future pit was determined by using data representative of the andesite rocks (column A). These “Column A” sample results represent groundwater from the andesite rocks after dewatering and mining to create the future pit.

### **Attachments**

Table 1. Summary of groundwater chemistry for Copper Flat open pit area

### **References**

- JSAI, 2014, Results of first year of Stage 1 investigation at the Copper Flat Mine Site, Hillsboro, New Mexico: Consultant’s report prepared by Steven T. Finch of John Shomaker & Associates, Inc. for New Mexico Copper Corporation.
- [JSAI] John Shomaker & Associates, Inc., 2014a, Model of groundwater flow in the Animas Uplift and Palomas Basin, Copper Flat Project, Sierra County, New Mexico: consultant’s report prepared for New Mexico Copper Corporation, August 15, 2014, 89 p. plus figures and appendices.

**Table 1. Summary of Copper Flat open pit influent groundwater chemistry**

Column:			A	B	C	AVERAGE A-C	
parameter	parameter name	unit	Groundwater chemistry (average of samples collected from wells GWQ96-22(A,B), GWQ96-23(A,B) between 1996 and 2013)	GWQ11-24B 2013 average	GWQ11-25B 2013 average	Blended Groundwater chemistry representative of inflow to current open pit <sup>b</sup>	Groundwater chemistry representative of inflow to future open pit
pH	pH	s.u	7.85	6.44	6.45	6.91	7.85
HCO3	bicarbonate	mg/L	408	191	350	316.3	408
Ag	silver	mg/L	0.009	nm	nm	0.009	0.009
Al	aluminum	mg/L	0.029	0.013	0.308	0.12	0.029
As	arsenic	mg/L	0.0023	nm	nm	0.0023	0.0023
B	boron	mg/L	0.136	nm	nm	0.136	0.136
Ba	barium	mg/L	0.089	nm	nm	0.089	0.089
Ca	calcium	mg/L	85.8	442	481	336	85.8
Cd	cadmium	mg/L	0.0008	0.001	0.001	0.001	0.0008
Co	cobalt	mg/L	0.008	0.017	0.004	0.010	0.008
Cr	chromium	mg/L	0.0066	nm	nm	0.0066	0.0066
Cu	copper	mg/L	0.0061	0.0024	0.0026	0.0	0.0061
F	fluoride	mg/L	2.1	3.80	7.90	4.60	2.1
Fe	iron	mg/L	1.48	nm	nm	1.48	1.48
Hg	mercury <sup>a</sup>	mg/L	0.000002	nm	nm	0.000002	0.000002
K	potassium	mg/L	2.96	6.2	4	4.4	2.96
Mg	magnesium	mg/L	19.3	79	75	57.8	19.3
Mn	manganese	mg/L	0.66	3.5	3.25	2.47	0.66
Mo	molybdenum	mg/L	0.012	nm	nm	0.0119	0.012
Na	sodium	mg/L	119	94	131	114.5	119
Ni	nickel	mg/L	0.0125	nm	nm	0.0125	0.0125
Pb	lead	mg/L	0.0025	nm	nm	0.0025	0.0025
Sb	antimony	mg/L	0.0009	nm	nm	0.0009	0.0009
Se	selenium	mg/L	0.0015	0.0024	0.0028	0.0022	0.0015
U	uranium	mg/L	0.0015	nm	nm	0.0015	0.0015
V	vanadium <sup>a</sup>	mg/L	0.0009	nm	nm	0.0009	0.0009
Zn	zinc	mg/L	0.03	0.18	0.02	0.08	0.03
SO4	sulfate	mg/L	84	1408	1370	954	84
Cl	chloride	mg/L	49	27	27	34	49
TDS	total dissolved solids	mg/L	649	2,440	2,540	1,876	649

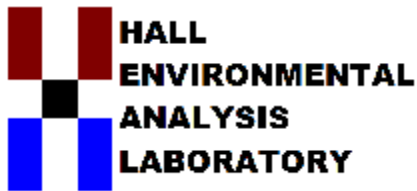
notes:

nm = not measured

<sup>a</sup> = results from sample analyzed for low detection limits for SRK geochemical model (samples collected July 10, 2013)

## **Appendix E – Water Supply Well Chemistry**

Production Well Water Quality Samples				
Parameter		PW1 May 1, 2012	PW3 May 3, 2012	Average
pH	pH	8.02	8.03	8.025
HCO <sub>3</sub>	Bicarbonate	150	120	135
Al	Aluminum	nd	nd	nd
As	Arsenic	0.0033	0.0074	0.00535
B	Boron	0.065	0.095	0.0800
Ba	Barium	0.011	0.0078	0.0094
Ca	Calcium	36	20	28
Cl	Chloride	32	50	41
Cu	Copper	nd	nd	nd
Cr	Chromium	nd	0.006	0.006
F	Fluoride	1	1.9	1.45
Fe	Iron	0.04	0.065	0.0525
Hg	Mercury	nd	nd	nd
K	Potassium	3.4	3.3	3.35
Mg	Magnesium	3.1	1	2.05
Mn	Manganese	0.0024	0.0026	0.0025
Mo	Molybdenum	nd	nd	nd
Na	Sodium	58	81	69.5
Ni	Nickel	nd	nd	nd
Pb	Lead	nd	nd	nd
SO <sub>4</sub>	Sulfate	28	26	27
Se	Selenium	nd	nd	nd
Si	Silica	17	21	19
U	Uranium	0.0032	0.0013	0.00225
V	Vanadium	nd	nd	nd
Tl	Thallium	nd	nd	nd
Zn	Zinc	0.024	0.021	0.0225



Hall Environmental Analysis Laboratory  
4901 Hawkins NE  
Albuquerque, NM 87109  
TEL: 505-345-3975 FAX: 505-345-4107  
Website: [www.hallenvironmental.com](http://www.hallenvironmental.com)

May 14, 2012

Katie Emmer

New Mexico Copper Corp  
2425 San Pedro Dr NE Ste 100  
Albuquerque, New Mexico 87109  
TEL: (505) 400-7925  
FAX

RE: Cu Flat

OrderNo.: 1205076

Dear Katie Emmer:

Hall Environmental Analysis Laboratory received 1 sample(s) on 5/2/2012 for the analyses presented in the following report.

These were analyzed according to EPA procedures or equivalent. To access our accredited tests please go to [www.hallenvironmental.com](http://www.hallenvironmental.com) or the state specific web sites. See the sample checklist and/or the Chain of Custody for information regarding the sample receipt temperature and preservation. Data qualifiers or a narrative will be provided if the sample analysis or analytical quality control parameters require a flag. All samples are reported as received unless otherwise indicated.

Please don't hesitate to contact HEAL for any additional information or clarifications.

Sincerely,

A handwritten signature in black ink, appearing to read "Andy Freeman", is written in a cursive style.

Andy Freeman  
Laboratory Manager  
4901 Hawkins NE  
Albuquerque, NM 87109

# Hall Environmental Analysis Laboratory, Inc.

Analytical Report

Lab Order 1205076

Date Reported: 5/14/2012

**CLIENT:** New Mexico Copper Corp

**Client Sample ID:** PW-1

**Project:** Cu Flat

**Collection Date:** 5/1/2012 2:00:00 PM

**Lab ID:** 1205076-001

**Matrix:** AQUEOUS

**Received Date:** 5/2/2012 7:30:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>EPA METHOD 300.0: ANIONS</b>						Analyst: <b>BRM</b>
Fluoride	1.0	0.10		mg/L	1	5/2/2012 12:52:03 PM
Chloride	32	10		mg/L	20	5/2/2012 1:03:17 PM
Nitrogen, Nitrite (As N)	ND	0.10		mg/L	1	5/2/2012 12:52:03 PM
Nitrogen, Nitrate (As N)	0.59	0.10		mg/L	1	5/2/2012 12:52:03 PM
Sulfate	28	0.50		mg/L	1	5/2/2012 12:52:03 PM
<b>EPA METHOD 200.7: DISSOLVED METALS</b>						Analyst: <b>ELS</b>
Aluminum	ND	0.020		mg/L	1	5/8/2012 8:02:55 AM
Barium	0.011	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Beryllium	ND	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Boron	0.065	0.040		mg/L	1	5/9/2012 8:36:51 AM
Cadmium	ND	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Calcium	36	1.0		mg/L	1	5/9/2012 8:36:51 AM
Chromium	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Cobalt	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Copper	ND	0.0060		mg/L	1	5/8/2012 8:02:55 AM
Iron	0.040	0.020		mg/L	1	5/9/2012 8:36:51 AM
Lead	ND	0.0050		mg/L	1	5/8/2012 8:02:55 AM
Magnesium	3.1	1.0		mg/L	1	5/9/2012 8:36:51 AM
Manganese	0.0024	0.0020		mg/L	1	5/8/2012 8:02:55 AM
Molybdenum	ND	0.0080		mg/L	1	5/8/2012 8:02:55 AM
Nickel	ND	0.010		mg/L	1	5/8/2012 8:02:55 AM
Potassium	3.4	1.0		mg/L	1	5/9/2012 8:36:51 AM
Silicon	17	0.40		mg/L	5	5/8/2012 8:06:09 AM
Silver	ND	0.0050		mg/L	1	5/8/2012 8:02:55 AM
Sodium	58	1.0		mg/L	1	5/9/2012 8:36:51 AM
Vanadium	ND	0.050		mg/L	1	5/8/2012 8:02:55 AM
Zinc	0.024	0.010		mg/L	1	5/8/2012 8:02:55 AM
<b>EPA 200.8: DISSOLVED METALS</b>						Analyst: <b>SNV</b>
Antimony	ND	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Arsenic	0.0033	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Selenium	ND	0.0010		mg/L	1	5/10/2012 2:28:58 PM
Thallium	ND	0.0010		mg/L	1	5/8/2012 1:15:26 PM
Uranium	0.0032	0.0010		mg/L	1	5/10/2012 2:28:58 PM
<b>EPA METHOD 245.1: MERCURY</b>						Analyst: <b>ELS</b>
Mercury	ND	0.00020		mg/L	1	5/9/2012 11:59:45 AM
<b>SM2340B: HARDNESS</b>						Analyst: <b>ELS</b>
Hardness (As CaCO3)	100	6.6		mg/L	1	5/9/2012
<b>EPA 120.1: SPECIFIC CONDUCTANCE</b>						Analyst: <b>DBD</b>
Conductivity	450	0.010		µmhos/cm	1	5/7/2012 12:31:49 PM

**Qualifiers:** \*/X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits  
 S Spike Recovery outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit



# Hall Environmental Analysis Laboratory, Inc.

Analytical Report

Lab Order 1205076

Date Reported: 5/14/2012

**CLIENT:** New Mexico Copper Corp

**Client Sample ID:** PW-1

**Project:** Cu Flat

**Collection Date:** 5/1/2012 2:00:00 PM

**Lab ID:** 1205076-001

**Matrix:** AQUEOUS

**Received Date:** 5/2/2012 7:30:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>SM4500-H+B: PH</b> Analyst: <b>JLF</b>						
pH	8.02	1.68	H	pH units	1	5/3/2012 1:22:52 PM
<b>SM2320B: ALKALINITY</b> Analyst: <b>JLF</b>						
Bicarbonate (As CaCO3)	150	20		mg/L CaCO3	1	5/3/2012 1:22:52 PM
Carbonate (As CaCO3)	ND	2.0		mg/L CaCO3	1	5/3/2012 1:22:52 PM
Total Alkalinity (as CaCO3)	150	20		mg/L CaCO3	1	5/3/2012 1:22:52 PM
<b>SM2540C MOD: TOTAL DISSOLVED SOLIDS</b> Analyst: <b>KS</b>						
Total Dissolved Solids	294	20.0		mg/L	1	5/8/2012 3:12:00 PM
<b>SM 2540D: TSS</b> Analyst: <b>KS</b>						
Suspended Solids	ND	4.0		mg/L	1	5/3/2012 5:30:00 PM

**Qualifiers:**

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- R RPD outside accepted recovery limits
- S Spike Recovery outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# Anatek Labs, Inc.

1282 Alturas Drive • Moscow, ID 83843 • (208) 883-2839 • Fax (208) 882-9246 • email moscow@anateklabs.com  
504 E Sprague Ste. D • Spokane WA 99202 • (509) 838-3999 • Fax (509) 838-4433 • email spokane@anateklabs.com

**Client:** HALL ENVIRONMENTAL ANALYSIS LAB      **Batch #:** 120503026  
**Address:** 4901 HAWKINS NE SUITE D      **Project Name:** 1205076  
ALBUQUERQUE, NM 87109  
**Attn:** ANDY FREEMAN

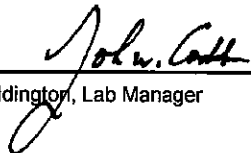
## Analytical Results Report

<b>Sample Number</b>	120503026-001	<b>Sampling Date</b>	5/1/2012	<b>Date/Time Received</b>	5/3/2012 12:24 PM
<b>Client Sample ID</b>	1205076-001D / PW-1	<b>Sampling Time</b>	2:00 PM	<b>Extraction Date</b>	
<b>Matrix</b>	Water	<b>Sample Location</b>			
<b>Comments</b>					

Parameter	Result	Units	PQL	Analysis Date	Analyst	Method	Qualifier
Cyanide	ND	mg/L	0.01	5/11/2012	CRW	EPA 335.4	

Authorized Signature

  
\_\_\_\_\_  
John Coddington, Lab Manager

MCL      EPA's Maximum Contaminant Level  
ND      Not Detected  
PQL      Practical Quantitation Limit

This report shall not be reproduced except in full, without the written approval of the laboratory.  
The results reported relate only to the samples indicated.  
Soil/solid results are reported on a dry-weight basis unless otherwise noted.

Certifications held by Anatek Labs ID: EPA:ID00013; AZ:0701; CO:ID00013; FL(NELAP):E87893; ID:ID00013; IN:C-ID-01; KY:90142; MT:CERT0028; NM: ID00013; OR:ID200001-002; WA:C595  
Certifications held by Anatek Labs WA: EPA:WA00169; ID:WA00169; WA:C585; MT:Cert0095

Friday, May 11, 2012

Page 1 of 1

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# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>72991</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	ND	0.020								
Barium	ND	0.0020								
Beryllium	ND	0.0020								
Cadmium	ND	0.0020								
Chromium	ND	0.0060								
Cobalt	ND	0.0060								
Copper	ND	0.0060								
Lead	ND	0.0050								
Manganese	ND	0.0020								
Molybdenum	ND	0.0080								
Nickel	ND	0.010								
Silicon	ND	0.080								
Silver	ND	0.0050								
Vanadium	ND	0.050								
Zinc	ND	0.010								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>72992</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	0.52	0.020	0.5000	0	105	85	115			
Barium	0.49	0.0020	0.5000	0	98.9	85	115			
Beryllium	0.52	0.0020	0.5000	0	103	85	115			
Cadmium	0.50	0.0020	0.5000	0	99.2	85	115			
Chromium	0.49	0.0060	0.5000	0	98.5	85	115			
Cobalt	0.47	0.0060	0.5000	0	94.9	85	115			
Copper	0.50	0.0060	0.5000	0	99.9	85	115			
Lead	0.50	0.0050	0.5000	0	99.3	85	115			
Manganese	0.48	0.0020	0.5000	0	96.9	85	115			
Molybdenum	0.49	0.0080	0.5000	0.002030	98.4	85	115			
Nickel	0.47	0.010	0.5000	0	93.9	85	115			
Silicon	2.6	0.080	2.500	0	104	85	115			
Silver	0.094	0.0050	0.1000	0	94.1	85	115			
Vanadium	0.52	0.050	0.5000	0	104	85	115			
Zinc	0.50	0.010	0.5000	0	101	85	115			

Sample ID <b>1205193-005EMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73030</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
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**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205193-005EMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2622</b>	RunNo:	<b>2622</b>					
Prep Date:		Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73030</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	0.54	0.020	0.5000	0	107	70	130			
Barium	0.52	0.0020	0.5000	0.02182	98.9	70	130			
Zinc	0.54	0.010	0.5000	0.03785	101	70	130			

Sample ID	<b>1205193-005EMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2622</b>	RunNo:	<b>2622</b>					
Prep Date:		Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73031</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	0.53	0.020	0.5000	0	106	70	130	1.33	20	
Barium	0.51	0.0020	0.5000	0.02182	97.2	70	130	1.71	20	
Zinc	0.53	0.010	0.5000	0.03785	98.0	70	130	2.48	20	

Sample ID	<b>1205193-005EMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74182</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Potassium	56	1.0	50.00	4.808	102	70	130			

Sample ID	<b>1205193-005EMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74183</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Potassium	57	1.0	50.00	4.808	104	70	130	2.44	20	

Sample ID	<b>1205193-005EMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74185</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Iron	4.5	0.10	2.500	2.034	99.6	70	130			
Magnesium	390	5.0	250.0	124.9	107	70	130			
Sodium	460	5.0	250.0	192.5	107	70	130			

Sample ID	<b>1205193-005EMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74186</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Iron	4.6	0.10	2.500	2.034	101	70	130	1.03	20	

**Qualifiers:**

- |  |  |
|--|--|
| *X Value exceeds Maximum Contaminant Level.  | B Analyte detected in the associated Method Blank    |
| E Value above quantitation range             | H Holding times for preparation or analysis exceeded |
| J Analyte detected below quantitation limits | ND Not Detected at the Reporting Limit               |
| R RPD outside accepted recovery limits       | RL Reporting Detection Limit                         |

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205193-005EMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74186</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Magnesium	390	5.0	250.0	124.9	106	70	130	0.684	20	
Sodium	460	5.0	250.0	192.5	106	70	130	0.966	20	

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74215</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	ND	0.040								
Calcium	ND	1.0								
Iron	ND	0.020								
Magnesium	ND	1.0								
Potassium	ND	1.0								
Sodium	ND	1.0								

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74216</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	0.51	0.040	0.5000	0	101	85	115			
Calcium	54	1.0	50.00	0	107	85	115			
Iron	0.47	0.020	0.5000	0.004190	93.2	85	115			
Magnesium	54	1.0	50.00	0	109	85	115			
Potassium	53	1.0	50.00	0	106	85	115			
Sodium	54	1.0	50.00	0	107	85	115			

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
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- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>						
Client ID:	<b>LCSW</b>	Batch ID: <b>R2629</b>		RunNo: <b>2629</b>						
Prep Date:		Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73283</b>		Units: <b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	0.023	0.0010	0.02500	0	92.8	85	115			
Arsenic	0.023	0.0010	0.02500	0	93.1	85	115			
Thallium	0.023	0.0010	0.02500	0	92.9	85	115			

Sample ID	<b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>						
Client ID:	<b>PBW</b>	Batch ID: <b>R2629</b>		RunNo: <b>2629</b>						
Prep Date:		Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73284</b>		Units: <b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	ND	0.0010								
Arsenic	ND	0.0010								
Thallium	ND	0.0010								

Sample ID	<b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>						
Client ID:	<b>LCSW</b>	Batch ID: <b>R2708</b>		RunNo: <b>2708</b>						
Prep Date:		Analysis Date: <b>5/10/2012</b>		SeqNo: <b>75447</b>		Units: <b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	0.026	0.0010	0.02500	0	104	85	115			
Uranium	0.025	0.0010	0.02500	0	99.2	85	115			

Sample ID	<b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>						
Client ID:	<b>PBW</b>	Batch ID: <b>R2708</b>		RunNo: <b>2708</b>						
Prep Date:		Analysis Date: <b>5/10/2012</b>		SeqNo: <b>75448</b>		Units: <b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	ND	0.0010								
Uranium	ND	0.0010								

**Qualifiers:**

- |  |  |
|--|--|
| *X Value exceeds Maximum Contaminant Level.  | B Analyte detected in the associated Method Blank    |
| E Value above quantitation range             | H Holding times for preparation or analysis exceeded |
| J Analyte detected below quantitation limits | ND Not Detected at the Reporting Limit               |
| R RPD outside accepted recovery limits       | RL Reporting Detection Limit                         |

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>MB-1862</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74223</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	ND	0.00020								

Sample ID	<b>LCS-1862</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74224</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.4	80	120			

Sample ID	<b>1204854-004AMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74226</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.2	75	125			

Sample ID	<b>1204854-004AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74227</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.1	75	125	0.0957	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70797</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	ND	0.10								
Chloride	ND	0.50								
Nitrogen, Nitrite (As N)	ND	0.10								
Nitrogen, Nitrate (As N)	ND	0.10								
Sulfate	ND	0.50								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70798</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.47	0.10	0.5000	0	93.8	90	110			
Chloride	4.6	0.50	5.000	0	92.9	90	110			
Nitrogen, Nitrite (As N)	0.93	0.10	1.000	0	92.9	90	110			
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	97.4	90	110			
Sulfate	9.5	0.50	10.00	0	94.8	90	110			

Sample ID <b>1205075-001BMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70800</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.68	0.10	0.5000	0.1911	98.1	72.9	113			
Nitrogen, Nitrite (As N)	1.0	0.10	1.000	0	101	77.6	111			
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	99.9	82.8	116			

Sample ID <b>1205075-001BMSD</b>	SampType: <b>MSD</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70801</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.65	0.10	0.5000	0.1911	90.9	72.9	113	5.39	20	
Nitrogen, Nitrite (As N)	0.90	0.10	1.000	0	90.2	77.6	111	10.8	20	
Nitrogen, Nitrate (As N)	2.3	0.10	2.500	0	91.3	82.8	116	8.94	20	

Sample ID <b>1205079-001AMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2544</b>		RunNo: <b>2544</b>							
Prep Date:	Analysis Date: <b>5/2/2012</b>		SeqNo: <b>70809</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Nitrogen, Nitrite (As N)	1.3	0.10	1.000	0	127	77.6	111			S
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	97.8	82.8	116			

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit



# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205079-001AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 300.0: Anions</b>						
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>						
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70810</b>	Units:	<b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Nitrogen, Nitrite (As N)	1.2	0.10	1.000	0	122	77.6	111	4.14	20	S	
Nitrogen, Nitrate (As N)	2.4	0.10	2.500	0	95.5	82.8	116	2.38	20		

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 300.0: Anions</b>						
Client ID:	<b>PBW</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>						
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70849</b>	Units:	<b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	ND	0.10									
Chloride	ND	0.50									
Nitrogen, Nitrite (As N)	ND	0.10									
Nitrogen, Nitrate (As N)	ND	0.10									
Sulfate	ND	0.50									

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 300.0: Anions</b>						
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>						
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70850</b>	Units:	<b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	0.50	0.10	0.5000	0	99.0	90	110				
Chloride	4.7	0.50	5.000	0	94.2	90	110				
Nitrogen, Nitrite (As N)	0.98	0.10	1.000	0	98.0	90	110				
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	98.3	90	110				
Sulfate	9.6	0.50	10.00	0	95.7	90	110				

Sample ID	<b>1205066-002AMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 300.0: Anions</b>						
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>						
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70852</b>	Units:	<b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	1.1	0.10	0.5000	0.5616	101	72.9	113				
Nitrogen, Nitrite (As N)	0.93	0.10	1.000	0	92.7	77.6	111				
Nitrogen, Nitrate (As N)	3.3	0.10	2.500	0.5059	111	82.8	116				
Sulfate	48	0.50	10.00	36.66	113	80.5	119				

Sample ID	<b>1205066-002AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 300.0: Anions</b>						
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>						
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70853</b>	Units:	<b>mg/L</b>				
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Fluoride	1.0	0.10	0.5000	0.5616	93.8	72.9	113	3.52	20		
Nitrogen, Nitrite (As N)	0.79	0.10	1.000	0	78.5	77.6	111	16.5	20		

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205066-002AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2544</b>	RunNo:	<b>2544</b>					
Prep Date:		Analysis Date:	<b>5/2/2012</b>	SeqNo:	<b>70853</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Nitrogen, Nitrate (As N)	3.0	0.10	2.500	0.5059	98.7	82.8	116	10.2	20	
Sulfate	47	0.50	10.00	36.66	101	80.5	119	2.50	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	1205170-001D	SampType:	DUP	TestCode:	EPA 120.1: Specific Conductance					
Client ID:	BatchQC	Batch ID:	R2646	RunNo:	2646					
Prep Date:		Analysis Date:	5/7/2012	SeqNo:	73516	Units:	µmhos/cm			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Conductivity	610	0.010						0	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205005-001A DUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM4500-H+B: pH</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71363</b>	Units:	<b>pH units</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	3.92	1.68						0.762		H

Sample ID	<b>1205120-001B DUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM4500-H+B: pH</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71373</b>	Units:	<b>pH units</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	7.73	1.68						0.645		H

**Qualifiers:**

\*X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205005-001A MS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71221</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	62.6	110			S

Sample ID	<b>1205005-001A MSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71222</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	59.9	111	0	10	S

Sample ID	<b>1205120-001B MS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71242</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	360	20	80.00	299.4	70.9	62.6	110			

Sample ID	<b>1205120-001B MSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71243</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	350	20	80.00	299.4	67.1	59.9	111	0.869	10	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>MB-1832</b>	SampType:	<b>MBLK</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73329</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	ND	20.0								

Sample ID	<b>LCS-1832</b>	SampType:	<b>LCS</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73330</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	1,020	20.0	1,000	0	102	80	120			

Sample ID	<b>1205078-002GMS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73337</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,890	20.0	1,000	3,791	110	80	120			

Sample ID	<b>1205078-002GMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2540C MOD: Total Dissolved Solids</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1832</b>	RunNo:	<b>2634</b>					
Prep Date:	<b>5/7/2012</b>	Analysis Date:	<b>5/8/2012</b>	SeqNo:	<b>73338</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,930	20.0	1,000	3,791	114	80	120	0.733	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205076

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>MB-1800</b>	SampType:	<b>MBLK</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1800</b>	RunNo:	<b>2570</b>					
Prep Date:	<b>5/3/2012</b>	Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71656</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0								

Sample ID	<b>LCS-1800</b>	SampType:	<b>LCS</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1800</b>	RunNo:	<b>2570</b>					
Prep Date:	<b>5/3/2012</b>	Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71657</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	93	4.0	96.60	0	96.3	82.9	110			

Sample ID	<b>1205034-001BDUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1800</b>	RunNo:	<b>2570</b>					
Prep Date:	<b>5/3/2012</b>	Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71663</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0						0	15	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit



Hall Environmental Analysis Laboratory  
 4901 Hawkins NE  
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 TEL: 505-345-3975 FAX: 505-345-4107  
 Website: www.hallenvironmental.com

# Sample Log-In Check List

Client Name: NEW MEXICO COPPER CORP Work Order Number: 1205076  
 Received by/date: AT 05/02/12  
 Logged By: Anne Thorne 5/2/2012 7:30:00 AM *Anne Thorne*  
 Completed By: Anne Thorne 5/2/2012 *Anne Thorne*  
 Reviewed By: AT 05/02/12

**Chain of Custody**

- 1. Were seals intact? Yes  No  Not Present
- 2. Is Chain of Custody complete? Yes  No  Not Present
- 3. How was the sample delivered? Client

**Log In**

- 4. Coolers are present? (see 19. for cooler specific information) Yes  No  NA
- 5. Was an attempt made to cool the samples? Yes  No  NA
- 6. Were all samples received at a temperature of >0° C to 6.0°C Yes  No  NA
- 7. Sample(s) in proper container(s)? Yes  No
- 8. Sufficient sample volume for indicated test(s)? Yes  No
- 9. Are samples (except VOA and ONG) properly preserved? Yes  No
- 10. Was preservative added to bottles? Yes  No  NA
- 11. VOA vials have zero headspace? Yes  No  No VOA Vials
- 12. Were any sample containers received broken? Yes  No
- 13. Does paperwork match bottle labels? (Note discrepancies on chain of custody) Yes  No
- 14. Are matrices correctly identified on Chain of Custody? Yes  No
- 15. Is it clear what analyses were requested? Yes  No
- 16. Were all holding times able to be met? (If no, notify customer for authorization.) Yes  No

# of preserved bottles checked for pH: 1  
 (2 or 12 unless noted)  
 Adjusted? \_\_\_\_\_  
 Checked by AT 05/02/12

**Special Handling (if applicable)**

- 17. Was client notified of all discrepancies with this order? Yes  No  NA

Person Notified:	_____	Date:	_____
By Whom:	_____	Via:	<input type="checkbox"/> eMail <input type="checkbox"/> Phone <input type="checkbox"/> Fax <input type="checkbox"/> In Person
Regarding:	_____		
Client Instructions:	_____		

- 18. Additional remarks:

**19. Cooler Information**

Cooler No	Temp °C	Condition	Seal Intact	Seal No	Seal Date	Signed By
1	3.4	Good	Not Present			



# Chain-of-Custody Record

Client: New Mexico Copper Corp  
 Mailing Address: 2425 San Pedro Dr NE  
Suite 100, ABO, NM  
 Phone #: 505.400.7925  
 email or Fax#: \_\_\_\_\_  
 QA/QC Package: \_\_\_\_\_  
 Standard  Level 4 (Full Validation)  
 Accreditation  
 NELAP  Other \_\_\_\_\_  
 EDD (Type) \_\_\_\_\_

Turn-Around Time: Need Results by May 11 via email  
 Standard  Rush  
 Project Name: Cu Flat  
 Project #: Production Well Sampling  
 Project Manager: Katie Emmer

Sampler: CMC  
 On Ice  Gas  No  
 Sample Temperature: 37  
 Container Type and #  
 Preservative Type  
 HEAL No

Date	Time	Matrix	Sample Request ID	Container Type and #	Preservative Type	HEAL No
5/1/12	1400	W	PW-1	500		
				125	H2SO4	-001
				125	HNO3 + filter	-001
				500	HNO3	-001
				500	NaOH	-001

**HALL ENVIRONMENTAL ANALYSIS LABORATORY**  
 www.hallenvironmental.com  
 4901 Hawkins NE - Albuquerque, NM 87109  
 Tel: 505-345-3975 Fax 505-345-4107

Analysis Request	
BTEX + MTBE + TMBs (8021)	
BTEX + MTBE + TPH (Gas only)	
TPH Method 8015B (Gas/Diesel)	
TPH (Method 418.1)	
EDB (Method 504.1)	
8310 (PNA or PAH)	
RCA 8 Metals	
Anions (F, Cl, NO <sub>3</sub> , NO <sub>2</sub> , PO <sub>4</sub> , SO <sub>4</sub> )	
8081 Pesticides / 8082 PCBs	
8260B (VOA)	
8270 (Semi-VOA)	
Air Bubbles (Y or N)	

Remarks:  
 Please email results to: Katie Emmer  
Kemmer@themacresourcesgroup.com  
Please add Hazardous, as take of s-3

Receiver by: [Signature] Date: May 2012 Time: 14:30  
 Relinquished by: [Signature]  
 Receiver by: [Signature] Date: 5/2/12 Time: 07:30  
 Relinquished by: [Signature]

If necessary, samples submitted to Hall Environmental may be subcontracted to other accredited laboratories. This serves as notice of this possibility. Any sub-contracted data will be clearly notated on the analytical report.

NMI Copper  
May 1, 2012

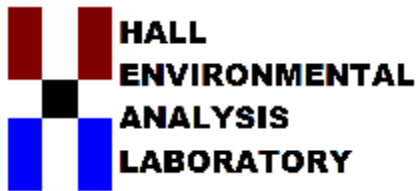
**Table 9-3**  
**Analytical Parameters and Analysis Methods for Groundwater Samples**

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
<b>Anions</b>		
Fluoride	EPA Method 300.0	0.1
Chloride	EPA Method 300.0	0.1
Nitrogen, Nitrite (as N)	EPA Method 300.0	0.1
Nitrogen, Nitrate (as N)	EPA Method 300.0	0.1
Sulfate	EPA Method 300.0	0.5
<b>Dissolved Metals</b>		
Aluminum	EPA Method 200.7	0.02
Antimony	EPA Method 200.8	0.005
Arsenic	EPA Method 200.8	0.02
Barium	EPA Method 200.7	0.002
Beryllium	EPA Method 200.7	0.002
Boron	EPA Method 200.7	0.04
Cadmium	EPA Method 200.7	0.002
Calcium	EPA Method 200.7	0.50
Chromium	EPA Method 200.7	0.006
Cobalt	EPA Method 200.7	0.006
Copper	EPA Method 200.7	0.0003
Iron	EPA Method 200.7	0.02
Lead	EPA Method 200.7	0.005
Magnesium	EPA Method 200.7	0.50
Manganese	EPA Method 200.7	0.002
Mercury	EPA Method 7470 CVAA	0.0002
Molybdenum	EPA Method 200.7	0.008
Nickel	EPA Method 200.7	0.01
Potassium	EPA Method 200.7	1.0
Selenium	EPA Method 200.8	0.02
Silicon	EPA Method 200.7	0.08
Silver	EPA Method 200.7	0.005
Sodium	EPA Method 200.7	0.5

NM Copper  
May 1, 2012

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
Thallium	EPA Method 200.7	0.01
<del>Titanium</del>	<del>EPA Method 200.7</del>	<del>0.005</del>
Uranium	EPA Method 200.8	0.01
Vanadium	EPA Method 200.7	0.005
Zinc	EPA Method 200.7	0.005
<b>Solids</b>		
Total Suspended Solids (TSS)	SM 2540D	1.0 µg/L
Total Dissolved Solids (TDS)	SM 2540C	10
<b>Alkalinity</b>		
Alkalinity, total (as CaCO <sub>3</sub> )	SM 2320B	20
Carbonate	SM 2320B	20
Bicarbonate	SM 2320B	20
<b>Other</b>		
pH	150.1	12.45
Specific Conductance	120.1	0.01 µS/cm
Cyanide	Kelada-01	0.005

Note: NA = not applicable as sample will not be analyzed for a given parameter.



Hall Environmental Analysis Laboratory  
4901 Hawkins NE  
Albuquerque, NM 87109  
TEL: 505-345-3975 FAX: 505-345-4107  
Website: [www.hallenvironmental.com](http://www.hallenvironmental.com)

May 14, 2012

Katie Emmer

New Mexico Copper Corp  
2425 San Pedro Dr NE Ste 100  
Albuquerque, New Mexico 87109  
TEL: (505) 400-7925  
FAX

RE: Cu Flat

OrderNo.: 1205153

Dear Katie Emmer:

Hall Environmental Analysis Laboratory received 1 sample(s) on 5/3/2012 for the analyses presented in the following report.

These were analyzed according to EPA procedures or equivalent. To access our accredited tests please go to [www.hallenvironmental.com](http://www.hallenvironmental.com) or the state specific web sites. See the sample checklist and/or the Chain of Custody for information regarding the sample receipt temperature and preservation. Data qualifiers or a narrative will be provided if the sample analysis or analytical quality control parameters require a flag. All samples are reported as received unless otherwise indicated.

Please don't hesitate to contact HEAL for any additional information or clarifications.

Sincerely,

A handwritten signature in black ink, appearing to read "Andy Freeman", is written in a cursive style.

Andy Freeman  
Laboratory Manager  
4901 Hawkins NE  
Albuquerque, NM 87109

# Hall Environmental Analysis Laboratory, Inc.

Analytical Report

Lab Order 1205153

Date Reported: 5/14/2012

CLIENT: New Mexico Copper Corp

Client Sample ID: PW-3

Project: Cu Flat

Collection Date: 5/2/2012 2:30:00 PM

Lab ID: 1205153-001

Matrix: AQUEOUS

Received Date: 5/3/2012 8:35:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>EPA METHOD 300.0: ANIONS</b>						Analyst: <b>BRM</b>
Fluoride	1.9	0.10		mg/L	1	5/3/2012 12:04:13 PM
Chloride	50	10		mg/L	20	5/3/2012 12:41:28 PM
Nitrogen, Nitrite (As N)	ND	0.10		mg/L	1	5/3/2012 12:04:13 PM
Nitrogen, Nitrate (As N)	0.70	0.10		mg/L	1	5/3/2012 12:04:13 PM
Sulfate	26	0.50		mg/L	1	5/3/2012 12:04:13 PM
<b>EPA METHOD 200.7: DISSOLVED METALS</b>						Analyst: <b>ELS</b>
Aluminum	ND	0.020		mg/L	1	5/8/2012 8:09:23 AM
Barium	0.0078	0.0020		mg/L	1	5/8/2012 8:09:23 AM
Beryllium	ND	0.0020		mg/L	1	5/8/2012 8:09:23 AM
Boron	0.095	0.040		mg/L	1	5/9/2012 8:40:03 AM
Cadmium	ND	0.0020		mg/L	1	5/8/2012 8:09:23 AM
Calcium	20	1.0		mg/L	1	5/9/2012 8:40:03 AM
Chromium	0.0060	0.0060		mg/L	1	5/8/2012 8:09:23 AM
Cobalt	ND	0.0060		mg/L	1	5/8/2012 8:09:23 AM
Copper	ND	0.0060		mg/L	1	5/8/2012 8:09:23 AM
Iron	0.065	0.020		mg/L	1	5/9/2012 8:40:03 AM
Lead	ND	0.0050		mg/L	1	5/8/2012 8:09:23 AM
Magnesium	1.0	1.0		mg/L	1	5/9/2012 8:40:03 AM
Manganese	0.0026	0.0020		mg/L	1	5/8/2012 8:09:23 AM
Molybdenum	ND	0.0080		mg/L	1	5/8/2012 8:09:23 AM
Nickel	ND	0.010		mg/L	1	5/8/2012 8:09:23 AM
Potassium	3.3	1.0		mg/L	1	5/9/2012 8:40:03 AM
Silicon	21	0.40		mg/L	5	5/8/2012 8:12:46 AM
Silver	ND	0.0050		mg/L	1	5/8/2012 8:09:23 AM
Sodium	81	1.0		mg/L	1	5/9/2012 8:40:03 AM
Vanadium	ND	0.050		mg/L	1	5/8/2012 8:09:23 AM
Zinc	0.021	0.010		mg/L	1	5/8/2012 8:09:23 AM
<b>EPA 200.8: DISSOLVED METALS</b>						Analyst: <b>SNV</b>
Antimony	ND	0.0010		mg/L	1	5/8/2012 1:19:22 PM
Arsenic	0.0074	0.0010		mg/L	1	5/8/2012 1:19:22 PM
Selenium	ND	0.0010		mg/L	1	5/10/2012 2:32:54 PM
Thallium	ND	0.0010		mg/L	1	5/8/2012 1:19:22 PM
Uranium	0.0013	0.0010		mg/L	1	5/10/2012 2:32:54 PM
<b>EPA METHOD 245.1: MERCURY</b>						Analyst: <b>ELS</b>
Mercury	ND	0.00020		mg/L	1	5/9/2012 12:01:31 PM
<b>SM2340B: HARDNESS</b>						Analyst: <b>ELS</b>
Hardness (As CaCO3)	53	6.6		mg/L	1	5/9/2012
<b>EPA 120.1: SPECIFIC CONDUCTANCE</b>						Analyst: <b>DBD</b>
Conductivity	460	0.010		µmhos/cm	1	5/7/2012 12:36:13 PM

**Qualifiers:** \*/X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits  
 S Spike Recovery outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# Hall Environmental Analysis Laboratory, Inc.

Analytical Report

Lab Order 1205153

Date Reported: 5/14/2012

**CLIENT:** New Mexico Copper Corp

**Client Sample ID:** PW-3

**Project:** Cu Flat

**Collection Date:** 5/2/2012 2:30:00 PM

**Lab ID:** 1205153-001

**Matrix:** AQUEOUS

**Received Date:** 5/3/2012 8:35:00 AM

Analyses	Result	RL	Qual	Units	DF	Date Analyzed
<b>SM4500-H+B: PH</b> Analyst: <b>JLF</b>						
pH	8.03	1.68	H	pH units	1	5/3/2012 5:14:04 PM
<b>SM2320B: ALKALINITY</b> Analyst: <b>JLF</b>						
Bicarbonate (As CaCO3)	120	20		mg/L CaCO3	1	5/3/2012 5:14:04 PM
Carbonate (As CaCO3)	ND	2.0		mg/L CaCO3	1	5/3/2012 5:14:04 PM
Total Alkalinity (as CaCO3)	120	20		mg/L CaCO3	1	5/3/2012 5:14:04 PM
<b>SM2540C MOD: TOTAL DISSOLVED SOLIDS</b> Analyst: <b>KS</b>						
Total Dissolved Solids	303	20.0		mg/L	1	5/8/2012 3:12:00 PM
<b>SM 2540D: TSS</b> Analyst: <b>KS</b>						
Suspended Solids	ND	4.0		mg/L	1	5/4/2012 4:36:00 PM

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- S Spike Recovery outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# Anatek Labs, Inc.

1282 Alturas Drive • Moscow, ID 83843 • (208) 883-2839 • Fax (208) 882-9246 • email moscow@anateklabs.com  
504 E Sprague Ste. D • Spokane WA 99202 • (509) 838-3999 • Fax (509) 838-4433 • email spokane@anateklabs.com

**Client:** HALL ENVIRONMENTAL ANALYSIS LAB  
**Address:** 4901 HAWKINS NE SUITE D  
ALBUQUERQUE, NM 87109  
**Attn:** ANDY FREEMAN

**Batch #:** 120504004  
**Project Name:** 1205153

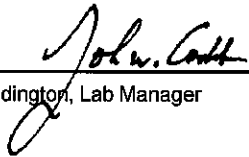
## Analytical Results Report

<b>Sample Number</b>	120504004-001	<b>Sampling Date</b>	5/2/2012	<b>Date/Time Received</b>	5/4/2012 10:18 AM
<b>Client Sample ID</b>	1205153-001D / PW-3	<b>Sampling Time</b>	2:30 PM		
<b>Matrix</b>	Water	<b>Sample Location</b>			
<b>Comments</b>					

Parameter	Result	Units	PQL	Analysis Date	Analyst	Method	Qualifier
Cyanide	ND	mg/L	0.01	5/8/2012	CRW	EPA 335.4	

Authorized Signature

  
\_\_\_\_\_  
John Coddington, Lab Manager

MCL EPA's Maximum Contaminant Level  
ND Not Detected  
PQL Practical Quantitation Limit

This report shall not be reproduced except in full, without the written approval of the laboratory.  
The results reported relate only to the samples indicated.  
Soil/solid results are reported on a dry-weight basis unless otherwise noted.

Certifications held by Anatek Labs ID: EPA:ID00013; AZ:0701; CO:ID00013; FL(NELAP):E87893; ID:ID00013; IN:C-ID-01; KY:90142; MT:CERT0028; NM: ID00013; OR:ID200001-002; WA:C595  
Certifications held by Anatek Labs WA: EPA:WA00169; ID:WA00169; WA:C595; MT:Cert0095

Thursday, May 10, 2012

Page 1 of 1

17225

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>72991</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	ND	0.020								
Barium	ND	0.0020								
Beryllium	ND	0.0020								
Cadmium	ND	0.0020								
Chromium	ND	0.0060								
Cobalt	ND	0.0060								
Copper	ND	0.0060								
Lead	ND	0.0050								
Manganese	ND	0.0020								
Molybdenum	ND	0.0080								
Nickel	ND	0.010								
Silicon	ND	0.080								
Silver	ND	0.0050								
Vanadium	ND	0.050								
Zinc	ND	0.010								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>72992</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Aluminum	0.52	0.020	0.5000	0	105	85	115			
Barium	0.49	0.0020	0.5000	0	98.9	85	115			
Beryllium	0.52	0.0020	0.5000	0	103	85	115			
Cadmium	0.50	0.0020	0.5000	0	99.2	85	115			
Chromium	0.49	0.0060	0.5000	0	98.5	85	115			
Cobalt	0.47	0.0060	0.5000	0	94.9	85	115			
Copper	0.50	0.0060	0.5000	0	99.9	85	115			
Lead	0.50	0.0050	0.5000	0	99.3	85	115			
Manganese	0.48	0.0020	0.5000	0	96.9	85	115			
Molybdenum	0.49	0.0080	0.5000	0.002030	98.4	85	115			
Nickel	0.47	0.010	0.5000	0	93.9	85	115			
Silicon	2.6	0.080	2.500	0	104	85	115			
Silver	0.094	0.0050	0.1000	0	94.1	85	115			
Vanadium	0.52	0.050	0.5000	0	104	85	115			
Zinc	0.50	0.010	0.5000	0	101	85	115			

Sample ID <b>1205193-005EMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 200.7: Dissolved Metals</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2622</b>		RunNo: <b>2622</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73030</b>		Units: <b>mg/L</b>					

Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
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**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit



# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2622		RunNo:	2622				
Prep Date:			Analysis Date:	5/8/2012		SeqNo:	73030		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	0.54	0.020	0.5000	0	107	70	130				
Barium	0.52	0.0020	0.5000	0.02182	98.9	70	130				
Zinc	0.54	0.010	0.5000	0.03785	101	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2622		RunNo:	2622				
Prep Date:			Analysis Date:	5/8/2012		SeqNo:	73031		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Aluminum	0.53	0.020	0.5000	0	106	70	130	1.33	20		
Barium	0.51	0.0020	0.5000	0.02182	97.2	70	130	1.71	20		
Zinc	0.53	0.010	0.5000	0.03785	98.0	70	130	2.48	20		

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74182		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Potassium	56	1.0	50.00	4.808	102	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74183		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Potassium	57	1.0	50.00	4.808	104	70	130	2.44	20		

Sample ID	1205193-005EMS		SampType:	MS		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74185		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Iron	4.5	0.10	2.500	2.034	99.6	70	130				
Magnesium	390	5.0	250.0	124.9	107	70	130				
Sodium	460	5.0	250.0	192.5	107	70	130				

Sample ID	1205193-005EMSD		SampType:	MSD		TestCode:	EPA Method 200.7: Dissolved Metals				
Client ID:	BatchQC		Batch ID:	R2670		RunNo:	2670				
Prep Date:			Analysis Date:	5/9/2012		SeqNo:	74186		Units: mg/L		
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual	
Iron	4.6	0.10	2.500	2.034	101	70	130	1.03	20		

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205193-005EMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74186</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Magnesium	390	5.0	250.0	124.9	106	70	130	0.684	20	
Sodium	460	5.0	250.0	192.5	106	70	130	0.966	20	

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74215</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	ND	0.040								
Calcium	ND	1.0								
Iron	ND	0.020								
Magnesium	ND	1.0								
Potassium	ND	1.0								
Sodium	ND	1.0								

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 200.7: Dissolved Metals</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2670</b>	RunNo:	<b>2670</b>					
Prep Date:		Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74216</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Boron	0.51	0.040	0.5000	0	101	85	115			
Calcium	54	1.0	50.00	0	107	85	115			
Iron	0.47	0.020	0.5000	0.004190	93.2	85	115			
Magnesium	54	1.0	50.00	0	109	85	115			
Potassium	53	1.0	50.00	0	106	85	115			
Sodium	54	1.0	50.00	0	107	85	115			

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2629</b>		RunNo: <b>2629</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73283</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	0.023	0.0010	0.02500	0	92.8	85	115			
Arsenic	0.023	0.0010	0.02500	0	93.1	85	115			
Thallium	0.023	0.0010	0.02500	0	92.9	85	115			

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2629</b>		RunNo: <b>2629</b>							
Prep Date:	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73284</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Antimony	ND	0.0010								
Arsenic	ND	0.0010								
Thallium	ND	0.0010								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2708</b>		RunNo: <b>2708</b>							
Prep Date:	Analysis Date: <b>5/10/2012</b>		SeqNo: <b>75447</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	0.026	0.0010	0.02500	0	104	85	115			
Uranium	0.025	0.0010	0.02500	0	99.2	85	115			

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA 200.8: Dissolved Metals</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2708</b>		RunNo: <b>2708</b>							
Prep Date:	Analysis Date: <b>5/10/2012</b>		SeqNo: <b>75448</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Selenium	ND	0.0010								
Uranium	ND	0.0010								

**Qualifiers:**

- |  |  |
|--|--|
| *X Value exceeds Maximum Contaminant Level.  | B Analyte detected in the associated Method Blank    |
| E Value above quantitation range             | H Holding times for preparation or analysis exceeded |
| J Analyte detected below quantitation limits | ND Not Detected at the Reporting Limit               |
| R RPD outside accepted recovery limits       | RL Reporting Detection Limit                         |

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>MB-1862</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74223</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	ND	0.00020								

Sample ID	<b>LCS-1862</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74224</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.4	80	120			

Sample ID	<b>1204854-004AMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74226</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.2	75	125			

Sample ID	<b>1204854-004AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 245.1: Mercury</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1862</b>	RunNo:	<b>2669</b>					
Prep Date:	<b>5/9/2012</b>	Analysis Date:	<b>5/9/2012</b>	SeqNo:	<b>74227</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Mercury	0.0049	0.00020	0.005000	0	97.1	75	125	0.0957	20	

**Qualifiers:**

- \*/X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>MB</b>	SampType: <b>MBLK</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>PBW</b>	Batch ID: <b>R2561</b>		RunNo: <b>2561</b>							
Prep Date:	Analysis Date: <b>5/3/2012</b>		SeqNo: <b>71254</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	ND	0.10								
Chloride	ND	0.50								
Nitrogen, Nitrite (As N)	ND	0.10								
Nitrogen, Nitrate (As N)	ND	0.10								
Sulfate	ND	0.50								

Sample ID <b>LCS</b>	SampType: <b>LCS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>R2561</b>		RunNo: <b>2561</b>							
Prep Date:	Analysis Date: <b>5/3/2012</b>		SeqNo: <b>71255</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.48	0.10	0.5000	0	95.5	90	110			
Chloride	4.8	0.50	5.000	0	96.2	90	110			
Nitrogen, Nitrite (As N)	0.98	0.10	1.000	0	98.2	90	110			
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	101	90	110			
Sulfate	9.8	0.50	10.00	0	97.5	90	110			

Sample ID <b>1205153-001AMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>PW-3</b>	Batch ID: <b>R2561</b>		RunNo: <b>2561</b>							
Prep Date:	Analysis Date: <b>5/3/2012</b>		SeqNo: <b>71257</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	2.4	0.10	0.5000	1.941	84.8	72.9	113			
Nitrogen, Nitrite (As N)	0.96	0.10	1.000	0	96.5	77.6	111			
Nitrogen, Nitrate (As N)	3.3	0.10	2.500	0.7031	102	82.8	116			
Sulfate	37	0.50	10.00	26.34	106	80.5	119			

Sample ID <b>1205153-001AMSD</b>	SampType: <b>MSD</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>PW-3</b>	Batch ID: <b>R2561</b>		RunNo: <b>2561</b>							
Prep Date:	Analysis Date: <b>5/3/2012</b>		SeqNo: <b>71258</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	2.4	0.10	0.5000	1.941	84.1	72.9	113	0.155	20	
Nitrogen, Nitrite (As N)	0.92	0.10	1.000	0	92.4	77.6	111	4.30	20	
Nitrogen, Nitrate (As N)	3.1	0.10	2.500	0.7031	97.9	82.8	116	3.36	20	
Sulfate	37	0.50	10.00	26.34	102	80.5	119	1.04	20	

Sample ID <b>1205167-005AMS</b>	SampType: <b>MS</b>		TestCode: <b>EPA Method 300.0: Anions</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>R2561</b>		RunNo: <b>2561</b>							
Prep Date:	Analysis Date: <b>5/3/2012</b>		SeqNo: <b>71285</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205167-005AMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71285</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Nitrogen, Nitrite (As N)	0.94	0.10	1.000	0	94.4	77.6	111			

Sample ID	<b>1205167-005AMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71286</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Nitrogen, Nitrite (As N)	0.94	0.10	1.000	0	94.4	77.6	111	0.0232	20	

Sample ID	<b>MB</b>	SampType:	<b>MBLK</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>71314</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	ND	0.10								
Chloride	ND	0.50								
Nitrogen, Nitrite (As N)	ND	0.10								
Nitrogen, Nitrate (As N)	ND	0.10								
Sulfate	ND	0.50								

Sample ID	<b>LCS</b>	SampType:	<b>LCS</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>71315</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	0.51	0.10	0.5000	0	101	90	110			
Chloride	4.7	0.50	5.000	0	93.9	90	110			
Nitrogen, Nitrite (As N)	0.96	0.10	1.000	0	96.1	90	110			
Nitrogen, Nitrate (As N)	2.5	0.10	2.500	0	98.0	90	110			
Sulfate	9.5	0.50	10.00	0	94.7	90	110			

Sample ID	<b>1205174-001BMS</b>	SampType:	<b>MS</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>71317</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	1.4	0.10	0.5000	0.9876	91.1	72.9	113			
Chloride	14	0.50	5.000	8.329	103	78	107			
Nitrogen, Nitrite (As N)	0.96	0.10	1.000	0	95.8	77.6	111			
Nitrogen, Nitrate (As N)	6.0	0.10	2.500	3.372	106	82.8	116			
Sulfate	45	0.50	10.00	35.20	102	80.5	119			

**Qualifiers:**

- \* / X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits
- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205174-001BMSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>EPA Method 300.0: Anions</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2561</b>	RunNo:	<b>2561</b>					
Prep Date:		Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>71318</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Fluoride	1.4	0.10	0.5000	0.9876	90.1	72.9	113	0.330	20	
Chloride	13	0.50	5.000	8.329	103	78	107	0.0337	20	
Nitrogen, Nitrite (As N)	0.96	0.10	1.000	0	95.7	77.6	111	0.0653	20	
Nitrogen, Nitrate (As N)	6.0	0.10	2.500	3.372	106	82.8	116	0.00611	20	
Sulfate	45	0.50	10.00	35.20	101	80.5	119	0.199	20	

**Qualifiers:**

\*X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205170-001D</b>	SampType:	<b>DUP</b>	TestCode:	<b>EPA 120.1: Specific Conductance</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2646</b>	RunNo:	<b>2646</b>					
Prep Date:		Analysis Date:	<b>5/7/2012</b>	SeqNo:	<b>73516</b>	Units:	<b>µmhos/cm</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Conductivity	610	0.010						0	20	

**Qualifiers:**

\*X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit



# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205005-001A DUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM4500-H+B: pH</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71363</b>	Units:	<b>pH units</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	3.92	1.68						0.762		H

Sample ID	<b>1205120-001B DUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM4500-H+B: pH</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71373</b>	Units:	<b>pH units</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
pH	7.73	1.68						0.645		H

**Qualifiers:**

\*X Value exceeds Maximum Contaminant Level.  
 E Value above quantitation range  
 J Analyte detected below quantitation limits  
 R RPD outside accepted recovery limits

B Analyte detected in the associated Method Blank  
 H Holding times for preparation or analysis exceeded  
 ND Not Detected at the Reporting Limit  
 RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>1205005-001A MS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71221</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	62.6	110			S

Sample ID	<b>1205005-001A MSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71222</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	ND	20	80.00	0	0	59.9	111	0	10	S

Sample ID	<b>1205120-001B MS</b>	SampType:	<b>MS</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71242</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	360	20	80.00	299.4	70.9	62.6	110			

Sample ID	<b>1205120-001B MSD</b>	SampType:	<b>MSD</b>	TestCode:	<b>SM2320B: Alkalinity</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>R2560</b>	RunNo:	<b>2560</b>					
Prep Date:		Analysis Date:	<b>5/3/2012</b>	SeqNo:	<b>71243</b>	Units:	<b>mg/L CaCO3</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Alkalinity (as CaCO3)	350	20	80.00	299.4	67.1	59.9	111	0.869	10	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID <b>MB-1832</b>	SampType: <b>MBLK</b>		TestCode: <b>SM2540C MOD: Total Dissolved Solids</b>							
Client ID: <b>PBW</b>	Batch ID: <b>1832</b>		RunNo: <b>2634</b>							
Prep Date: <b>5/7/2012</b>	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73329</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	ND	20.0								

Sample ID <b>LCS-1832</b>	SampType: <b>LCS</b>		TestCode: <b>SM2540C MOD: Total Dissolved Solids</b>							
Client ID: <b>LCSW</b>	Batch ID: <b>1832</b>		RunNo: <b>2634</b>							
Prep Date: <b>5/7/2012</b>	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73330</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	1,020	20.0	1,000	0	102	80	120			

Sample ID <b>1205078-002GMS</b>	SampType: <b>MS</b>		TestCode: <b>SM2540C MOD: Total Dissolved Solids</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>1832</b>		RunNo: <b>2634</b>							
Prep Date: <b>5/7/2012</b>	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73337</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,890	20.0	1,000	3,791	110	80	120			

Sample ID <b>1205078-002GMSD</b>	SampType: <b>MSD</b>		TestCode: <b>SM2540C MOD: Total Dissolved Solids</b>							
Client ID: <b>BatchQC</b>	Batch ID: <b>1832</b>		RunNo: <b>2634</b>							
Prep Date: <b>5/7/2012</b>	Analysis Date: <b>5/8/2012</b>		SeqNo: <b>73338</b>		Units: <b>mg/L</b>					
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Total Dissolved Solids	4,930	20.0	1,000	3,791	114	80	120	0.733	20	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

# QC SUMMARY REPORT

## Hall Environmental Analysis Laboratory, Inc.

WO#: 1205153

14-May-12

**Client:** New Mexico Copper Corp

**Project:** Cu Flat

Sample ID	<b>MB-1808</b>	SampType:	<b>MBLK</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>PBW</b>	Batch ID:	<b>1808</b>	RunNo:	<b>2606</b>					
Prep Date:	<b>5/4/2012</b>	Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>72551</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0								

Sample ID	<b>LCS-1808</b>	SampType:	<b>LCS</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>LCSW</b>	Batch ID:	<b>1808</b>	RunNo:	<b>2606</b>					
Prep Date:	<b>5/4/2012</b>	Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>72552</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	94	4.0	96.60	0	97.3	82.9	110			

Sample ID	<b>1205122-001BDUP</b>	SampType:	<b>DUP</b>	TestCode:	<b>SM 2540D: TSS</b>					
Client ID:	<b>BatchQC</b>	Batch ID:	<b>1808</b>	RunNo:	<b>2606</b>					
Prep Date:	<b>5/4/2012</b>	Analysis Date:	<b>5/4/2012</b>	SeqNo:	<b>72556</b>	Units:	<b>mg/L</b>			
Analyte	Result	PQL	SPK value	SPK Ref Val	%REC	LowLimit	HighLimit	%RPD	RPDLimit	Qual
Suspended Solids	ND	4.0						0	15	

**Qualifiers:**

- \*X Value exceeds Maximum Contaminant Level.
- E Value above quantitation range
- J Analyte detected below quantitation limits
- R RPD outside accepted recovery limits

- B Analyte detected in the associated Method Blank
- H Holding times for preparation or analysis exceeded
- ND Not Detected at the Reporting Limit
- RL Reporting Detection Limit

**Sample Log-In Check List**

Client Name: **NEW MEXICO COPPER CORP** Work Order Number: 1205153  
 Received by/date: AT 05/03/12  
 Logged By: **Anne Thorne** 5/3/2012 8:35:00 AM *Anne Thorne*  
 Completed By: **Anne Thorne** 5/3/2012 *Anne Thorne*  
 Reviewed By: AT 05/03/12

**Chain of Custody**

- 1. Were seals intact? Yes  No  Not Present
- 2. Is Chain of Custody complete? Yes  No  Not Present
- 3. How was the sample delivered? Client

**Log In**

- 4. Coolers are present? (see 19. for cooler specific information) Yes  No  NA
- 5. Was an attempt made to cool the samples? Yes  No  NA
- 6. Were all samples received at a temperature of >0° C to 6.0°C Yes  No  NA
- 7. Sample(s) in proper container(s)? Yes  No
- 8. Sufficient sample volume for indicated test(s)? Yes  No
- 9. Are samples (except VOA and ONG) properly preserved? Yes  No
- 10. Was preservative added to bottles? Yes  No  NA
- 11. VOA vials have zero headspace? Yes  No  No VOA Vials
- 12. Were any sample containers received broken? Yes  No
- 13. Does paperwork match bottle labels?  
(Note discrepancies on chain of custody) Yes  No
- 14. Are matrices correctly identified on Chain of Custody? Yes  No
- 15. Is it clear what analyses were requested? Yes  No
- 16. Were all holding times able to be met?  
(If no, notify customer for authorization.) Yes  No

# of preserved bottles checked for pH: 3  
 (<2 or >12 unless noted)  
 Adjusted? \_\_\_\_\_  
 Checked by: AT 05/03/12

**Special Handling (if applicable)**

- 17. Was client notified of all discrepancies with this order? Yes  No  NA

Person Notified:	_____	Date:	_____
By Whom:	_____	Via:	<input type="checkbox"/> eMail <input type="checkbox"/> Phone <input type="checkbox"/> Fax <input type="checkbox"/> In Person
Regarding:	_____		
Client Instructions:	_____		

18. Additional remarks:

**19. Cooler Information**

Cooler No	Temp °C	Condition	Seal Intact	Seal No	Seal Date	Signed By
1	1.6	Good	Not Present			

# Chain-of-Custody Record

Client: NMCC

Mailing Address: 2425 San Pedro NE Ste 100

Albuquerque, NM

Phone #: 505-799-1925

email or Fax#:

QA/QC Package:  
 Standard  Level 4 (Full Validation)

Accreditation  
 NELAP  Other \_\_\_\_\_

EDD (Type) \_\_\_\_\_

Turn-Around Time: Need Results May 11  
 Standard  Rush

Project Name: Cu Flat

Project #: Production Well Sampling

Project Manager: Kate Farmer

Sampler: CMC  
 Office:  Yes  No  
 Sample Temperature: 11.0

Date	Time	Matrix	Sample Request ID	Container Type and #	Preservative Type	HEAL No.
5-2-12	14:30	H <sub>2</sub> O	PW-3	500	none	205153
				125	H <sub>2</sub> SO <sub>4</sub>	
				125	HNO <sub>3</sub> filter	
				500	HNO <sub>3</sub>	
				500	NaOH	

Date: 5-8-12 Time: 8:135 Relinquished by: [Signature]  
 Date: \_\_\_\_\_ Time: \_\_\_\_\_ Relinquished by: \_\_\_\_\_

Received by: [Signature] Date: 5/10/12 Time: 13:35  
 Received by: \_\_\_\_\_ Date: \_\_\_\_\_ Time: \_\_\_\_\_

Analysis Request	
BTEX + MTBE + TMBs (8021)	
BTEX + MTBE + TPH (Gas only)	
TPH Method 8015B (Gas/Diesel)	
TPH (Method 418.1)	
EDB (Method 504.1)	
8310 (PNA or PAH)	
RCRA 8 Metals	
Anions (F <sub>2</sub> , Cl <sub>2</sub> , NO <sub>3</sub> , NO <sub>2</sub> , PO <sub>4</sub> , SO <sub>4</sub> )	
8081 Pesticides / 8082 PCB's	
8260B (VOA)	
8270 (Semi-VOA)	
Air Bubbles (Y or N)	

Remarks: Please add Hardware per Andy of Lab  
Need Results by May 11 5/3  
Please email to: Kemmer@the-mac-resource-group.com



**HALL ENVIRONMENTAL ANALYSIS LABORATORY**  
 www.hallenvironmental.com

4901 Hawkins NE - Albuquerque, NM 87109  
 Tel. 505-345-3975 Fax 505-345-4107

If necessary, samples submitted to Hall Environmental may be subcontracted to other accredited laboratories. This serves as notice of this possibility. Any sub-contracted data will be clearly notated on the analytical report.

**Table 9-3  
Analytical Parameters and Analysis Methods for Groundwater Samples**

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
<b>Anions</b>		
Fluoride	EPA Method 300.0	0.1
Chloride	EPA Method 300.0	0.1
Nitrogen, Nitrite (as N)	EPA Method 300.0	0.1
Nitrogen, Nitrate (as N)	EPA Method 300.0	0.1
Sulfate	EPA Method 300.0	0.5
<b>Dissolved Metals</b>		
Aluminum	EPA Method 200.7	0.02
Antimony	EPA Method 200.8	0.005
Arsenic	EPA Method 200.8	0.02
Barium	EPA Method 200.7	0.002
Beryllium	EPA Method 200.7	0.002
Boron	EPA Method 200.7	0.04
Cadmium	EPA Method 200.7	0.002
Calcium	EPA Method 200.7	0.50
Chromium	EPA Method 200.7	0.006
Cobalt	EPA Method 200.7	0.006
Copper	EPA Method 200.7	0.0003
Iron	EPA Method 200.7	0.02
Lead	EPA Method 200.7	0.005
Magnesium	EPA Method 200.7	0.50
Manganese	EPA Method 200.7	0.002
Mercury	EPA Method 7470 CVAA	0.0002
Molybdenum	EPA Method 200.7	0.008
Nickel	EPA Method 200.7	0.01
Potassium	EPA Method 200.7	1.0
Selenium	EPA Method 200.8	0.02
Silicon	EPA Method 200.7	0.08
Silver	EPA Method 200.7	0.005
Sodium	EPA Method 200.7	0.5

Analytical Parameter	Analysis Method	Lab Detection Limit (mg/L unless noted)
Thallium	EPA Method 200.7	0.01
<del>Titanium</del>	<del>EPA Method 200.7</del>	<del>0.005</del>
Uranium	EPA Method 200.8	0.01
Vanadium	EPA Method 200.7	0.005
Zinc	EPA Method 200.7	0.005
<b>Solids</b>		
Total Suspended Solids (TSS)	SM 2540D	1.0 µg/L
Total Dissolved Solids (TDS)	SM 2540C	10
<b>Alkalinity</b>		
Alkalinity, total (as CaCO <sub>3</sub> )	SM 2320B	20
Carbonate	SM 2320B	20
Bicarbonate	SM 2320B	20
<b>Other</b>		
pH	150.1	12.45
Specific Conductance	120.1	0.01 µS/cm
Cyanide	Kelada-01	0.005

**Note:** NA = not applicable as sample will not be analyzed for a given parameter.



## **Appendix F – JSAI Review of Methods and Assumptions for Predicting Open Pit Water Quality**

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## TECHNICAL MEMORANDUM

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To: Steve Raugust, New Mexico Copper Corporation  
Katie Emmer, New Mexico Copper Corporation

From: Steven T. Finch, Jr., Principal Hydrogeologist-Geochemist, JSAI

Date: December 17, 2014

Subject: Review of methods and assumptions for predicting open pit water quality, Copper Flat Project, New Mexico

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New Mexico Copper Corporation (NMCC) is in the process of obtaining a mining permit for the Copper Flat property near Hillsboro, New Mexico. To determine if the proposed Copper Flat open-pit water would meet New Mexico Water Quality Control Commission (NMWQCC) standards for stock and wildlife use, SRK (2013) prepared a report titled *Predictive Geochemical Modeling of Pit Lake Water Quality at the Copper Flat Project, New Mexico*. The SRK (2013) geochemical model incorporated the water model developed by JSAI (2013). Reviewers of the SRK (2013) report have raised questions about the following issues:

1. More detail is needed to validate the assumption of 10-percent average fracture density in the pit walls and the amount of wall rock available for leaching.
2. More detail is needed to demonstrate that the proposed open pit water body will be well mixed, remain oxygenated, and not chemically stratify.
3. The geochemical model needs to be calibrated to chloride concentrations in the existing open pit to make sure the effects of evaporation are accounted for.

This Technical Memorandum consists of three sections for addressing the issues listed above. Sections 1.0 and 2.0 compare the SRK (2013) approach and assumptions to other open pit geochemical investigations, Section 3.0 presents calibration and sensitivity analysis results of the water model (JSAI, 2013) to historical water-quality data from the existing open pit, and Section 4.0 is a summary of findings.

## 1.0 REVIEW OF OPEN PIT WALL-ROCK STUDIES

### 1.1 SRK (2013) Copper Flat Model

SRK (2013) used different conceptual models of wall rock available for leaching: one for the existing and one for the future Copper Flat open pit. The difference is due to the blasting technique; the existing pit was mined in 1982 using production blasting similar to the blasting effects analyzed by Siskind and Fumanti (1974), and the proposed pit would be mined using presplit drilling and smooth wall blasting practices. The two conceptual models are summarized below.

#### 1.1.1 Existing Open Pit

For the existing Copper Flat open pit, SRK (2013) estimated 10-percent fracturing in the first 2 ft of open pit wall rock (crushed zone) and 5-percent fracturing for a 3.8-ft-thick transition zone. The limit of oxidation and depth to undisturbed rock was assumed to be about 6 ft behind the pit wall (see fig. 3-9; SRK, 2013). A reactive rim of 0.04 ft around the fractures was assumed for the rock in the pit walls (based on HCT results).

Quintana Minerals only used production blasting to create the existing pit. Production blasting uses large widely-spaced explosive charges that are designed to fragment a large amount of *burden* (the rock that lies between the existing slope face and the blast hole). Production blasting is the most efficient way to remove large rock burdens, but it typically creates radial fractures around the blast hole and back break (fractures that extend into the final slope face), which reduce the strength of the remaining rock mass and increase its susceptibility to slope raveling and rock fall.

#### 1.1.2 Proposed Open Pit

For the future Copper Flat open pit, SRK (2013) estimated fracturing is 10 percent of rock volume for the first 1 ft of open pit wall rock (crushed zone), with no transition zone between the crushed zone and undisturbed zone (see fig. 3-3; SRK, 2013). The open pit wall rock approximate 1 ft from the surface was assumed to be the limit of oxidation and the depth to undisturbed rock (see fig. 3-9, SRK, 2013). A reactive rim of 0.04 ft around the fractures was assumed for the rock in the pit walls. The 1-ft crushed zone and no transition zone represent presplit drilling and smooth wall blasting practices. Presplit holes are blasted before production blasts. Procedure uses small diameter holes at close spacing and lightly loaded with distributed charges. Presplit holes protect the final pit wall cut by producing a fracture plane along the final slope face that fractures from production blasts cannot pass.

#### 1.1.3 Rock Mass Available for Leaching

For both scenarios, water flow is assumed to be mobile in the crushed zone and oxidized rind. The calculation of reactive mass was based on an average rock density of 169 lb/ft<sup>3</sup> (2,700 kg/m<sup>3</sup>).

Chemistry of open pit run-off, for each pit wall material type, is estimated from scaled kinetic test cell (HCT) leachate concentrations. Average HCT solute concentrations are scaled up based on the pit wall water-rock ratio, and computed based on the estimated degree of fracturing and thickness of the reactive rind (SRK, 2013; p. 30).

## 1.2 Review of Pit Wall Fracturing References

### 1.2.1 Blasting Effects

Siskind and Fumanti (1974), a key reference used by SRK (2013), studied the fracturing produced in the vicinity of large-diameter blast holes (production blasting) in Lithonia Granite. The purpose of the Siskind and Fumanti (1974) study was to evaluate the use of production blasting to increase permeability for in-situ mining, where the amount of fracturing between holes is intended to be maximized for economic efficiency. A severely fractured zone was found to extend approximately 25 inches (64 cm) from the center of the 6-1/2-inch (16.5 cm) blast holes. A second zone, characterized by a lesser degree of fracturing, extended from 25 to 45 inches (64 to 114 cm). Beyond 45 inches (114 cm), the rock was undamaged. Carroll and Scott (1966) evaluated blasting effects on quartz monzonite and granodiorite (Climax Stock near Mercury, Nevada) and found that production blasting created an altered zone 0 to 8 ft in depth, and blast damage 2 to 4 ft in depth.

Kelsall and others (1984) found that in granite and basalt blasting enhanced permeability by about 10 times near the blast face, but the extent of blast effects were generally limited to <3.3 ft (<1 m), and possibly as little as 1 ft (0.3 m) when using low-charge blast methods.

It is important to note that granite, granodiorite, and quartz monzonite are similar intrusive rocks with similar rock properties. The primary difference is the quartz and feldspar content. The quartz monzonite at Copper Flat is therefore analogous to the granite and granodiorite in the blasting studies cited above. The Siskind and Fumanti (1974) study cites physical properties of the Lithonia Granite. Recent physical properties of the principal rock types of the Copper Flat Ore are presented in a 2013 report prepared by Mine Design Engineering of Kingston, Ontario, Canada for THEMAC Resources (Mine Design, 2013). The Mine Design report (2013) was prepared for the purposes of engineering the future pit walls for geotechnical stability. Table 1 presents a comparison of selected physical properties Lithonia Granite to the Copper Flat Quartz Monzonite and Quartz Monzonite Breccia.

Figure 1 presents the Copper Flat pit outline (Pre-Feasibility Study; PFS) from the 2013 Mine Design report, which shows the major rock types, their distribution, and the locations of the geotechnical drill holes where the samples from Table 1 were collected. From information presented in Mine Design (2013), and other available information, the Definitive Feasibility Study (DFS) pit geometry was developed. For geochemical characterization purposes, the PFS pit is very similar to the DFS Pit (SRK, 2014).

**Table 1. Summary of the physical properties of the Lithonia Granite with Copper Flat Quartz Monzonite (QM) and Quartz Monzonite Breccia (QMBX)**

Laboratory Analysis	Lithonia Granite (Tested by previous investigators)	Lithonia Granite (Tested by authors at H-100 control hole)	QM (Average Values)	QM (Maximum Values)	QM (Minimum Values)	QMBX (Average Values)	QMBX (Maximum Values)	QMBX (Minimum Values)
Specific Gravity	2.63	-	2.68	-	-	2.57	-	-
Density (lb/ft <sup>3</sup> )	164	-	167	-	-	160	-	-
Tensile Strength (lb/in <sup>2</sup> )	450	-	2,132	3,075	493	1,247	1,697	653
Compressive Strength (lb/in <sup>2</sup> )	30,000	28,000	18,490	29,400	11,810	6,614	6,614	6,614
Young's Modulus (lb/in <sup>2</sup> )	3,000,000	6,400,000	5,018,000	6,135,000	3,626,000	2,973,000	2,973,000	2,973,000
Poisson's Ratio	0.26	-	0.10	0.09	0.11	0.12	0.12	0.12

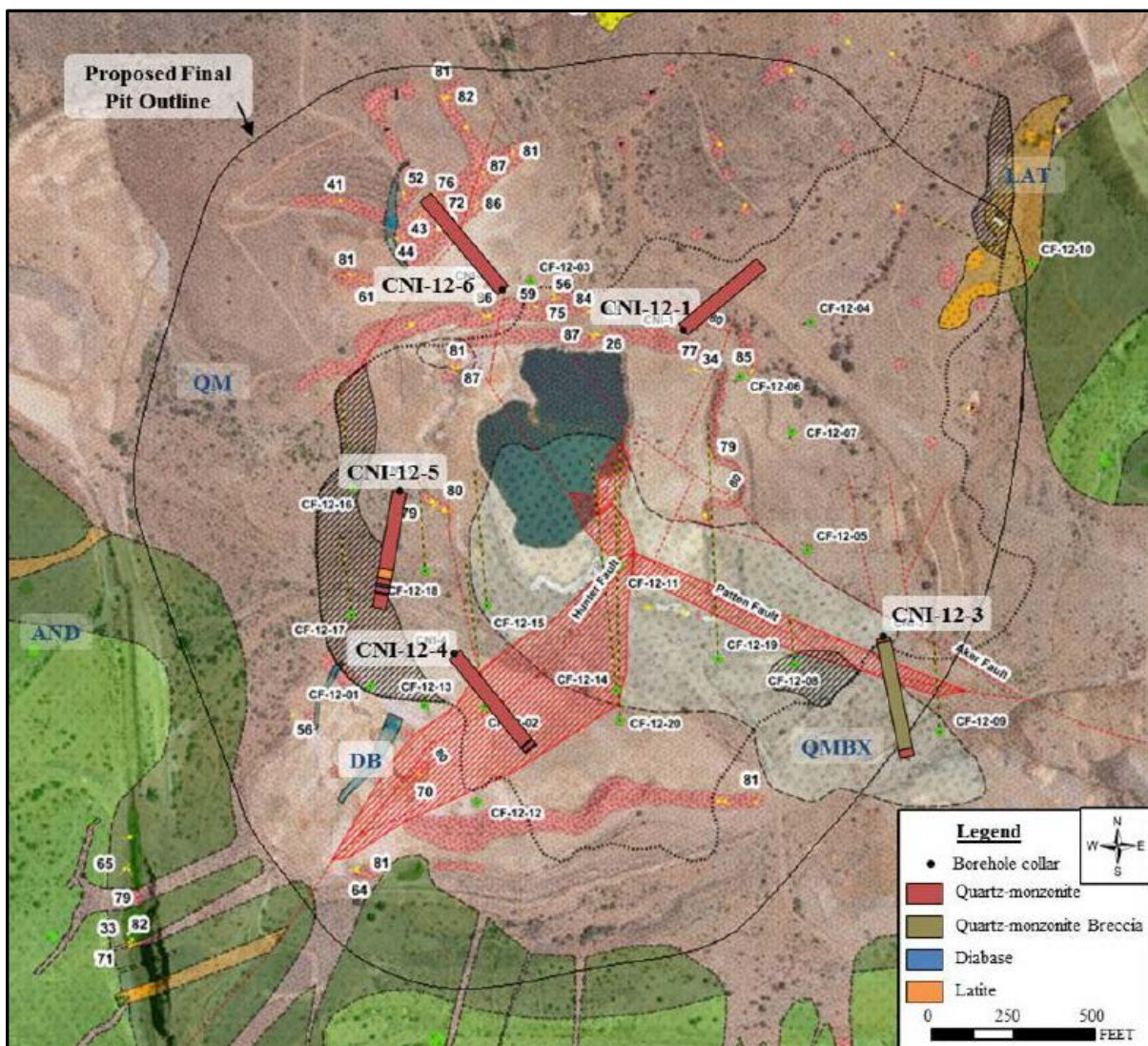


Figure 1. Geotechnical drill hole locations and the Pre-Feasibility Study pit outline (Mine Design, 2013).

### **1.2.2 Fracture Permeability**

Molebatsi and others (2009) noted that many open-pit mines are located in fractured rock systems where water flow paths are complex and difficult to predict. These flow paths are typically controlled by a small subset of fractures that are permeable and interconnected. Most models of flow in fractured rock systems are based on a network of interconnected fractures that are all assumed to be permeable. However, this assumption is rarely observed in natural rocks where a significant number of the fractures within a connected cluster may be impermeable.

Field observations have shown that only a small proportion of fractures contribute to the overall flow, resulting in a complex and heterogeneous flow system. Up to 20 percent of the total number of fractures may contribute to overall flow (Bear et al., 1993). Although fracture connectivity has been used to explain heterogeneous phenomena (de Marsily, 1985), it is likely that additional aspects such as the effect of partial or total closure of individual fractures could further increase flow heterogeneity and tortuosity. Effectively impermeable fractures that (although mappable) will not conduct flow will thus need to be excluded from the conductive fracture cluster.

Not discussed in detail by Molebatsi and others (2009) is the rock type and mineralization of fractures, degree of fracturing, hydraulic conductivity in comparison to fracture density, and specific yield of rock. Obviously, fractured rock with low hydraulic conductivity would have more impermeable fractures than high hydraulic conductivity fractured rock that effectively behaves as a porous medium.

## **1.3 Other Open-Pit Geochemical Models**

### **1.3.1 URS (2009) Little Rock Mine Post-Closure Pit Lake Model**

The Little Rock open pit mine is located near Silver City, New Mexico, and is currently operating. URS (2009) assumed that a mixture of the in-situ field leaching tests and the HCT leachates represents the pit wall runoff. For the most likely case, an equal-weight mixture of the mean in-field leachate results, week-0 HCT results, and HCT results from the first 4-week idle period was used to represent run-on from the exposed pit walls above the pit lake. URS (2009) assumed: 1) rock samples collected within 100 ft of the final pit wall are representative of the exposed wall rock, and 2) a combination of the in-situ field leachates and the HCT leachates mimics weathering of pit wall rock. There is no discussion of blasting effects or increased fracture density on leaching of wall rock.

### **1.3.2 Tetra Tech (2010) Rosemont Copper Project**

The Rosemont Copper project is located in southeastern Arizona. For simulating the initial flushing of blast-fractured pit walls, Tetra Tech (2010) used the first rinse from the HCTs to represent the chemical source terms. The HCT concentrations were generally higher than from the Synthetic Precipitation Leaching Procedure (SPLP) results, which generally correspond to rock that has had more time to weather before contacting water.

The near-surface wall rock of the anticipated ultimate pit shell is expected to be affected by blasting. An initial chemical flushing of the blast-affected pit wall rock was incorporated into the pit lake model. The near-pit wall rock is anticipated to have altered hydraulic properties and increased fracture density as a result of blasting and the extraction of surrounding rock. An increase in the porosity and specific yield (3 to 15 percent) of the near-surface wall rock is expected. The blast-affected wall rock was considered to extend for a distance of six (6) ft behind the ultimate pit wall; there was no basis provided for this assumption.

Where available, the chemical source terms used for flushing of the blast-affected wall rock for each formation were developed using the averaged first-rinse HCT data. Scaling of HCT data was not considered. For formations without HCT data, the concentrations of major cations and anions derived from SPLP tests were multiplied by a factor of three (3) and the trace metals were multiplied by a factor of two (2). Three (3) pore volumes of the blast-affected wall rock were considered in the model for the initial flush, after which standard groundwater inflow chemistry was assumed.

### **1.3.3 Schafer (2007) Betze Pit Lake Water Quality Predictions**

Schafer (2007) estimated the thickness of the weathered zone behind the pit wall by applying the approximate analytical solution (shrinking core model) derived by Davis and others (1986). The shrinking core model considers that particle size and the reactive core shrink simultaneously; therefore, sulfide oxidation rates decrease over time. A porosity of 2 percent was used to represent the highwall, while the rate of interparticle diffusion was determined from historical humidity cell tests. The rate of interparticle diffusion was calculated using the Millington Quirk equation (Jury et al., 1991). For portions of the highwall with relatively low sulfide levels, oxygen can penetrate nearly 16.4 ft (5 m) after 400 years, while the depth of oxygen penetration is closer to 9.8 ft (3 m) after 400 years for higher sulfide zones. The overall average thickness of the oxidized wall rock was estimated to be 9.8 ft (3 m).

### **1.3.4 Schafer (2010) Dee Pit Lake, Arturo Mine**

Schafer (2010) assumes the thickness of a weathered highwall increases with increasing exposure to oxidation. The thickness of the weathered zone was estimated for the Dee pit lakes by applying the approximate analytical solution derived by Davis and others (1986). A porosity of 3 percent was used to represent the highwall. Other data needed to calibrate the Davis and others (1986) equations were determined from pyrite weathering rates observed in humidity cell tests. The rate of interparticle diffusion was calculated using the Millington Quirk equation (Jury et al., 1991). For portions of the highwall with relatively low sulfide levels, oxygen can penetrate over 15 ft (5 m) after 400 years, while the depth of oxygen penetration is closer to 10 ft (3 m) after 400 years for higher sulfide zones (see Fig. 2 below).

### **1.3.5 Adrian Brown (1997) Cunningham Hill Mine Open Pit**

A water model and geochemical model were coupled to predict open pit water quality. The model was calibrated to existing water levels and water-quality data (alkalinity, calcium, and sulfate). Inputs from existing acid wall seepage (AWS) were used to simulate open pit water-rock interactions. The water-quality model was simply a mixing model if open pit water quality remained under-saturated with respect to gypsum.

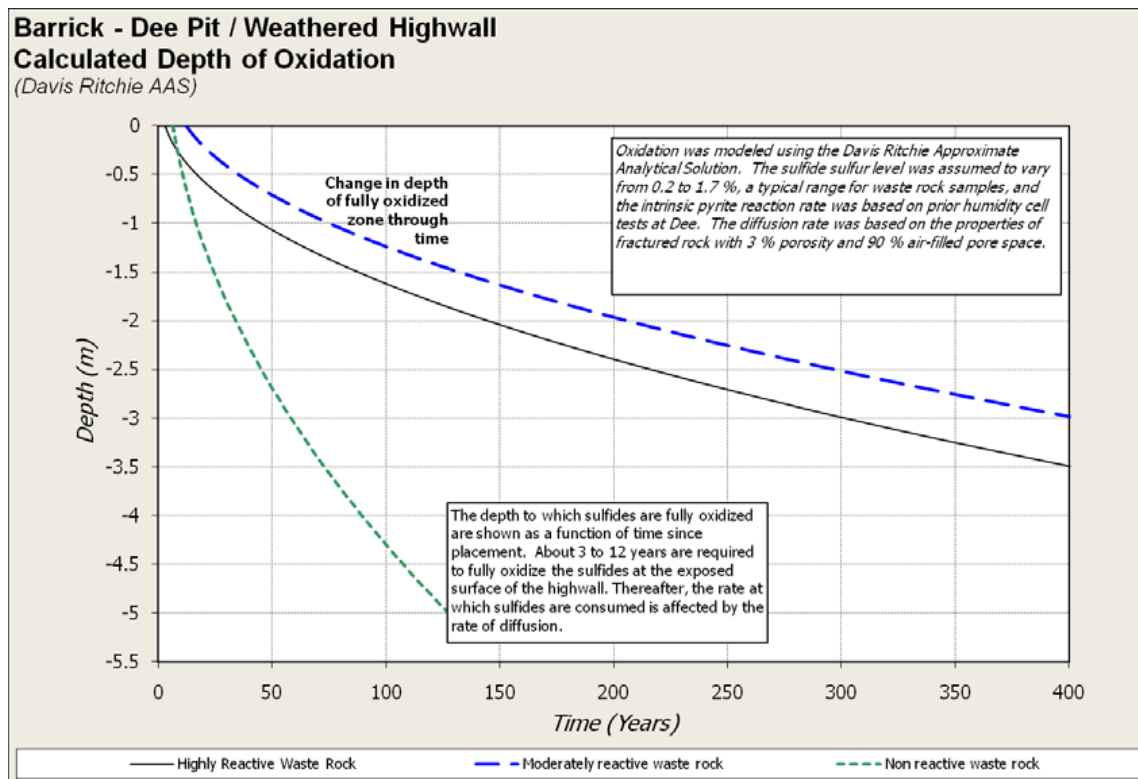


Figure 2. Graph showing depth of oxygen penetration based on the Davis and others (1986) approximate analytical solution (Schafer (2010) Fig. 13).

A groundwater flow and solute transport model of the open pit and surrounding groundwater system was developed by JSAI (1999), and later updated and recalibrated by JSAI (2011). It was demonstrated that the open pit general chemistry is more influenced by water budget components (mixing) than by mineral precipitation reactions.

### 1.3.6 Kempton and Atkins (2009)

Kempton and Atkins (2009) provide a review of methods for predicting water quality in open pits where sulfide oxidation is a major source term. Shrinking core models have been demonstrated to effectively simulate conditions in uniform materials, such as tailings. However, it is difficult to evaluate accuracy in the more heterogeneous pit benches and walls.

Kempton and Atkins (2009) evaluated a method for direct measurement of sulfide oxidation rates in mine pit benches by sealing a drape-chamber apparatus to the surface. They found that application of this method to benches and waste rock have not found the measured oxidation rates to be meaningfully correlated to sulfide sulfur, presence of surface rubble, moisture conditions, or carbonate content of the underlying rock. This suggests that physical processes such as blast-induced wall rock porosity and depth of pit-wall oxidation were more important than chemical processes. It was noted that fracturing is lower in competent rock, such as granite, and that careful blasting can reduce fracturing. Kempton and Atkins (2009) concluded that reliable comparisons of model-simulated versus observed pit lake water quality are needed to accurately assess model capabilities; this is exactly what SRK (2013) has done.



## 1.4 Discussion

Geochemical models for predicting open pit water quality are commonly most sensitive to the water budget components and the calculated solute contributions from sulfide oxidation. Open pit water-quality models with the least accurate predictions have under-estimated the potential for sulfide oxidation in wall rock and poorly represented water budget components (Kuipers and others, 2006). One reason for inaccurate water quality predictions is the lack of historical data for model calibration; most projects do not have an existing open pit water body with good time-series data. In contrast, the proposed Copper Flat open pit geochemical and groundwater flow model is calibrated to an existing open pit water body with 30 years of data.

Open pit wall blast damage for granite, granodiorite, and quartz monzonite rocks extends 2 to 4 ft in depth when assessing effects from production type blasting (Carroll and Scott, 1966; Siskind and Fumanti, 1974; and Kelsall and others, 1984).

Kelsall and others (1984) found that production blasting enhances permeability by about 10 times near the blast face. Molebatsi and others (2009) indicate that a small percentage (<20 percent) of the total fractures will contribute to permeability of the system. Typically, fractured rock groundwater systems are assumed to have a specific yield of less than 5 percent, and commonly less than 1 percent. The calibrated Copper Flat groundwater flow model simulates a specific yield of 0.001 (0.1 percent) in the quartz monzonite. If blast fracturing increased the effective porosity (specific yield) by an order of magnitude, the specific yield of the blast zone would be 1 percent. The 5 to 10 percent fracture density used by SRK (2013) can be considered conservative given the properties of the open pit wall rock estimated from the calibrated groundwater flow model.

A summary of the case studies reviewed is presented in Table 2. SRK (2013) is the only open pit water-quality model that includes blasting effects in the pit walls, scaled HCT data, and calibration to existing pit water chemistry.

**Table 2. Summary of open pit water-quality prediction studies**

reference	open pit	pit wall fracture assumptions	sulfide oxidation model	calibration to existing pit
SRK (2013)	Copper Flat	5 - 10 % fracture density (porosity) with depth based on blasting method; ranging from 1 to 6 ft	based on scaled HCT data	yes
Adrian Brown (1997)	Cunningham Hill	used measured acid wall seepage (AWS) data	used measured AWS data	yes
URS (2009)	Little Rock	none	based on HCT data	no
Tetra Tech (2010)	Rosemont	3 to 6% porosity, 6 ft depth	based on HCT data	no
Schafer (2007)	Betze	2 % porosity with oxidation depth increasing with time; 10 to 16 ft after 400 years	shrinking core model	no
Schafer (2010)	Dee	3 % porosity with oxidation depth increasing with time; 10 to 15 ft after 400 years	shrinking core model	no

## 2.0 STRATIFICATION OF OPEN PIT WATER BODIES

SRK (2013) concluded the proposed Copper Flat pit will not stratify, and will remain oxygenated. The proposed Copper Flat open pit water body will have a maximum depth of approximately 200 ft with a maximum surface area of about 22 acres.

### 2.1 Overview

Based on elevation and latitude, the Copper Flat open pit water body is classified as a warm monomitic type lake (Wetzel, 2001; fig 6-7). A warm monomitic lake mixes freely once a year in the winter at or above 4 °C. However, wind effects and water body geometry can have an effect on the degree and frequency of mixing. Baseline data (INTERA, 2012) from the existing pit water body provides evidence that a thermocline develops in the summer and mixing occurs in the winter. A chemocline does not develop, and the water body remains oxygenated (dissolved oxygen = 6 to 9 mg/L) throughout the full water column year-round. The existing open pit water body has an area of about 5 acres, maximum depth of 30 ft, and length of about 460 ft.

The relative depth (RD) of the predicted Copper Flat open pit water body at the maximum pit water stage is approximately 18 percent. RD relates the maximum depth of a lake (Z) to the width (d). Assuming an approximately circular lake, the width is a function of surface area (A) and can be determined from:

$$d = 2(A/\pi)^{0.5}$$

The percent RD is defined as:

$$RD = (Z/d)*100 \text{ percent}$$

The estimated RD of 18 percent is considerably greater than 5 percent, which typically suggests that the lake is likely to stratify. Such stratification may result in oxidizing conditions in the upper portions of the lake and more chemically reducing (oxygen-deprived) conditions at depth. However, pit lakes that form in arid regions are unlikely to stratify, relative to lakes that form in cooler, wetter climates (Jewell, 2009). A prerequisite for permanent stratification is that precipitation plus runoff is greater than evaporation during the summer months when the water body is potentially undergoing temporary thermal stratification (Jewell, 2009).

While stratification of an open pit water body has implications for water quality at depth, the near-surface waters will remain oxidized. These near-surface waters are considered the most important from an open pit water-quality perspective given the potential ecological risks associated with them. The water quality at depth is less important given the expected terminal nature of the open pit water body.

## 2.2 Case Studies

Jewell (2009) evaluated six permanently-stratified and eight open pit lakes with seasonal thermocline, and concludes that permanently stratified lakes have vertical density contrast greater than  $0.0005 \text{ g/cm}^3$  and a Wedderburn number greater than 1. The Wedderburn number considers thermocline depth, maximum lake length, water density, and wind speed. Jewell (2009) failed to note that most permanently-stratified open pit lakes receive AWS inputs and have acidic water. A summary table of existing open pit water bodies and their characteristics is presented in Table 3.

**Table 3. Summary of open pit water bodies and stratification characteristics**

open pit	location	effective length (ft)	maximum depth (ft)	relative depth (percent)	thermocline depth (ft)	acidic
<b>permanently stratified</b>						
Brenda	B.C.	2,296	492	21	39	no
Spenceville	California	253	50	20	13	yes
Berkeley	Montana	5,900	426	7	23	yes
<b>Seasonal thermocline and well mixed</b>						
Humbolt	Nevada	944	137	15	8	no
Blackhawk	Utah	492	na	na	33	no
Blowout	Utah	656	230	35	39	no
Colosseum	California	482	157	33	na	no
Cunningham Hill	NM	407	90	22	20	no
Copper Flat (existing)	NM	537	30	6	20	no <sup>1</sup>
Copper Flat (proposed)	NM	1,105	200	18	TBD	no
Yerington	Nevada	5,412	400	13	49	no

<sup>1</sup> there have been temporary acidic conditions where the pit water naturally neutralizes over time

TBD - to be determined

## 2.3 Discussion

The proposed Copper Flat open pit is expected to have a seasonal thermocline, be well mixed, oxygenated, and not acidic. Relative depth does not appear to govern the conditions for creating a permanently stratified open pit water body; however, acidic water and higher latitude are key conditions for creating permanent stratification.

### 3.0 COPPER FLAT OPEN PIT WATER MODEL

The Copper Flat open pit and groundwater flow model (water model) developed by JSAI (2013) was calibrated to water levels, water budgets, and hydraulic properties. The water model was used by SRK (2013) in the geochemical model. The JSAI (2013) water model was an interim version that was finalized in 2014, but the pit water balance did not change.

The water model is used here to address calibration to the Copper Flat open pit evaporation. Evaporation accounts for all of the outflow from the open pit water body; however, the water model only simulates average climate conditions. Figures 3 through 5 illustrate the model-simulated effects of evaporation on total dissolved solids, (TDS), sulfate, and chloride concentrations in the open pit when considering mixing without mineral precipitation.

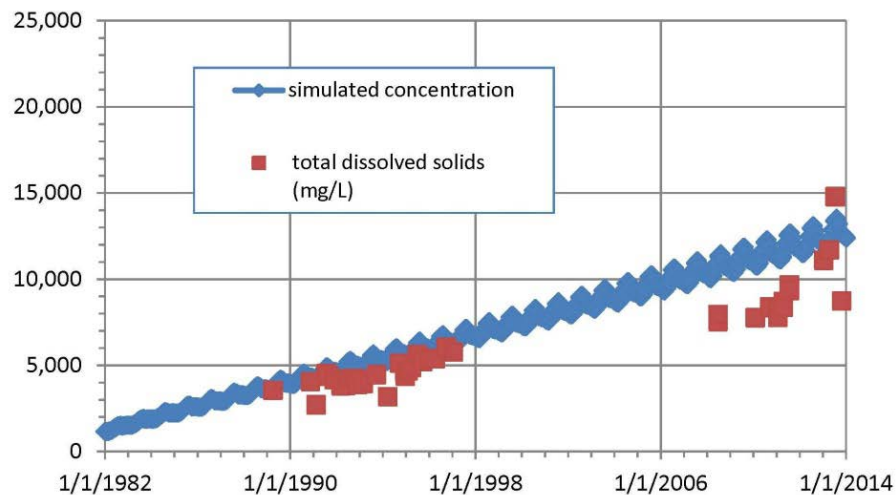


Figure 3. Graph showing water-model simulated and measured TDS concentrations for the Copper Flat open pit water body.

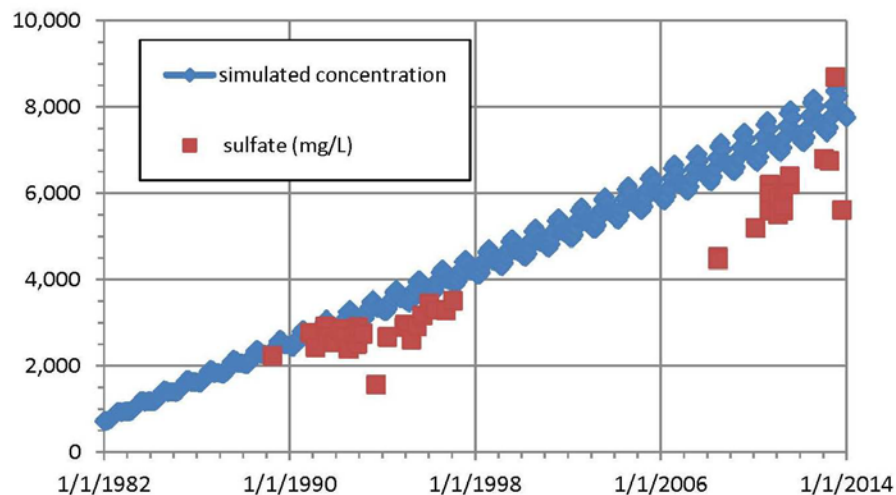


Figure 4. Graph showing water-model simulated and measured sulfate concentrations for the Copper Flat open pit water body.

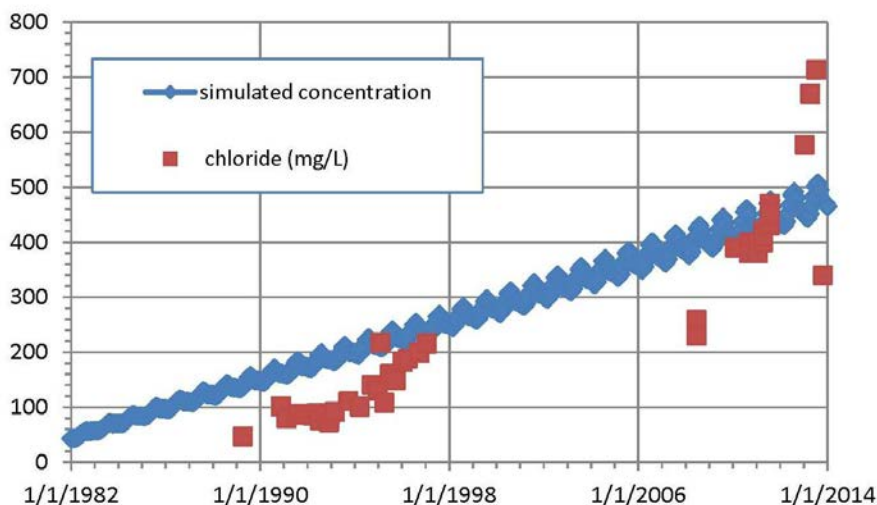


Figure 5. Graph showing water-model simulated and measured chloride concentrations for the Copper Flat open pit water body.

Data collected during 2013 show the evapo-concentration effects of extreme drought with concentrations well above the model-simulated concentrations, but 4<sup>th</sup> quarter 2013 concentrations were well below the model-simulated concentrations, due to a heavy monsoon period (Figs. 3 through 5). The model appears to reasonably simulate the average climate conditions.

SRK (2013) calibration of the geochemical model to existing pit conditions was performed for the 2011 dataset. The geochemical model considers mixing from the water model and mineral precipitation reactions. The geochemical model calibrates to TDS and sulfate better than the water model with mixing alone, but the water model calibrates better to chloride concentrations than the geochemical model (Table 4). The effects of evaporation are reasonably calibrated in the water model and reflected in the geochemical model.

**Table 4. Comparison of water-model and geochemical-model simulated TDS, chloride, and sulfate concentrations to measured concentrations, Copper Flat open pit**

constituent	2010-2011 measured range (mg/L)	geochemical- model results (mg/L)	water-model results (mg/L)
total dissolved solids (TDS)	7,770 to 9,410	7,751	11,621
sulfate	5,200 to 6,400	5,152	7,263
chloride	380 to 470	235	436

mg/L - milligrams per liter

#### 4.0 SUMMARY OF FINDINGS

In summary, SRK (2013) assumptions used for reactive wall thickness and fracture density for the existing and proposed future pit are reasonable and supported by detailed studies pertaining to blasting effects on quartz monzonite rocks cited in Section 1.0. SRK (2013) used fracture-density results reflective of production blasting for the existing Quintana pit walls, and fracture density results reflective of low-charge blasting methods for the future open pit. Sensitivity of model results to fracture density and reactive wall thickness is reflected in these two simulations.

Out of the case studies reviewed (Table 2), SRK (2013) is the only open pit water quality model that considers blasting effects in the pit walls, scaled HCT data, and calibration to existing pit water chemistry. Calibration of the water model and geochemical model to existing data strengthens the ability to accurately predict future conditions.

Relative depth does not appear to govern the conditions for creating a permanently stratified open pit water body; however, significant acidic water inputs and higher latitude are key conditions for creating permanent stratification. The proposed Copper Flat open pit is expected to be seasonally stratified (thermocline only), well mixed, oxygenated, and not acidic. Baseline data from profiles in the existing pit at Copper Flat support the conclusion that the proposed pit will be well mixed and oxygenated.

Using the water model to simulate mixing and evapoconcentration effects on chloride, sulfate, and TDS demonstrates that the water model is calibrated to the effects of evaporation. The results in Table 4 compare simulated evapoconcentration with no mineral precipitation (water model only) to simulated evapoconcentration with mineral precipitation (water model and geochemical model). This comparison of model results to historical data is a sensitivity analysis that shows that the water and geochemical models are well calibrated to effects of evaporation.

The SRK (2013) geochemical model is representative of expected conditions at Copper Flat, and presents the best technical approach for predicting water quality at the future Copper Flat open pit.

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## **Appendix G – JSAI Future Pit Water Balance**



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## TECHNICAL MEMORANDUM

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To: Jeff Smith, New Mexico Copper Corporation [jsmith@themacresourcesgroup.com](mailto:jsmith@themacresourcesgroup.com)

From: Steve Finch, Principal Hydrogeologist-Geochemist  
Michael A. Jones, Principal Hydrologist

Date: September 25, 2017

Subject: Post reclamation open pit surface area storm-water runoff calculations, Copper Flat Project, New Mexico Copper Corporation

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John Shomaker & Associates, Inc. (JSAI) developed and calibrated a groundwater flow model for the New Mexico Copper Corporation (NMCC) Copper Flat project (JSAI, 2014), which included the proposed Copper Flat open pit. The model was calibrated to historical and current conditions at the Copper Flat Open Pit, and used to predict effects of the proposed mining plan.

The purpose of this technical memorandum is to establish storm-water runoff coefficients and watershed areas representative of the post-mining reclamation of the proposed Copper Flat Open Pit Surface Drainage Area (OPSDA). The post-mining OPSDA and watershed areas discussed in this memo are shown on Figure 1.

After reclamation, there will be three areas with different runoff coefficients inside the OPSDA:

1. Reclaimed watershed area surrounding the open pit;
2. Reclaimed sections of the Open Pit shell; and
3. Un-Reclaimed sections of the Open Pit shell.

Curve numbers for the different areas shown on Figure 1 and listed in Table 1 were derived from the NRCS Part 630 Hydrology National Engineering Handbook. The curve number equation (from NRCS, 2004) and precipitation statistics from the Hillsboro station were used to develop the assigned runoff coefficients presented in Table 1.

Post mining OPSDA reclamation will include re-contouring, placement of cover materials, and revegetation. As described in the NMCC Baseline Data Report, cover materials will resemble sandy to silty loam representative of Hydrologic Soil Group B (NRCS, 2009).

The hydrologic conditions of the reclaimed OPSDA will be classified as poor to fair, resembling desert shrub with less than 40 percent vegetative cover (NRCS, 2004). A Curve Number of 75 is representative of Desert Shrub landscape, Hydrologic Soil Group B, and less than 40 percent vegetative cover (NRCS, 2004; table 9-1).

**Table 1. Summary of corresponding Curve Number and assigned Runoff Coefficient for sub-regions within the reclaimed Copper Flat Open Pit Surface Drainage Area**

sub-region name	corresponding Curve Number	assigned Runoff Coefficient
Reclaimed OPSDA	75	0.071
Reclaimed Pit Shell	90	0.303
Un-Reclaimed Pit Shell	80	0.126

The reclaimed pit shell includes the haul road and potentially other accessible areas. Reclaimed surface is expected to resemble improved dirt road, and have a corresponding runoff curve number of 90 (NRCS, 2004; table 9-1).

The un-reclaimed pit shell was assigned a runoff curve number of 80, which has been derived from water balance studies for other open pits, such as the Cunningham Hill Mine Reclamation Project (JSAI, 2012).

Precipitation statistics were used with the runoff curve number to calculate the runoff coefficient presented in Table 1. Surface-water runoff is calculated from daily precipitation data, and soil conditions represented by a runoff curve number (NRCS, 2004a). Runoff is estimated using the following equations:

$$I_a = S * 0.2$$

$$S = (1,000/CN) - 10$$

$$Q = \frac{(P - I_a)^2}{(P - I_a) + S}$$

where,

**I<sub>a</sub>** equals the initial abstraction including surface storage, interception by vegetation and infiltration prior to runoff, in inches depth over the drainage area.

**S** equals the potential maximum retention of water by the soil in equivalent inches depth over the drainage area.

**CN** equals the runoff curve number

**P** equals the accumulated rainfall in inches depth over the drainage area

**Q** equals the accumulate volume of runoff in inches depth over the drainage area

The runoff equations (above) are used to calculate the average annual runoff for the period of record from the Hillsboro Station. An example for Curve Number equal 90 is presented in Table 2. The calculated average annual runoff for period of record is divided by the average annual precipitation for period of record (12.5 in./yr) to derive the runoff coefficient.

**Table 2. Summary of Hillsboro Station precipitation statistics and calculated runoff used to derive runoff coefficient for reclaimed pit shell area (CN=90)**

Range in daily precipitation on events	No. of daily precipitation events within range for period of record*	Average number of precipitation events per year for period of record	average magnitude of precipitation event for range (in.)	P-Ia for CN =90	runoff per average event for range (in.)	average runoff per year (in.)
>3	3	0.031	3.29	3.070	2.86	0.090
2 - 3	21	0.219	2.31	2.090	1.89	0.414
1 - 2	168	1.752	1.32	1.100	0.92	1.606
0.5 - 1	490	5.109	0.7	0.480	0.33	1.682
sum						3.79
Runoff coefficient (CN=90) = (3.79 in)/(12.5 in) = 0.303						

\* Hillsboro station period of record equals 95.9 years or 35,037 days with average annual precipitation of 12.5 inches per year

**Attachments**

Figure 1. Map showing post-mining watershed areas for the Copper Flat Open Pit Drainage Area.

**References**

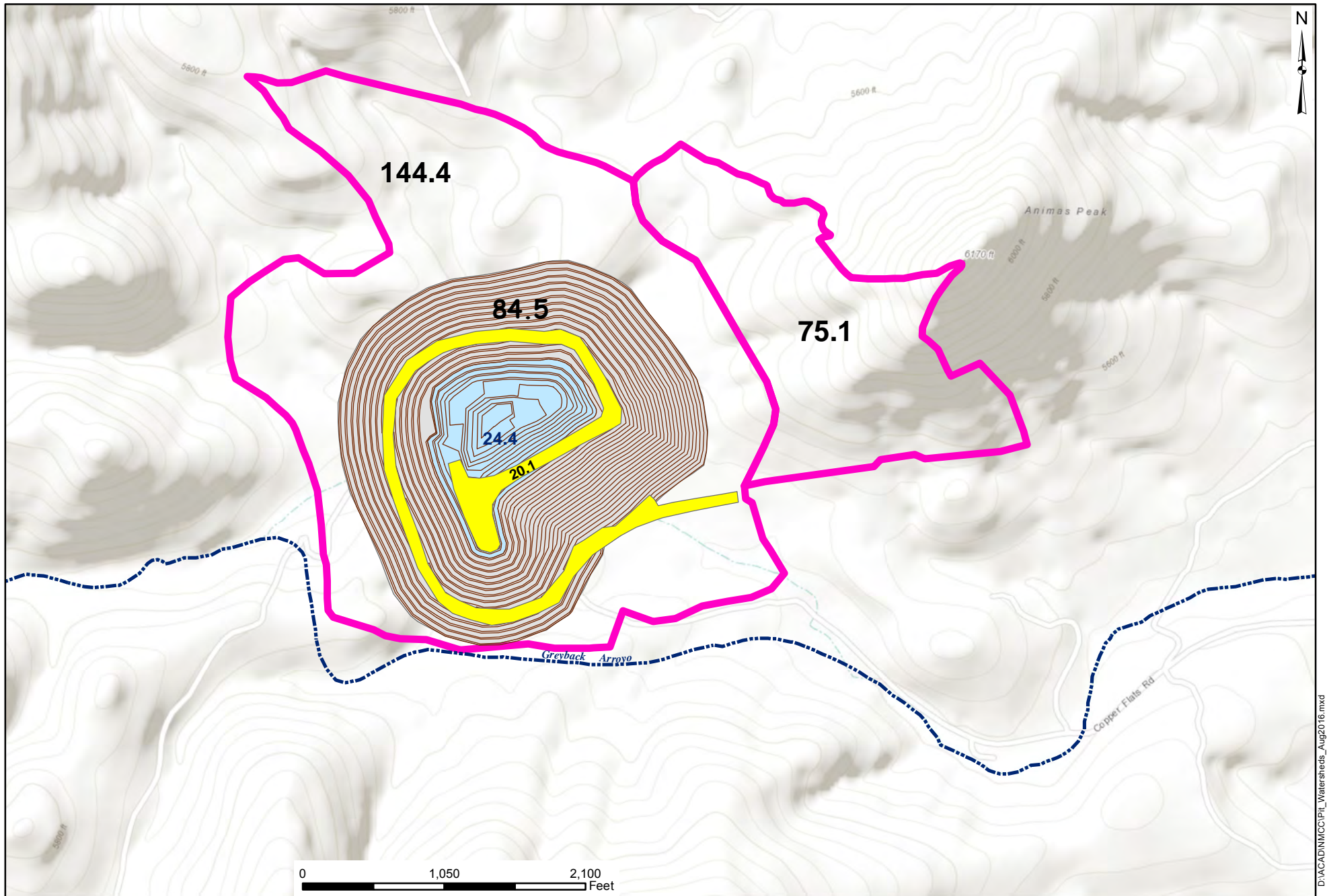
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Figure 1. Map showing post-mining watershed areas for the Copper Flat Open Pit Drainage Area.

**JSAI Pit Water Balance  
Summary Statistics**

<b>Parameter</b>	<b>Units</b>	<b>Pit Model</b>	
		<b>Un Reclaimed</b>	<b>Reclaimed</b>
Model Date		Jun 2017	Jul 2017
Pit Fill Method		Natural Fill	Rapid Fill
Total Pit Watershed	Acres	314	314
Watershed Ex-Pit	Acres	185	185
Watershed In-Pit	Acres	129	129
Pit Reclaimed Surfaces	Acres	0	46
Pit Unreclaimed Surfaces	Acres	0	83
Pit Lake Surface Area at Static Level	Acres	22	22
Annual Precipitation Rate	Inches	12	12
Annual Evaporation Rate	Inches	50	50
Runoff Coefficient, Ex-Pit Watershed		0.071	0.071
Runoff Coefficient, In-Pit Watershed Reclaimed		0.303	0.303
Runoff Coefficient, In-Pit Watershed Unreclaimed		0.126	0.126
Fresh Water Fill	Acre-Feet	0	2,201
Pit Lake Annual Evaporation @ Static Level	Acre-Feet	91	92
Annual Groundwater Inflow	Acre-Feet	36	36
Annual Stormwater Inflow, Total Watershed	Acre-Feet	54	57
Annual Stormwater Inflow, Ex-Pit	Acre-Feet	14	14
Annual Stormwater Inflow, In-Pit	Acre-Feet	41	43
Pit Lake Volume at Static Level	Acre-Feet	2,278	2,286
Pit Lake Depth at Static Level	Feet	247	248
Pit Lake Surface Elevation at Static Level	Feet AMSL	4,897	4,898

## **Appendix H – PHREEQC Input Files (electronic)**

## **Appendix J – Aquatic Consultants Inc. Biological Assessment of the Existing Copper Flat Pit Lake**



# Copper Flat Mine

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Biological Assessment  
November 2014



**Aquatic Consultants Inc.**

*"Your Lake & Stream Experts"*

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# Copper Flat Mine – Biological Assessment

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## METHODS

All samples were collected and field measurements were completed on 06 November 2014. All testing was conducted by Aquatic Consulting & Testing, Inc. (Arizona Laboratory License No. AZ0004). Measurements and samples were collected from three locations (coordinates shown below) except where otherwise described.

Location 1: N 32°58'11.97"      W 107°32'03.52"  
Location 2: N 32°58'15.24"      W 107°32'03.08"  
Location 3: N 32°58'14.48"      W 107°32'00.80"

Light transmission was measured using an Apogee MQ300 quantum meter and remote sensor. Transparency was measured using a standard Secchi Disk.

Temperature and oxygen profiles were measured using a YSI Model 550A dissolved oxygen meter with remote sensor. Light extinction coefficient was calculated using the quantum meter data and the following formula:

Light extinction coefficient  $k = (\ln I_0 - \ln I_d) \times 1/z$

Where  $k$  = extinction coefficient

$I_0$  = light intensity at surface

$I_d$  = light intensity at depth  $\mu\text{mol}/\text{m}^2/\text{s}$  [or  $\mu\text{E}$ ]

$Z$  = depth (m)

Water samples were collected from a depth of 0.5 meter at the three sampling locations and composited into a single sample. Depth-integrated samples were not required because the water was not vertically stratified. Sample preservation and chemical analyses were performed using EPA or APHA (Standard Methods) procedures licensed by Arizona Department of Health Services. Specific test methods are referenced in the attached laboratory reports. Algae identification and counts were made using a Nikon Diaphot phase/contrast inverted microscope. Samples were concentrated using an Utermohl settling chamber. Identifications were made using the following taxonomic references:

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- Warner, D. 1977. The Biology of the Diatoms. University of California Press, Berkeley, CA. 498 pp.

# Copper Flat Mine – Biological Assessment

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Zooplankton was collected using an 80-um Wisconsin plankton net. A vertical tow from the bottom to top of the water column was made at each location and combined to produce a composite. At the laboratory, the concentrated sample volume was measured and recorded, an aliquot was transferred to a counting chamber, and the sub-sample was observed using a dissecting microscope. Zooplankton forms were identified and counted.

Benthic (sediment) samples were collected with a stainless steel Ponar dredge. The sediment was hand sorted and screened in the laboratory to retrieve and isolate macroinvertebrates. Particle size analysis was conducted using an ATM Arrow shaker equipped with stacked U.S. standard sieves.

Fish sampling was conducted using an 18' Smith-Root Electro-fishing Boat. Running Direct Current (DC) at 15 pulses per second. Percent of range selected was 40% with output at approximately 200 volts. Pulse width at 40% produced a pulse duration of 2.4 milliseconds. Electro-fishing amperage was between 8 and 10. Electro-fishing effort was continuous at 1800 seconds during daylight and 1800 seconds after dark. Additionally, three experimental mesh gill nets were deployed for 21 hours over night. Two sets were shoreline sets and one in the middle of the pit. Each net was 120ft long and made up of six monofilament 20ft sections with the following mesh sizes ½", 1", 1 ½", 2", 2 ½", and 3".

## RESULTS AND DISCUSSION

### Physical Conditions

#### Stratification

Temperature and oxygen profiles are presented below. Water temperature measurements varied from 12.6 to 13.7 C and dissolved oxygen concentrations ranged from 8.9 to 9.4 mg/L. At the time of sampling, the pit water was vertically mixed. The greatest change in temperature with depth occurred at Location 2, with a change of only 0.7 C from top to bottom (7.5 m) of the water column. Accordingly, dissolved oxygen was essentially unchanged through the water column at each location, with only a 0.2 mg/L maximum change from surface to pit bottom. Raw data and profiles are presented in Figure 1.

# Copper Flat Mine – Biological Assessment

The temperature and oxygen data, aside from other limiting factors, indicate the water should be supportive of a warm water or possibly a cold water fishery (summertime profiles were not available).

## Transparency and Solar Radiation

The Secchi disk depth was approximately 4.0 m. PAR measurements indicated penetration through the entire water column. Approximately 110  $\mu\text{mol}/\text{m}^2/\text{s}$  PAR was available at the pit bottom (extinction coefficient 0.35). Sufficient light existed to support a phytoplankton population. Light intensity at the pit bottom was similar to the recommended light intensity for algae cultures (Lavens and Sorgeloos 1996) and possibly benthic algae, although the minimum light requirement for benthic algae is poorly understood (Stevenson et al 1996). Light extinction data and graph are presented in Figure 2.

## Sedimentation and Substrate Type

The amount of compacted sediment on the pit bottom ranged from 4 to 6 inches, with up to a 20-inch covering of iron floc. The sediment contained a very low (0.21%) organic carbon concentration, but did contain organic nitrogen (2160 mg/kg) and phosphorus (880 mg/kg). These data indicate that benthic algae or even submerged rooted macrophytes could only exist in areas where the iron floc was limited (littoral zone).

Seive analysis (see Figure 3) indicated that all particles were less than 1.18 mm and 89 percent was finer than 0.6 mm. The sediment is classified as silt (all particles less than 2 mm). The silt provides little to no substrate for diversity in a macroinvertebrate population.

The sieve analysis is presented on the following page. Sediment chemistry data are presented as part of laboratory report package presented at the end of the narrative.

## Nutrients

Low nutrient concentrations, typical of oligotrophic lakes, were measured. An available N:P ratio of 3:1 was found. Because of the low pH (4.6 SU) and reported (Hall Environmental Analysis Laboratory) acidity of the water (180 mg/L as  $\text{CaCO}_3$ ), bicarbonate and carbonate ions would be essentially absent (Geller et al.). Sufficient inorganic carbon would be available to algae through the equilibrium reactions of absorbed atmospheric carbon dioxide and carbonic acid (University of Montana).

# Copper Flat Mine – Biological Assessment

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Biologically-available phosphorus (0.018 mg/L phosphate-P), nitrogen (0.24 mg/L NO<sub>3</sub>-N and 0.03 NH<sub>3</sub>-N) would be adequate to support a modest phytoplankton population. This projection was supported by the very low chlorophyll-a concentration measurement of 0.8 ug/L. At measured pH, no ammonia toxicity could exist. The low pH would be detrimental to cyanobacteria (blue-green algae growth, but not eukaryotic algae species (Brock 1973). The complete water quality report is provided at the end of the report narrative.

## Biological Conditions

The pit waters contained a depauperate algal assemblage composed of only six genera of algae. The six consisted of the diatoms (Bacillariophyta) *Diatoma*, *Cymbella*, *Synedra*, and *Navicula*; the cryptomonad, *Cryptomonas*; and the blue-green (Cyanophyta) alga *Chroococcus*. *Cryptomonas* was the dominant organism and is common in cold, acidic waters (Holopaenin 1992; Ojala and Jones 1993). Diatoms have also been found in a number of acidic environments, especially where high concentrations of iron exist as in some pit water environments (Nicola 2000). *Chroococcus* has been reported to dominate acidified Canadian lakes (Seckbach 2007). The total cell count was 603 cells per mL. However, many of the diatoms were frustules only (no protoplasm or chlorophyll observed), suggesting that these were dead and settling cells. The viable cell count is estimated at 312 cells/mL. The algae composition is summarized below (Table 1).

**Table 1. Algae composition of Copper Flat pit water 11/06/14**

Genus	Division/ form	Count per mL	Percent Comp.
<i>Diatoma</i>	Bacillariophyta (diatom) unicell	22	3.7
<i>Cryptomonas</i>	Cryptophyta (cryptophytes) flagellate	223	37.0
<i>Cymbella</i>	Bacillariophyta (diatom) unicell	34	5.6
<i>Synedra</i>	Bacillariophyta (diatom) unicell	212	35.2
<i>Navicula</i>	Bacillariophyta (diatom) unicell	22	3.7
<i>Chroococcus</i>	Cyanophyta (blue-green) colony	89	14.8

Sediment samples, primarily in the littoral zone, contained diatom frustules (most void of protoplasm or chlorophyll) and a very small number of *Hormidium* (Chlorophyta) filaments. *Hormidium* grows in acid environments as low as pH 3.5 and is least susceptible to copper and zinc toxicity at the pH range of 3.5 to 4.0 SU (Hargraves and Whitton 1976).

No zooplankton were recovered from multiple vertical tows at each location.

# Copper Flat Mine – Biological Assessment

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No macroinvertebrates were recovered from the sediment.

No fish species were recovered from either electro-fishing or gill nets.

A very small stand (20 sq ft) of cattail (*Typha* sp.) was found along the lake edge, in the dampened soil. No floating or submerged macrophytes were present.

## Integrated Conditions and Biological Integrity

Because of the limited variety of organisms recovered from the sampling activities, only a few basic indices were calculated to characterize the pit water. The indices typically characterize the pit water as oligotrophic, with insignificant amounts of organic pollution, but with one or more other water quality variables reducing productivity.

*Carlson Trophic Index* (Carlson 1976) uses chlorophyll-a, transparency and phosphorus concentration to quantitatively categorize the status of a lake ranging from oligotrophic (unproductive) to highly eutrophic (productive). The range of TSI was 28-69. Transparency and chlorophyll were indicative of an oligotrophic lake, but total phosphorus was characteristic of a eutrophic lake.

*Nygaard Trophic Index* (Nygaard 1976) proposed five indices to evaluate the organic pollution of water bodies based on the tolerance of various groups of planktonic algae occurring in them. These indices include Cyanophycean or Myxophycean index, Chlorophycean index, Bacillariophycean or Myxophycean index, Chlorophycean index, Bacillariophycean index, Euglenophycean index and a combination of these called compound coefficient index. Because of the paucity of phytoplankton, only the diatom index was appropriate. The Index value was 0, indicating oligotrophic conditions.

*Palmer Organic pollution Index* (Person 1989). The metric evaluates the degree of organic pollution based on pollution tolerance of key algal genera. The pit water score was 5 indicating minimal or no organic pollution, or that another variable is interfering with algae growth.

# Copper Flat Mine – Biological Assessment

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## CONCLUSION

The collected and historic data demonstrate that the pit waters do not and cannot support a balanced ecosystem. Higher aquatic life forms are absent because of likely chemical toxicity, lack of suitable habitat, and lack of food resources.

The pH of the water is below the range (6.5 to 9.0 SU) typically considered supportive of aquatic ecosystems (EPA 1986). pH has been considered the most important determinant of water quality in a pit environment (Miller 2002), impacting divalent metals solubility and creating toxicity. Groundwater interaction with the walls and surrounding host rock of the pit create oxidation reactions that release sulfate, acid, and metals into the lake. Copper Flat Pit water pH (4.6 SU) is well below the typical tolerance range of most aquatic organisms and the copper concentration (18 mg/L) is well above minimum phytotoxic concentrations.

Although adequate light and some nutrients are available, there is a paucity of primary producers in the pit water. Without available food, zooplankton species are essentially absent. A high concentration of copper in the water and low pH appear likely factors limiting algal growth and survival.

Macroinvertebrates are absent, including those typically considered tolerant of pollution. Habitat availability and diversity is limited. Most of the pit bottom and edge is composed of fine particulates; rocks and rubble are essentially absent. Organic matter is limited. The layer of precipitated iron covering a layer extremely fine silt is not suitable habitat for most benthic organisms. Food reserves for shredders and scrapers is highly limited, as the depauperate and sparse periphyton consisted of a single species of filamentous algae.

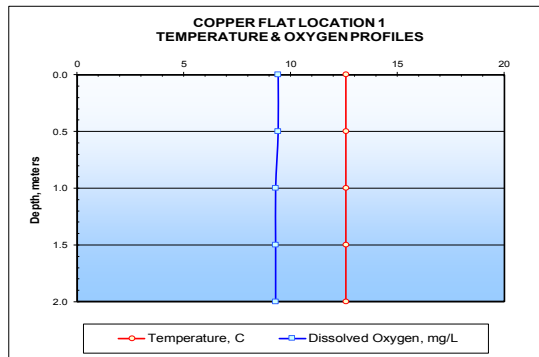


# FIGURE 1

**Copper Flat - 11/06/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 1**

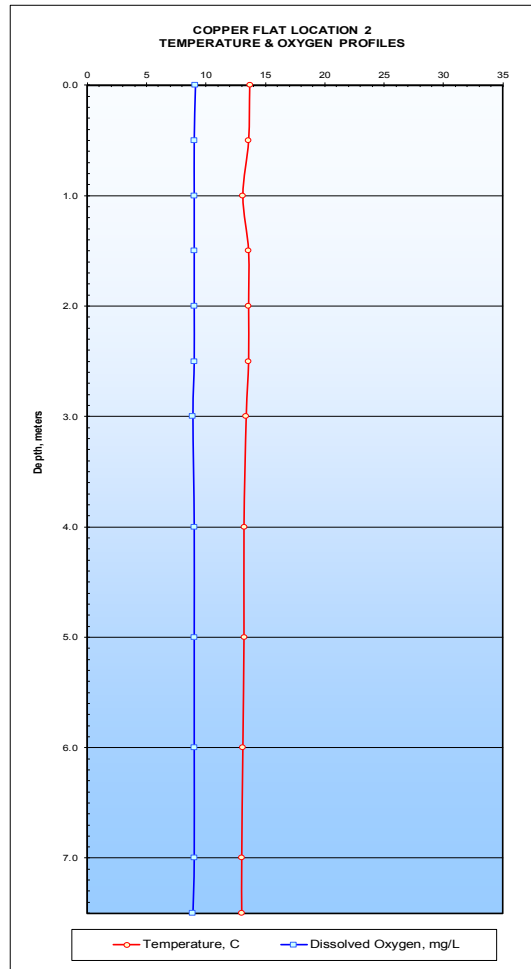
Depth_m	Temp_C	O2_mg/L
0.0	12.6	9.4
0.5	12.6	9.4
1.0	12.6	9.3
1.5	12.6	9.3
2.0	12.6	9.3



**Copper Flat - 11/06/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 2**

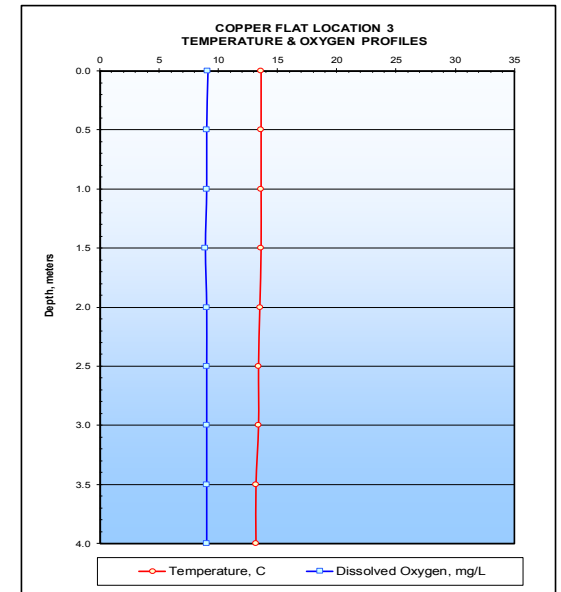
Depth_m	Temp_C	O2_mg/L
0.0	13.7	9.1
0.5	13.6	9.0
1.0	13.1	9.0
1.5	13.6	9.0
2.0	13.6	9.0
2.5	13.6	9.0
3.0	13.4	8.9
4.0	13.2	9.0
5.0	13.2	9.0
6.0	13.1	9.0
7.0	13.0	9.0
7.5	13.0	8.9



**Copper Flat - 11/06/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 3**

Depth_m	Temp_C	O2_mg/L
0.0	13.6	9.1
0.5	13.6	9.0
1.0	13.6	9.0
1.5	13.6	8.9
2.0	13.5	9.0
2.5	13.4	9.0
3.0	13.4	9.0
3.5	13.2	9.0
4.0	13.2	9.0

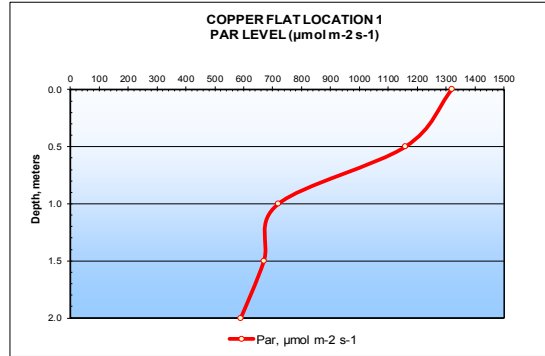


## FIGURE 2

**Copper Flat - 11/6/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 1**

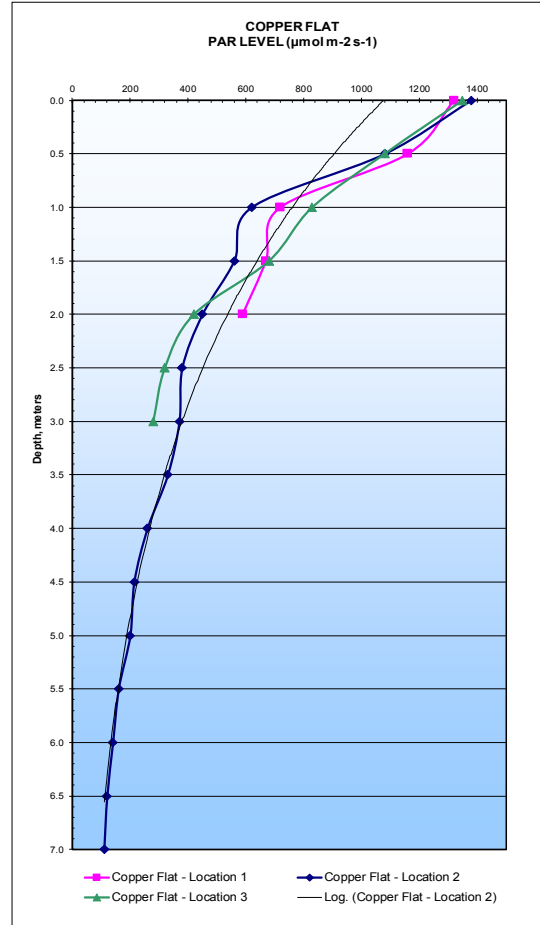
Depth_m	Par_μmol m-2 s-1
0.0	1320.0
0.5	1160.0
1.0	720.0
1.5	670.0
2.0	590.0



**Copper Flat - 11/6/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 2**

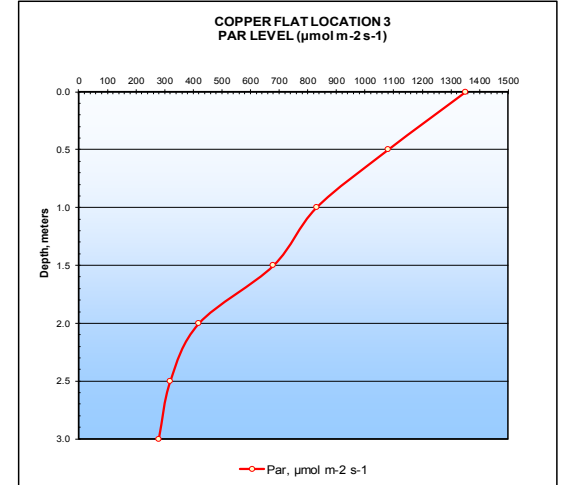
Depth_m	Par_μmol m-2 s-1
0.0	1380.0
0.5	1080.0
1.0	620.0
1.5	560.0
2.0	450.0
2.5	380.0
3.0	370.0
3.5	330.0
4.0	260.0
4.5	214.0
5.0	200.0
5.5	160.0
6.0	140.0
6.5	120.0
7.0	110.0



**Copper Flat - 11/6/14**  
Aquatic Consulting & Testing, Inc.

**Copper Flat - Location 3**

Depth_m	Par_μmol m-2 s-1
0.0	1350.0
0.5	1080.0
1.0	830.0
1.5	680.0
2.0	420.0
2.5	320.0
3.0	280.0

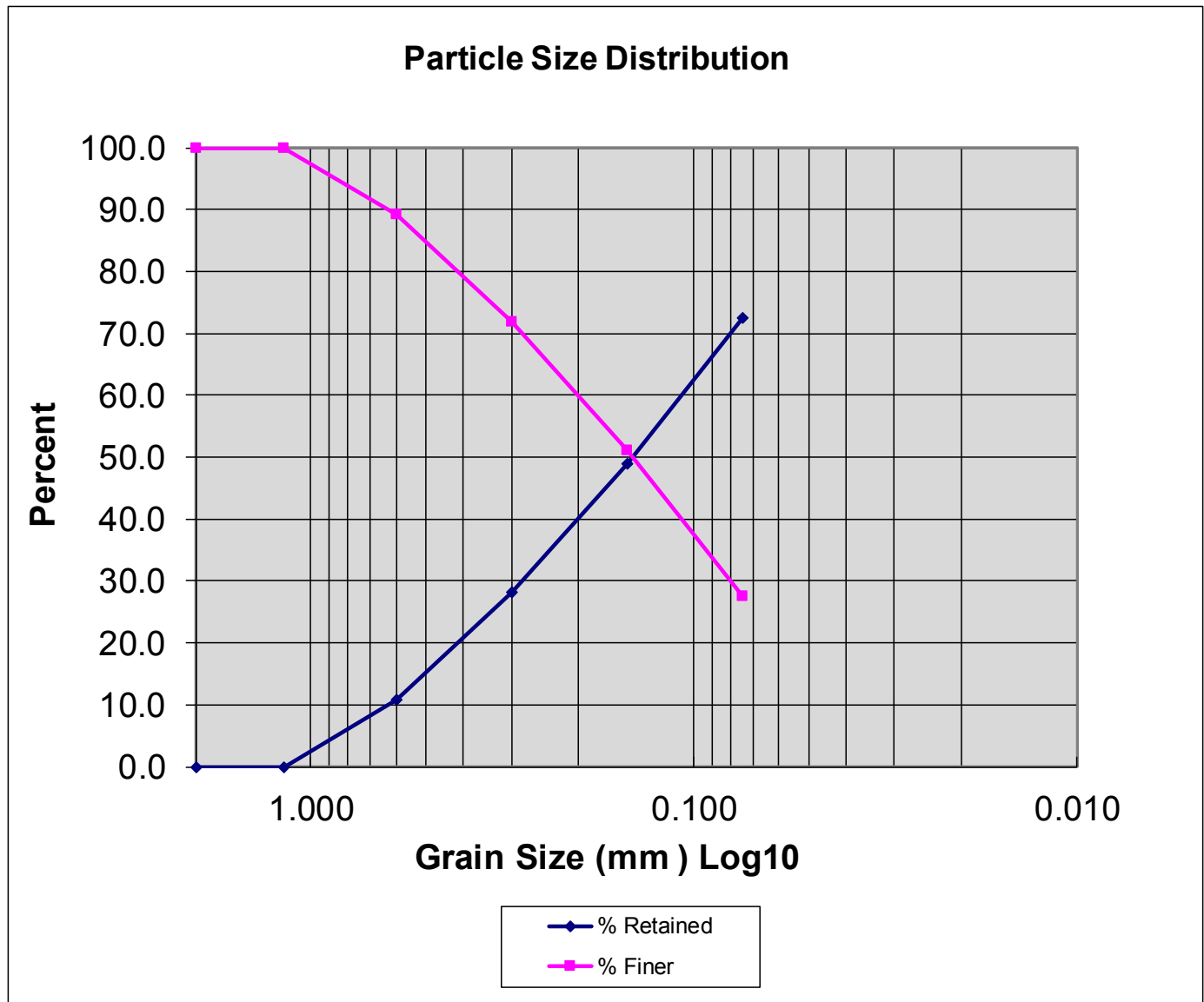


# FIGURE 3

Laboratory ID: **BW10543**

Client ID: **Aquatic Consultants - Copper Flat New Mexico**

Mesh Size	Nominal Opening (inches)	Grain Size (mm)	Grams retained	Each sieve % Retained	Cumulative % Retained	% Finer
8	0.0937	2.360	0	0.0	0.0	100.0
10	0.0787	2.000	0	0.0	0.0	100.0
16	0.0469	1.180	0	0.0	0.0	100.0
30	0.0234	0.600	34.52	10.8	10.8	89.2
50	0.0117	0.300	55.01	17.3	28.1	71.9
100	0.0059	0.150	66.72	20.9	49.0	51.0
200	0.0029	0.075	74.76	23.5	72.5	27.5
	<0.0029		87.64	27.5	100.0	0.0





# AQUATIC CONSULTING & TESTING, INC.

1525 W. University Drive, Suite 105  
P.O. Box 1610  
Tucson, Arizona 85721  
Phone: (480) 921-8044 • Fax: (480) 921-0049

Lic. No. AZ0003

## LABORATORY REPORT

Client: Aquatic Consultants  
4421 Irving Blvd. NW  
Albuquerque, NM 87114

Date Submitted: 11/07/14  
Date Reported: 12/02/14

Attr: Paul Cassidy

Project: Copper Flat

### RESULTS

Client ID: H2O Comp. Loc 1-3  
ACT Lab No.: BW10542

Sample Type: Surface Water  
Sample Time: 11/06/14 14:30

Parameter	Analysis Date		Method No.	Result	Unit
	Start	End			
Total Organic Carbon	11/24/14	11/24/14	SM 5310 C	0.7	mg/L
Algae Count	12/01/14	12/01/14	SM 10200 F	See Attached	cells/mL
Algae Identification	12/01/14	12/01/14		See Attached	
Chl/Phae Ratio	11/25/14	11/25/14	SM10200 H	1.14	
Chlorophyll a	11/25/14	11/25/14	SM10200 H	0.80	ug/L
Phaeophytin a	11/25/14	11/25/14	SM10200 H	3.12	ug/L
Zooplankton	11/20/14	11/20/14	SM10200 G	<10.	#/cu. meter
Oxygen, Dissolved Field	11/06/14	11/06/14	SM4500 O G	9.0	mg/L as O <sub>2</sub>
pH, Field	11/06/14	11/06/14	SM4500H+ B	4.8	BU
Secchi Disk Depth	11/06/14	11/06/14	NALMS	4.0	meters
Temperature, Field	11/06/14	11/06/14	SM2580 B	13.6	C
Ammonia - N	11/20/14	11/20/14	SM4500NH3 D	0.03	mg/L as N
Nitrate + Nitrite - N	11/25/14	11/25/14	SM4500NO3 E	0.24	mg/L as N
Phosphate, ortho	11/07/14	11/07/14	385.3	0.018 *	mg/L as P
Phosphorus, Total	11/25/14	11/25/14	385.3	0.087	mg/L as P
Total Inorganic Carbon	11/24/14	11/24/14	SM 5310 C	0.9	mg/L
Total Kjeldahl Nitrogen	11/20/14	11/20/14	SMNorg C,NH3 C/D	0.3	mg/L as N

\* R13-RPDRSD exceeded the method acceptance limit. Result <3 times the PQL.

## RESULTS

Client ID: Sed Comp. Loc 1-3  
ACT Lab No.: BW10543

Sample Type: Sediment  
Sample Time: 11/08/14 14:30

<u>Parameter</u>	<u>Analysis Date</u>		<u>Method No.</u>	<u>Result</u>	<u>Unit</u>
	<u>Start</u>	<u>End</u>			
Soil TOC	11/19/14	11/19/14	WalkleyBlack	0.21	% Org-C
Sieve Test	11/25/14	11/25/14	ASTM	See Attached	
Kjeldahl Nitrogen - Soil	11/19/14	11/19/14	SM4500NargC mod.	2160.	mg/kg as N
Nitrate + Nitrite - N	11/18/14	11/18/14	SM4500NO3E mod.	<1.	mg/kg as N
Phosphorus, Total	11/23/14	11/24/14	385.3 mod.	880. *	mg/kg as P
Total Solids	11/10/14	11/14/14	SM2540 G	10.8	%

\* R9-Sample RPD exceeded the method acceptance limit.

Client ID: Sed Floc  
ACT Lab No.: BW10544

Sample Type: Sediment  
Sample Time: 11/08/14 14:30

<u>Parameter</u>	<u>Analysis Date</u>		<u>Method No.</u>	<u>Result</u>	<u>Unit</u>
	<u>Start</u>	<u>End</u>			
Microscopic Identification	12/01/14	12/01/14		See Attached	

Reviewed by

  
Frederick A. Ansell, Ph.D.  
Laboratory Director





DECEMBER 2017



New Mexico Copper Corporation



PROBABLE HYDROLOGIC  
CONSEQUENCES OF THE  
COPPER FLAT PROJECT  
SIERRA COUNTY  
NEW MEXICO



JOHN SHOMAKER & ASSOCIATES, INC.  
WATER-RESOURCE AND ENVIRONMENTAL CONSULTANTS  
PREPARED BY JSAI

17282



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**PROBABLE HYDROLOGIC CONSEQUENCES  
OF THE  
COPPER FLAT PROJECT,  
SIERRA COUNTY, NEW MEXICO**

prepared by

Michael A. Jones

Steven T. Finch, CPG, PG

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prepared for

**NEW MEXICO COPPER CORPORATION**  
a wholly owned subsidiary of THEMAC Resources Group, Ltd.  
4253 Montgomery Blvd NE, Suite 130  
Albuquerque, NM 87109

December 12, 2017



# **PROBABLE HYDROLOGIC CONSEQUENCES OF THE COPPER FLAT PROJECT, SIERRA COUNTY, NEW MEXICO**

## **EXECUTIVE SUMMARY**

The proposed Copper Flat Project includes a mine pit, supply wells, tailings facility, and waste rock facilities (Fig. 1.1) located in the Hillsboro Mining District, Sierra County, New Mexico.

Presented in this report is the evaluation of the hydrologic consequences of the proposed operating plan detailed in the New Mexico Copper Corporation (NMCC) Updated Mining Operation and Reclamation Plan for Copper Flat Mine, Rev. 1 (THEMAC, 2017a) and in the New Mexico Copper Corporation Discharge Permit Application, Rev. 1 (THEMAC, 2017b). The operating plan reviewed herein reflects a nominal processing rate of 30,000 tons of ore per day for 11.5 years and aligns with “Alternative 2” in the Copper Flat Draft Environmental Impact Statement (BLM, 2015).

The objective of this report is to develop a determination of the probable hydrologic consequences of the operation and reclamation on both the permit and affected areas with respect to the hydrologic regime, quantity and quality of surface and groundwater systems that may be affected by the proposed operations (NMAC 19.10.6.602.(13)(g)(v)) of the Mining Act regulations.

Groundwater systems include:

- The regional Santa Fe Group (SFG) aquifer.
- Quaternary-age alluvial aquifers along Animas Creek and Percha Creek.
- The crystalline bedrock of the Animas Uplift.

Surface water includes:

- Perennial flow in the Rio Grande and Caballo Reservoir that is supplied in part by discharge from the SFG aquifer.
- An area of perennial flow and riparian vegetation along Animas Creek where the Quaternary-age alluvial aquifer discharges to the surface.
- An area of perennial flow and riparian vegetation along Percha Creek, atop the crystalline bedrock.
- Springs discharging from the crystalline bedrock.
- Storm water flows in Grayback Arroyo.

“Consequences” considered here are the resulting effects on the hydrologic regime of NMCC’s proposed operation and reclamation including both water use, and surface and groundwater impact mitigation measures.

The sources of possible hydrologic consequences of the Project include:

1. Groundwater withdrawals from the SFG aquifer: The mine water supply will be withdrawn from pumping wells PW-1, PW-2, PW-3, and PW-4. Water level in the SFG aquifer will be lowered around the well field and then gradually recover after mining. Secondary effects evaluated include:
  - a. Reduced groundwater discharge to Rio Grande and Caballo Reservoir.
  - b. Reduced flow to artesian wells and other effects to local groundwater users.
  - c. Potential reduced discharge to shallow aquifers along Animas Creek and Percha Creek, leading to lower alluvial water levels and reduced discharge to the perennial flow and riparian areas along Animas Creek.
  - d. Potential ground subsidence.
2. Groundwater withdrawals from the crystalline bedrock associated with the open pit. Water levels in the bedrock around the pit will be permanently lowered, and groundwater will flow to the pit and evaporate. Groundwater flow rates to the pit and the future open pit water level and water balance area assessed. Secondary effects evaluated include:
  - a. Potential groundwater discharge from the open pit.
  - b. Potential effects on springs discharging from the crystalline bedrock and on the Percha Creek perennial (riparian) area.
3. Potential for groundwater discharge from the tailings storage facility (TSF) and waste rock stockpiles (WRSPs).

The consequences were evaluated using the numerical groundwater flow model (JSAI, 2014) developed for the Copper Flat Project. Effects include the following:

#### **Santa Fe Group (SFG) Aquifer**

- Water-level drawdown in the SFG aquifer is projected to reach a maximum of about 70 ft at the well field, at the end of mining. Drawdown will decrease with distance from the well field. Water levels will then recover over a period of about 20 to 30 years.
- Total reductions in discharge to the system from the SFG aquifer are projected to peak at a total of about 3,100 acre-feet per year (ac-ft/yr) shortly after the end of mining, then diminish to near-zero over about 30 years (Fig 3.3; Table 3.1).
- Flow induced from the Palomas Graben north of the study area is projected to reach a maximum of less than 800 ac-ft/yr at the end of mining, which is estimated to result in an additional reduction of discharge to the Rio Grande by a maximum of 275 ac-ft/yr.
- Potential impairment of existing water rights from reduced discharge to flowing wells may occur.
- Effects on shallow groundwater (riparian) systems along Las Animas Creek and Percha Creek are projected to be minimal, with a maximum of less than 2 ft of groundwater-level change on Percha Creek, less than 1 ft of groundwater-level change on Animas, and non-measurable small changes in surface flow and riparian evapotranspiration.
- Depletion to the Rio Grande is projected to peak around 2,080 ac-ft/yr at the end of mining, then reduce to 28 ac-ft/yr 100 years after mining (Fig. 3.3; Table 3.1)

As required by New Mexico Office of the State Engineer (NMOSE), NMCC will mitigate the effects of pumping of the SFG aquifer by offsetting reductions in discharge to the Rio Grande by lease or purchase of additional water rights in the amount of the model-simulated reductions to flow.

NMCC will work with the NMOSE to ensure that impairment to existing water rights (including permitted wells) according to NMOSE criteria, by NMCC pumping, will be appropriately mitigated.

- Pumping of the production water-supply wells is not expected to result in measurable ground subsidence. No water-quality effects are expected from pumping the proposed supply wells in the affected area.

### **Crystalline Bedrock**

- At the end of mining, groundwater-level drawdown in the bedrock around the open pit is projected to reach a maximum of about 800 ft at the pit.
- A permanent cone of depression will form around the pit, with maximum drawdown of about 600 ft at the edge of the pit.
- The pit, which currently is an evaporative hydrologic sink, will form an evaporative hydrologic sink again in the future.

After mining, the pit will be filled with fresh water from the production water-supply wells to inundate portion of the pit walls and create a steady-state hydraulic sink with the surrounding groundwater system (rapid fill). The rapid fill will begin immediately after mining and will be completed in approximately 6 months. The rapid fill requires pumping 2,200 ac-ft into the pit and will fill the pit to elevation 4,894 ft amsl. At hydrologic equilibrium, the final pit water level is projected to be about 4,897 ft amsl, about 580 ft below the pit crest at the haul road entrance. The post-mining pit water body that forms after mining from rapid fill remediation will be about 250 ft in depth and have a steady-state surface area of about 22 acres. Steady state groundwater inflow is estimated at 36 ac-ft/yr and captured storm-water runoff is estimated at 57 ac-ft/yr. Pit water evaporation is projected to be about 93 ac-ft/yr. Evaporation will maintain the hydraulic sink in perpetuity.

Long-term, indirect effects to springs discharging in and around the Animas Uplift are projected to be minimal and not measureable. Water quality effects for the open pit water body are addressed in a separate report prepared for the project.

### **Storm-Water Flows**

Storm-water flow through Grayback Arroyo will not be affected. During operations and after reclamation, storm-water flows from Grayback Arroyo will be conveyed around the open pit in the existing bypass channel and through the mine area with no expected hydrologic consequences.

### **TSF and WRSPs**

Infiltration to groundwater from the tailings and waste rock storage areas is not expected due to installation of liner under the TSF and placement of WRSPs on low permeable crystalline bedrock. Any meteoric water that might infiltrate to groundwater is expected to remain in the immediate area for centuries, due to the low permeability of the SFG sediments near the Animas Uplift and due to the presence of flow-inhibiting faults. The impact to groundwater chemistry is expected to be minimal.

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Appendix A. Projected Groundwater-Level Hydrographs at Selected Locations

Appendix B. Technical Memo Regarding Liner Leakage Rates



# PROBABLE HYDROLOGIC CONSEQUENCES OF THE COPPER FLAT PROJECT, SIERRA COUNTY, NEW MEXICO

## 1.0 INTRODUCTION

This report presents an evaluation of the probable hydrologic consequences of the proposed Copper Flat Project (Project) in Sierra County, New Mexico. Hydrologic consequences refer to any changes, resulting from the Project, to groundwater and surface water systems, including changes to flow, water level, or chemical composition.

The Project is located in the Hillsboro Mining District, shown on Figure 1.1. Effects on both the mine permit area (Fig. 1.1) and the surrounding affected area are evaluated with respect to the hydrologic regime, quantity, and quality of surface and groundwater systems that may be affected by the proposed operations (NMAC 19.10.6.602.(13)(g)(v)).

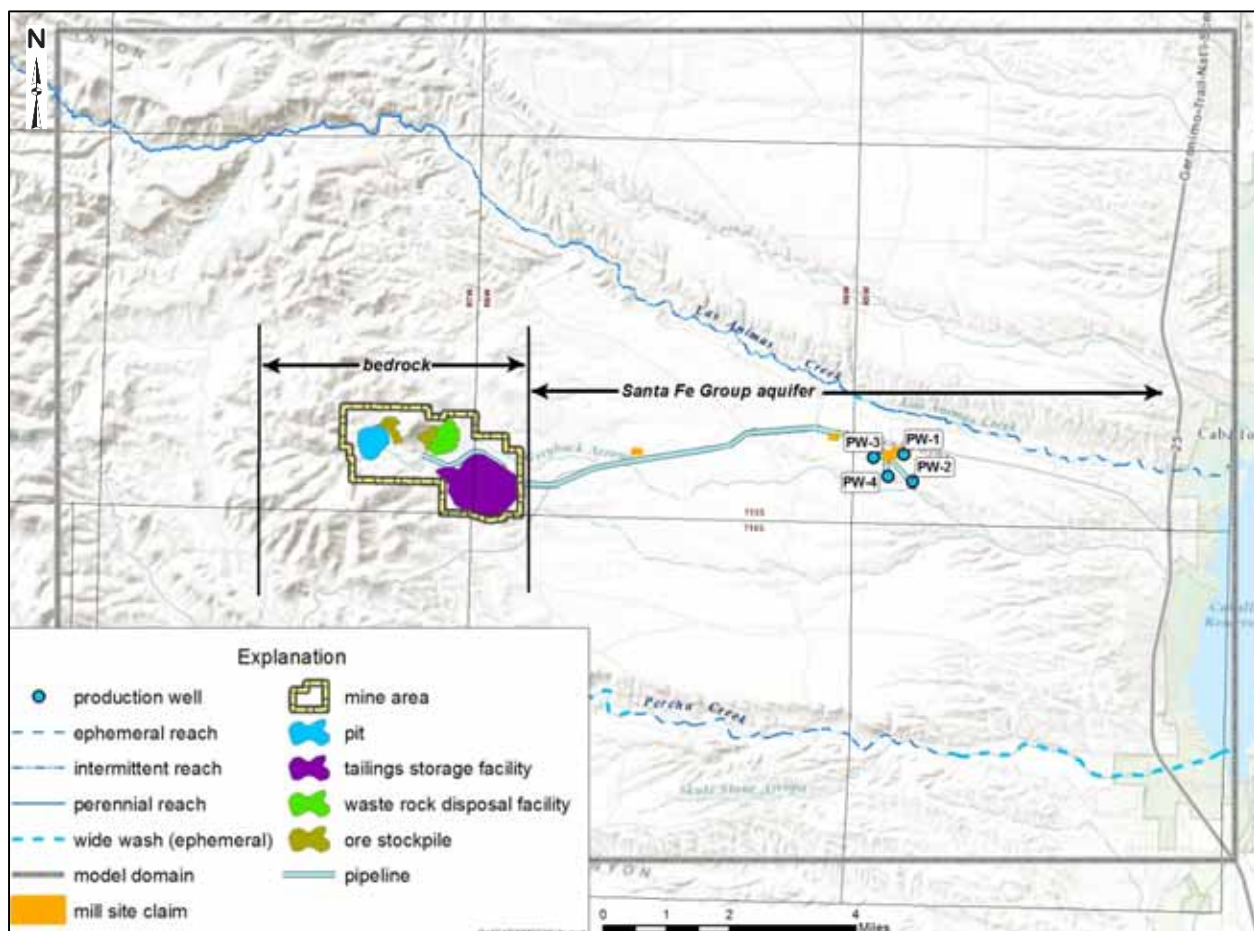


Figure 1.1. Map showing New Mexico Copper Corporation proposed mine facilities, mine area, and the affected area evaluated, Sierra County, New Mexico.

For the analysis of probable hydrologic consequences, the affected area includes the mine permit area containing the open pit and surrounding facilities, located on the andesite and quartz monzonite crystalline bedrock of the Animas Uplift (Fig. 1.1), as well as the affected area including the Santa Fe Group (SFG) aquifer around water supply wells PW-1 through PW-4 and surface and groundwater under Las Animas and Percha Creeks. The area evaluated for potential effects was the “model domain” shown on Figure 1.1.

## 1.1 Project Description

NMCC proposes to expand the existing open pit, previously developed by Quintana Minerals Corporation (Quintana) during a brief period of operation in 1982.

The existing pit was excavated to about 100 ft below original ground surface, with bottom elevation at about 5,400 feet above mean sea level (ft amsl). A permanent pool of water is present in the existing pit. The current water body has a surface area of about 5 acres, ranges from 10 to 35 ft deep, and contains 60 to 80 ac-ft of water. A diversion channel routes Grayback Arroyo around the pit.

Other facilities from 1982, including processing plant, waste rock storage and tailings storage, have been partially reclaimed. Water-supply wells PW-1, PW-2, PW-3, and PW-4 (Fig. 1.1) have been unused since 1982, except for pumping tests conducted by NMCC in 2012 to 2013.

Features of the Project include (Fig. 1.1) an expanded pit, processing plant, a lined tailings storage facility (TSF), and waste rock stockpiles (WRSPs). The water-supply wells will be re-activated. The Grayback Arroyo diversion would be maintained. Other diversions will route surface runoff around the processing plant and waste rock and tailings storage facilities.

The proposed operating scenario is detailed in NMCC’s Updated Mining Operation and Reclamation Plan for Copper Flat Mine (MORP; THEMAC, 2017a, Rev. 1) and in NMCC’s Discharge Permit Application (DP: THEMAC, 2017b, Rev 1). The planned scenario reflects a processing rate of 30,000 tons of ore per day for 11 to 12 years, and aligns with “Alternative 2” in the Copper Flat Draft Environmental Impact Statement (BLM, 2015). Upon receiving the required permit approvals, the Project will begin site preparation and construction, which will last approximately 2 years.

The operating life (period of mining) of the project is anticipated to be 11 to 12 years as noted in the MORP. NMCC will mine approximately 113 million tons of ore and 45 million tons of waste rock during the operating life of the mine (158 million tons). Depending on operational conditions, the mining operation will supply 8.9 to 10.8 million tons per year of copper ore to the mill for processing.

The pit will be expanded to occupy a footprint of 129 acres, reaching an ultimate bottom elevation of 4,650 ft amsl, about 825 ft below original ground surface. At the end of mining, the pit would be rapid-filled with good quality water from the production wells to the projected long-term stable water level and prevent oxidation of sulfates below the pit lake water line, thus optimizing pit water quality.

The WRSPs will be placed completely on crystalline bedrock, which provides a natural low-permeability liner. During operations, surface-water runoff collection trenches will be constructed, as needed, to collect and route runoff from the WRSPs to storm-water impoundments at the toe. These trenches will be constructed into the andesite bedrock to prevent water from entering the alluvial surface material down-gradient of the WRSPs. After mining ceases, the WRSPs will be reclaimed and covered with a 3-ft-thick engineered layered system of fill materials designed to store precipitation until it evaporates and prevent infiltration into the underlying WRSPs.

The TSF will be placed on an engineered liner system to prevent subsurface infiltration. The lined TSF will include an over-liner drainage system to maximize reclaim of water and minimize pressure on the liner. Underdrains beneath the dam will collect seepage and preserve dam stability. Water will be reclaimed from the surface of the tailings in a supernatant pond. After mining, the facility will be drained down reclaimed and covered with a 3-ft-thick layered system of fill materials to prevent infiltration into the tailings.

Ore will be trucked from the pit to the processing plant for crushing, grinding, and flotation recovery of copper. The mill will process ore at an average rate of 27,890 tons per day over the life of the operation. Milling will also include a molybdenum processing circuit and a gravity gold recovery circuit.

After mining, the site will be closed and reclaimed per an approved Reclamation and Closure Plan. NMCC has prepared a Reclamation and Closure Plan described in the Mine Operation and Reclamation Plan submitted to the Mining and Minerals Division as part of NMCC's Permit Application Package (THEMAC, 2017a; Golder, 2017).

The objective of the Reclamation and Closure Plan is to reclaim and close the facility in a manner protective of groundwater in conformance with the NM Copper Rules, meet the reclamation requirements of the New Mexico Mining Act, and return the mine area to conditions similar to those present before NMCC's re-establishment of the mine. The Reclamation and Closure Plan is designed to re-establish grazing in the area and allow for long-term use of the reclaimed areas by wildlife known to historically use the area without affecting the potential for other uses such as mining and recreation.

## 1.2 Analysis Method

The model of groundwater flow in the Animas Uplift and the Palomas Basin (JSAI, 2014) was used to project the hydrologic consequences of development of the Copper Flat Project. The numerical model was peer reviewed and adopted by the New Mexico Office of the State Engineer (NMOSE) in its deliberations regarding NMCC water rights declarations, and used for the Copper Flat Draft Environmental Impact Statement (BLM, 2015).

The mine site water balance developed for the proposed Mining Operation and Reclamation Plan (THEMAC, 2017a) was simulated in the numerical model to estimate potential effects on groundwater and surface-water levels and flows for the pre-mining, mining, and post-mining periods.

This analysis meets the requirements of NMAC 19.10.6.602.(13)(g)(v) by evaluating the probable hydrologic consequences of the operation and reclamation on both the permit and affected areas, with respect to the hydrologic regime, quantity, and quality of surface and groundwater systems that may be affected by the proposed operations.

The analysis takes into account both water use by the proposed operation and proposed mitigation strategies to reduce or eliminate the effects of the proposed operation. The “hydrologic regime” is considered to be surface and groundwater systems potentially affected by NMCC’s proposed operation and reclamation of Copper Flat.

Surface and groundwater systems in the area include the following.

Groundwater is found in:

- The regional Santa Fe Group (SFG) aquifer.
- Quaternary-age alluvial aquifers along Animas Creek and Percha Creek.
- The crystalline bedrock of the Animas Uplift.

Surface water includes:

- Perennial flow in the Rio Grande and Caballo Reservoir that is supplied in part by discharge from the SFG aquifer.
- An area of perennial flow and riparian vegetation along Animas Creek where the Quaternary-age alluvial aquifer discharges to the surface.
- An area of perennial flow and riparian vegetation along Percha Creek, atop the crystalline bedrock.
- Springs discharging from the crystalline bedrock.
- Storm water flows in Grayback Arroyo.

“Consequences” considered here are the resulting effects on the hydrologic regime of NMCC’s proposed operation and reclamation including both water use, and surface and groundwater impact mitigation measures.

The sources of possible hydrologic consequences of the Project include:

1. Groundwater withdrawals from the SFG aquifer: The mine water supply will be withdrawn from pumping wells PW-1, P W-2, P W-3, and P W-4. Water level in the SFG aquifer will be lowered around the well field and then gradually recover after mining. Secondary effects evaluated include:
  - a. Reduced groundwater discharge to Rio Grande and Caballo Reservoir.
  - b. Reduced flow to artesian wells and other effects to local groundwater users.
  - c. Potential reduced discharge to shallow aquifers along Animas Creek and Percha Creek, leading to lower alluvial water levels and reduced discharge to the perennial flow and riparian areas along Animas Creek.
  - d. Potential ground subsidence.
2. Groundwater withdrawals from the crystalline bedrock associated with the open pit. Water levels in the bedrock around the pit will be permanently lowered, and groundwater will flow to the pit and evaporate. Groundwater flow rates to the pit and the future open pit water level and water balance are assessed. Secondary effects evaluated include:
  - a. Potential effects on springs discharging from the crystalline bedrock and on the Percha Creek perennial (riparian) area.
3. Potential for groundwater discharge from the WRSPs and TSF.

The consequences were evaluated using the numerical model (JSAI, 2014), which was developed using the United States Geological Survey (USGS) groundwater-flow modeling code MODFLOW (McDonald and Harbaugh, 1988).

Water supply pumping from the SFG aquifer was simulated at rates specified in the mine-site water balance using the MODFLOW module WEL. Pumping was simulated for the pre-mining period of construction, for the period of mining and for post-mining filling of the open pit. The period-of-pumping simulation is followed by simulation of the post-pumping recovery of water levels.

Pit-area dewatering is simulated initially as pumping from the open pit, represented using MODFLOW module LAK2 (JSAI, 2014, appendix D). After the initial dewatering of the existing pit, a set of drain boundary conditions (MODFLOW module DRN) simulate a lowering of groundwater levels as the open pit depth increases. The simulated drain elevations initially represent the extent and elevation of the current pit. The drain elevations are then lowered and new drains are added through the simulation time, to transform the boundary conditions to represent the ultimate pit. The post-mining pit filling and pit water balance is simulated using module LAK2.

Potential for groundwater discharge from the WRSPs and TSF are estimated independently of the numerical model.

## 1.3 Report Structure

The contents of the report are organized as follows:

Section 1.0 – Describes the Project and analysis methods and outlines the report

Section 2.0 – Projected water demand for mine water supply and rapid-filling in mine area, and estimated open-pit dewatering

Section 3.0 – Probable hydrologic consequences for mine area including the following:

3.1 Groundwater withdrawals from the SFG aquifer

3.1.1 Regional groundwater level drawdown

3.1.2 Effects on water balance

3.1.3 Flow from north Palomas Graben

3.1.4 Operational plans for no net effect on the Rio Grande

3.1.5 Other water rights

3.1.6 Effects of reduced flowing well pressure

3.1.7 Effects on Quaternary-age alluvial aquifers and Animas Creek perennial flow and riparian zones

3.1.8 Ground subsidence

3.2 Groundwater withdrawals from the crystalline bedrock

3.2.1 End-of-mining groundwater drawdown

3.2.2 Open pit water balance

3.2.3 Potential open pit discharge to groundwater

3.2.4 Effects on springs and on the Percha Creek perennial (riparian) area

3.3 Potential groundwater discharge from tailings and waste rock

3.3.1 Tailings infiltration

3.3.2 Waste rock infiltration

3.3.3 Groundwater flow paths and travel times

Section 4.0 – Report conclusions with a summary of results

Section 5.0 – References

Appendix A – Additional results regarding projected groundwater-level hydrographs at different locations

Appendix B – Technical Memorandum regarding the analysis of liner leakage rates

## 2.0 PROJECT WATER DEMAND

The projected water demand is based on the proposed mine plan for Copper Flat as detailed in the Mining Operation and Reclamation Plan, Rev. 1 (THEMAC, 2017a), which includes a water balance accounting for seasonal effects of climate, recycled process water, makeup water from supply wells, open pit dewatering, and diverted and captured storm-water runoff from the mine area.

The projected monthly water demand was obtained in electronic form (spreadsheet file “Nov 2016 Water Balance Prod Well GPM.xlsx,” NMCC personal communication, February 2017). Operational demand increases in summer and decreases in winter, averaging 6,105 ac-ft/yr over the 11.5-year life of the mining operation.

Water will be withdrawn from the SFG aquifer to provide the main water supply for the mine. Water will also be withdrawn from the crystalline bedrock, to dewater the pit. After mining, water will be withdrawn from the SFG aquifer to rapid-fill the open pit.

### 2.1 Water-Supply Pumping

The estimated rates of groundwater use are summarized on Table 2.1. Project water demand includes the mine construction and start up, 11.5-year mining period, and post-mining reclamation water demand requirements. Pumping for rapid fill reclamation of the open pit will require 2,200 ac-ft over 0.5 year.

**Table 2.1. Projected water-supply pumping**

component	unit	result
pumping duration (includes construction, operation, reclamation)	years	23.0
average pumping rate over full project duration	gpm	2,180
summer maximum pumping rate	gpm	4,224
winter minimum pumping rate	gpm	3,388
water removed from aquifer over pumping duration	ac-ft	73,856
average annual pumping rate over pumping duration	ac-ft/yr	3,211
maximum annual withdrawal rate	ac-ft/yr	6,095

gpm - gallons per minute

ac-ft/yr - acre-feet per year

The Project water use is presented in more detail in Table 2.2, showing year-by-year projections of water needs. The table presents the water balance for the mine operation that has been provided to the U.S. Bureau of Land Management in response to comments on the Draft Environmental Impact Statement, with the exception in listing a smaller volume of water (2,200 ac-ft instead of 2,800 ac-ft) used for post-mining filling of the pit.

**Table 2.2. Projected water-supply pumping (acre-feet per year)**

<b>year</b>	<b>production wells</b>	<b>operation</b>	<b>construction</b>	<b>startup</b>	<b>rapid fill</b>	<b>reclamation</b>
1	132	0	132	0	0	0
2	673	0	233	440	0	0
3	6,081	6,081	0	0	0	0
4	6,087	6,087	0	0	0	0
5	6,071	6,071	0	0	0	0
6	6,088	6,088	0	0	0	0
7	6,078	6,078	0	0	0	0
8	6,086	6,086	0	0	0	0
9	6,090	6,090	0	0	0	0
10	6,095	6,095	0	0	0	0
11	6,095	6,095	0	0	0	0
12	6,090	6,090	0	0	0	0
13	6,093	6,093	0	0	0	0
14	5,472	2,621	0	0	2,200	651
15	321	0	0	0	0	321
16	97	0	0	0	0	97
17	97	0	0	0	0	97
18	50	0	0	0	0	50
19	24	0	0	0	0	24
20	15	0	0	0	0	15
21	10	0	0	0	0	10
22	6	0	0	0	0	6
23	5	0	0	0	0	5
24	0	0	0	0	0	0
25	0	0	0	0	0	0
----	----	----	----	----	----	----
<b>Total</b>	<b>73,856</b>	<b>69,575</b>	<b>365</b>	<b>440</b>	<b>2,200</b>	<b>1,276</b>



This smaller post-mining filling of the pit volume is a refinement of the plan that does not measurably change the effects of the Project. The revised pit water balance is reflected in the analysis of pit water (SRK, 2017). Other, smaller adjustments to the estimated water balance may arise as the Project develops, with no measureable change to the effects of the Project.

## 2.2 Open-Pit Dewatering and Refilling

Pit dewatering is simulated assuming initial pit sump pumping of 100 gallons per minute (gpm), projected to empty the existing pit, with a water volume of about 60 ac-ft (INTERA et al., 2012), in about 4-1/2 months. During operations, groundwater and runoff flowing to the pit will be collected in sumps and pumped out. Projected pit dewatering during mining is summarized in Table 2.3.

**Table 2.3. Pit dewatering**

<b>pit dewatering duration</b>	years	11.4
<b>average pit dewatering rate</b>	gpm	28
<b>total water withdrawn by pumping over full project duration</b>	ac-ft	499
gpm - gallons per minute		ac-ft – acre-feet

The schedule of dewatering is shown on Figure 2.1 including projected pit bottom elevation, pit-area groundwater elevation, and dewatering rates. Long-term total flow is expected to range between about 35 and 65 gpm (56 and 105 ac-ft/yr) with an initial minimum of about 20 gpm (32 ac-ft/yr) and a maximum of about 70 gpm (113 ac-ft/yr), as the pit bottom approaches final elevation of 4,650 ft amsl.

After mining is complete, the pit will be rapid filled to the projected steady-state post-mining equilibrium water level.

Current and projected final pit geometry are summarized on Figure 2.2 showing the water surface area as a function of water level. The existing pit currently has a water surface area of about 5.2 acres. The proposed pit would have water surface area of about 22 acres, with a final water level near 4,897 ft amsl. Rainfall, runoff, and groundwater inflows to the ultimate pit are projected (Section 3.2 below) to be about 100 ac-ft/yr, sufficient to sustain evaporation from a water surface of about 22 acres.

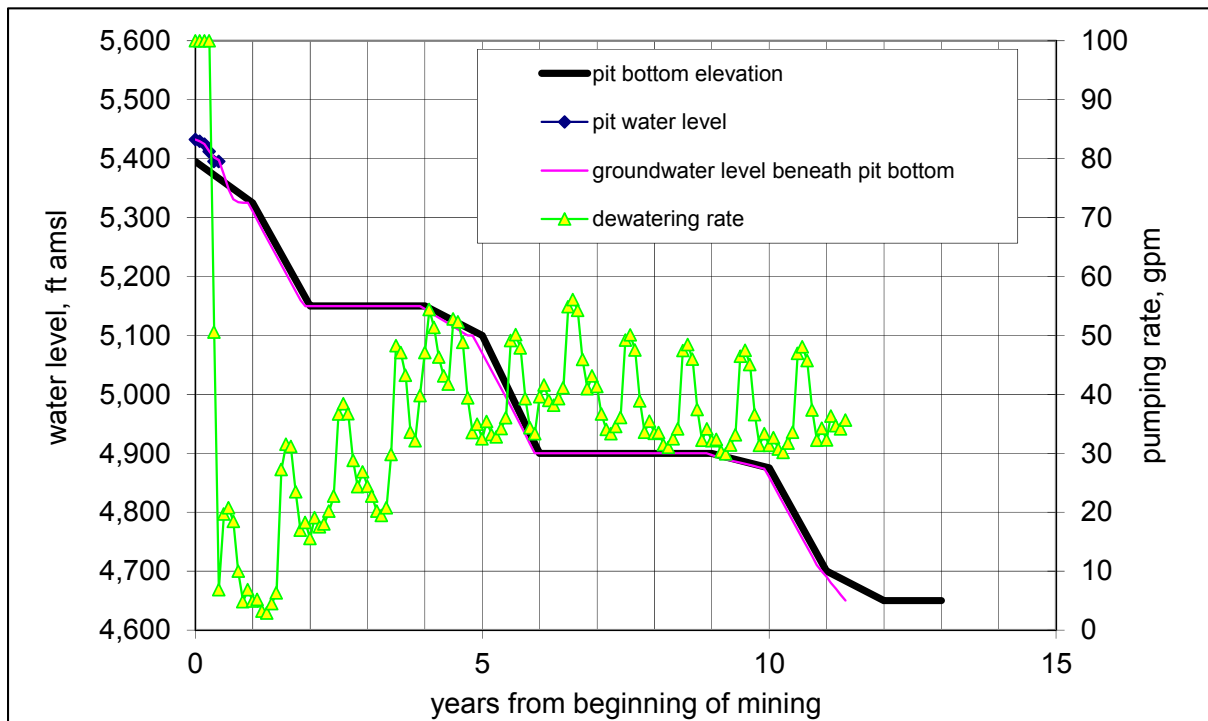


Figure 2.1. Projected pit bottom elevation, groundwater level, and dewatering rate.

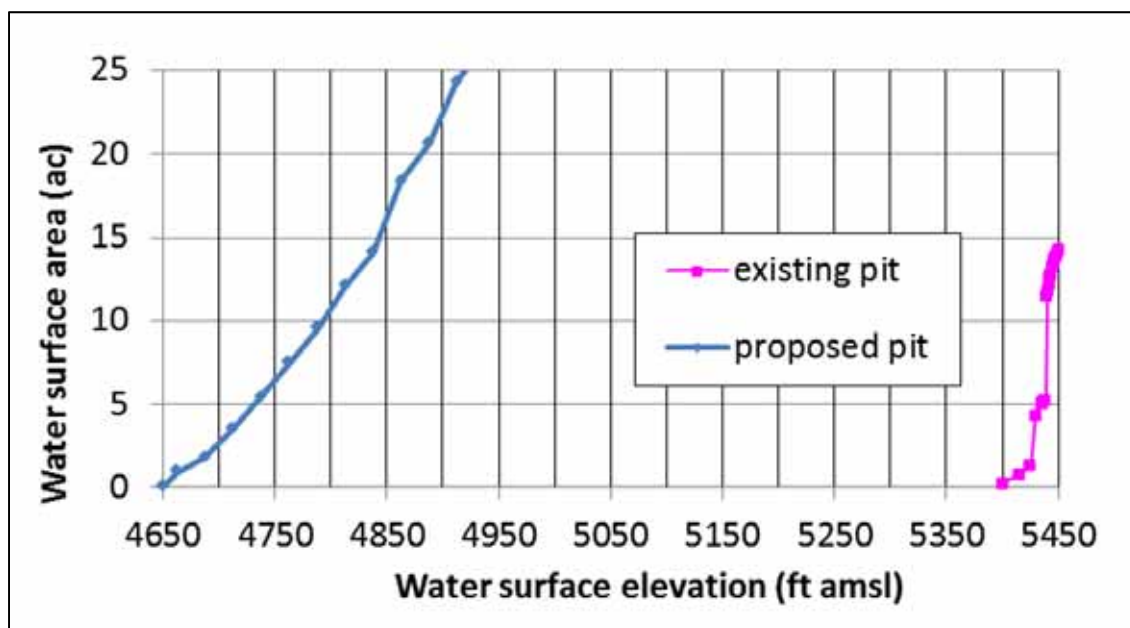


Figure 2.2. Current pit and final pit elevations and water-surface areas.

## **3.0 PROBABLE HYDROLOGIC CONSEQUENCES**

Probable hydrologic consequences are related to the direct hydrologic consequences of the Project:

1. Groundwater withdrawal from the SFG aquifer for mine water supply.
2. Groundwater withdrawal from the crystalline bedrock around the open pit.
3. Potential for infiltration of water from the TSF and WRSPs to groundwater systems.

### **3.1 Groundwater Withdrawals From the SFG Aquifer**

The most direct consequence of groundwater withdrawal from the SFG aquifer will be groundwater-level drawdown in the aquifer (Sec. 3.1.1). This will in turn result in changes to the aquifer water balance (Sec. 3.1.2), including increased inflow from the north Palomas Graben (Sec. 3.1.3), reduced discharge to the Rio Grande and Caballo Reservoir, reduced discharge to flowing wells, and reduced discharge to the Quaternary-age alluvial aquifers.

The consequences of reduced discharge to the Rio Grande and Caballo are discussed in Section 3.1.4. Potential consequences to other groundwater rights are discussed in Section 3.1.5, with the consequences of reduced discharge to flowing wells discussed in Section 3.1.6.

The potential consequences of reduced discharge to Quaternary-age alluvial aquifers, including reduced discharge to the perennial and riparian zone along Animas Creek, are discussed in Section 3.1.7.

Potential land subsidence, another possible consequence of groundwater drawdown, is discussed in Section 3.1.8.

#### **3.1.1 Regional Groundwater Level Drawdown**

Contours of projected groundwater-level drawdown at the end of mining in the SFG aquifer around the water-supply wells are shown on Figure 3.1. After the end of mining, water levels in the SFG aquifer will gradually recover to pre-mining levels over about 20 to 30 years.

The groundwater-level drawdown over time will in turn cause reduced discharge from the SFG aquifer to the Rio Grande and Caballo, and reduced discharge to other related hydrogeologic systems.

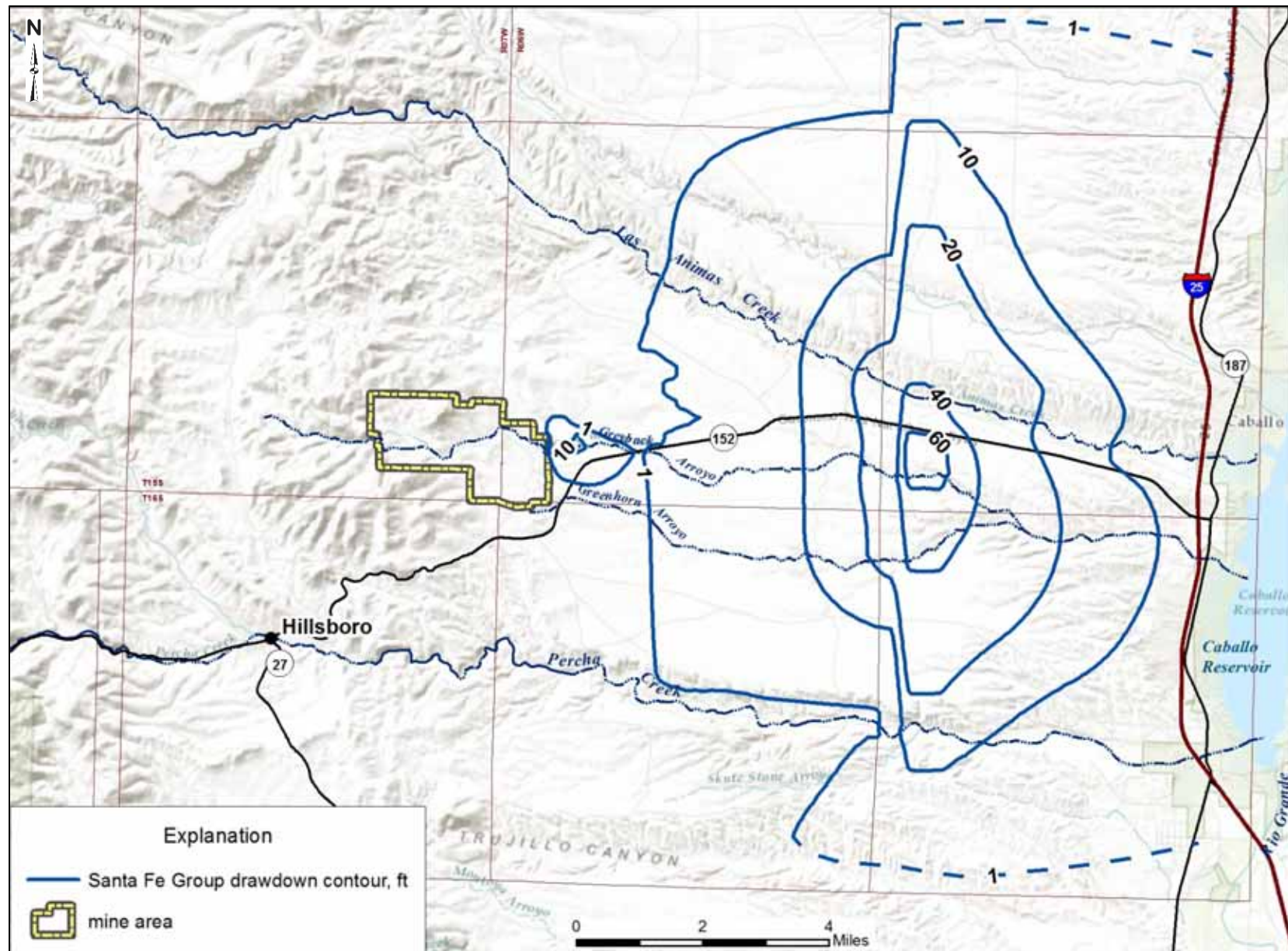


Figure 3.1. Projected end-of-mining groundwater drawdown in the SFG aquifer.

### 3.1.2 Effects on Water Balance

The groundwater pumped is initially removed from aquifer storage. Over time, more water is provided by increased inflow from the Palomas Graben north of the study area and by reduced discharge out of the study area. The sources of the water pumped are shown on Figure 3.2.

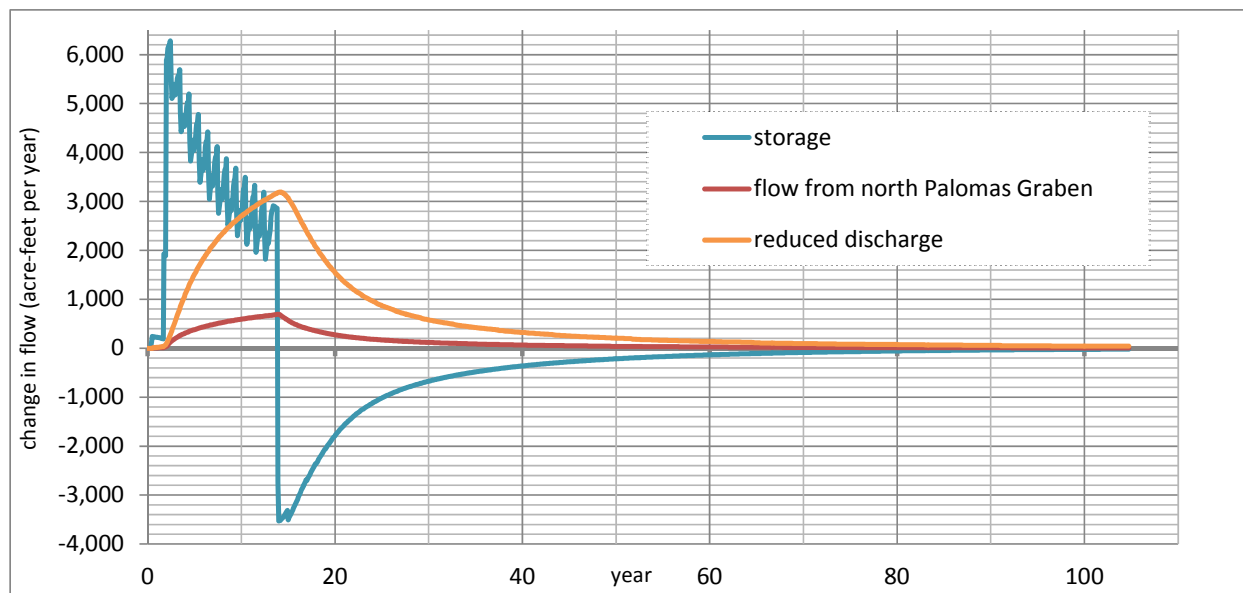


Figure 3.2. Projected sources of water pumped.

The hydrologic effect of additional inflow from the north Palomas Graben on the Rio Grande is estimated in Section 3.1.3.

The reductions in discharge are presented in detail on Figure 3.3, and include components of (1) reduced discharge to the Rio Grande both above and below Caballo Reservoir, (2) reduced discharge to flowing wells, and (3) reduced discharge to Quaternary-age alluvial aquifers and the Animas Creek perennial (riparian) zone.

The effects of reduced discharge to Caballo Reservoir and the Rio Grande are discussed in Section 3.1.4. The potential effects on other groundwater rights are discussed in Section 3.1.5. The potential hydrologic effects of reduced discharge to flowing wells are discussed in Section 3.1.6.

The potential hydrologic effects of reduced discharge to Quaternary-age alluvial aquifers and the Animas Creek perennial (riparian) zone are discussed in Section 3.1.7.

The projected water balance changes are summarized in Table 3.1.

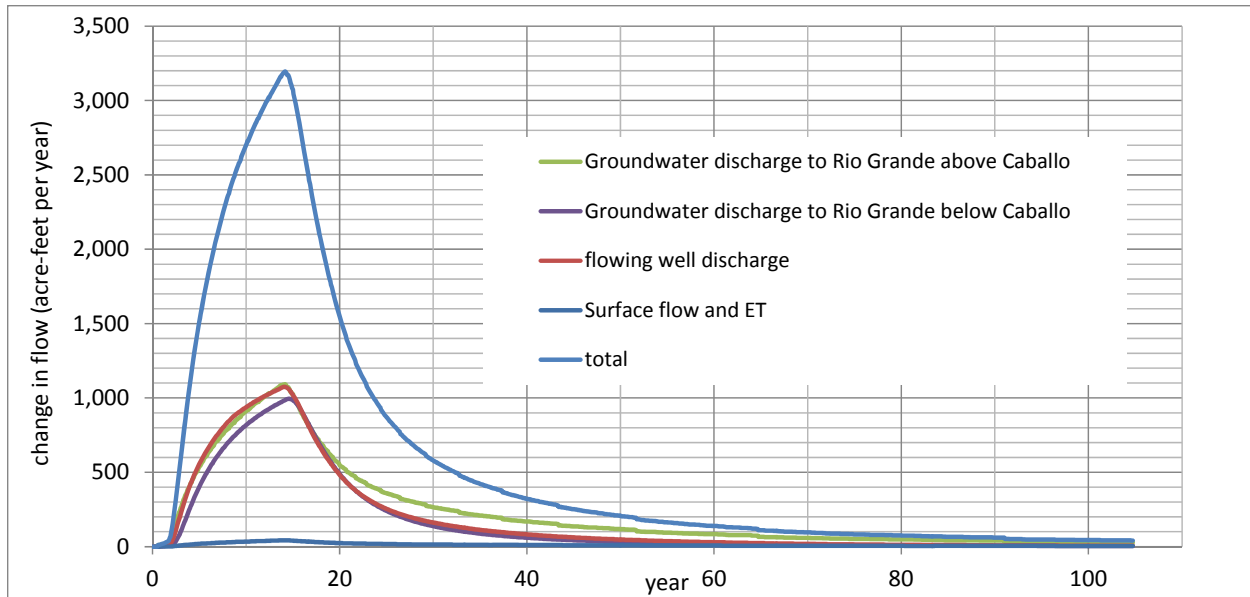


Figure 3.3. Projected reductions in discharge.

**Table 3.1. Summary of results**

change in flow, acre-feet/year		
parameter	rate 3 months after pit filling	rate 100 yrs after mining
storage	-3,525	-12
groundwater discharge to Rio Grande above Caballo Dam	1,089	25
groundwater discharge to Rio Grande below Caballo Dam	983	3
discharge from flowing wells	1,075	5
Animas Creek evapotranspiration and flow reduction	18	0
Percha Creek evapotranspiration and flow reduction	25	2
flow to open pit	28	29
inflow from graben north of study area	686	3
cumulated change in volume, acre-feet		
parameter	volume change 3 months after pit filling	
storage	42,813	
Rio Grande above Caballo Dam	8,878	
Rio Grande below Caballo Dam	7,504	
flowing wells	9,007	
Animas Creek flow and evapotranspiration	147	
Percha Creek flow and evapotranspiration	180	
flow to open pit	-467	
inflow from graben north of study area	5,924	
<b>total</b>	<b>73,987</b>	

### 3.1.3 Flow From North Palomas Graben

Induced groundwater flow from the Palomas Graben (Fig. 3.2) north of the study area would result in reduced discharge to the Rio Grande, beyond the reductions shown in Figure 3.3.

Based on discussions with the NMOSE, the effect of increased flow from north of the study area on the Rio Grande is estimated here using an analytical solution (Glover and Balmer, 1954; Theis, 1941) for the effect on streamflow of pumping a well.

The solution applied here simulates an impermeable barrier west of the Palomas Graben, reflecting the fault barrier and lack of aquifer transmissivity west of the graben.

A computer program employed by NMOSE (E. Keyes, personal communication, 2015) was used to compute the effect on the Rio Grande from removal of (the numerical model-computed) water from the graben, using assumptions listed in Table 3.2.

**Table 3.2. Parameters for Glover-Balmer solution**

transmissivity (ft <sup>2</sup> /day)	3,700
storage coefficient (percent)	10
distance from well to river (miles)	6
distance from well to barrier (mile)	1

Results are shown on Figure 3.4 for a scenario pumping a constant 6,100 ac-ft/yr for 12 years. The computed effect on the Rio Grande would be added to the “Rio Grande above Caballo” effect shown on Figure 3.3.

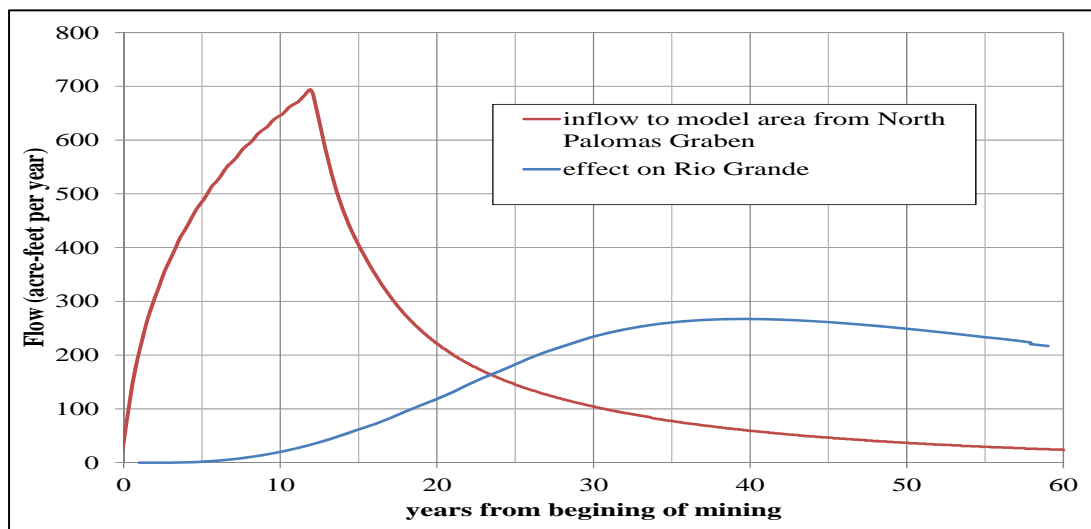


Figure 3.4. Projected effect on Rio Grande of increased flow from north Palomas Graben.

### **3.1.4 Operational Plans for No Net Effect on the Rio Grande**

NMCC has committed to offset the effects of reduced discharge to the Rio Grande system (Figs. 3.3 and 3.4) during and after the operation of the Copper Flat Mine to ensure no net reduction in flows of the Rio Grande, in a manner approved by the NMOSE.

NMCC has procured a lease for water from the Jicarilla Apache Nation (Nation) that has been approved by the United States Secretary of the Interior.

The Nation is the owner of water rights through a water rights settlement agreement authorized and adopted by the United States Congress and the State of New Mexico in the Jicarilla Apache Tribe Water Rights Settlement Act of October 23, 1992 (Settlement Act).

The Settlement Act expressly permits trans-basin transfers and the Nation currently has the right to lease 6,500 ac-ft/yr. The Jicarilla lease water is diverted from three tributaries in Colorado, diverted through the San Juan Chama project tunnels and is stored in Heron Reservoir in northern New Mexico.

The water purchased by NMCC for offset purposes will travel down the Chama River and into the Rio Grande in the same manner that other Jicarilla-leased water is allowed with the approval of the Secretary of Interior and NMOSE.

Flow of Jicarilla lease water arriving at Caballo Reservoir will be computed based on agreed-upon evaporation and conveyance losses between Heron Reservoir and Caballo Dam. NMCC will provide sufficient water arriving at Caballo Dam to offset the groundwater-flow model-computed effects (Figs. 3.3 and 3.4) both above and below Caballo Dam.

The Jicarilla lease has been executed by NMCC and the Nation and the agreement has been reviewed and approved by the United States Bureau of Reclamation action with the full authority of the United States Secretary of Interior. The lease specifically allows water to be utilized at the locations where NMCC pumping effects on the Rio Grande are predicted to take place.

All that remains to allow the diversion of Jicarilla lease water is NMOSE approval of the NMCC plan to use wells LRG-4652 through LRG-4652-S-3 (PW-1 through PW-4), which is pending an on-going proceeding and negotiation. NMCC is working with NMOSE to incorporate into the permit all monitoring, offsets, and replacement requirements deemed necessary to avoid impairment to other water users and impacts to the Rio Grande.

When the permit is issued, the conditions of approval will include an express condition by NMOSE, that the pumping effect on the Rio Grande will be offset by the water purchased under the lease from the Nation. The permit will address the length of time offsets and monitoring are necessary to protect the Rio Grande and existing water users after mine operations cease.



If NMCC, at some point after mine operation ceases and impacts to the river are decreasing, elects to stop leasing water from the Nation to provide for offsets on the river, NMCC will either secure another lease of equally effectual water or secure and permanently retire water rights. NMCC will supply the offset water in the quantity and location sufficient to offset the effects of NMCC pumping, in a manner agreed by NMOSE.

In the case of the permanent retirement of water rights, the offset would continue to have a positive effect on the Rio Grande even after the NMCC effect ceases. In any case, NMCC will take steps to ensure that no net reduction of flow to the Rio Grande occurs.

### **3.1.5 Other Water Rights**

The SFG aquifer will have a limited area of significant drawdown, which may directly affect a small number of private wells. During the operation of its production wells, NMCC will work with NMOSE to ensure that impairment to existing water rights, according to NMOSE criteria, shown to be caused by NMCC pumping, will be mitigated, as appropriate, so that there is no net loss of available water to the existing water right.

Flowing wells along the eastern ends of Animas Creek and Percha Creek will experience a reduction in artesian pressure and reduced flow, as described in Section 3.1.6.

Groundwater model projections indicate that private wells in the shallow aquifer along Animas Creek and Percha Creek will not be affected by the pumping of the NMCC production wells, as described in Section 3.1.7.

### **3.1.6 Effects of Reduced Flowing Well Pressure**

The model estimates a peak reduction in discharge to flowing wells of 1,054 ac-ft/yr, out of a pre-mining discharge of 2,030 ac-ft/yr (Table 3.1). The effect builds gradually from zero, to a maximum of 1,054 ac-ft/yr shortly after the end of mining, then gradually diminishes to near-zero over 30 years (Fig. 3.3). The possible consequences of reduced discharge to flowing wells are discussed below.

The flowing wells are located in the lower (eastern) section of the study area, upstream of Caballo Reservoir. Most of the wells are located along Animas Creek, with the remainder along Percha Creek. Estimated pre-mining discharge to flowing wells of 2,030 ac-ft/yr consists of 1,750 ac-ft/yr of discharge to Animas Creek wells and 280 ac-ft/yr to wells along Percha Creek.

In general, discharge from the flowing wells is used to fill unlined ponds, which in turn serve as reservoirs for irrigation systems. Most wells are allowed to flow continually, maintaining permanent ponds; these are visible in Google Earth images taken both inside and outside the irrigation season.

The discharge from flowing wells to ponds can evaporate from the pond, infiltrate into the shallow groundwater system or be pumped to irrigate fields. Water applied to the fields may be discharged as evapotranspiration or infiltrate to the shallow groundwater system.

Discharge from the flowing wells does not contribute significantly to streamflow, as there are no perennial stream sections in the artesian zone of the lower Animas and Percha Creek basins (INTERA et al., 2012). Flowing well discharge instead contributes to the shallow groundwater systems along Animas Creek and Percha Creek.

The pond and field areas along Animas Creek were estimated based on Google Earth, at 3.9 and 125.8 acres, respectively. By comparison, the 1966 hydrographic survey indicates 8.4 acres of pond and 191.2 acres of field. The estimated discharge from flowing wells is larger than would be required to irrigate the areas indicated. Pond and field areas are listed in Table 3.3, along with the maximum rate of evaporation and evapotranspiration (JSAI, 2014, section 2.4) that could occur from the given areas.

**Table 3.3. Areas and potential evapotranspiration for Animas Creek ponds and fields**

	<b>area (acres)</b>	<b>maximum ET (in./yr)</b>	<b>ET (ac-ft/yr)</b>
ponds	3.9	65	21
fields	125.8	65	681
<b>total</b>	<b>130</b>		<b>703</b>

ac-ft/yr - acre-feet per year

As indicated in Table 3.3, the maximum evaporation and evapotranspiration that could occur from the given areas of pond and field is 703 ac-ft/yr. This implies that most of the 1,750 ac-ft/yr of flowing well discharge along Animas Creek infiltrates to the shallow aquifer, either from the fields or through the ponds.

Current water balance for Animas Creek flowing wells was estimated assuming (1) typical application of irrigation water, with 70-percent evapotranspiration of the water applied and 30-percent infiltration to the shallow groundwater system, and (2) infiltration of any remaining flowing well discharge through the ponds. Results are presented in Table 3.4.

Some wells with reduced artesian pressure may be pumped in order to maintain water supply. Model-projected additional drawdown at the end of mining, due to pumping flowing wells at pre-mining rates, is shown on Figure 3.5. Incremental drawdown reaches a maximum of less than 10 ft in the lower reach of Animas Creek basin.

**Table 3.4. Estimated water balance for Animas Creek flowing wells**

flowing well discharge	<b>1,750</b>
evapotranspiration (ET)	703
infiltration (fields)	301
infiltration (ponds)	746
<b>Total (ac-ft/yr)</b>	<b>1,750</b>

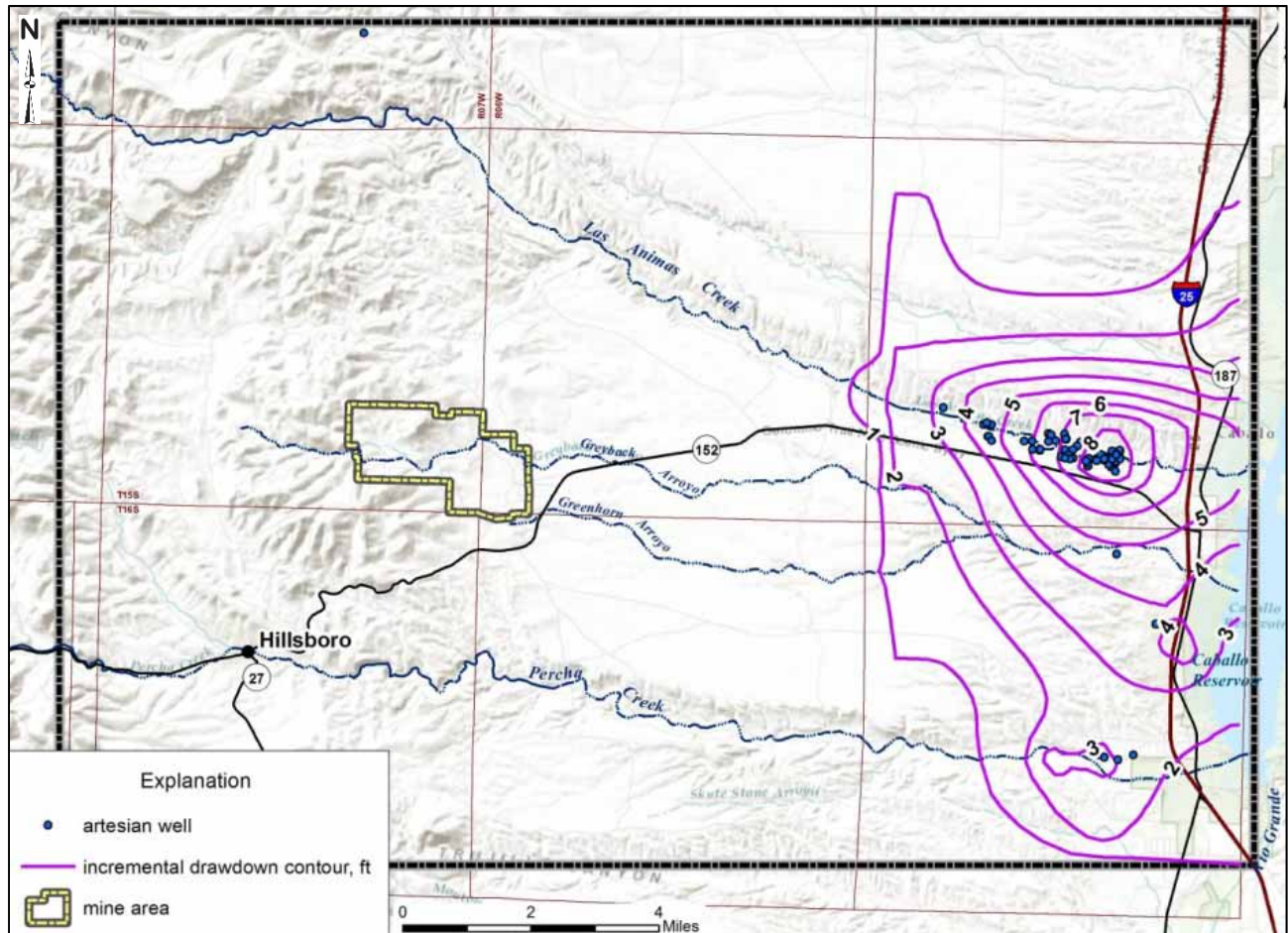


Figure 3.5. Projected incremental drawdown due to pumping of flowing wells at current flow rates.

### **3.1.7 Effects on Quaternary-Age Alluvial Aquifers and Animas Creek Perennial Flow and Riparian Zone**

The shallow groundwater and riparian systems along Animas Creek and Percha Creek overlie the SFG sediments. Geology of the study area is shown on Figure 3.6, showing faulting within the SFG. An important fault-bounded feature is the Palomas Graben, in which the Copper Flat water-supply wells are completed.

West of the graben, the SFG sediments are thinner and less permeable, and do not yield substantial flow to wells. Within and east of the graben, the SFG forms an aquifer capable of yielding substantial flow. The hydrologic relationship of the shallow alluvial systems to the SFG is illustrated in cross-section C-C' (Fig. 3.7) along Animas Creek.

West of the graben, the low transmissivity of the SFG results in elevated water levels reaching the level of the shallow alluvium. Flow between the SFG and the alluvium is limited by low transmissivity and the small water-level gradient between the two.

Near the graben, the increased transmissivity of the SFG results in water levels dropping below the bottom of the alluvium, forming a hydraulic disconnection between the SFG aquifer and the alluvial groundwater system (Fig. 3.8). As a result, water flows from the alluvium to the SFG, through low-permeability clay beds, only by gravity; pumping from the SFG does not increase the flow or change water levels in the alluvium.

East of the graben, water flows down-dip along the permeable SFG beds. In the lower part of the basin, water level in the SFG pressurizes the confining clay beds from below. Water discharges from the SFG to the alluvium and to Caballo reservoir by flowing slowly across the resistant clay beds, or by discharging to flowing wells.

As a result, groundwater-level changes in the shallow alluvium, due to pumping in the SFG, will be highly attenuated. The main area of groundwater drawdown in the SFG (Fig. 3.1) will be in the graben, where the alluvium is disconnected from the SFG (Fig. 3.7).

Away from the graben, SFG drawdown will be smaller, and the connection to the alluvium is limited by low-permeability clay beds (Fig. 3.8).

A contour map of projected groundwater-level drawdown within Quaternary-age alluvial aquifers at the end of mining is shown on Figure 3.9. The figure indicates that peak groundwater-level drawdown along Animas Creek and most of Percha Creek will be less than 1 ft. Drawdown in a small area along lower Percha Creek is projected to be greater than 1 ft and less than 2 ft. The projected effects on evapotranspiration and surface discharge from the shallow aquifers are correspondingly small (Table 3.1). After mining ends water levels will slowly recover to pre-mining levels.

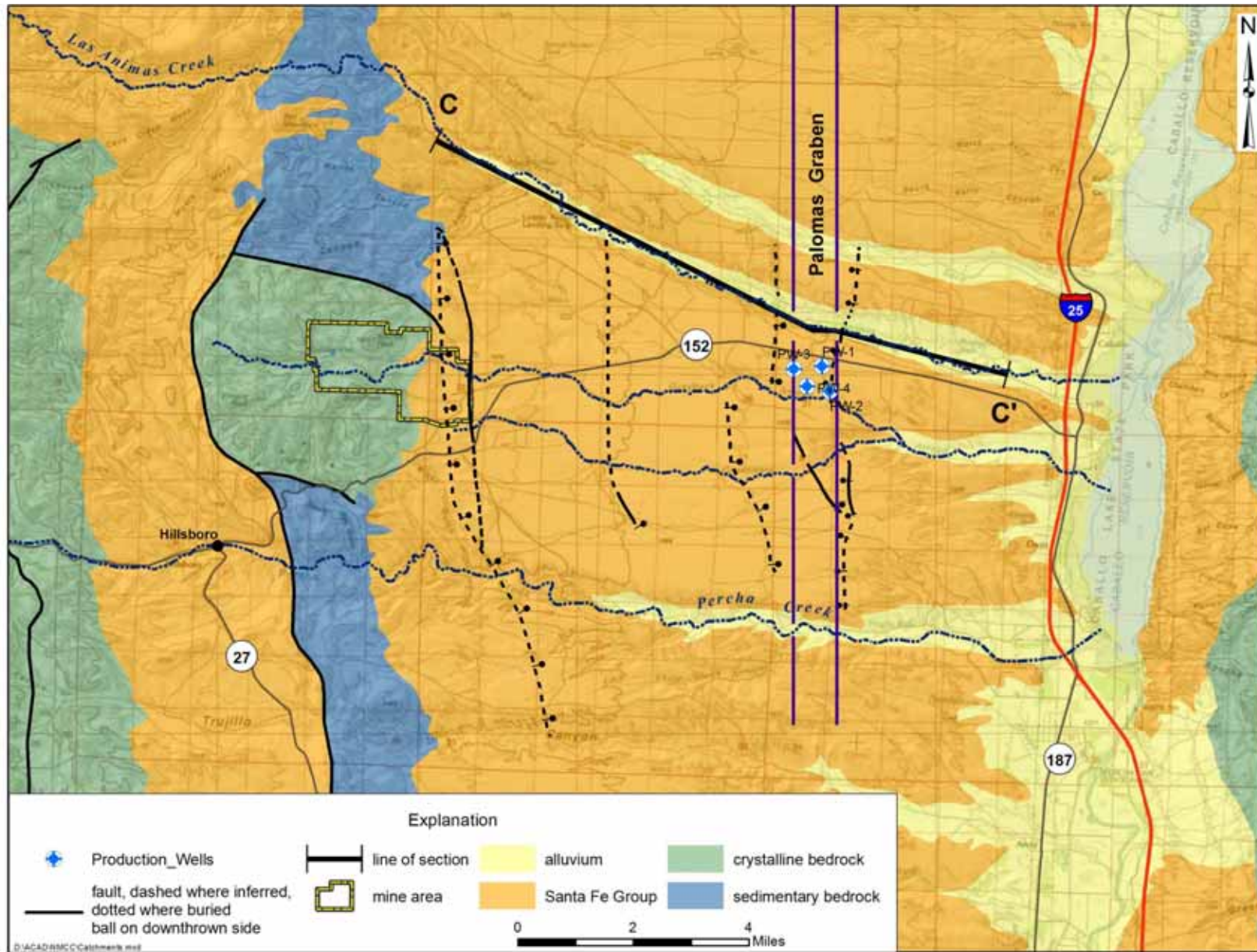


Figure 3.6. Geologic map.

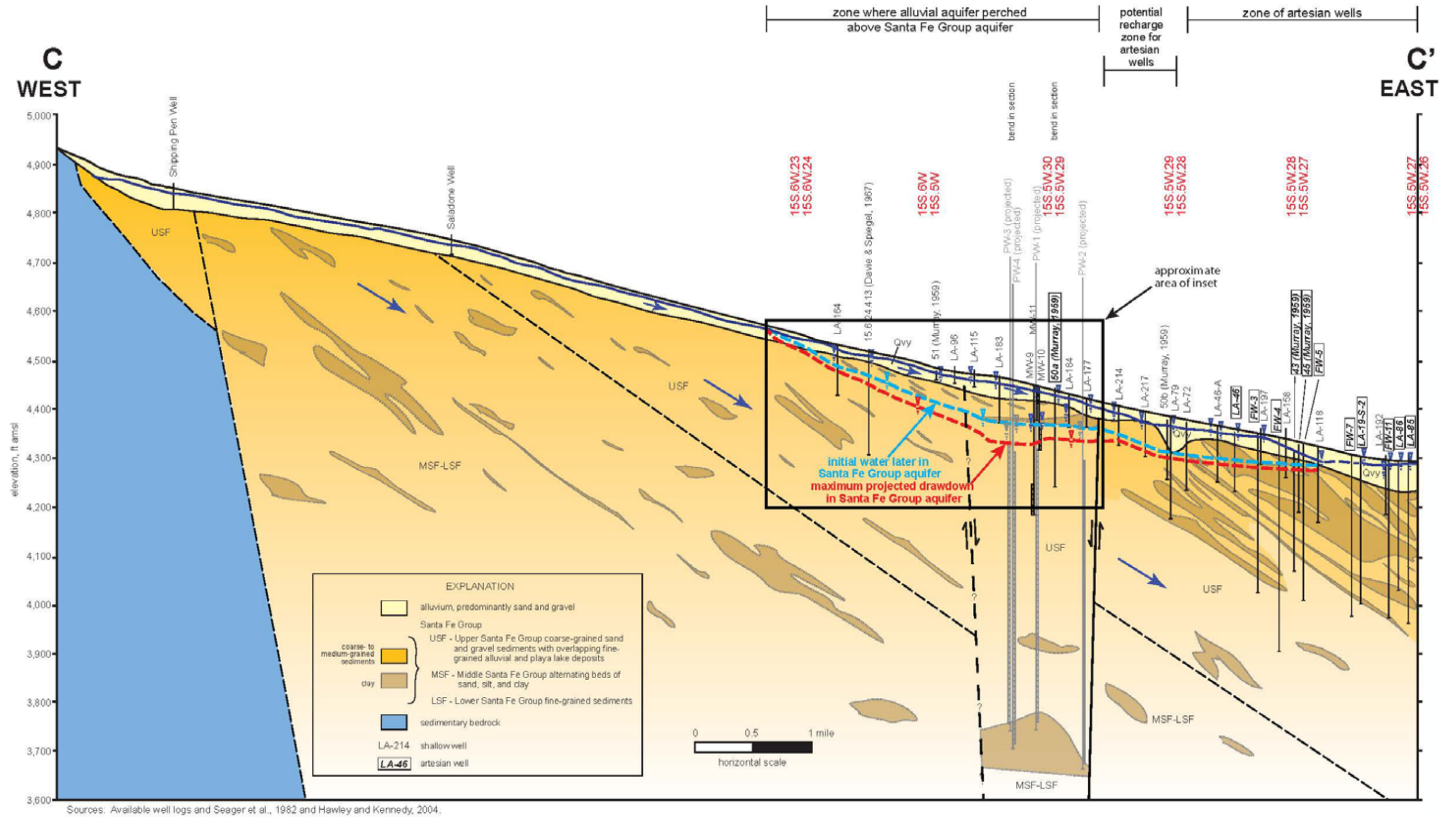


Figure 3.7. Cross-section C-C'.

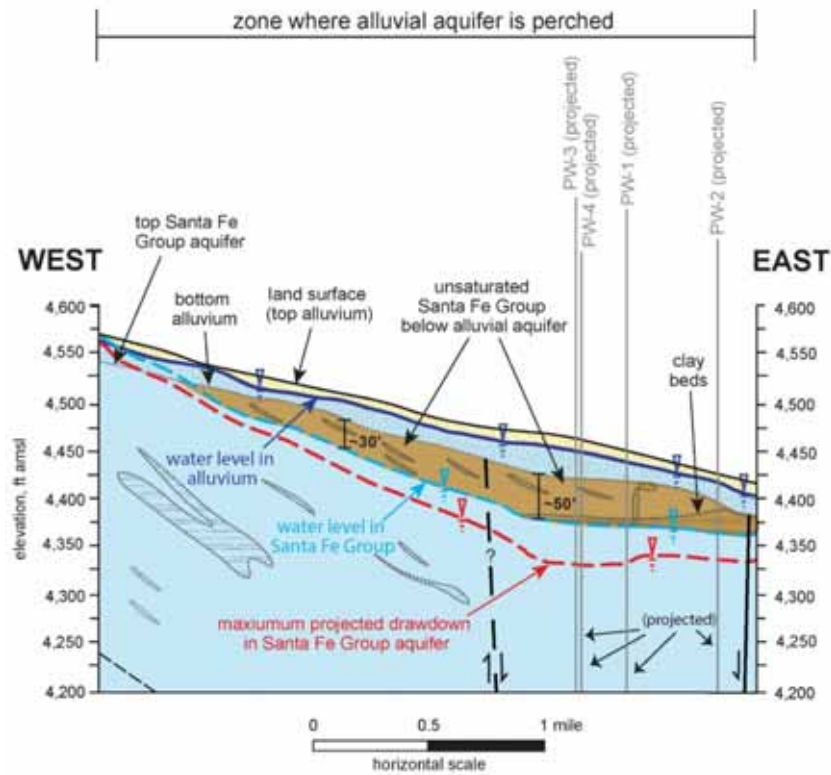


Figure 3.8. Section C-C', inset area of perched shallow aquifer.

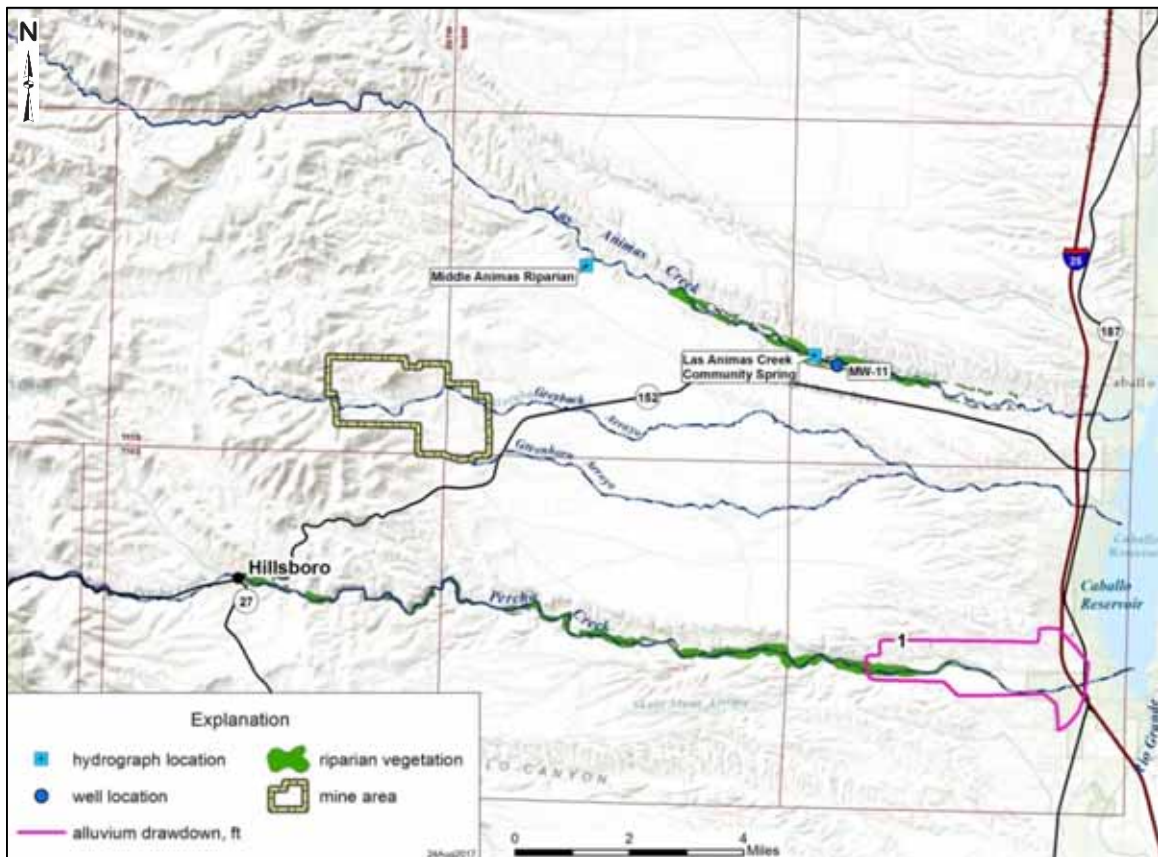


Figure 3.9. Projected end-of-mining groundwater drawdown, shallow aquifers.

### 3.1.8 Ground Subsidence

The potential for land surface subsidence due to groundwater-level drawdown was evaluated using the method of Hoffman and others (Hoffman et al., 2003). Potential subsidence due to dewatering of the crystalline bedrock is negligible; therefore, subsidence potential was evaluated only for the SFG aquifer around the well field.

Projected maximum drawdown (maximum drawdown near the well field occurs at the end of mining; maximum drawdown farther away may occur later) is shown on Figure 3.10, with an area-wide maximum drawdown of about 70 ft occurring at the well field.

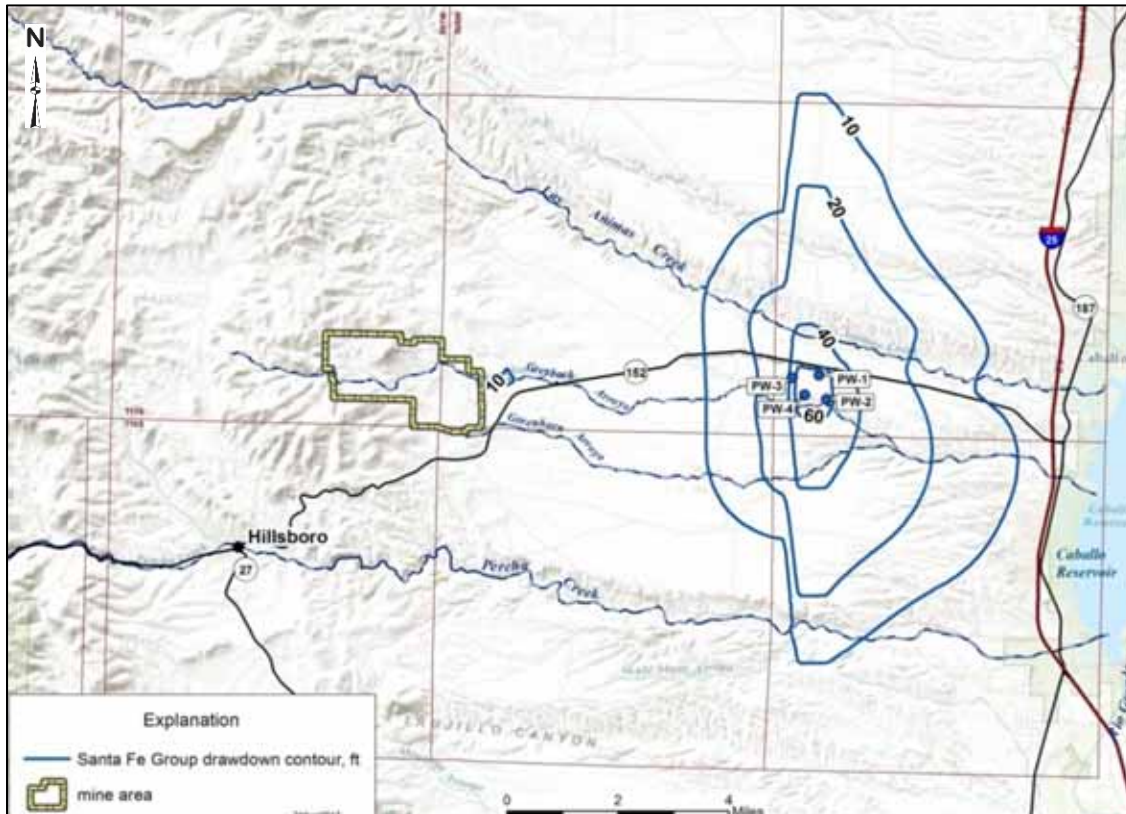


Figure 3.10. Projected maximum drawdown in Santa Fe Group aquifer.

Subsidence is estimated using equation (1) (Hoffman et al., 2003, equation 9):

$$\Delta b = S_s b \Delta h \tag{1}$$

where,  $b$  is the saturated thickness of compressible beds  
 $\Delta b$  is land surface subsidence  
 $S_s$  is the specific storage of the compressible beds  
 $\Delta h$  is drawdown

Thickness of compressible beds is assumed at 5,000 ft. Specific storage (storage coefficient per unit aquifer thickness) for SFG is modeled at  $2.0 \times 10^{-6}/\text{ft}$ . Maximum subsidence is then estimated using equation (2):

$$\Delta b = (2 \times 10^{-6} / \text{ft}) \times (5,000 \text{ ft}) \times (70 \text{ ft}) = 0.70 \text{ ft} \tag{2}$$



By using conservative assumptions, a maximum potential subsidence of 0.7 ft is calculated for the immediate area of the well field, where drawdown reaches a maximum. Subsidence decreases with distance from the well field area in proportion to drawdown. Contours of maximum potential subsidence are illustrated on Figure 3.11.

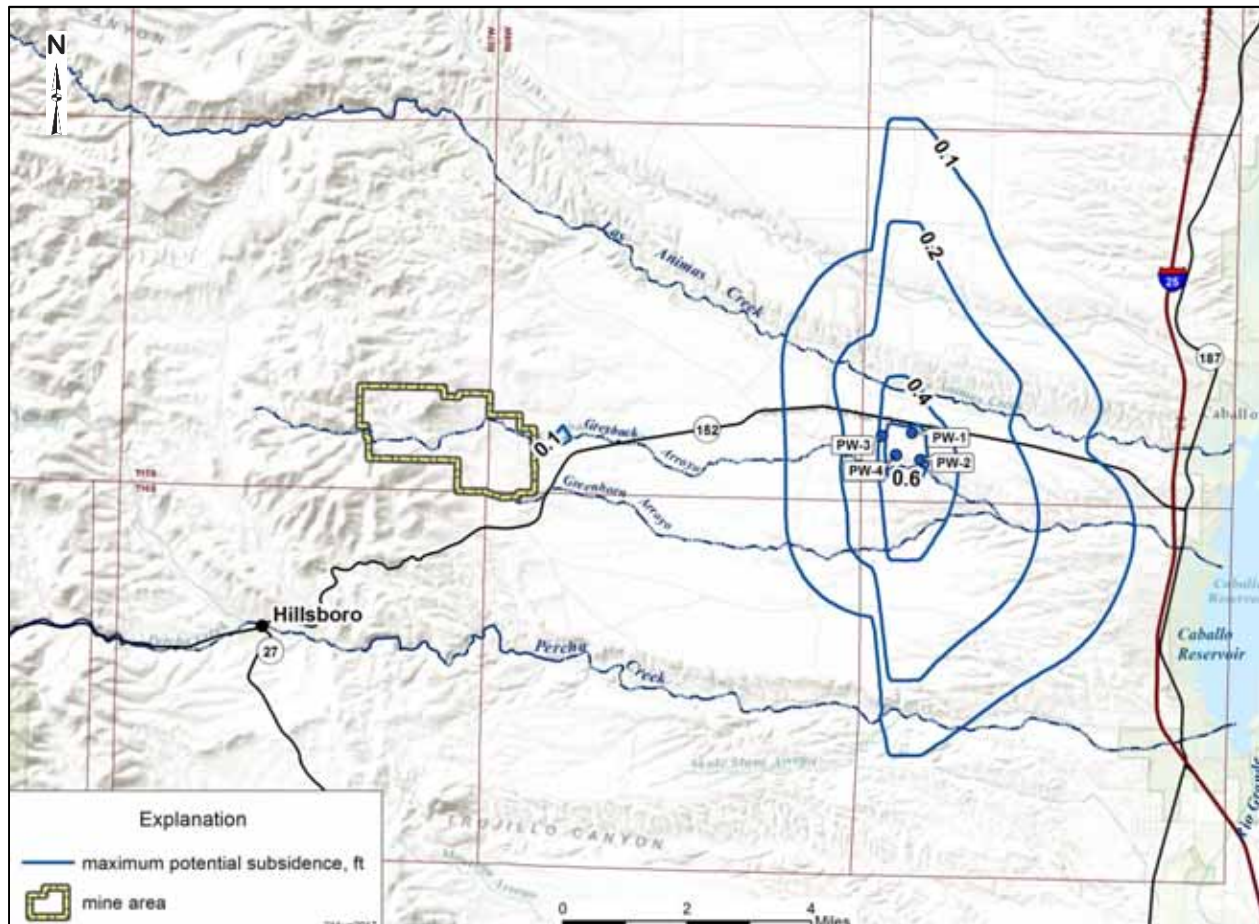


Figure 3.11. Projected worst-case potential maximum subsidence.

Outside of the well field area, the maximum potential subsidence shown on Figure 3.11 is less than about 0.4 ft (less than 5 inches), not noticeable over many years, but still over-estimated; it represents the total long-term subsidence that might be expected if groundwater drawdown is maintained.

Because the maximum groundwater drawdown would only occur near the end of mining, and would be immediately followed by post-mining water-level recovery, the drawdown would not persist for an extended period, and most of the potential subsidence would not occur. Actual subsidence is expected to be minimal at the well field and nil elsewhere.

## 3.2 Groundwater Withdrawals From the Crystalline Bedrock

Groundwater withdrawals from the crystalline bedrock will occur during dewatering of the open pit and after mining as groundwater flows into the pit. Consequences considered below include the following:

- Groundwater drawdown occurring during dewatering of the open pit is presented in Section 3.2.1.
- Groundwater discharge to the pit and the post-mining pit water balance are presented in Section 3.2.2.
- Potential discharge of groundwater from the open pit is discussed in Section 3.2.3.
- Long-term groundwater drawdown and potential effects on springs discharging from the crystalline bedrock are discussed in Section 3.2.4.

### 3.2.1 End-of-Mining Groundwater Drawdown

Groundwater drawdown in the crystalline bedrock at the end of mining is shown on Figure 3.12. Drawdown approaches a maximum of about 750 ft at the bottom of the dewatered pit. Drawdown of 1 ft extends for an approximately 2-mile radius around the pit.

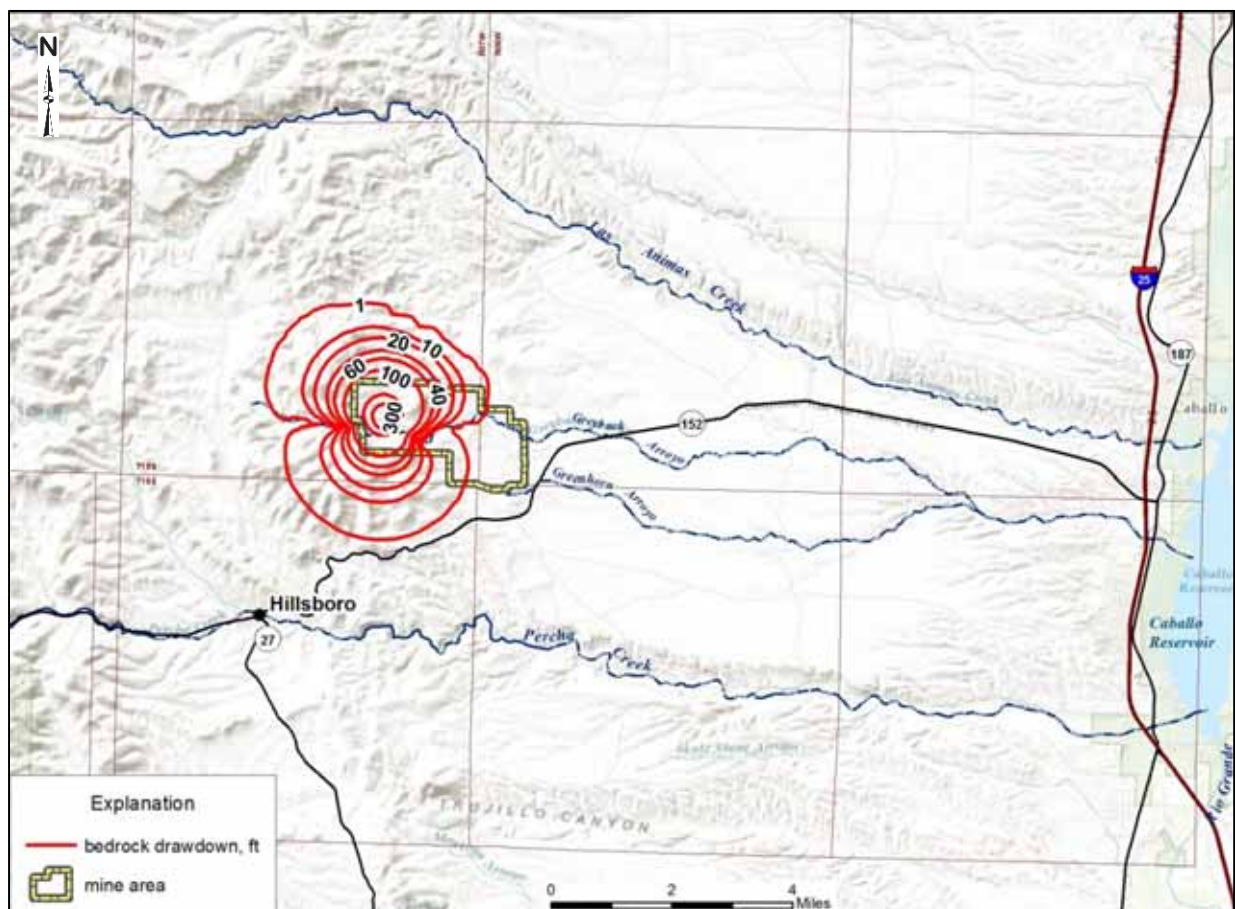


Figure 3.12. Projected end-of-mining groundwater drawdown in the crystalline bedrock.

### 3.2.2 Open Pit Water Balance

The post-mining pit water level and water balance were simulated assuming the pit geometry and watershed shown on Figure 3.13. The area within the pit highwall is about 129 acres, and the total pit watershed area is about 314 acres.

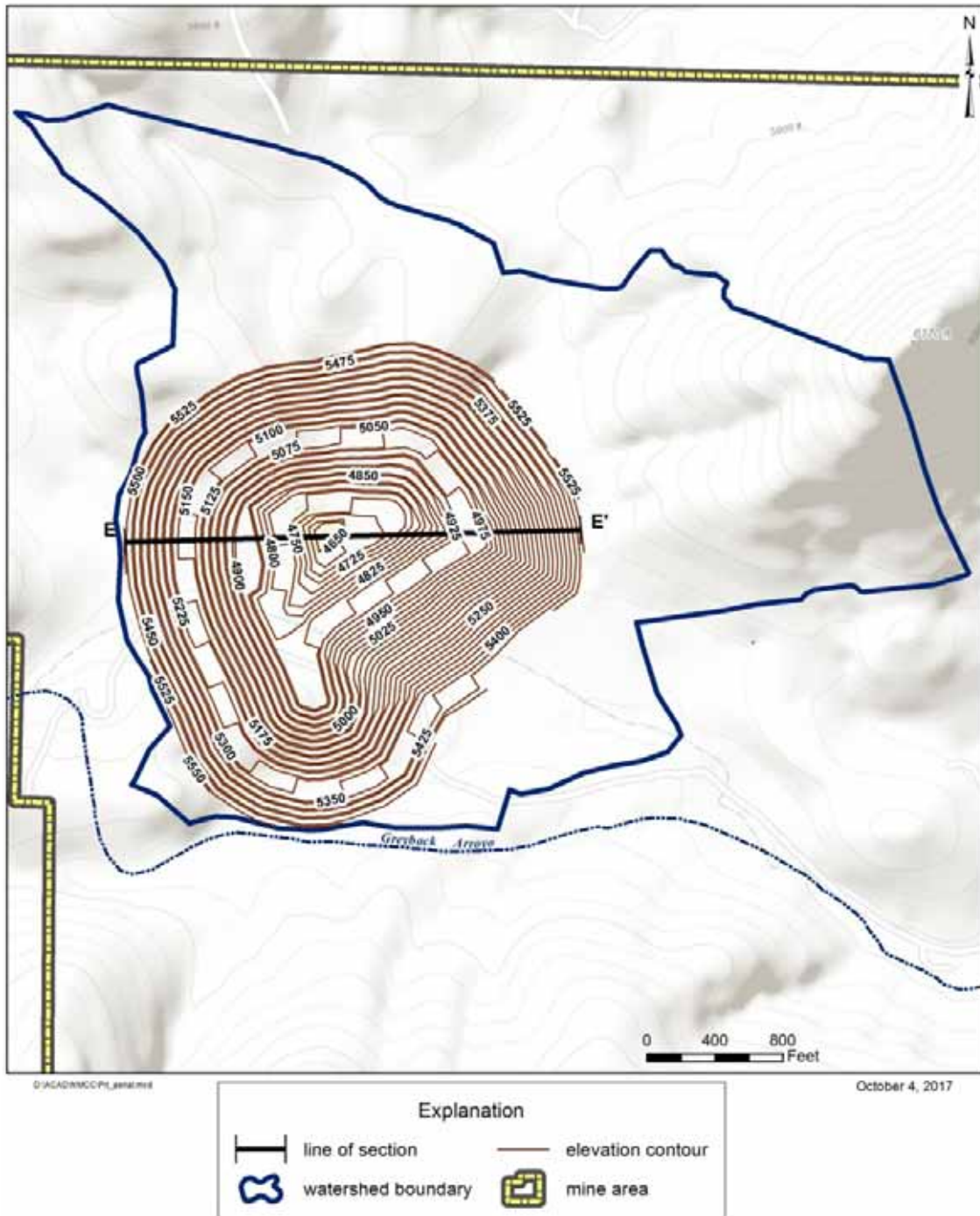


Figure 3.13. Ultimate open pit and watershed area.

Precipitation on the pit area was estimated for each month based on the record at Hillsboro (JSAI, 2014, section 2.0), with annual average precipitation of 12.5 in. Runoff from the un-reclaimed sections of the pit was simulated at 12.6 percent of precipitation, and runoff from reclaimed sections of the pit was simulated at 30.3 percent. Runoff from the remainder of the watershed was simulated at 7.1 percent of precipitation.

Evaporation from the open pit was assumed at 50 in./yr, less than the 65 in./yr estimated potential evaporation (JSAI, 2014, section 2.4) for the area. The lower rate reflects the wind and sun sheltering effects of the deep pit. Monthly evaporation rates based on the record at Hillsboro were scaled to match the annual rate of 50 in./yr.

Post-mining reclamation would include use of the water-supply wells PW-1 through PW-4, and a temporary pipeline to the bottom of the pit, to rapidly fill the pit to the expected long-term post-mining equilibrium water level. The post-mining simulation assumes this “rapid fill” scenario. Rapid filling will result in better water quality in the open pit by filling it with clean water and inhibiting oxidation of sulfide by submerging potential acid-generating sections of the pit wall (SRK, 2017).

A pumping rate of 2,726 gpm is simulated in the model, sufficient to fill the pit to elevation 4,894 ft amsl in 6 months. Total volume pumped from the supply wells will be 2,200 ac-ft. The open pit water body elevation of about 4,894 ft amsl corresponds to a water-surface area of about 21.7 acres. Water levels will fluctuate around this mean by a few feet, rising and falling seasonally and with wet and dry climatic conditions.

Simulated water level in the pit after the end of mining is presented on Figure 3.14. The final long-term water level of about 4,897 ft amsl corresponds to a water-surface area of about 22.3 acres. Water levels will fluctuate around this mean, rising and falling seasonally and with wet and dry climatic conditions.

The simulated annual pit water balance is presented on Figure 3.15, showing a final pit water balance of about 93 ac-ft/yr, with about 57 ac-ft/yr from precipitation and runoff, and 36 ac-ft/yr from groundwater inflow, all discharging as evaporation from the pit water surface.

After reclamation, groundwater levels in the bedrock around the open pit will remain below pre-mining levels, due to groundwater flowing to the open pit and discharging as evaporation from the hydrologic sink. Future water-level patterns can be seen in the hydrographs at selected locations, presented in Appendix A.

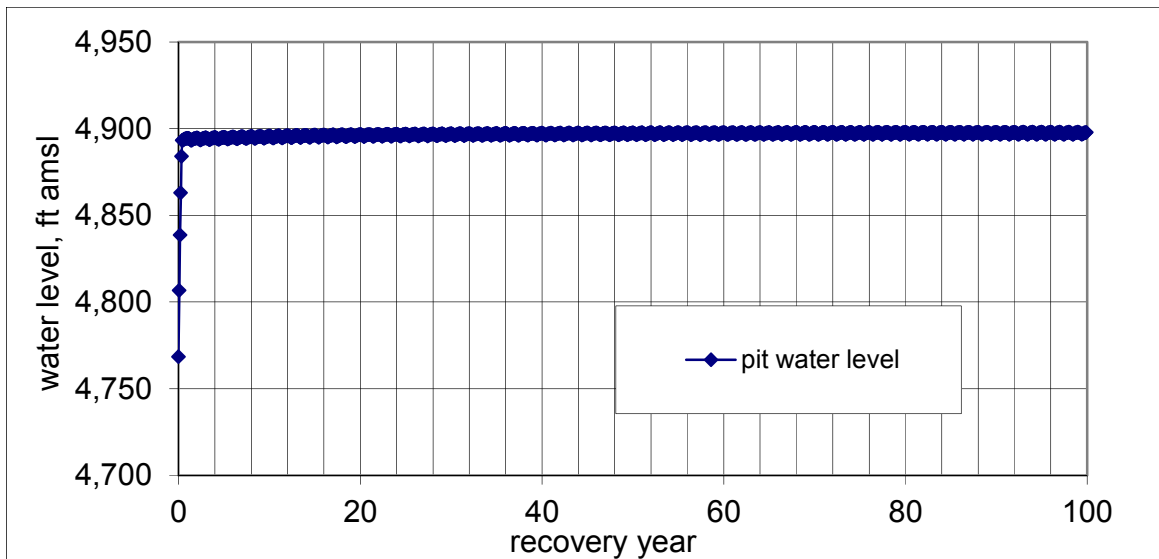


Figure 3.14. Projected open-pit water level (rapid fill in year 1).

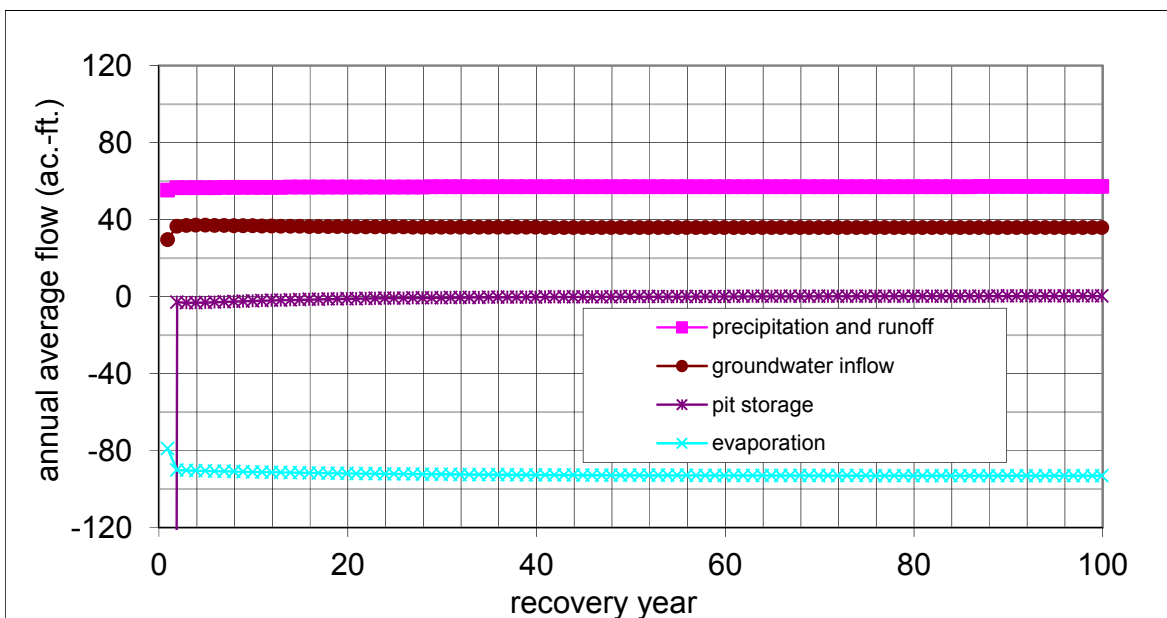


Figure 3.15. Projected open-pit water balance (rapid fill in year 1).

### 3.2.3 Potential Open Pit Discharge to Groundwater

The post-mining pit will be a groundwater sink, with the open pit water level below surrounding groundwater levels in the crystalline bedrock. The pit will remain a hydraulic sink after rapid filling of the pit during reclamation, and after precipitation events that raise the pit water level.

For a short period immediately following rapid fill, water may flow out of the pit into the dewatered space around it, then return to the pit as conditions equilibrate. Model-simulated flow to this dewatered space during the 6-month rapid filling totals 0.74 ac-ft. This water remains in the immediate vicinity of the pit wall before returning to the pit.

The hydraulic conditions around the pit are shown in cross-section on Figure 3.16 for pre-mining, end-of-mining, and 100-year post-mining conditions. The pit will remain as a hydraulic sink during temporary water level fluctuations because of the deep cone of depression caused by dewatering and maintained by water surface evaporation.

In order for it to be possible for water to flow from the pit to groundwater, the open pit water level would have to be higher than surrounding groundwater (>5,100 ft elevation). No conceivable storm event, wet year, or even wet decade could possibly add enough water to the pit to reach the water level required to achieve flow-through.

The projected post-mining potentiometric surface, including the closed contours around the hydraulic sink of the open pit, is shown in plan view on Figure 3.17.

### **3.2.4 Effects on Springs**

Spring locations identified in the area (INTERA et al., 2012; BLM, 2015) are shown on Figure 3.18. The springs fall into several groups: (1) springs discharging on the Animas Uplift, (2) springs discharging in the Animas graben west of the uplift and (3) springs discharging to the Palomas Basin, at the eastern edge of the uplift and along parallel fault trends stepping down from the Uplift into the Basin.

The springs of the Animas Uplift (BG1, BG2 and other occasional seeps) are fed by local, perched groundwater systems or by near-surface circulation of local precipitation, and are ephemeral (INTERA et al., 2012), flowing only after precipitation events. These would not be affected by the flow of groundwater toward the open pit within the crystalline bedrock.

Springs of the Animas Graben, including Warm Spring (WS), WSCS-A, CSCS-B, CSCS-C and Cave Creek Spring, discharge from the SFG deposits west of the Animas Uplift. The source of their water is the Las Animas Creek and Percha Creek watersheds west of the Animas Uplift. The andesite of the uplift acts as a barrier to flow at depth (JSAI, 2014, p.24) and the groundwater systems of the graben and the uplift are separate. Flow at springs in the Animas Graben will therefore not be directly affected by the movement of groundwater in the Animas Uplift toward the open pit.

Springs discharging at the east edge of the Animas Uplift include Warm Spring on Animas Creek and PCS-A on Percha Creek. In the Palomas Basin east of the uplift, springs discharge from alluvium along Las Animas Creek, along a set of fault structures parallel to the uplift.

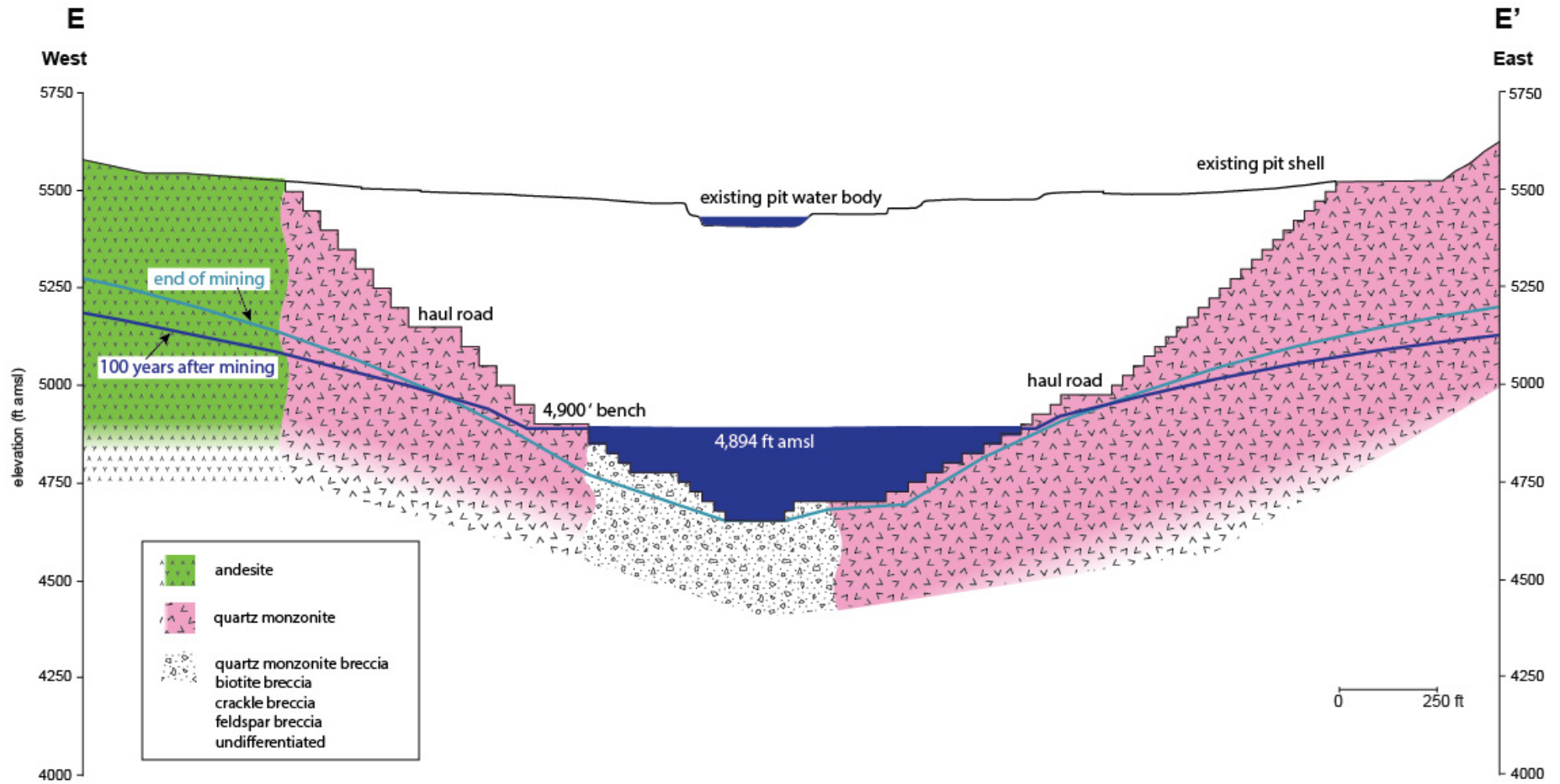


Figure 3.16. West-to-east hydrogeologic cross-section E-E' showing water-level profile across existing pit and proposed open pit after rapid fill.

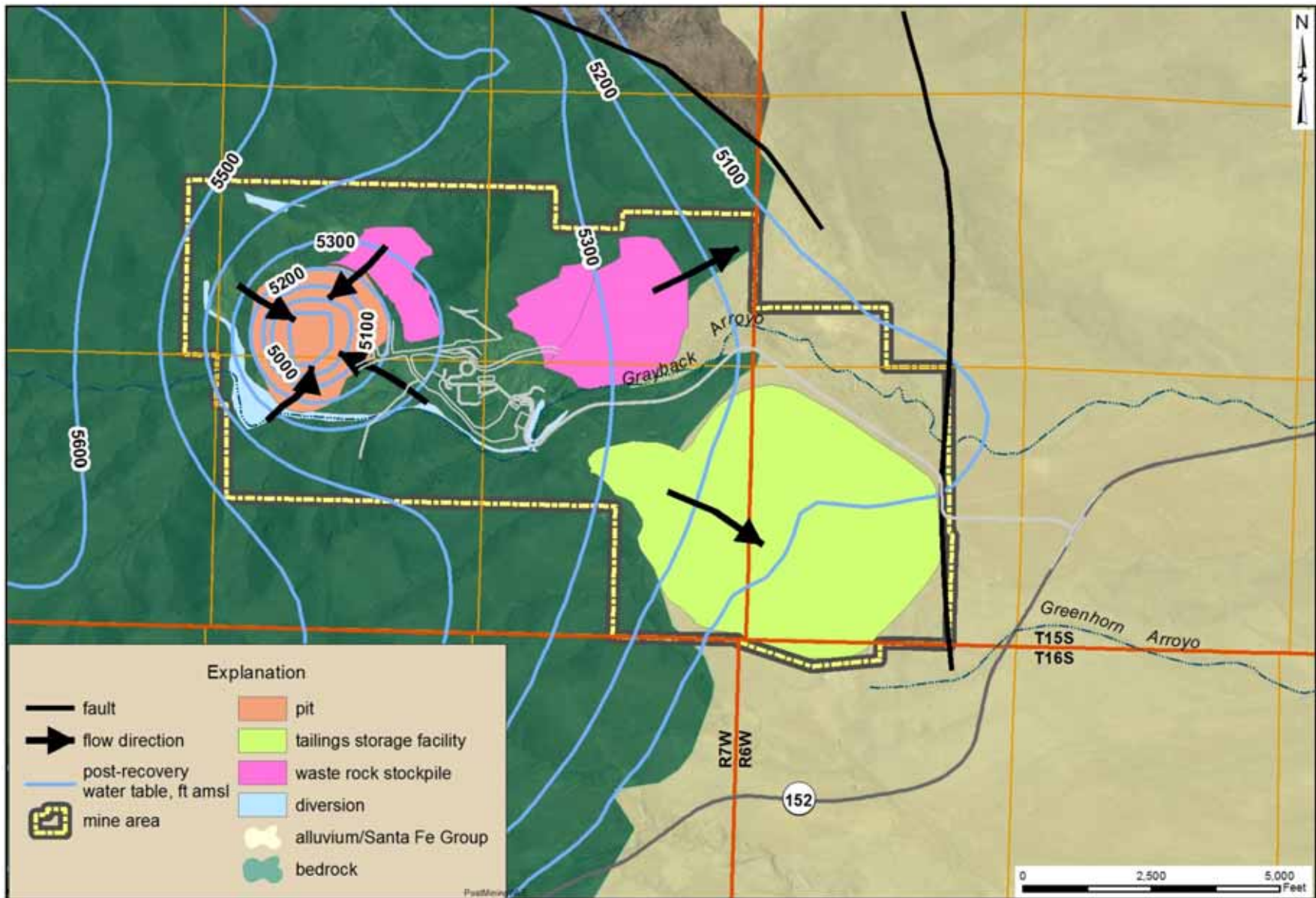


Figure 3.17. Proposed mine facilities and projected post-mining groundwater elevation.



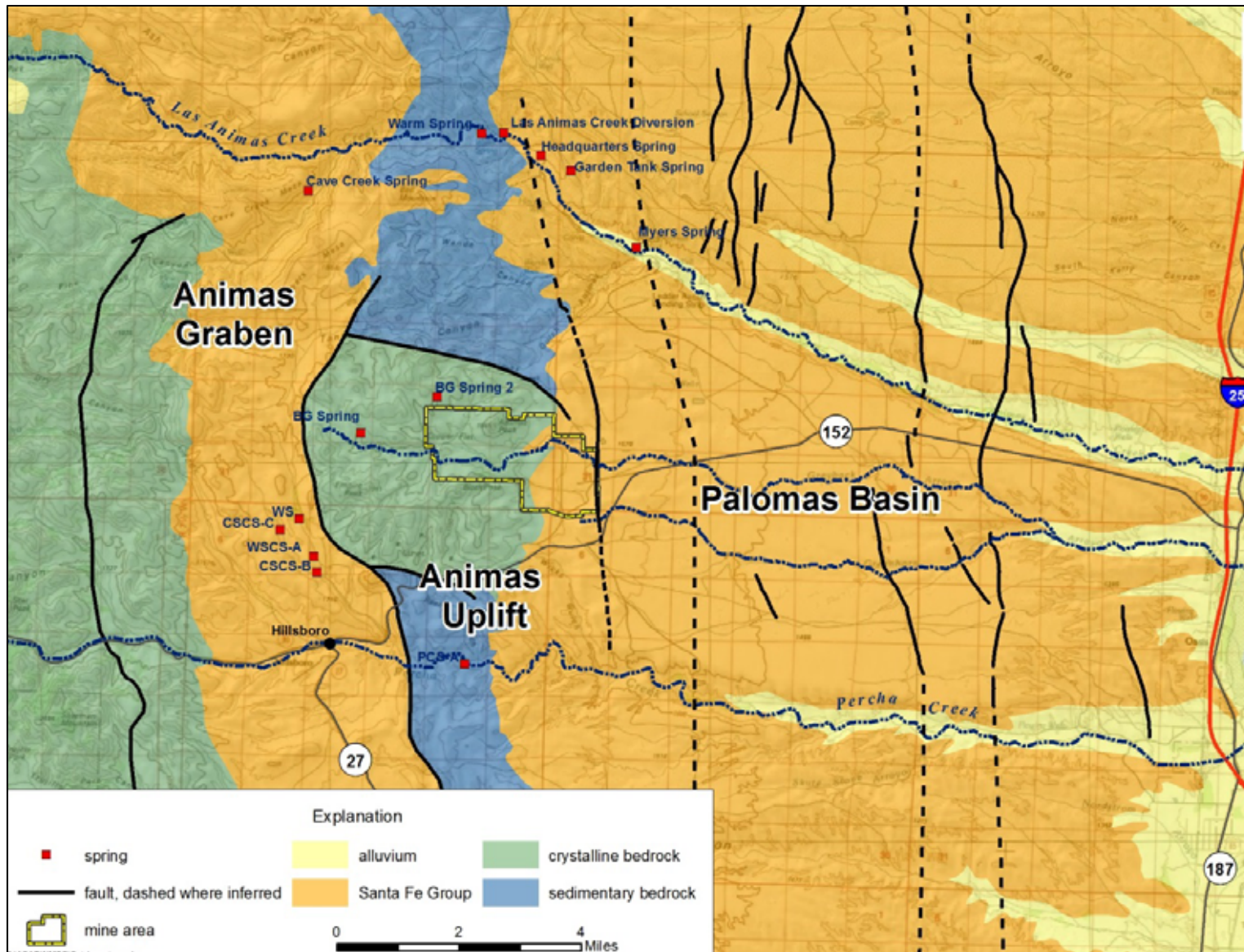


Figure 3.18. Locations of springs in and around the Animas Uplift.

The active springs of the Animas Graben and of the eastern edge of the uplift and the Palomas basin could be indirectly affected by the project if groundwater levels were lowered through indirect connection to the Animas Uplift. Future groundwater level change at each potentially affected location was evaluated using the numerical model. Results are summarized on Table 3.5.

**Table 3.5. Projected groundwater-level change (in ft) at spring locations**

	end of mining	100y post- mining
CSCS-C	0.01	0.16
WS	0.02	0.19
WSCS-A	0.01	0.13
CSCS-B	0.01	0.12
Cave Creek	0.05	0.15
PCS-A	0	0
(Animas) Warm	0.02	0.05
Myers	0.01	0.01

For the Animas Graben springs, groundwater level is projected to decline by up to 0.19 ft (2.3 in.), 100 years after the end of mining. Discharge is not expected to decrease because the source of water for these springs is west of the Animas Uplift (JSAI, 2014, p. 24). However, discharge locations could move a short distance due to a change in water level.

In the eastern part of the uplift, projected maximum change in water level is 0.05 ft (0.6 in.) at Animas Warm Spring. On Percha Creek, no water level change is projected at PCS-A, either during mining or in the 100 years following the end of mining. In the Palomas Basin, water level at Myers Spring is projected to decline by 0.01 ft (0.12 in.).

No direct effects to identified springs are predicted to occur as a result of the project, because (1) the springs of the Animas Uplift are ephemeral, precipitation-event-fed springs unrelated to the bedrock groundwater system, (2) the springs of the Animas Graben are fed by groundwater from the west and from depth, chemically unrelated to groundwater of the Uplift.

Small indirect effects may occur, however, due to lowering of groundwater levels in the Animas Graben or in the western edge of the Palomas Basin, due to an attenuated connection with the crystalline bedrock of the Animas Uplift. The small, long-term projected effects presented on Table 3.5 conservatively assume that these attenuated connections exist, although they have not been observed in reality.

In conclusion, the direct effects of the Project on mapped springs are projected to be zero. The long-term indirect effects presented (maximum of 2.3 inches over 100 years) are too small and manifest too slowly to be measureable or significant.

### 3.3 Potential Discharge From Tailings and Waste Rock Stockpiles

Potential for groundwater infiltration from the TSF is evaluated in Section 3.3.1. Potential for groundwater infiltration from the WRSPs is evaluated in Section 3.3.2. Groundwater flow paths and travel times down-gradient from the facilities are evaluated in Section 3.3.3.

The area of the mine including the open pit, waste rock storage facilities, and the tailings impoundment are shown above on Figure 3.17. The WRSPs lie on crystalline bedrock, while the TSF lies partially on SFG sediments.

Contours of the projected post-mining water-table surface, and arrows indicating the directions of groundwater flow, are also shown on Figure 3.17. Any infiltration from the WRSP around the pit would flow into the pit, while any infiltration from the eastern-most WRSP or from the TSF would flow northeast and southeast, respectively.

#### 3.3.1 Tailings Infiltration

Because the tailings impoundment will be lined, infiltration from the tailings is not expected. However, unexpected sources of potential infiltration include manufacturing defects in the liner and other holes, in the liner and along the seams, developed during placement.

NMCC considers the potential for leaks in the liner to be very unlikely. Nonetheless, the potential occurrence of leaks in the tailings facility liner was evaluated based on previous analyses presented in Appendix B. An assumed liner leak occurrence for the purpose of evaluation is one circular defect per acre, with a standard defect area of 1.0 cm<sup>2</sup> (corresponding to a round hole diameter of 1.128 cm).

The rate of leakage through the defect, assuming a compacted bedding layer beneath the liner and an underdrain system above the liner (Golder, 2016), is given (Appendix B, equation 1) by

$$q = \beta_c [1 + 0.1(h_w/L_s)^{0.95}] a_d^{0.1} h_w^{0.9} K_s^{0.74}$$

- where,
- q is flow through a circular defect
  - $\beta_c$  is the coefficient relating to liner contact with bedding material (0.21 for good contact)
  - $h_w$  is the depth of water above the geomembrane
  - $L_s$  is the thickness of bedding material
  - $a_d$  is the area of the defect (1 cm<sup>2</sup>)
  - $K_s$  is the saturated hydraulic conductivity of bedding material

Because the impoundment is designed with a 1.5-ft-thick drainage layer above the liner (Golder, 2016), head on the liner  $h_w$  will be less than 1.5 ft. Assuming the standard defect size ( $a_d = 1.0 \text{ cm}^2$ ) occurring once per acre and the design bedding layer conductivity ( $K_s = 10^{-6} \text{ cm/s}$ ), leakage from the lined 536-acre (Golder, 2016) tailings storage facility is estimated in Table 3.6 at about 0.5 gpm. The total area of the tailings storage including surrounding facilities is approximately 630 acres, but the active storage area is 536 acres. As shown by the data in Table 3.6, the probable hydrologic consequence from a postulated leak in the liner is nil.

**Table 3.6. Potential tailings liner leakage**

$B_c$	0.21
$h_w$	1.5 ft
$L_s$	1 ft
$a_d$	1.0 $\text{cm}^2$
$K_s$	$1 \times 10^{-6} \text{ cm/s}$
$q$	0.0009 gpm/acre
total flow	0.5 gpm

### 3.3.2 Waste Rock Stockpile Infiltration

Significant infiltration from the waste rock is unlikely because:

1. The waste rock lies on low-permeability (model-calibrated permeability  $< 10^{-6} \text{ cm/s}$ ) andesite bedrock. The andesite will function as a liner.
2. The waste rock will be deposited dry; precipitation infiltrating into the waste rock will tend to be held in storage above the water table. Depth to water in the area ranges from about 50 to 100 ft below ground level.
3. The waste rock deposits will be reclaimed and covered with a 3-ft-thick engineered low-infiltration store-and-release type cover. The cover will have the capacity to retain most precipitation events in storage, without percolation through to the waste rock below. Stored precipitation will eventually be released as evapotranspiration. The cover would only allow water into the waste rock after extreme precipitation events.

To summarize, the waste rock cover will prevent most precipitation from reaching the waste rock. The water infiltrating will be held in storage due to the unsaturated state of the waste rock. During operations, any water that flows through the waste rock will reach the low-permeability andesite liner, and will flow along the andesite surface to collection

ponds at the base of the facility. Post-mining, the reclamation cover will prevent water infiltration into the waste rock. Surface water run-on will be diverted around and away from the reclaimed WRSPs.

Therefore, water available to enter the SFG aquifer is expected to not provide a significant probable hydrologic consequence.

Infiltration through the cover was evaluated based on reasonable hydraulic properties for a single-layer cover material, shown in Table 3.7, and on the distribution of daily evaporation and the more than 100-year record of daily precipitation at Hillsboro (JSAI, 2014, fig. 2.3), using a 1-dimensional model of variably saturated flow and infiltration (Niswonger et al., 2006).

**Table 3.7. Waste rock cover properties**

saturated water content (percent)	20
initial water content (percent)	6
residual water content (percent)	6
Brooks-Corey exponent	2.5
cover thickness (ft)	3.0
saturated hydraulic conductivity (cm/s)	1.0E-04
specific storage (ft <sup>-1</sup> )	1.00E-06

cm/s - centimeter per second

Results indicate long-term infiltration through the cover of about 2 percent of precipitation, or about 0.25 in./yr. Out of the total area of the waste rock stockpiles, approximately 287 acres is subject to infiltration. Over the 287 acres, total infiltration through the cover would be about 2.1 gpm.

Of the estimated infiltration through the cover, most is expected to be retained in the waste rock. Assuming field capacity (the water content retained before downward percolation begins) of the waste rock of 6 percent, 0.25 in. of infiltration would wet a 4.2-in. thickness of waste rock to field capacity. At this rate, it would require hundreds of years of repeated infiltration events to produce internal flow of water within the waste rock.

Of the infiltration through the cover that is not retained in the waste rock (discharging through preferential flow paths), most will flow on top of the andesite. Discharge into the groundwater system (SFG aquifer) is expected to be nil.

## 4.0 CONCLUSIONS

The probable hydrologic consequences from development of the Copper Flat Project were evaluated for the mine area and affected area using the numerical model of groundwater flow developed by JSAI (2014).

The objective of this report was to develop a determination of the probable hydrologic consequences of the operation and reclamation, on both the permit area and the affected area, with respect to the hydrologic regime, quantity and quality of surface and groundwater systems that may be affected by the proposed operations (NMAC 19.10.6.602.(13)(g)(v) of the Mining Act regulations).

Groundwater systems include:

- The regional SFG aquifer.
- Quaternary-age alluvial aquifers along Animas Creek and Percha Creek.
- The crystalline bedrock of the Animas Uplift.

Surface water includes:

- Perennial flow in the Rio Grande and Caballo Reservoir that is supplied in part by discharge from the SFG aquifer.
- An area of perennial flow and riparian vegetation along Animas Creek where the Quaternary alluvial aquifer discharges to the surface.
- An area of perennial flow and riparian vegetation along Percha Creek, atop the crystalline bedrock.
- Springs discharging from the crystalline bedrock.
- Storm water flows in Grayback Arroyo.

The sources of possible hydrologic consequences of the Project include:

1. Groundwater withdrawals from the SFG aquifer: The mine water supply will be withdrawn from pumping wells PW-1, PW-2, PW-3, and PW-4. Water level in the SFG aquifer will be lowered around the well field and then gradually recover after mining. Secondary effects evaluated include:
  - a. Reduced groundwater discharge to Rio Grande and Caballo Reservoir.
  - b. Reduced flow to artesian wells and other effects to local groundwater users.
  - c. Potential reduced discharge to shallow aquifers along Animas Creek and Percha Creek, leading to lower alluvial water levels and reduced discharge to the perennial flow and riparian areas along Animas Creek.
  - d. Potential ground subsidence.

2. Groundwater withdrawals from the crystalline bedrock associated with the open pit. Water levels in the bedrock around the pit will be permanently lowered, and groundwater will flow to the pit and evaporate. Groundwater flow rates to the pit and the future open pit water level and water balance area assessed. Secondary effects evaluated include:
  - a. Potential groundwater discharge from the open pit.
  - b. Potential effects on springs discharging from the crystalline bedrock and on the Percha Creek perennial (riparian) area.
3. Potential for groundwater discharge from the WRSPs and TSF.

#### **4.1 Groundwater Withdrawals From the SFG Aquifer**

Water-level drawdown in the SFG aquifer is projected to reach a maximum of about 70 ft at the well field, at the end of mining. Maximum drawdown decreases with distance from the well field. Water levels will then recover over a period of about 20 to 30 years.

Total reductions in discharge to the system are projected to peak at a total of about 3,100 ac-ft/yr shortly after the end of mining, then diminish to near-zero over about 30 years (Fig. 3.3).

- Flow induced from the Palomas Graben north of the study area is projected to reach a maximum of less than 800 ac-ft/yr at the end of mining, which is estimated to result in an additional reduction of discharge to the Rio Grande by a maximum of 275 ac-ft/yr.
- Effects on the shallow groundwater (riparian) systems along Las Animas Creek and Percha Creek are projected to be minimal, with a maximum of less than 2 ft of groundwater-level change on Percha Creek, less than 1 ft of groundwater-level change on Animas, and non-measurable small changes in surface flow and riparian evapotranspiration.
- Depletion to the Rio Grande is projected to peak around 2,080 ac-ft/yr at the end of mining, then reduce to 28 ac-ft/yr 100 years after mining (Fig. 3.3; Table 3.1)
- Groundwater withdrawals for water supply are not expected to result in measurable ground subsidence.

As required by NMOSE, NMCC will offset any reductions in discharge to the Rio Grande by lease or purchase of additional water rights in the amount of the model-simulated reductions to flow.

NMCC will work with the NMOSE to ensure that impairment to existing water rights by NMCC pumping, according to NMOSE criteria, will be mitigated, as appropriate, so that there is no net loss of available water to existing water rights.

No water-quality effects are expected from pumping the proposed supply wells in the affected area.

## **4.2 Groundwater Withdrawals From the Crystalline Bedrock**

At the end of mining, groundwater-level drawdown in the bedrock around the open pit reaches a maximum of about 800 ft at the pit. A permanent cone of depression will form around the pit, with maximum drawdown of about 600 ft at the edge of the pit. The pit, which currently is an evaporative hydrologic sink, will form an evaporative hydrologic sink again in the future.

Final pit water level after mining is projected to be about 4,894 ft amsl, about 640 ft below the pit rim. The open pit water body that forms after mining and rapid fill remediation will be about 250 ft in depth and have a steady-state surface area of about 22 acres. Steady state groundwater inflow is estimated at 36 ac-ft/yr and captured storm-water runoff is estimated at 57 ac-ft/yr. Pit water evaporation is projected to be about 93 ac-ft/yr.

During operations and after reclamation, storm-water flows from Grayback Arroyo will be conveyed around the open pit in the existing bypass channel and through the mine area with no expected hydrologic consequences. Water quality effects for the open pit water body are addressed in a separate report prepared for the project.

Long-term, indirect effects to springs discharging in and around the Animas Uplift are projected to be minimal and not measureable.

## **4.3 Potential Groundwater Discharge From Tailings and Waste Rock**

Infiltration to groundwater from the tailings and waste rock storage areas is not expected. The meteoric water that may infiltrate is expected to remain in the immediate area for centuries, due to the low permeability of the SFG sediments near the Animas Uplift and due to the presence of flow-inhibiting faults. The impact to groundwater chemistry is expected to be minimal.



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**APPENDICES**

**Appendix A.**  
**Projected Groundwater-Level Hydrographs at Selected Locations**

## APPENDIX A. HYDROGRAPHS

Projected groundwater drawdown 100 years after mining is shown on Figure A1. Water-level change in the bedrock will be about 580 ft near the bottom of the pit.

Projected water-level hydrographs for most well locations shown on Figure A1 are shown on Figures A2 through A24.

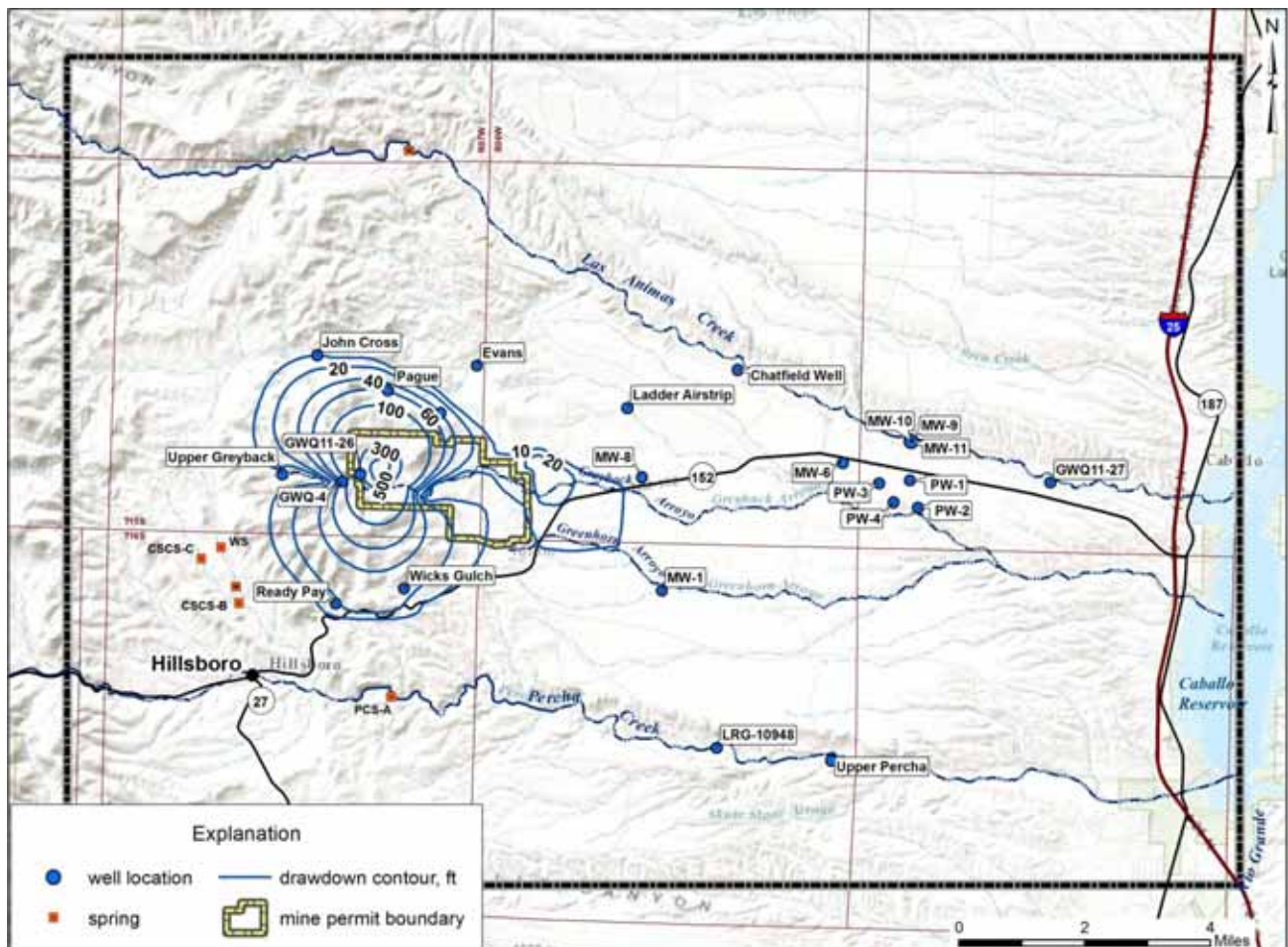


Figure A1. Projected groundwater drawdown 100 years after mining.

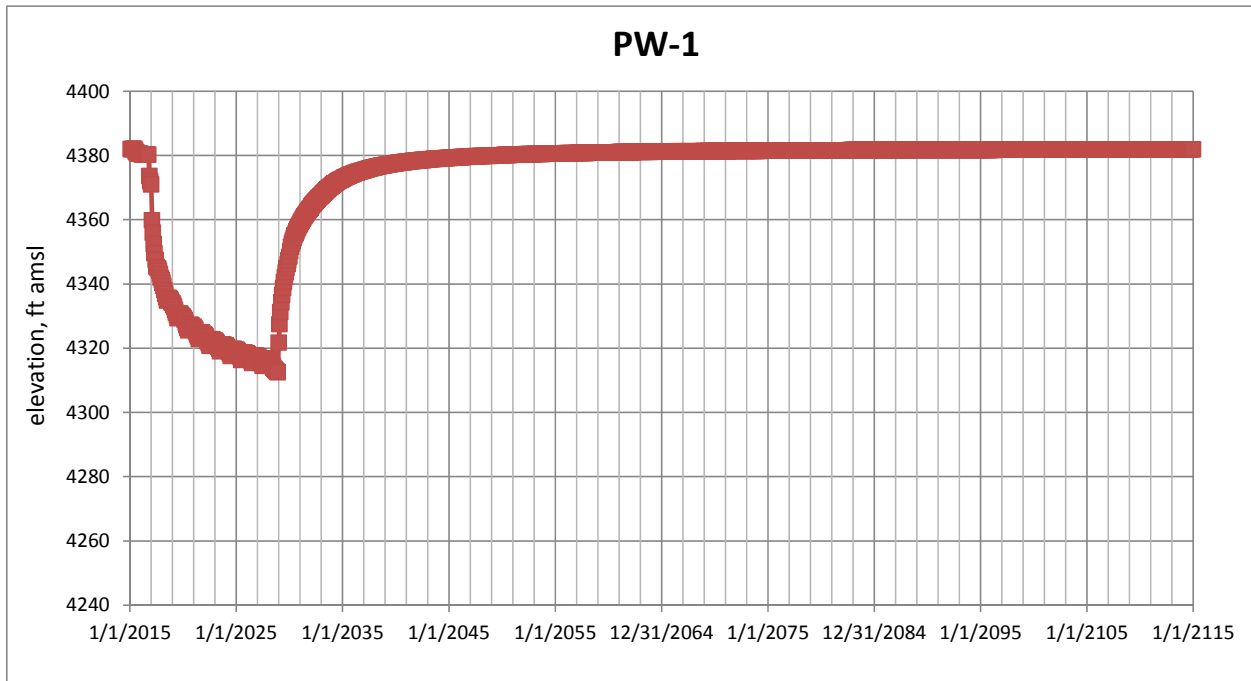


Figure A2. Projected water levels at PW-1.

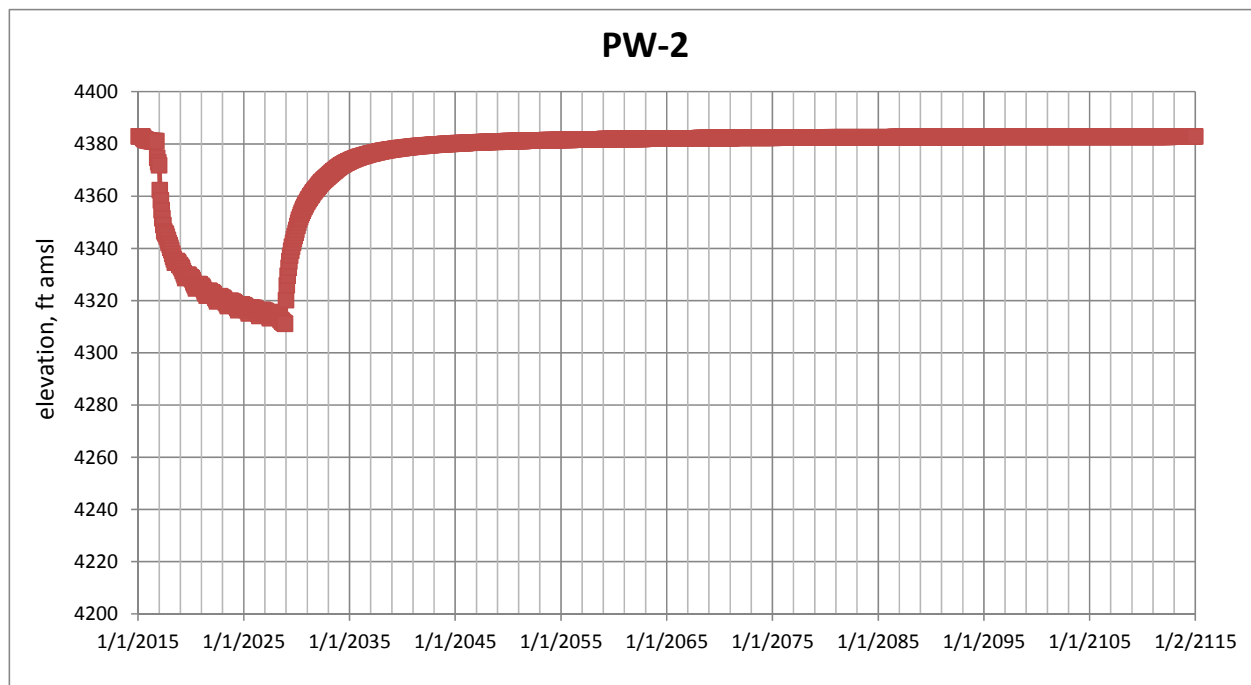


Figure A3. Projected water levels at PW-2.

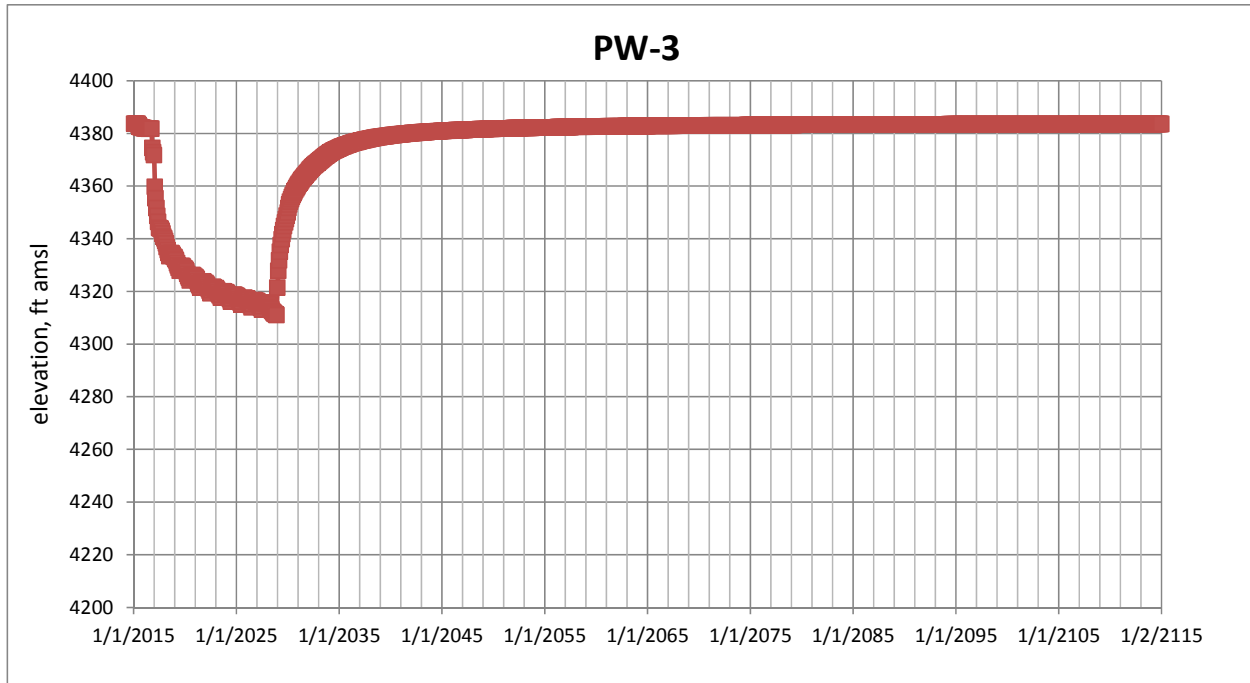


Figure A4. Projected water levels at PW-3.

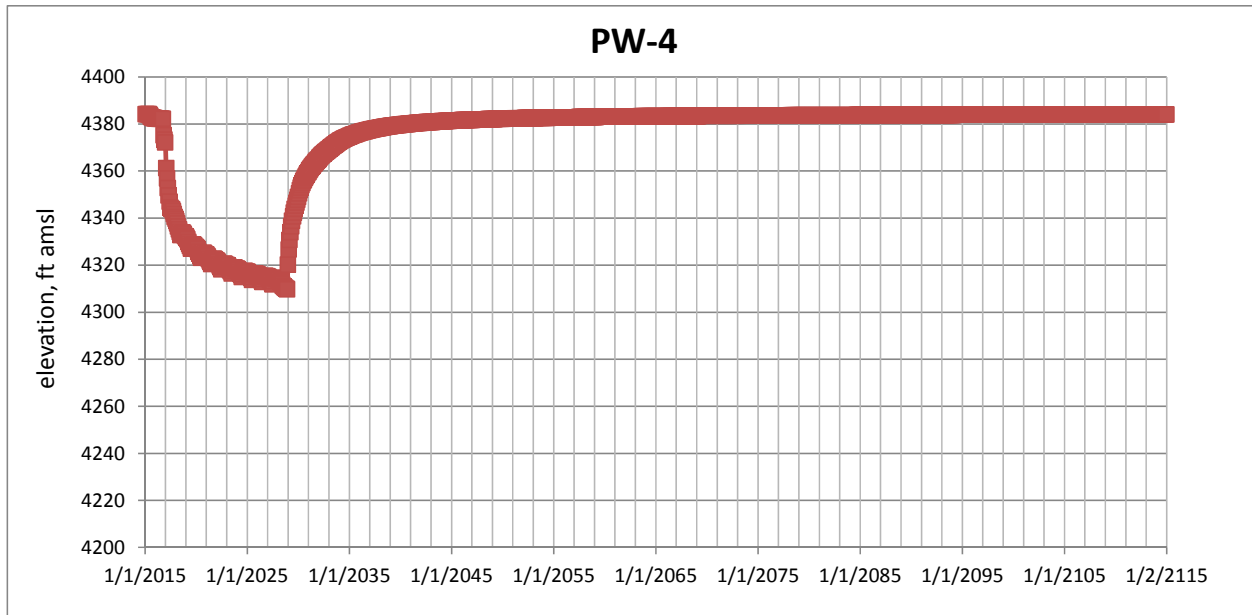


Figure A5. Projected water levels at PW-4.

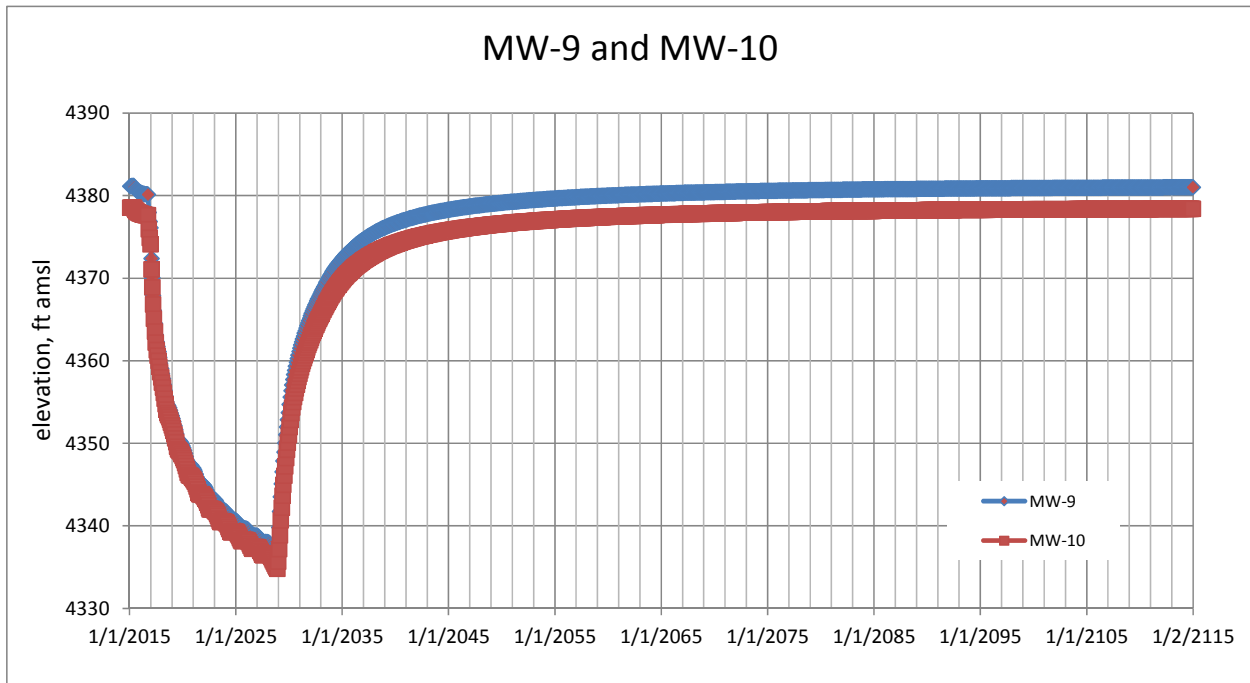


Figure A6. Projected water levels at MW-9 and MW-10.

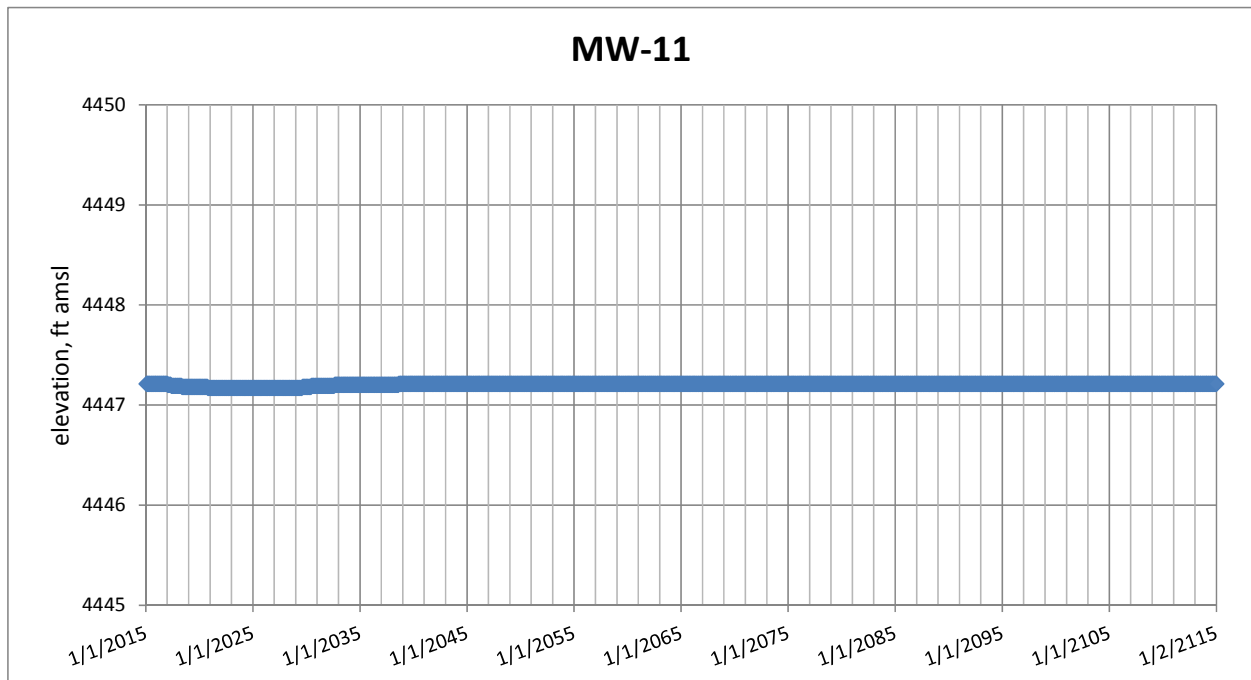


Figure A7. Projected water levels at MW-11.



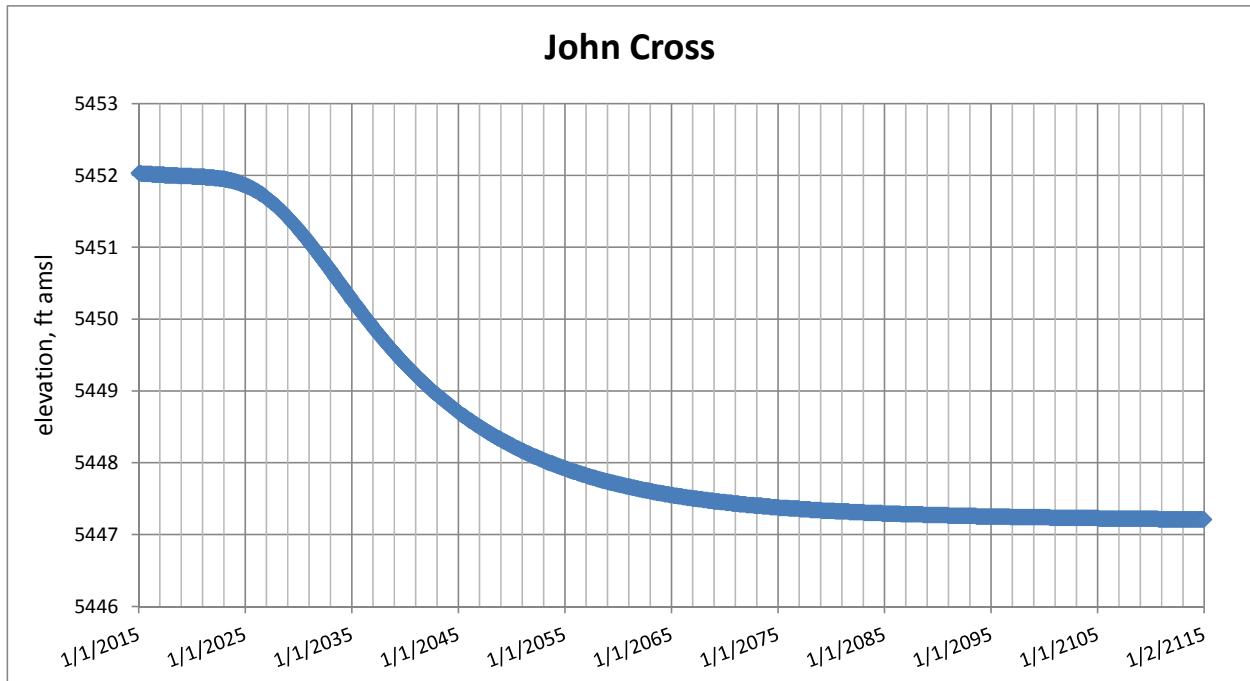


Figure A8. Projected water levels at John Cross.

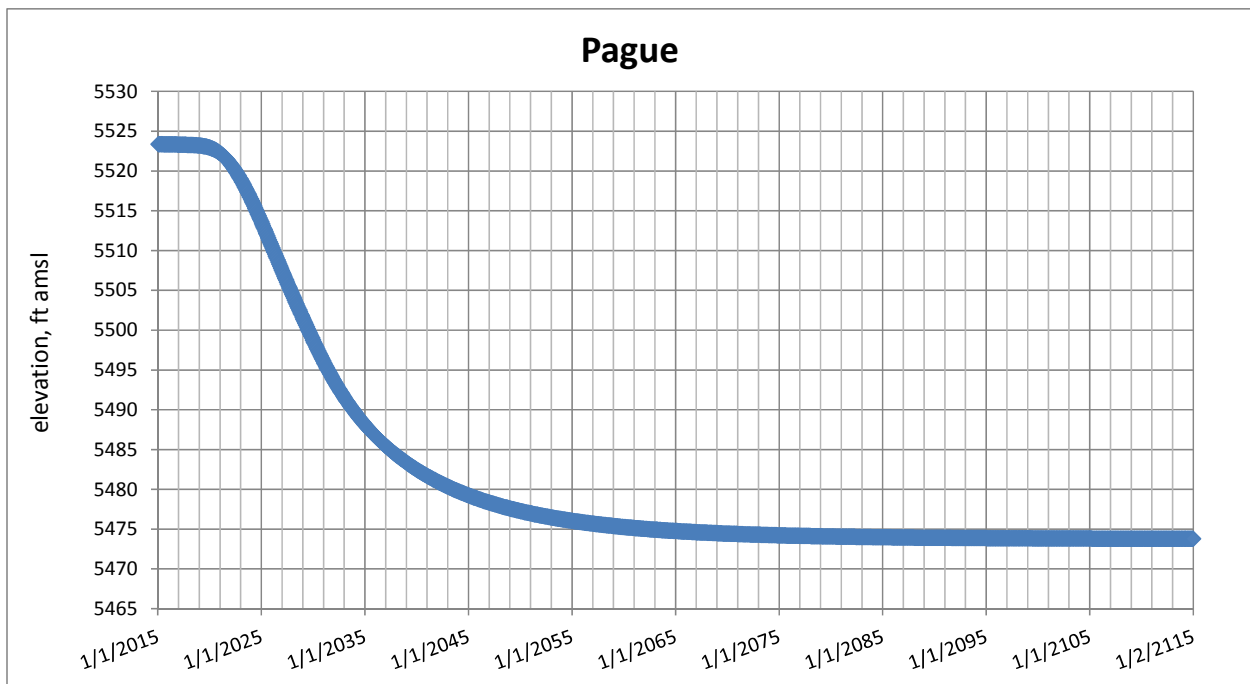


Figure A9. Projected water levels at Pague.

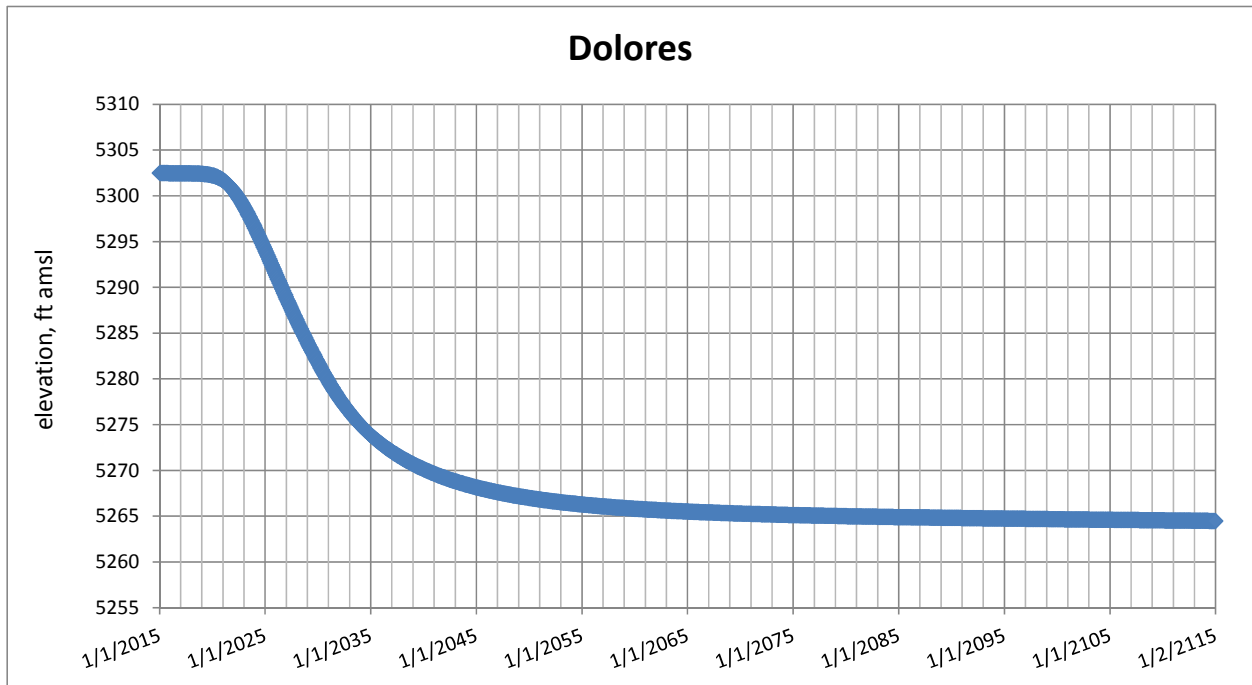


Figure A10. Projected water levels at Dolores.

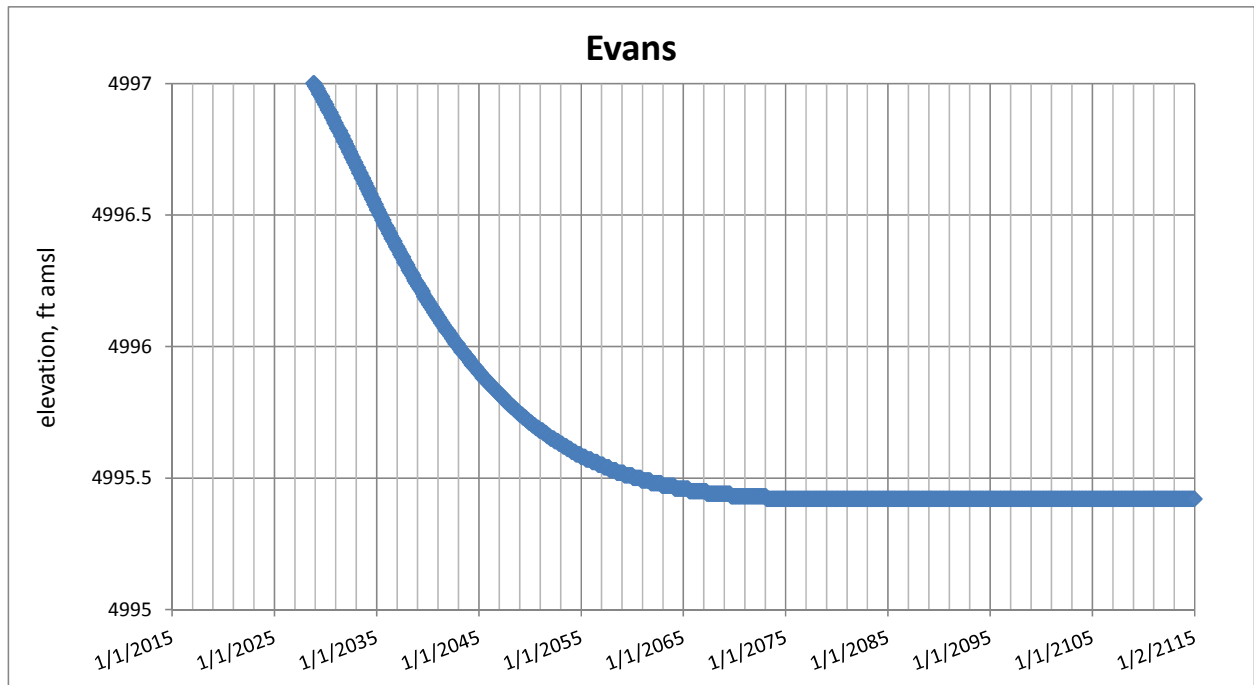


Figure A11. Projected water levels at Evans.

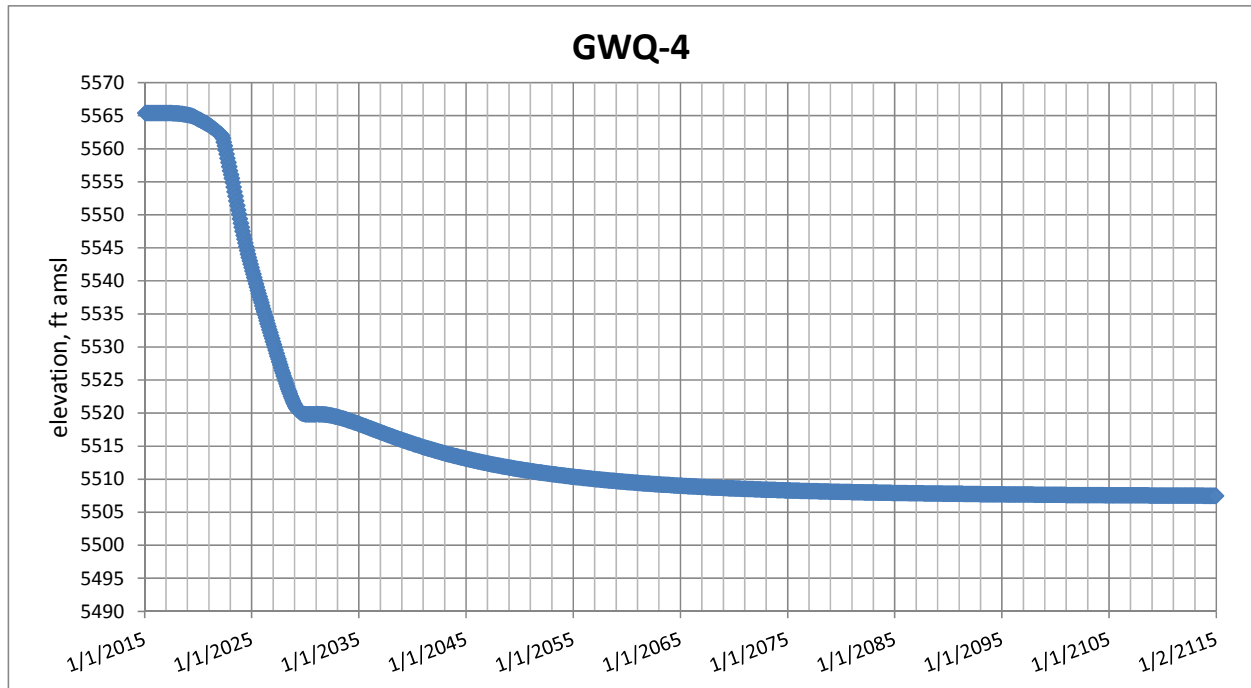


Figure A12. Projected water levels at GWQ-4.

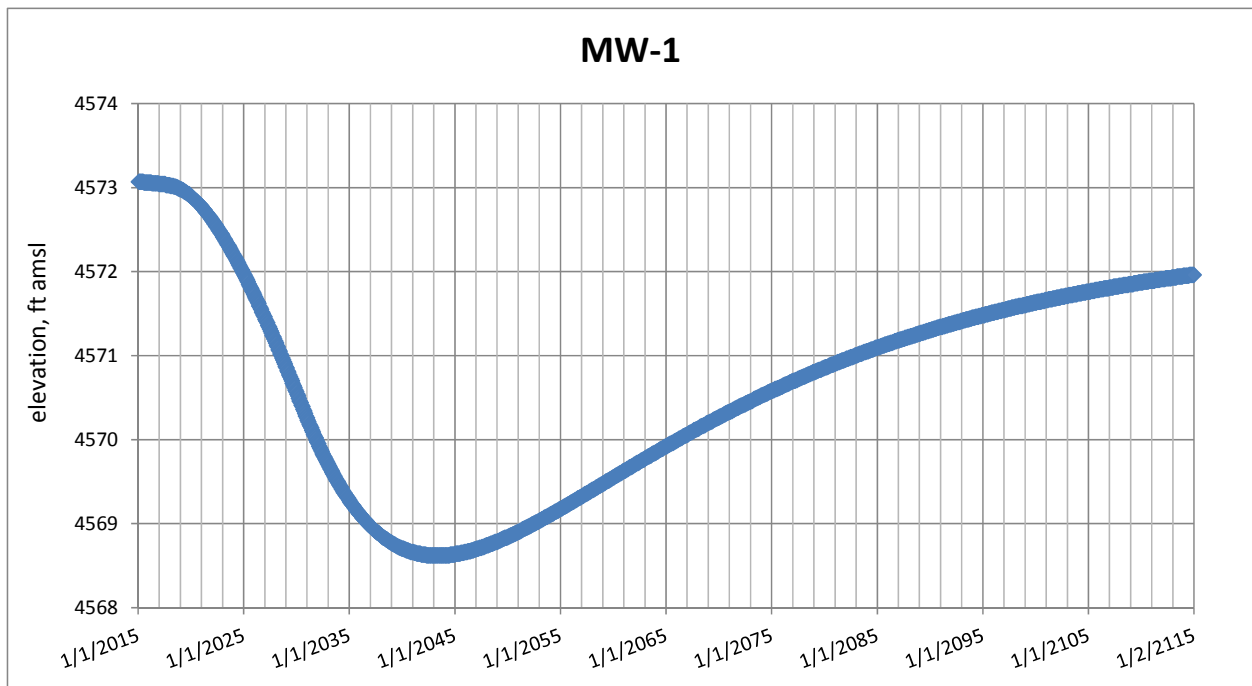


Figure A13. Projected water levels at MW-1.

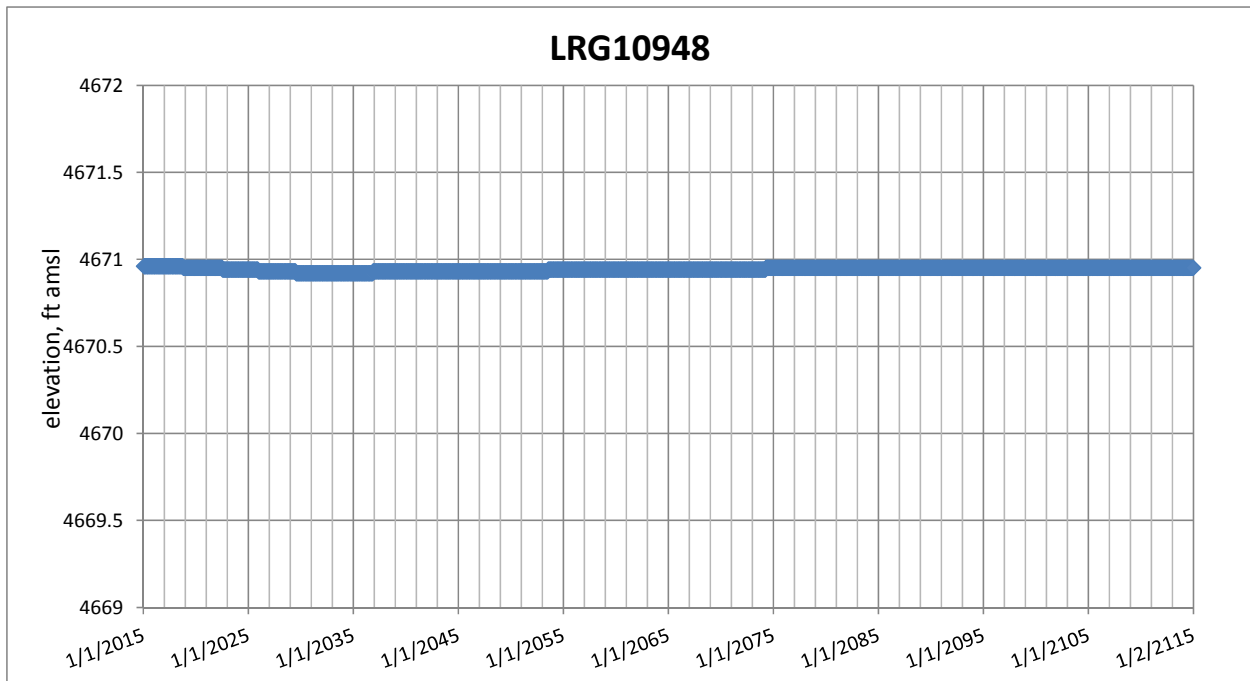


Figure A14. Projected water levels at LRG-10948.

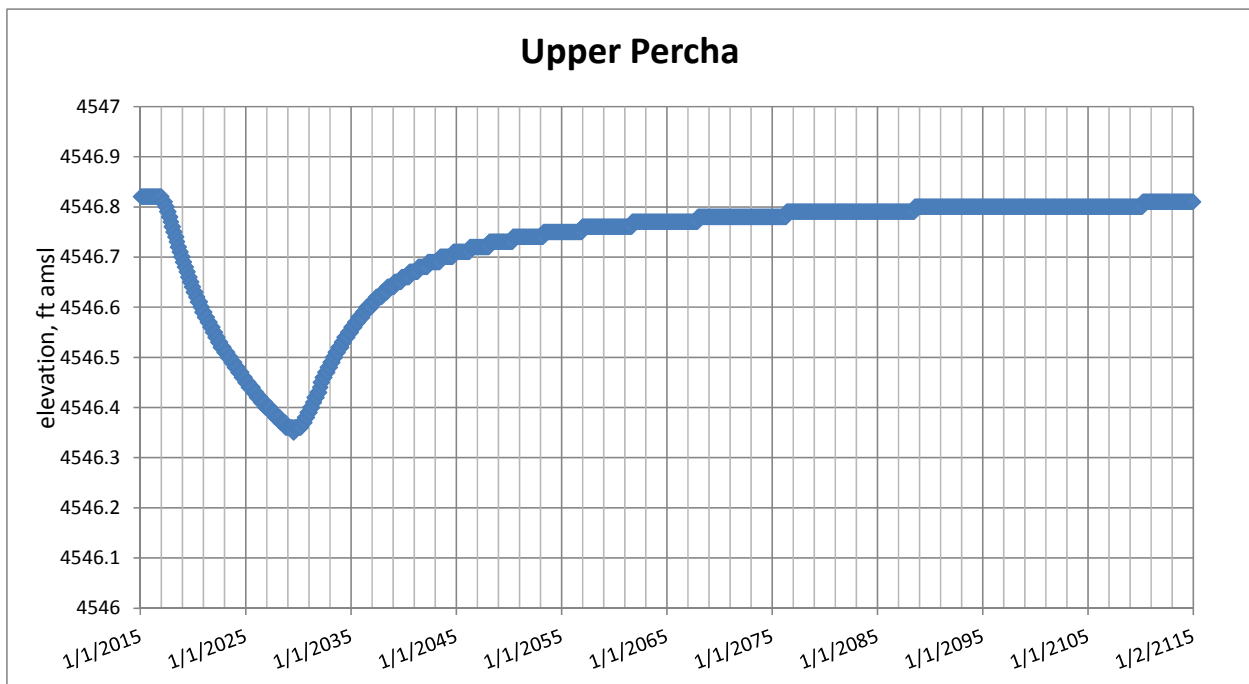


Figure A15. Projected water levels at Upper Percha.

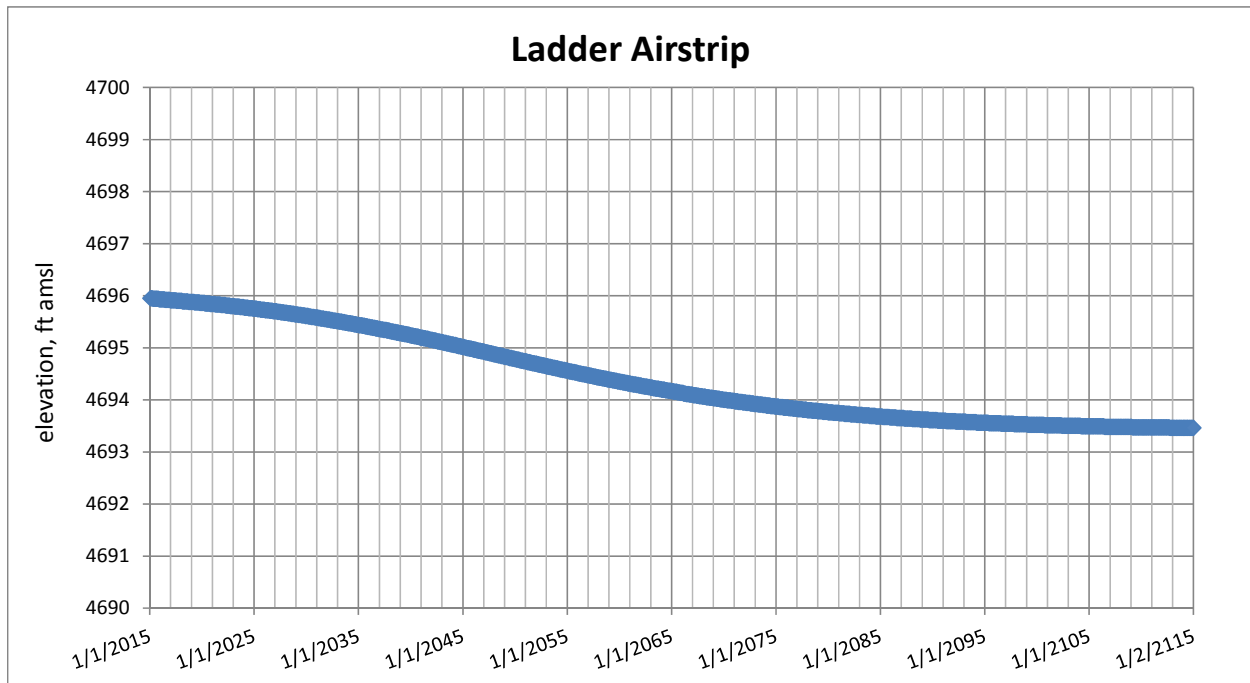


Figure A16. Projected water levels at Ladder Airstrip.

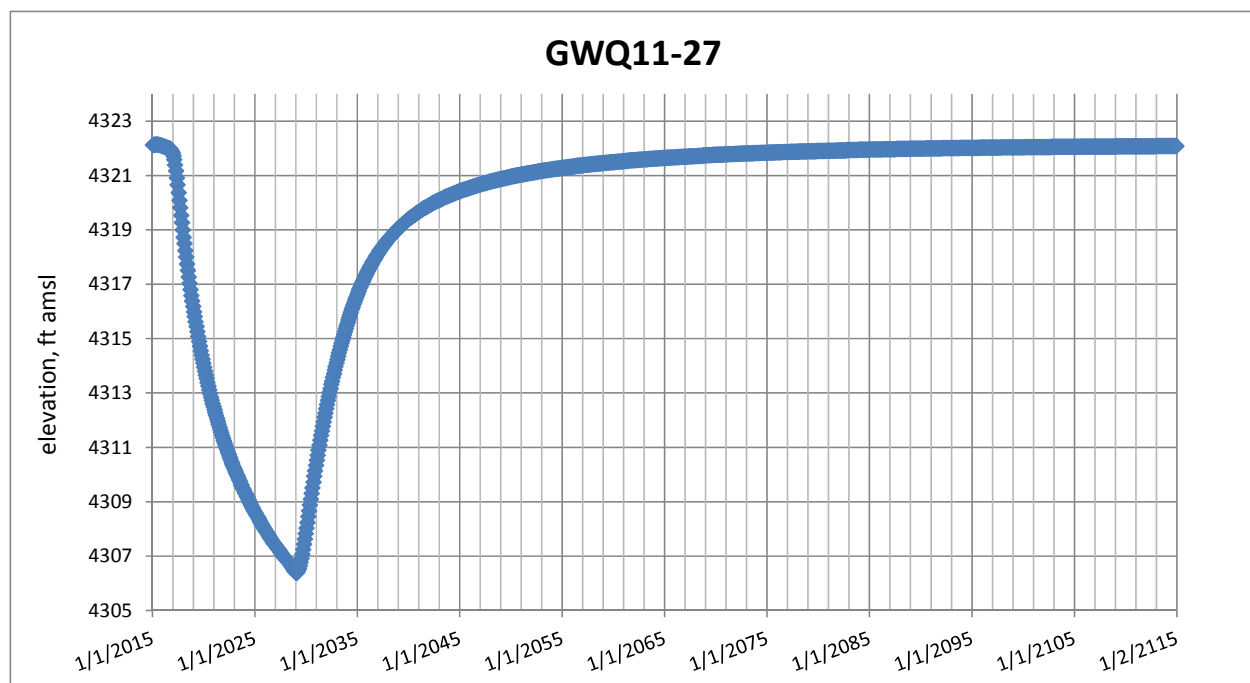


Figure A17. Projected water levels at GWQ11-27.

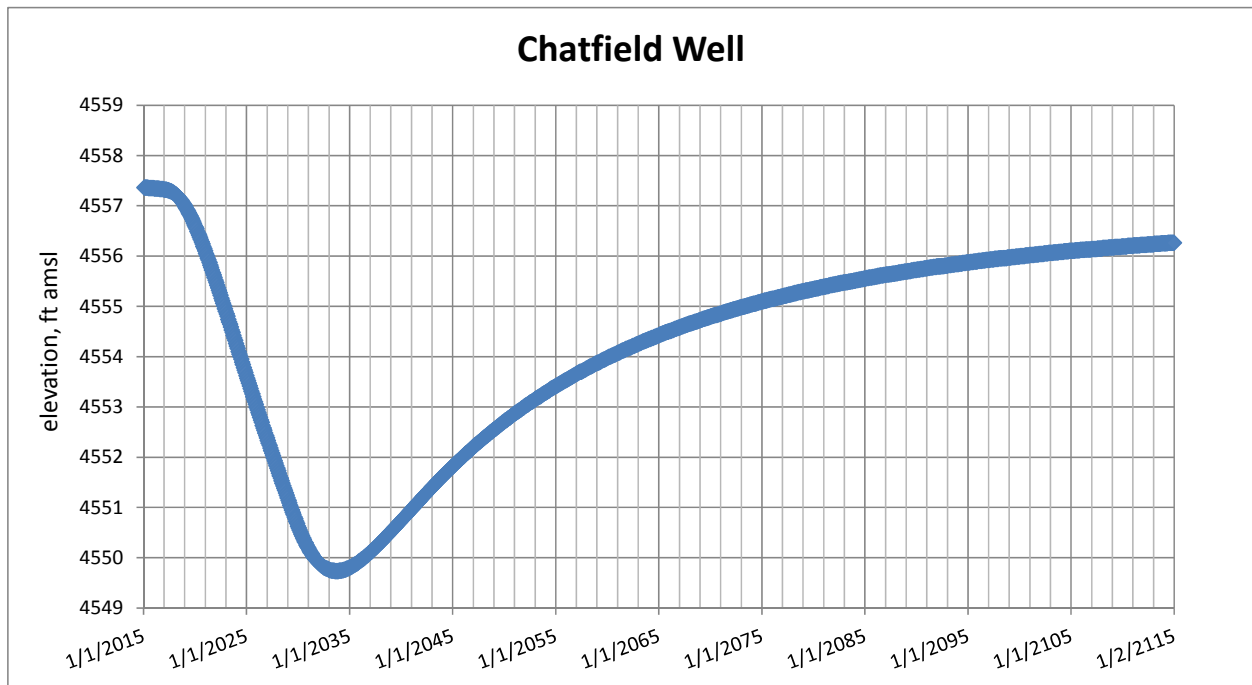


Figure A18. Projected water levels at Chatfield Well.

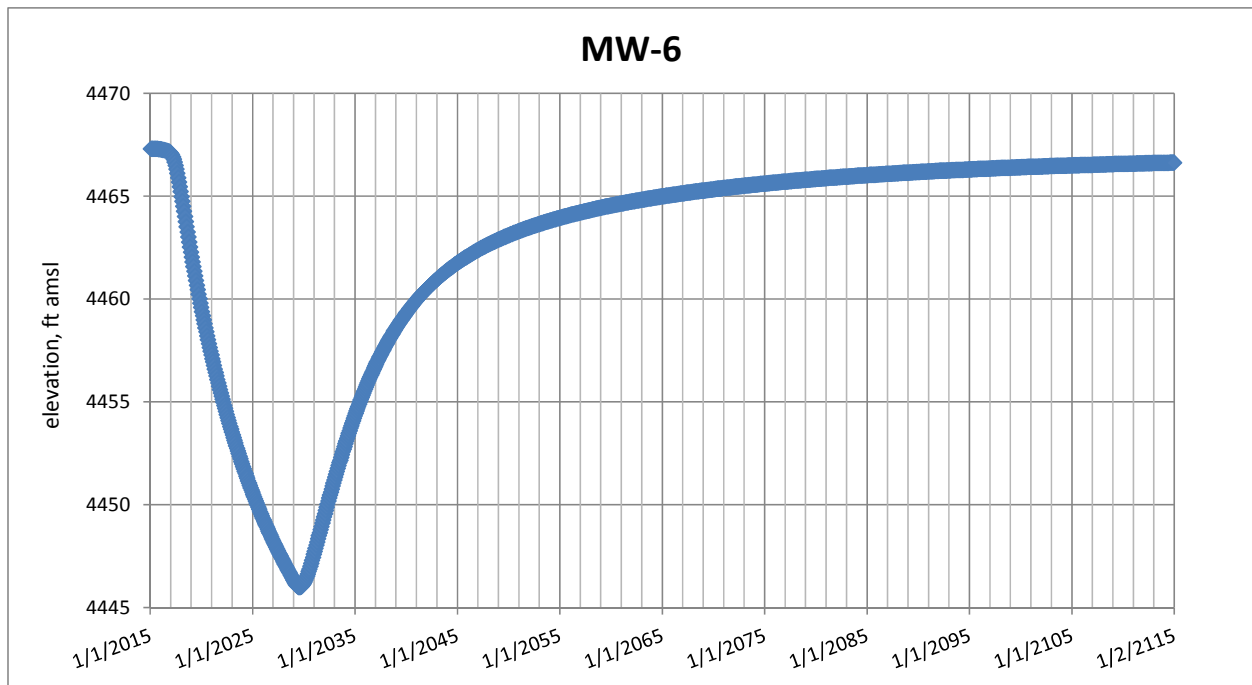


Figure A19. Projected water levels at MW-6.

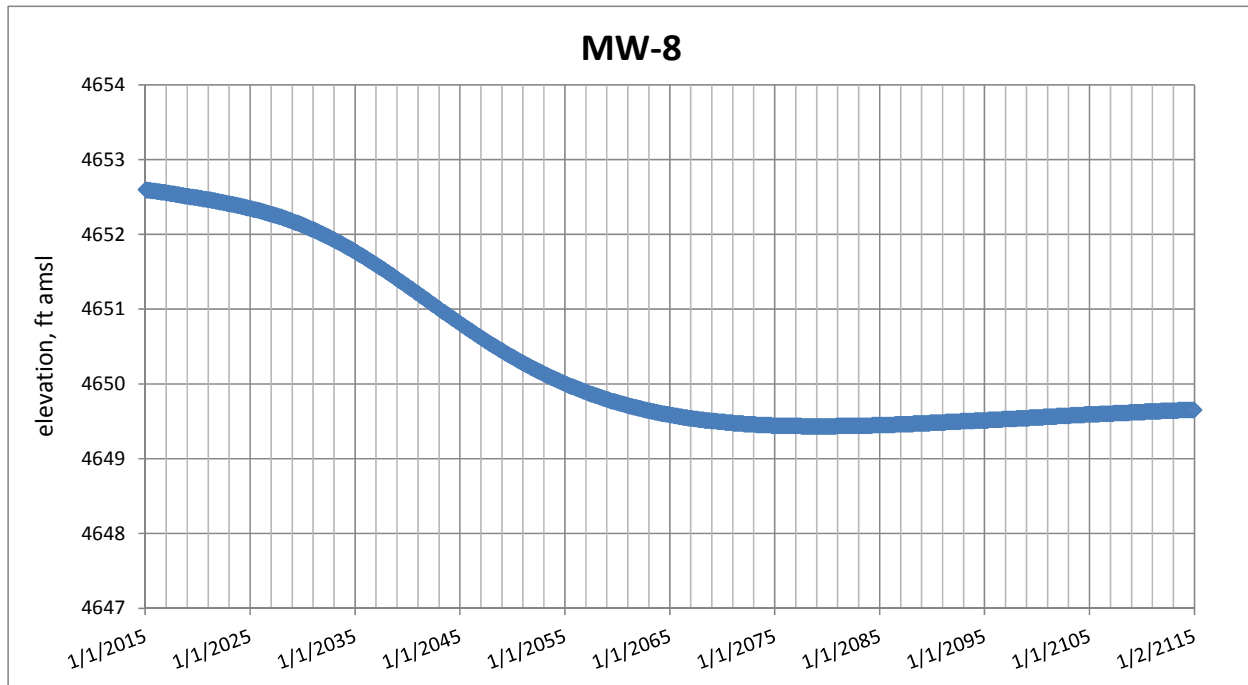


Figure A20. Projected water levels at MW-8.

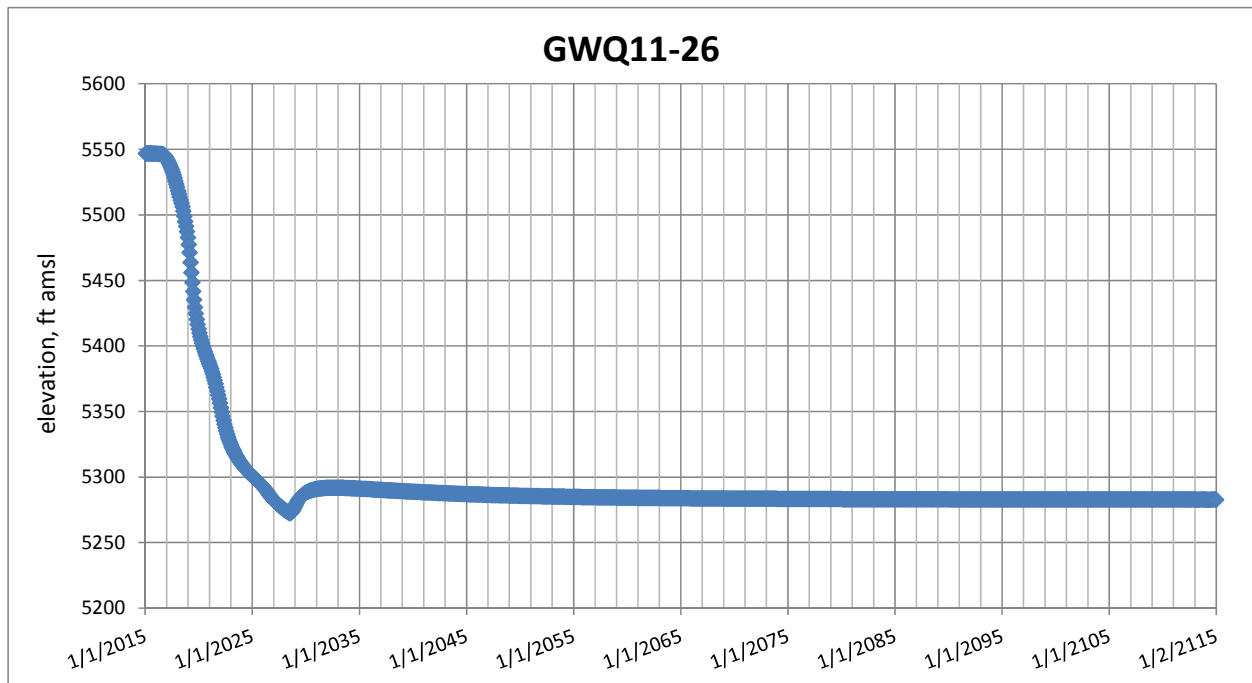


Figure A21. Projected water levels at GWQ11-26.

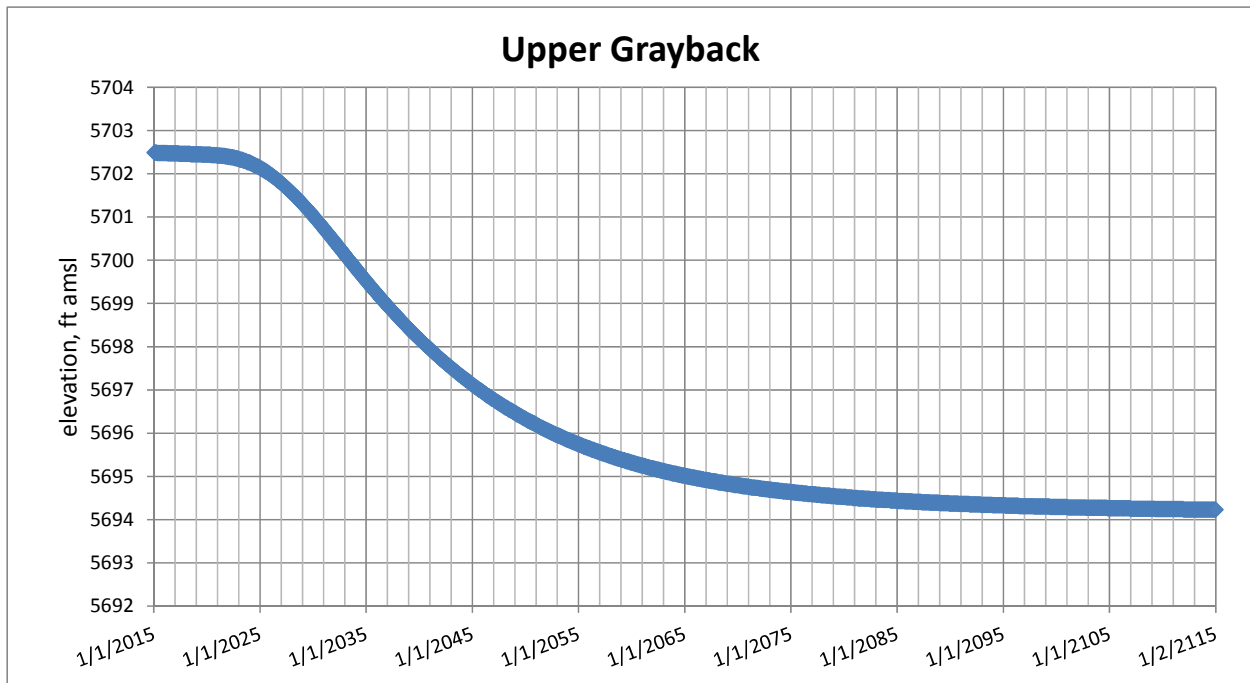


Figure A22. Projected water levels at Upper Grayback.

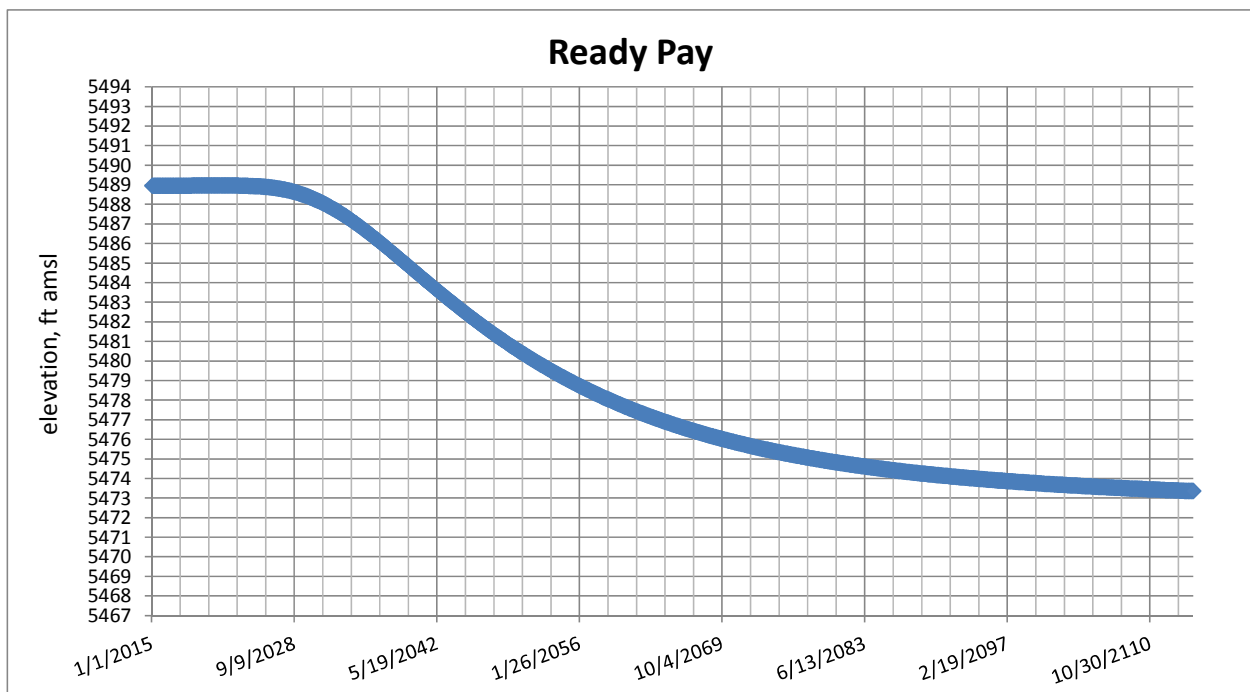


Figure A23. Projected water levels at Ready Pay.



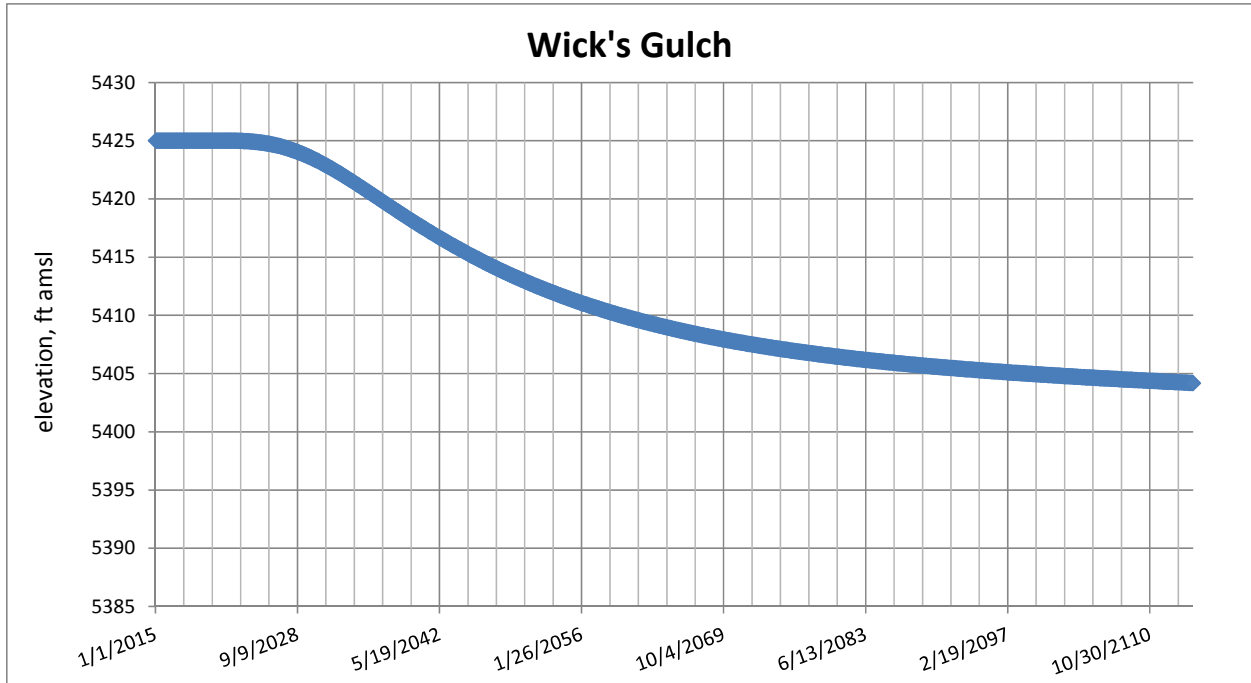
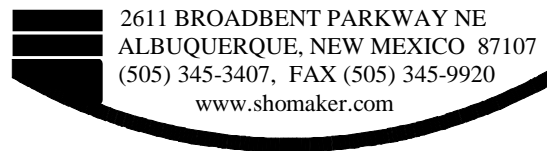


Figure A24. Projected water levels at Wick's Gulch.

**Appendix B.**  
**Technical Memo Regarding Liner Leakage Rates**

**JOHN SHOMAKER & ASSOCIATES, INC.**

WATER-RESOURCE AND ENVIRONMENTAL CONSULTANTS



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## **MEMORANDUM**

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To: JSAI Internal Memo

From: Michael A. Jones, Principal Hydrologist

Date: December 8, 2010

Subject: liner leakage projection

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### **Introduction**

Synthetic liners have been widely used in the modern mining industry to minimize/eliminate mine contact water intrusion to the surrounding surface water and groundwater systems. Even though the liner materials are virtually impermeable, holes and tears regularly occur and synthetic liners leak. In general, the leakage rates depend on many factors including liner quality, installation quality, stress due to weight of the impounded material and traffic, water pressure on the liner, over-liner/under-liner material hydrogeologic and geotechnical properties and conditions, and so on.

Environmental Impact Assessment (EIS) on any new project always requires estimating leakage through the lined mine facilities including leach pads, tailing storage facilities (TSF), contact water ponds, and waste rock dumps. Based on the estimated seepage (source) and hydrologic properties of the underlying aquifers (receiver), evaluation of solute transport downstream can be carried out using numerical or analytical methods. In certain circumstances, the liner leakage must be estimated in order to properly design the seepage collection systems.

Various assumptions and methods have been used by different professionals to estimate liner leakage. Depending on which firm is contracted, different seepage estimates can be obtained for the same facility.

This memorandum intends to provide guidance on how to estimate liner leakage for future projects. Standardizing the approach will make the liner leakage estimates more defensible and irrelevant to the selection of consulting firm.

**Liner Defect Assumptions**

There are few papers on the size and frequency of occurrence of defects in liners (Erickson and Thiel, 2002; Colucci and Lavagnolo,1995; Rowel, 2005). The studies are generally in agreement. In a 3-year field study, Colucci and Lavagnolo (1995) found that the size of liner defects in waste landfills varies substantially with a median hole area of about 1 cm<sup>2</sup> (Table 1).

Holes can be detected by electrical leak survey. Rowel et al. (2005) found that (1) no holes were detected for 30% of electrical leak surveys, and (2) fewer than 5 holes/ha were detected for 50% of the surveys with remaining 20% surveys having more than 5 holes/ha.

Some analyses have assumed a more frequent occurrence of smaller defects. In an EPA funded study, defect hole diameters were assumed to be 0.3 and 1 cm, but the corresponding numbers of holes were assumed to be 9 and 3.6 hole/ha, respectively (Barlaz et al., 2002).

**Table 1. Reported size of holes in geomembranes  
(after Colucci & Lavagnolo, 1995)**

Leak area (mm <sup>2</sup> )	Equivalent radius for circular hole, $r_o$ (mm)	Percentage (%)	Cumulative percentage (%)
0-20	0-2.5	23.2	23.2
20-100	2.5-5.64	26.3	49.5
100-500	5.64-12.6	28.2	77.7
500-1000	12.6-17.8	8.8	86.5
10 <sup>3</sup> -10 <sup>4</sup>	17.8-56.4	7.8	94.3
10 <sup>4</sup> -10 <sup>5</sup>	56.4-178	4.5	98.2
10 <sup>5</sup> -10 <sup>6</sup>	178-517	1.2	100

For estimating liner leakage, we recommend using the following assumptions for the occurrence and size of liner defects:

- 1 circular defect per acre (or 2.5 defects per hectare)
- Area of defect = 1 cm<sup>2</sup> (equivalent hole diameter of about 1.13 cm)

These recommendations are in agreement with Giroud and Bonaparte (1989) for calculations to size the components of the lining system, and have been used by some consulting firms.

**Liner Leakage Equation 1 (for non TSF Facility)**

We recommend an equation (Giruoud et al., 1997) to estimate liner leakage for non TSF facilities. The equation represents an impeded flow condition with a geomembrane underlain by a low permeable medium such as a (compacted) soil foundation.

The Giruoud et al. (1997) Equation is listed below:

$$q = \beta_c \left[ 1 + 0.1 \left( \frac{h_w}{L_s} \right)^{0.95} \right] a_d^{0.1} h_w^{0.9} K_s^{0.74}$$

$q$  = leakage through a circular defect in composite liner (m<sup>3</sup>/sec)  
 $\beta_c$  = coefficient relating to liner contact (0.21 for good and 1.15 for poor)  
 $h_w$  = depth of water above the geomembrane (m)  
 $L_s$  = thickness of soil liner (m)  
 $a_d$  = area of defect (m<sup>2</sup>)  
 $K_s$  = saturated hydraulic conductivity of soil liner (m/s)

It should be noted that, in the above equation, the leakage rate has a non-linear relationship with the area of the defect. Therefore, the leakage through a single hole should be calculated first; then total leakage through the facility should be calculated based on the total number of defect holes within the facility footprint.

**Liner Leakage Equation 2 (for TSF Facility)**

The Giruoud et al. (1997) Equation is only suitable for lined leach pads, waste dumps and landfills where leakage is only impeded by defect size and conductance of the underlying soil liner. In a TSF, however, seepage through a liner defect will be most likely restricted by the permeability of tailings around the hole. In other words, hydraulic properties of both the over-liner tailings and the under-liner soil restrict the flow of water through the defect.

Coffey (Appendix A) has proposed an analytical solution to calculate liner leakage through a defect confined by both aquifers:

$$Q = (h_T - h_A) \pi D_H / (1/k_T + 1/k_A) \tag{1}$$

Where, Q is leakage rate through a defect; hT and hA are, respectively, total head in the tailings and in the underlying soil; kT and kA are, respectively, hydraulic conductivity of the tailings and underneath soil; and DH is the diameter of the defect.

If the underlying soil is not pressurized, i.e., in an unsaturated condition, the above equation can be simplified to:

$$Q = h_T \pi D_H k_T \tag{2}$$

Derivation of equations is provided in Appendix A. We have reviewed and verified the Coffey work and found it is correct mathematically.

The analytical solution proposed by Coffey was also validated by John Shomaker & Associates, Inc. (JSAI) using a spreadsheet-based numerical model and U.S. Geological Survey (USGS) finite difference code MODFLOW. Results obtained for an example problem, using both analytical and numerical solutions, are compared in Table 2. Apparently, they are in close agreement.

**Table 2. Calculated seepage through a defect - numerical and analytical solutions**

	<b>Case</b>
D <sub>H</sub> (cm)	1.128
A (cm <sup>2</sup> )	1.000
h <sub>T</sub> (m)	30
K <sub>T</sub> (cm/s)	1.00E-06
Coffey - Eq2 Q (cm <sup>3</sup> /s)	0.011
JSAI - Spreadsheet Q (cm <sup>3</sup> /s)	0.011
JSAI - MODFLOW Q (cm <sup>3</sup> /s)	0.012

**Discussion**

Rowe (2005) reports landfill liner seepage as detected by liner detection systems (LDS) for various liner configurations (Table 3). It was found that (1) average leakage rates through single geomembrane liners were between 130-190 liters per ha per day (lphd), and (2) average leakage rates through geomembrane plus compacted clay liners were between 50- 90 lphd.

The following assumptions were used in an example calculation:

$$\beta_c = 0.21, h_w = 60 \text{ cm}, L_s = 30 \text{ cm}, a_d = 0.0001 \text{ m}^2, K_s = 1.00\text{E-}7 \text{ m/s}, \text{ and defect frequency (n) is 1 hole/acre,}$$

Estimated liner leakage from the Giruoud et al. (1997) Equation is:

$$Q = n \times q = 36 \text{ liters/acre/day} = 89 \text{ liters/ha/day (lphd)}$$

The calculated result is in close agreement with the Rowe (2005) field measurements. Therefore, we suggest a general rule that leakage of a lined leach pad (or waste dump) is likely about 100 lphd.

**Table 3. Field-measured liner seepage (after Rowe, 2005)**

Liner/stage	No. of cells	Average monthly flows: lphd			Peak monthly flows: lphd		
		Mean*	SD†	Max‡	Mean*	SD†	Max‡
Single liner: GM alone							
Active	25	190	330	1600§ 790¶	360	610	3070§ 1830¶
Post-closure	6	130	120	330	330	30	1130
Composite GM/GCL liner							
Active	22	1.5	2.7	11¶	9	16	54¶
Post-closure	5	0.6	0.9	2	4	5	10
Composite GM/CCL or GM/GCL/CCL liner							
Active	11	90	90	370§ 260¶	250	370	1990§ 1240¶
Post-closure	3	50	50	220	60	90	250

\*Mean and †standard deviation of reported average and peak average monthly flows: these were obtained for different cells over different periods, and include data obtained for systems with sand, gravel and GN LDS.

‡Maximum value reported.

§Largest value reported, but it is for sand LDS and so may reflect stored water in the LDS shortly after construction.

¶Largest value for liner system with GN LDS.

**References**

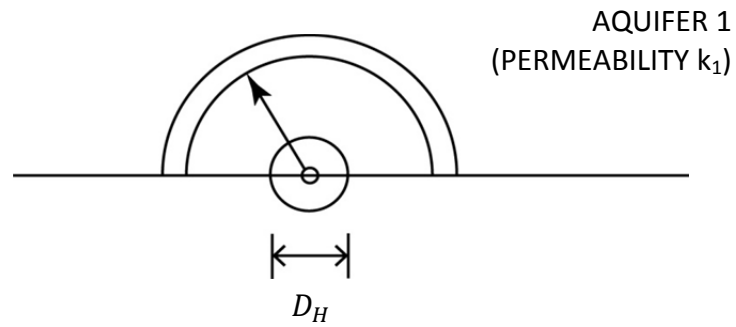
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**Appendix A**

**Seepage Loss through a Circular Hole in Geomembrance  
(Coffey, 2010)**

CONSIDER SEEPAGE LOSS THROUGH A CIRCULAR HOLE OF DIAMETER  $D_H$   
IN A MEMBRANE SEPARATING TWO MATERIALS OF DIFFERENT PERMEABILITY.



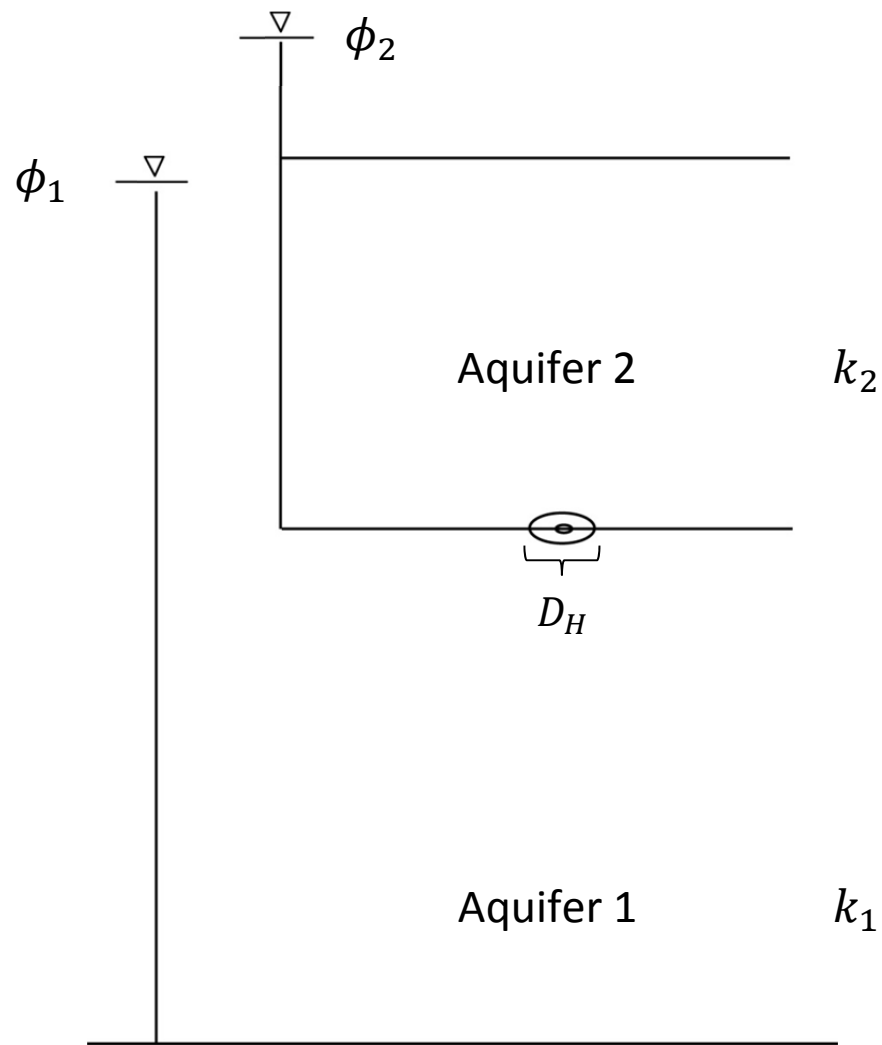
*area of half sphere*

$$Q = k \overbrace{2\pi r^2} \frac{d\phi}{dr}$$

Under steady state,  $Q$  is uniform over  $r$

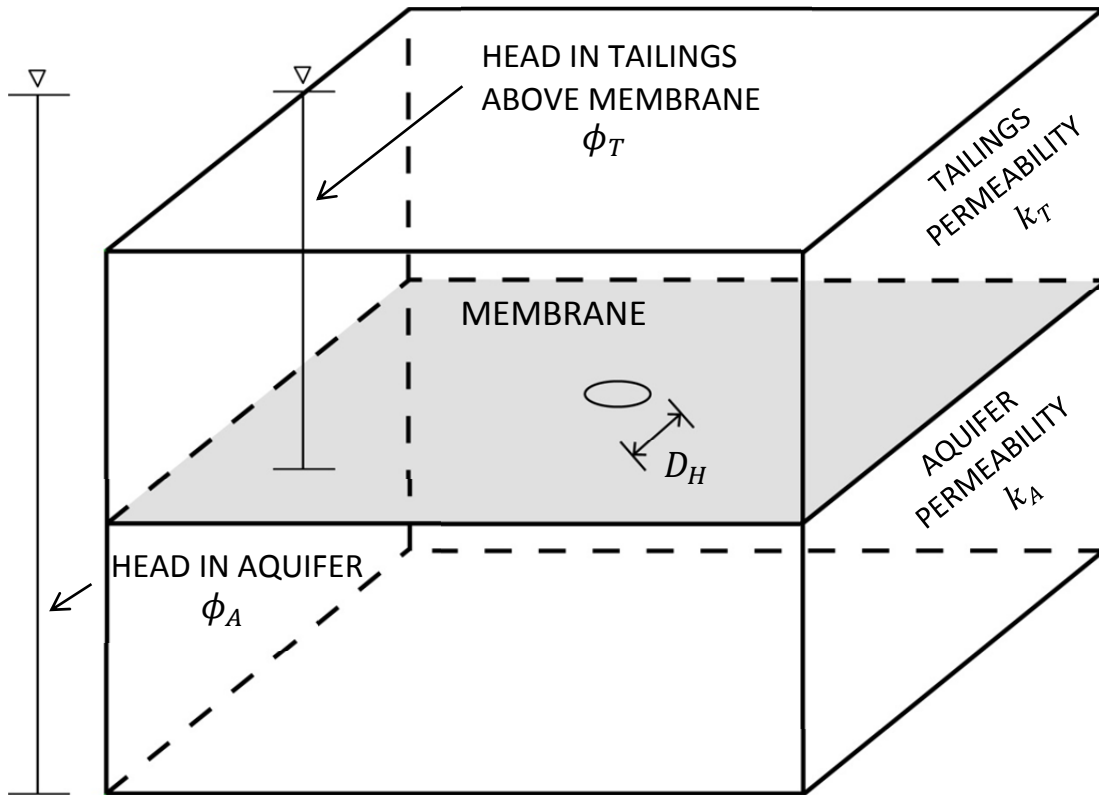
$$\phi = \int_{r_a}^{\infty} \frac{Q}{2\pi k r^2} dr = - \left[ \frac{Q}{2\pi k r} \right]_{r_a}^{\infty} = \frac{Q}{2\pi k r_a}$$

Head loss to hole:  $\Delta\phi = \frac{Q}{\pi k D_H}$  (noting  $2r_H = D_H$ )



$$\phi_2 - \phi_1 = \frac{Q}{\pi D_H k_2} + \frac{Q}{\pi D_H k_1} = \frac{Q}{\pi D_H} \left( \frac{1}{k_2} + \frac{1}{k_1} \right)$$

$$Q = \frac{\pi D_H (\phi_2 - \phi_1)}{\left( \frac{1}{k_2} + \frac{1}{k_1} \right)}$$



LEAKAGE THROUGH HOLE OF DIAMETER  $D_H$ :

$$Q = \frac{\pi D_H (\phi_T - \phi_A)}{\left(\frac{1}{k_A} + \frac{1}{k_T}\right)}$$

$Q = \pi \phi_T k_T D_H$  for fully drained layer below